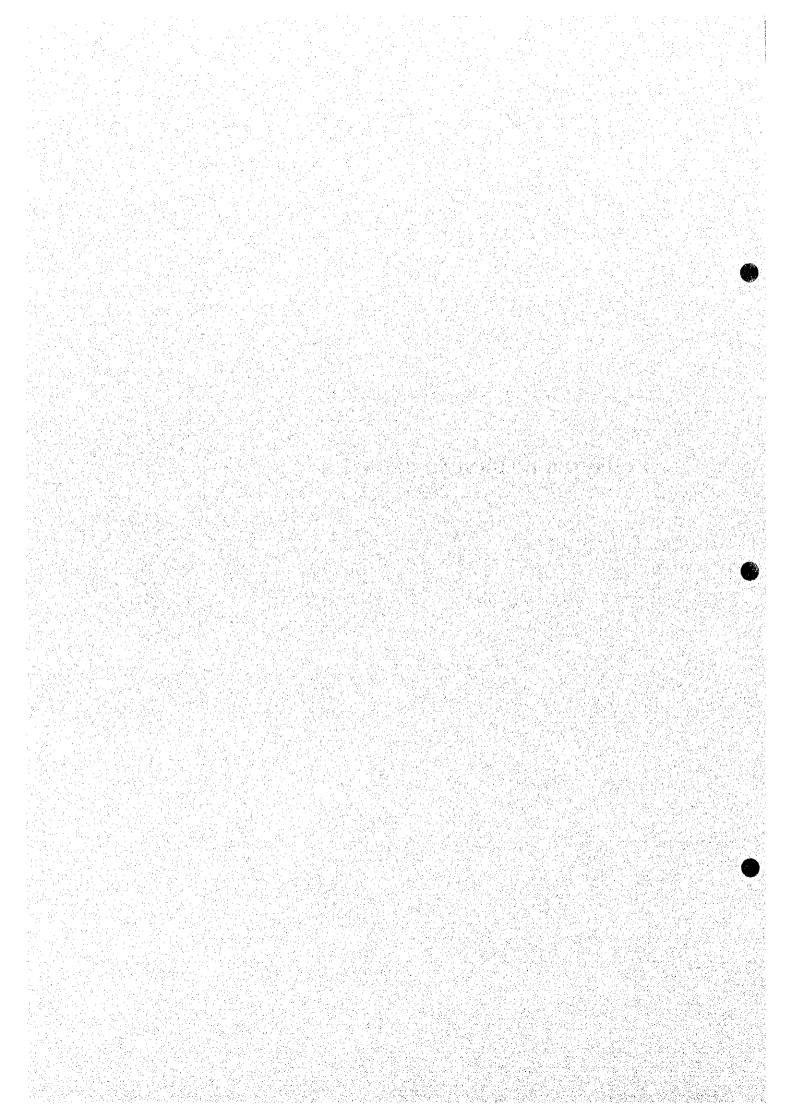
CHAPTER 4 GROUND SILL



4.1 Ground Sill with Head at WF. 124

TYPE OF WORK LOCATION

ON : GROUNDSILL WITH HEAD AT WF.124

CALCULATION	RESULT
□ STRUCTURAL EXCAVATION	
4.70	
$V_1 = 90.70 \times (0.75 + 1.50 + 1.375) \times 2 = 657.58$	
$V_2 = 59.10 \times (8.25 + 1.00) \times 2 = 546.68$	
V ₂ = 39.10 X (8.23 × 1.00) X 2	
$V_3 = 18.50 \times 18.625 \times 2 = 689.13$	
$V_4 = (59.1 + 21.50) \times 1/2 \times 10.00 \times 2 = 806.00$	
206.00	
$V_5 = 19.30 \times 10.00 \times 2 = 386.00$	
$V_6 = 11.20 \times 10.00 \times 2 = 224.00$	
V6 - 11.20 X 10.00 X 2	· · · · · · · · · · · · · · · · · · ·
$V_7 = (3.00 + 11.18 + 1.00) \times 3.00 \times 0.50 \times 2 = 45.54$	
	the second second
$V_8 = (3.00 + 7.826 + 1.00) \times 3.00 \times 0.50 \times 2 = 35.48$	
TO TAL = 3390.41	3390.41 m³
TO TAL = 3390.41	3390.41 111
□ BACKFILL WITH SELECTED SOIL	
$V_1 = (0.50 + 2.80) \times 39.40$ = 130.02	
$V_1 = (0.50 + 2.80) \times 39.40$ = 130.02	
$V_2 = (9.70 + 0.40) \times 10.50 \times 2 = 212.10$	
$V_3 = (0.80 + 0.40) \times 20.00 \times 2$ = 48.00	
	<u> </u>
$V_4 = (97.0 + 2.0) \times \frac{1}{2} \times 10.00 \times 2 + 0.40 \times 10.00 \times 2$ = 125.00	
$V_5 = (0.80 + 0.40) \times 10.00 \times 2 = 24.00$	
$V_6 = (1.10 + 0.40) \times 10.00 \times 2 = 30.00$	* <u> </u>
	* · · · · · · · · · · · · · · · · · · ·
	570 103
TO TAL = 569.120	569.12 m ³

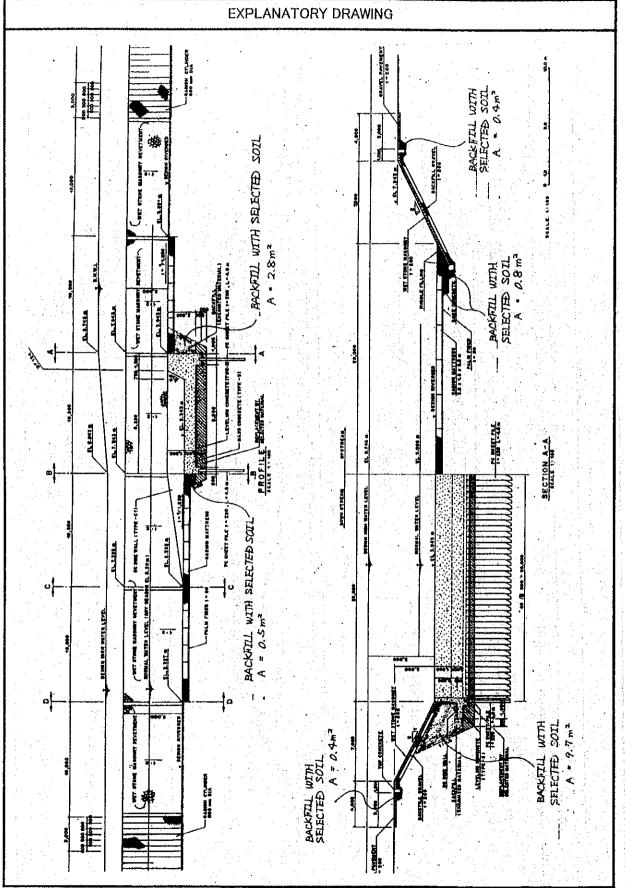
TYPE OF WORK

:_STRUCTURAL EXCAVATION LOCATION (1/2) **EXPLANATORY DRAWING** A3= 18.5 m2 STRUCTURAL EXCAUATION A = 2.0 m2

:_STRUCTURAL EXCAVATION

LOCATION $(\frac{2}{2})$ **EXPLANATORY DRAWING** VET STORE MASON STRUCTURAL EXCAVATION A = 19.8 m2 MUNICATERNAS WAL FIRICHIPPAL EXCAVATION STRUCTURAL EXCAUATION $A = 3/.5 m^2$ STRUCTURAL EXCAUATION A . 11.2 m2

TYPE OF WORK BACKFILL WITH SELECTED SOIL LOCATION $(\frac{1}{2})$ **EXPLANATORY DRAWING**



*

LOCATION $(\frac{2}{2})$ **EXPLANATORY DRAWING**

Γ-			1	·				-	,	 	•			,	γ	•						
F = 1000	NESOL 1					,									551.699 m							
124 CALCUI ATION	TYDE TO		V1 = 1.50 × 0.60 × 39.40 = 35.460		V2 = 1,00 × 9.90 × 39.90		$\sqrt{3} = 1.00 \times 0.50 \times 39.40$ = 19.700		14 = (1.50 + 2.25) × 1/2 × 1.50 × 39.40 = 110.813	(Deduction for PC Sheet Pile)		Vs = - (0.22×0.25) x 39.40 x 2 = -4.334			707AL = 551,699							
GROUND SILL WITH HEAD AT WF. 124	MAIN BODY	1.7	1,500	6	Y					V_2	3,200	10.	500		1,000 1,500	7.9	100	И	500	000.8	 	
TYPE OF WORK:	LOCATION:									SE	CTI sc.	ON	D -	<u>D</u>								

TYPE OF WORK:	GROUND STIL WITH HEAD AT FORM	AT WF. 124 CALCULATION	RESULT
	MAIN BODY	CH < 4.0 m)	
	000'1 000	Ar = 1.50 × 39.40	
	1200		
	300 80	Az = 0.40 × 39.40 = 15.760	-
	000 A	A3 = 0.40 × 39.40 = 15.760	
		A4=3.00 × 39.40 = 118.200	
SE		AS = 150 x1.118 x 39,40 = 66,074	
CT I	8,250	(REVISING)	
ON			
D -	,,500	Ab = \ (0.60 × 1.50) + (1.00 × 9.90) + (0.50 × 1.50)	
D		+ (1.50+2.25)×1/2×1.50 { x 2	
	1,000 1,500	= 28.725	
	Ā <i>5</i>)		
	A ₃	(Beduction for PC Sheet Pile)	
· · · · · · · · · · · · · · · · · · ·	100		
:	1,50	An = - (0.22×0.25) × 2×2 = -0.220	
	The second in the plane of the second		
	100		
	000	TOTAL = 303,399 3	303, 399 m2
	poor's	<u> </u>	

· A

TYPE OF WORK

: CONCRETE

LOCATION : SIDE WALL

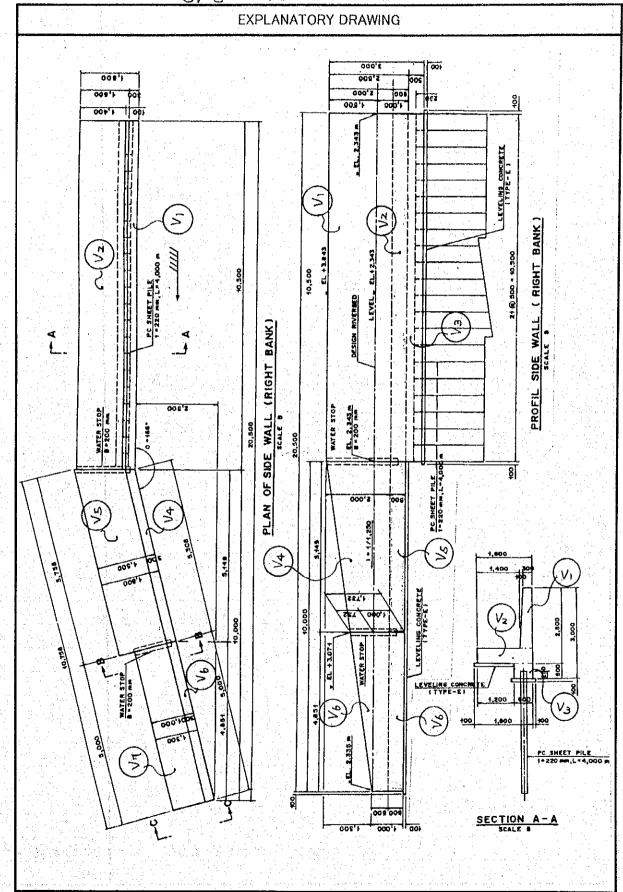
	RESULT
(TYPE - CI)	
$V_1 = (0.30 + 0.40) \times \frac{1}{2} \times 2.00 \times 10.50 \times 2 = 14.700$	
$V_2 = 0.50 \times 1.80 \times 10.50 \times 2 = 18.900$	
V3 = 0.60 × 0.50 × 10.50 × 2 = 6.300	
<u>。这样是一个的人,但是是某人。这些是是这种是一个人的地位是一种重要</u>	
V4 = (2.00 + 1.232) × 1/2 × 5.308 × 0.30 × 2 = 5.147	
15 = (5.308 + 5.758) ×/2 × 1.80 × 0.50 × 2 = 9.959	
$16 = (1.232 + 0.50) \times 1/2 \times 5.00 \times 0.30 \times 2 = 2.598$	
$l_7 = 1.30 \times 5.00 \times 0.50 \times 2 = 6.500$	
Deduction for PC Sheet Pile)	
V8 = -(0.22 × 0.25 × 10.50) × 2 = -1.155	
V8 = -(0.22 × 0.25 × 10.50) × 2 = -1.155	
V8 = -(0.22 × 0.25 × 10.50) × 2 = -1.155 70TAL = 62.949	62.949 m³
	62.949 m³
	62.949 m³
TOTAL = \$2.949	62.949 m³
TOTAL = 62.949	
TOTAL = \$2.949	
TOTAL = \$2.949	
70TAL = \$2.949	
TOTAL = \$2.949	
TOTAL = 62.949	
TOTAL = 62.949	
TOTAL = \$2.949	
TOTAL = 62.949	
TOTAL = 62.949	

TYPE OF WORK

CONCRETE (TYPE-CI)

LOCATION

: SIDE WALL



_ GROUND SILL WITH HEAD AT WF. 124 : FORM : SIDE WALL

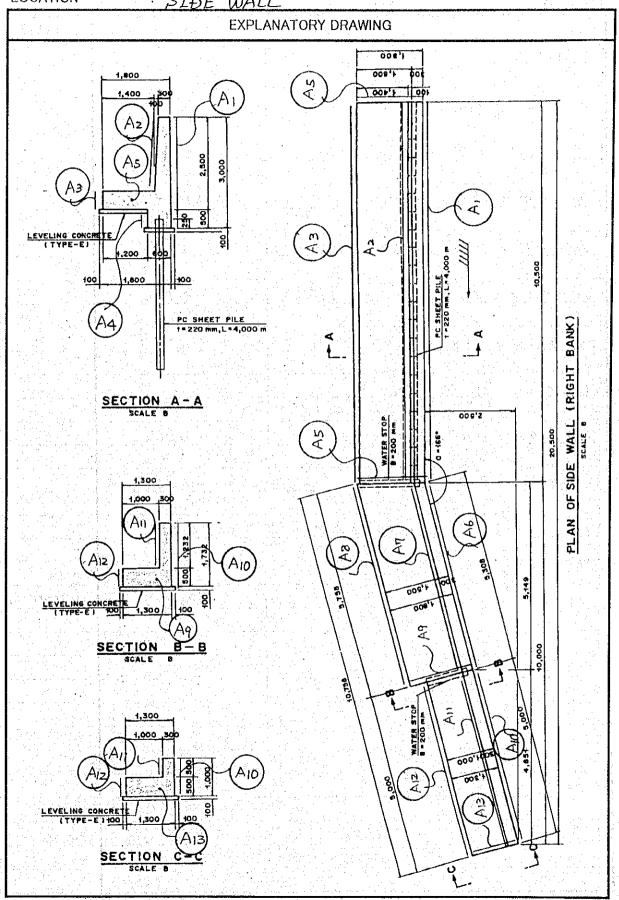
TYPE OF WORK

LOCATION

CALCULATION	RESULT
(H < 4.0 m)	
A) = 3.00 × 10.50 × 2 = 63.000	
$A_2 = \sqrt{3.00^2 + 0.10^2} \times 10.50 \times 2 = 63.035$	
$A_3 = 0.50 \times 10.50 \times 2 = 10.500$	
0.000	
$A_4 = 0.40 \times 10.50 \times 2$ = 8.400	
$A5 = \left\{ (0.30 + 0.40) \times \frac{1}{2} \times 2.00 + (1.80 \times 0.50) + (0.50 \times 0.60) \right\} \times 2 \times 2$	
713 - (CU.30+U,40)×/2×2·W (V.80 × U.30) 4(U,SU×U,BU) 1×2×2	
= 7.600	
$A6 = (2.50 +).732) \times \frac{1}{2} \times 5.308 \times 2 = 22.463$	
$A_7 = (2.00 + 1.232) \times /2 \times 5.308 \times 2 = 17.155$	
A8 = 0.50 × 5.758 × 2 = 5.758	
$A9 = \{(1.232 \times 0.30) + (0.50 \times 1.30)\} \times 2 = (2.039)$	
A10 = (1.732 + 1.00) × 1/2 × 5.00 × 2 = 13.660	
A10 = (1.732 + 1.00) × 1/2 × 5.00 × 2 = 13.660	
A11 = (1.232 + 0.50) × 1/2 × 5.00 × 2 = 8.660	
A12 = 0.50 x 5.00 x 2 = 5.000	
A13 - \((0.30 \times 0.50) + (0.50 \times 1.30)\{\times 2 = 1.600	
(Deduction for PC Sheet Pile)	
$A_{14} = -(0.22 \times 0.25 \times 2 \times 2) = -0.220$	
TOTAL = 228.650	228,650 m ²

FORM

LOCATION SIDE WALL



TYPE OF WORK

: LEVELING CONCRETE

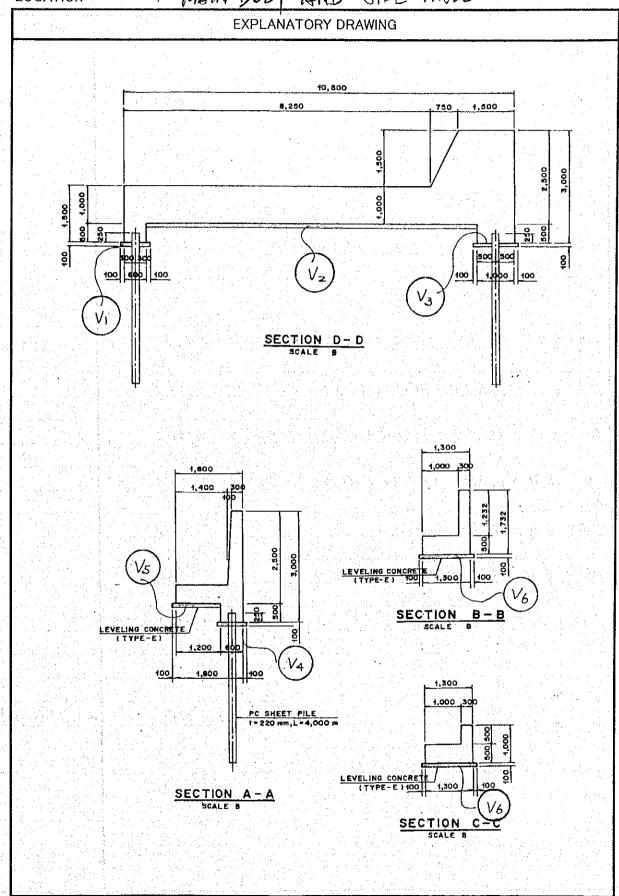
LOCATION : MAZN BODY AND SIDE WALL

CALCULATION CALCULATION	RESULT
(TYPE-E)	
$V_1 = 0.10 \times 0.80 \times 39.90$ = 3.152	
$V_2 = 0.10 \times 8.90 \times 39.40$ = 35.066	
$V_3 = 0.10 \times 1.20 \times 39.40 = 4.728$	
$V_4 = 0.10 \times 0.80 \times C(0.50 + 5.308 + 0.10) \times 2 = 2.545$	
- 하고의 회사로 가는 경기를 하는 경기를 참고하고 있습니다. 그는 경기를 하는 것으로 함께 되었다. 그런 그를 받는 것으로 하는 것으로 하는 것으로 함께 되었다. 그런 그를 받는 것으로 보다 그리 	
$V_{5} = 0.10 \times 1.30 \times (10.50 + 5.308 + 0.10) \times 2 = 4.136$	
$V6 = 0.10 \times 1.50 \times (5.00 + 0.10) \times 2 = 1.530$	
Deduction for PC Sheet Pile)	
$V_{q} = -(0.22 \times 0.10 \times 39.40) = -0.867$	
18 = - (0.22 × 0.10 × 39.40) = -0.867	
19 = - (0.22 × 0.10 × (10.50 + 5.308)) × 2 = -0.696	
70TAL = 48.727	48.727 m ³
가는 보고 하는 사람들은 경기 화장이 하고 있다. 그 사람이 있는 하고 보는 회복을 하고 있다고 하고 말했다. 역 기가 있는 경기 기가 있다면 하는 것이 되었다면 하는 것이 되었다는 것이 되었다. 기가 되었다는 것이 되었다.	

TYPE OF WORK

LEVELING CONCRETE

LOCATION : MAIN BODT AND SIDE WALL



TYPE OF WORK LOCATION

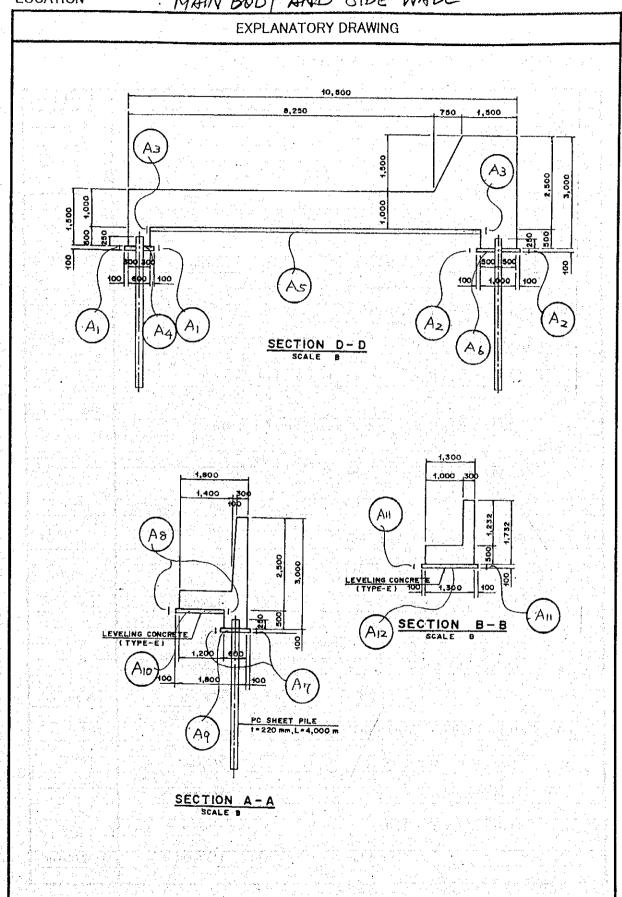
FORM FOR LEVELING CONCRETE : MAIN BODY AND SIDE WALL

CALCULATION	RESULT
(H < 4.0m)	
$A_1 = 0.10 \times 39.40 \times 2 = 7.880$	
$A_z = 0.10 \times 39.40 \times 2 = 7.880$	
$A_3 = 0.10 \times 39.40 \times 2$ = 7.880	
$A_4 = \{0.10 \times 0.80 - (0.22 \times 0.10)\} \times 2 = 0.116$	
$As = 0.10 \times 8.90 \times 2 = 1.780$	
A6= {0.10 x 1.20 - (0.22 x 0.10) } x 2 = 0.196	
$A_7 = 0.10 \times (10.50 + 5.308 + 0.10) \times 2 \times 2 = 6.363$	
$A8 = 0.10 \times (10.50 + 5.308 + 0.10) \times 2 \times 2 = 6.363$	
요. 그리는 가능 수 있는 그리는 하는 그 사는 그들은 그들은 사람들이 되었다. 그는 그들은 그들은 그를 보는 것이다. 그는 그를 모르는 것이다. 그는 그를 보는 것이다. 그렇게 그는 16 분들은 그렇게 되었다. 그는 그들은 그를 보는 것이다.	
$A9 = \{0.10 \times 0.80 - (0.22 \times 0.10)\} \times 2 \times 2 = 0.232$	
$A_{10} = 0.10 \times 1.30 \times 2 \times 2 = 0.520$	
A = 0.40 × 6.50 × 0.40 × 0.50 × 5.50 × 0.40 × 0.50 × 0.40 × 0.50 × 0.40 × 0.50 × 0.40 × 0.50 × 0.40 × 0.50 × 0.40 × 0.50 × 0.50 × 0.40 × 0.50 × 0.40 × 0.50 × 0.40 × 0.50 × 0.40 × 0.50 × 0.40 × 0.50 × 0.40 × 0.50 × 0.40 × 0.50 × 0.40 × 0.50 × 0.40 × 0.50 × 0.40 × 0.50	
$A_{11} = 0.10 \times (5.00 + 0.10) \times 2 \times 2 = 2.040$	
A12 = 0.10 × 1.50 × 2 × 2 = 0.600	
TOTAL = 41.850	41.850 m ²

TYPE OF WORK

: FORM FOR LEVELING CONCRETE

LOCATION : MAIN BODY AND SIDE WALL



TYPE OF WORK: LOCATION: GROUNDSILL WITH HEAD AT WF.124

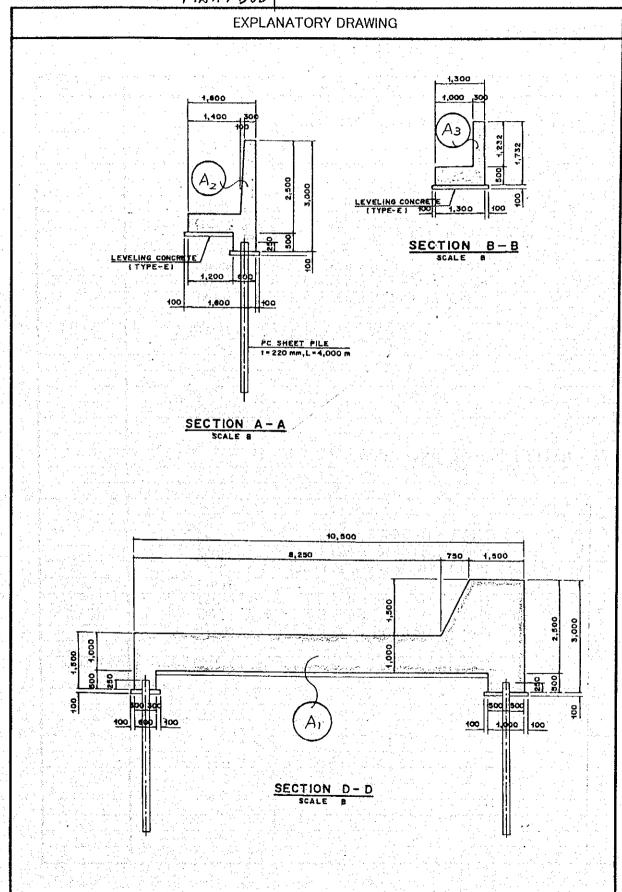
	CALCULATION		RESULT
	The Control of the Co		
<u>~</u>	WATER STOP (SIDE WALL)		
	B = 200 mm		
	B - 200 mm		
	$L_1 = (2.25 + 1.65) \times 2 =$	7.800	
	$L_2 = (1.482 + 1.15) \times 2 =$	5.264	<u> </u>
	TOTAL =	13.064	12 064 m
	TOTAL =	15.004	13.064 m
<u>-</u> 7	JOINT FILTER (MAIN BODY)		
	t = 10, ELASTIC MATERIAL		
1 - 1	$A_1 = \{(1.50 + 0.60) + (9.90 \times 1.00) + (1.00 \times 0.50) + (2.00) + $	25 ± 1.50)	
	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	28,225	
<u> </u>		20,223	
- /	$A_2 = \{(0.30 + 0.40) \times 1/2 \times 2.00 + (0.50 \times 1.80) + (0.60 \times $	0.50)} x 2	
-		3.800	
•	(0.20 - 1.200) + (0.50 - 1.200)	0.000	
	$A_3 = \{(0.30 \times 1.232) + (0.50 \times 1.30)\} \times 2 =$	2.039	
	TOTAL =	34.064	34.064 m ²
177			
5	GAGION MATTRESS (MAIN BODY)		
-	t = 500		
	$V_1 = 27.00 \times 10.00 \times 0.50 =$	135.000	
	$V_2 = (34.40 + 39.40) \times \frac{1}{2} \times 10.00 \times 0.50 =$	184.500	
	V 22.00 v 10.00 v 0.50	140 000	
	$V_3 = 32.00 \times 10.00 \times 0.50 =$	160.000	
- 4	$V_3 = 32.00 \times 10.00 \times 0.50 = $	160.000 479.500	479.500 m ³
	TOTAL =		
5	TOTAL = PALM FIBER FILTER (MAIN BODY)		479.500 m ³
5	TOTAL = PALM FIBER FILTER (MAIN BODY)	479.500	479.500 m ³
5	$TOTAL =$ PALM FIBER FILTER (MAIN BODY) $A_1 = (35.00 + 0.50 \times 2) \times 10.00 =$	479.500 360.000	479.500 m ³
5	$TOTAL =$ PALM FIBER FILTER (MAIN BODY) $A_1 = (35.00 + 0.50 \times 2) \times 10.00 =$	479.500 360.000	479.500 m ³
5	TOTAL = PALM FIBER FILTER (MAIN BODY) $A_1 = (35.00 + 0.50 \times 2) \times 10.00 = $ $A_2 = (34.40 + 39.40) \times \frac{1}{2} \times 10.00 = $	479.500 360.000	479.500 m ³
5	$TOTAL =$ PALM FIBER FILTER (MAIN BODY) $A_1 = (35.00 + 0.50 \times 2) \times 10.00 =$	479.500 360.000 369.000	479.500 m ³
6	TOTAL = PALM FIBER FILTER (MAIN BODY) $A_1 = (35.00 + 0.50 \times 2) \times 10.00 = $ $A_2 = (34.40 + 39.40) \times \frac{1}{2} \times 10.00 = $	360.000 369,000	479.500 m ³

TYPE OF WORK

JOINT FILLER

LOCATION

: MAIN BODY



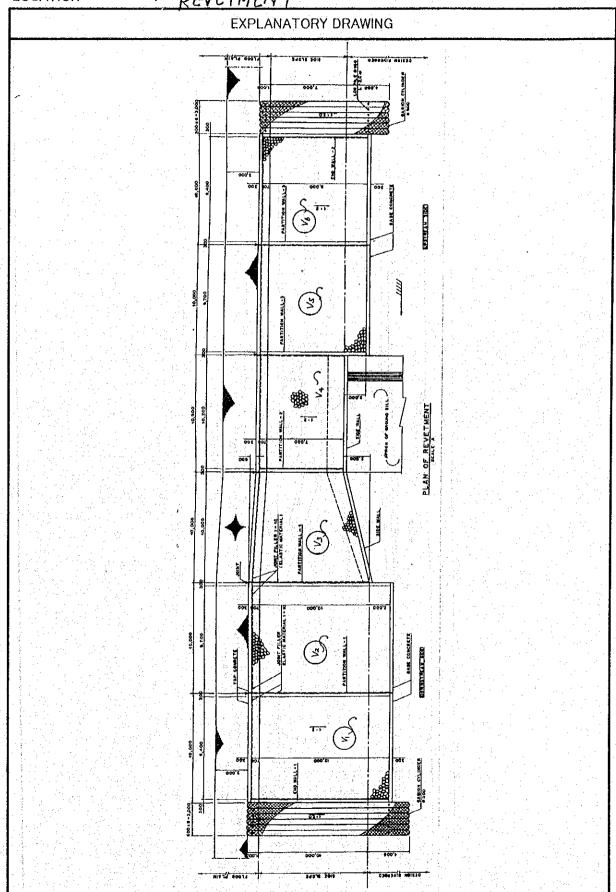
TYPE OF WORK: LOCATION: GROUNDSILL WITH HEAD AT WF.124

	CALCULATION		RESULT
WET STO	NE MASONRY (REVETMENT)		
		8 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 -	
t = 250			
		[1] [1] [1] [1] [1] [2] [2] [2] [3] [4] [4] [4] [4] [4] [4] [4] [4] [4] [4	
	$0 \times (12.00 \times 1.118 + 0.70) \times 0.25 \times 2$		
	$0 \times (12.00 \times 1.118 + 0.70) \times 0.25 \times 2$		a sa
$V_3 = 10$	00 x {(10.00 + 7.00) x ½ x 1.118 + 0.7	0} x 0.25 x 2	nn goden de
		= 51.015	
$V_4 = 10$	$20 \times (7.00 \times 1.118 + 0.70) \times 0.25 \times 2$	= 43.483	
$V_5 = 9$	$70 \times (9.00 \times 1.118 + 0.70) \times 0.25 \times 2$	= 52.196	
$V_6 = 9$	$40 \times (9.00 \times 1.118 + 0.70) \times 0.25 \times 2$	= 50.581	
			220 002
	TOTAL	= 332.083	332.083
<u> </u>		North Notae (1986) in the Commence of the Comm	
<u>i kon istrostjelovani</u> Konstantinos			
GRAVEI	DEDUCK OFFICERMENT		
GRAVEL	BEDDING (REVETMENT)		
gi sang bala Kabupatèn			
		电电路 化二氯化二甲二甲基二甲二甲二甲二甲二甲二甲二甲二甲二甲二甲二甲二甲二甲二甲二甲二甲二甲	化二甲基二甲基二甲基二甲基二甲基二甲基二甲基二甲基二甲基二甲基二甲基二甲基二甲基二
t == 250			
t = 250			
	0 x (12.00 x 1.118 + 0.70) x 0.25 x 2	= 66,345	
$V_1 = 9$	0 x (12.00 x 1.118 + 0.70) x 0.25 x 2 0 x (12.00 x 1.118 + 0.70) x 0.25 x 2		
$V_1 = 9.4$ $V_2 = 9.4$	$70 \times (12.00 \times 1.118 + 0.70) \times 0.25 \times 2$	= 68.463	
$V_1 = 9.4$ $V_2 = 9.4$		= 68.463	
$V_1 = 9.$ $V_2 = 9.$ $V_3 = 10.$	0 x (12.00 x 1.118 + 0.70) x 0.25 x 2 00 x {(10.00 + 7.00) x ½ x 1.118 + 0.7	= 68.463 70} x 0.25 x 2	
$V_1 = 9.$ $V_2 = 9.$ $V_3 = 10$ $V_4 = 10$	0 x (12.00 x 1.118 + 0.70) x 0.25 x 2 .00 x {(10.00 + 7.00) x ½ x 1.118 + 0.70) .20 x (7.00 x 1.118 + 0.70) x 0.25 x 2	= 68.463 (0) x 0.25 x 2 = 51.015 = 43.483	
$V_{1} = 9.$ $V_{2} = 9.$ $V_{3} = 10$ $V_{4} = 10$ $V_{5} = 9$	0 x (12.00 x 1.118 + 0.70) x 0.25 x 2 00 x {(10.00 + 7.00) x ½ x 1.118 + 0.7 20 x (7.00 x 1.118 + 0.70) x 0.25 x 2 70 x (9.00 x 1.118 + 0.70) x 0.25 x 2	= 68.463 (0) x 0.25 x 2 = 51.015 = 43.483 = 52.196	
$V_{1} = 9.$ $V_{2} = 9.$ $V_{3} = 10$ $V_{4} = 10$ $V_{5} = 9$	0 x (12.00 x 1.118 + 0.70) x 0.25 x 2 .00 x {(10.00 + 7.00) x ½ x 1.118 + 0.70) .20 x (7.00 x 1.118 + 0.70) x 0.25 x 2	= 68.463 (0) x 0.25 x 2 = 51.015 = 43.483 = 52.196 = 50.581	
$V_{1} = 9.$ $V_{2} = 9.$ $V_{3} = 10$ $V_{4} = 10$ $V_{5} = 9$	0 x (12.00 x 1.118 + 0.70) x 0.25 x 2 00 x {(10.00 + 7.00) x ½ x 1.118 + 0.7 20 x (7.00 x 1.118 + 0.70) x 0.25 x 2 70 x (9.00 x 1.118 + 0.70) x 0.25 x 2	= 68.463 (0) x 0.25 x 2 = 51.015 = 43.483 = 52.196	332.083
$V_{1} = 9.$ $V_{2} = 9.$ $V_{3} = 10$ $V_{4} = 10$ $V_{5} = 9$	0 x (12.00 x 1.118 + 0.70) x 0.25 x 2 .00 x {(10.00 + 7.00) x ½ x 1.118 + 0.7 .20 x (7.00 x 1.118 + 0.70) x 0.25 x 2 .70 x (9.00 x 1.118 + 0.70) x 0.25 x 2 .40 x (9.00 x 1.118 + 0.70) x 0.25 x 2	= 68.463 (0) x 0.25 x 2 = 51.015 = 43.483 = 52.196 = 50.581	
$V_{1} = 9.$ $V_{2} = 9.$ $V_{3} = 10$ $V_{4} = 10$ $V_{5} = 9$	0 x (12.00 x 1.118 + 0.70) x 0.25 x 2 00 x {(10.00 + 7.00) x ½ x 1.118 + 0.7 20 x (7.00 x 1.118 + 0.70) x 0.25 x 2 70 x (9.00 x 1.118 + 0.70) x 0.25 x 2 40 x (9.00 x 1.118 + 0.70) x 0.25 x 2 TOTAL	= 68.463 (0) x 0.25 x 2 = 51.015 = 43.483 = 52.196 = 50.581 = 332.083	
$V_{1} = 9.$ $V_{2} = 9.$ $V_{3} = 10$ $V_{4} = 10$ $V_{5} = 9$ $V_{6} = 9$	0 x (12.00 x 1.118 + 0.70) x 0.25 x 2 00 x {(10.00 + 7.00) x ½ x 1.118 + 0.7 20 x (7.00 x 1.118 + 0.70) x 0.25 x 2 70 x (9.00 x 1.118 + 0.70) x 0.25 x 2 40 x (9.00 x 1.118 + 0.70) x 0.25 x 2 TOTAL	= 68.463 20} x 0.25 x 2 = 51.015 = 43.483 = 52.196 = 50.581 = 332.083	
$V_{1} = 9.$ $V_{2} = 9.$ $V_{3} = 10$ $V_{4} = 10$ $V_{5} = 9$ $V_{6} = 9$	0 x (12.00 x 1.118 + 0.70) x 0.25 x 2 00 x {(10.00 + 7.00) x ½ x 1.118 + 0.7 20 x (7.00 x 1.118 + 0.70) x 0.25 x 2 70 x (9.00 x 1.118 + 0.70) x 0.25 x 2 40 x (9.00 x 1.118 + 0.70) x 0.25 x 2 TOTAL	= 68.463 70} x 0.25 x 2 = 51.015 = 43.483 = 52.196 = 50.581 = 332.083	
$V_{1} = 9.$ $V_{2} = 9.$ $V_{3} = 10$ $V_{4} = 10$ $V_{5} = 9$ $V_{6} = 9$	0 x (12.00 x 1.118 + 0.70) x 0.25 x 2 00 x {(10.00 + 7.00) x ½ x 1.118 + 0.7 20 x (7.00 x 1.118 + 0.70) x 0.25 x 2 70 x (9.00 x 1.118 + 0.70) x 0.25 x 2 40 x (9.00 x 1.118 + 0.70) x 0.25 x 2 TOTAL	= 68.463 70} x 0.25 x 2 = 51.015 = 43.483 = 52.196 = 50.581 = 332.083	
$ \begin{array}{rcl} V_1 &=& 9. \\ V_2 &=& 9. \\ V_3 &=& 10 \\ \hline V_4 &=& 10 \\ V_5 &=& 9 \\ V_6 &=& 9 \end{array} $	0 x (12.00 x 1.118 + 0.70) x 0.25 x 2 00 x {(10.00 + 7.00) x ½ x 1.118 + 0.7 20 x (7.00 x 1.118 + 0.70) x 0.25 x 2 70 x (9.00 x 1.118 + 0.70) x 0.25 x 2 40 x (9.00 x 1.118 + 0.70) x 0.25 x 2 TOTAL	= 68.463 (0) x 0.25 x 2 = 51.015 = 43.483 = 52.196 = 50.581 = 332.083	
$V_{1} = 9.$ $V_{2} = 9.$ $V_{3} = 10$ $V_{4} = 10$ $V_{5} = 9$ $V_{6} = 9$	0 x (12.00 x 1.118 + 0.70) x 0.25 x 2 00 x {(10.00 + 7.00) x ½ x 1.118 + 0.7 20 x (7.00 x 1.118 + 0.70) x 0.25 x 2 70 x (9.00 x 1.118 + 0.70) x 0.25 x 2 40 x (9.00 x 1.118 + 0.70) x 0.25 x 2 TOTAL	= 68.463 (0) x 0.25 x 2 = 51.015 = 43.483 = 52.196 = 50.581 = 332.083	
$ \begin{array}{rcl} V_1 &=& 9. \\ V_2 &=& 9. \\ V_3 &=& 10 \\ \hline V_4 &=& 10 \\ V_5 &=& 9 \\ V_6 &=& 9 \end{array} $	0 x (12.00 x 1.118 + 0.70) x 0.25 x 2 00 x {(10.00 + 7.00) x ½ x 1.118 + 0.7 20 x (7.00 x 1.118 + 0.70) x 0.25 x 2 70 x (9.00 x 1.118 + 0.70) x 0.25 x 2 40 x (9.00 x 1.118 + 0.70) x 0.25 x 2 TOTAL	= 68.463 70} x 0.25 x 2 = 51.015 = 43.483 = 52.196 = 50.581 = 332.083	
$ \begin{array}{rcl} V_1 &=& 9. \\ V_2 &=& 9. \\ V_3 &=& 10 \\ \hline V_4 &=& 10 \\ V_5 &=& 9 \\ V_6 &=& 9 \end{array} $	0 x (12.00 x 1.118 + 0.70) x 0.25 x 2 00 x {(10.00 + 7.00) x ½ x 1.118 + 0.7 20 x (7.00 x 1.118 + 0.70) x 0.25 x 2 70 x (9.00 x 1.118 + 0.70) x 0.25 x 2 40 x (9.00 x 1.118 + 0.70) x 0.25 x 2 TOTAL	= 68.463 70} x 0.25 x 2 = 51.015 = 43.483 = 52.196 = 50.581 = 332.083	
$ \begin{array}{rcl} V_1 &=& 9. \\ V_2 &=& 9. \\ V_3 &=& 10 \\ \hline V_4 &=& 10 \\ V_5 &=& 9 \\ V_6 &=& 9 \end{array} $	0 x (12.00 x 1.118 + 0.70) x 0.25 x 2 00 x {(10.00 + 7.00) x ½ x 1.118 + 0.7 20 x (7.00 x 1.118 + 0.70) x 0.25 x 2 70 x (9.00 x 1.118 + 0.70) x 0.25 x 2 40 x (9.00 x 1.118 + 0.70) x 0.25 x 2 TOTAL	= 68.463 20} x 0.25 x 2 = 51.015 = 43.483 = 52.196 = 50.581 = 332.083	

WET STONE MASONRY

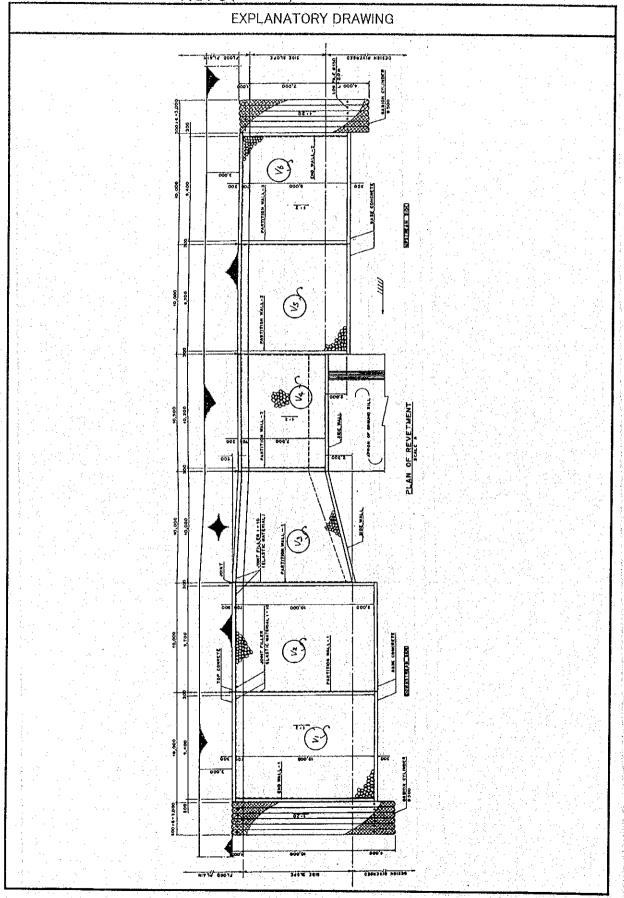
LOCATION : REVETMENT

TYPE OF WORK



TYPE OF WORK : GRAVEL BEDDING

LOCATION : REVETMENT



TYPE OF WORK

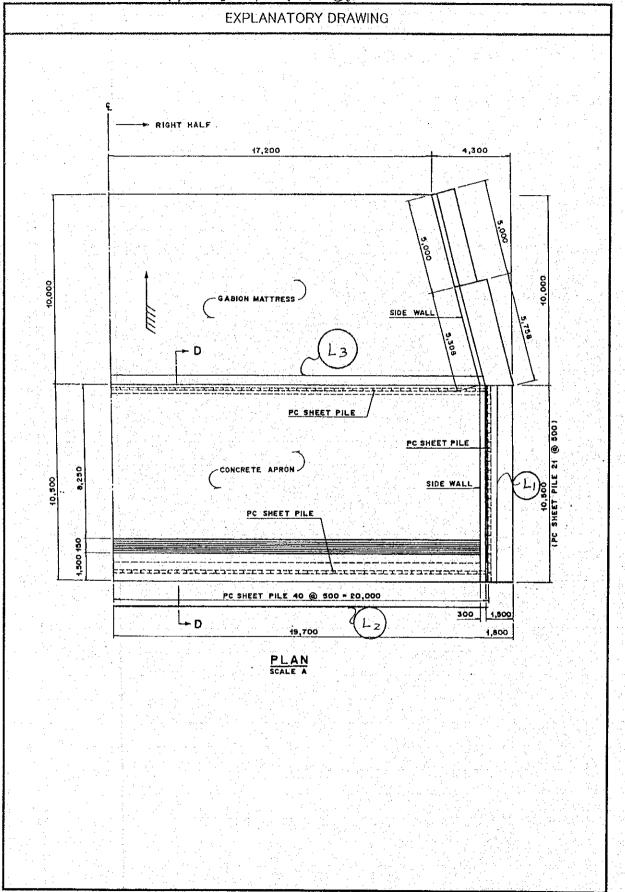
LOCATION

: PC SHEET PILE

: MAIN BODY AND SIDE WALL CALCULATION RESULT · PC SHEET PILE (L= 4.00 m/pile) $n_1 = 10.50 \div 0.50 = 21$ piles L1 = 21 piles x 4.00 x 2 = 168,000 nz = 20.00 : 0.50 = 40 piles 12 = 40 piles x 4.00 x 2 = 320.000 na = 20.00 ÷ 0.50 = 40 piles 13 = 40 piles x 4.00 x 2 = 320.000 70TAL = 808.000 808.000 m - PILING N Value: N = 27 (Average) 1 = (2) + 40 + 40) piles x 2 x 4.00 = 808.000 808.000m

TYPE OF WORK

PC SHEET PILE MAIN BODY AND SIDE WALL LOCATION



TYPE OF WORK LOCATION

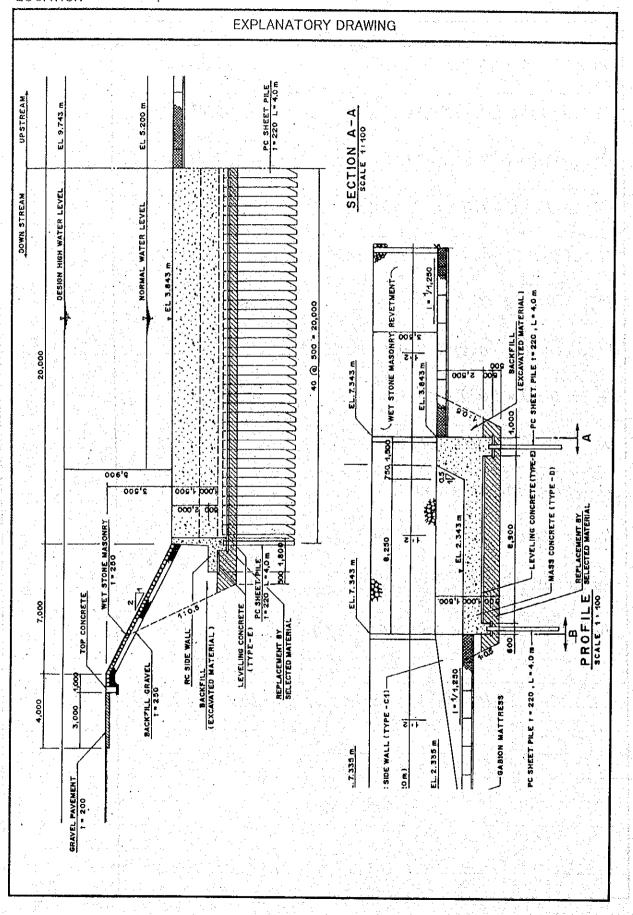
GROUND SILL WITH HEAD AT WF. 124 : REPLACEMENT OF BASE SOIL

CALCULATION	RESULT
$A_1 = (44.00 + 45.00) \times \frac{1}{2} \times 1.00 = 44.500 \text{ m}^2$	
$V_1 = 44.50 \times (11.50 + 12.50) \times \frac{1}{2} = 534.000$	
$A_2 = (2.80 + 3.80) \times \frac{1}{2} \times 1.00$ = 3.300 m ²	
V2 = 3.30 x (5.758 + 5.308) x /2 x 2 = 36.518	
$A_3 = (2.30 + 3.80) \times \frac{1}{2} \times 1.00 = 3.050 \text{ m}^2$	
7.3 (2.50 + 3.60) / 1/2 / 1.60 (3.60) / 1/2 / 1.60 (3.60)	
V3 = 3.05 × 5.00 × 2 = 30.500	
(Deduction for Ground sill)	
$V_4 = -(0.50 \times 1.00) \times 39.40 = -19.700$	
. 하고 마이에는 전쟁, 클로벌맞으로 살로 생생하게 했다. 그런 말을 하는 것 같아 하는 것을 보면 그렇는 것이다. 얼마를 하는 것이다. 그를 그릇 말을 맛있는 것을 맛있는 것이다.	
V5 = - (0.50 x 0.60) x 39.40 = -11.820	
(Deduction for Side wall)	
Carron for Site Wall)	
V6 = - (0.60 × 0.50) × 10.50 × 2 = - 6.300	
Deduction for PC Sheet Pile>	
Vy = - (122 piles x 0.50 x 0.22) x 0.40 = -5.368	
- 프로프 현대를 보고 있는데, 이번도 1965년에 전급 학교 후 하는 스트라이라고 하는데 그는데, 그는 보는데 그를 되면 하는데 모습니다. 그를 기급하는데 하나 하는데 이 기급을 하는데 하는데 그리고 있는데 하는데 그런 그를 하는데	
	000 3
707AL (V1+V2+V3+V4+ U5+V6+ U7) = 557.830	557.830 m³

REPLACEMENT OF BASE SOIL

LOCATION

TYPE OF WORK



TYPE OF WORK: LOCATION: **GROUNDSILL WITH HEAD AT WF.124**

CALCULATION		RESU
BASE CONCRETE (REVETMENT)		
_ = 40.00 m / side		
3 = 40.00 m7 side		
CONCRETE (TYPE - C1)		
$V = 40.00 \times 2.20 / 10.00 \text{ m}$	= 17.600	17.600
		1
GRAVEL BEDDING		
$V = 40.00 \times 2 \times 0.70 / 10.00 \text{ m}$	= 5.600	5.600
FORM (H < 4.0 m)		
$A = 40.00 \times 2 \times 10.83 / 10.00 \text{ m}$	= 86.640	86.460
REINFORCING BAR	0.000	0.000
$W = 40.00 \times 2 \times 0.100 / 10.00 \mathrm{m}$	= 0.800	0.800
JOINT FILTER	<u>i kan di katang selamban di Bulata.</u> Panggalanggan	
$A = 40.00 \times 2 \times 0.22 / 10.00 \text{ m}$	= 1.760	1.760
化氯甲酚 化氯化甲酚 医乳腺 化二甲酚 化二氯化氯化物 医乳腺性病 化二氯化 化电压 化二二甲基甲基甲基二甲基甲基	ang Salah kalanggang kabahan bangan bilanggan di sa	
<u>보면 되는 것으로 한 것으로 하는 경험 하는 것으로 하는 것으로 하는다.</u> 보고 있는 것은 것은 하는 것은 사람들은 것은 사람들은 것으로 보고 있다.	<u>ng langan ng til sa sa s</u> Managang lang tagan na	
OP CONCRETE (REVETMENT)		
TOP CONCRETE (REVETMENT) . = 60.50 m/side		
. = 60.50 m / side		
. = 60.50 m / side CONCRETE (TYPE - C1)	= 21780	21.780
. = 60.50 m / side	= 21.780	21.780
CONCRETE (TYPE - C1) V = 60.50 x 1.80 / 10.00 m	= 21.780	21.780
. = 60.50 m / side CONCRETE (TYPE - C1)	= 21.780 = 9.075	
CONCRETE (TYPE - C1) V = 60.50 x 1.80 / 10.00 m GRAVEL BEDDING	ninga na kasa da kabada s Desarra na kabada sa kabada sa kaba	
CONCRETE (TYPE - C1) V = 60.50 x 1.80 / 10.00 m GRAVEL BEDDING V = 60.50 x 2 x 0.75 / 10.00 m FORM (H < 4.0 m)	ninga na kasa da kabada s Desarra na kabada sa kabada sa kaba	9.075
CONCRETE (TYPE - C1) V = 60.50 x 1.80 / 10.00 m GRAVEL BEDDING V = 60.50 x 2 x 0.75 / 10.00 m FORM (H < 4.0 m)	ninga na kasa da kabada s Desarra na kabada sa kabada sa kaba	9.075
CONCRETE (TYPE-C1) V = 60.50 x 1.80 / 10.00 m GRAVEL BEDDING V = 60.50 x 2 x 0.75 / 10.00 m FORM (H < 4.0 m) A = 60.50 x 2 x 12.18 / 10.00 m	= 9.075	9.075
CONCRETE (TYPE - C1) V = 60.50 x 1.80 / 10.00 m GRAVEL BEDDING V = 60.50 x 2 x 0.75 / 10.00 m FORM (H < 4.0 m) A = 60.50 x 2 x 12.18 / 10.00 m	= 9.075 = 147.378	9.075
CONCRETE (TYPE-C1) V = 60.50 x 1.80 / 10.00 m GRAVEL BEDDING V = 60.50 x 2 x 0.75 / 10.00 m FORM (H < 4.0 m) A = 60.50 x 2 x 12.18 / 10.00 m	= 9.075	9.075
CONCRETE (TYPE - C1) V = 60.50 x 1.80 / 10.00 m GRAVEL BEDDING V = 60.50 x 2 x 0.75 / 10.00 m FORM (H < 4.0 m) A = 60.50 x 2 x 12.18 / 10.00 m REINFORCING BAR W = 60.50 x 0.094 / 10:00 m	= 9.075 = 147.378	9.075
CONCRETE (TYPE - C1) V = 60.50 x 1.80 / 10.00 m GRAVEL BEDDING V = 60.50 x 2 x 0.75 / 10.00 m FORM (H < 4.0 m) A = 60.50 x 2 x 12.18 / 10.00 m REINFORCING BAR W = 60.50 x 0.094 / 10.00 m	= 9.075 = 147.378 = 1.137	9.075
CONCRETE (TYPE - C1) V = 60.50 x 1.80 / 10.00 m GRAVEL BEDDING V = 60.50 x 2 x 0.75 / 10.00 m FORM (H < 4.0 m) A = 60.50 x 2 x 12.18 / 10.00 m REINFORCING BAR W = 60.50 x 0.094 / 10:00 m	= 9.075 = 147.378	9.075
CONCRETE (TYPE - C1) V = 60.50 x 1.80 / 10.00 m GRAVEL BEDDING V = 60.50 x 2 x 0.75 / 10.00 m FORM (H < 4.0 m) A = 60.50 x 2 x 12.18 / 10.00 m REINFORCING BAR W = 60.50 x 0.094 / 10.00 m	= 9.075 = 147.378 = 1.137	9.075 147.378 1.137
CONCRETE (TYPE - C1) V = 60.50 x 1.80 / 10.00 m GRAVEL BEDDING V = 60.50 x 2 x 0.75 / 10.00 m FORM (H < 4.0 m) A = 60.50 x 2 x 12.18 / 10.00 m REINFORCING BAR W = 60.50 x 0.094 / 10.00 m	= 9.075 = 147.378 = 1.137	9.075
CONCRETE (TYPE - C1) V = 60.50 x 1.80 / 10.00 m GRAVEL BEDDING V = 60.50 x 2 x 0.75 / 10.00 m FORM (H < 4.0 m) A = 60.50 x 2 x 12.18 / 10.00 m REINFORCING BAR W = 60.50 x 0.094 / 10.00 m	= 9.075 = 147.378 = 1.137	9.075
CONCRETE (TYPE - C1) V = 60.50 x 1.80 / 10.00 m GRAVEL BEDDING V = 60.50 x 2 x 0.75 / 10.00 m FORM (H < 4.0 m) A = 60.50 x 2 x 12.18 / 10.00 m REINFORCING BAR W = 60.50 x 0.094 / 10.00 m	= 9.075 = 147.378 = 1.137	9.075

TYPE OF WORK:

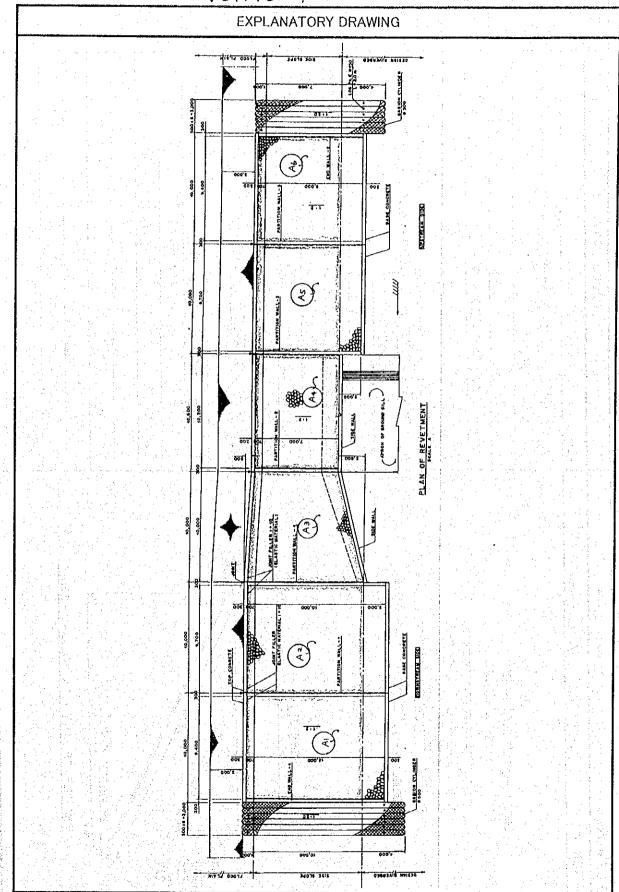
LOCATION : GROUNDSILL WITH HEAD AT WF.124

	CALCULATION	RESULT
	CEMENT MORTAR POINTING (REVETMENT) $A_1 = 9.40 \times (10.00 \times 1.118 + 0.70) \times 0.25 \times 2 = 223.344$	
	$A_1 = 9.40 \times (10.00 \times 1.118 \pm 0.70) \times 0.23 \times Z = 223.344$	
	$A_2 = 9.70 \times (10.00 \times 1.118 + 0.70) \times 0.25 \times 2 = 230.472$	
		17 <u>18</u> 1, 71 (11)
	$A_3 = 10.00 \times \{(10.00 + 7.00) \times \frac{1}{2} \times 1.118 + 0.70 \times 2$	
-	= 204.060	
	$A_4 = 10.20 \times (7.00 \times 1.118 + 0.70) \times 2 = 173.930$	
	$A_5 = 9.70 \times (7.00 \times 1.118 + 0.70) \times 2 = 165.404$	33430 H. J. 1814
	$A_6 = 9.70 \times (7.00 \times 1.118 + 0.70) \times 2 = 160.289$	
	$A_6 = 9.70 \times (7.00 \times 1.118 + 0.70) \times 2 = 160.289$	
	TOTAL = 1157,499	1157.499 m²
	RUBBLE STONE FILLING (REVETMENT)	
	$A = \frac{1}{2} \times 1.00 \times 0.50 \times 2 = 0.500 \text{ m}^2$	Aller Bridger American Land Arthur Erich
-	$A = 72 \times 1.00 \times 0.30 \times 2$ = 0.300 m	
	$V = 0.50 \times (20.00 + 20.00) \times 2 = 40.000$	40.000 m ³
	GABION MATTRESS (REVETMENT)	
	$A = (3.00 \times 0.50) + (1.50 \times 0.50)$ = 2.250 m ²	
	$A = (3.300 \times 0.30) \cdot (1.30 \times 0.30) = 2.230 \text{ m}$	
	$V = 2.25 \times 40.00 \times 2 = 180.000$	180.000 m ³
6	GABION CYLINDER	
1 12/4	GABION CYLINDER Ø 500	
	$L_1 = 4.00 + 10.00 \times 1.118 + 1.00 = 16.180 \text{ m/unit}$	
		Laba yang di Affilia dan Nahari Tanggalan Kasas
	$L_2 = 4.00 + 7.00 \times 1.118 + 1.00 = 12.826 \text{ m/unit}$	
·		
. 12.1	$V_1 = \pi /4 \times 0.50^2 \times 16.180 \times 6 \text{ unit } \times 2 = 38.123$	
	$V_2 = \pi /4 \times 0.50^2 \times 12.826 \times 6 \text{ unit } \times 2 = 30.221$	en de la servició de la composició de la c La composició de la composició
- 1	30.221	
:	TOTAL = 68.344	68.344 m³
	SOIL FILLING $V_1 = (16.180 \times 3.00 \times 0.50) \times 2 - 38.123 = 10.417$	
-	$V_1 = (16.180 \times 3.00 \times 0.50) \times 2 - 38.123 = 10.417$ $V_2 = (12.8260 \times 3.00 \times 0.50) \times 2 - 30.221 = 8.257$	
	1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1	in de de partire. Viviendo
	TOTAL = 18.674	18.674 m ³

CEMENT MORTAR POINTING

TYPE OF WORK LOCATION

REVETMENT



GROUND SILL WITH HEAD AT WF. 124		
TYPE OF WORK: CONCRETE (TYPE-CI)	CALCULATION	RESULT
LOCATION: PARTITION WALL (REVETHENT)	-WALL-1	
	$A1 = (13.416 + 13.324) \times \frac{1}{2} \times 0.50 + (0.70 + 0.582)$	
	×//2 × 0.50	
JOINT FILLER.1 = 10 50 200 50	z w 900'2 =	
	$V_1 = (7.006 \times 0.30) \times 2 \times 2$ = 8.407	
002 9	· WALLE 2	
GI 61 (MAN) AND 300 IGO GRAVEL BEDDING	Az = (7.826+7.734)×1/2 ×0.50+(0.70+0.582)	
ار الم المارية	×/2×0,50	
PARTITION WALL	1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1	
	Vz = (4.211 × 0.30) × 1 × 2 = 2.527	
2,000 (WALL-1) 5,000 (WALL-2) 5,000 (WALL-2)	• WALL=3	
TOP CONCRETE	A3 = (10.062 +9.97) × 1/2 × 0.50 + (0.70 + 0.582)	
2000	×/2×0.50	
6 000	= 5.329 m²	
- TANKE	V3 = (5.329 × 0.30) × 2 × 2 = 6.395	
15,18		
	707AL = 17.329	17.329 m3
	The second secon	-
DROFII F OF DARTITION WAI		
SCALE A		

\$77 \ \$137

	CALCULATION	-WAUL+)	$A_1 = \{ c_{13}, 416 + 13.324 \} \times \frac{1}{2} \times 0.50 + (0.70 + 0.582) \}$	x/2 x 0.50 { x 2 x 2 x 2	= 56,044	-WALL-2		= { (7.826 + 7.734) × 1/2 × 0.50 + (0.70 + 0.582)	x/2×050 + × 2 × 1 × 2	16.842 The second secon	-WALL-3		A3 = 1 (10.062+9970) ×1/2×0.50 + (0.70+0.582)	x/2 x 0.50 + x 2 x 2 x 2	= 42.628		TOTAL = 1/5:514 115.514 m2				and the second of the second o	
GROUND SILL WITH HEAD AT WF 124	TYPE OF WORK: FORM (H< 4.0m)	LOCATION: PARTITION WALL (REVETMENT) - WA		300.	JOINT FILLER 1 = 10 50 200 150 (ELASTIC MATERIAL) 0.10(3)00 0.10(3)00	m- X 5 000 000 000 000 000 000 000 000 000	. 1	CRETE, TYPE-C1		SCALE C. SCA		7,000 (WALL-2) 9,000 (WALL-3) 7,000 (WALL-3)	WALL TO TOP CONCRETE	1000 ov. (2	OO!	13.416		 84 BASE CONCRETE 6,918 WALL-21	PROFILE OF PARTITION WALL -1, -28-3	SCALE OF A CONTROL		化对射性 化多氯化物 医克克氏病 经现代的 医动物 医克里克氏病 医克勒氏试验检尿病 医克勒氏试验检尿病 医动物性神经 医多种性 医多种性 医多种性 医多种性 医多种性 医多种性 医多种性 医多种性

TYPE OF WORK

: REINFORCING BAR : PARTITION WALL (REVETMENT) LOCATION

CALCULATION WALL (REVETMENT)	RESULT
· WALL -)	
(DI3, W=1.04 kgf/m)	
n= 6 Bars	
L = (13.416+0.70) - 0.05 ×2 = 14.016 m/Bar	
$W_1 = 6 \text{Bars} \times 14.016 \times 1.04 \times 2 \times 2 = 349.839$	
(DIO, W=0.617 kafm)	
$n = (14.016 \div 0.30) + 1 = 47.72 \div 48 Bars$	
$L = (0.20 \times 2 + 0.40 \times 2 + 15 \times 0.01) = 1.350 \frac{m}{Bar}$	
Wz = 48 Barx x 1.35 x 0.617 x 2x2 = 159.926	
- WALL-2	
CD13. W = 1.04 kg/m)	
n=6Bats	
$L = (7.826 + 0.70) - 0.05 \times 2 = 8.426 \text{ m/Bar}$	
$W_3 = 6 Bars \times 8.426 \times 1.04 \times 2 \times 2 = 210.313$	
$(D10, W = 0.617 + 8 f_m)$	
$n = (8.426 \div 0.30) + 1 = 29.09 \div 30 Bars$	
L = 1.350 m/Bat	
W4 = 30 Bars x 1.35 x 0.617 x 2 x 2 = 99.954	
·WALL-3	
(D13, W=1.04 kgf/m)	
n=6 Bars	
1= (10.062+0.70)-0.05×2 = 10.662 m/Bat	
$Ws = b Bars \times 10.662 \times 1.04 \times 2 \times 2 = 266.124$ (D10, W = 0.617 kg fm)	
$n = (10.662 \div 0.30) + 1 = 36.54 \div 37 \text{ Bars}$	
L = 1.35 m/Bar	
W6 = 37 Bars x 1.35 x 0.617 x 2 x 2 = 123,277	
TOTAL (W, +W2+W3+W4+W5+W6) = 1209.433 kgf	1.210 tf
	71-70
enga kana anggarang kanasa ang ang ang ang ang ang ang ang ang an	

RESULT																		55,52) m2	1		
4 CALCULATION	t=10, ELASTIC MATERIAL	-WALL-I	A1=1(13.416+13.324)×1/2 ×0.50 + (0.70+0.582)×1/2	×0.50. { × × × 2	**************************************		WALL-2		Az= 1(7.826+7.734) x1/2×0.50+(0.70+0.582)x1/2	x0.50 { x 2 x 2	= 16.842		-WALL+3	A3 = \((10.062+9.97) \(\frac{1}{2}\times \frac{1}{2}\times \frac{1}{2}\tin \frac{1}{2}\times \frac{1}{2}\times \frac{1}{2}\times \frac{1}{	x0.50 x1 x 2	20 10 10 10 10 10 10 10 10 10 10 10 10 10		TOTAL = 55:52)			
)RK:	OCATION: PARTITION WALL (REVETMENT)				300	(ELASTIC MATERAL) (ELASTIC MATERAL) (5) (5)		002.00	3.00	ICRETE, TYPE-C1	E	SALE C									(1) (1) (1) (2) (2) (2) (3) (3) (3) (4) (4) (4) (4) (4) (4) (4) (4) (4) (4

RESULT													8.882 x					
/24 CALCULATION	$-\omega_{AH}-1$	$A_1 = (13.416 + 13.351) \times \frac{1}{2} \times 0.60 + (0.70 + 0.558)$	×/2×0.60 = 8.408 m²		V, = 8,408 x 0.30 x 2 = 5,045	· WALL-2		$A_2 = (10.062 + 9.997) \times 1/2 \times 0.60 + (0.70 + 0.558)$ $\times 1/2 \times 0.60$	36E.9 = 100 000 000 000 000 000 000 000 000 0		Vz = 6.395 × 0.30 × 2 - 3.837		70TAL = 8,882					
GROUND SILL WITH HEAD AT WF.124 TYPE OF WORK: CONCRETE CTYPE-CL)	LOCATION: END WALL (REVETMENT)		300 GRAVEL FILLING	(ELASTIC MATERIAL) D13 GAGION CYLINDER D1000 CYLINDER		0522	CRAVEL BEDDING	END WALL		42.000 (WATH 1.9.1)		Too of	OC.		Control of the Contro	(2 2 2 6 9.942 (WALL - 2)	SOLE OF BALL 19-K	《《《··································

GROUND SILL WITH HEAD AT WIF.	$J24^{\circ}$, we see that the second of the s		
	CALCULATION	RESULT	
3	$\cdot \omega \wedge \omega - 1$		
The second of th			
	A) = (0.50 × 0.10) = 0.050 m²		
9,000 (WALL-2) TOP CONCRETE	$V_1 = \{0.05 \times (13, 35) + 0.558\} \{\times 2 = 1.39\}$		
174	· WALL - 2		
	Az = 0.050 m²		
			٠
	10=10.05 × 69.997 + 0.558>1×2 = 1.056		
SE CONCRETE 11,542 (WALL-1)			-
200 B. 942 T. WALL - 2.1			
PROFILE OF END WALL -18-2	70101 = 2.447	2.447 m3	
(1) 「日本の本人」である。 「日本の本人」である。」である。「日本の本人」である。」である。「日本の本人」である。」では、「日本の本人」である。「日本の本人」である。」では、「日本の本人」である。」では、「日本の本人」である。」では、「日本の本人」である。」では、「日本の本人」である。」では、「日本の本人」である。」では、「日本の本人」である。」では、「日本の本人」である。」では、「日本の本人」では、「日本、「日本、「日本、「日本、「日本、「日本、「日本、「日本、「日本、「日本			
	e de la composition	Control of the Contro	

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; ; ;	NESOL.									S9.210m²							1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	
TYPE OF WORK CALL WITH HEAD AT WA 124	END WALL (REVETHENT) WALL-1	300	JOHNT FILLER 1 * 10 SOO SO SOO SOO SOO SOO SOO SOO SOO SOO	P10@300\	8 9 WALL - 2	1	$\langle \Delta \rangle$ $\frac{1}{100}$ $\frac{1}{100$	END WALL = 25.580	The second secon	2,000 (WALL-1)	9,000 (WALL-2) TOP-CONCRETE		WALL.		8 14:942 (WALL-1) 558	PROFILE OF END WALL -18-2		
									4	1-3	6 _i							

TYPE OF WORK

: REINFORCING BAR : END WALL (REVETMENT) LOCATION

$\begin{array}{llllllllllllllllllllllllllllllllllll$	
$\begin{array}{llllllllllllllllllllllllllllllllllll$	
) - ($[3.416+0.70)-0.05\times2$ = $[4.016]^m/Bar$ $W_1 = 6Bars \times [4.016\times1.04\times2] = [74.920]$ ($[0.0]$ $W = 0.617 + [9f/m]$) $M_1 = ([4.016] - 0.30) + 1 = 47.72 = 48 Bars$ $M_2 = 48 Bars \times [0.50 \times 2 + [5 \times 0.01]] = [0.550]^m/Bar$ $M_2 = 48 Bars \times [0.55 \times 0.617 \times 2] = 9[0.810]$ • WALL - 2 ($[0.13]$ $W = [0.04 + [9f/m]]$ $M_1 = 6 Bars$ $M_2 = 6 Bars \times [0.662 \times [0.04 \times 2]] = [0.662]^m/Bar$ $M_3 = 6 Bars \times [0.662 \times [0.04 \times 2]] = [0.662]^m/Bar$ $M_3 = [0.662] - [0.30] + 1 = [0.662]^m/Bar$ $M_4 = [0.662] - [0.30] + 1 = [0.662]^m/Bar$ $M_4 = [0.662] - [0.30] + 1 = [0.662]^m/Bar$ $M_4 = [0.662] - [0.30] + 1 = [0.662]^m/Bar$	
$W_1 = 6 \text{ Bars} \times 14.016 \times 1.04 \times 2 = 174.920$ $(£10, w = 0.617 \frac{1}{10} \frac{1}{10})$ $N = (14.016 \div 0.30) + 1 = 47.72 \doteqdot 48 \text{ Bars}$ $L = (0.20 \times 2 + 0.50 \times 2 + 15 \times 0.01) = 1.550 \frac{1}{10} \text{ Bar}$ $W_2 = 48 \text{ Bars} \times 1.55 \times 0.617 \times 2 = 91.810$ $WALL - 2$ $(£13, w = 1.04 \frac{1}{10} \frac{1}{10})$ $N = 6 \text{ Bars}$ $L = (10.062 + 0.70) - 0.05 \times 2 = 10.662 \frac{1}{10} \frac{1}{10} \text{ Bar}$ $W_3 = 6 \text{ Bars} \times 10.662 \times 1.04 \times 2 = 133.062$ $(£10, w) = 0.617 \frac{1}{10} \frac{1}{10} \text{ Bar}$ $N = (10.662 \div 0.20) + 1 = 36.54 \doteqdot 3.7 \text{ Bars}$ $1 = 1.550 \frac{1}{10} \text{ Bar}$ $W_4 = 37 \text{ Bars} \times 1.55 \times 0.617 \times 2 = 70.770$	
$W_{1} = 6 \text{ Bars } \times 14.016 \times 1.04 \times 2 = 174.920$ $(f) 10, w = 0.617 \text{ kgf/m})$ $n = (14.016 \div 0.30) + 1 = 47.72 \div 48 \text{ Bars}$ $1 - (0.20 \times 2 + 0.50 \times 2 + 15 \times 0.01) = 1.550 \text{ m/Bar}$ $W_{2} = 48 \text{ Bars} \times 1.55 \times 0.617 \times 2 = 91.810$ $\cdot \text{WALL} - 2$ $(f) 13, w = 1.04 \text{ kgf/m})$ $n = 6 \text{ Bars}$ $1 = (10.062 + 0.70) - 0.05 \times 2 = 10.662 \text{ m/Bar}$ $W_{3} = 6 \text{ Bars} \times 10.662 \times 1.04 \times 2 = 133.062$ $(f) 10, w = 0.617 \text{ kgf/m})$ $n = (10.662 \div 0.30) + 1 = 36.54 \div 37 \text{ Pars}$ $1 = 1.550 \text{ m/Bar}$ $W_{4} = 37 \text{ Bars} \times 1.55 \times 0.617 \times 2 = 70.770$	
$n = (14.016 \div 0.30) + 1 = 47.72 \doteqdot 48 Bars$ $L = (0.20 \times 2 + 0.50 \times 2 + 15 \times 0.01) = 1.550 \text{ M/par}$ $W_2 = 48 Bars \times 1.55 \times 0.617 \times 2 = 91.810$ $WALL - 2$ $(D13, W = 1.04 \text{ M/m})$ $n = 6 Bars$ $L = (10.062 + 0.70) - 0.05 \times 2 = 10.662 \text{ M/par}$ $W_3 = 6 Bars \times 10.662 \times 1.04 \times 2 = 133.062$ $(D10, W = 0.617 \text{ M/m})$ $n = (10.662 \div 0.20) + 1 = 36.54 \div 37 Bars$ $L = 1.550 \text{ M/par}$ $W_4 = 37 Bars \times 1.55 \times 0.617 \times 2 = 70.770$	
L = $(0.20 \times 2 + 0.50 \times 2 + 15 \times 0.01) = 1.550 \text{ M/Bar}$ $W_2 = 48 \text{ Bars} \times 1.55 \times 0.617 \times 2 = 91.810$ • WALL - 2 (D13. $W = 1.04 \text{ kgf/m}$) N = 6 Bars $L = (10.062 + 0.70) - 0.05 \times 2 = 10.662 \text{ M/Bar}$ $W_3 = 6 \text{ Bars} \times 10.662 \times 1.04 \times 2 = 133.062$ (D10. $W = 0.617 \text{ kgf/m}$) $N = (10.662 \div 0.20) + 1 = 36.54 \div 37 \text{ Bars}$ L = 1.550 M/Bar $W_4 = 37 \text{ Bars} \times 1.55 \times 0.617 \times 2 = 70.770$	
$W_{2} = 48 \text{ Bars} \times 1.55 \times 0.617 \times 2 = 91.810$ $W_{3} = 48 \text{ Bars} \times 1.04 $	
•WALL-2 (D13. $W = 1.04 \frac{196}{m}$) $h = 6 Bars$ $L = (10.062 + 0.70) - 0.05 \times 2 = 10.662 \frac{m}{Bar}$ $W3 = 6 Bars \times 10.662 \times 1.04 \times 2 = 1.33.062$ (D10. $W = 0.617 \frac{196}{m}$) $M = (10.662 \div 0.20) + 1 = 36.54 \div 37 Bars$ $L = 1.550 \frac{m}{Bar}$ $W4 = 37 Bars \times 1.55 \times 0.617 \times 2 = 70.770$	
(£13. $W = 1.04 \frac{kgf_{m}}{m}$) $h = 6 Bars$ $L = (10.062 + 0.70) - 0.05 \times 2 = 10.662 \frac{m}{Bar}$ $W3 = 6 Bars \times 10.662 \times 1.04 \times 2 = 133.062$ (£10. $W = 0.617 \frac{kgf_{m}}{m}$) $h = (10.662 \div 0.30) + 1 = 36.54 \div 37 Bars$ $L = 1.550 \frac{m}{Bar}$ $W4 = 37 Bars \times 1.55 \times 0.617 \times 2 = 70.770$	
(£13. $W = 1.04 \frac{196}{m}$) $N = 6 Bars$ $L = (10.062 + 0.70) - 0.05 \times 2 = 10.662 \frac{m}{Bar}$ $W3 = 6 Bars \times 10.662 \times 1.04 \times 2 = 133.062$ (£10. $W = 0.617 \frac{196}{m}$) $N = (10.662 \div 0.30) + 1 = 36.54 \div 37 Bars$ $N = 1.350 \frac{m}{Bar}$	and the second of the second
$h = 6 Bars$ $L = (10.062 + 0.70) - 0.05 \times 2 = 10.662 \text{ // Bar}$ $W3 = 6 Bars \times 10.662 \times 1.04 \times 2 = 133.062$ $(D10. W = 0.617 + 8 f/m)$ $1 = (10.662 \div 0.20) + 1 = 36.54 \div 37 Bars$ $1 = 1.550 \text{ // Bar}$ $W4 = 37 Bars \times 1.55 \times 0.617 \times 2 = 70.770$	
$L = (10.062 + 0.70) - 0.05 \times 2 = 10.662 \text{ M/Bar}$ $W3 = 6 \text{ Bars} \times 10.662 \times 1.04 \times 2 = 133.062$ $(£10. W = 0.617 \text{ kgf/m})$ $N = (10.662 \div 0.30) + 1 = 36.54 \div 37 \text{ Bars}$ $L = 1.550 \text{ M/Bar}$ $W4 = 37 \text{ Bars} \times 1.55 \times 0.617 \times 2 = 70.770$	
$W3 = 6 \text{ Bars} \times 10.662 \times 1.04 \times 2 = 133.062$ $(£10. W = 0.617 + 8 f/m)$ $N = (10.662 \div 0.30) + 1 = 36.54 \div 37 \text{ Bars}$ $L = 1.550 \text{ m/par}$ $W4 = 37 \text{ Bars} \times 1.55 \times 0.617 \times 2 = 70.770$	
(£10. $w = 0.617 + 8f_m$) $n = (10.662 \div 0.20) + 1 = 36.54 \div 27 $ Parp $1 = 1.550 \frac{m}{par}$ $w_4 = 27 $ Bars $\times 1.55 \times 0.617 \times 2 = 70.770$	
N = (10.662 ÷ 0.30) + 1 = 36.54	
) = 1.550 m/par W4 = 37 Bars x 1.55 x 0.617 x 2 = 70.770	
W4 = 37 Bars x 1.55 x 0.617 x 2 = 70.770	
$707AL(W_1+W_2+W_3+W_4) = 470.562 kgf$ 0.4	
	7/tf
<u>- 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1</u>	
	1.7 (A. 1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1

RESULT													29.605 m²					-
RES												 ·	29.60					
AT WH, 124 CALCULATION	t=10 ELASTIC MATERIAL	ω_{ALC}		$A_1 = \frac{1}{12346+13335} \times \frac{1}{2} \times 0.60 + \frac{10.70+0.558}{10.70+0.558}$	×1/2 × 0, 60 (× 2	5/89/=		WALL-2	Az= {(10:062+9.997) × 1/2 ×0.60+(0.70+0.558)	×1/2×0.60 4×2	12,790 and		TOTAL = 29,605					
TYPE OF WORK: TOINT FILL WITH HEAD AT W	LOCATION: END WALL (REVETHENT)		TOOK TO THE CONTRACT OF THE CO	81	D-10(@3co) D-13 GABION CYLINDER		2 06			END WALL								