

4.2 Ground Sill without Head at WF.173

TYPE OF WORK :

LOCATION : GROUNDSILL WITHOUT HEAD AT WF.173

CALCULATION			RESULT
<b>☐ STRUCTURAL EXCAVATION</b>			
$V_1 = 32.70 \times (1.50 + 1.50) \times 2$	=	196.20	
$V_2 = 25.6 \times (12.50 + 9.00) \times 2$	=	1100.80	
$V_3 = 6.00 \times (3.00 + 3.00) \times 2$	=	72.00	
$V_4 = 50.30 \times 3.50 \times 2$	=	352.10	
TOTAL			
	=	1721.10	1721.10 m <sup>3</sup>
<b>☐ BACKFILL WITH SELECTED SOIL</b>			
$V_1 = (6.0 + 0.3) \times (1.50 + 1.50) \times 2$	=	37.80	
$V_2 = (7.6 + 0.3) \times (12.50 + 9.00) \times 2$	=	339.70	
$V_3 = 0.7 \times (3.00 + 3.00) \times 2$	=	4.20	
$V_4 = (4.9 + 0.3) \times 3.50 \times 2$	=	36.40	
$V_5 = 1.70 \times 39.0$	=	66.30	
TOTAL			
	=	484.40	484.40 m <sup>3</sup>

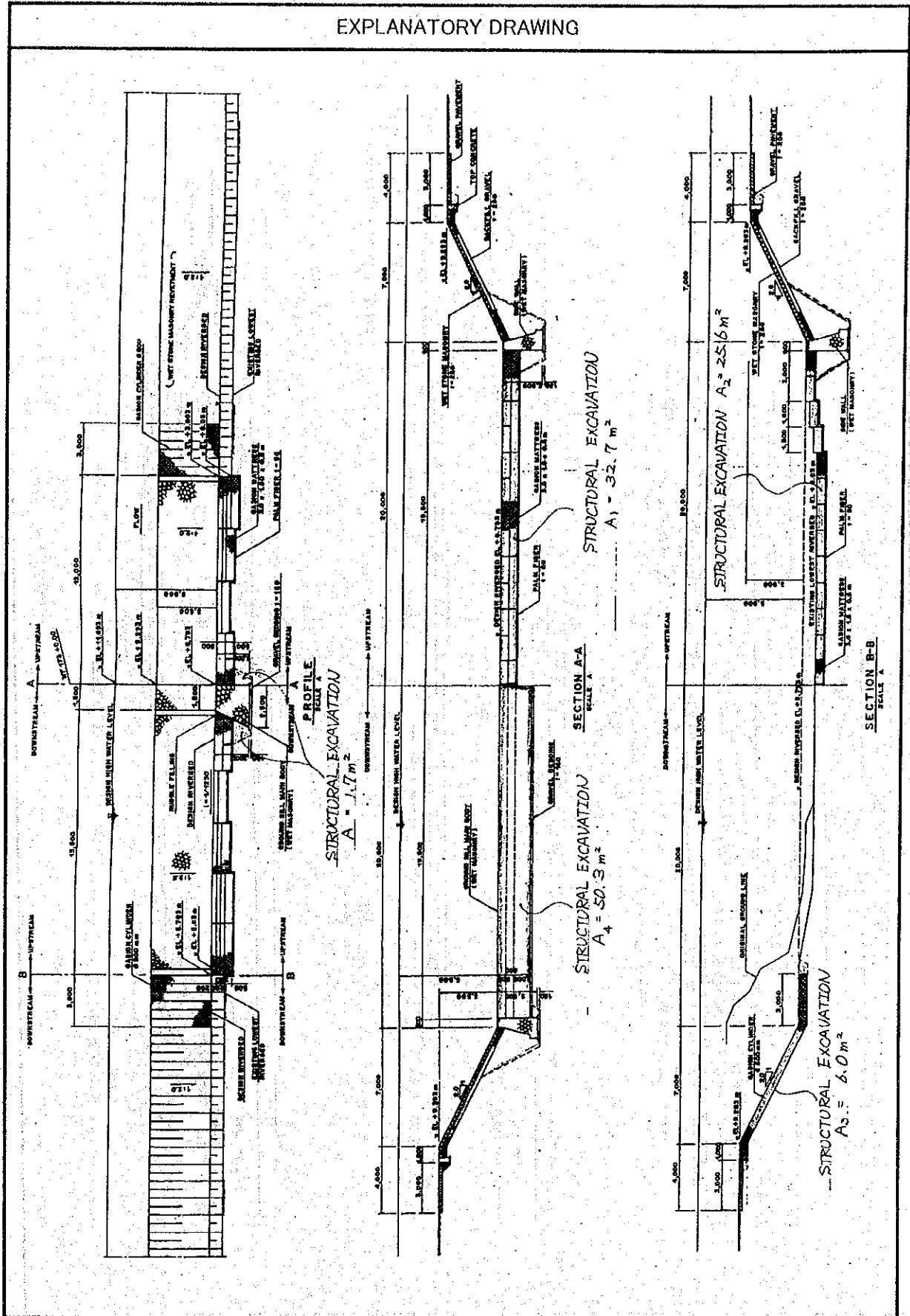
TYPE OF WORK

GROUND SILL WITHOUT HEAD AT WF. 173

LOCATION

STRUCTURAL EXCAVATION

EXPLANATORY DRAWING





TYPE OF WORK : MAIN BODY AND SIDEWALL  
 LOCATION : GROUNDSILL WITHOUT HEAD AT WF.173

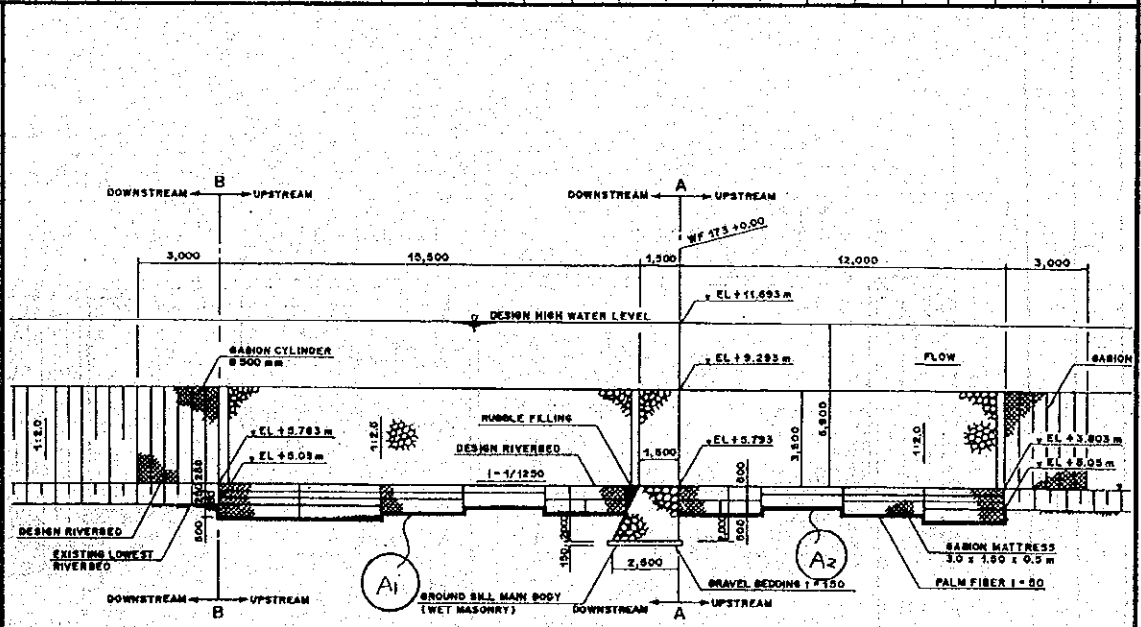
CALCULATION		RESULT
<b>☐ WET STONE MASONRY</b>		
$A_1 = (1.50 + 2.50) \times \frac{1}{2} \times 2.00$	=	4.000 m <sup>2</sup>
$V_1 = 4.00 \times 39.00$	=	156.000
$A_2 = (0.50 + 1.00) \times \frac{1}{2} \times 2.00 + (0.50 \times 1.20)$	=	2.10 m <sup>2</sup>
$V_2 = 2.10 \times 29.00 \times 2$	=	121.800
TOTAL (V <sub>1</sub> + V <sub>2</sub> )		= 277.800
		277.800 m <sup>3</sup>
<b>☐ GRAVEL BEDDING</b>		
$A_1 = 1.50 \times (2.50 + 0.15 \times 2)$	=	0.420 m <sup>2</sup>
$V_1 = 0.42 \times 39.00$	=	16.380
$A_2 = 0.15 \times (1.20 + 0.15 \times 2)$	=	0.225 m <sup>2</sup>
$V_2 = 0.225 \times (29.00 + 0.15 \times 2) \times 2$	=	13.185
TOTAL (V <sub>1</sub> + V <sub>2</sub> )		= 29.565
		29.565 m <sup>3</sup>

TYPE OF WORK : MAIN BODY  
 LOCATION : GROUNDSILL WITHOUT HEAD AT WF.173

CALCULATION		RESULT
<b>☐ RUBBLE STONE FILLING</b>		
$V = \frac{1}{2} \times 1.00 \times 0.50 \times 39.00$	= 9.750	9.750 m <sup>3</sup>
<b>☐ JOINT FILTER</b>		
t = 10, ELASTIC MATERIAL		
$A_1 = (1.50 + 2.50) \times \frac{1}{2} \times 2.00 \times 3$	= 12.000	
$A_2 = \{(0.50 + 1.00) \times \frac{1}{2} \times 2.00 + (0.50 \times 1.20) \times 2$	= 4.200	
TOTAL	= 16.200	16.200 m <sup>2</sup>
<b>☐ GABION MATRESS</b>		
t = 500		
$V_1 = 40.00 \times 6.00 \times 0.50$	= 120.000	
$V_2 = 40.00 \times 3.00 \times 0.50 \times 2$	= 120.000	
$V_3 = 40.00 \times 3.00 \times 0.50 \times 2$	= 120.000	
$V_4 = 40.00 \times 3.00 \times 1.00 \times 2$	= 240.000	
$V_5 = 40.00 \times 3.00 \times 0.50$	= 60.000	
TOTAL	= 660.000	660.000 m <sup>3</sup>

GROUND SILL WITHOUT HEAD AT WF.173

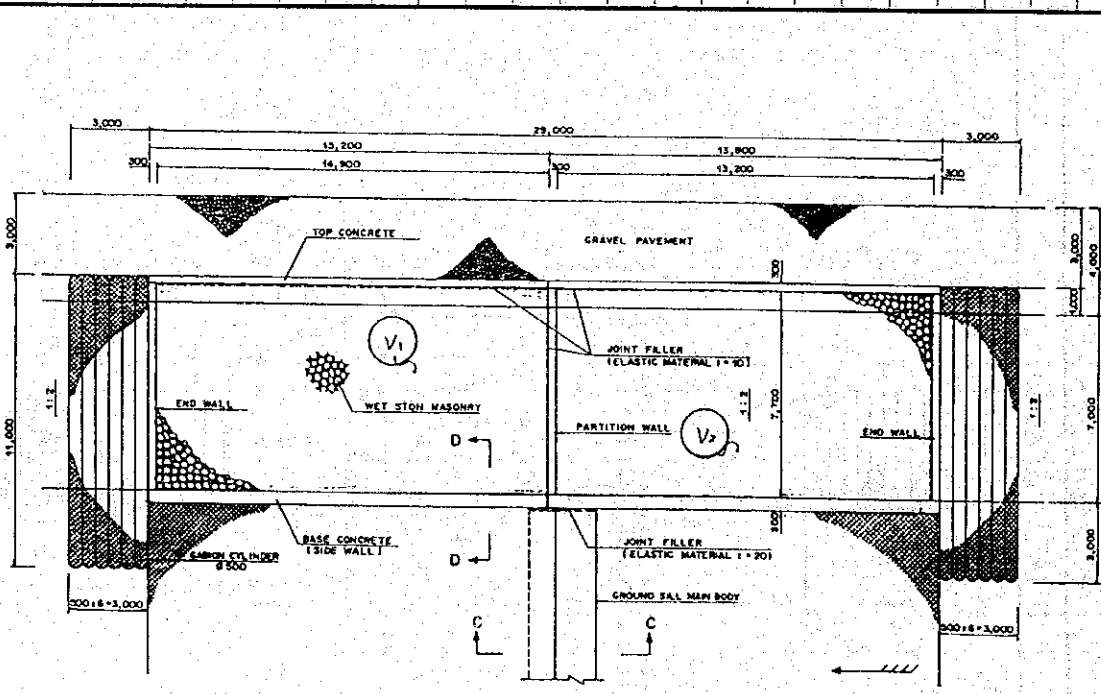
TYPE OF WORK :	LOCATION :	RESULT
PALM FIBER FILTER	MAIN BODY	
	$A_1 = 40.00 \times (15.50 + 0.247 + 0.25 + 0.26 + 0.25)$ $= 660.280$	
	$A_2 = 40.00 \times (12.00 + 0.247 + 0.24 + 0.25 + 0.25)$ $= 519.480$	
	$TOTAL = 1179.760$	1179.760 m <sup>2</sup>



PROFILE SCALE A

GROUND SILL WITHOUT HEAD AT WF. 173

TYPE OF WORK :	WET STONE MASONRY	CALCULATION	RESULT
LOCATION :	REVETMENT	t = 250	
		$V_1 = (7.70 \times 1.118 + 0.70) \times 14.90 \times 0.25 \times 2$ $= 69.349$	
		$V_2 = (7.70 \times 1.118 + 0.70) \times 13.20 \times 0.25 \times 2$ $= 61.437$	
		TOTAL = 130.786	130.786 m <sup>3</sup>



PLAN OF REVETMENT  
SCALE: A

TYPE OF WORK : REVETMENT  
 LOCATION : GROUNDSILL WITHOUT HEAD AT WF.173

CALCULATION		RESULT
<b>☐ GRAVEL BEDDING</b>		
t = 250		
$V_1 = (7.70 \times 1.118 + 0.70) \times 14.90 \times 0.25 \times 2$	=	69.349
$V_2 = (7.70 \times 1.118 + 0.70) \times 13.20 \times 0.25 \times 2$	=	61.437
TOTAL		= 130.786
		130.786
<b>☐ CEMENT MORTAR POINTTING</b>		
$A_1 = (7.826 + 1.00) \times 14.90 \times 2$	=	263.015
$A_2 = (7.826 + 1.00) \times 13.20 \times 2$	=	233.006
TOTAL		= 496.021
		496.021 m <sup>2</sup>



TYPE OF WORK : REVETMENT  
 LOCATION : GROUNDSILL WITHOUT HEAD AT WF.173

CALCULATION	RESULT
<b>☐ TOP CONCRETE</b>	
t = 29.00 m / side	
• CONCRETE (TYPE - C1)	
V = 29.00 x 2 x 1.80 / 10.00 m = 10.440	10.440 m <sup>3</sup>
• GRAVEL BEDDING	
V = 29.00 x 2 x 0.75 / 10.00 m = 4.350	4.350 m <sup>3</sup>
• FORM (H < 4.0 m)	
V = 29.00 x 2 x 12.18 / 10.00 m = 70.644	70.644 m <sup>2</sup>
• REINFORCING BAR	
W = 29.00 x 2 x 0.094 / 10.00 = 0.545	0.545 tf
• JOINT FILTER	
A = 29.00 x 2 x 2.605 / 10.00 = 15.109	15.109 m <sup>2</sup>
<b>☐ GABION CYLINDER</b>	
• GABION CYLINDER (Ø 500)	
L = 3.00 + 7.00 x 1.118 + 1.00 = 11.826	
V = π / 4 x 0.50 <sup>2</sup> x 11.826 x 6 x 2 x 2 = 55.729	55.729 m <sup>2</sup>
• SOIL FILLING	
V = (11.826 x 0.50 x 3.00) x 2 x 2 - 55.729 = 15.227	15.227 m <sup>3</sup>

TYPE OF WORK : END WALL (REVETMENT)  
 LOCATION : GROUNDSILL WITHOUT HEAD AT WF.173

CALCULATION		RESULT
<b>▣ GRAVEL BEDDING</b>		
A = $(0.10 + 0.30) \times \frac{1}{2} \times 0.20 + (0.40 \times 0.10)$		= 0.080 m <sup>2</sup>
V = $0.080 \times (7.818 + 0.558) \times 2 \times 2$		= 2.680
		2.680 m <sup>3</sup>
<b>▣ CONCRETE (TYPE - C1)</b>		
(TYPE - C1)		
A = $(7.826 + 7.818) \times \frac{1}{2} \times 0.60 + (0.70 + 0.558) \times \frac{1}{2} \times 0.60$		= 5.071 m <sup>2</sup>
V = $5.071 \times 0.30 \times 2 \times 2$		= 6.058
		6.058 m <sup>3</sup>
<b>▣ FORM (H &lt; 4.0 m)</b>		
(H < 4.0 m)		
A = $\frac{\{(7.826 + 7.818) \times \frac{1}{2} \times 0.60 + (0.70 + 0.588) \times \frac{1}{2} \times 0.60\}}{2 \times 2 \times 2}$		= 40.565
		40.565 m <sup>2</sup>

TYPE OF WORK : END WALL (REVETMENT)  
 LOCATION : : GROUNDSILL WITHOUT HEAD AT WF.173

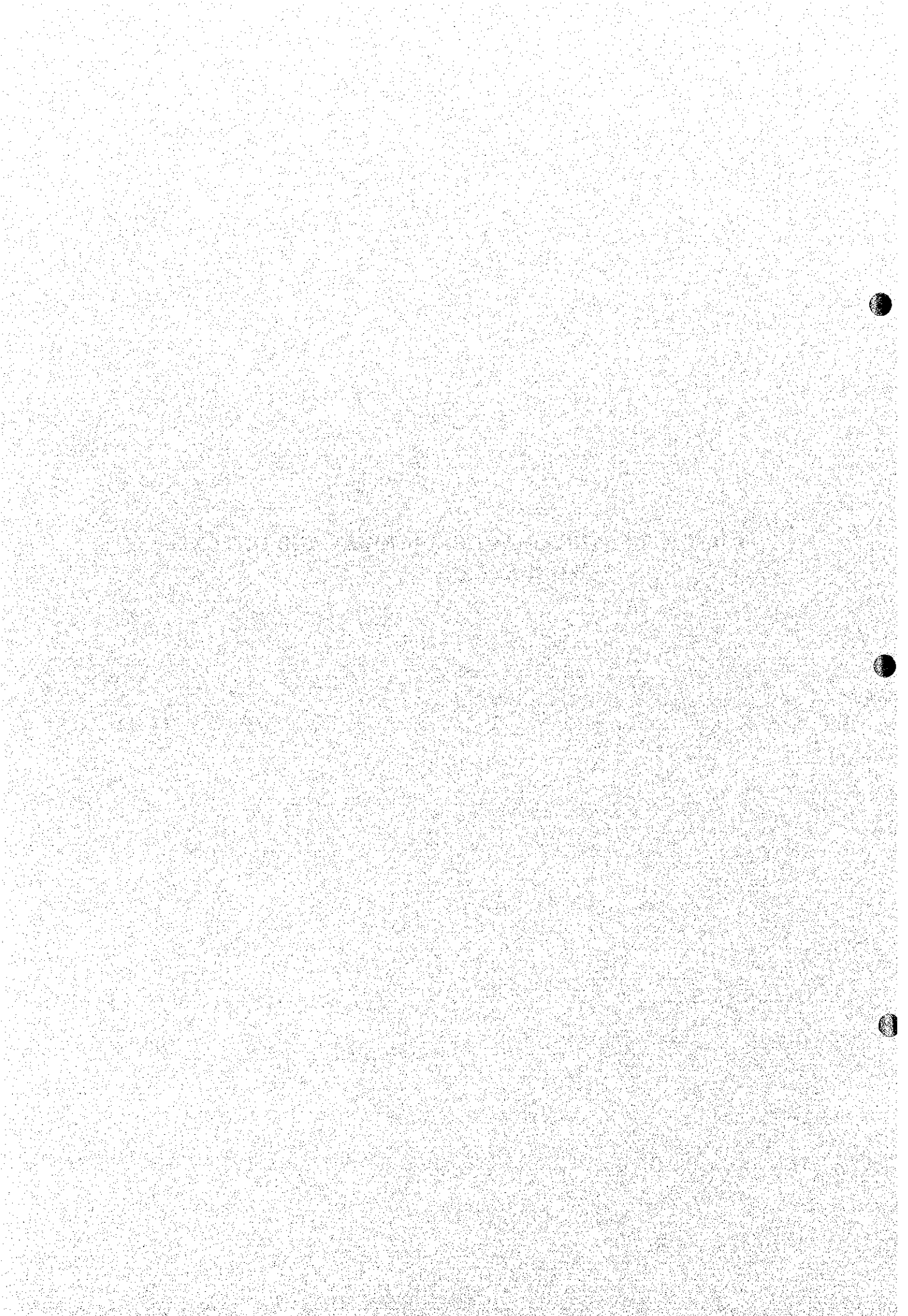
CALCULATION		RESULT
<b>REINFORCING BAR</b>		
• D 13 (W = 1.04 kgf/m)		
n =	6 Bars	
L =	$(7.826 + 0.70) - 0.05 \times 2$	= 8.426 m / Bar
W <sub>1</sub> =	6 Bars x 8.426 x 1.04 x 2 X 2	= 210.313
• D 10 (W = 0.617 kgf/m)		
n =	$(8.426 : 0.30) + 1$	= 29.09 ≈ 30 Bars
L =	$(0.20 \times 2 + 0.50 \times 2 + 15 \times 0.01)$	= 1.550 m / Bar
W <sub>2</sub> =	30 Bars x 1.55 x 0.617 x 2 x 2	= 114.762
TOTAL (W <sub>1</sub> + W <sub>2</sub> )		= 325.075 kgf
		0.325 tf
<b>JOINT FILTER</b>		
t = 10, ELASTIC MATERIAL		
A =	$(7.826 + 0.70) \times 0.25 \times 2 \times 2$	= 8.526
		8.526 m <sup>2</sup>



TYPE OF WORK : PARTITION WALL (REVETMENT)  
 LOCATION : GROUND SILL WITHOUT HEAD AT WF.173

CALCULATION		RESULT
<b>REINFORCING</b>		
• D 13 (W = 1.04 kgf/m)		
n =	6 Bars	
L =	$(7.826 + 0.70) - 0.05 \times 2$	= 8.426 m / Bar
W <sub>1</sub> =	6 Bars x 8.426 x 1.04 x 2	= 105.156
• D 10 (W = 0.617 kgf/m)		
n =	$(8.426 : 0.30) + 1$	= 29.09 ≅ 30 Bars
L =	$(0.20 \times 2 + 0.40 \times 2 + 15 \times 0.01)$	= 1.350 m / Bar
W <sub>2</sub> =	30 Bars x 1.35 x 0.617 x 2	= 49.977
TOTAL (W <sub>1</sub> + W <sub>2</sub> )		= 155.133 kgf
		0.155 tf
<b>JOINT FILTER</b>		
t = 10, ELASTIC MATERIAL		
A =	$(7.826 + 0.70) \times 0.25 \times 2$	= 4.263
		4.263 m <sup>2</sup>

**CHAPTER 5 DRAINAGE SLUCEWAY AND OUTLET  
WORKS**



5.1 Drainage Sluiceway at WF.172R+15m

TYPE OF WORK : DRAINAGE SLUICEWAY AT WF.172R+15 m  
 LOCATION :

CALCULATION		RESULT
<b>☐ DEMOLITION AND REMOVAL OF EXISTING STRUCTURE</b>		
$V_1 = \{(1.70 \times 1.80) - (1.10 \times 1.10)\} \times 17.50$	=	32.375
$V_2 = \{(1.50 \times 1.40) - (0.90 \times 0.70)\} \times 19.50$	=	28.665
$V_3 = (1.00 \times 0.50) \times 65.00$	=	32.500
$V_3 = \{(1.50 \times 1.50) - (0.90 \times 0.80)\} \times 11.50$	=	17.595
$V_4 = \{(1.70 \times 1.50) - (0.90 \times 1.00)\} \times 12.00$	=	19.800
$V_5 = 2.00 \times 1.00 \times 11.00$	=	22.000
<b>TOTAL</b>	=	152.938
		153.0 m <sup>3</sup>
<b>☐ CLEARING AND GRUBBING</b>		
$A_1 = 5.20 \times (18.70 + 3.00)$	=	112.840
$A_2 = 3.00 \times 16.70$	=	50.100
$A_3 = 4.778 \times 16.70$	=	79.793
$A_4 = (16.70 \times 1.00) + (16.70 + 14.20) \times \frac{1}{2} \times 3.70$	=	73.865
$A_5 = 14.20 \times 3.155$	=	44.801
$A_6 = (5.00 + 3.00) \times \frac{1}{2} \times 5.00$	=	20.000
$A_7 = 1.27 \times 4.00$	=	5.080
$A_8 = 2.00 \times 55.00$	=	110.00
$A_9 = 1.27 \times 3.50$	=	4.445
$A_{10} = 1.27 \times 2.50$	=	3.175
<b>TOTAL</b>	=	504.099
		504.0 m <sup>2</sup>



DRAINAGE SLUICeway AT WF.172R+15M

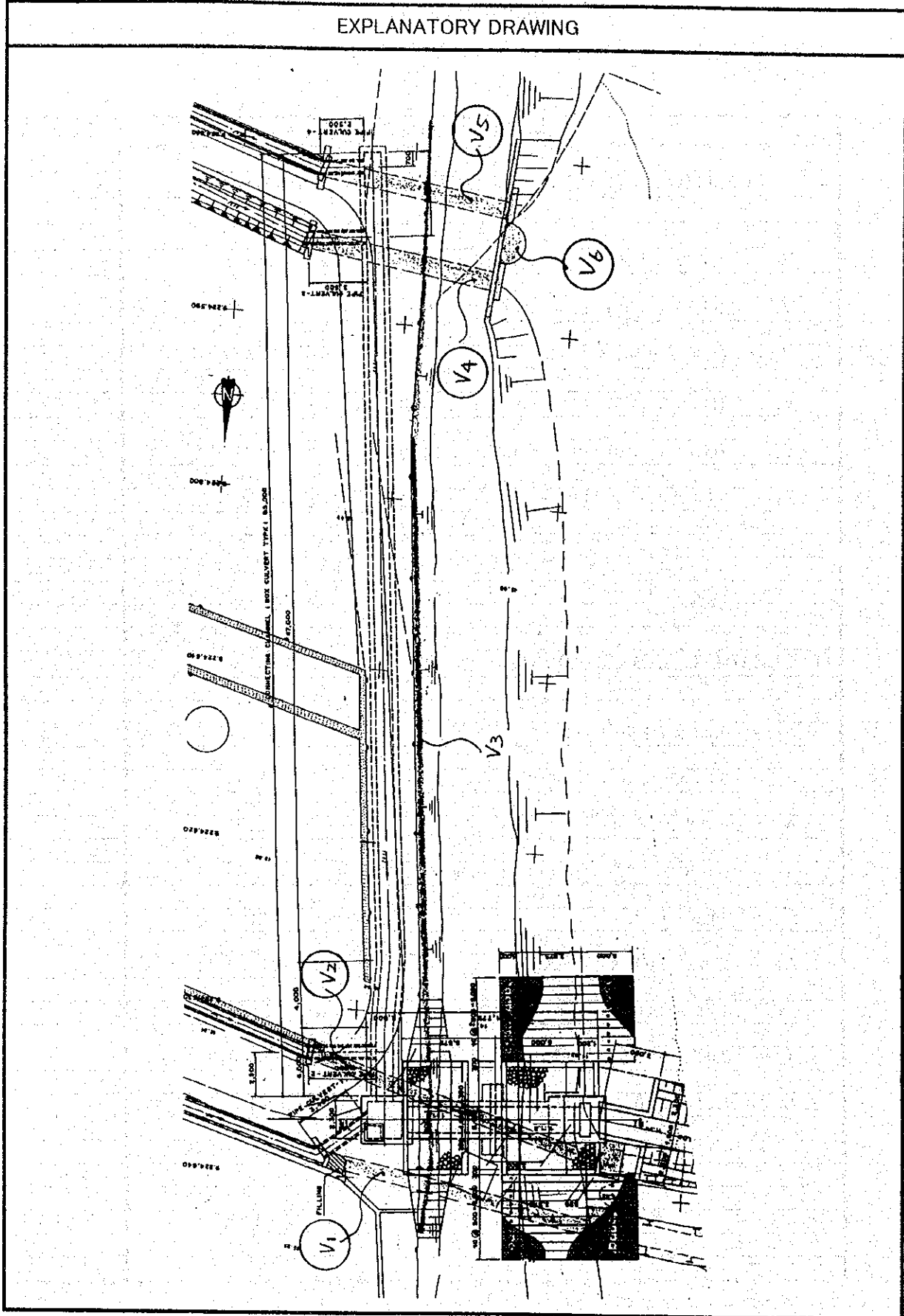
TYPE OF WORK

: DEDOLITION AND REMOVAL OF EXISTING STRUCTURES

LOCATION

:

EXPLANATORY DRAWING

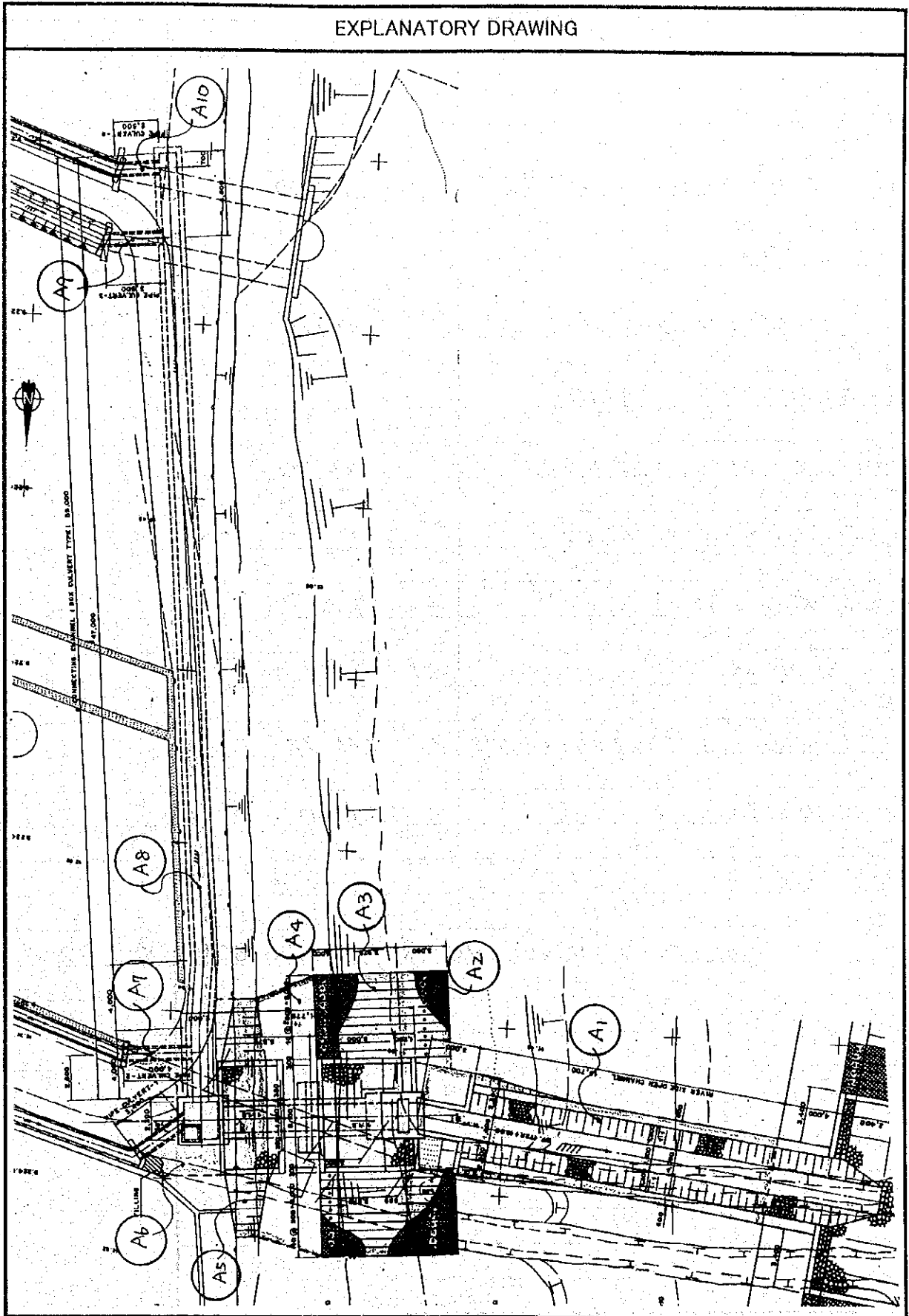


TYPE OF WORK :

DRAINAGE SLUCEWAY AT WF. 172R + 15M  
CLEARING AND GRUBBING

LOCATION :

EXPLANATORY DRAWING



DRAINAGE SLUICeway AT WH. 172R+15m

TYPE OF WORK

: STRUCTURAL EXCAVATION

LOCATION

: MAIN STRUCTURE

CALCULATION	RESULT
$A_1 = 27.60 \text{ m}^2$	
$V_1 = 27.60 \times (2.341 + 3.659) \times \frac{1}{2} = 82.80$	
$A_2 = 26.40 \text{ m}^2$	
$V_2 = 26.40 \times (0.50 + 1.00) = 39.60$	
$A_3 = 23.50 \text{ m}^2 \times 2 = 47.00$	
$V_3 = (47.00 + 61.00) \times \frac{1}{2} \times 4.20 = 226.80$	
$A_4 = 30.50 \text{ m}^2 \times 2 = 61.00$	
$V_4 = 61.00 \times 1.30 = 79.30$	
$A_5 = 31.10 \text{ m}^2$	
$V_5 = (23.50 + 31.10) \times 2 \times \frac{1}{2} \times 4.775 = 260.72$	
$V_6 = 31.10 \times 3.00 \times 2 = 9186.60$	
$TOTAL (V_1 + V_2 + V_3 + V_4 + V_5 + V_6) = 875.82$	875.82 m <sup>3</sup>



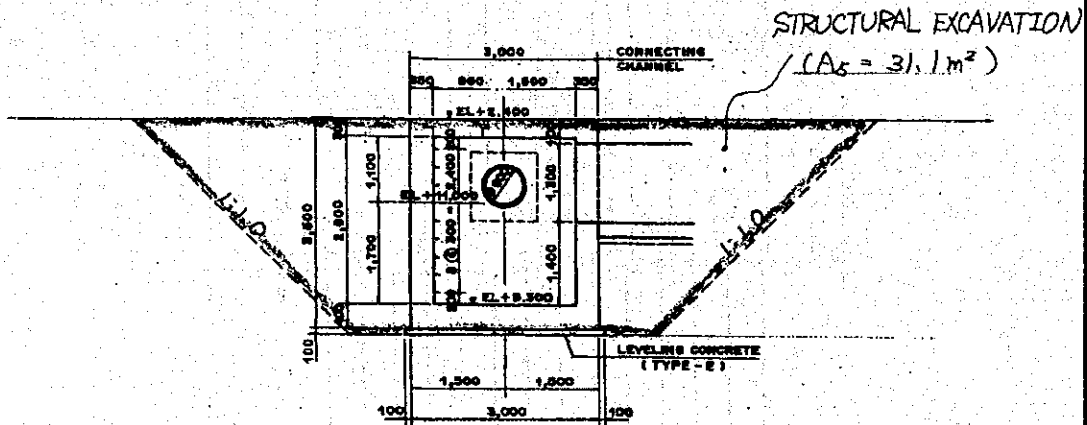
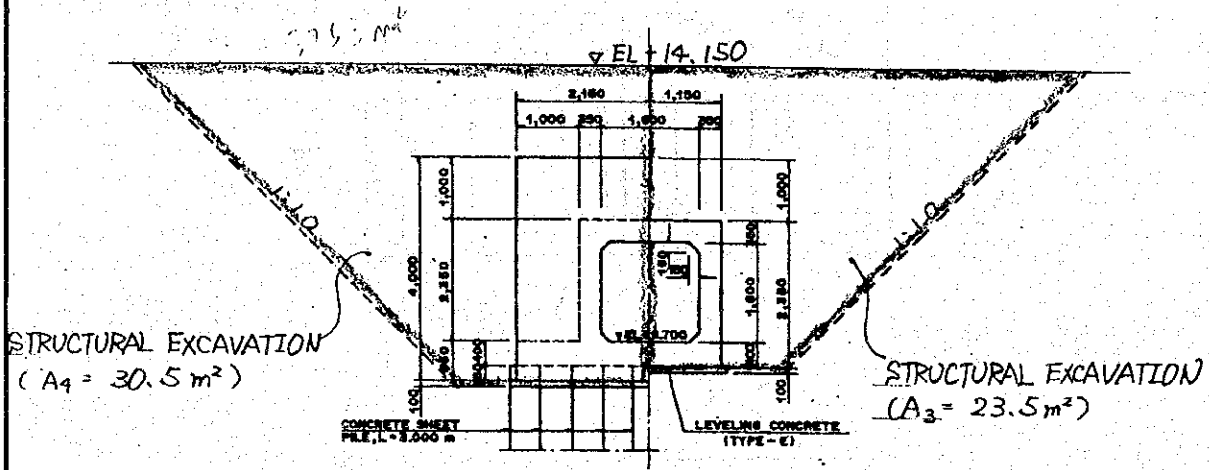
DRAINAGE SLUCEWAY AT WF. 172R + 15m

TYPE OF WORK : STRUCTURAL EXCAVATION

LOCATION : MAIN STRUCTURE

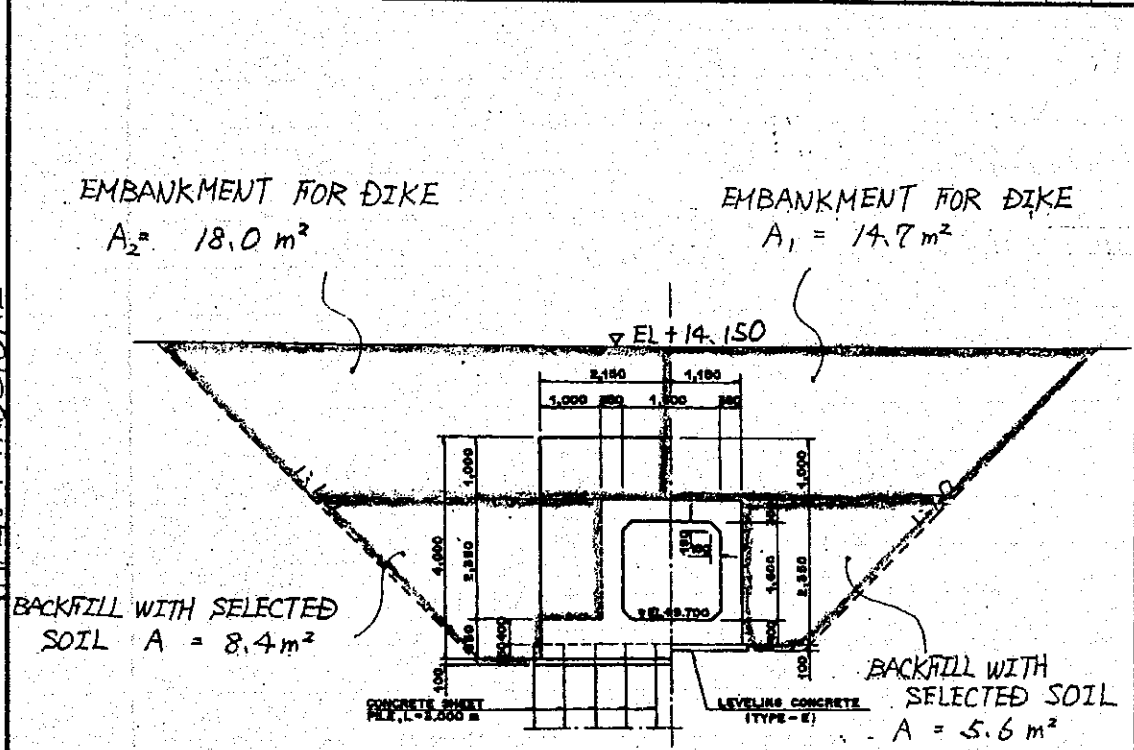
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EXPLANATORY DRAWING



DRAINAGE SLUICeway AT WF 172R + 15m

TYPE OF WORK:	EMBAKMENT FOR DIKE	CALCULATION	RESULT
LOCATION:	MAIN STRUCTURE		
	<p>EMBANKMENT FOR DIKE  <math>A_2 = 18.0 \text{ m}^2</math></p>	$V_1 = \frac{1}{3} \times 14.70 \times 2 \times 3.75 = 36.75$	
		$V_2 = 14.70 \times 2 \times 1.30 = 38.22$	
		$V_3 = 18.00 \times 2 \times 1.30 = 46.80$	
		$V_4 = 14.70 \times 2 \times 1.40 = 41.16$	
		$V_5 = \frac{1}{3} \times 14.70 \times 2 \times 3.75 = 36.75$	
		(Deduction for Revetment)	
		$V_6 = -(3.466 + 2.493) \times 6.70 = -39.93$	
		(Deduction for Gabion Cylinder)	
		$V_7 = -(4.778 + 1.00) \times 0.50 \times 5.00 \times 2 = -28.89$	
		TOTAL = 130.86	130.86 m <sup>3</sup>



TYPE OF WORK  
LOCATION

DRAINAGE SLUICeway AT WF. 172R + 15m  
: BACKFILL WITH SELECTED SOIL  
: MAIN STRUCTURE

CALCULATION	RESULT
$A_1 = 9.50 \text{ m}^2$	
$V_1 = 9.50 \times 2 \times (2.341 + 3.659) \times \frac{1}{2} = 57.00$	
$A_2 = 8.30 \text{ m}^2$	
$V_2 = 8.30 \times 2 \times (0.50 + 1.00) = 24.90$	
$A_3 = 5.60 \text{ m}^2 \times 2 = 11.20$	
$V_3 = (11.20 + 16.80) \times \frac{1}{2} \times 4.20 = 58.80$	
$A_4 = 8.40 \text{ m}^2 \times 2 = 16.80$	
$V_4 = 16.80 \times 1.30 = 21.84$	
$A_5 = 2.60 \text{ m}^2$	
$V_5 = 2.60 \times 3.00 = 7.80$	
$A_6 = 10.0 \text{ m}^2$	
$V_6 = (11.20 + 20.00) \times \frac{1}{2} \times 4.775 = 74.49$	
$V_7 = 10.00 \times (3.00 + 3.00) = 60.00$	
TOTAL ( $V_1 + V_2 + V_3 + V_4 + V_5 + V_6 + V_7$ ) = 304.83	304.83 m <sup>3</sup>

DRAINAGE SLUICeway AT WF. 172R +15M

TYPE OF WORK

: BACKFILL WITH SELECTED SOIL

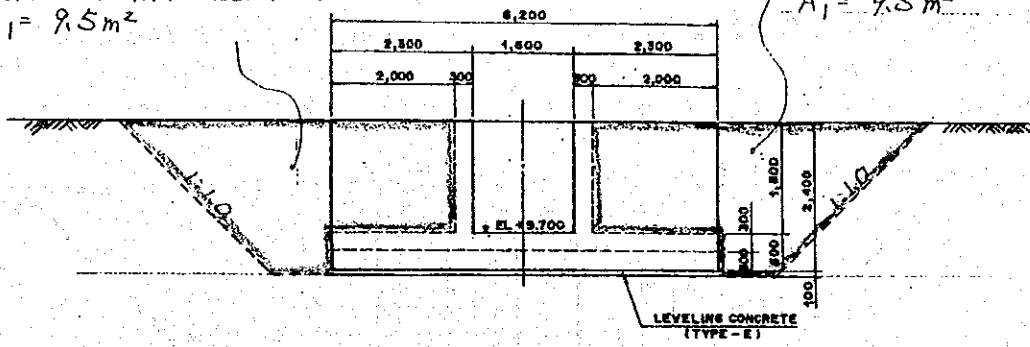
LOCATION

: MAIN STRUCTURE

(1/2)

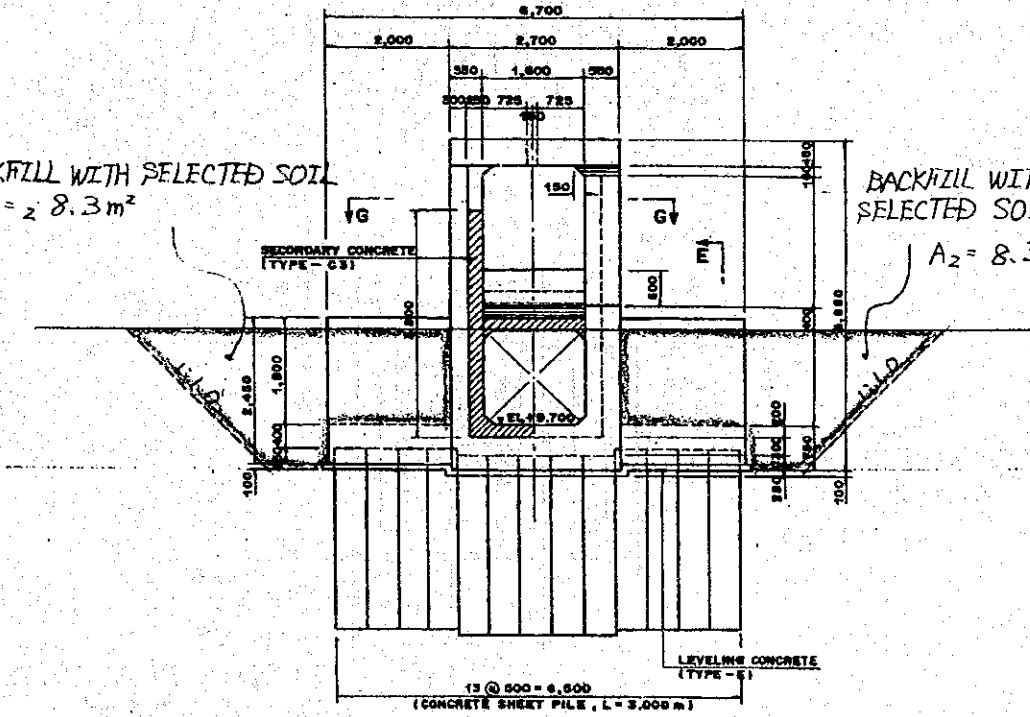
EXPLANATORY DRAWING

BACKFILL WITH SELECTED SOIL  
 $A_1 = 9.5 m^2$



BACKFILL WITH SELECTED SOIL  
 $A_1 = 9.5 m^2$

BACKFILL WITH SELECTED SOIL  
 $A_2 = 8.3 m^2$



BACKFILL WITH SELECTED SOIL  
 $A_2 = 8.3 m^2$



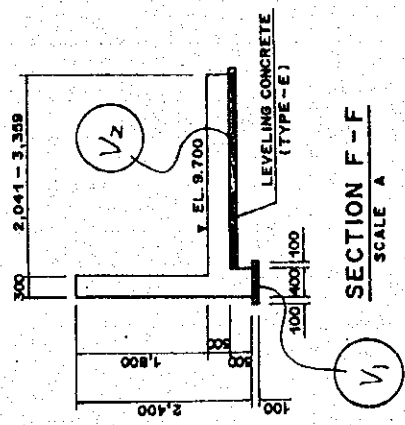






DRAINAGE SLUICE WAY AT W.F. 172R+15m

TYPE OF WORK :	CALCULATION	RESULT
LEVELING CONCRETE		
WING WALL	(TYPE - E)	
	$V_1 = 0.60 \times (6.20 + 0.10 \times 2) \times 0.10 = 0.384$	
	$V_2 = (3.380 + 1.920) \times \frac{1}{2} \times (6.20 + 0.10 \times 2) \times 0.10$	
	$= 1.664$	
	TOTAL = 2.048	2.048 m <sup>3</sup>



DRAINAGE SLUICeway AT WF. 172R +15 m

TYPE OF WORK

: FORM

LOCATION

: WING WALL

CALCULATION	RESULT
$(H < 7.0m)$	
$A_1 = 0.60 \times 6.20$	$= 3.720$
$A_2 = 2.30 \times 1.80 \times 2$	$= 8.280$
$A_3 = 2.00 \times 1.80 \times 2$	$= 7.200$
$A_4 = 2.934 \times 1.80$	$= 5.281$
$A_5 = 3.170 \times 1.80$	$= 5.706$
$A_6 = 2.830 \times 1.80$	$= 5.094$
$A_7 = 2.466 \times 1.80$	$= 4.439$
$A_8 = (2.341 \times 0.30) + (0.30 \times 0.40)$	$= 0.822$
$A_9 = (3.659 \times 0.30) + (0.30 \times 0.40)$	$= 1.218$
$A_{10} = 0.30 \times 1.80 \times 2$	$= 1.080$
$A_{11} = 0.20 \times 6.20$	$= 1.240$
$TOTAL = 44.080$	$44.080 \text{ m}^2$



DRAINAGE SLIDICEWAY AT WF. 172R + 15m

TYPE OF WORK:	LOCATION:	CALCULATION	RESULT
FORM FOR LEVELING CONCRETE	WING WALL	(CH < 4.0 m)	
		$A_1 = (6.20 + 0.10 \times 2) \times 0.10 \times 2$	= 1.280
		$A_2 = 0.60 \times 0.10 \times 2$	= 0.120
		$A_3 = (6.20 + 0.10 \times 2) \times 0.10$	= 0.640
		$A_4 = (6.339 + 0.10 \times 2) \times 0.10$	= 0.654
		$A_5 = 3.259 \times 0.10$	= 0.326
		$A_7 = 1.941 \times 0.10$	= 0.194
		TOTAL = 3.214	3.214 m <sup>2</sup>

