lame of work	Height	Wide	Lenth	Qty.	Subtotal	Total
	<u>a (M) e est</u>	(M)	(M)		(M3)	(M3)
		2		A WAR AND		1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 -
	0,30	0.30	7,40	4.00	2.66	
	0,15	0.25	7.10	1,00	0.27	
	0.15	0.15	7.10	3.00	0.48	
	0.15	0,15	3.60	16.00	1.30	
	0.15	0.20	7,50	9.00	2.03	artis Antonio de Cartos
	0.25	0,50	7,50	4.00	3.75	
	0.20	0.30	7.50	4.00	1.80	
	0.15	0,20	3.75	1.00	0.11	
	0.15	0.20	7,50	1.00	0.23	
	0.15	0.20	2.00	11.00	0.41	
	0.15	0.20	3.75	3.00	0.21	
	0.15	0.20	1.50	2.00	0.21	
	f					
	0.15	0.20	1.75	5,00	0.26	an an tao
	0.15	0.20	1.00	2.00	0.06	
	0.20	0.30	7.50	1.00	0.45	
	0.25	0.50	7.50	1.00	0.94	tang site a Nation
	0.15	0,20	2.00	7.00	0.42	
	0.15	0.20	3.00	5.00	0.45	
	0.15	0.20	1,50	6.00	0.27	
	0.15	0.20	1.00	1.00	0.03	
	0.20	0.30	6.50	1.00	0.39	
	9.25	0.20	1.25	1.00	2.31	
	0.60	0.25	1.25	1.00	0.19	
	0.20	1,25	20.00	1.00	5.00	
	0.30	0,30	1.90	4.00	0.68	
	0.90	0.25	1.50	4.00	1.35	26.10

SIMONGAN CONTROL OFFICE VOLUME CALCULATION

Name of work	Diameter (mm)	Each	Lenth (M)	Qty.	Weight (Kg/m)	Total (Kg)
STEEL BAR	16.00	8,00	7.50	4.00	1.58	379.44
	16.00	4.00	7.20	1.00	1.58	45.53
	16.00	11.00	7.60	4.00	1.58	528.68
	16.00	6,00	7.60	4.00	1.58	288.37
	16.00	5.00	7.60	1.00	1.58	60.08
	16.00	11.00	7.60	1.00	1.58	132.17
	16.00	5.00	6,60	1.00	1.58	52.17
	25.00	2.00	0.80	1.00	3.86	6.18
	25.00	2.00	0.80	1.00	3.86	6.18
	16.00	1.00	9.14	7.00	1.58	101.15
	16.00	1.00	8.20	7.00	1.58	90.75
	16.00	8.00	2.35	4.00	1.58	118.89
	16.00	11.00	3.24	4.00	1.58	225.38
	16.00	11.00	3.24	4.00	1,58	225.38
	······································					2,260.35

		4					
	Name of work	Diameter (mm)	Each	Lenth (M)	Qty.	Weight (Kg/m)	Total (Kg)
	STEEL BAR	8.00	50,00	1.05	4.00	0.39	82.84
		8.00	48.00	0.65	1.00	0.39	12.31
		12.00	4.00	7.20	3.00	0.89	76.69
		8.00 12.00	48.00	0.45 3.70	3.00 16.00	0,39 0,89	25.56 210.19
		8.00	25.00	0.45	16.00	0.39	71.01
an a		12.00	4.00	7.60	9.00	0.89	242.85
		8.00	51.00	0.55	9,00	0.39	99.59
		12.00	2.00	7,60	4.00	0.89	53.97
		8,00	51.00	1.35	4.00	0.39	108.64
		8.00 12.00	51.00 4.00	0.85 3.85	4.00 1.00	0.39 0.89	68.40 13.67
		8.00	26.00	0.55	1.00	0.39	5.64
		12.00	4.00	7.60	1.00	0.89	26.98
		8.00	51.00	0.55	1.00	0.39	11.07
		12.00	4.00	2.10	11.00	0.89	82.01
		8.00	14.00	0.55	11.00	0.39	33.41
		12.00 8.00	4.00 26.00	3.85 0.55	3.00 3.00	0.89 0.39	41.01 16.92
		12.00	4.00	1.60	2.00	0.89	11.36
		8.00	11.00	0.55	2.00	0,39	4,77
		12.00	4.00	1.85	5.00	0.89	32.84
		8.00	12.00	0.55	5.00	0,39	13.02
		12.00 8.00	4.00 7,00	1.10 0,55	2.00 2.00	0.89 0.39	7.81
		8,00	51.00	0.85	1.00	0.39	17.10
		8.00	51.00	1.35	1.00	0.39	27.16
		12.00	4.00	2.10	7.00	0.89	52.19
		8.00	14.00	0.55	7.00	0,39	21.26
		12.00	4.00	3.10	5.00 5.00	0.89 0.39	55.03 22.78
		8.00 12.00	4.00	0.55 1.60	6.00	0.89	34.08
		8,00	11.00	0.55	6.00	0.39	14.32
		12.00	4.00	1.10	1.00	0.89	3.91
		8.00	7.00	0,55	1.00	0.39	1.52
		8.00	44.00	0.85	1.00	0.39 0.89	14.75 4.69
		12.00	2.00	2.64 0.50	1.00 1.00	0.89	0.89
		12.00	2.00	0.90	1.00	0.89	1,60
		8.00	1.00	1.85	1.00	0.39	0.73
		8.00	1.00	1.57	1.00	0.39	0.62
		8.00	1.00	1.37	1.00	0.39	0.54
	(1) (1) (1) (1) (1) (1) (1) (1) (1) (1)	8.00 8.00	1.00	1.05 0.81	1.00 1.00	0.39 0.39	0.41 0.32
			2.00	2.55	1.00	0.39	4,53
		12.00	2.00	0.50	1.00	0.89	0.89
		12,00	2.00	0.90	1,00	0.89	1,60
		8.00	1.00	1.57	1,00	0.39	0,62
		8.00	1.00	1.37 1.05	1.00 1.00	0.39 0.39	0.54 0.41
	가 이 같아요. 가지 말했 [편집] 제 12 년 13 년 14	8.00 8.00	1.00	0.81	1.00	0.39	0.32
		10.00	1.00	2.20	7.00	0.62	9,49
		10.00	1.00	1.40	7.00	0.62	6.04
		10.00	1.00	1.60	7.00	0.62	6.90
		10.00	1.00	2.10	7.00	0.62	9.06
		10.00 10.00	7.00	0.80 173.00	19.00 1.00	0.62 0.62	65.58 106.64
		8.00	13,00	1,05	4.00	0.82	21.54

				· .	ang ang Tanàna ang	
Type of Foundation		Wide	Height	Lengt	Sub-total	Total
Stone masonry		(m)	(m)	(m)	(m3)	(m3)
F-1	******	0.55	0,80	5.00	2.20	(110)
		0.55	0.80	7.50	3.30	4.4 1
		0.55	0.80	7.50	3.30	8,8
F-2		0.55	0.80	5.00	2.20	0.0
		0.55	0,80	0.38	0,17	
		0.55	0.80	2.00	0.88	3.2
F-4		0.55	0.80	7.50	3,30	5.2
		0.55	0.80	7.50	3.30	
		0.55	0.80	1.50	0.66	7.2
F-5		0.55	0.80	10.00	4.40	1.2
		0.55	0.80	1.50	4.40 0.66	
		0.55	0.80	1.50	0.66	
F-6	and a second s	0.55	0.80	5.00	2.20	5.7
		0.55	0.80	2.00	0.88	
		0.55	0.80	2.00	0.88	
		0.55	0.80	2.00	0.88	
		0.55	0.80	1.50	0.66	5.5
Type of Foundation	Height	Wide	Each	Unit	Sub-total	Total
Brick block	(m)	(m)	Laci	Ofm	(m3)	(m3)
F-7	0.25	0.40	2.00	2.00	0.40	(iiis)
	0.25	0.40	1,00	2.00	0.20	
	0.20	0.30	1.00	2.00	0.12	0.4
F-8	0.25	0.30	3.00	1.00	0.12	0.4
F-9	0.25	0.45	1.00	3.75	0.61	0.3
	0.25	0.65	1.00	1.50	0.24	
			· · · · ·	(a) A set of the se		
	0.25 0.25	0.65 0.65	1.00	7.00 7.50	1.14	
	0.25	0.65	1.00	2.00	(a) S = [2] (a) (b) (b) [1]	
$= \left[\begin{array}{c} -1 & -1 & -1 & -1 & -1 \\ -1 & -1 & -1 &$	0.25	1976 - 1976 - 1976 - 1976 - 1976 - 1976 - 1976 - 1976 - 1976 - 1976 - 1976 - 1976 - 1976 - 1976 - 1976 - 1976 -			0.33	
n <mark>La construction de la construction de la </mark>	U.20	0.65	1.00	6.50	1.06	4.59
Type of Foundation	<u> </u>	Wide	Height	Lengt	Sub-total	Tatal
Dry stone masonry		(m)	(m)	Lengt (m)	(m3)	Total (m3)
F-1		0.20	1.15	5.00	1.15	(m3)
		0.20	1.15	7.50	1.73	
医子 にも 見たい たけもうにいたう		0.20	1.15	7.50	1.73	4.60
						4.00
F-2				5 00	1 15	김 김 영영 것이 있어?
F-2		0.20	1.15	5.00 0.38	1.15	
F-2		0.20 0.20	1.15	0.38	0.09	1 7/
		0.20 0,20 0.20	1,15 1,15 1,15	0.38 2.00	0.09 0.46	1.70
F-2 F-4		0.20 0.20 0.20 0.20	1.15 1.15 1.15 1.15	0.38 2.00 7.50	0.09 0.46 1.73	1.70
		0.20 0.20 0.20 0.20 0.20 0.20	1,15 1,15 1,15 1,15 1,15 1,15	0.38 2.00 7.50 7.50	0.09 0.46 1.73 1.73	
F-4		0.20 0.20 0.20 0.20 0.20 0.20 0.20 0.20	1,15 1,15 1,15 1,15 1,15 1,15 1,15	0.38 2.00 7.50 7.50 1.50	0.09 0.46 1.73 1.73 0.35	
		0.20 0.20 0.20 0.20 0.20 0.20 0.20 0.20	1,15 1,15 1,15 1,15 1,15 1,15 1,15 1,15	0.38 2.00 7.50 7.50 1.50 10.00	0.09 0.46 1.73 1.73 0.35 2.30	
F-4		0.20 0.20 0.20 0.20 0.20 0.20 0.20 0.20	1,15 1,15 1,15 1,15 1,15 1,15 1,15 1,15	0.38 2.00 7.50 7.50 1.50 10.00 1.50	0.09 0.46 1.73 1.73 0.35 2.30 0.35	3.80
F-4 F-5		0.20 0.20 0.20 0.20 0.20 0.20 0.20 0.20	1.15 1.15 1.15 1.15 1.15 1.15 1.15 1.15	0.38 2.00 7.50 7.50 1.50 10.00 1.50 1.50	0.09 0.46 1.73 1.73 0.35 2.30 0.35 0.35	3.80
F-4		0.20 0.20 0.20 0.20 0.20 0.20 0.20 0.20	1,15 1,15 1,15 1,15 1,15 1,15 1,15 1,15	0.38 2.00 7.50 7.50 1.50 1.50 1.50 1.50 1.50 5.00	0.09 0.46 1.73 1.73 0.35 2.30 0.35 0.35 0.35 1.15	3.80
F-4 F-5		0.20 0.20 0.20 0.20 0.20 0.20 0.20 0.20	1,15 1,15 1,15 1,15 1,15 1,15 1,15 1,15	0.38 2.00 7.50 7.50 1.50 1.50 1.50 1.50 1.50 5.00 2.00	0.09 0.46 1.73 1.73 0.35 2.30 0.35 0.35 1.15 0.46	3.80
F-4 F-5		0.20 0.20 0.20 0.20 0.20 0.20 0.20 0.20	1,15 1,15 1,15 1,15 1,15 1,15 1,15 1,15	0.38 2.00 7.50 7.50 1.50 1.50 1.50 1.50 1.50 5.00	0.09 0.46 1.73 1.73 0.35 2.30 0.35 0.35 0.35 1.15	1.70 3.80 2.99

	<u>.</u>			- 1			
	Earth cut		T 148 -	°T			
			Wide	Height	Lengt	Sub-total	Tota
	F-1		(m) 1.40	(m) 0.90	(m) 5.00	(m3)	(m3)
			1.40	0.90	7,50	1	
		an an an Araba. An an an Araba an Araba	1.40	0.90	7.50	9,45	25
	F-2		1.40	0,90	5.00	6.30	
			1.40	0.90	0.38	0.47	
			1.40	0.90	2.00	2.52	9.
	F-4		1.40	0.90	7.50	9.45	
			1.40	0.90	7.50	9.45	
	F-5		1.40	0.90	1.50	1.89	20.
	1.4		1.40	0.90	10.00	12.60	
			1.40 1.40	0.90	1.50	1,89	
	F-6		1.40	0.90	1.50 5.00	1.89	16.3
			1.40	0.90	2.00	6.30 2.52	e et al. E ta
			1.40	0.90	2.00	2.52	
			1.40	0.90	2.00	2.52	
			1.40	0.90	1.50	1.89	15.7
	Earth cut	Wide	Height	Each	Cont Unit	Sub-total	Total
		(m)	(m)			(m3)	(m3)
	F-7	0.55	0.40	2.00	2.00	0.88	
		.0.55	0.40	1.00	2.00	0.44	
	F-8	0.50	0.30	1.00	2.00	0,30	1.0
	F-9	0.55	0.40	3.00 1.00	1.00	0.66	0.6
		0.55	0.65	1.00	3.75 1.50	1.34	
		0.55	0.65	1.00	7.00	0.54 2.50	
		0.55	0.65	1.00	7.50	2.50	
		0.55	0.65	1.00	2.00	0.72	
**		0.55	0.65	1.00	6.50	2.32	10.10
	Sand fill		VAR-	Цател I			
			Wide (m)	Height (m)	Lengt (m)	Sub-total	Total
	F-1		1.05	0.10	5.00	(m3) 0.53	<u>(m3)</u>
			1.05	0.10	7.50	0.33	
			1.05	0.10	7.50	0.79	2.10
	F-2		1.05	0,10	5.00	0.53	
 A state of the sta			1.05	0,10	0.38	0.04	
		a da an fan st	العضاد	0.10	2.00	0.21	0.77
			1.05				e je ta jtil
	F4	n an	1.05	0.10	7,50	0.79	1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1
	F4		1.05 1.05	0.10 0.10	7.50	0,79	
			1.05 1.05 1.05	0.10 0.10 0.10	7.50 1.60	0,79 0.16	1.73
	F-5		1.05 1.05 1.05 1.05	0.10 0.10 0.10 0.10	7.50 1.60 10.00	0,79 0.16 1.05	1.73
			1.05 1.05 1.05 1.05 1.05	0.10 0.10 0.10 0.10 0.10 0.10	7.50 1.60 10.00 1.50	0.79 0.16 1.05 0.16	
			1.05 1.05 1.05 1.05	0.10 0.10 0.10 0.10	7.50 1.60 10.00	0,79 0,16 1,05 0,16 0,16	1.73
	F-5		1.05 1.05 1.05 1.05 1.05 1.05	0.10 0.10 0.10 0.10 0.10 0.10 0.10	7.50 1.60 10.00 1.50 1.50	0.79 0.16 1.05 0.16	
	F-5		1.05 1.05 1.05 1.05 1.05 1.05 1.05 1.05	0.10 0.10 0.10 0.10 0.10 0.10 0.10 0.10	7.50 1.50 10.00 1.50 1.50 5.00 2.00 2.00 2.00	0,79 0.16 1.05 0.16 0.16 0.53	
	F-5		1.05 1.05 1.05 1.05 1.05 1.05 1.05 1.05	0.10 0.10 0.10 0.10 0.10 0.10 0.10 0.10	7.50 1.50 10.00 1.50 1.50 5.00 2.00 2.00 2.00 2.00	0.79 0.16 1.05 0.16 0.16 0.53 0.21 0.21 0.21	
	F-5 F-6		1.05 1.05 1.05 1.05 1.05 1.05 1.05 1.05	0.10 0.10 0.10 0.10 0.10 0.10 0.10 0.10	7.50 1.50 10.00 1.50 1.50 5.00 2.00 2.00 2.00 2.00 1.50	0,79 0,16 1,05 0,16 0,16 0,53 0,21 0,21 0,21 0,21 0,16	
	F-5	0.35	1.05 1.05 1.05 1.05 1.05 1.05 1.05 1.05	0.10 0.10 0.10 0.10 0.10 0.10 0.10 0.10	7.50 1.50 10.00 1.50 1.50 5.00 2.00 2.00 2.00 1.50 2.00	0,79 0,16 1,05 0,16 0,16 0,53 0,21 0,21 0,21 0,21 0,16 0,14	1.37
	F-5 F-6	0.35	1.05 1.05 1.05 1.05 1.05 1.05 1.05 1.05	0.10 0.10 0.10 0.10 0.10 0.10 0.10 0.10	7.50 1.50 10.00 1.50 5.00 2.00 2.00 2.00 1.50 2.00 1.50 2.00 2.00 2.00 2.00 2.00	0.79 0.16 1.05 0.16 0.16 0.53 0.21 0.21 0.21 0.21 0.16 0.14 0.07	<u>1.37</u> <u>1.31</u>
	F-5 F-6	0.35 0.35	1.05 1.05 1.05 1.05 1.05 1.05 1.05 1.05	0,10 0,10 0,10 0,10 0,10 0,10 0,10 0,10	7.50 1.50 10.00 1.50 1.50 2.00 2.00 2.00 1.50 2.00	0.79 0.16 1.05 0.16 0.16 0.53 0.21 0.21 0.21 0.21 0.16 0.14 0.07 0.07	1.37 1.31 0.14
	F-5 F-6 F-7	0.35 0.35 0.35	1.05 1.05 1.05 1.05 1.05 1.05 1.05 1.05	0.10 0.10 0.10 0.10 0.10 0.10 0.10 0.10	7.50 1.50 10.00 1.50 1.50 2.00 2.00 2.00 1.50 2.00 2.00 2.00 2.00 2.00 2.00 1.50 2.00 2.00 1.50	0.79 0.16 1.05 0.16 0.16 0.53 0.21 0.21 0.21 0.21 0.16 0.14 0.07 0.07 0.07	<u>1.37</u> <u>1.31</u>
	F-5 F-6 F-7 F-8	0.35 0.35 0.35 0.35	1.05 1.05 1.05 1.05 1.05 1.05 1.05 1.05	0.10 0.00 0.00	7.50 1.50 10.00 1.50 1.50 5.00 2.00 2.00 2.00 1.50 2.00 2.00 2.00 2.00 2.00 2.00 2.00 3.75	0.79 0.16 1.05 0.16 0.16 0.53 0.21 0.21 0.21 0.21 0.16 0.16 0.14 0.07 0.07 0.07 0.07	1.37 1.31 0.14
	F-5 F-6 F-7 F-8	0.35 0.35 0.35	1.05 1.05 1.05 1.05 1.05 1.05 1.05 1.05	0.10 0.10 0.10 0.10 0.10 0.10 0.10 0.10	7,50 1,50 10,00 1,50 1,50 2,00 2,00 2,00 2,00 1,50 2,00	0.79 0.16 1.05 0.16 0.16 0.53 0.21 0.21 0.21 0.21 0.16 0.14 0.07 0.07 0.07 0.07 0.11 0.13 0.05	1.37 1.31 0.14
	F-5 F-6 F-7 F-8	0.35 0.35 0.35 0.35 0.35 0.35	1.05 1.05 1.05 1.05 1.05 1.05 1.05 1.05	0.10 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00	7.50 1.50 10.00 1.50 1.50 5.00 2.00 2.00 2.00 1.50 2.00 2.00 2.00 2.00 2.00 2.00 2.00 3.75	0.79 0.16 1.05 0.16 0.16 0.53 0.21 0.21 0.21 0.21 0.16 0.16 0.14 0.07 0.07 0.07 0.07	1.37 1.31 0.14
	F-5 F-6 F-7 F-8	0.35 0.35 0.35 0.35 0.35 0.35 0.35	1.05 1.05 1.05 1.05 1.05 1.05 1.05 1.05	0.10 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00	7,50 1,50 10,00 1,50 1,50 2,00	0.79 0.16 1.05 0.16 0.16 0.53 0.21 0.21 0.21 0.21 0.16 0.14 0.07 0.07 0.07 0.11 0.13 0.05 0.25	1.37 1.31 0.14

			· .			•
· · · ·			e de la composition Notationes			•
Type of Truss	Dimention	Weight/m	Lengt	Each	Sub-total	Total
	and the second second	(kg)	(m)		(m3)	(m3)
TRUSS K-1	50,50.5	3,74	3.15	4.00	47.15	11.1.10.10.10.10.10.10.10.10.10.10.10.10
	an an an Artana an Ar	3,74	5.00	4.00	74,85	
		3.74	1.05	2.00	7.86	an af in an
		3.74	1.40	4.00	20,96	
		3.74	2.10	2.00	15.72	166,5
a an tha an t			1.00	X 1	166,54	166.5
TRUSS K-1'	50.50.5	3.74	3.15	4,00	47,15	
		3.74	5.00	4.00	74.85	
		3.74	1.05	2.00	7.86	
		3,74	1.40	4.00	20.96	
PIPEC	= 101.6 t=5.6	14.02	2.10	1.00	29.44	180.2
			1.00	Х	180.25	180.2
TRUSS K-2	50.50.5	3.74	3.15	2.00	23.58	
		3.74	2.50	2.00	18.71	
		3.74	1.05	2.00	7.86	
		3.74	1.40	2.00	10.48	60.6
			1.00	Х	60.63	60,6
IRUSS K-3	60.60.6	5.65	6.08	4.00	137.46	
		5.65	10.60	2.00	119,82	
		5,65	0.92	4.00	20.80	
		5.65	1.80	4.00	40.69	
		5.65	1.96	4.00	44,31	
		5.65	2.48	4.00	56.07	
PIPE D	≠ 101.6 t=5.6	14.02	3.30	1.00	46.26	465.4
		1.1228	1.00	X	465.41	465.4
RUSS K-4	60.60.6	5.65	6.08	2.00	68.73	
		5.65	5.30	2.00	59.91	
		5.65	0.92	2.00	10.40	
		5,65	1.80	2.00	20.35	
		5.65	1.96	2.00	22.16	
		5.65	2.48	2.00	28.03	209.58
			2.00	X	209.58	419.15
RUSS K-5	60.60.6	5.65	3.75	2.00	42.39	
		5.65	1.05	2,00	11.87	
		5.65	2.10	2.00	23.74	
		5,65	1.48	2.00	16.73	
		5.65	2.08	2.00	23.51	
		5.65	4,80	2.00	54.26	172.50
ante de la composición			2.00	X	172.50	345,00
RUSS KS-1	70.70.7	7.69	0.72	1.00	5.54	
		7.69	0.93	1.00	7,12	
	te Periodet Pi	7,69	0,96	1.00	7.39	20,04
			14.00	X	20.04	280,56
RUSS KS-2	70.70.7	7,69	0.78	1.00	5.96	
		7.69	0.60	1.00	4.62	
	() : : : : : : : : : : : : : : : : : : :	7.69	0.76	1.00	5.85	16.42
and the second	(生産) しかい かいしょみい		15.00	X	16.42	246.37

Name of work	Dimention	Weight/m	Lengt	Each	Sub-total	Total
		(kg)	(m)		(m3)	(m3)
Steel purtin	150.50.2.3.2	7.28	1.70	2.00	24.77	
		7.28	6,50	2.00	94,70	
		7.28	5.30	2.00	77.22	
		7.28	15.30	1.00	111.46	
		7.28	7,30	1.00	53,18	
	1 <u>1</u>	7.28	1.00	1.00	7.28	1 A.
		7.28	3.00	1.00	21.85	÷
		7.28	5.00	1.00	36.42	
		7.28	7.00	1.00	50.99	· · · · ·
		7.28	9,30	4.00	270.99	
		7.28	7.20	4.00	209.80	
		7.28	5,00	4.00	145.70	
		7.28	3.00	4.00	87.42	10 E E
		7.28	1.00	4.00	29.14	
		7.28	9,50	2.00	138.41	
		7.28	8.00	2.00	116.56	
		7.28	2.50	2.00	36.42	1,512.32
						1,312.32
						and so t
	영화 가지 말았다.					

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Name of work	Height	Wide	Lenth	Qty.	Subtotal	Total
	(M)	(M)	(M)		(M3)	(M3)
					. ·	
CONCRETE	0.15	0.15	3.80	14.00	1.20	
	0.15	0.20	2.00	9.00	0,54	
	0.15	0.20	1.40	4,00	0.17	
	0.15	0.20	5,00	4.00	0.60	
	0.15	0.20	3.00	4.00	0.36	
	0.15	0.15	5.00	2.00	0.23	
	0.15	0.15	3.00	2.00	0.14	
	0.15	0.20	4.00	8.00	0.96	
	0,15	0.20	2.00	6.00	0.36	
	0.15	0.15	4.00	3.00	0.27	
	0,15	0.15	2.00	2.00	0.09	4.91
	1	L			·	· · · · · · · · · · · · · · · · · · ·
Name of work	Diameter	Each	Lenth	Qty.	Weight	Total
talle of for	(mm)	Cuon	(M)		(Kg/m)	(Kg)
	(nui)		(141)		(((g))))	(1(9)
	10.00	100	2.05	14.00	n on	194.22
STEEL BAR	12.00	4.00	3.90	2011 N. 19	0.89	
	6.00	26.00	0.45	14.00	0.22	36.42
	12.00	4.00	2.10	9.00	0.89	67.23
	6.00	14.00	0.55	9.00	0.22	15.41
	12,00	4.00	1.50	4.00	0.89	21.34
	6.00	10.00	0.55	4,00	0.22	4.89
	12.00	4.00	5,10	4.00	0.89	72.57
	6.00	24.00	0.55	4.00	0.22	11.74
	12.00	4.00	3.10	4.00	0.89	44.11
	6.00	21.00	0,55	4.00	0.22	10.27
	12.00	4.00	5,10	2.00	0.89	36.28
	6.00	34.00	0.45	2.00	0.22	6.80
a de la calencia de la composición de l Composición de la composición de la comp	12.00	4.00	3,10	2.00	0.89	22.05
	6.00	21.00	0.45	2.00	0.22	4.20
	12.00	4.00	4.10	8.00	0.89	116.68
	6.00	27.00	0.55	8.00	0.22	26.41
	12.00	4.00	2.10	6.00	0.89	44.82
	1	{		6.00	0.83	10.27
	6.00	14.00	0.55		and the second	
	12.00	4.00	4.10	3.00	0.89	43.75
	6.00	27.00	0.45	3,00	0.22	8.10
	12.00	4.00	2.10	2.00	0.89	14.94
	6.00	14.00	0,45	2,00	0.22	2.80
						815.32
		····	n an			
Name of work	Wide	Height	Each	Unit	Sub-total	Total
a da serie da construction de la construcción de la construcción de la construcción de la construcción de la co A construcción de la construcción de	(m)	(m)			(m3)	(m3)
Earth cut F-1	1.35	1.10	1.00	14.00	20.79	
F-2	1.35	1.10	1.00	3.00	4.46	
F-3	1.35	1.10	1.00	14.00	20.79	
F-4	1,35	1.10	1.00	4.00	5.94	
F-5	1.35	1,10	1.00	8.00	11.88	
F-6	0.55	0.65	1.00	6.00	2.15	66.00
Name of work	Wide	Height	Each	Unit	Sub-total	Total
	(m)	(m)			(m3)	(m3)
Dry stone F-1	1.05	0.20	1.00	14.00	2.94	
masonry F-2	1.05	0.20	1.00	3.00	0.63	
F-3	1.05	0.20	1,00	14.00	2.94	
F-4	1.05	0.20	1.00	4.00	0,84	
	1 1144					and the second second
F-5	1.05	0.20	1.00	8.00	1.68	a e classificados de

ELECTRICAL BUILDING VOLUME CALCULATION

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;	Name of work		Wide	Height	Each	Unit	Sub-total	Total	
	Sand fill	F-1	(m) 1.1	(m) 5 0.10	1.00	14.00	(m3) 1.61	(m3)	-
		F-2	1.1	1	1.00		0.35		
		F-3	1.1		1.00	14.00	1.61	1	
		F-4	1.18		1.00	4.00	0.46		
		F-5 F-6	1.15		1.00	8,00	0.92	1	
		<u>r-o</u>	0.35	5 0.10	1.00	6.00	0.21	5.16	ļ
	Name of work		Wide	Height	Each	Unit	Sub-total	Total	יייי ^ו ד
		<u> </u>	(m)	(m)			(m3)	(m3)	
	Stone masonry	∑ F-1 : 1 11	0.63		1.00	14.00	7.00		7
		F-2 F-3	0.63		1.00	3.00	1.50		
		F-4	0.63		1.00 1.00	14.00 4.00	7.00 2.00		
		F-5	0.63		1.00	8.00	4.00	21,50	
		F-6	0.25		1.00	6.00	0,90	0.90	1
			1						-
	Earth fill	Wide	Height	Each	Stone m.	D. stone m.	Sand fill	Total	
	F-1	(m) 1.40	(m) 0.90	14.00	(m3) 7.00	(m3) 2.94	(m3) 1.61	(m3) 6.09	4
	F-2	1.40	0,90		1.50	0.63	0.35	1.31	
	F-3	1.40	0.90	14.00	7.00	2,94	1.61	6.09	
	F-4	1.40	0.90	4.00	2.00	0.84	0.46	1.74	
	F-5 F-6	1.40	0.90	8.00	4.00	1.68	0.92	3.48	
	1-40	0.55	0.65	6.00	0.90	<u> </u>	0.21	1.04	
		<u> </u>						19.74	
	Type of Truss		Dimention		Lengt	Each	Sub-total	Total]
	TRILOOMA	an an Araba an Araba. Tanàna amin'ny tanàna mandritra dia mampika mangka mangka mangka mangka mangka mangka mangka mangka mangka mang		(kg)	(m)		(m3)	(m3)	
	TRUSS K-1		50,50,5	3.74 3.74	1.30 1.20	4.00 4.00	19.46		
				3.74	0.92	4.00 4.00	17.96 13.77		n ne series Nationalista
				3.74	1.00	4.00	14.97		
				3.74	0 76	4.00	11.38		
			04-5-5	3.74	1.16	4.00	17.36		
		PIPE D= 101.	.0 (=3.6	, 14.02	1.70	1.00	23.83	118.74	
L		_		1	2.00	<u> </u>	118.74	237.47	
ſ	Type of Truss		Dimention	Weight/m	W	ide	Each	Sub-total	Tota
				(kg)	(r	n)		(m3)	(m3)
	TRUSS K-1	PLATE	8 mm	62.76	0.20	0.40	2.00	10.04	
				62.76	0.20	0.25	2.00	6.28	
				62.76 62.76	0.24 0.20	0.40 0.26	2.00 2.00	12.05 6.53	
				62.76	0.14	0.20	2.00	4.22	
			80.80.8	10.04	0.16	4.00	2.00	12.85	
	fa fi e se a cont. Contra contra		D = 22	2.98	0.40	2.00	4.00	9.54	61
				1	2.00	<u> </u>		61.50	123
F	Type of Truss		Dimention	Weight/m	Lengt	Each	Sub-total	Total	
				(kg)	(m)		(m3)	(m3)	
ente esti de L	TRUSS K-2		50.50.5	3.74	1.30	4.00	19.46		
	(a) A state of the state of	and the second		3.74	1.20	4.00	17.96		· · · ·
				3.74	0.92	4.00	13.77		a status -
					1 00	المملا			
				3.74	1.00 0.76	4.00 4.00	14.97		
					0.76	4.00 4.00 4.00	11.38		
				3.74 3.74	1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1	4.00		107.62	

Type of Truss	Dimention	Weight/m (kg)	Wide (m)	Each	Sub-total (m3)	Total (m3)
TRUSS K-2 PLATE	8 mm	62.76 62.76	0.20 0.4 0.20 0.2		10.04 6,28	
		62.76	0.24 0.4	0 2.00	12.05	
		62.76 62.76	0.20 0.2 0.2 0.14 0.2		6.53 4.22	
	80.80.8	10.04	0.16 4.0		6.43 9.54	55.08
	D = 22	2.98	0.40 2.0 1.00 X	<u>a 4.00</u>	55.08	55.08

		*				
Type of Truss	Dimention	Weight/m (kg)	Lengt (m)	Each	Sub-total (m3)	Total (m3)
TRUSS K-3	50.50.5	3.74	1.30	2.00	9.73	
		3,74	1.20	2.00	8,98	
		3.74	0.92	2.00	6.89	
		3,74	1.00	2.00	7.48	
		3.74	0.76	2.00	5.69	
		3.74	1.16	1.00	4.34	43.11
			2.00	x	43.11	86.22

Type of Truss	Dimention	Welght/m (kg)	Wide (m)	Each	Sub-total (m3)	Total (m3)
TRUSS K-3 PLATE	8 mm	62.76 .62.76	0.20 0.20	0.40 2.00 0.25 2.00	6,28	
		62.76 62.76	0.24 0.20	0.40 2.00 0.26 2.00	6.53	
	80.80.8	62.76 10.04	0.14 0.16	0.24 2.00 4.00 1.00	6,43	
	D = 22	2.98	0.40 2.00 X	2.00 4.00	9.54 55.08	55.0 110.1

Type of Truss	Dimention	Weight/m (kg)	Lengt (m)	Each	Sub-total (m3)	Total (m3)
TRUSS KS	70.70.7	7.69 7.69	0.56 0.44	1.00	4.31 3.38	
		7.69	0.63	1.00	4.81	12.50 149.96

Type of Truss	Dimention	Weight/m (kg)		/ide (m)	Each	Sub-total (m3)	Total (m3)
TRUSS KS PLATE	8 mm	62.76 62.76	0.14 0.10	0.16 0.20	2.00 2.00	2.81 2.51	
	D = 12	62.76 0,89	0.32 0.13	0.20 2.00	2.00 1.00	8.03 0.22	13.58
	0-12	0.03	12.00	X		13.58	162.92

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Name of work	Height	Wide	Lenth	Qty.	Subtotal	Total
	(M)	(M)	(M)		(M3)	(M3)
Concrete	0.25	0.30	4.35	8.00	2.61	
	1.50	0.15	4.35	2.00	1.96	
	0.15	0.25	3.35	1.00	0.13	1
	0,15	0.20	1.58	1.00	0.05	
	0.20	1	3,35	4.00	1,07	112
	0.20	0.40	3.30	2.00	0.53	
	0.15	0.20	1.50	1.00	0.05	
	0.20	1	3.25	4.00	0.52	
	0.15	1	3.25	1.00	0.10	
		1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1		4.00	0.65	
	0.20		3.25	1	, , , , ,	:
	0.20		3.30	2.00	0.33	
	0.15	0.25	3,35	1.00	0.13	
	0.20	- 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1	10.00	1.00	0.60	
and a state of the second s	0.15		3.50	1.00	0.13	, 1 1 .
가 말 친구 않는	0.25	1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1	7.00	4.00	3,15	
	0.15	0.25	3.50	1.00	0.13	
	0.20	0.20	1.08	14.00	0.60	
	0,15	0.20	3.50	1.00	0.11	
	0.15	0.20	1.50	2.00	0.09	
	0.20	2 South an exercise	7.00	4.00	1.40	
	0.12	1	8,95	1.00	12.83	6 - 1 ^{- 1} - 1 - 1
	0.10	1 1. N. A. N.N.N.	42.60	1,00	1.70	
	0.12		2.00	1.00	0.87	
	0.12	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	2.00	1.00	0.83	30.
<u> </u>	0.12	3.47	2.00	1.00	0.00	
63 E. 1.1.1	Diameter	Each	Lenth	Qty.	Weight	Total
Name of work	1 1 1	Edvii	 A State of the sta	αιγ.	(Kg/m)	(Kg)
en en en fan en de service en de fan en de service en En en	(mm)		(M)		(((())))	(1.3)
		~~~	4.45	8.00	1.58	337
Steel bar	16.00	6,00	البر الدام الأراج ال	1	1.58	56.
	16.00	4.00	4.45	2.00	L	
	16.00	5.00	3.45	1.00	1.58	27.
	16.00	1 A.A.A.	1.60	1.00	1.58	10.
	16.00	A second seco	3,40	2.00	1.58	86
	16.00		1,60	1.00	1,58	10.
	16.00	4.00	3.45	1.00	1.58	21.
	16.00	4.00	3.40	2.00	1.58	43.
	16.00	5.00	3.45	1.00	1,58	27.
	16.00		10.10	1.00	1.58	79.
	16.00		3.60	1.00	1.58	22.
	16.00		7.10	4.00	1.58	449
	16.00	 A state of the sta	7.10	4.00	1.58	179.
						1,350
		and a state of the				
Name of work	Height	Wide	Lenth	Qty.	Subtotal	Total
	(M)	(M)	(M)		(M3)	(M3)
Earth cut	1.20	the second s	42.00	1.00	45.36	
Ediul VUL			6,85	1.00	0.96	
	0.35	[1] P. B. K. K. M.	and the second	14.00	25.20	71
	1.20		1.20	the second s		
Earth fill	0.68		42.00	1.00	19.99	
(a) R. S. S. M. S. M.	0.07	2	6.85	1.00	0.10	
		0,94	1.20	14.00	15,79	35
	1.00					
Sand fill	1.00		42.00	1.00	4.20	
Sand fill	a second and the second se	0.10	42.00 6.85	1.00	0.34	
Sand ful	1.00	0.10 0.10	 A the first second secon		1	4.

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Name of work	Height	Wide	Lenth	Qty.	Subtotal	Total
****	(M)	(M)	(M)		(M3)	(M3)
tone masonry	0.55	0.80	42.00	1.00	18.48	
	0,30	0.43	6.80	1.00	0.88	19.36
rick masonry	0.30	0.15	14.00	1.00	0.63	0.63
		·				
ame of work	Diameter	Each	Lenth	Qty.	Weight	Total
	(mm)		(M)		(Kg/m)	(Kg)
teel bar	8.00	30.00	0.95	8.00	0,39	89.94
	8.00	30.00	0.45	2.00	0.39	
	8.00	23.00	0.65	1.00	0.39	10.65
	8.00	11.00	0.55	1.00		5.90
	12.00	8.00	3.45	4.00	0.39	2.39
	8.00	23.00	1.05	4.00	0.89	97.99
	8.00	23.00	1.05	· · · · · · · · · · · · · · · · · · ·	0.39	38.11
	8.00	11.00	0.55	2.00	0.39	19,05
	12.00	4.00		1.00	0.39	2.39
	8.00	22.00	3.35	4.00	0.89	47.58
	8.00	23.00	0.65	4.00	0.39	22.56
	12.00	4.00	0.55	1.00	0.39	4.99
	8.00	23.00	3.45	4.00	0.89	49.00
	8,00	and the second	0.75	4.00	0.39	27.22
	8.00	23,00	0.75	2,00	0.39	13.61
	8.00	23.00	0.65	1.00	0.39	5.90
	8.00	67.00	0.85	1.00	0,39	22.47
		24.00	0.65	1.00	0.39	6.15
	12.00	2.00	7.10	4.00	0.89	50.42
	8.00	47.00	1.25	4,00	0.39	92,71
	12.00	4.00	3.60	1.00	0.89	12.78
	8,00	24.00	0.65	1.00	0.39	6.15
	12.00	5.00	1.18	14.00	0.89	73.01
	8.00	8.00	0,65	14.00	0.39	28.72
	12.00	4.00	3.60	1.00	0.89	12.78
	8.00	24.00	0.55	1.00	0,39	5.21
	12.00	4.00	1.60	2.00	0.89	11.36
	8.00	11.00	0.55	2.00	0.39	4.77
	8.00	47.00	0.75	4.00	0.39	55.62
	10.00	82.00	9.25	1.00	0.62	467.53
	6.00	82.00	9.25	1.00	0.22	168,31
	10.00	82.00	2.03	2.00	0.62	204.70
	10.00	82.00	1.85	1.00	0.62	93.51
	12.00	41.00	3.62	1.00	0.89	131.74
	12.00	41.00	3.62	1.00	0.89	131.74
	10.00	62.00	12.20	1.00	0.62	466.24
	6.00	62.00	12.20	1.00	0.22	167.85
	10.00	62.00	1.98	2.00	0.62	150.95
	10.00	62.00	1.70	2.00	0.62	129.94
	12.00	25,00	2.05	1.00	0.89	45.49
	12.00	25.00	2.05	1.00	0.89	
	12.00	82.00	4.05	2.00	0.89	45.49
a sa ang sa at at 🖡 s	12.00	3.00	12.20	2.00	0.89	589.55
	12.00	62.00	4.05	2.00	0.89	64.97
	12,00	3.00	9.25	2.00		445.76
	12.00	82.00	0.89		0.89	49.26
	12.00	6,00	12.20	2.00	0.89	129.55
	12.00	62.00	0.89	2.00	0.89	129.95
	12.00	6.00	9.25	2.00	0.89	97.96
		0.00	8.23	2.00	0.89	98,52

- APPLAR PAULINUM		AWAAAAC 1/AL 1040	041 010 1700H
OFFICE EQUIPMENT		STORAGE VOLUME	CALCULATION.
	A 10110314-07014-06		0/10/0/0/0/0/0/0/0/0/0/0/0/0/0/0/0/0/0/

Name of work	Height (M)	Wide (M)	Lenth (M)	Qty.	Subtotal (M3)	Total (M3)
CONCRETE	0.30	0.30	4.35	16.00	6.26	
	0.25	0.45	5.00	14.00	7.88	
	0.20	0.25	5.00	14.00	3.50	1
	0.15	0.15	5.00	1.00	0.11	
	0.25	0.45	6,50	8.00	5.85	
	0,15	0.15	6,50	1.00	0,15	
	0.15	0.15	4.35	7.00	0.69	
	0.20	0.30	5.00	7.00	2.10	
	0.15	0.20	5.00	1.00	0.15	
	0,40	0.10	74.30	1.00	2.97	
			*: ** **			
	0.40	0.10	17.30	1.00	0.69	
	37.15	8.65	0.12	1.00	38.56	
	29,50	1.50	0.12	1.00	5.31	
	0.20	0.25	6.50	8.00	2.60	
	0.15	0.20	2.00	1.00	0.06	
	0.20	0.20	1.08	36.00	1.55	
	0.20	0.20	1.50	12.00	0.72	79,15

÷	÷.,	e		÷ .								з <u>с</u>					1.11	
			1.1	- 21	÷	. 11			- C.	÷.	$\mathcal{L} = \mathcal{L}$	1		1.11	1.1		1.1	
Ń	2						1				- 9	1.57	•			1	- 193	11.

Name of work	Diameter (mm)	Each	Lenth (M)	Qty.	Weight (Kg/m)	Total (Kg)
STEEL BAR	16.00	12.00	4.45	16.00	1.58	1,350,79
	16.00	9.00	5.10	14.00	1.58	1,015.94
	16.00	4.00	5.10	1.00	1.58	32.25
	16.00	9.00	6.60	8.00	1.58	751.28
	16.00	4.00	6,60	1.00	1.58	41.74
	16.00	4.00	4.45	7.00	1.58	196.99
	16.00	6,00	5.10	7.00	1.58	338.65
	16.00	4.00	5.10	1.00	1,58	32.2
	16.00	4.00	2.10	1.00	1,58	13.2
	10.00	1.00				3,773.17

Name of work	Height (M)	Wide	Lenth (M)	Qty.	Subtotal (M3)	Total (M3)
		(M)		1.00		(1415)
EARTH CUT	1.20	0.90	103.00	1.00	111.24	
이 집안 이 같은 것 같아요. 이 것 같	0.35	0.40	51.50	1.00	7.21	
	1.20	1.25	1.20	24.00	43.20	181.65
EARTH FILL	0.68	0.70	103.00	1.00	49.03	
	0.07	0.20	51.50	1.00	0.72	
	1.00	0.94	1.20	24.00	27.07	76.82
SAND FILL	1.00	0.10	103.00	1.00	10.30	
	0.20	0.10	51.50	1.00	1.03	
	1.20	0.10	1.20	24.00	3.46	14.79
DRY STONE MASONRY	1.10	0.20	103,00	1.00	22.66	22.66
STONE MASONRY	0,55	0.80	103.00	1.00	45,32	45,32
BRICK BLOCK	0.30	0.15	26.00	1.00	1.17	
	0.30	0.65	25.05	1.00	4,88	6.05

	Name of work	Diameter	Each	Lenth	Qty,	Weight	Total
		(mm)		(M)		(Kg/m)	(Kg)
5	STEEL BAR	8.00	30.00	1.05	16.00	0.39	198.82
		12.00	2.00	5,10	14.00	0.89	126.75
		8.00	34.00	1.25	14.00	0.39	234.72
· ·		12.00	4.00	5,10	14.00	0.89	253.50
		8.00	34.00	0.75	14.00	0.39	140.83
		8.00	34.00	0.45	1.00	0.39	6.04
		12.00	2.00	6.60	8.00	0.89	
		8.00	44.00	1.25	8.00	0.89	93.73
		8.00	44.00	0.45	1.00		173.58
		8,00	30.00	0.45	7.00	0.39	7.81
		8.00	34.00	0.85	7.00	0.39	37.28
		8.00	34.00	0.55	1.00	0.39	79.81
1		12.00	4.00	6.50	and the second	0.39	7.38
		8.00	44.00	0.75	8.00	0.89	184.62
		8.00	14.00	· · ·	8.00	0.39	104.15
		10.00	248.00	0.55	1.00	0.39	3.04
		10.00		8.70	1.00	0.62	1,329.93
			248.00	8.70	1.00	0.62	1,329.93
		10.00	248.00	1.93	2.00	0.62	588.53
	영상 김 승규는 것이 같다.	10.00	248.00	1.65	1.00	0.62	252.23
		10.00	58.00	37.20	1.00	0.62	1,329.93
		6.00	58.00	37.20	1.00	0.22	478,77
		10.00	58.00	1.73	2.00	0.62	123.34
		10.00	58.00	1.25	13.00	0.62	580.95
		10.00	197.00	1.55	1.00	0.62	188,22
		12.00	197.00	1.55	1.00	0.89	271.03
		12.00	11.00	29,65	1.00	0.89	288.52
		10.00	11.00	29.55	1.00	0.62	200.36
	n de la companya de l	<u></u>	ADLA (1418)				8,613.78

CONTROL HOUSE 1 IN SIMONGAN WEIR

······						
Name of work	Height	Wide	Lenth	Qty.	Subtotal	Total
	<u>(M)</u>	(M)	(M)		(M3)	(M3)
Concrete	0.25	0.25	3.25	4.00	0.81	
$\left(\frac{1}{2} + \frac$	0.15	0.15	3,25	2.00	0.15	
	0.20	0.25	6.45	4,00	1.29	
	0.20	0.40	6.45	4.00	2.06	
	0.20	0.20	3.60	2.00	0.29	
	0.20	0.20	1.80	2.00	0.14	4.74
		ayan tarihi ta	e din a da ci	· · · · · · · · · · · · · · · · · · ·	·····	
Name of work	Steel bar	Each	Lengt	Qty	Weight	Total
	Diameter			and and an and a second se	Kg/m	Weight
STEEL BAR	16.00	3.35	4.00	4.00	1.58	84,63
	16.00	6.55	4.00	4.00	1.58	165,48
	16.00	6.55	4.00	9.00	1.58	372.32
		요즘 문제 문제				622.43
STEEL BAR	12.00	3,35	2.00	4.00	0.89	23.79
	8.00	0.85	4.00	33.00	0.39	44.26
	8.00	0.45	4.00	22.00	0.39	15,62
	8.00	0.75	4.00	44.00	0,39	52.07
	8.00	1.05	4.00	44.00	0.39	72,90
	12.00	3.70	2.00	4.00	0.89	26.27
	8.00	0.65	2.00	25.00	0.39	12.82
	12.00	1.90	2,00	4.00	0.89	13.49
	8.00	0.65	2.00	13.00	0.39	6.67
		light in Norsent				267.90

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CONTROL HOUSE 2 IN SIMONGAN WEIR

Name of work	Height	Wide	Lenth	Qty.	Subtotal	Total
	(M)	(M)	(M)		(M3)	(M3)
	0.25	0.25	3.25	6.00	1.22	
	0.15	0.15	3.25	4.00	0.29	
	0.20	0.25	6.48	4.00	1.30	
	0.20	0.25	6,60	3.00	0.99	
	0.20	0.40	6.48	4.00	2.07	
	0.20	0.40	6.60	2.00	1.06	
	0.20	0.20	3.60	2.00	0.29	
	0.20	0.20	1.80	2.00	0.14	7.36

Name of work	Height	Wide	Lenth	Qty.	Subtotal	Total
	(M)	(M)	(M)		(M3)	(M3)
STEEL BAR	16.00	3.35	4.00	6.00	1.58	126.95
	16,00	6,58	4.00	4.00	1.58	166.11
	16.00	6,70	4,00	3.00	1.58	126.95
	16.00	6.58	9.00	4.00	1.58	373.74
	16.00	6.70	9.00	2.00	1.58	190,42
						984.17
STEEL BAR	12.00	3.35	4.00	4.00	0.89	47.58
	8.00	0.85	33,00	6.00	0.39	66.39
	8.00	0.45	22.00	4.00	0.39	15.62
	8.00	0.75	44.00	4.00	0.39	52.07
	8.00	0.75	45.00	3.00	0.39	39,94
	8.00	1.05	44.00	4.00	0,39	72.90
	8.00	1.05	44.00	2.00	0.39	36.45
	12.00	3.70	2.00	2.00	0.89	13.14
	8.00	0.65	2.00	2.00	0.39	1.03
	12.00	1,90	2.00	2.00	0.89	6.75
	8.00	0.65	2.00	2.00	0.39	1.03
	NAME IN					352.89

SECURITY HUT VOLUME

l

	· · · · · · · · · · · · · · · · · · ·					
Name of work	Height	Wide	Lenth	Qty.	Subtotal	Total
	(M)	(M)	(M)		(M3)	(M3)
CONCRETE	0,15	0.15	3,35	7.00	0.53	
	0.15	0,20	17.00	1.00	0,51	
	0.15	0.20	17.00	1.00	0.51	1.55
an a						
Name of work	Steel bar	Each	Lengt	Qty -	Weight	Total
	Diameter				Kg/m	Weight
STEEL BAR	12.00	3.45	7.00	4.00	0.89	85,74
	8.00	0.45	7.00	23.00	0.39	28.58
	12.00	3.55	4.00	4.00	0,89	50.42
 Algebraic constraints of the second se	12.00	1.55	2.00	4.00	0.89	11.01
	8.00	0.55	4.00	24.00	0.39	20,83
(4) A set of the se	8.00	0.55	2.00	11.00	0.39	4.77
	12.00	3.55	4.00	4.00	0.89	50.42
	12.00	1.55	2.00	4.00	0.89	11.01
na hair a baile ann an t-aire ann an t-airte. Tha tha ann an t-airte ann an t-airte	8.00	0.55	4,00	24.00	0.39	20.83
	8.00	0.55	4.00	11.00	0.39	9.55
and a finite of an angle in the sector of th		in a contra				293.15
ng an talah sa	alara (alara) a sa sa					n service de la deservice A constant de la deservice de l A constant de la deservice de la
Name of work		W	H	Each	Sub-total	Volume
		(M)	(M)		(M3)	(M3)
EARTH CUT		1.80	1.05	16.03	30.29	30.29
EARTH FILL		1.20	0.80	16.03	15.38	15.38
SAND FILL		0,10	1.05	16.03	1.68	1.68
DRY MASONRY		0.15	1.15	16.03	2.76	2,76
STONE MASONRY		0.55	0.80	16.03	7.05	7.05
BRICK BLOCK		0.30	0.45	3.60	0,49	0.49

TYPE OF WORK : BOX CULVERT LOCATION :

: Weir Management Complex

CALCULATION		RESULT
CONCRETE (TYPE-CI)		
		e da se est
$A = 3.7 \times 3.11 - 3.0 \times 2.26 + 0.2 \times 0.2 + 2 \times 4$	= 4.807	
T 4000 01 (C0 00)		
$L = 4.000 : Sin (68.0^{\circ})$	= 4.314	
n en		
$\overline{V} = 4.807 \times 4.314 + 0.35 \times 0.35 : 2 \times 3.11 \times 4$	$= 21.499 \text{ m}^3$	21.499 m ³
	21,777 III	21.433 11
FORM		and a second
A OUTSIDE = $3.11 \times 4.314 \times 2$	= 26.833	
$A \text{ INSIDE} = 2.26 \times 4.314 \times 2 + 3.0 \times 4.314$	= 32.441	8
TOTAL	- 50.094	50.074
<u>in an an</u>	= 59.274	59.274 m ²
CONCRETE (TYPE-CI)	<u>n na spisie se anna spisie se anna s</u> Star anna stàitean anna an t-	
$1, A = 1.0 \times 1.0$	= 1.000	
2. $A = (0.1 + 3.61) \times 2.8 : 2$	= 5.194	
3. $A = 3.99 \times 3.61 - 3.236 \times 2.260 + 0.2 \times 0.2 : 2 \times 4$	= 7.171	
A A (1.00C+2.CIO) 1.0		
4. $A = (1.886 + 3.610) \times 1.9$:	= 5.221	
TOTAL	= 18.586	
	- 16.380	
$V = 18.586 \times 0.350$	= 6.505	6.505 m ³
		0.505 III
FORM		
	= 18.586	
	= 18.586	
A SIDE = $1.0 \times 0.35 + 1.886 \times 0.35$	= 1.010	
άλτα τ	- 20102	
TOTAL	= 38.182	38.182 m ²
<u>a de la construcción de la constru Construcción de la construcción de l</u>	n en transministra de la Angleria. Angleria de la companya de la companya	
and a state of the second state The second state of the second s		
	and the second secon	



TYPE OF WORK : BOX CULVERT LOCATION :

:

CALCULATI	ION	RESULT
CONCRETE (TYPE -CI)		an a
$1. A = 1.0 \times 0.444$	= 0.444	
2. $A = (0.1 + 3.610) \times 2.816 : 2$	= 6.491	
$3. A = 3.99 \times 3.610 - 3.236 \times 2.260 + 0.2 \times 10^{-3}$	$0.2: 2 \times 4 = 7.171$	
4. $A = (1.886 + 3.610) \times 2.816 : 2$	= 6.491	
	- 0.491	
$5. A = 1.0 \times 0.444$	= 0.444	
TOI	<u>`AL = 21.041</u>	an ta kan da da shi ya m
$V = 21.041 \times 0.35$	= 7.364	7.364 m ³
FORM	n a bha chuir a bha chuir an bha chartan 18 chartan an 19 chartan. Ta chuir chuir a bha chuir chuir chuir ann an ta chuir chuir an ta	
A FRONT	= 21.041	
A BACK	= 21.041	
A SIDE = $1.0 \times 0.35 \times 2$	= 0.700	
	an a	and the second second
TOT	AL = 42.782	42.782 m ²
TOTAL A,B,C		a an
CONCRETE = 21.499 + 6.505 + 7.364	= 35 368	35.368 m ³
FORM = $59.274 + 38.182 + 42.782$		140.238 m ²
	n - An the second s	
		and the state
SUPPORTING		
$V = 2.26 \times 3.0 \times 4.314$	$= 29.249 \text{ m}^3$	29.249 m ³
SCAFFOLDING	ne standen er en	
UCAT I UNDING	ter per el caracter y de la terre de la construction de la terre de la construction de la terre de la construct En alterna de la construction de la	
SECTION A-A	en en en la companya de la companya En la companya de la c	
	en e	
$A = 3.610 \times 4.314 \times 2$	= 3.142	
SECTION B-B		
$A = 9.347 \times 3.610 + (9.377 - 3.99) \times 3.610$	Sec	and the second second

TYPE OF WORK : BOX CULVERT LOCATION :

	CALCULATION		RESULT
SECTION C-C			na i Alta da Ara
$A = 10.590 \times 3.610 + (10.59)$	9 – 3.99) x 3.610	= 62.056	
	TOTAL	= 115.318	115.318 m ²
	IOIAL	115.516	113.510
LEVELING CONCRETE			
ΤΥΙΏΡ Γ			
ТҮРЕЕ			 A state of the sta
$\overline{A} = (3.99 + 0.1 \times 2) \times 0.1$	and a second	= 0.419	n an
$V = 0.419 \times 4.314$		= 1.808	1.808 m ³
FORM	an an an an Anna an Anna an Anna an Anna Anna an Anna an Anna an Anna an Anna an Anna		
FUNM			
(3.99 + 0.20) x 0.1 x 2		= 0.838	
(4.70 + 0.20) x 0.1 x 2		= 0.980	
	TOTAL	= 1.818	1.818 m ²
		1.010	1.010 //
SURFACE (A,T,B)			
and the second		en ang produktion persent	
$\frac{t \text{ ave }}{1 \text{ ave }} = (0.1 + 0.1 +$		= 0.125	
	15) : 2 9.397) : 2	= 0.125 = 9.994	
L ave = (10.59 + 10.59)	9.397) : 2		
		= 9.994	
L ave = $(10.59 + 2.0\%)$	9.397) : 2		
L ave = (10.59 + 10.59	9.397) : 2	= 9.994	
L ave = $(10.59 + 2.0\%)$	9.397) : 2	= 9.994	
L ave = $(10.59 + 2.0\%)$	9.397) : 2	= 9.994	
L ave = $(10.59 + 2.0\%)$	9.397):2	= 9.994	
L ave = $(10.59 + 2.0\%)$	9.397):2	= 9.994	
L ave = $(10.59 + 2.0\%)$ 2.0% 2.000 2.000 2.000 2.000 2.000 2.000 2.000 2.000 2.000 2.000	9.397):2	= 9.994	A 1073
L ave = $(10.59 + 2.0\%)$	9.397):2	= 9.994	4.197.m ³
L ave = $(10.59 + 2.0\%)$ 2.0% 2.000 2.000 2.000 2.000 2.000 2.000 2.000 2.000 2.000 2.000	9.397):2	= 9.994	4.197 m ³ 9.653 t
L ave = $(10.59 + 2.0\%)$ 2.0% 2.000 2.000 2.000 2.000 2.000 V = 0.42 × 9.994	9.397):2	= 9.994 = 0.419 = 4.197	

TYPE OF WORK : BOX CULVERT LOCATION :

CALCULATION		RESULT
A = (0.5 + 4.5) + 2.0 + 2.1 + 2.0 + 4.5 + (0.0 + 4.5)		
$A = (9.5 + 4.5) \times 3.0 : 2 + 2.0 \times 4.5 + (8.0 + 4.5)$ × 3.0 : 2	= 48.75 m ²	
X J.U. Z	= 48.75 m	
$V = 48.750 \times 3.710$	= 180.863	180.863 m ³
REINFORCING BAR		
[BOX CULVERT SECTION]		
D1(AW = 1.50 + 4.5)	en al su charachtaí an tha agus an 11 ga leann an tha ann an tha an tha	
D16 (W = 1.58 kg/m)		
$L_1 = 3.52 \text{ m/Bar x 4 Bar x 2}$	= 28.16 m/m	
	20.10 11/11	
$L_2 = (1.76 + 1.45 + 30 \times 0.016) \times 4 \times 8 Bar$	= 118.08 m/m	
$L_3 = 2.905 \times 8 \text{ Bar}$	= 23.24 m/m	
$L_4 = 1.00 \text{ m/Bar x 42 Bar x 2}$	= 84.00 m/m	
$W_1 = 253.48 \text{ m/m} \times 1.58 \text{ kg/m} = 400.50 \text{ kg/m}$	= 0.401 t/m	
	0.401 011	
D13 (W = 1.04 kg/m)	n an	
$L_5 = (0.85 + 15 \times 0.013 \times 2) \times 4 \text{ Bar x } 2$	= 9.92 m/m	
	0.010.14	
$W_2 = 9.92 \text{ m/m x } 1.04 \text{ kg/m} = 10.32 \text{ kg/m}$	= 0.010 t/m	
$\Sigma W_1 = 0.411 t/m \ge 5.10$	= 2.096 t	
	- 2.0901	
[WING WALL SECTION]		
	en de la construcción de la construcción En la construcción de la construcción	
D16 (W = 1.58 kg/m)		
$L_1 = 10.41 \times 3 \text{ Bars x } 2$	= 62.46 m/m	
$L_2 = (5.205 + 0.80) \times 1/2 \times 10 \times 2 \times 2$	- 120 10 /	
L2 - (J.20J 0.00) X 1/2 X 10 X 2 X 2	= 120.10 m/m	
$L_3 = 3.00 \times 2 \text{ Bars} \times 2 \times 2$	= 24.00 m/m	
$L_4 = 1.35 \times 9 \text{ Bars } \times 2 \times 2$	= 48.60 m/m	

TYPE OF WORK : BOX CULVERT LOCATION : :

CALCULATION	RESULT
$W_3 = 255.16 \text{ m/m x } 1.58 \text{ kg/m} = 403.15 \text{ kg/m} = 0.403 \text{ t/place}$	
D13 (W = 1.04 kg/m)	
$W_4 = (0.82 + 3.405) \times \frac{1}{2} \times 19 \text{ Bars } \times 1.04 = 41.74 \text{ kg}$	
$\Sigma W_2 = 0.445 t/place x 2 place = 0.890 t$	
TOTAL = 2.986	2.986 t
	en la substance de la substance
이 것이 있는 것이 같은 것은 관계를 갖추지 않는 것이 같은 것이 것을 것 같다.	
	ala albani ana Arifi ana Arifi ang
	an a
	$\mathcal{X}^{(1)} = \{i_1, \dots, i_n\}$
en in each an an an agus an airtean ann an Annaidh a' fhair ann an Annaichtean Annaichtean Annaichtean ann an Annaichtean ann annaichtean ann ann annaichtean ann ann annaichtean ann annaichtean ann ann annaichtean annaich	
an gan a sa kanang kanang kanang kanang baga kanang kanang kanang kanang kanang kanang kanang kanang kanang ka Kanang kanang	الا براسان ۲۰۰۵ و از
a na ban na ang ang ang ang ang ang ang ang an	

TYPE OF WORK: BOX CULVERT FOR REVETMENTLOCATION: RIGHT SIDE::

CALCU	ILATION	RESULT
GRAVEL BEDDING		
1. REVETMENT		
(0.7.) 2.0	= 1.125	
$A = (3.7 + 3.8) \times 0.3 / 2$	1.125	
$V = 1.125 \times (4.5 + 3.4) / 2$	≕ 4.444	
2. BASE CONCRETE		Martin Martin and State and Stat State and State and Stat State and State
	= 0.272	
$V = 0.8 \text{ m}^3 / 10.0 \text{ m} \times 3.4 \text{ m}$	0,272	
3. TOP CONCRETE		
$V = 0.540 \text{ m}^3 / 10.0 \text{ m x } 4.5 \text{ m}$	= 0.243	
	TOTAL = 4.959	4.959 m ³
	TOTAL = 4.959	4.737 11
WET STONE MASONRY		
$A = (4.2 + 3.8) \times 0.3 / 2$	= 1.2	
	= 4.74	4.740 m ³
$V = 1.2 \times (4.5 + 3.4) / 2$	= 4,74	4.740 III
CEMENT MORTAR POINTING		
$A = 4.2 \times (4.5 + 3.4) / 2$	= 16.59	16.590 m ³
		l de la constante de la consta La constante de la constante de
CONCRETE TYPE C1		
1. BASE CONCRETE		n an
$W = 3.15 \text{ m}^3 / 10.0 \text{ m x} 3.606$	= 1.136	
	References and the second s References and the second se	
2. TOP CONCRETE		na se konzerta en la del se del s Nel se del se
$W = 5.90 \text{ m}^3 / 10.0 \text{ m x} 4.794$	= 2.828	
W 5.70 III / 10.0 III A 1.77		
3. PARTITION WALL (A)		
		4.445
$W = 1.362 \text{ m}^3 / 10.0 \text{ m x} 3.493$	= 0.476	4.440 m ³
	영화 가슴 것 같은 것 같은 것 같이 많이 많이 했다.	

TYPE OF WORK : BOX CULVERT FOR REVETMENT LOCATION : RIGHT SIDE :

CALCULATION	aga ta Agguna e 🖓	RESULT
FORM		
1. BASE CONCRETE		
$A = 13.240 \text{ m}^2 / 10.0 \text{ m x} 3.606$	= 4.774	an de la companya de Recorde de la companya
2. TOP CONCRETE		
$A = 26.0 \text{ m}^3 / 10.0 \text{ m x} 4.794$	= 12.464	
3. PARTITION WALL (A)	<u>n an an</u>	<u>i de la composición de</u> Esta de la composición
$A = (3.818 + 3.168) \times 0.65 / 2 \times 2$	= 4.541	in a star and a star
TOTAL	= 21.779	21.779 m ²
DEFORMED REINFORCING BAR		
1. BASE CONCRETE		
	0.027	0.000
W = 0.101 tf / 10.0 m x 3.606	= 0.036	0.036 tf
2. TOP CONCRETE		
W = 0.162 tf / 10.0 m x 4.50	= 0.073	0.073 tf
3. PARTITION WALL		
5. FARTITION WALL		<u>n si na si </u>
W = 0.093 tf / 10.0 m x 3.493	= 0.032	0.032 tf
JOINT FILLER		
1. BASE CONCRETE		
$A = 0.315 \text{ m}^2 / \text{place x 1}$	= 0.315	0.315 m ²
2. TOP CONCRETE		
$A = 4.95 \text{ m}^2 / 10.0 \text{ m x } 4.5$	= 2.228	2.228 m ²
	- 2.220	4.220 111
3. PARTITION WALL		n an the second seco
$A = 3.250 \text{ m}^2 / 10.0 \text{ m x} 3.493$	= 1.135	1.135 m ²

TYPE OF WORK: BOX CULVERT FOR REVETMENTLOCATION: RIGHT SIDE:

CALCULATI	ON	RESULT
WEEP HOLE		
PVC PIPE Ø 50		
PVC PIPE Ø 50		
n = 2		
1 = 0.65		
$L = 2 \times 0.65$	= 1.300	1.300 m
FILTER CLOTH		
$A = 0.64 \text{ m}^2 / \text{place x 2}$	= 1.28	1.280 m ²
A = 0.04 m / place x 2	- 1.28	1.280 m
GRAVEL BEDDING		
1. REVETMENT		
$A = (2.6 + 2.9) \times 0.3 / 2$	= 0.825	
$V = 0.825 \times 3.45$	= 2.846	
2. BASE CONCRETE	<u>i kan an a</u>	
		and a second
$V = 0.800 \text{ m}^3 / 10.0 \text{ m} \times 3.864$	= 0.309	
3. GRAVITY WALL		
J. URAVIII WALL		
$A = (1.3 + 1.4) \times 0.1 / 2$	= 0.135	
$V = 0.135 \times 3.535$	= 0.477	rational de la companya de la compa Altra de la companya d Altra de la companya d
TOT	$^{\circ}AL = 3.602$	3.602 m ³
WET STONE MASONRY		
$A = (3.111 + 2.121) \times 0.35/2$	= 0.916	<u>i i kao indrina dia 12.</u> Ny GMT+1. Ny
$V_{\rm c} = 0.916 {\rm x} 3.45$	= 3.16	3.160 m ³
	<u>ar bara da serie da s</u> Esta da serie	<u>e de la contra de la contra</u> 1944 - Esta de la contra de la co

TYPE OF WORK	: BOX CULVERT FOR REVETMEN		
LOCATION	: RIGHT SIDE		

CALCULATION		RESULT
GRAVITY WALL		
$A = (0.4 + 1.175) \times 1.550$	= 2.441	an an an tha an
$V = 2.441 \times 3.533$	= 8.624	8.624 m ³
CEMENT MORTAR POINTING		
$L_1 = 3.111$		
$A_1 = 3.111 \times 3.45$	= 10.733	
$L_2 = 1.0 + 0.4$	= 1.4	
$A_2 = 1.4 \times 3.533$	= 4.946	
ng na hara wasan ya kata kata na kata n		
$\mathbf{A} = \mathbf{A}_1 + \mathbf{A}_2$	= 15.679	15.679 m ²
CONCRETE TYPE C1		
1. BASE CONCRETE		
T. DASE CONCRETE		
$\overline{V} = 3.150 \text{ m}^3 / 10.0 \text{ m} \times 3.864$	= 1.217	en al anna an tha anna an taon an taon Taon an taon an
	1.6.1	
2. PARTITION WALL		
$A = (2.461 + 3.111) \times 0.65 / 2$	= 1.811	
$V = 1.811 \times 0.3$	= 0.543	
FORM		1.760 m ²
1. BASE CONCRETE		
$V = 13.240 \text{ m}^3 / 10.0 \text{ m} \times 3.864$	= 5.116	a a transformation and a second s
• 13.470 III / 10.0 III X 3.604	- 3.110	
2. PARTITION WALL	<u>e a tre per un conservation i agri da</u> Alternationalitationalitationalitationalitationalitationalitationalitationalitationalitationalitationalitationali	
	<u>e i a transferio de la contra de</u> Esta de la contra de	
$A = (2.461 + 3.111) \times 0.65 + 2 \times 2$	= 3.622	n an
TOTAL	= 8.738	8.738 m ²

TYPE OF WORK : BOX CULVERT FOR REVETMENT LOCATION : RIGHT SIDE :

CALCULATI	ON	RESULT
DEFORMED REINFORCING BAR		
1. BASE CONCRETE		
I. BASE CONCRETE		
W = 0.101 tf / 10.0 m x 3.864	= 0.039	0.039 tf
2. PARTITION WALL		
W = 0.093 tf / 10.0 m x 2.786	= 0.026	0.026 tf
JOINT FILLER		
1. BASE CONCRETE		
1. DASE CONCRETE		
$A = 0.315 \text{ m}^2 / \text{place x 1}$	= 0.315	0.315 m ²
2. PARTITION WALL		
$A = 3.250 \text{ m}^2 / 10.0 \text{ m} \times 2.786$	= 0.905	0.905 m ²
WEEP HOLE		
PVC PIPE Ø 50	n an an tha chair an	
	<u>na se </u>	
n = 2		
$1 = 0.65 \mathrm{m}$		ne Nagina di Sala ne ana esta Angela de la companya de la companya Angela de la companya
$L = 2 \times 0.65$	= 1.300	1.300 m
FILTER CLOTHS		
$A = 0.64 \text{m}^2 / \text{place x } 2$	= 1.28	1.280 m ²
A = 0.04 m / place x 2	= 1.28	1.260 11
		internet i serie de la composición de Enternet de la composición de la composi

TYPE OF WORK : BOX CULVERT FOR REVETMENT LOCATION : LEFT SIDE :

CALCULATION		RESULT
STRUCTURAL EXCAVATION		
A = { $(0.8 + 2.2) \times 0.9 / 2 + 1.1 \times 1.1 / 2 + 0.9 \times 1.1$		
x 2 + (5.0 + 2.8) x 1.2 / 2	= 10.57	an a
L = (3.0 + 2.0) / 2	= 2.50	
V = 10.57 X 2.50	= 26.425	26.425 m ³
BACKFILL WITH SELECTED SOIL		
$A = (1.9 + 0.5) \times 1.4 / 2 \times 2 + (3.0 + 2.0) \times 0.5$		
/ 2 + 2.0 x 0.7	= 4.33	
V = 4.33 X (3.0 + 2.0) / 2	= 10.825	10.825 m ³
GRAVEL BEDDING		
GRAVEL BEDDING		
1. REVETMENT		la de la constante de la const La constante de la constante de
I. KEYBIMENI		
$A = 0.3 \times 3.1 + 0.3 \times 0.3 / 2$	0.076	
$A = 0.3 \times 3.1 \pm 0.3 \times 0.3 / 2$	= 0.975	
$L_1 = (2.1 + 1.3)/2$	- 1 700	
$L_1 = (2.1 + 1.5)/2$	= 1.700	
$L_2 = (4.0 + 4.2)/2$	= 4.100	
$L_2 \sim (4.0 + 4.2)/2$	[™] 4.100	
$V = 0.450 \times 3.168 \times 2/10$	= 0.285	
<u>v = 0.450 x 5.100 x 2710</u>	- 0.265	n an an an an an Articlean. Tha an an an Articlean an Article
2. TOP CONCRETE		n de diverse de la constant de la c Esta de la constant d
	<u>i a estre d'a estre d'a deserve</u> Transformations	
$V = 0.540 \times (1.474 + 4.560) / 10$	= 0.326	
	0.520	
2. BASE CONCRETE		and an
		<u>in a seo ann ann ann</u> Ann ann ann ann ann ann ann ann ann ann
$V = 0.800 \times (2.698 + 3.794) / 10$	= 0.519	
3. PARTITION WALL		n de la companya de Esta de la companya d
$V = 0.450 \times 3.168 \times 2/10$	= 0.285	ante da la compañía de la seconda de la Este de la seconda de la se
TOTAL	= 6.785	6.785 m ³
		0.703 III
	an an an an tha an an an an an an tha tha gan an a	

TYPE OF WORK
LOCATION: BOX CULVERT FOR REVETMENT
: LEFT SIDE

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CALCULATION	RESULT
WET STONE MASONRY	
$A = (3.4 + 3.8) \times 0.35 / 2 = 1.26$	
$V = 1.26 \times (1.7 + 4.10) = 7.308$	7.308 m ³
	7.308 m
CEMENT MORTAR POINTING	
$A = 3.800 \times (1.7 + 4.1) = 22.04$	22.040 m ²
DEFORMED REINFORCING BAR	<u>. An anna 1</u> 70 an Anna Anna Anna Anna Anna Anna Anna
1. BASE CONCRETE	
W = 0.101 tf / 10.0 m x (2.698 + 4.0) = 0.068	0.068 tf
2. TOP CONCRETE	
W = 0.162 tf / 10.0 m x (1.474 + 4.56) = 0.098	0.098 tf
3. PARTITION WALL	
W = 0.093 tf / 10.0 m x 3.493 x 2 = 0.065	0.065 tf
CONCRETE TYPE C1	
1. BASE CONCRETE	
$V = 3.150 \text{ m}^3 / 10.0 \text{ m x} (2.698 + 4.0) = 2.110$	
2. TOP CONCRETE	
$V = 5.900 \text{ m}^3 / 10.0 \text{ m x} (1.474 + 4.560) = 3.560$	
3. PARTITION WALL	
$A = (3.168 + 3.818) \times 0.65 / 2 = 2.270$	
$\mathbf{V} = 2.270 \times 0.3 \times 2$ = 1.362	
TOTAL = 7.032	7.032 m ³
<u>같은 사람이 있는 것은 </u>	

TYPE OF WORK: BOX CULVERT FOR REVETMENTLOCATION: LEFT SIDE

CALCULATION		RESULT
FORM		
1. BASE CONCRETE		
I. DASE CONCRETE		
$A = 13.240 \text{ m}^2 / 10.0 \text{ m x} (2.698 + 4.0)$	= 8.868	
2. TOP CONCRETE		
$A = 26.0 \text{ m}^2 / 10.0 \text{ m x} (1.474 + 4.560)$	= 15.688	
3. PARTITION WALL		
$A = (3.168 + 3.818) \times 0.65 / 2 \times 2$	= 4.541	
TOTAL	= 29.097	29.097 m ²
JOINT FILLER		
1. BASE CONCRETE		
$A = 0.315 \text{m}^2 / \text{place x 2}$	= 0.63	0.630 m ²
2. TOP CONCRETE		
$A = 4.95 \text{ m}^2 / 10.0 \text{ m x} (1.474 + 4.56)$	= 2.987	2.987 m ²
3. PARTITION WALL		an a
5. PARTITION WALL		
$A = 3.250 \text{ m}^2 / 10.0 \text{ m x} 3.493 \text{ x} 2$	= 2.270	2.270 m ²
WEEP HOLE		
WEEP HOLE	ato a construction de la construction En la construction de la construction	
PVC PIPE Ø 50		
n = 3		
n = 3		
$1 = 0.65 \mathrm{m}$		
I - 2-0.45		
$L = 3 \times 0.65$	= 1.950	1.950 m
FILTER CLOTH		
$A = 0.64 \text{ m}^2 / \text{place x 3}$	= 1.92	1.920 m ²

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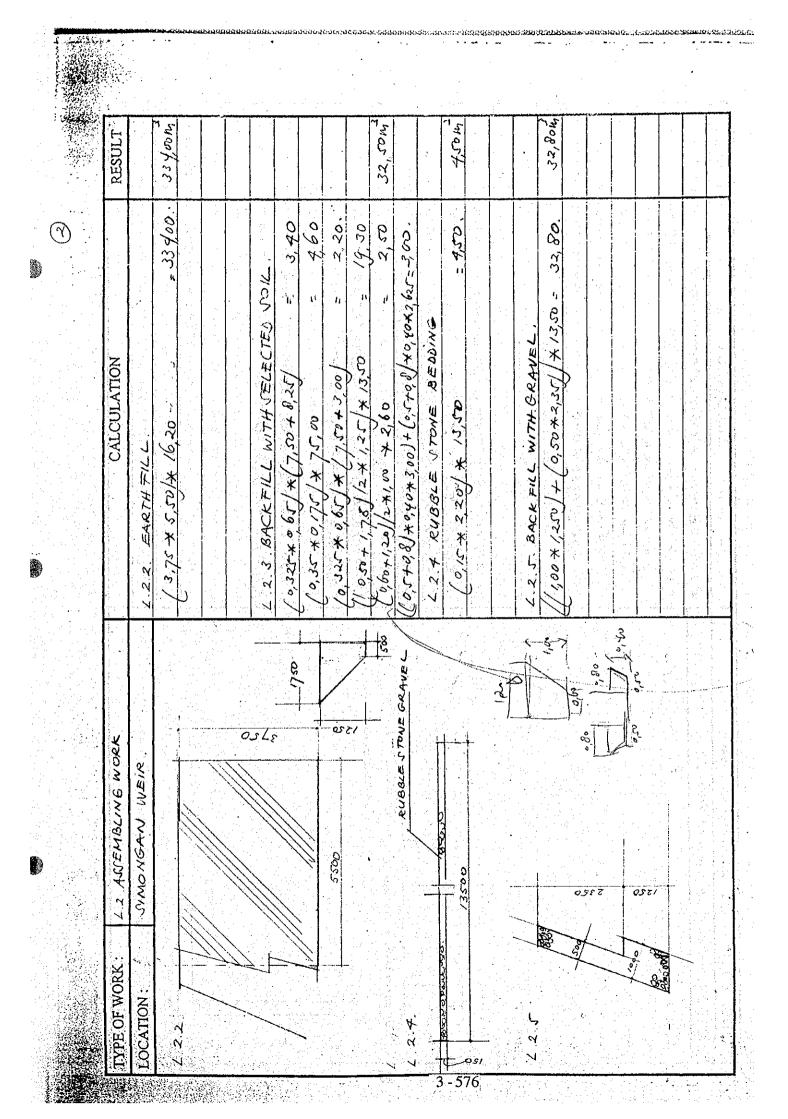
3.8 Preservation of Part of Existing Simongan Weir : 62 ASSEMBLING WORK AT PRESERVATION SITE

TYPE OF WORK

. SIMONGAN WEIR LOCATION

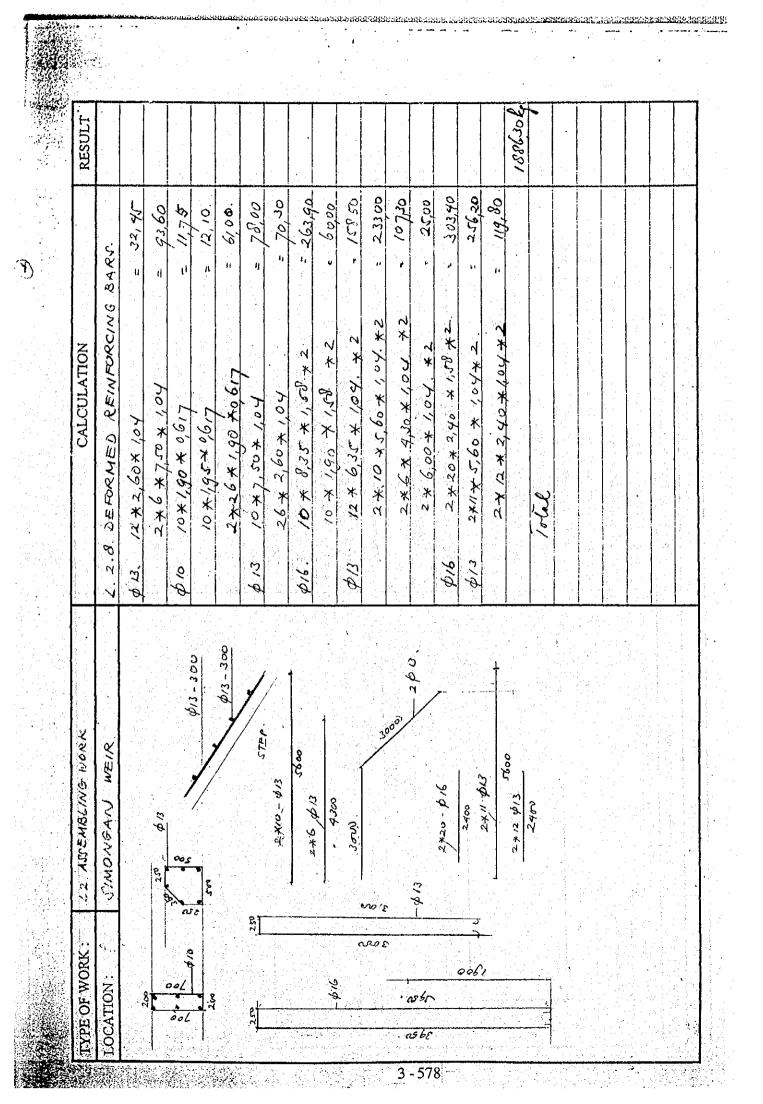
CALCULATION	RESULT
L. 2.1. STRUCTURAL EXCAVATION	149.00 m
L 2.2. FARTH FILL	334.00 m3
L.2.3. BACK FILL WITH VELECTED SOIL	32.50 m
1.2.4 RUBBLE STUNE BEDDING	4.50 m ³
125 BACKFILL WITH GRAVEL	32.80.47
1.2.6. LEVELING CONCRETE TYPE E	46.00 m3
1.2.7. CONCRETE TYPE CI INCLUDING FORM WORK	15.00 m3
4.2 8 DEFORMING REINFORCING BARS	1886.00 kg
L.2.9 WETNFONE MASONRY	145.80 113
1.2.10. CEMENT MORTAR POINTING ON THE SURFACE OF	
MASONRY	70.00m²
L. 2. 11 JOINT FILLER, 10 MM THICK (ELANTIC MATERIAL)	6.50 m²
L. 2 12 STEEL FENCE (GALVANIZE) , H= 110 CM	54.00M
L. 2. 13. ASSEMBLING BLOCKS INTO ORIGINAL SHAPE	778.00 m
L.2. 14. JOINTING BLOCKS WITH NON-SHRINKAGE MORTAL	- 3,50 m
12 IS DRILLING HOLE (\$45 mm / IN THE BLOCK	700,004,
L.2. 17. GRAVEL BEDDING	34.500
L.2. 18 CONCRETE TYPE D INCLUDING FORM WORK	69.00 m3
n <u>na standar versionale en anterna en anterna de la constante de la constante de la constante de la constante d</u> En anterna en anterna de la constante de la cons	

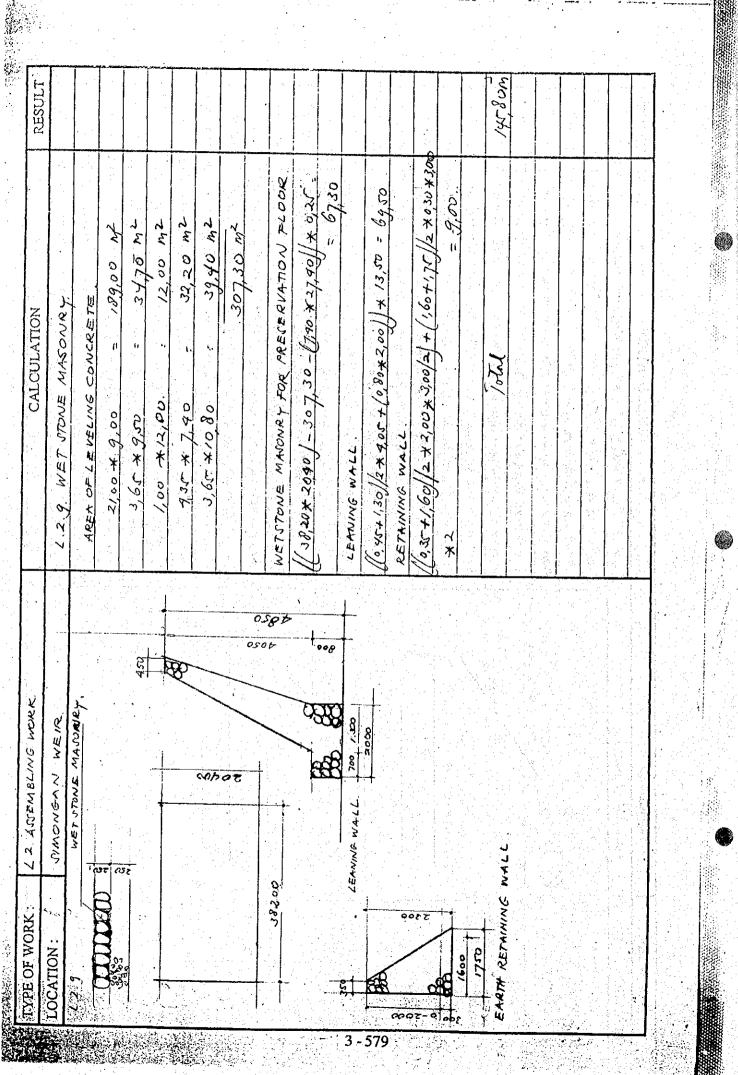
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RESULT								mas 551												- 	学校学校 かいてい いっかい
CALCULATION	L 2.1 STRUCTURAL EXCAVATION.	1/15,00+3,20//2 ×1,25 + /0,63× 125//× 13,50 = 79,80	50+ 150/	✓✓✓✓✓	1+[a3c* a'14] + 19'00 =	X															
IYPE.OF. WORK: 2. ASSEMBLING WORK	LOCATION: SUMONGAN WER	2.2.1. 2.000 5.000 630			2200 ·····				1000 1 - 2500 1 - 2500 1 - 1500						vection of		/200				

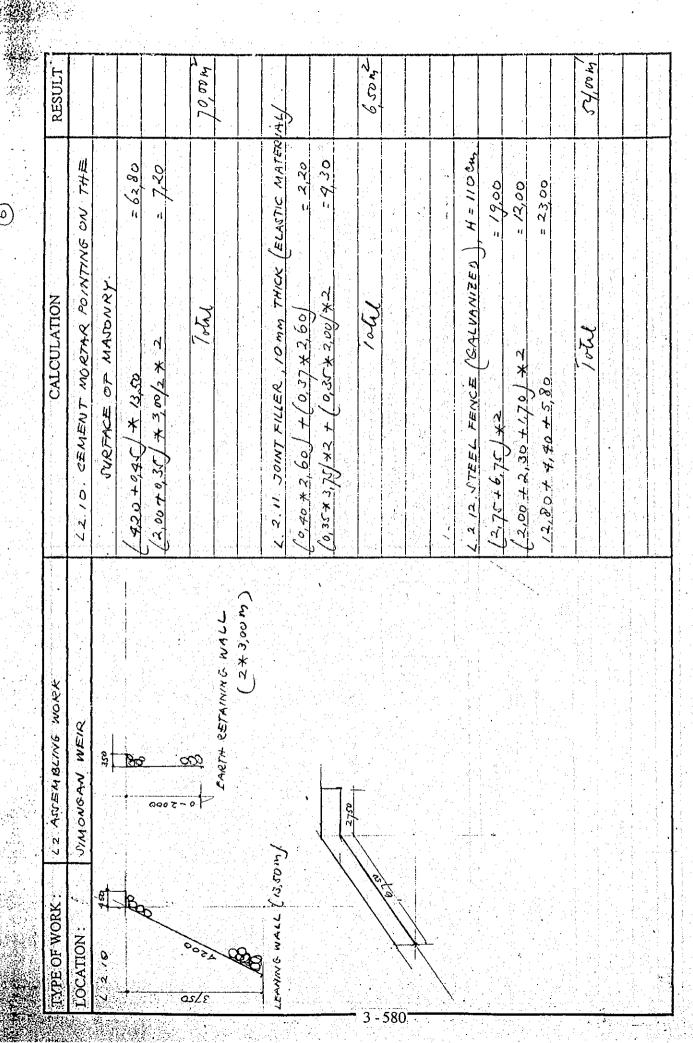


G. G. G. BERLEY CO. MAR E.A. 68950 RESULT 40804 28.713m ż L.2.7. CONCRETE TYPE CI INCLUDING FORM WORK 065 28:40 0.57 5,90 360 80 Se 625 15 4 4,80 8,213 5.040 28.713 Ŀ i, 4 4 00 2,30+ 4, at //2 ×/ 0,35+0,40//2/×2,625 × 2 1 ŧ, 11 ~* 14 LEVELING CONCRETE TYPE l, ŕ, ij TOTAL 010+ 06 1/2 × 0,30 + 0,30 × 0 60 2 2 CALCULATION ×. * (035+0.40)×12×3.65×3.00 0,30 × 0,30/ × 16 90 +0,60/ 2×10×040/ ×3620×2 2 × 0' 1 3,65 * 10,80 * 0,10 9,36× 240× 0,1 21,00 * 9,00 * 0,15 100 * 13, 2 4 0 17 1stal (210×0.40)×3.00 3,65 * 9.50 × 030 × 0,80/ × 2,60 0 ۲ Ś BASE CONCRETE ASSEMBLING WORK 009 SIMONGAN WEIR 300 JOO 80 300 300 JIAM HOIS 309 27 TYPE OF WORK 008 TOP CONCRETE 2500 LOCATION : 5 5 ş 1 1505-0052 3 - 577

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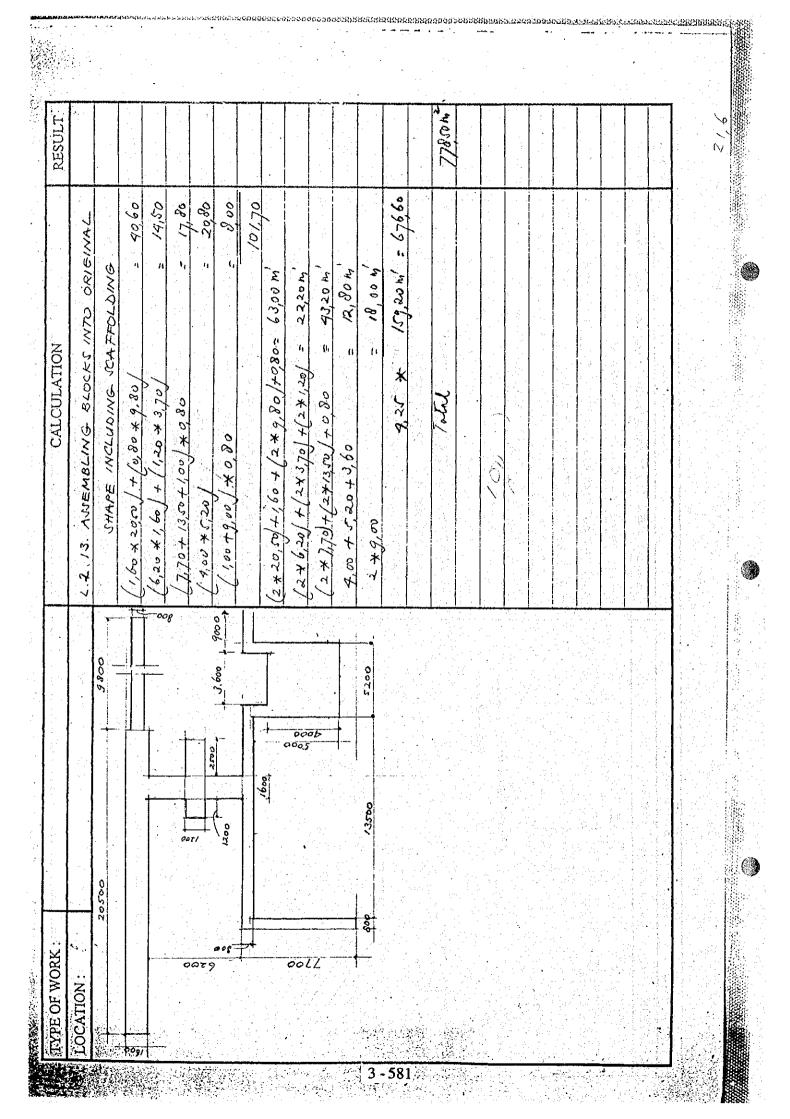




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34.40% 700% RESULT Ser. 4ND FILLING THE HOLE WITH NON - SHRINKAGE IN THE BLOCK ANCHORING WITH NTEEL DEFORMED BAR (016) = 2690 JOINTING BLOCKS WITH NON-SHRINKAGE : 2 Z 4000 ġ (3820×20,40/-307,30-(7,40×27,40//× 0,10 9 ı١ A SS MIN CALCULATION L.2. 17. GRAVEL BEDDING NON MORTAL SHRINKAGE. L. 2. 15 DRILLING HOLE 75×002 0,30×2,60/×9,50 MORTAL 75-* 400 B MORTAR 4 Ч DRILLING HOLE (DIA 45 mm) IN THE 1 BLOCK - 4004 YON MURTAL SHRINKAGE FUR TRLOCK = 0,02 m3 WOLUME = 101,70 × 4,27 = 434,30 m GRAVEL REDDING 300 434,30 = 175 BLOCKS. 2.2.14.72.15 GOL **LYPE OF WORK** : LOCATION 2 1 GRANEL 582 3

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