DRAINAGE SLUICEWAY AT WF. 172R+15m

TYPE OF WORK

: BASE CONCRETE LOCATION : REVETMENT

CALCULAT	ION 4. 44.4		RESULT
· CONCRETE (TYPE-CI)			
<u> </u>			
V= (12.00 + 1.85 × 2) × 220	/10.00m =	3,454 3,	454 m ³
- GRAVEL BEDDING			
V = (12.00 + 1.85x2) x 0.70/16	2.00m = /	099 1	099 m3
· FORM (H<4.0m)			
A = (12.00 + 1.85×2) × 10.83/1	0,00m = 17	, 003 17.	003 m²
· REINFORCING BAR			
$W = (12.00 + 1.85 \times 2) \times 0.10/10$.00m = 0.	157 0.1	157 tf
JOINT FILLER (t=10. FLASTIC M	ATERIAL)		
A = 0.220 x 2 places	= 0.4	70 0.	440 m²
			Standard Company
	Taranga Parabahan at Anggar	The second of the second of the second	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
		I	1. The second of the

TYPE OF WORK:

LOCATION

REVETMENT DRAINAGE SLUICEWAY AT WF.172R + 15 m

CALCULA'	TION	RESULT
GABION CYLINDER		
Ø 500		
$V_1 = \pi/4 \times 0.50^2 \times (3.00 + 7.826 + 1.00)$	$\frac{30) \times 6 \times 2}{= 27.864}$	<u> </u>
	= 27.804	
$V_2 = \pi /4 \times 0.50^2 \times (3.00 + 4.778 + 1.00)$	20) × 10 × 2	<u> </u>
V ₂ - 1/4 x 0.30 x (3.00 + 4.778 + 1.0	= 34.471	
TOTAL	= 62.335	62.335 m ³
SOIL FILLING		
$V_1 = (3.00 + 7.826 + 1.00) \times 0.50 \times 3.0$	00 x 2 - 27.864	
	= 7.614	
$V_2 = (3.00 + 4.778 + 1.00) \times 0.50 \times 5.0$	00 v 2 - 34 471	
Y ₂ = (3.00 4.776 1.00) x 0.30 x 3.0	= 9.419	
TOTAL	= 17.033	17.033
		No. 10 Control of the
GABION MATTRESS		
- GADAON MATIKESS		
$A = 0.50 \times 3.00 + 0.50 \times 1.50$	= 2.250 m ²	1000
*	= 27.000	
 In the part of the contract of th		
		Marie Carlotte Carlot

TYPE OF WORK : REVETMENT

LOCATION : DRAINAGE SLUICEWAY AT WF.172R + 15 m

	ALCULATION	RESULT
DUDDLE COONE ELL LING		· · · · · · · · · · · · · · · · · · ·
RUBBLE STONE FILLING		
$A = \frac{1}{2} \times 1.00 \times 0.50 + (0.3)$	0 + 1.30) x ½ x $0.50 = 0.650$ 1	n ²
	0.030 I	
$V = 0.65 \times 12.00$	= 7.800	7.800 m ³
	<u> 1985 - All Marie Branch Bran</u>	and the second
CEMENT MORTAR POIN	PTINC	
CEMENT MORTARTOIN		Antaga yang beranggan beranggan beranggan beranggan beranggan beranggan beranggan beranggan beranggan berangga Beranggan beranggan
$A_1 = (7.826 + 0.70) \times 3.70 \times$	= 63.092	
$A_2 = (7.826 + 5.143) \times \frac{1}{2} \times$	$1.20 \times 2 = 15.563$	
$A_3 = 5.143 \times 1.60$	8.229	
$A_4 = \{(0.525 + 4.778 + 0.70)$	$x 1.70 + (3.967 + 0.70) \times 0.65$ x 2	
	= 26.477	<u>, januar jarawan nganggan katangan lua</u>
	20.477	
$A_5 = (3.155 + 0.70) \times 2.35 \times$	2 = 18.119	
$(oldsymbol{x}, oldsymbol{x}, oldsymbol{x}, oldsymbol{x})$. The first section $oldsymbol{ au}$	OTAL = 131.480	131.480 m ²
	To carrie as fair to the action of the carrier of the	
	<u>n om svienie in einer 1845 in 1866.</u> Pre Filippe Politike in 1861 in 1867.	

TYPE OF WORK:

LOCATION

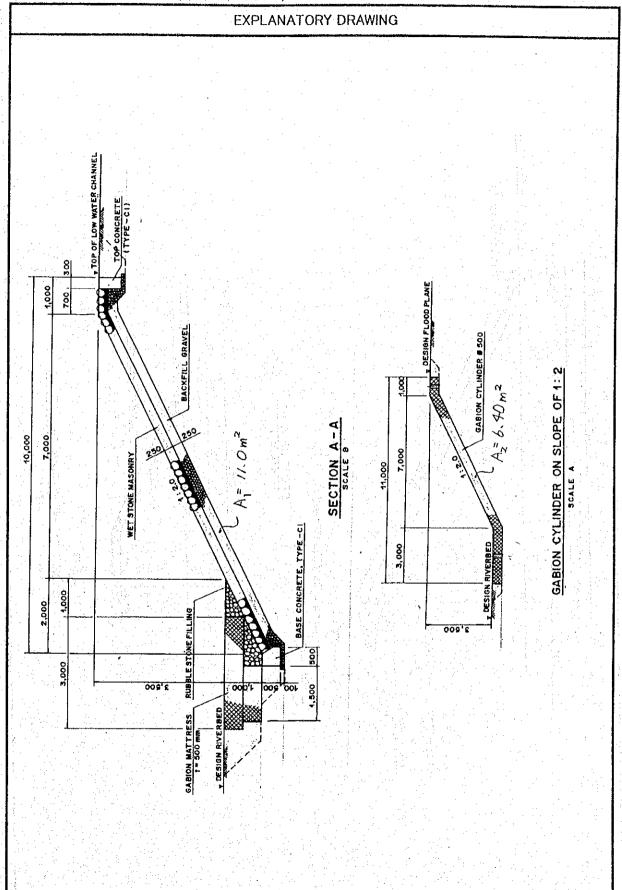
K : REVETMENT : DRAINAGE SLUICEWAY AT WF.172R + 15 m

CALCULATION	RESULT
F SOLID SODDING	多语 对邻语 流流
$A = (1.563 + 3.155) \times \frac{1}{2} \times 4.70 \times 2 = 22.175$	22.175 m ²
	22,175 111
erte det megak kun lung di mengengangan pengangan berangan belangan pengangan menggungan mengan belang di peng Pengangan pengangan	
□ STRUCTURAL EXCAVATION	
$A_1 = 11.0 \text{ m}^2$	
V 110 120	
$V_1 = 11.0 \times 12.0 = 132.0$	
$A_2 = 6.40 \text{ m}^2$	
$V_2 = 6.40 \times (3.00 + 0.50) \times 2 = 44.8$	
$TOTAL(V_1 + V_2) = 176.8$	176.0
$(v_1 + v_2) = (v_1 + v_2)$	176.8 m³
BACKFILL WITH SELECTED SOIL	
마음이 있는 사람들이 되는 것을 하는 것이 되었다. 그런 사람들이 되는 것이 되었다는 것이 되었다는 것이 되었다. 그렇게 되었다는 것이 되었다면 하는 것이 되었다는 것이 되었다는 것이 되었다.	
$A_1 = 0.9 \text{ m}^2$ $A_2 = 0.5 \text{ m}^2$ $A_3 = 0.60 \text{ m}^2$	
$V_1 = (0.9 + 0.5 + 0.6) \times 12.0 = 24.0$	
$A_4 = 0.4 \text{ m}^2$	
$V_2 = (0.4 \times 3.00 \times 2) + (3.00 + 7.826 + 1.00) \times 0.4 \times 2$	
= 11.9	
$TOTAL (V_1 + V_2) = 35.9$	250>
$TOTAL (V_1 + V_2) = 35.9$	35.9 m³

ORAINAGE SLUICEWAY AT WF.172R + 15 m STRUCTURAL EXCAVATION

TYPE OF WORK

LOCATION REVETMENT



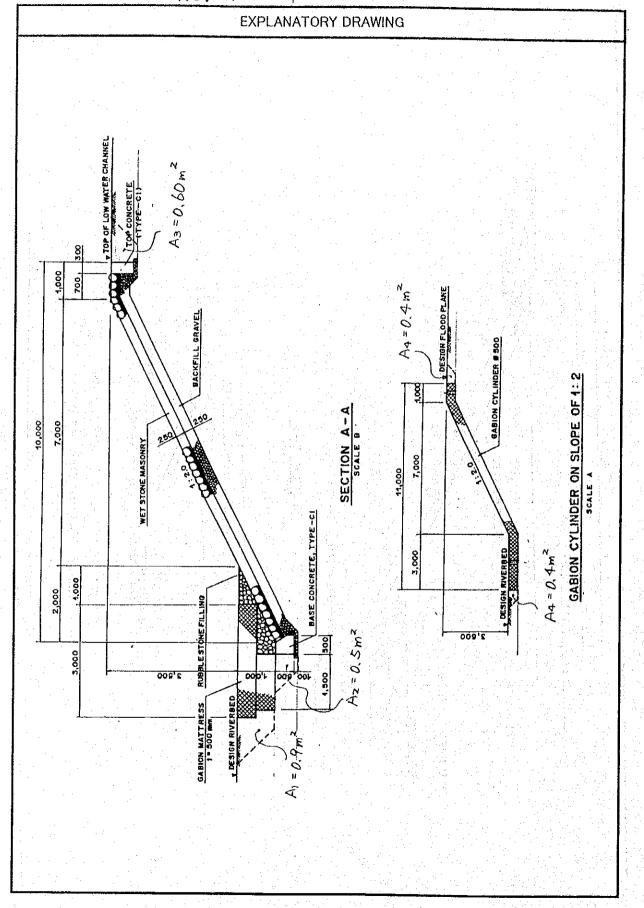
DRAINAGE SLUICEWAY AT WF. 172R + 15M

TYPE OF WORK

BACKFILL WITH SELECTED SOIL

LOCATION

: REVETMENT



DRAINAGE SLUICEWAY AT WF. 172R+15m

TYPE OF WORK

PARTITION WALL

LOCATION : PARTITION WALL LOCATION : REVETMENT	(1/2)
CALCULATION	RESULT
· CONCRETE (TYPE-CI)	
$A_1 = (3.60b + 3.065) \times 1/2 \times 0.866 + (0.489 + 1.00) \times 1/2 \times 0.70$	
$+\frac{1}{2} \times 0.961 \times 0.40 - \frac{1}{2} \times 0.20^{2}$	
$= 3.582 \text{ m}^2$	
$V_1 = 3.582 \times 0.30 \times 2 = 2.149$	
$A2 = (2.794 + 3.048) \times \frac{1}{2} \times 0.866 + (0.489 + 1.00) \times \frac{1}{2} \times 0.70$	
+ ½ ×0.65 ×0.30	
= 3.148 m ²	
V 2149 (222)	
$V_2 = 3.148 \times 0.30 \times 2$ = 1.889	
70.701 (11.11) - 4.02 0	4000
70TAL (U1+U2) = 4,038	4.038 m ³
• GRAVEL BEDDING	
$A_3^2 (0.50 + 0.70) \times \frac{1}{2} \times 0.10 = 0.060 \mathrm{m}^2$	
V3 = 0.060 × (3.065 + 0.489) × 2 = 0.426	
$V_4 = 0.060 \times (3.048 + 0.489) \times 2 = 0.424$	
TOTAL (V3+V4) = 0.850	0.850 m ³
· FORM (H<4.0m)	
$A_4 = 3.582 \times 2 \times 2 = 14.328$	
$A_5 = 3.148 \times 2 \times 2 = 12.592$	
TOTAL (A4 + As) = 26.920	26.920 m ²
	AND THE RESIDENCE OF THE PROPERTY OF

DRAINAGE SLUICEWAY AT WF. 172R+15m

TYPE OF WORK

: PARTITION WALL

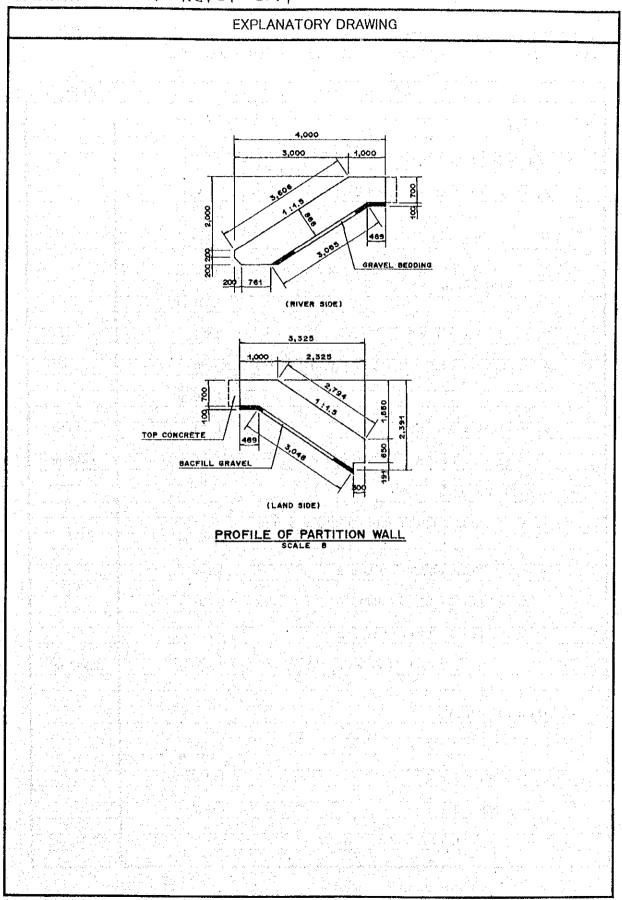
LOCATION : PARTITION WALL LOCATION : REVETMENT	$(\frac{2}{2})$
CALCULATION	RESULT
· REINFORCING BAR	
(DI3, W=1:04 kgf/m)	
n,= 8Bars	
$L_1 = (3.606 + 1.00) - 0.05 \times 2 = 4.506 \frac{m}{par}$	
$W_1 = 8Barp \times 4.506 \times 1.04 \times 2 = 74.980 kgf$	
$n_2 = 8 Bars$	
$L_2 = (2.794 + 1.00) - 0.05 \times 2 = 3.694 \text{ M/Bar}$	
$W_2 = 8 Bars \times 3.694 \times 1.04 \times 2 = 61.468 kgf$	
(DIO, W=0.617 kgfm)	
$n_3 = (4.50b = 0.30) + 1 = 16.02 = 17 Bars$	
$L_3 = 0.20 \times 2 + 0.766 \times 2 + 15 \times 0.01 = 2.082 \text{ m/Bar}$	
W3 = 17 Bars x 2.082 x 0.617 x 2 = 43.676 kgf	
$n_4 = (3.694 \div 0.30) + 1 = 13.31 \div 14 Bars$	
$L_4 = 2.082^{m}/Bar$	
W4 = 14 Bars x 2.082 x 0.617 x 2 = 35.969 t8f	
	<u> </u>
TOTAL (W1+W2+W3+W4) = 216.093 tof	0.216
· JOINT FILLER (t=10, ELASTIC MATERIAL)	
$A_{6} = (3.967 + 0.70) \times 0.30 \times 2 = 2.800$	
$A_7 = (3.155 + 0.70) \times 0.30 \times 2 = 2.313$	
$A_7 = (3.155 + 0.70) \times 0.30 \times 2 = 2.313$	
707AL (A6+A7) = 5.113	5,113 m ³
INIAL CABTAY) - 5.115	10.110 m 40.1
	<u> </u>

DRAINAGE SLUICEWAY AT WF. 172R + 15m

TYPE OF WORK

PARTITION WALL REVETMENT

LOCATION



TYPE OF WORK : LOCATION :

REVETMENT DRAINAGE SLUICEWAY AT WF.172R + 15 m

CALCULATION	RESULT
₽ END WALL	
□ END WALL	
• CONCRETE (TYPE-C1)	
$A_1 = (10.062 + 9.846) \times \frac{1}{2} \times 0.60 + (0.70 + 0.558) \times \frac{1}{2} \times 0.60$	
$= 6.350 \text{ m}^2$	
$V_2 = 6.350 \times 0.30 \times 2 = 3.810$	
$A_3 = (0.525 + 0.737) \times 1/2 \times 0.70 + (4.777 + 0.49) \times (0.70 + 0.488) + 1/2 \times 0.70$	
$= 4.201 \text{ m}^2$	
$V_2 = 4.201 \times 0.30 \times 2 = 2.521$	
$A_3 = (3.155 + 2.965) \times \frac{1}{2} \times 0.70 + (0.70 + 0.49) \times \frac{1}{2} \times 0.70$	
= 2.559 m ²	
$V_2 = 2.559 \times 0.30 \times 2 = 1.535$	
$T O T A L (V_1 + V_2 + V_3) = 7.866$	7.866 m ³
GRAVEL BEDDING	
$A_4 = (0.40 \times 0.10) + (0.10 + 0.30) \times \frac{1}{2} \times 0.10$	
$= 0.060 \text{ m}^2$	
$A_4 = (9.846 + 0.558) \times 0.06 \times 2 = 1.248$	
ting of the confidence of the	
$A_5 = (0.737 + 4.777 + 0.488) \times 0.06 \times 2 = 0.720$	
$A_6 = (2.965 + 0.490) \times 0.06 \times 2 = 0.414$	
$TOTAL (V_4 + V_5 + V_6) = 2.382$	2 202 3
$\frac{101RE}{101RE} \left(\frac{1}{4} + \frac{1}{3} + \frac{1}{6} \right) = \frac{2.582}{100} = \frac{2.582}{100} = \frac{1}{100}$	2.382 m³
EODW (II < 40 m)	
• FORM (H < 4.0 m)	
$A_5 = 6.350 \times 2 \times 2 = 25.400$	
$A_6 = 4.201 \times 2 \times 2$ = 16.804	
10.001	
······································	

TYPE OF WORK: LOCATION:

DRAINAGE SLUICEWAY AT WF.172R + 15 m

CALCULATION		RESULT
REINFORCING BAR		
		1.
(D 13, W = 1.04 kgf/m)		1 1 1 1 1 1 1 1
$n_1 = 6 \text{ Barss}$		
$L_1 = (10.062 + 0.70) - 0.05 \times 2 =$	10.662 m /Bar	
$W_1 = 6 \text{ Bars } \times 10.662 \times 1.04 \times 2 = $	131.062	
and the second of the second o		
$n_2 = 6 \text{ Barss}$		a grant and a
$L_2 = (0.525 + 0.70) - 0.05 \times 2 =$	3.755 m/Bar	
$W_2 = 6 \text{ Bars x } 5.902 \text{ x } 1.04 \text{ x } 2 =$	73.657	
		The section
(D 10, W = 0.617 kgf/m)		
$n_3 = 6$ Bars		
$L_3 = (3.155 + 0.70) - 0.05 \times 2$	3.755 m/Bar	A to
$W_3 = 6 \text{ Bars } \times 3.755 \times 1.04 \times 2 = $	46.862	and the second
		53 FT 2 ST 1
$n_4 = (10.662 : 0.30) + 1 = 36.54 \cong$		and the second
$L_4 = 0.20 \times 2 + 0.50 \times 2 + 15 \times 0.01 =$	1.550 m /Bar	
$W_4 = 37 \text{ Bars } \times 1.55 \times 0.617 \times 2 =$	70.770	
$n_5 = (6.002 : 0.30) + 1 = 21.006 \cong$	22 Bars	and the second
L ₅ = 1.550 m/Bar		
$W_5 = 32 \text{ Bars } \times 1.55 \times 0.617 \times 2 = 3.55 \times 0.617 \times$	42.079	
$n_6 = (3.755 : 0.30) + 1 = 13.52 \cong$	14 Bars	1919
$L_6 = 1.550 \text{ m/Bar}$		
$W_6 = 14 \text{ Bars } \times 1.55 \times 0.617 \times 2$	26.778	
$TOTAL (W_1 + W_2 + W_3 + W_4 + W_5 + W_6) =$	393 208 kgf	0.393 tf
	N. A. Martin	
JOINT FILTER (t = 10, ELASTIC MATERIAL)		
$A_8 = (10.062 + 0.70) \times 0.25 \times 2 =$	5.381	5.381 m ³
		territoria de la composición dela composición de la composición de la composición de la composición de la composición dela composición dela composición dela composición de la composición de la composición dela composici

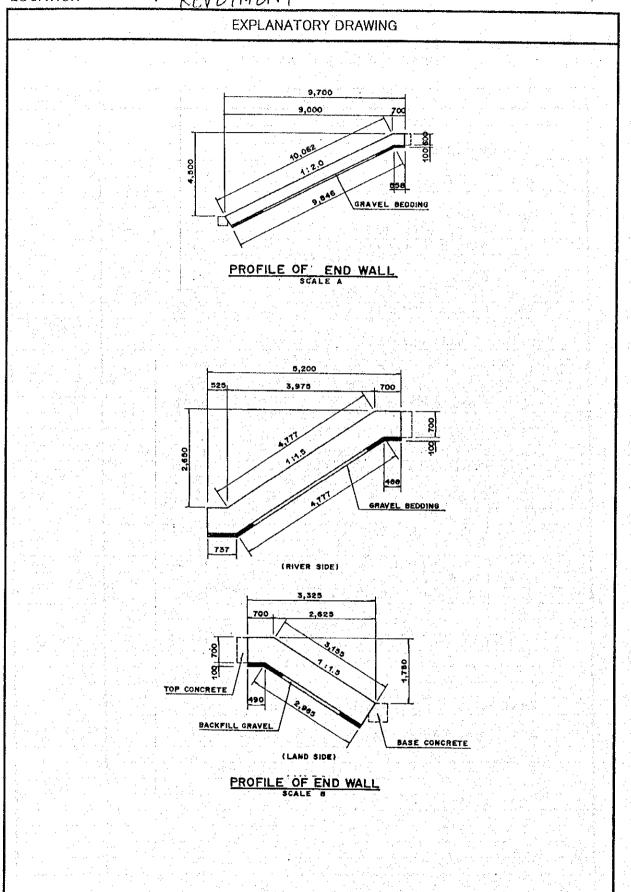
DRAINAGE SLUICEWAY AT WF-172R+ 15M

TYPE OF WORK

END WALL

LOCATION

REVETMENT



TYPE OF WORK: CONNECTING CHANNEL

LOCATION : DRAINAGE SLUICEWAY AT WF.172R + 15 m

CALCULATION		RESULT
CONCRETE (TYPE – C1)		
(TYPE-CI)		10 0.00
(TIFE-CI)		
Number of concrete cover: n = 110 nos		
	salah bilangan	
$V = (0.50 \times 1.18 \times 0.15) =$	9.735	9.735 m ³
		Park Francisco
FORM (H < 4.0 m)		
$A_1 = (0.50 \times 0.15) \times 2 \times 110 =$	16.500	
$A_2 = (1.18 \times 0.15) \times 2 \times 110 =$	38.940	
	en fransk filmen fra 1960 i 1960 Primar kantana i 1960 i 19	
TOTAL =	55.440	55.440 m
REINFORCING BAR	<u> 1, 6 , 7 , 6 , 7 , 7 , 7 , 7 , 7 , 7 , 7 </u>	
RESTRUCTION DAN	Andrew Commencer	
Number of concrete cover : $n = 110$ nos		
(D.12 W1.04 loc/m)		
(D 13, W = 1.04 kgf/m)	and a second of the second of	
$W_1 = (4 \text{ Bars x } 1.12 \text{ x } 1.04) \text{ x } 110 =$	512,512	
$W_2 = (9 \text{ Bars x } 0.44 \text{ x } 1.04) \text{ x } 110 =$	453.024	
$(\varnothing 13, W = 1.04 \text{ kgf/m})$	<u>z nach beginne de personal factories de la companya de la company</u>	
$W_3 = (2 \text{ Bars } \times 0.41 \times 1.04) \times 110 =$	93.808	
$W_4 = (2 \text{ Bars } \times 0.31 \times 1.04) \times 110 =$	70.928	
$TOTAL (W_1 + W_2 + W_3 + W_4) =$	1130.272 kgf	1.130 tf
The probability of the control of th	1130.212 Ng1	1.150 11
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TYPE OF WORK : LOCATION :

CONNECTING CHANNEL DRAINAGE SLUICEWAY AT WF.172R + 15 m

CALCULATION	RESULT
WET STONE MASONRY	
$A_1 = (0.30 + 0.50) \times \frac{1}{2} \times 1.70 \times 2 = 1.360$	
$A_2 = 0.25 \times 1.00 = 0.250$	
$A_3 = -(0.10 \times 0.15) \times 2 = -0.030$	
TOTAL = 1.580 m	
77	
$V_1 = 1.580 \times 55.00 = 86.900$	
V = (0.20 + 0.50) + 1/ = 1.70 - 1.60	
$V_2 = (0.30 + 0.50) \times \frac{1}{2} \times 1.70 \times 1.60 = 2.176$	
$V_3 = -\pi/4 \times 0.74^2 \times 0.30 \times 4 = -0.516$	
$V_3 = -\pi/4 \times 0.74^2 \times 0.30 \times 4 = -0.516$	
TOTAL = 88,560	88.560 m ³
101AL - 86,300	88.30U III
<u>and and the state of the state</u>	<u>al Parting plants y la Maria de Maria.</u> Antara de la Carla de Maria de Carla de
GRAVEL BEDDING	
$V = (2.00 + 0.10 \times 2) \times 0.10 \times 55.00 = 12.100$	12,100 m ³
	and the state of t
and the control of th	

TYPE OF WORK : CONNECTING CHANNEL LOCATION : DRAINAGE SLUICEWAY

CALCULATION	RESULT
☐ STRUCTURAL EXCAVATION	
$V_1 = 8.50 \times 55.00 = 467.50$	
$V_2 = (0.50 + 1.55) \times \frac{1}{2} \times 2.10 \times 3.00 = 6.46$	
$V_2 = (0.30 + 1.55) \times 72 \times 2.10 \times 5.00 = 0.40$	
	<u> </u>
TOTAL = 473.96	473.96 m ³
	1
	<u> </u>
Burn Karagan Baragan Karangan Baragan	
re elleration of the participation of the first of the contraction of the participation of the contraction o	
➡ MORTAR PLASTERING	
	4.
$A_1 = 1.30 \times 55.00 = 71.500$	-
$A_2 = 1.00 \times 55.00 = 55.000$	
A ₂ = 71.00 x 55.00 = 55.000	
$A_3 = (1.30 \times 55.00) - \pi \times 0.74^2 \times 4 = 69.780$	
(1.50 × 55.00) - 1 × 0.14 × 4	
$A_4 = 1.30 \times 1.00 = 1.300$	
	10 Aug 10 1
TOTAL = 197.580	197.580 m ²

TYPE OF WORK : LOCATION :

CONNECTING CHANNEL DRAINAGE SLUICEWAY AT WF.172R + 15 m

CALCULATION		RESULT
☐ BACKFILL WITH SELETED SOIL		
$V_1 = 5.20 \times 55.00 =$	= 286.00	
$V_2 = (0.50 + 1.55) \times \frac{1}{2} \times 2.10 \times 3.00 =$	= 6.46	
Y ₂ = (0.30 + 1.33) x /2 x 2.10 x 3.00	0.40	
TOTAL =	= 292.46	292.46 m ³
- COMONINA CIVINA	and the second of the second	
F CONCRETE (TYPE-C1)		
CONCRETE (TYPE-C1)		
CONCRETE (TITE-CI)		
$A_1 = (2.240 \times 0.23 \times 0.30) \times 4 =$	- 0.618	
$A_2 = (0.94 \times 0.30 \times 0.50 \times 2) \times 4 =$	1.128	
	a la come de la come de la come	
$A_3 = (0.35 \times 0.60 \times 0.50 \times 2) \times 4 =$	= 0.840	
	<u> </u>	
TOTAL =	= 2.586	2.586 m ³
TOTAL STATE OF THE	- 2.380	2.380 m
FORM (H < 4.0 m)		
$A_1 = \{(2.24 \times 1.52) - (1.24 \times 1.29)\} \times 4 =$	7.221	
$A_2 = (0.23 \times 2.24) \times 4$	2.061	
$A_3 = (0.94 \times 0.50) \times 2 \times 4$	= 3.760	
$A_4 = (0.35 \times 0.50) \times 2 \times 4 =$	= 1.400	
14 (0.35 x 0.35) x 2 x 4	- 1.400	
$A_5 = \{(1.17 \times 0.30) + (0.35 \times 0.60)\} \times 2 \times 4 =$	4.488	
TOTAL =	18.930	18.930 m ²

TYPE OF WORK : DRAINAGE PIPE

LOCATION : DRAINAGE SLUICEWAY AT WF.172R + 15 m

CALCULATION	RESULT
F CONCRETE (TYPE – D)	
Pipe Length: L = 3.35 m /place (Average)	
CONCRETE (TYPE – D)	
$V = \{(1.24 \times 1.29) - \pi / 4 \times 0.74^2\} \times 3.35 \times 4 = 15.672$	15.672 m ³
	s a justinije
	grande and additional
FORM (H < 4.0 m)	
$A_1 = (1.29 \times 3.35) \times 2 \times 4 = 34.572$	
$A_2 = \{(1.24 \times 1.29) - \pi / 4 \times 0.74^2\} \times 4 = 4.678$	
And the first of the first property of the second of the state of the second of the se	14 14 4
	samile mine
TOTAL Section 39.250.	39.250 m ²
<u>a partimenti i mangorito di kenangingti nyawa na mangatangan ngantangan ana ana angan manang</u>	4
and Conservation of the Co	
la digustra lugat giung tu lulah selu daka kebisaran kalendaran dibungga dan berasti bandar digusah alam balam Digungga kebisah digunah sebagai digunah sebagai berasti dan berasti dan berasti bandar berasti dan berasti da	
GRAVEL BEDDING	
<u>ar kanada ang kanada kanada ang ang ang kanada kanada kanada kanada kanada kanada ang kanada ang kanada ang k</u> Banggang kanada kan	
Pipe Length: L = 3.35 m/place (Average)	
$V_1 = (1.24 + 0.10 \times 2) \times 0.10 \times (3.35 - 0.70) \times 4 = 1.526$	e Charles and
Barbara Barbaranga kada mengalah berangan beranjak berangan berangan berangan berangan berangan berangan beran	
$V_2 = (2.24 + 0.10 \times 2) \times 0.10 \times 0.80 \times 4 = 0.781$	
TOTAL = 2,307	
	er vega etc.
	e de la companya de

TYPE OF WORK:

DRAINAGE PIPE DRAINAGE SLUICEWAY AT WF.172R + 15 m LOCATION

CALCULATION	RESULT
「 REINFORCING BAR	
D12 (W=1.04 levf/m)	
D13, $(W = 1.04 \text{ kgf/m})$	
$W_1 = 4 \text{ Bars } \times 2.14 \times 1.04$	9 003
$W_1 = 4 \text{ Bars } \times 2.14 \times 1.04$ $W_2 = 22 \text{ Bars } \times 0.65 \times 1.04$	= 8.902 = 14.872
$W_2 = 22 \text{ Bats } \times 0.03 \times 1.04$ $W_3 = 8 \text{ Bars } \times 0.38 \times 1.04$	= 3.162
$W_4 = 6 \text{ Bars } \times 3.185 \times 1.04$	= 19.874
$W_4 = 6 \text{ Bars } \times 3.183 \times 1.04$ $W_5 = 6 \text{ Bars } \times 1.45 \times 1.04$	= 9.048
W5 - 1 0 Data X 1.43 X 1.04	7.040
TOTAL	= 55.858 kgf/place
TOTAL CONTROL OF THE PROPERTY	- 33.636 kg17 place
$W = 55.858 \times 4$	= 223.432 kgf 0.223 tf
	223.432 kgi 0,223 ti
<u>and the control of t</u>	
andriana anno an deilean an t-airean an t-airean an t-airean an t-airean an t-airean an t-airean an t-airean a Ta airean an t-airean an t	
taran da ang katalong a sa katalong a sa Banan da ang katalong a sa	
ing the state of t	
The state of the control of the state of the	
And the second of the second o	
and the second second of the second s	
F CONCRETE PIPE Ø600	
<u> 1. Tengan merapatan kemanan merapatan berapatan berapa</u>	
L = (3.40 + 4.00 + 3.50 + 2.50) =	
	e symplike er
the control of the co	

TYPE OF WORK : DRAINAGE PIPE

LOCATION : DRAINAGE SLUICEWAY AT WF.172R + 15 m

$V_2 = 5.40 \times (0.70 + 0.50) \times 4 = 25.92$	
Drainage Pipe Length: $L = 3.35 \text{ m/place (Average)}$ $V_1 = 6.10 \text{ x } (3.35 - 0.70) \text{ x } 4 = 64.66$ $V_2 = 5.40 \text{ x } (0.70 + 0.50) \text{ x } 4 = 25.92$	
$V_1 = 6.10 \times (3.35 - 0.70) \times 4 = 64.66$ $V_2 = 5.40 \times (0.70 + 0.50) \times 4 = 25.92$	
$V_1 = 6.10 \times (3.35 - 0.70) \times 4 = 64.66$ $V_2 = 5.40 \times (0.70 + 0.50) \times 4 = 25.92$	
$V_1 = 6.10 \times (3.35 - 0.70) \times 4 = 64.66$ $V_2 = 5.40 \times (0.70 + 0.50) \times 4 = 25.92$	
$V_2 = 5.40 \times (0.70 + 0.50) \times 4 = 25.92$	
TOTAL = 90,58	e e je strovini i over
	90.58 m ³
大学的 (1994年) "大学的","我们的一个大学的"大学","大学"的"大学"的"大学","大学"的"大学"。	Marie Par
F BACKFILL WITH SELECTED SOIL	
Drainage Pipe Length: L = 3.35 m / place (Average)	
$V_1 = 4.40 \times (3.35 - 0.70) \times 4 = 46.64$	
$V_2 = 4.60 \times 0.50 \times 4 = 9.20$	
$V_3 = 2.20 \times 0.70 \times 4 = 6.16$	<u> </u>
TOTAL = 62.00	62.00 m ³
	reserve to the second
	·

5.2 Drainage Outlet
TYPE OF WORK: OUTLET STRUCTURE

LOCATION

: WF 72+22 [R.3A].

CALCULATION	RESULT
G.2 . OUTLET STRUCTURE	
6.2.1. Structural exeavation	288 1113
G 2.2 Backfill With Selected soil	42.188 m3
G.2-3 log pile, Dia 150 mm, L=2.0 m	
G. 2.9 Gravel Bedding.	3.753 m3
6.25 Chipping on Existing Structure.	6.380 m ²
6.2.6 Concrete, type C1 Including Form work.	0756 m3
G. 2.7. Deformed Reinforcing Bars.	56.197 kg
628 Wet stone masonry	38.280 m3
G. 2.9. Cement Mortar Pointing on Riverside Surface	
of wet stone masonry	
G. 2. Secondary Concrete	0.260 m ³

-	·····																								·
RESULT				0.756 m3						-					56.197 kg				-	-					
CALCULATION	6.26 Conricte, type. CI Including Form work.	V1=2/0.3x0.6/x1.69 = 0.288	Vz= > 1.80x 2.60/x 0.10 } = 0.968	70-46 0.756			G. 2.7 Deformed Reinforcing Bars.	W1=1(6×1.5/×0.617) = 5.553.	1(6x1.6/x1.04) = 9.984		W2=1(7x2.5/x0.617) =10.788	119×1.7/×1.58} =24.174.	W2 =34.972	W3=1/12x0-3/x1-3/	761.85 74707	6.2.8 Wet stone masonry	VI= 1/0-3+0.8/x1/2x168x1.788x2=3.197	f(0.8 × 0.8/×1.788 /×2 = 2.289.	110.3×1.6/×1.7881 = 0.858	11 = 6.299	42=7(0.3+0.915/x1/2×0.33x2.0x2=0.429	110.3+0.8/x1/2x163x2.0x2 = 3.520.	110.8x0.8/x 2.04x2 = 2.560	110.3×16/×2.03	V2
OUTLET STRUCTURE	WF 72+22 (R.34)	(7) Foot slab bridge		V. V.	7.00	10.10	1.80		(2) Foot state Crity	D10-300	0 000-300			/ 5= 50 (L=30)		Foot Net bridge. Secondary Concrete	200 016 016	2.5.	, - 85: 1	annel.		10.3	The second secon	× 2.0 08 08 1 03 7 0.3	8.0 3.1 8.0
TYPE OF WORK:	LOCATION:	6.2.6	10 70th Pagarate	0.3	$^{\perp}/$	9:0		6.2.7	Conco	10.0	005-000	 1	ް5	(2) Anchor bars		Top concrete To	D10 D13	1.5 1.6	0.01.1.0.01	6.2.8 outlet channel	7.2072	' <u> </u>	6 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	12(3.576) 2.0	1,788

								 				 					· · · · ·	 	 1
RESULT					38.280 m					q	0260 M							 3,302%	
CALCULATION	V3=1/0.4+1.0/x/2x2.03x5.02xx2=14.067	\$10.8x1.0/x5.029 gx2. = 8.038	110.3×1.6/×5.029 9 = 2.412	43-67	70746 38.280	G. 2.9. Cement Mortar Pointing on Riverside	Surface of wet stone Masoury.	No work (Because Existing Revenient).	G. 2 - Secondary Concrete.	y=1/0.2+0.319/x/2×1.73×1.9-	11.0×1.0)×10.2+0.312/x1.3 = 0.260		form for type C2		A1 = 1.40 × 1.70 - (1,00×1.00) = 1,380	Az > (0.20 + 0.319) × /2 × 1,70× 2 = 0.882	= 0.26×1.60 × 4	70TAL = 3.302	
TYPE OF WORK:	LOCATION WE12+22 (R.34)					233			G. 2. Jecondary com				6.30	\mathcal{T}					

OUTLET STRUCTURES WF.75R + 3.00 TYPE OF WORK : LOCATION :

	CAL	CULATION		RESULT
STRU	ICTURAL EXCAVATION			
	(1.00) 0.00			
A ₁ =	$(7.70 + 8.25) \times \frac{1}{2} \times 0.53$	5 =	4.386 m ²	
	(7.70 10.272)	670	22.110	
A ₂ =	$(7.70 + 10.272) \times \frac{1}{2} \times 2$.572 =	23.112 m ²	
V ₁ =	(4.386 + 23.112) x ½ x	4.544 =	62.475	
	(1.500 · 25.112) X 72 X	7,544	04.473	
V ₂ =	23.112 x 2.00		46.224	+
			10.224	
1.5		TOTAL =	108.70	108.70 m ³
e e e secul				
<u> </u>				
1 1		of with the plant of the		
<u> </u>				
BAC	CONTRACTOR OF THE CONTRACTOR O			the west of the
BAU	KFILL WITH SELECTE	D SOIL		
V ₁ =	(0.50 + 0.775)) x ½ x 0.	55 - 7.700		
<u> </u>	(0.50 + 0.775)) X 72 X 0.	33 x 7./00 =	2.700	
V ₂ =	{(0.50 + 0.775) x ½ x 0	55 × (0.50 ± 1.786)	v 1/ v 2 572)	
	x ½ x 4.044 x 2	.55 X (0.50 1 1.760) .	× /2 × 2.3/2}	
. 12 .			13.306	
V ₃ =	(0.50 + 1.786) x ½ x 2.5	$72 \times 2.00 \times 2 =$	11.759	
1 1		ta tuali eta 15 de 18e et		
		TOTAL =	27.77	27.77 m³
	and the first of t			
_' .				

TYPE OF WORK : OUTLET STRUCTURES

LOCATION : WF.74R + 3.00

CALCULATION		RESULT
를 CONCRETE (TYPE – C1)		2
CONCRETE (TYPE - CI)		
$V_1 \approx 0.25 \times 0.40 \times 6.70 =$	0.670	
$V_2 = 4.444 \times 0.45 \times 6.70 =$	13.399	
$V_3 = \frac{1}{2} \times 4.044 \times 2.022 \times 0.35 \times 2 =$	2.862	
$V_4 = 2.022 \times 0.40 \times 0.35 \times 2 =$	0.566	
TOTAL =	17.497	17.497 m ³
• FORM (H < 4.0 m)		
$A_1 = 0.75 \times 6.70 + 0.25 \times 6.70 + 0.45 \times 6.70 =$	9.715	
$A_2 = (0.25 \times 0.40 + 4.444 \times 0.45) \times 2 =$	4.200	
$A_3 = (1/2 \times 4.044 \times 2.022 + 0.40 \times 2.022) \times 4 =$	19.589	
TOTAL =	33.504	33.504 m ³
₽ REINFORCING BAR		
• D 16 (W = 1.58 KGF/m)		
$W_1 = (2.247 + 0.225) \times \frac{1}{2} \times 19 \text{ Bars } \times 2 \times 2 \times 1.58 = $	148 410	
$W_2 = (0.20 + 4.294) \times \frac{1}{2} \times 7 \text{ Bars } \times 2 \times 2 \times 1.58 =$	100.513	
$W_3 = 6.55 \times 40 \text{ bars } \times 1.58 =$	413.960	
W ₄ = 4.294 x 48 Bars x 1.58 =	325.657	
TOTAL = 988.549 kgf =	0.989 tf	0.989 tf

TYPE OF WORK : LOCATION :

OUTLET STRUCTURES

WF.74R + 3.00

	ATION	RESUL
CONCRETE (TYPE – E)		
CONODETE (TYPE TY		
• CONCRETE (TYPE – E)		
$V_1 = 0.10 \times 0.60 \times 6.90$	= 0,414	
$V_2 = 0.10 \times 4.044 \times 6.90$	= 2.790	
<u>an an an Air an</u>	OTAL = 3.204	3.204 m
• FORM (H < 4.0 m)		
$A_1 = 0.10 \times 6.90 \times 3$	= 2.070	
$A_2 = 0.10 \times 4.044 \times 2$	= 0.809	
<u></u>	OTAL = 2.999	0.000
<u> 1 - Particular de La Carte de La Carte de Cart</u>	OTAL = 2.999	2.999 m
TOCDUE		
LOG PILE		
LOGPILE		
	ile	
• Ø 150 mm L = 2.00 m/p		
	ile = 12,000	12.000 n
• Ø 150 mm L = 2.00 m/p		12.000 n
• Ø 150 mm L = 2.00 m/p		12.000 n
• Ø 150 mm L = 2.00 m/p		12.000 n
• Ø 150 mm L = 2.00 m/p L = 2.00 m/pile x 6 piles	= 12.000	
• Ø 150 mm L = 2.00 m/p L = 2.00 m/pile x 6 piles	= 12,000	
• Ø 150 mm L = 2.00 m/p L = 2.00 m/pile x 6 piles	= 12,000	
• Ø 150 mm L = 2.00 m/p L = 2.00 m/pile x 6 piles	= 12.000	
• Ø 150 mm L = 2.00 m/p L = 2.00 m/pile x 6 piles	= 12.000	
• Ø 150 mm L = 2.00 m/p L = 2.00 m/pile x 6 piles	= 12.000	
• Ø 150 mm L = 2.00 m/p L = 2.00 m/pile x 6 piles	= 12,000	
• Ø 150 mm L = 2.00 m/p L = 2.00 m/pile x 6 piles	= 12,000	
• Ø 150 mm L = 2.00 m/p L = 2.00 m/pile x 6 piles	= 12.000	
• Ø 150 mm L = 2.00 m/p L = 2.00 m/pile x 6 piles	= 12,000	

TYPE OF WORK : OUTLET STRUCTURES

LOCATION : WF.74R + 3.00

CALCULATION		RESULT
JOINT FILTER		
	<u> </u>	
+ - 10 ELACTIC MATERIAL		
t = 10, ELASTIC MATERIAL		
$A_1 = 0.45 \times 6.70 =$	3.015	
	3.013	
$A_2 = 0.35 \times 2.022 \times 2 =$	4.430	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
	erivita vega i revi vida	
u di la cale di manda di seria di TOTAL	7.445	7.445 m²
HOLE WITH NON - SHRINKAGE MORTAR		
• ANCHOR BAR (D 16, L = 0.60 m/Bar)		
W = 1.58 kgf/m		
N = 20 places		
W = 0.60 m/Bar x 20 x 1.58 =	18.960	18.960 kgf
0.00 M / Dal X 20 X 1.00	10.700	10.700 Kg1
NON – SHINKAGE MORTAR		
$V = \pi / 4 \times 0.03^2 \times 0.30 \times 20 =$	0.004	0.004 m ³
	<u> Paramanan da kabupatèn kabupatèn kabupatèn kabupatèn kabupatèn kabupatèn kabupatèn kabupatèn kabupatèn kabup</u>	
		11 to 1
	the state of the s	
公子 医克德二氏试验 经基础 医多种性 医动物性 医电影 医皮肤皮肤 化二甲基甲基甲基甲基甲基甲基甲基甲基甲基甲基甲基甲基甲基甲基甲基甲基甲基甲基甲基	and the second of the second o	

TYPE OF WORK

: OUTLET STRUCTURE

LOCATION

: WF 157R (R9).

CALCULATION	RESULT
G.2. DUTLET STRUCTURE	
G.2.1 Structural Excavation	262.695 m ³
G. 2.2 Backfill With Selected soil .	90.531 m3
G. 2.3. Log Pile, Dia 150 mm, L= 2.0 m	4.0 m
G. 2.4. Gravel Bedding	15.664 m3
G 2.5 Chipping on Existing structure	9.0 m2
626 Concrete, Type-CI Including Formwork.	11.1374 m3
627. Deformed Reinforcing Bars.	264.680 kg
G 2.8 Wet stone masonry.	82.891 m3
629 Cement Morter Pointing on Rivercide Surface	
of Wet stone Masonry	18.499 m
G. Secondary Concrete	0.385 m3
G. Gabion motthess.	4.500 m3
G gabion cylinder	35.478 m3
6. Rubble stone filling	9.60 m

RESULT	-													ſ	262 695 m							90.531 m			4.0 m
CALCULATION	G.2.1 Structural Excavazion	Vi=1/1:1+2.55/x/2x2.9/x2x10.12 = 107.120	flig x 2.9 /x 10.12 } = 34.003	111.1+2.05/x12x1.9/x2x5.24 = 31.361	111.9 × 1.9 / × 5.29 g = 10.270	10.6 + 1.2/x 1/2 x 1.2 3 x x x 5.0 = 10 800	411.9×0.9/×5.0 } = 6.300	199.859	V2 = 310.6+0.95/x 12 x 0.7 3x 2x 8.826 = 9.576	105x2.2/x8.826} = 9.709	1(4.0+4.25/x/2x0.5/x3.0 =6.188	10.7+1.4/x/20.63×3.0 = 1,890	3.0x2 =35.978	62.841	70746 262.695	G.z.z. Backfill with selected soil	VI=1/0.15+1.6/x/xx.9/x2x10.12+1/2x07/x1/x2/x1012=65.527	10015+11/4/2x134x2x529+H1x0.31/x1/xx24x524=14.279	1(0,15+0)5/x/2x/2xxx5.0+1(0.5x0.8/x/x2xx50=6.900		1/2=1/0.15+0.55/x/2×073×2×8 826 = 4.325	185.06 78407		6.2.3 log. Pile Dia 150 mm, 6 = 2.0 m.	L=1/220x2/f == == == == == == == == == == == == ==
OUTLET STRUCTURE	WF.157 R (R.9)	O over channel	1.9 2.55 2.05 1.9 2.05	611 811	10.15			2.0		(6) Base concrete	100	1 26.0	0.5		10.61	920	[]	S	Dutlet Shannel		1.1.0.35 2.7.2 2.7.3 1.0.7.5		ł	7 6.2.3	-7 2.5 U 2.0.
TYPE OF WORK	LOCATION	6.21	2.50					$\left\langle \left\langle \left$	9.0	6 Oriotal +	Code wall	0.95 2.2		10.7	0.6	() gabion Collinder	3:0		G-2-2 - 1) Dut/et) '9'	1	2.9	2.80 H	2	1.7. A

RESULT.											100 W. M. D. C.	15.66900		C	20.0									11.379 m	
					7.189	0.180	1.193	0.272			6.830	15.669		•	0.6 =		Form work	=0.432	685.8 =		= 0.660.		=6.702	11.374	
		6.919	0.360	0.210	lı	[5	= 66	11	5.530	1.300	(1 	・フセ		tructure			Including Fo			+		1/x3.9-			
CALCULATION	5Wi	3.0 ====================================	1	ls		£x30	6x 5x 8.389		li .	11		707		chipping on Existing Structure	X1.2/9			2	7×166	V3=1/0.5+0.3/x/2x0.3/x3.0			3		
CALC	el bedding	x 6.22.0 x/c	12 gx 3.0	0.E X		0.5x+0.2/x/2x0.1fx3.0	1.0 x 1/ X /	1×3.40}	1×15.36	1×5.0/				ng on Ex	011-10		Concrete type-C1	5/x 2.4	8/x9.989	3/x/2x0	105×12.0×201	2/4/0.8x	1,0×12/×022		
	4. gravel	VI=/(7.826+1.0/x0.25/x3.0	\$10.6×0.4/x 12 3x3.0	110.1x0.7/5x3.0		10.5.40.3	13= 105+07/x/x019x2x	V9=210.1x0.8/x	15=110.1×3.6/×15.36	5/0.1 x 2.6 J x 5.0 }				- chippi	A=1(3.70×3.0)-11.0×1.2		:	=1/0.3×0.6/x2.9	(0.3×0.6)	10.540	10-5 x0.2	=4/0.6×2.2/+(0.8×0.932)	11.0×1.2		
	6.2.4	1/=11	14	1/6) / / X	12=/	ــــــــــــــــــــــــــــــــــــــ	-	=51	7) >			5.2.5	A=1/		5.2.6	12 13	12=4	1/3=;		- 64			
			de walk.	3.945	(X) (X)	1.01	120	Concrete woll	4.12		1.0.1 1.0.1										0.6 3.4	 - -	3.0	0.933	Concrete wall
URE	CR.9/.	(3026		2.5		1	(A)	†		-[1]	9	7			1	0.7	<u>. 1</u>	\			_\	0.3	10 1 10 1 10 10 10 10 10 10 10 10 10 10	(Q)
STRUCTURE	8		30	\ -	22.0		3 TOP CONCRIC	/m		10+/30	6.3				_ ; - -	2.6				07/2	7	Spase Concrete	\\ \		<u>:</u>]
OUTLET	WF. 157		011				© 7.			<i>/</i> -		de jon	12.	<u></u>				wall	• • • • • • • • • • • • • • • • • • • •		(0.3 1.3	86.6	ان (م	
TYPE OF WORK:			O for revetment	٥	18.	 		1/0//	0			S owler channel				3.6		concrete wall				\ \			
×	LOCATION:		r reut						4.0	7,	$\begin{bmatrix} \vdots \\ \vdots \end{bmatrix}$	ر م						6.2.5			9.2.6	47.4 6.03 7.7.4	e .		٠

	·						 · } 				*****				,	Parama				,	 		~~~~
RESULT		: .	,	18.499 10		·		0385 11		To	4.500 M				35.478 m				0.60 m				
CALCULATION	G. 2.9 Cement Mortar Pointing on Riverside	surtace of wet stone masonry.		4 = 16.708+1.0/x2.9/ = 18.999		G. Z Secondary concrete.	V= 160.2+0.333/x 1/2x 1/3/ x 1/90 =	\$/1.0 x1.2/x0x75 = 0.385	G. 2 gabion mottress		V= 1/3.0 x 3.0 x 0.5}		G. 2 gabion cylinder		V=2/3:0+7.826+1.0/x0.5/x3.0x2 = 35.978		G.2 Rubble stone Filling		V= 1/0.5×1.00/x12x2.90} = 0.60				
OUTLET STRUCTURE	WF-157 R (R3)				Secondary concrete	10元人		0,233		6.2	gabion Cyli	200	13.0 1.8	(xx)		Oukhlo stone Filling						A Company of the Company o	
TYPE OF WORK:	LOCATION:	6.29	Revetment	801.						6.2.	gabion metricss		3.0		2.0	A.16616	17 17.	407	10.5				

TYPE OF WORK

: OUTLETSTRUCTURE

LOCATION

R11 WF 162 + 41,00

CALCULATION	RESULT
G.2.1. STRUCTURAL EXCAVATION	376,33143
G. 2.2 BACKFILL WITH SELECTED SOIL	115,46 11,3
G.2.3 LOG PILE \$150 mm 1=2,00 m.	4,00 m'
G.2.4. GRAVEL BEDDING	638 mi
G.2 5 CHIPPING ON EXISTING STRUCTURE	7.00 m
G. 2.6 CONCRETE, TYPE C-1	9.30 14
SECONDARY CONFRETE TYPE C-2	0,34 11,3
G. 2. 7 DEFORMED REINFORCING BARS	25400kg
G.28 WETSTONE MAJONRY	147,341,3
G29 CYLINDER GABION \$500 Min.	20,0011,
(GALVANIZED & PUC COATED)	
G. 2 10. CEMENT MORTAR POINTING ON	76,504,2
RIVER SIDE SURFACE	
G. 2 11. RUBBLE STONE FILLING	0,751,7

, ,	-			-	 at also Teams	 	·	-											·			
RESULT.																				37633 12		ۍ ^ر
CALCULATION	G21 STRUCTURAL EXCAVATION.	L1 = 17,928+3,00+0,60=22,5704,	Vol a. = (2,35+0,90//2 x 2,90 x 22,570= 106,36.	, A	2,90,59	23 = 3,610 m	Vola = 11.45+0,60/2*1,70*3,610 = 6,29	•		1962		63= 4,00 h	Vola= (1,25+0,60//2 *1,30 * 4,00 = +,81.	10 L-0,90 × 150 × 400 5,40	Vol @ = Vol a = +, 81.	70.51	20 m	Vol = 1060*10,00/x 350 . 51,00.		Told the second of the second		
ロロナム声子 ふフペンCアイが高。	R11 WF 162R+ 41,00.				0 12												000					00000/
TYPE OF WORK:	LOCATION:	7/7	2350 . 2350	•	2006	900 800		77	1450 1450		100		600			7530	13000	0037	600 600	<i>b</i> 7	200	

TYPE OF WORK: OUTLET STRUCTINE	CALCULATION	RESULT
	G22 SACKFILL WITH SELECTED SOIL	
	6,= 22,570 m	
	Vola - 11/15+1,25/12 * 050 + (1,25+0,10/12 /* 2,40)	THE LANGE OF THE REAL PROPERTY OF THE PARTY
	, ,	
006		
	100,12.	
20° 20° 20° 20° 20° 20° 20° 20° 20° 20°	22 = 3,61 h	
	Vola - ((0)540,85)/2 x 0,20+(0,85+0,10)/2 /x150/	
3000		
	Velt = Vola	
011		
	10 a: 10,70+410/2 x 1,20 x 4,00 = 192.	
700/		
d	384.	
	24 = 850m	
	11.4 = (0,60×050) × 850×2	
009		
2000	Total	1154647
00/		
	6.2.3 LOG PILE \$ 150 mm L= 2,00 m.	
05/0/0/2	2 * 3 00 =	4,00 %
•		**************************************

											to government or the								 ·. ·			
RESULT		638 14		70005		æ			7	930 1/2		03412										A A
CALCULATION	G24 GRAVEL SEDDING.	Vol. of gravel trabling. (750 +1,00/4 0,25 * 3,00	G.Z. S. CHIPPING ON EXISTING STRUCTURE	(2,50 * 3,10/-(1,0 * 0,70/=	G. 2 6. CONCRETE TYPE C-1	11) - JOE'O X JOJEX 0 30C- (11)	= 5,04 SEC. 2-5 (SIME WALL! (0,30 * 0,60/*2*8,50 = 3.06	0,60/* 3,00		(10,30+0,50)/2 * 0,30+(0,20 *0,50)/ * 3,00 = 0,66.	SECONDARY CONCRETE TYPE C-2	+010/-ns/-			G.2.7 DEFORMED REINFORCING BARS.	\$16/ANCHORS A / 12 * 030 * 1,53 = 1570	31 29 4040 4 150	\$13 SIDE WALL 1 2464 9,50 \$1,04 = 118,55	= 601 *005 * 9 /77			
OUTLIT STRUCTURE	RII WF 162 - 11,00.	oor g		500 100 100 100 100 100 100 100 100 100			000	55		Soci		005 504			0	• • • • • • • • • • • • • • • • • • •	0		,005/	9,00		
TYPE OF WORK:	LOCATION:	WETSTONE MAJONRY	GRAVEL	Se Louis .	005	003	001	, co	000	· · · · · · · · · · · · · · · · · · ·	200 600		ev.	wr.	3r 001		60	•	300		201 = も	4 200 11

5 - 99

RESULT				254006	9													
CALCULATION		\$10\00=WALL 2*39*150 * 0617 = 53.70.	/ BASEWA	Mol														
TYPE OF WORK	LOCATION	200 450 450 450 450 450 00 00 00 00 00 00 00 00 00 00 00 00 0	0 -0 -0 -0 -0 -0 -0 -0 -0 -0 -0 -0 -0 -0		5000		000000000000000000000000000000000000000	00/7	, o	しか かんしん かんしん かんしん かんしん かんしん かんしん しんしん し	000-010	,	• • • • • • • • • • • • • • • • • • •	COK.		000-014	05/	

		. c. 11 to
	CALCULATION	KESOLI
TYPE OF WOKK: OUT LET 3 KOUT THE	G. 2. S. WETSTONE MASONRY	
300	L, = 22, C70 n	
	Vola = /(0,30+0,30//2 * 150 + (0,80 * 9,80) x 22,570	
	= 3307m	
	1/2 &=/1,50 × 0,30/ × 22,570	
	11.10= 1040	
008	16.30 m²	
	2, = 36/0 m.	
	1 2	
000		
	11 1 1. 11.00 4030 1 4 361 = 16247	
	106	
<u>ක්</u> -		<u> </u>
1		
300	1,00 1 = 1/030+0501/2 * 0,60+ (0,60 * 0,50) * 4,60=	
85°	1	
3000	10/4-1, 1,00,00 x 4,00	
4 1500	vela	
W OU	44 = 8,50 m	
Job STUMBER GABION	1025 x 2,70 1 x 8,50	147.34
\$ 13-14 XXXX International LXXXXX		
3000		
《中国》的《中国》的《中国》的《中国》的《中国》的《中国》的《中国》的《中国》的		
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1916										i, n						ger just	• * * ;		**********	•	
	RESULT			200043	,				76,5043			0,75 mg	,								
	CALCULATION	G. 2. 9 CYLMDER GABION \$ 500 mm	(CALVANIZED AND PVC COATED)	0,25*3,14*0,50 ** 0,50 *12 :		6210 CEARENT MORTOR POINTING ON .		MASON	9,00 7 8,50		G 2. 11 RUBBLE STONE FILLING.	1001 * 050									
	TYPE OF WORK: OUTLET STRUCTURE	LOCATION: R // WF /628+ 41,00	CYLINDER GHBION			Sold Filt ING	600 000 φ 500 φ 500 φ 500 00 20 00 00 00 00 00 00 00 00 00 00 0	(-00/				SO SOUTH TILLING		102							

TYPE OF WORK

LOCATION

: OUTLET STRUCTURE : R/2 WF 165+9,00

CALCULATION	RESULT
G 2 1 STRUCTURAL EXCAVATION	363,69 M
G 2 2 BACKFILL WITH SELECTED SOIL	114,00 M3
G 2 3 LOG PILE \$ 150 mm L = 2,00 m.	4,00 M
G. 2.4 GRAVEL BEDDING	600 m
G.2.5 CHIPPING ON EXISTING STRUCTURE	7.10 m²
G 26. CONCRETE, TYPE C-1	9.10 14
SECONDARY CONCRETE, TYPE C-2	0,36 m
G 27 DEFORMED REINFORCING BARS.	246,00 kg
G 28 WETSTONE MASONRY.	80 11 mg
G 2 9 CYLINDER GABION \$500 MM.	1860m
/GALVANIZED & PVC COATED }	
G 2.10. CEMENT MORTAR POINTING ON	72,00 m
RIVERSIDE SURFACE	
G 2 11 RUBBLE STONE FILLING	0,75 m ³

RESULT										36369	>									1190017	
CALCULATION	GEL STRUCTURAL EXCAVATION	1/= 11,60 +3,00 + 0,60 +2,90 = 22,10 m	12,45+0,90/2 * 3,10 * 22,10=	Vol 6= 1,50 x 2,60 x 22,10 = 8619	16 e= 10 la		L3 = 800 x.	Vol = 1060 × 1000 1 × 800		7020				G.2.2 BACKFILL WITH VELECTED VOIL	Z = 22/0 m	\(\) = \(\)	10 Ca	2, = 8 00 m		Total	
TYPE OF WORK	のイン					O C	005	005/005				(a) VIIIIIIIIIIIIII	0000/		00t 00t	009					

															: '					,	 <u> </u>	
RESULT		4,00m			600 m			710 42		Q						9.10 12		0,364,			A Me - 1	
CALCULATION	G. 2 3 LOG PILE \$ 150 mm, L=2.0 m	44,000		G. 2 4 GRAVEL BEDDING	10(0) gravel bestown: 17.00+1.00/ *0,25 * 3,00.		G 2 C. CHIPPING ON EXISTING STRUCTURE	(2,60 x 3,10) - (1,10 x 0,90)-	G. 2. 6. CONCRETE, TYPE-C1.	12,60 * 3,10/*0,60+/0,80 * 3,10/*0,305-/1,10 * 0,90/4960	5.00	SIDE WALL (SEC 1-5)/0,30x060/*2*8,00 = 2,88.	ŧI	SASE WALL	((0,30+0,50)/2*0,30+(0,20*0,50)/*3,00 = 0,66.	10th	VECONDARY CONCRETE TYPE-C2	1-05/ *09/ *				
OUTLET STRUCTURE	R12.		1	1,00.1	300	0009	3000	7-7 - 7 - 2 - 2 - 2 - 2 - 2 - 2 - 2 - 2	700 600	007		09 Z	008	360	000 Comments							
TYPE OF WORK:	LOCATION:		GRAVE! BEDDING	ıl	300 000				0011		0	-9 z		800 1500	3/00							

	RESULT			See 211								24600h										
31	CALCULATION	G. 2. 7. DEFORMED REINFURCING BARS	\$16/ANCHORS 4/12 * 0,30 * 1,28 = 5,70	\$16/ANCHORS 8/5 29 * 0,40 * 1,58 = 18,55	SIDE WALL	- 5	CBACE WALL	2619	700 WALL / 11 * 1.50 * 0,617	1/1		The state of the s										
	K: OUTLET STRUCTURE	8 12								0	1180											
	TYPE OF WORK	LOCATION:		200			001	00%	000	- 05%	38.		305	300	<i>5</i> -	- 10	6					

							مستقد فسنديس يسرينين				*******	şt.		: 	: .		 · · · · · · · · · · · · · · · · · · ·		
RESULT							00,114			18604						12,00m		0,75.23	
CALCULATION	G 2 8 WETSTONE MAJONRY	10la=(10,30+0,80)/2* 1,50+(0,80*0,80)/*22,10	Val6= (150 x 0,30/x 22,10 = 895			(0,25x 2,70/x 8,00m) = 5,40.	The state of the s	G 2.9. CYLINDER GABION & SOO MM.	\sim	0,25 + 3,14 + 0,502 + 8,00 + 12			G.2.10 CEMENT MORTAR POINTING ON	RIVERSIDE SURFACE OF WET STONE	MASONRY	· · · · · · · · · · · · · · · · · · ·	G 2.11 RYBBLE STONE FILLING	(050 * 1,00)/2 * 3,00	
TYPE OF WORK	R /2		oop.	3)	0		• · · · · · · · · · · · · · · · · · · ·							CONCERTE TOPE OF		Coe	0000	0,000	

TYPE OF WORK

OUTLET STRUCTURE

LOCATION

WF 176 R + 27 (R.18)

CALCULATION	RESULT
F.A. Connecting channel and Drainage pipe	
F. 4.1. Structural Excavation.	9.69 m³
F. q.2 Backfill with selected soil	12.89 m ³
F. 4.3. Gravel Bedding.	1.00 m3
F. q. q. Wet stone masonry.	5.829 m3
F. 9.5 Mortar Plastering on Surface of wet stone masonry.	10.840 m2
F. 4.6 Furnishing and Installing Concrete pipe \$ 600 mm	2.20 m.
F. 1.7 Concrete type D Including Formwork.	1.279 m3
F. 4.8. Concrete type CI Including Formwork.	
F. 4.9. Deformed Reinforcing Bars.	
[1] 京庆民港 自然信息 建新基本的 基础的 建筑层 电电压电压电压	
	14. 图图 第一

ECT II T.	KESULI		•	9.69 11	ļ					12.89 m3				.00 m						,	5.829 m		
CAT CITT ATTON	CALCULATION	F-4 . Connecting channel and Drainage pipe.	F.4.1. Struetural Excavation.	V= 3(2.0 +37/x/2/x2.0. = 9.63.			F.g.z. Back Fill With selected poil	= 1(2.0+3.0/x1/2×1.03x &.0x2 = 10.00	1(0.85×1.7)×1/2×2.03×2	12.89. 13	F. 9.3. Gravel Bedding	=/(2.0 × 2.0 / × 0.1)	1(0.5×1.2/x/2×2.0); = 0.60	1 001 24207	· · · · · · · · · · · · · · · · · · ·	F. g.g Wet stone masonry.	=1(0.9x2.6/x1.8}. = 1.872.	1(0.4x1.6/x1.0)	160-9×1-9/x2-63×2.	16.0×1.0/×0.7/	7077		
OF WOOV.		LOCATION: WF. 176 R + 27. (R.18)				2.7	2.0		(\frac{\gamma_{\gamma}}{\gamma}	.07	2.0				2.6	977	12.0						The second secon

TYPE OF WORK : OUTLET STRUCTURE .

LOCATION

CALCULATION	RESULT
F. q. Connecting Channel and Drainage pipe.	
F.q.1. Structural Excavation	16.25 m3.
F.q.2. BackFill with selected soil.	12.83 m
F. 9.3. Gravel Bedding.	1.89 m
F. q. q. Wet stone masonry.	6,928 m3
F. 9.5 Mortar Plastering on Surface of wetstone masonry.	13.12 M2
F. 4.6. Furnishing and Installing Concrete pipe Dia 600 mm	2.35 M
F. 4.7. Concrete type D Including Formwork	1.37 m3
F. 4.8. Concrete type CI Including Formwork	
F. 9. Deformed Reinforcing Bars	

OUTLET STRUCTURE CALCULATION F WF.176 R + 91	RESULT			16.25 m3					12.83 m					1.89 m3				The state of the s		6.928 m		
125 176 R + 91 (R.19) WE.176 R + 91 (R.19) 125 125 125 125 125 125 125 125 125 125		`	structural	1/2.0+5.5/x1/2x2.5/x2.0 = 16.25.		4.2 BackFill With Selected	=1/20+217/x1/2×0.7/x20x2	fi.	12.83		F-9-3 grovel Bedding	1/20x2-0/x0-1/	x 2.0 }	1.84	4.9 Wet	l (ij	2	C)	6.928		· · · · · · · · · · · · · · · · · · ·
	67	(R.19)				-	2.0		2.0	317		†			3.2		1,0				1.90	A Service Se

TYPE OF WORK: OUTLET STRUCTURE	CALCULATION	RESULT
LOCATION: WF.176 R + 41 (R.19)	F.g.s Mortar Plastering on curtace of wet-	
7. 9.5 H + H A.K	stone masonsy.	
9.7	A=41.0x2.8/+(0.7×1.8/f. = 3.52	
7.2	411.6×1.8/+(1.2×1.0/+(0.9×1.9/1×2 = 8.00	
.009\$	161.2×1.0/+(0.4×1.0)} = 1.60	
2.35	TOTAL = 13.12	13.12 m
7.0.		
19.7	F. 4-6. Furnishing and Installing Concrete paper	
	DIA. 600 mm.	2.35 m
	F. 4.7 Concrete type D. Including Formwork	
	V=1/0.9×0.9/-(24,×0.32/5×2.15 = 1,133	
	160.1×1.1/x 2-1.5% = 0.237.	
	70796 1.370	1.370 m
	F. J. 8. Concrete type CI Including Formwork	
	- NOTHING -	1
	F. 4-9. Deformed Reinforcing Bars.	
	- NOTHING -	-
「おおからない。」というではないできます。 日本のはまたはまたはまたはまでは、このは、日本のではない。 いいかい かいかい かいかい かいかい かいかい かいかい しゅうしゅう しゅうしゅうしゅう しゅうしゅう		