REVETMENT

TYPE OF WORK

LOCATION

: WET STONE MASONRY : WF.97+36.32 ~ WF. 98+31.90

JET STANE		CALCULATIO	11				RESULT
VLI PIUIVL	HASONRY						
- WET STONE M	MASONRY (1:1	i.5) (t = 250)					
STATION	PARTIAL	SLOP LENGTH	AVERAGE	AREA	VOLUME		
NO.	DISTANCE (m)	(m)	(m)	(m²)	(m ³)		
WF97R+36.320		5.512			(1117)		网络黄金 医鼻孔
WF97R+41.320	7.099	5.512		39,131	9.783		
WF98R+1.400	11.745	5.523		64.804	16.201	·	
WF98R+11.900	9.740	4.432		48.482	12.120		
WF98R+21.900	9.740	3.547	3.990	38.858	9.715		
WF98R+31.900		3.561	3.554	34.617	8.654		1.3 2. 17.2
		1 0.001	0.004	TOTAL			
WF98L+31,900		3.561		TUTAL	56.473	# 3 1 1 1 h	
WF98L+21.900		3.547	3.554	04.475	0.010		
WF98L+11.900		4,432		34.475	8.619	<u> </u>	
WF98L+1.400	10.200	5.523	3.990	38.699	9.675	1. A	
WF97L+41.320	11.700			50,772	12.693	_	
WF97L+36.320	5.000	5.512 5.512		64.556	16.139		
777 072 00:020	3.000	3.512	5.512	27.561	6.890		1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
. <u> </u>			Alberton British	TOTAL	54.016		
						1	
		to the state of the		AND SECTION AND ADMINISTRATION OF THE PARTY			
	internal est estimate						
					<u>- vi</u>		
							for the first of the
			77	DTAL =	110.48	9 11	0. 489 m
			7/	07AL =	110.48	9 11	0. 489 m
			77	27AL =	110.48	9 11	0. 489 m
			7.	DTAL =	110.48	9 //	0. 489 m
			7.	OTAL =	110.48	9 //	0. 489 m
			77	DTAL =	110.48	9 //	0. 489 m
			77.	0TAL =	110.48	9 //	0.489 m
			77.	07AL =	110.48	9 //	0.489 m
			77.	07AL =	110.48	9 //	0.489 m
			7.	07A)_ =	110.48	9 //	0.489 m
			77.	07A)_ =	110.48	9 //	0.489 m
			77.	07A)_ =	110.48	9 //	0.489 m
			77	07A)_ =	110.48	9 //	0.489 m
			77	07A)_ =	110.48		0.489 m
			77	07A)_ =	110.48	9 //	0.489 m
			77	07A)_ =	110.48		0.489 m
			7	07A)_ =	110.48		0.489 m
			7	27A). =	110.48		0.489 m
				27A). =	110.48		0.489 m
				27A). =	110.48		0.489 m
				07A). =	110.48		0.489 m
			7	07A). =	110.48		0.489 m
			7	07A). =	110.48		0.489 m
				07A). =	110.48		0.489 m
				27A). =	110.48		0.489 m
				27A). =	110.48		0.489 m

REVETHENT

TYPE OF WORK

: BACKFILL GRAVEL : WF. 97+36.32 ~ WF. 98+31.90 LOCATION

		CALCULATION					RESULT
				41 - 21 -		,	
DAOKERLOS	AND I A	•		1 - 1			
BACKFILL GRA	AVEL (t = 300)					
STATION	PARTIAL	SLOP LENGTH	AVERAGE	AREA	VOLUME	1	
5 A 4 A 5 A 5 A 5 A 5 A 5 A 5 A 5 A 5 A	DISTANCE (m)		(m)	(m²)	(m ³)		
WF97R+36.320		5.512	(111)	(11)	(m)	100	
WF97R+41.320	5.020	5.512	5.512	27.671	8.301		
WF98R+1.400	11.745	5.523	5.518	64.804		_: :	
WF98R+11.900	9.740	4.432	4.978	48.482		1.	
WF98R+21.900		3.547	3.990	38.858			
WF98R+31.900	9.740	3,561	3.554	34.617	10.385		
11/5051 01 000			N. S	TOTAL	64.330	1.15	and the second of
WF98L+31.900	0.700	3.561	110 5 3				
WF98L+21.900 WF98L+11.900		3.547	3.554	34.475			
WF98L+1,400	10.200	4.432 5.523	3.990	38.699	11.610		
WF97L+41.320	11.700	5.512	4.978 5.518	50.772	15.232		
WF97L+36.320	5.000	5.512	5.512	64.556 27.561	19.367 8.268	- 23	
	3,000	0.012	0.012	TOTAL	64.819	200	
4.	Mark and participation of the		L	TOTAL	07.010		
				State of the	sa in edian		
			TOTAL	= /2	5.14-9		128.149 m
							1
	and the second of the second o						

TYPE OF WORK:

REVETMENT TOP CONCRETE

LOCATION : TOP CONCRETE

: WF 97 + 36.32 ~ WF. 98 + 31.90

CALCULATION		RESULT
(RIGHT BANK)		
• CONCRETE (TYPE - CI)		
	4000	
L = 10.04 + 10.04 + 10.54 + 12.045 + 7.099 =	17.701 111	10.100
$V = 0.30 \times 0.70 \times 49.764$	10.450	10.450 m ³
GRAVEL BEDDING		
L = 49.764 m		
$V = (0.30 + 0.10) \times 0.10 \times 49.764 =$	1.991	1.991 m ³
• FORM (H < 4.0 m)		
$A_1 = 0.70 \times 49.764 \times 2 =$	69.670	
$A_2 = 0.70 \times 0.30 \times 4$	0.840	
ΣA =	70.510	70.510 m ²
REINFORCING BAR		
• D13 (W = 1.04 kgf/m)		and the state of the state of
$W_1 = 6 \text{ Bars x } (10.04 - 0.05 \text{ x 2}) \text{ x } 1.04 =$	62.026	
$W_2 = 6 \text{ Bars } \times (10.04 - 0.05 \times 2) \times 1.04 =$	62.026	
$W_3 = 6 \text{ Bars } x (10.54 - 0.05 \times 2) \times 1.04 =$	65.146	
$W_4 = 6 \text{ Bars } x (12.045 - 0.05 \times 2) \times 1.04 =$	74.537	
$W_5 = 6 \text{ Bars } x (7.099 - 0.05 \times 2) \times 1.04 =$	43.674	
• D10 (W = 0.617 kgf/m)		
$N_6 = (10.04 - 0.05 \times 2) \div 0.30 = 33.13 \cong$	34 Bars	
$L = 0.20 \times 2 + 0.60 \times 2 + 15 \times 0.01 =$	1.75 m/Bars	
$W_6 = 34 \text{ Bars } \times 1.75 \times 0.671 =$	36.712	
$N_7 = (10.04 - 0.05 \times 2) \div 0.30 = 33.13 \cong$	34 Bars	
L = 1.75 m/Bars		
$W_2 = 34 \text{ Bars x } 1.75 \text{ x } 0.617$	36.712	4 1 1 4 4 4 4 4
$N_8 = (10.54 - 0.05 \times 2) \div 0.30 = 34.80 \cong$		
$W_8 = 35 \text{ Bars } \times 1.75 \times 0.617$	37.791	
$N_9 = (12.045 - 0.05 \times 2) \div 0.30 = 39.82 \cong$		
$W_9 = 40 \text{ Bars } \times 1.75 \times 0.617$	43.190	
119		
$W_{10} = 24 \text{ Bars } \times 1.75 \times 0.617$	43.71	
Z 111 -	487.728 kgf	0.488 tf
<u> </u>	407.720 Kgi	0.400 11
(마르크) 그 아이들 그들의 유명의 본 아이들이 아무리는 그 그들이 한 생생님은 아이들이 아니는 아니는 그렇게		

TYPE OF WORK:

LOCATION

REVETMENT TOP CONCRETE WF.97 + 36.32 ~ WF.98 + 31.90

CALCULATION	RESULT
	100001
JOINT FILTER t = 10 (ELASTIC MATERIAL)	
$A_1 = 0.70 \times 0.30 \times 5 = 1.050$	
$A_2 = 9.74 \times 0.25$ = 2.435	
$A_3 = 9.74 \times 0.25 = 2.435$	10.0
$A_4 = 10.24 \times 0.25 = 2.560$	
$A_5 = 11.745 \times 0.25$ = 2.936 $A_6 = 7.099 \times 0.25$ = 1.775	
$A_6 = 7.099 \times 0.25 = 1.775$	
TOTAL = 13 101	10101
TOTAL = 13.191	13.191 m ³
	the state of
	
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TYPE OF WORK : REVETMENT LOCATION : TOP CONCRET

TOP CONCRETE WF.97 + 36.32 ~ WF.98 + 31.90

CALCULATION	RESULT
	i de trada
(LEFT BANK)	
• CONCRETE (TYPE - C)	
L = 10.00 + 10.00 + 10.50 + 12.00 + 7.071 = 49.571 m	
$V = 0.30 \times 0.70 \times 49.571 = 10.410$	
GRAVEL BEDDING	<u> </u>
L = 49.571 m	
$V = (0.30 + 0.10) \times 0.10 \times 49.571 = 1.983$	
	emple autom
• FORM (H < 4.0 m)	
4 0 20 40 25 1	
$A_1 = 0.70 \times 49.571 \times 2 = 69.399$	ar yan ara a san
$A_2 = 0.70 \times 0.30 \times 4 = 0.840$	<u> </u>
∑ A = 70.239	
REINFORCING BAR	
	Markey was seen
• D 13 (W = 1.04 kgf/m)	
$W_1 = 6 \text{ Bars } \times (10.00 - 0.05 \times 2) \times 1.04 = 61.776$	
$W_2 = 6 \text{ Bars } x (10.00 - 0.05 \times 2) \times 1.04 = 61.776$	The second secon
$W_3 = 6 \text{ Bars } \times (10.50 - 0.05 \times 2) \times 1.04 = 64.896$	e de la company de la comp La company de la company d
$W_4 = 6 \text{ Bars } \times (12.00 - 0.05 \times 2) \times 1.04 = 74.256$	Tagain san migawan
$W_5 = 6 \text{ Bars } x (7.07 - 0.05 \times 2) \times 1.04 = 43.499$	
-0.00 W - 0.0171 g/s	
• D10 (W = 0.617 kgf/m)	
$N_6 = (10.00 - 0.05 \times 2) \div 0.30 = 33 \text{ Bars}$ $L = 0.20 \times 2 \div 0.60 \times 2 \div 15 \times 0.0 = 1.75 \text{ m/Page}$	<u> 1941 - 1945 - 1940 - 1945 - 1945 - 1945 - 1945 - 1945 - 1945 - 1945 - 1945 - 1945 - 1945 - 1945 - 1945 - 194</u>
I./J libbats	
$N_7 = (10.00 - 0.05 \times 2) \div 0.30$ = 33 Bars $W_7 = 33$ Bars x 1.75 x 0.617 = 35.632	
$N_8 = (10.54 - 0.05 \times 2) \div 0.30 = 34.67 \cong 35 \text{ Bars}$ $W_8 = 35 \text{ Bars } \times 1.75 \times 0.617 = 37.791$	
	a aktor (1755) i filozofia. Takin kirajan dia aktor (1
12 10 Dats	
$N_{10} = (7.071 + 0.05 \times 2) \div 0.30 = 23.24 \cong 24 \text{ Bars}$ $W_{10} = 24 \text{ Bars } \times 1.75 \times 0.617 = 25.914$	
$W_{10} = 24 \text{ Bars } \times 1.75 \times 0.617 = 25.914$	
	0.404+6
$\sum W = 485.362 \text{ kgf}$	0.484 tf

TYPE OF WORK : APPROACH LOCATION : TOP CONCRETE

WF 97 + 36.32 ~ WF. 98 + 31.90

CALCULATION	RESULT
• JOINT FILTER t = 10 (ELASTIC MATERIAL)	
JOINT FILTER t = 10 (ELASTIC MATERIAL)	
$A_1 = 0.70 \times 0.30 \times 5 = 1.050$	
$A_2 = 9.70 \times 0.25$ = 2.425	
$A_3 = 9.70 \times 0.25$ = 2.425	
$A_4 = 10.20 \times 0.25 = 2.550$	
$A_5 = 11.70 \times 0.25$ = 2.925	
$A_6 = 7.071 \times 0.25 = 1.768$	1 2 2 2
	* * * * * * * * * * * * * * * * * * * *
TOTAL = 13.143	13.143 m ³
	-
	100000000000000000000000000000000000000
	1 1 1 1 1 1 4 1
	[[1] 25 A. A. A. A.
	-
	and the first of
경험의 악선 환경에는 회약을 가장 하는 사람들은 사람들이 가지 않는데 없는데 되었다. 그 사람들은 회장	

TYPE OF WORK

OCATION	: WF.97 +36.32~ WF.98 +31.90
	EXPLANATORY DRAWING
	(m) 1,058 1,058 1,058 1,058
	TABLE OF DIMENSION L1 L2 (m) (m) (m) 21,900 3,732 3,451 1,400 4,823 4,542 41,320 4,812 4,531 PARTITION WALLE
	LE OF DI (m) 500 3,732 50 3,732 50 4,823 7TITION
	STAT WF. 98+ WF. 98+ WF. 98+ WF. 97+ SCAL
	BEDDING 1001 1001 1001 1001 1001 1001 1001 10
	1.5H2 + 1,000 1.5H2 1,000 1.5H2 1,000 1.5H2 1,000
	HS

REVETMENT

TYPE OF WORK

CEMENT MORTAR POINTING

LOCATION : WF. 97 + 36. 32 ~ WF. 98 + 31.90

	· · · · · · · · · · · · · · · · · · ·	CALO	ULATION		1 - 1,		RESULT
	<u> </u>					. •	
	OFMENT HOD				and the finite		
	CEMENT MOR	IAR POINTING					
	STATION	PARTIAL	SLOP LENGTH	AVEDACE	ADEA	<u> </u>	
·	1	DISTANCE (m)		(m)	AREA (m²)		
	WF97R+36.320	BIOTATIOE (III)	5.512		(m)		
e godenne	WF97R+41.320	7.099	5.512		39.131		
	WF98R+1.400	11.745	5.523		64.804		
	WF98R+11.900	9.740	4.432	4.978	48.482		Holyman 1
	WF98R+21.900 WF98R+31.900	9.740 9.740	3.547 3.561	3.990	38,858	-	
 .	717 001(101,000	9.740	3.301	3.554 TOTAL	34.617 225.893	100	
<u> </u>	WF98L+31.900		3.561	TOTAL	225.893		
4 6 5	WF98L+21.900	9.700	3.547	3.554	34.475		
	WF98L+11.900	9.700	4.432	3.990	38.699		
	WF98L+1,400 WF97L+41.320	10.200	5.523		50.772	- 1	
	WF97L+36.320	11.700 5.000	5.512 5.512		64.556		
	3.3.00.02.0	0.000	0.012	TOTAL	27.561 216.063		
				TOTAL	210.003		
			77	70) -	141.956		441.956 m
- 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1			117				1 4-47, 73 0 11
and the second		Application of the second		1/1/2 -	+ +11 100		11/100 111
				(/)2	F FT(100		
				<i>(/) L</i>	F F1. 100		
					777.700		
					777.100		
							

TYPE OF WORK: LOCATION:

PARTITION WALL

WF.97 + 36.32 ~ WF.98 + 31.90

	CALCU	JLATIC	N			RESULT
COMODETT CT	DE CIL	<u> 11 - 12 15 1</u>	<u> </u>	<u> </u>		
CONCRETE (TY	PE-CI)		<u> </u>		a lala de la galación e est	
	STATION	AREA	WIDTH	VOLUME		
	No.	(m ²)	(m)	(m³)	ya ejaku. Pada a	
	WF, 98+21,900	2.731	0.300	0.819	<u> </u>	
	WF, 98+11,900	2.696	0.300			
	WF. 98+1.400	3.296				
	WF 97+41.320	3,290	0.300	0.987		
			TOTAL	3.604		
			<u> </u>			
			3. 65		<u> Name in Amerika</u>	
	TOTAL		104 - 0			
	TOTAL	= 3.6	504 x 2	=	7.208	7.208 M ³
GRAVEL BEDDI	NG (t = 100	<u> </u>				
OXXXX DEDDI	(t – 100	,				
$V_1 = (0.50 + 0.70)$	0) x ½ x 0.10 x (3	515 ±	1.058) v 1) =	0.549	
)) x ½ x 0.10 x (3				0.541	
	0) x ½ x 0.10 x (4				0.672	
	0) x ½ x 0.10 x (4				0.671	
		TOTA	L	=	2.433	2.433 m ³
			<u> </u>			
JOINT FILTER	(t = 10, ELASTI	C MATE	ERIAL)	Barrier Contract	i kiringan dib	
2.721			<u> </u>	<u> 1910 - 1914</u>	<u> Angelija kang kang</u>	
$\frac{x_1}{x_2} = \frac{2.731 \times 2}{2.696 \times 2}$				<u> </u>	5.102	
$\frac{1}{2} = 2.696 \times 2$ $\frac{1}{3} = 3.296 \times 2$				<u> </u>	5.392	<u> </u>
$\frac{3}{4} = \frac{3.296 \times 2}{3.290 \times 2}$				=	6.592	
4 J.270 A Z			en de la composición		6.580	
		TOTAL		22	24.026	24.026 m ²
		TOTAL			24.020	24.020 in
REINFORCING E	SAR			· v William		1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
	· · · · · · · · · · · · · · · · · · ·					
• D 13 (W = 1	.04 kgf/m)	i ja Kit	en e e			
		× 2.4 ()	artin i ar	and the second		
	.796 + 1.00 - 0.05			= 58	.606	
$V_2 = 6 \text{ Bars x } (3)$.732 + 1.00 - 0.05	x 2) x	1.04		.807	
	.823 + 1.00 - 0.05				.423	
$V_4 = 6 \text{ Bars x } (4)$.812 + 1.00 - 0.05	x 2) x	1.04		.286	muur Ang amur
		1 1 1 T		ou d' <u>da</u>		1 1

TYPE OF WORK:

LOCATION

PRATITION WALL WF.97 + 36.32 ~ WF.98 + 31.98

CALCULATION	RESULT
• D10 (W = 0.617 kgf/m)	
$L = 0.45 \times 2 + 0.20 \times 2 + 15 \times 0.01 = 1.45 \text{ m/Bars}$. 1.
$N_1 = (3.796 + 1.00 - 0.05 \times 2) \div 0.30 + 1 = 16.65 \cong 17 \text{ Bars}$	1 4
$N_2 = (3.732 + 1.00 - 0.05 \times 2) \div 0.30 = 16.44 \cong 21 \text{ Bars}$	
$N_3 = (4.823 + 1.00 - 0.05 \times 2) + 0.30 = 20.08 \approx 21 \text{ Bars}$	
$N_4 = (4.812 + 1.00 - 0.05 \times 2) \div 0.30 = 20.04 \cong 21 \text{ Bars}$	
$W_5 = 17 \text{ Bars } \times 1.45 \times 0.617 \times 2 = 30.418$	
$W_6 = 17 \text{ Bars } \times 1.45 \times 0.617 \times 2 = 30.418$	
$W_7 = 21 \text{ Bars } \times 1.45 \times 0.617 \times 2 = 37.575$	
$W_8 = 21 \text{ Bars } \times 1.45 \times 0.617 \times 2 = 37.575$	
$\sum W = 395.108 \text{ kgf}$	0.395 tf
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	<u> </u>
	4.1
	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
	<u> </u>
Barrier and the state of the st	

TYPE OF WORK: LOCATION:

REVETMENT

WF.97 + 36.22 ~ WF.98 + 31.90

			CAL	CULA	TION		riu arkuri		RESULT
FOR	RM (H < 4.0 m)				organis de la companya de la company	<u> </u>			
	CTATION	T							
	STATION	LI	L2	L3	A1	A2		FORM AREA _	1 11 1 2 1 2
	NO.	(m)	(m)	(m)	(m²)	(m²)	(m²)	(m²)	Mark (1964)
	WF. 98+21.900	3.796	3.515	1.058	0.720	2.011	2.731	5.462	
	WF. 98+11.900	3./32	3.451	1.058	0.720				Military of the
	WF. 98+1.400		4.542	1.058	0.720			6.591	
	WF. 97+41.320	4.812	4.531	1.058	0.720	2.569		6.579	
						Salaki	TOTAL	24.024	
	ration and a second constant		- 10 mg - 10 m		() 		1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		
		TOTA		24.02	M × 2			8.048	49.049 - 2
		TOTA	<u> </u>	24.02	. 4 X Z	<u> </u>	4	8.048	48.048 m ²
GR	AVEL BEDDING		(t = 1	00)			<u> </u>		
	2. DD DDDINO	van een ee	(1 1		<u>e tellusi.</u> Ellen en paet			The service state of the first of the service state	
. =	(0.50 + 0.70)	(½ x	0.10 ×	(3.515	+ 1.05	8) v 2		0.549	
<u>'</u>	(0.50 + 0.70)	. 1/2 x	$0.10 \times$	(3.451	+ 1.05	8) x 2			
· =	(0.50 + 0.70)	√2 x	0.10 x	(4 542	+ 1.05	8) x 2			
4 =	(0.50 + 0.70)	√2 X	0.10 x	(4:531	+ 1.05	8) x 2		0.671	
				<u> </u>	1,00	0) X Z		0.071	
	al and the restriction		1.7.97	TO	DTAL		=	2.433	2.433 m ³
		1.11						2.733	2,433 111
44.44									
JOI	NT FILTER	(t = 10)	ELAS	STIC M	ATERI	AL)			
						/			
.1 =	2.731 x 2			. 1			=	5.462	
2 =	2.696 x 2		1 1 11			1 1 1 1		5.392	
3 =	3.296 x 2	- 1.7 %						6.592	
4 =	3.290 x 2	1111			4.5	F 2	=	6.580	
. 19 g			- 17						
<u> 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1</u>				TO	TAL	1,677.00	-	24.026	24.026 m ²
FOF	RM (H < 4.0 m)			<u> </u>					
								es, a file a company	
				-					e comment to a pro-
		<u> </u>	<u> </u>		<u> </u>				1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
		<u> </u>	· · · · · ·	11 5. 4.			<u> </u>	sali ali ji	
						<u> </u>	<u> </u>		
	<u> </u>		· · · · ·	<u> </u>		1 1 2 2 2			
<u> </u>	<u> Parti di Cara, Cara de Cara</u> Mangangan	<u> </u>	<u> </u>	<u> </u>		<u> </u>			e jakat
1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1					+ + - ²⁷ .	<u> </u>		3. 特别·克克克克克	
					in and the				and the state of
		6 TO 150	1	4 12	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1				
	A CAR CONTRACTOR	•					x 2 =	48.048	48.048 m ²

TYPE OF WORK : REVETMENT FOR SIDE SLOPE OF 1:2.0 (WET STONE MASONRY) LOCATION : WF.96L \sim WF.97L + 41.32 m

September 1981 Annual CALCULATION	RESULT
STRUCTURAL EXCAVATION	in the figure
	2.
$A_1 = (1.10 + 2.50) \times \frac{1}{2} \times 0.60 = 1.080 \text{ m}^2$	
$A_2 = (4.60 + 6.10) \times \frac{1}{2} \times 0.50 = 2.675 \text{ m}^2$	
	174 to 1
$A_3 = (1.118 \times 7.00 + 1.00) \times 0.50 = 4.413 \text{ m}^2$	
$A_4 = (0.50 + 1.20) \times \frac{1}{2} \times 0.70 = 0.595 \text{ m}^2$	
TOTAL A = 8.763 m2	
$V = 8.763 \times 89.867 = 787.50 \mathrm{m}^3$	787.50 m ³
(R = 46.65)	
BACKFILL WITH SELECTED SOIL	11/5/4/4/11
$A_1 = (0.50 + 1.00) \times \frac{1}{2} \times 0.50 \times 2 = 0.750 \text{ m}^2$	April 12 day
$A_2 = (0.50 + 1.10) \times \frac{1}{2} \times 0.60 = 0.480 \text{ m}^2$	
$TOTAL A = 1.230 \text{ m}^2$	
$V = 1.23 \times 89.867 = 110.54 \text{ m}^3$	110.54 m ³
(R = 46.65)	
WEEPHOLE	
n = 6 places / 10.0 m x 89.867 = 54 places	
(R = 46.65)	
PVC PIPE \emptyset 50 (L = 0.80 m/pipe)	
T - 54.1	
L = 54 places x 0.8 = 43.200	43.200 m
FILTER CLOTH	
FIETER CDUTT THE ENGLISH STORE STORE AND A STORE	34.3
A = 0.956 + 2.7 = 1000 + 5.4	46 204 2
$A = 0.856 \text{ m}^2/\text{place x } 54 \text{ places} = 46.224$	46.224 m ²
GRAVEL BEDDING	
ANA LET DENDINA THE STATE OF THE PROPERTY OF T	
$V = (1.119 \times 9.00 \pm 0.70) \times 0.05 \times 0.70$	
$V = (1.118 \times 8.00 + 0.70) \times 0.25 \times 9.70 = 23.387 \mathrm{m}^3 / 10.00 \mathrm{m}$	
V2 = 22 297 = 3 / 10 00 = 90 967	212.152
$V' = 23.387 \text{m}^3 / 10.00 \text{m} \times 89.867 = 210.172$	210.172 m ³
(R = 46.65)	
	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1

TYPE OF WORK : REVETMENT FOR SIDE SLOPE OF 1:2.0 (WET STONE MASONRY) LOCATION : WF.96L ~ WF.97L + 41.32 m

WET STONE MASONRY	
$\overline{V} = (1.118 \times 8.00 + 0.70) \times 0.25 \times 9.70 = 23.387 \text{ m}^3 / 10.00 \text{m}$	
$V' = 23.387 \text{ m}^3 / 10.00 \text{ m} \times 89.867$ = 210.172 (R = 46.65)	210.172 m ³
CEMENT MORTAR POINTING	
$A = (1.118 \times 7.00 + 0.70) \times 0.25 \times 9.70 = 82.702 \text{ m}^2 / 10.00 \text{m}$	
$A' = 82.702 \text{ m}^2 / 10.00 \text{ m} \times 89.605 = 741.051$ $(R = 46.15)$	741.051 m ²
GABION MATTRESS	
$A = 0.50 \times 3.00$ = 1.50 m ²	
$V = 1.50 \times 97.930$ = 146.895	146.895 m ³
(R = 52.5)	
RUBBLE STONE FILLING	
$A = \frac{1}{2} \times 0.50 \times 1.00 = 0.250 \text{ m}^2$	
$V = 0.250 \times 96.883 = 24.221$	24.221 m ³
(R = 50.5)	
BASE CONCRETE	
CONCRETE (TYPE-C1)	
$V = 2.20 \text{ m}^3 / 10.00 \text{ m x } 92.276$ = 20.301	20.301 m ³
(R = 51.25) GRAVEL BEDDING	
$V = 0.70 \text{ m}^3 / 10.00 \text{ m x } 92.276$ = 6.459	6.459 m ³
FORM (H < 4.0 m)	
$A = 10.83 \text{ m}^2 / 10.00 \text{ m x } 92.276$ = 99.935	99.935 m ²

TYPE OF WORK : REVETMENT FOR SIDE SLOPE OF 1:2.0 (WET STONE MASONRY)

LOCATION : WF.96L ~ WF.97L + 41.32 m

CALCULATION		RESULT
REINFORCING BAR		
$W = 0.10 \text{ tf} / 10.00 \text{ m} \times 92.276$	= 0.923	0.923 tf
JOINT FILLER		
JOHN TELEBIN		
$A = 0.22 \text{ m}^2 / 10.00 \text{ m x } 92.276$	= 2.030	2.030 m ²
LOG PILE		
L = 10.00 m / 10.00 m x 92.276	= 92.000	02 000
C = 10.00 M / 10.00 M x 92.270	- 92.000	92.000 m
TOP CONCRETE		
CONCRETE (TYPE-C1)		
V 100 3/1000 92 000		11.222
$V = 1.80 \text{ m}^3 / 10.00 \text{ m} \times 82.090$ $(R = 42.15)$	= 14.776	14.776 m ³
GRAVEL BEDDING		
$V = 0.75 \text{ m}^3 / 10.00 \text{ m} \times 82.090$	= 6.157	6.157 m ³
FORM (H < 4.0 m)		
$A = 12.18 \mathrm{m}^2 / 10.00 \mathrm{m} \times 82.090$	= 99.986	99.986 m²
	<i></i>	29.960 in
REINFORCING BAR		
W = 0.094 tf / 10.00 m x 82.090	= 0.772	0.772 tf
JOINT FILLER		
SOUT TELEK	<u>a la company de la Capital III de la Capi</u> El legister, el grego de la capital de la	
$A = 2.605 \mathrm{m}^2 / 10.00 \mathrm{m} \times 82.090$	= 21,384	21.384 m ²
END WALL		
n = 3 places		
II - 3 phaces with the second and the second and the		
CONCRETE (TYPE-C1)	en de la composition de la composition De la composition de	
	<u>and the second of the second </u>	The second secon
$V = 1.736 \mathrm{m}^3 / \mathrm{place} \times 3 \mathrm{place}$	= 5.208	5.208 m ³

TYPE OF WORK : REVETMENT FOR SIDE SLOPE OF 1:2.0 (WET STONE MASONRY)
LOCATION : WF.96L ~ WF.97L + 41.32 m

CALCULATIO	N sa mala an e supre de la co	RESULT
GRAVEL BEDDING		
$V = 0.772 \text{ m}^3 / \text{place x 3 places}$	= 2.316	2.316 m ³
FORM (H < 4.0 m)		
1 Oddy (11 53.0 m)		
$A = 11.573 \text{ m}^2/\text{place x 3 places}$	= 34.719	34.719 m ²
REINFORCING BAR		
W = 0.091 tf / place x 3 places	= 0.273	0.273 tf
W 1007 H7 place x3 places		
JOINT FILLER		
		7 2 2 2
$A = 2.411 \text{m}^2/\text{place x 3 places}$	= 7.233	7.233 m ²
PARTITION WALL		
TAXIIION WALL I WAS IN A CONTROL		
n = 89.867 : 10.00 8 places		
and the second s		
CONCRETE (TYPE-C1)	and the second of the second o	
	11.000	11.626
$V = 1.447 \text{m}^3 / \text{place x 8 places}$	= 11.576	11.576 m³
GRAVEL BEDDING		
$V = 0.579 \text{ m}^3 / \text{place x 8 places}$	= 4.632	4.632 m ³
FORM (H < 4.0 m)		
$A = 9.644 \mathrm{m^2/place} \times 8 \mathrm{places}$	= 77.152	77.152 m ²
A Soly in a page of a page of		
REINFORCING BAR		
		0.606.18
W = 0.087 tf / place x 8 places	= 0,696	0.696 tf
JOINT FILLER		
JOHN HUDDIN		
$A = 2.411 \text{ m}^2/\text{place x 8 places}$	= 19.288	19.288 m ²
		All the second s
والمراجع والمراجع والمراجع والمراجع والمراجع والمناجع والمناجع والمراجع والمراجع والمراجع والمراجع والمراجع والمراجع		alim ya Parana in Jamasa

TYPE OF WORK : REVETMENT FOR SIDE SLOPE OF 1:2.0 (WET STONE MASONRY)

LOCATION : WF.96L ~ WF.97L + 41.32 m

CALCULATION	RESULT
GABION CYLINDER	
GABION CYLINDER Ø 500	
$V = \frac{\pi}{4} \times 0.50^2 \times (3.00 + 7.826 + 1.00) \times 6 \text{ places} = 13.932$	13.932 m³
SOIL FILLING	
$V = (3.00 + 7.826 + 1.00) \times 3.00 \times 0.50 - 13.932 = 3.807$	3.807 m ³
	gistan mari iya
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<u>kassaturing sa tribit on traja traja sa kanang manang bibat sa trajak pang at na at at at pang at traja.</u> Bandat Manang at at trajak pang at ang manang pang at	
	Arthur San San San San
	The first of the
<u>and the control of t</u>	A STATE OF THE STATE OF

TYPE OF WORK : REVETMENT FOR SLOPE OF 1:2.0 (WET STONE MASONRY TYPE)
LOCATION : WF.100R ~ WF.100R+20.0m

CALCULATION		RESULT
STRUCTURAL EXCAVATION		
$V = 180.390 \mathrm{m}^3 / 15.0 \mathrm{m} \times 10.0 \mathrm{m}$	= 120.260	120.260 m ³
BACKFILL WITH SELECTED SOIL		
$V = 20.640 \text{ m}^3 / 15.0 \text{ m} \times 10.0 \text{ m}$	= 13.760	13,760 m ³
GRAVEL BEDDING		
1. BASE CONCRETE		
$V = 0.70 \mathrm{m}^3 / 10.0 \mathrm{m} \times 17.0 \mathrm{m}$	= 1.190	1.190 m³
2. FOR STANDARD SECTION		
$V = 51.876 \mathrm{m}^3 / 15.0 \mathrm{m} \times 10.0 \mathrm{m}$	= 34.584	34.584 m ³
TOTAL	= 35.774	35.774 m³
WET STONE MASONRY		
$V = 51.656 \mathrm{m}^3 / 15.0 \mathrm{m} \times 10.0 \mathrm{m}$	= 34.437	34.437 m ³
CEMENT MORTAR POINTING		
$A = 174.636 \text{ m}^2 / 15.0 \text{ m} \times 10.0 \text{ m}$	= 116.424	116.424 m²
DEFORMED REINFORCING BAR		
BASE CONCRETE		
$W = 0.1 \text{ tf} / 10.0 \text{ m} \times 17.0 \text{ m}$	= 0.17	0.170 tf
CONCRETE, FORM		
BASE CONCRETE .		
CONCRETE		
$V = 2.20 \text{ m}^3 / 10.0 \text{ m} \times 17.0 \text{ m}$	= 3.74	3.740 m ³
		3:740 III

TYPE OF WORK : REVETMENT FOR SLOPE OF 1:2.0 (WET STONE MASONRY TYPE) LOCATION : WF.100R \sim WF.100R+20.0m

CALCULATION CALCULATION		RESULT
FORM		
$A = 10.830 \text{ m}^2 / 10.0 \text{ m} \times 17.0 \text{ m} =$	18.411	18.411 m ²
JOINT FILLER		
JOHN FILLER		
BASE CONCRETE		
The state of the s		
$A = 0.22 \mathrm{m^2} / 10.0 \mathrm{m} \mathrm{x} 17.0 \mathrm{m} =$	0.374	0.374 m ³
CADIONAL COMPTOS		
GABION MATTRESS		
t = 500:		
$V_1 = 17.0 \times 0.5 \times 3.0$	25.5	
$V_2 = 17.0 \times 0.5 \times 1.5$	12.75	
TOTAL =	38.250	38.250 m ³
Many control of the c	30.230	38.230 m
RUBBLE STONE FILLING		
$A = 1/2 \times 1.0 \times 0.5 \times 2$	0.500 m ²	
$V = 0.500 \times 17.0 \text{ m}$	8.500 m ³	9.500 3
V - 0.500 X 17,0 iii	8.000 m	8.500 m ³
WEEP HOLE		
		The state of the s
PVC PIPE ø 50		
$n = 9/15.0 \text{ m} \times 10.0 \text{ m} =$	6 - 1 - 2 - 1 - 2 - 2 - 2	
$1 = 6 \times 0.8 =$	4.8 m	4.800 m
	A STATE OF THE STA	4.500 111
JOINT FILLER		
n = 6		
$A = 0.064 \text{ m}^2 / \text{place x 6} =$	0.294	0.2942
$A = 0.064 \text{ m}^2 / \text{place x 6} =$	0.384	0.384 m ²
	<u>ing ing panggarang dalah k</u> Panggarang langgarang dalah ka	

TYPE OF WORK : REVETMENT FOR SLOPE OF 1:2.0 (WET STONE MASONRY TYPE)
LOCATION : WF.100L ~ WF.100L+15.0 m

CALCULATION		RESULT
STRUCTURAL EXCAVATION		
$V = 180.390 \text{m}^3 / 15.0 \text{m} \times 7.0 \text{m} = 82$	J.182	84.182 m ³
BACKFILL WITH SELECTED SOIL		
$V = 20.640 \mathrm{m}^3 / 15.0 \mathrm{m} \times 7.0 \mathrm{m}$ = 9.	632	9.632 m ³
GRAVEL BEDDING		
1. BASE CONCRETE		
$V = 0.70 \text{ m}^3 / 10.0 \text{ m} \times 11.5 \text{ m} = 0.$	805	0.805 m ³
2. STANDARD SECTION		
$V = 51.876 \mathrm{m}^3 / 15.0 \mathrm{m} \times 7.0 \mathrm{m}$ = 24	.209	24.209 m ³
TOTAL = 25	.014	25.014 m ³
WET STONE MASONRY		
$V = 51.656 \mathrm{m}^3 / 15.0 \mathrm{m} \mathrm{x} 7.0 \mathrm{m} = 24$.106	24.106 m ³
CEMENT MORTAR POINTING		
$A = 174.636 \mathrm{m^2/15.0 m x 7.0 m} \qquad = 81$.497	81.497 m²
DEFORMED REINFORCING BAR		
1. BASE CONCRETE		
$W = 0.1 \text{ tf}/10.0 \text{ m} \times 11.5 \text{ m} = 0.$	115	0.115 tf
CONCRETE, FORM		
BASE CONCRETE.		
CONCRETE		
$V = 2.20 \text{ m}^3 / 10.0 \text{ m} \times 11.5 \text{ m} = 2.$	53	2.530 m ³

TYPE OF WORK : REVETMENT FOR SLOPE OF 1:2.0 (WET STONE MASONRY TYPE) LOCATION : WF.100L ~ WF.100L+20.0m

CALCULATIO	ON	RESULT
FORM		
$A = 10.830 \mathrm{m}^3 / 10.0 \mathrm{m} \times 11.5 \mathrm{m}$	= 12.455	12.455 m ³
TOTALE BUT I DO		
JOINT FILLER		
BASE CONCRETE		
Bride contendite		
$A = 0.22 \text{ m}^2 / 10.0 \text{ m} \times 11.5 \text{ m}$	= 0.253	0.253 m ²
GABION MATTRESS		
t = 500 mm		
$V_1 = 11.5 \times 0.5 \times 3.0$	= 17.250	
$V_2 = 11.5 \times 0.5 \times 1.5$	- 9.625	
$V_2 = 11.3 \times 0.3 \times 1.3$	= 8.625	
TOTA	AL = 25.875	25.875 m ³
		25.675 III
RUBBLE STONE FILLING	<u>antara da kabupatèn di Kabupatèn Bandara da Kabupatèn Bandara. Kabupatèn Bandara da Kabupatè</u>	The state of the s
$A = 1/2 \times 1.0 \times 0.5 \times 2$	$= 0.500 \mathrm{m}^2$	
$V = 0.500 \times 11.5 \text{ m}$	$= 5.750 \mathrm{m}^3$	5.750 m ³
WEEP HOLE		
WEEP HOLE		
PVC PIPE Ø 50		
$n = 9/15.0 \mathrm{m} \mathrm{x} 7.0 \mathrm{m}$	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	
$1 = 4 \times 0.8$	= 3.2 m	3.200 m
FILTER CLOTH		
		1
$A = 0.064 \text{m}^2/\text{place x 4}$	= 0.256	0.256 m ²
		<u>3</u>

TYPE OF WORK : EARTH RETAINING WALL : WF.100 + 22.48 m ~ WF.101 + 9.06 m (RIGHT BANK) :

CALCULATION		RESULT
WET STONE MASONRY		
V = ((0.40 + 2.180) v. V = 2.002 + (0.40 + 0.180)		
$V_1 = \{(0.40 + 2.189) \times \frac{1}{2} \times 2.982 + (0.40 + 2.183)\}$		
x ½ x 2.972} x ½ x 12.50	= 48.116	
V = ((0.40 ± 2.182) - 1/ - 2.072 ± (0.40 ± 1.402)		
$V_2 = \{(0.40 + 2.183) \times \frac{1}{2} \times 2.972 + (0.40 + 1.428) \times \frac{1}{2} \times 1.713 \} \times \frac{1}{2} \times 12.50$		
X /2 X 1./13 } X /2 X 12.50	= 33.775	e David Berger
$V_3 = \{(0.30 + 1.328) \times \frac{1}{2} \times 1.713 + (0.30 + 0.60)\}$		
$\times \frac{1.326}{2} \times \frac{1.713}{2} \times \frac{1.713}{2} \times \frac{1.703}{2} \times \frac{1.713}{2} $	= 9.740	and a series
X 72 X 0.30} X 72 X 12.029	≈ 9./40	
TOTAL		
Language Anna Carlos and Carlos a	= 91.631	91.631 m ³
GRAVEL BEDDING (t = 100)		
~~~. Bu bebuild (t = 100)		2 (1)
$V_1 = (2.389 + 2.383) \times \frac{1}{2} \times (12.50 + 0.10) \times 0.10$	7.000	
(2.50) X 72 X (12.50 + 0.10) X 0.10	= 3.006	
$V_2 = (2.283 + 1.628) \times \frac{1}{2} \times 12.50 \times 0.10$		3 7 8 7 3 9 3 9
12 (2.203 ) 1.020) X 72 X 12.30 X 0.10	= 2.444	mezatê na Giri 👔
$V_3 = (1.528 + 0.80) \times \frac{1}{2} \times (12.029 + 0.10) \times 0.10$		
$(1.020 + 0.00) \times 72 \times (12.029 + 0.10) \times 0.10$	.=\ 1.41Z	
TOTAL	= 6.862	
TOTAL	- 0.802	6.862 m ³
SCAFFOLDING		
$A_1 = (2.982 + 2.972) \times \frac{1}{2} \times 12.50$	= 37.213	
	37,213	
$A_2 = (2.982 + 2.972) \times 1.166 \times \frac{1}{2} \times 12.50$	= 43.390	1 15 13 2 (AVX 12 (AVX 2)
(REVISING COEFFICIENT)	43.320	
$A_3 = (2.972 + 1.713) \times \frac{1}{2} \times 12.50$	= 29.281	
$A_4 = (2.972 + 1.713) \times 1.166 \times \frac{1}{2} \times 12.50$	= 34.142	
TOTAL	= 144.026	144.026 m ²
JOINT FILLER t = 10, ELASTIC MATERIAL	ATRIVAO (III.) ATRIVAO (IIII.) ATRIVAO (III.) ATRIV	174.020 III
$A_1 = (0.40 + 2.189) \times 1/2 \times 2.982$	= 3.860	
$A_2 = (0.40 + 2.183) \times \frac{1}{2} \times 2.972$	= 3.838	
$A_3 = (0.40 + 1.428) \times \frac{1}{2} \times 1.713$	= 1.566	
TOTAL	= 9.264	0.2642
TO IAL	J.20 <del>7</del>	9.264 m ²

TYPE OF WORK : EARTH RETAINING WALL

LOCATION : WF.100 + 22.48 m  $\sim$  WF.101 + 9.06 m (RIGHT BANK)

CALCULATION	RESULT
CEMENT MORTAR POINTING	
$A_1 = (2.482 + 2.472) \times \frac{1}{2} \times 12.50 = 30.963$	
$A_2 = 0.40 \times 12.50 = 5.000$	
$A_3 = (2.472 + 1.213) \times \frac{1}{2} \times 12.50 = 23.031$	
$A_4 = 0.40 \times 12.50 \times 1.005 = 5.025$	
(REVISING COEFFICIENT)	
$A_5 = \frac{1}{2} \times 12.029 \times 1.213 = 7.296$	and the later
$A_6 = 0.30 \times 12.029 \times 1.005$ = 3.627	and the second
	1 2 1 1 1 4
TOTAL = 74.942	74.942 m ²
[ 발표 보통 - 1964년 전 1971년 시간 1일 시간 1964년 1965년 1965년 1965년 1967년 1967년 1967년 1967년 1967년 1967년 1967년 1967년 1967년 1967년 1967년 1	
도보는 기계의 기계를 보았다. 이 전투 전문 기계를 발표했습니다. 그는 경우 기계를 보고 있다. 이 기계를 보고 있다. 그 기계를 보고 있다. - 1일 기계를 보고 있는 것이 되었습니다. 기계를 보고 있는 것이 되었습니다. 그 기계를 보고 있는 것이 되었습니다. 그 기계를 보고 있습니다.	
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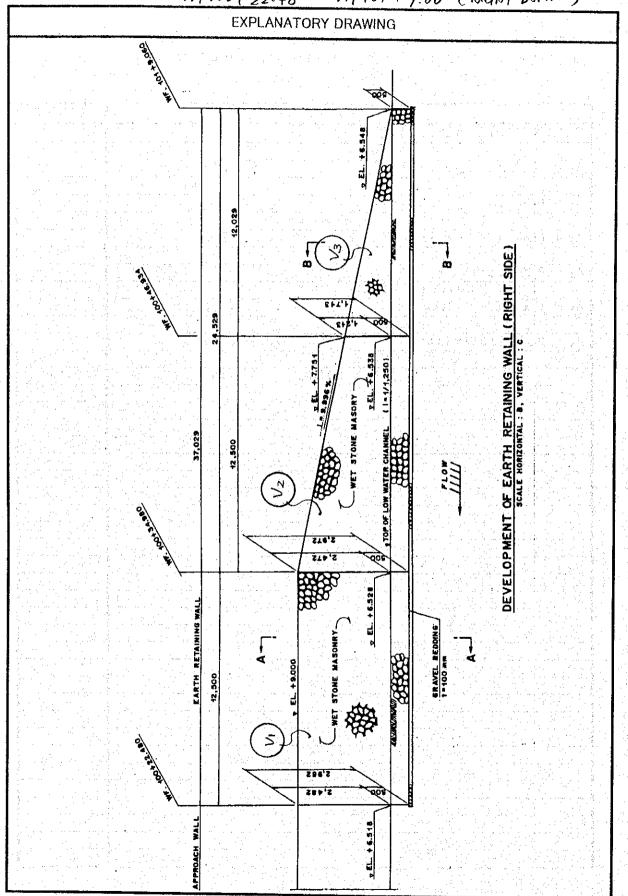
TYPE OF WORK

EARTH RETAINING WALL

WET STONE MASONRY

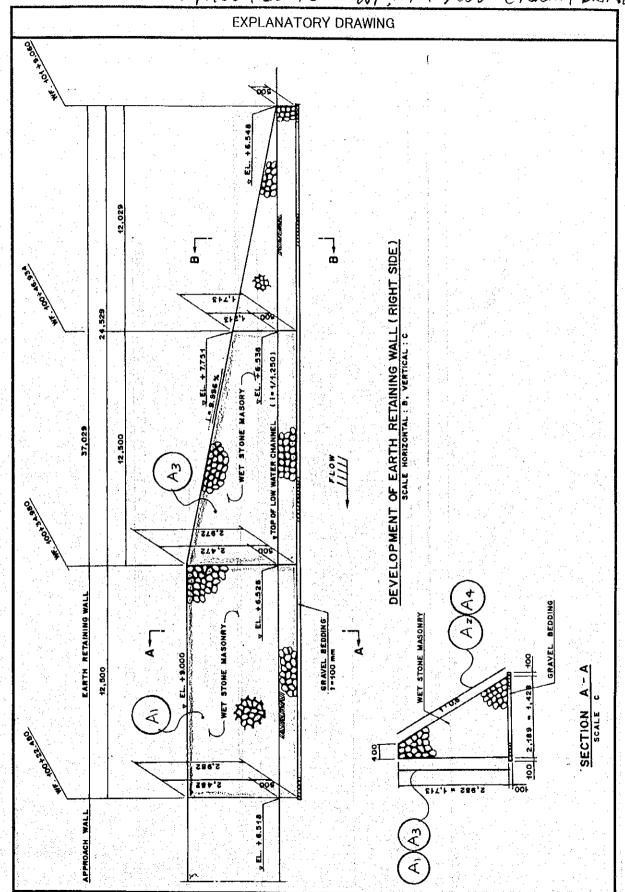
LOCATION

WF100+ 22-48 ~ WF101+9.06 (RIGHT BANK)



TYPE OF WORK

SCATFOLDING WF. 100 + 22-48~ WF. 101 + 9-06 (RIGHT BANK) LOCATION



TYPE OF WORK

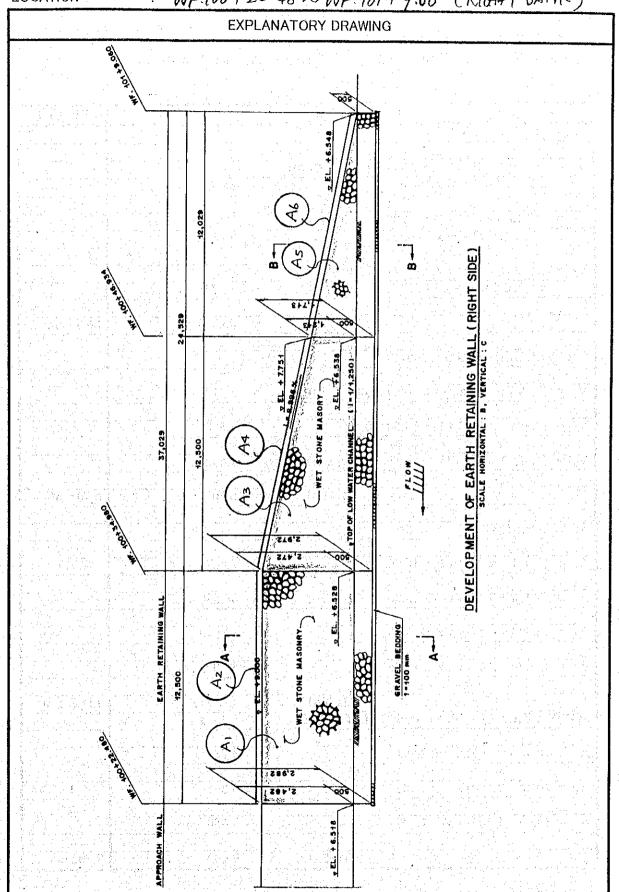
JOINT FILLER

LOCATION WF. 100+ 22.48 ~ WF 101+ 9.06 ( RIGHT BANK) **EXPLANATORY DRAWING** DEVELOPMENT OF EARTH RETAINING WALL (RIGHT SIDE) APPROACH WALL

TYPE OF WORK : STATUT MADTAR ROLLING

CEMBNT MORTAR POINTING

LOCATION : WF. (00+22-48 ~ WF. 101+9.06 (RIGHT BANK)



TYPE OF WORK : EARTH RETAINING WALL

LOCATION : WF.100 + 10.88 m ~ WF.100 + 45.881 m (LEFT BANK)

CALCULATION	RESULT
WET STONE MASONRY	
$V_1 = \{(0.40 + 2.195) \times \frac{1}{2} \times 2.991 + (0.40 + 2.190)\}$	
$x \frac{1}{2} \times 2.983 \times \frac{1}{2} \times 10.00 = 38.719$	
$V_2 = \{(0.40 + 2.190) \times \frac{1}{2} \times 2.983 + (0.40 + 1.466)\}$	
$x \frac{1}{2} \times 1.777 \times \frac{1}{2} \times 13.000 = 35.886$	i jako ja
$V_3 = \{(0.30 + 1.366) \times \frac{1}{2} \times 1.777 + (0.30 + 0.60)\}$	
$ x \frac{1}{2} \times 0.50 \times 2 \times 1.777 + (0.30 + 0.60) $ $ = 11.748 $	
11./46	
TOTAL = 86,353	86.353 m ³
	00.555 III
GRAVEL BEDDING (t = 100)	
$V_1 = (2.195 + 0.10 \times 2) \times 0.10 \times (10.00 + 0.10) = 2.419$	
$V_2 = (2.395 + 1.666) \times \frac{1}{2} \times 13.00 \times 0.10 = 2.640$	that ye florthe ver
V = (1566+0.90) + V + (12770+0.10) 0.10	
$V_3 = (1.566 + 0.80) \times \frac{1}{2} \times (13.779 + 0.10) \times 0.10 = 1.642$	
TOTAL = 6.701	6.701 m ³
	0.701 III
SCAFFOLDING	
$A_1 = (2.991 + 2.983) \times \frac{1}{2} \times 10.00 = 29.870$	
$A_2 = (3.488 + 3.478) \times \frac{1}{2} \times 10.00 = 34.830$	
A = (2.092   1.777)   17.00	
$A_3 = (2.983 + 1.777) \times \frac{1}{2} \times 13.00 = 30.940$	
$A_4 = (3.478 + 2.072) \times \frac{1}{2} \times 13.00 = 36.075$	
7-4 (2.772) X 72 X 13.00	
<del>1988年,1988年,1988年,1988年,1988年,1988年,1988年,1988年,1988年,1988年,1988年,1988年,1988年</del> ,1988年	131.715 m²
TOTAL = 131.715	
TOTAL = 131.715  JOINT FILLER t = 10, ELASTIC MATERIAL	191.713 11
JOINT FILLER t = 10, ELASTIC MATERIAL	
TOTAL = 131.715  JOINT FILLER t = 10, ELASTIC MATERIAL $A_1 = (0.40 + 2.195) \times \frac{1}{2} \times 2.991 = 3.881$	
JOINT FILLER $t = 10$ , ELASTIC MATERIAL $A_1 = (0.40 + 2.195) \times 1/2 \times 2.991 = 3.881$	
JOINT FILLER t = 10, ELASTIC MATERIAL	
JOINT FILLER t = 10, ELASTIC MATERIAL $A_1 = (0.40 + 2.195) \times \frac{1}{2} \times 2.991$ = 3.881 $A_2 = (0.40 + 2.190) \times \frac{1}{2} \times 2.983$ = 3.863	
JOINT FILLER t = 10, ELASTIC MATERIAL $A_1 = (0.40 + 2.195) \times 1/2 \times 2.991 = 3.881$	
JOINT FILLER t = 10, ELASTIC MATERIAL $A_1 = (0.40 + 2.195) \times \frac{1}{2} \times 2.991 = 3.881$ $A_2 = (0.40 + 2.190) \times \frac{1}{2} \times 2.983 = 3.863$	9.402 m ²

TYPE OF WORK : EARTH RETAINING WALL

LOCATION : WF.100 + 10.88 m ~ WF.100 + 45.881 m (LEFT BANK)

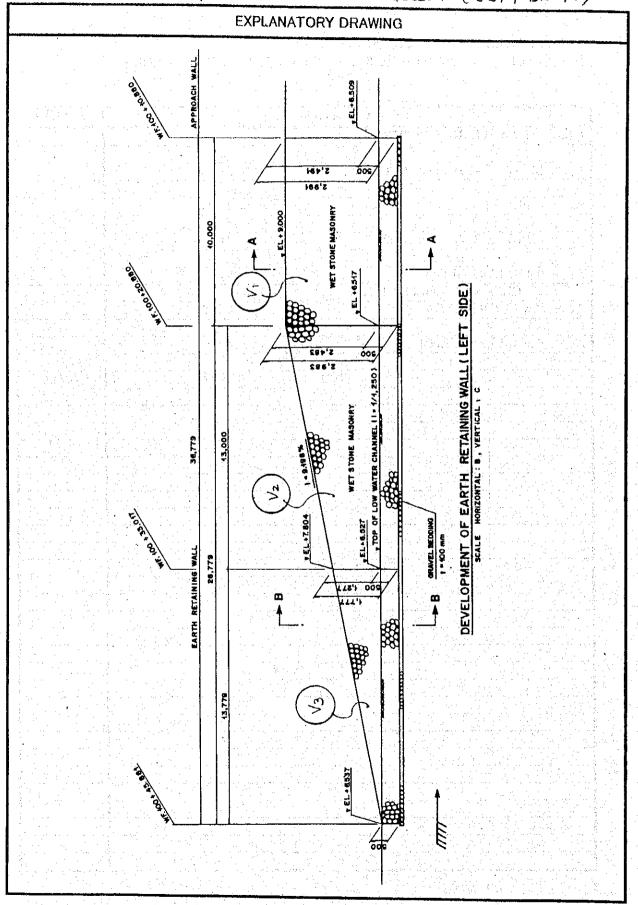
CALCULATION	RESULT
CEMENT MORTAR POINTING	
$A_1 = (2.941 + 2.483) \times 10.00 = 27.120$	1.
$A_2 = 0.40 \times 10.00 = 4.000$	
A = (2.482 + 1.072) - 1/ - 12.00	
$A_3 = (2.483 + 1.277) \times \frac{1}{2} \times 13.00 = 24.440$	
$A_4 = 0.40 \times 13.00 \times 1.004 = 5.221$	
(DELIGERIA CORRESPONDICIONALE)	
(REVISING COEFFICIENT) = 8.798	
5 72 X 13.777 X 1.277	
$A_6 = 0.30 \times 13.779 \times 1.004 = 4.150$	
TOTAL = 73.729	73.729 m ²
	75772
	1 1 4 1 1 1 1
	. Jageria
그는 사람들이 되었다는 그 등 경향에 하는 사람들은 그렇게 하는 것이 되는 것이 되는 것이 되었다. 그는 것이 되었다. 그는 것이 되었다. 그 그 그는 것이 되었다. 그는 것이 되었다. 그런 그는 사 그는 것이 되었다. 그렇게 하를 받는 것이 되었다. 그렇게 하는 것이 되었다. 그런 것이 되었다. 그런 것이 되었다. 그런 것이 되었다. 그런 그런 것이 되었다. 그런 그런 그런 것이 되었다. 그런	
	25.00
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TYPE OF WORK

WET STONE HASONRY

LOCATION

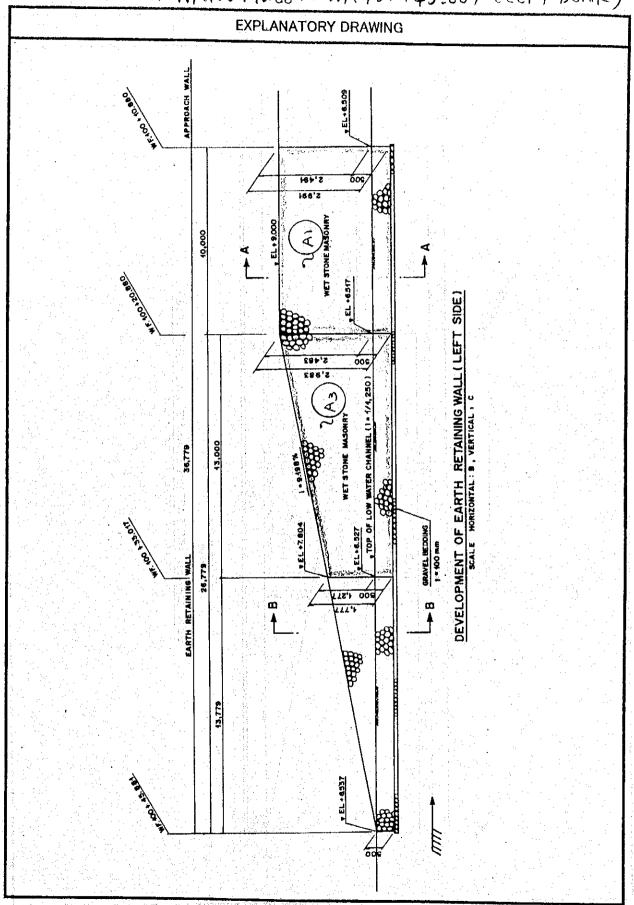
: WF.100+10.88 ~ WF.100 + 45.881 (LEFT BANK)



SCAFOLDING

TYPE OF WORK

LOCATION : WF. 100+ 10.88 ~ WF. 100+ 45.881 (LEFT BANK)

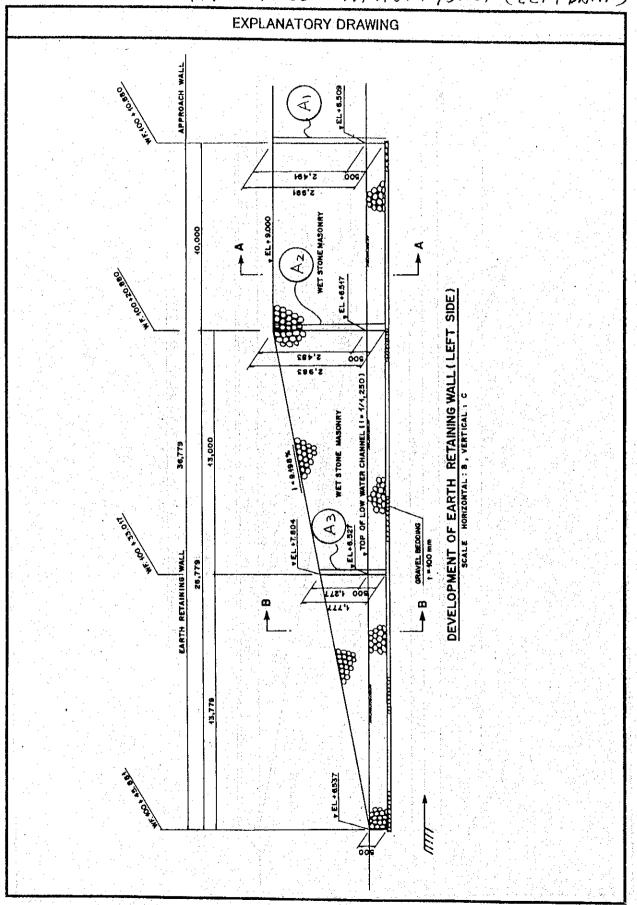


TYPE OF WORK

: JOINT THLLER

LOCATION

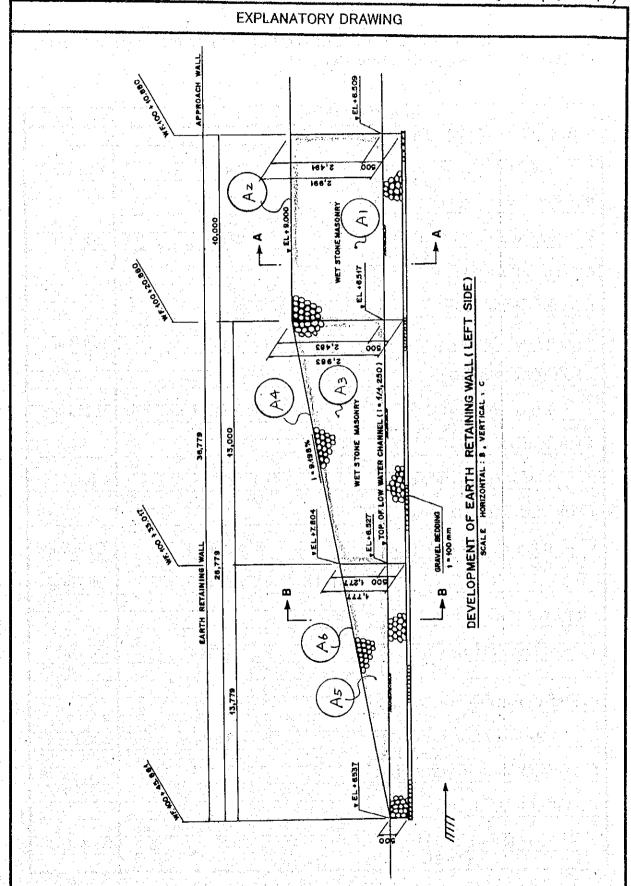
WF. 100+10.88 ~WF. 100+45.881 (LEFT BANK)



TYPE OF WORK

CEMENT MORTAR POINTING

LOCATION : WF. 100+10.88 ~ WF. 100 + 45.881 (LEFT BANK)



STYPE OF WORK : REVETMENT FOR SLOPE OF 1:2.0 (WET STONE MASONRY TYPE) LOCATION : WF.100R + 20.0 m  $\sim$  WF.104R

CALCULATION		RESULT
STRUCTURAL EXCAVATION		
$V = 180.390 \text{ m}^3 / 15.0 \text{ m} \times 213.239$	= 2564.412	2564.412 m ³
(R = 221.5)		
BACKFILL WITH SELECTED SOIL		
$V = 20.640 \text{ m}^3 / 15.0 \text{ m} \times 213.239$	= 293.417	293.417 m ³
(R = 221.5)		1.4
GRAVEL BEDDING		
1. BASE CONCRETE		
$V = 0.70 \text{ m}^3 / 10.0 \text{ m} \times 213.614$	= 14.953	14.953 m ³
(R = 221.5)		ala a grafi vario
2. FOR TOP CONCRETE		
		16016 3
$V = 0.75 \mathrm{m}^3 / 10.0 \mathrm{m} \times 213.550$	= 16.016	16.016 m ³
(R = 221.5)		
3. FOR PARTITION WALL		
	0.004	9.884 m³
$V = 0.706 \text{ m}^3 / \text{place x } 14 \text{ places}$	= 9.884	9.004 III
4 FOR STANDARY SECTION		
4. FOR STANDARD SECTION		
$V = 51.876 \mathrm{m}^3 / 15.0 \mathrm{m} \times 213.239$	= 737,466	737.466 m ³
	- /3/,400	757.400 IX
(R = 221.5)	= 778.319	778.319 m³
TOTAL	- 770,319	770.517 III
WET STONE MASONRY		
WEI STONE MASONAT		
$V = 51.656 \mathrm{m}^3 / 15.0 \mathrm{m} \times 213.239$	= 734,338	734.338 m ³
(R = 221.5)		
CEMENT MORTAR POINTING		
	in the second of	
$A = 174.636 \text{ m}^2 / 15.0 \text{ m} \times 213.239$	= 2482.614	2482.614 m ²
(R = 221.5)		
DEFORMED REINFORCING BAR		
1. BASE CONCRETE		
$W = 0.1 \text{ tf} / 10.0 \text{ m} \times 213.614$	= 2.136	2.136 tf
(R = 215.5)		
(-1, 2, 2, 2, 2, 2, 2, 2, 2, 2, 2, 2, 2, 2,		

TYPE OF WORK : REVETMENT FOR SLOPE OF 1:2.0 (WET STONE MASONRY TYPE) LOCATION : WF.100R + 20.0 m  $\sim$  WF.104R

CALCULATION		RESULT
2. TOP CONCRETE	again the art are a significant to the control of	
	= 2.007	2.007 tf
(R = 228.35)		
3. PARTITION WALL		
W = 0.102 (6/.114)	1.600	1 700 0
	= 1.722	1.722 tf
IOIAL	= 5.865	5.865 tf
CONCRETE, FORM		
CONGRETE, FORM		
1. BASE CONCRETE		
CONCRETE		
$V = 2.20 \text{ m}^3 / 10.0 \text{ m} \times 213.614$	= 46.995	46.995 m ³
(R = 215.5)		
FORM		
$A = 10.830 \text{ m}^2 / 10.0 \text{ m} \times 213.614$	= 231.344	231.344 m ³
2 TOR CONCERT		
2. TOP CONCRETE		
CONCRETE		
CONCRETE:		
$V = 1.80 \mathrm{m}^3 / 10.0 \mathrm{m} \times 213.550$	= 38.439	38.439 m ³
		30.137 m
FORM		
$A = 12.180 \text{ m}^2 / 10.0 \text{ m} \times 213.550$	= 260.104	260.104 m ³
		sa a a mark
3. PARTITION WALL		
CONCRETE		
	00.000	00 (00
$V = 2.117 \text{ m}^3 / \text{place x } 14 \text{ places}$	= 29.638	29.638 m ³
FORM		
FOREST CONTROL OF THE STREET CONTROL OF THE		
$A = 14.116 \text{m}^2/\text{place x } 14 \text{places}$	= 197.624	197.624 m ³
11 14.110 m / prace x 14 praces	177.024	197.024 111
TOTAL CONCRETE =	= 115.072	115.072 m ³
	= 689.072	689.072 m ²
		207, V 1 M 343

TYPE OF WORK : REVETMENT FOR SLOPE OF 1:2.0 (WET STONE MASONRY TYPE) LOCATION : WF.100R + 20.0 m  $\sim$  WF.104R

CALCULATION		RESULT
JOINT FILLER		
1. BASE CONCRETE		
		<b>使我们是</b> 在1967年1
$A = 0.22 \mathrm{m^2} / 10.0 \mathrm{m} \times 213.614$	= 4.700	4.700 m ²
(R ≈ 215.5)		
2. TOP CONCRETE		
$A = 2.645 \mathrm{m^2}/10.0 \mathrm{m} \times 213.550$	= 56.484	56 4942
(R = 228.35)	- 30.464	56.484 m ²
3. PARTITION WALL		
$A = 3.529 \text{m}^3 / \text{place} \times 14 \text{places}$	= 49.406	49.406 m ³
TOTA	= 110.590	110.590 m ³
GABION MATTRESS		· 自由进入) / (
t = 500		
$L_1 = 213.445 \text{ m } (R = 215)$		PARVEUR ( § )
$L_2 = 213.361 \text{ m } (R = 214.75)$		
$L_2 = 213.301 \mathrm{m} (\mathrm{R} = 214.73)$		
$V_1 = 213,445 \times 0.5 \times 3.0$	= 320.168	
11 213,773 X 0.3 X 3.0	<b>320.108</b>	
$V_2 = 213.361 \times 0.5 \times 1.5$	= 160.021	
$V = V_1 + V_2$	= 480.189	480.189 m ³
RUBBLE STONE FILLING		3 5 1 5 1 3 1 A 1
$A = 1/2 \times 1.0 \times 0.5 \times 2$	$= 0.500 \mathrm{m}^2$	
$V = 0.500 \times 213.451$	$= 106.726 \mathrm{m}^3$	106.726 m ³
(R = 216.5)		

TYPE OF WORK : REVETMENT FOR SLOPE OF 1:2.0 (WET STONE MASONRY TYPE) LOCATION : WF.100R + 20.0 m  $\sim$  WF.104R

CALCULATION	RESULT
WEEP HOLE	
PVC PIPE Ø 50	
$n = 9/place \times 14 = 126$	
$n = 9 / place \times 14 = 126$	
$1 = 126 \times 0.8 = 100.8$	100.800 m
	100.000 111
FILTER CLOTH	
$\mathbf{n} = 126$	
$A = 0.064 \text{ m}^2 / \text{place x } 126 = 8.064$	8.064 m ²
그는 사용적으로 함께 함께 하는 사용적으로 가득하는 것이 되었다. 그 사용 사용 사용 기계를 받는 것이 되었다. 그는 사용자 기계를 받는 것이 되었다. 사용자 기계를 받는 것이 되었다. 그는 사용자 기계를 받는 것이 되었다.	
는 마음에 가장 하는 것이 되었다. 그는 사람들은 사람들은 사람들이 되었다. 그는 사람들이 되었다. 그는 사람들이 되었다. 그는 사람들이 되었다. - 1988년 - 1988	
	ila la l
- [4일] 보고 있는 14일 (1942년) - 1945년 1일 전 12일 (1942년) - 1945년 1일 전 194 - 1945년 1월 1947년 1일 1947년 1일 1947년 1월 1947년 1월 1947년 1일 1947년 1일 1947년 1일 1947년 1일 1947년 1일 1947년 1일 1947년 1월	
- 제공하다 그 가는 그는 보다 하는 그 이번 가는 그 사람들이 되었다. 그는 것이 되었다. - [사람들의 : 18] - 18] - 18] - 18] - 18] - 18] - 18] - 18] - 18] - 18] - 18] - 18] - 18] - 18] - 18] - 18] - 18]	
	<del></del>

TYPE OF WORK : REVETMENT FOR SLOPE OF 1:2.0 (WET STONE MASONRY TYPE) LOCATION : WF.100L+15.0 m  $\sim$  WF.104L

CALCULATION		RESULT
STRUCTURAL EXCAVATION		
$V = 180.390 \mathrm{m}^3 / 15.0 \mathrm{m} \times 212.229$	= 2552.266	2552,266 m ³
(R = 178.5)		
BACKFILL WITH SELECTED SOIL		
V 20.640 3/15.0 210.000		a and the second
$\dot{V} = 20.640 \text{ m}^3 / 15.0 \text{ m} \times 212.229$ (R = 178.5)	= 292.027	292.027 m ³
GRAVEL BEDDING		
GRAVEL DEDDING		
1. BASE CONCRETE		
		ing garage at the second of th
$V = 0.70 \text{ m}^3 / 10.0 \text{ m} \times 216.754 \text{ m}$	= 15.173	15.173 m ³
(R =184.5)		Serve August August
2. FOR TOP CONCRETE		
$V = 0.75 \text{ m}^3 / 10.0 \text{ m} \times 206.918$	= 15.519	15.519 m ³
(R=171.65)		tangah daga dan salah salah daga da
3. FOR PARTITION WALL		
$V = 0.706 \mathrm{m}^3 /\mathrm{place}  \mathrm{x}  14 \mathrm{places}$	= 9.884	9.884 m³
4. FOR STANDARD SECTION		
4. FOR STAINDARD SECTION		
$V = 51.876 \mathrm{m}^3 / 15.0 \mathrm{m} \times 212.229$	= 733,973	733.973 m³
(R = 178.5)	, <u>, , , , , , , , , , , , , , , , , , </u>	133.513 M
TOTAL	= 774.549	774.549 m³
		77. <del>1.51</del> 7 M
WET STONE MASONRY		
$V = 51.656 \mathrm{m}^3 / 15.0 \mathrm{m} \times 212.229$	= 730.860	730,860 m ³
(R =178.5)		
CEMENT MORTAR POINTING		
$A = 174.636 \text{ m}^2 / 15.0 \text{ m} \times 212.229$	= 2470.855	2470.855 m ²
(R =178.5)		
DEFORMED REINFORCING BAR		
1. BASE CONCRETE		
I. DADE CONCRETE		
$W = 0.1 \text{ tf} / 10.0 \text{ m} \times 216.754$	= 2.168	2.168 tf
(R =184.5)	Z.100	4.108 U
(IC 10T.J)	green and the conservation of the reservations	a sa garanta jarah dah

TYPE OF WORK : REVETMENT FOR SLOPE OF 1:2.0 (WET STONE MASONRY TYPE)
LOCATION : WF.100L+15.0 m ~ WF.104L

CALCULATION CALCULATION		RESULT
2. TOP CONCRETE		
$W = 0.094 \text{ tf} / 10.0 \text{ m} \times 206.918$ $(R = 171.65)$	= 1.945	1.945 tf
3. PARTITION WALL		
3.11ACTITION WALL		
W = 0.123  tf/place x  14  places	= 1.722	1.722 tf
mom. ·	= 5.835	5.835 tf
CONCRETE, FORM		
I. BASE CONCRETE		Art Carrier
1. BASE CONCRETE		
CONCRETE	is the majority of the conservation of the con	
$V = 2.20 \text{ m}^3 / 10.0 \text{ m} \times 216.754$	= 47.686	47.686 m³
(R =184.5)		
FORM		
$A = 10.830 \mathrm{m^2/10.0 m x 216.754}$	= 234.745	234.745 m ²
2. TOP CONCRETE		
2. TOT CONCRETE		
CONCRETE		
		Va Dara e A
	= 37.245	37.245 m ³
(R=171.65)		
FÖRM		n de la signa en de con
$A = 12.180 \text{ m}^2 / 10.0 \text{ m} \times 206.918$	= 252.026	252.026
A = 12.180 iii / 10.0 iii \( \frac{1}{2} \) 200.918	- 232.026	252.026 m ²
3. PARTITION WALL		
CONCRETE		
		A Company of the second
$V = 2.117 \text{m}^3/\text{place} \times 14 \text{places}$	= 29.638	29.638 m ³
FORM	<u> </u>	
FUNITE CONTRACTOR CONT		
$A = 14.116 \text{ m}^2/\text{ place x } 14 \text{ places}$	= 197.624	197.624 m ²
7 THEO II / place & 17 places	= 197.624	177.024 III
TOTAL CONCRETE	= 114.569	114.569 m ³
	= 684.395	
	2 2 110 07	114.569 m ³ 684.395 m ²

TYPE OF WORK : REVETMENT FOR SLOPE OF 1:2.0 (WET STONE MASONRY TYPE) LOCATION : WF.100L+15.0 m  $\sim$  WF.104L

	CULATION		RESULT
JOINT FILLER			
1. BASE CONCRETE			
1. BASE CONCRETE		a la	
$A = 0.22 \mathrm{m^2} / 10.0 \mathrm{m} \times 216.754$		4.769	4.769 m ²
(R =184.5)		4.709	4.769 m
2. TOP CONCRETE			
$A = 2.645 \mathrm{m}^2 / 10.0 \mathrm{m} \times 206.918$	<b>=</b>	54.730	54.730 m ²
(R =171.65)			
3. PARTITION WALL		we also tall, tage fo	
$A = 3.529 \text{ m}^3 / \text{place x } 14 \text{ places}$		40.406	40.482
A = 3.323 m 7 place x 14 places		49.406	49.406 m ³
	TOTAL =	108.905	108.905 m ³
		10 1 0 N 10 1 1 1 1 1 1 1 1 1 1 1 1 1 1	100.203 III
GABION MATTRESS			
			14 1 14 14 14 15 15
t = 500			
$L_1 = 217.007 \mathrm{m}$			
$L_1 = 217.007 \mathrm{m}$		nga nga kabatawa 18 ₀ ka <del>Tanàn</del> a	
$L_2 = 216.923 \text{ m}$			
$V_1 = 217.007 \times 0.5 \times 3.0$	=	325.511	
$V_2 = 216.923 \times 0.5 \times 1.5$		162.692	
$V = V_1 + V_2$		488.203	488.203 m ³
RUBBLE STONE FILLING			
ACEDER STORE PILEMAG			
$A = 1/2 \times 1.0 \times 0.5 \times 2$		0.500 m ²	
$V = 0.500 \times 216.417$	Salaka ja Baran	108.209 m ³	108.209 m ³
(R =183.5)			
		55 Sec. 1	
		ovijski vojstali. Atvati kojs	

TYPE OF WORK : REVETMENT FOR SLOPE OF 1:2.0 (WET STONE MASONRY TYPE)

LOCATION		: WF.100L+15.0 m ~	WF.104L
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CALCULATION	RESULT
WEEP HOLE	
PVC PIPE Ø 50	
	1 1
n = 9 / place x 14 = 126	1
	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
$i = 126 \times 0.8 = 100.8$	100.800 m
THE CLEAN AND A STATE OF THE ST	
FILTER CLOTH	
n = 126	
$A = 0.064 \text{ m}^2/\text{place} \times 126 = 8.064$	8.064 m ²
- 8.004	8.004 III
Note that the second of the production of the contract of the second of the second of the second of the second	
	1 1

RESULT								1		1947 m ³	
CONCRETE CTYPE - D)	y = 0.50 × 0.30 × 1.20 == 0.180	1 1	V3 = 1.10 × 0.30 × 1.20 = 0.396	V4 = 1.40 x 0.30 x 1.20 = 0.504	Vs = 1.95 × 0.30 × 1.20 = 0.630	CDeduction for Step)	V6=-1/2 x 0.03 x 0.03 x 1.20 x 5 = -0.003	(Deduction for Blockout)	Vq = -(0.20 × 0.20 × 0.30) × 4 = -0.048	7774 = 1,947	
MAINTENANCE STEPS TYPE OF WORK: CONCRETE LOCATION: GATE PIER			500 38200=800 360 C = 3000 250 58200=1,000 200 200 200 200 200 200 200 200 200			100 100 100 100 100 100 100 100 100 100	100 100 100 100 100 100 100 100 100 100	000 g + 12 1 2 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	L=300mm	150 C 100 look	

	RESULT															7.960 m ²				
	Control of the contro	( <b>m</b> 0*+>H)	$A_1 = 1.75 \times 1.50 \times 2$		$A_2 = 1.20 \times 1.50$		A3 = 0.27 x 1.20 x 5	A4 = 1/2 x 0.03 x 1.20 x S = 0. 255	As = - (0.35 x 1.20 + 0.30 x 0.90 + 0.30 x 0.60	$+0.30 \times 0.30) \times 2 = -1.920$	A6 = - /2 × 0.03 2 × 2 × 5 = -0.005		$A_T = (0.3 \times 0.20 \times 4) \times 4 = 0.960$			DTAL = 7.960				
MAINTENANCE STEPS	TYPE OF WORK	LOCATION: GATE PIER	《《新香》 《《新香》 《新香》 《《新香》 《《新香》 《《新香》 《《新香》 《新香》	《《《··································		D 500 3900=800 C -	STEEL PIPE #50. (LO INCH! t=2.3mm	ROUND BAR #16 (CALWAWZED)	(GSZIMMANO)	0000	\(\) \(\)	-(A2) / (A) (A) (A) (A)		000 6-13 47 47 47 47 47 47 47 47 47 47 47 47 47	S ELIO BARE THOO		1po C - 150 C			

TYPE OF WORK LOCATION

## MAINTENANCE STEPS HANDRAIL

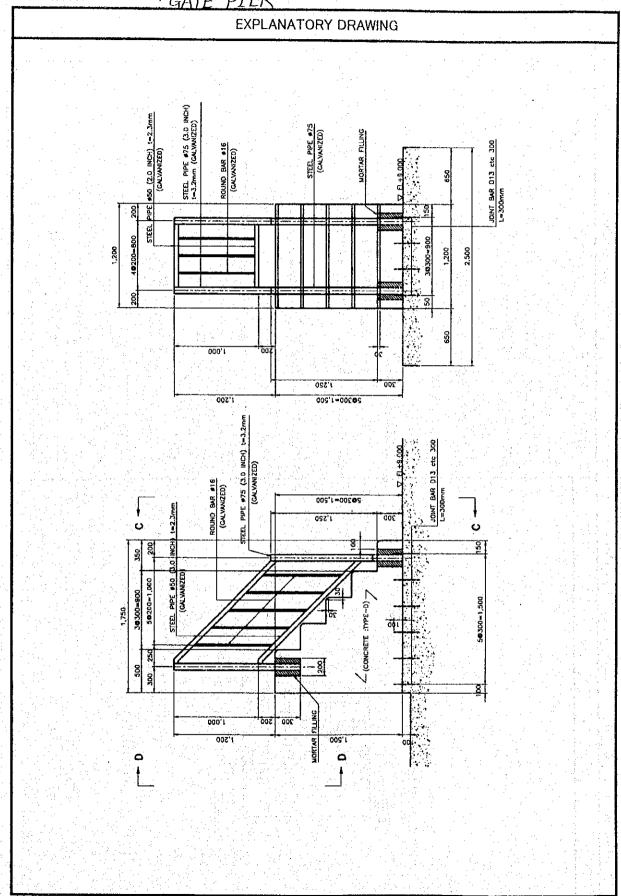
	RESULT
(GALVANIRED)	
· STEEL PIPE \$75 (3.0 inch ) t = 3.2mm	
n=z, L=1.50 m Unit Weight: w= 5.77 kg/m	
$W_1 = 2 \times 1.50 \times 5.77$ = 17.310	
n = 2, $L = 1.55 m$	
$W_2 = Z \times 1.55 \times 5.77 = 17.887$	
STEEL PIPE \$50 (2.0 inch) t=2.3mm	
$n = 4$ , $L = 1.663 \text{ m}$ Unit Weight: $W = 2.63 \frac{19}{m}$	
w3 = 4 x 1.663 x 2.63 = 17.495	
100 = 4 x 1 663 x 2 x 2	
ROUND BAR 9/6	
ROUND BAR \$16	
ROUND BAR \$16 $n = 10$ , $l = 0.929  \text{m}$ Unit Weight: $w = 1.58  \frac{149}{m}$	
ROUND BAR 9/6 $n = 10$ , $1 = 0.929  \text{m}$ Unit Weight: $w = 1.58  \frac{1}{8}  \frac{1}{m}$	
ROUND BAR 9/6 $n = 10$ , $l = 0.929  \text{m}$ Unit Weight: $w = 1.58  \frac{149}{m}$	
ROUND BAR 9/6 $n = 10$ , $L = 0.929  m$ Unit Weight: $w = 1.58  \frac{149}{m}$	67.370 kg
ROUND BAR $\%/6$ $n = 10$ , $l = 0.929  \text{m}$ Unit Weight: $W = 1.58  \%/m$ $W4 = 10 \times 0.929 \times 1.58$ = 14.678	67.370 kg
ROUND BAR $\%/6$ $n = 10$ , $l = 0.929  \text{m}$ Unit Weight: $W = 1.58  \%/m$ $W4 = 10 \times 0.929 \times 1.58$ = 14.678	67.370 kg
ROUND BAR \$16 $n = 10$ , $l = 0.929  \text{m}$ Unit Weight: $W = 1.58  \frac{149}{m}$ $W4 = 10 \times 0.929 \times 1.58$ = 14.678	67.370 kg
ROUND BAR 9/6 $n = 10$ , $l = 0.929  \text{m}$ Unit Weight: $w = 1.58  \frac{140}{m}$ $w_4 = 10 \times 0.929 \times 1.58$ = 14.678	67.370 kg
ROUND BAR 9/6 $n = 10$ , $l = 0.929  \text{m}$ Unit Weight: $w = 1.58  \frac{140}{m}$ $w_4 = 10 \times 0.929 \times 1.58$ = 14.678	67.370 kg
ROUND BAR 9/6 $n = 10$ , $L = 0.929  \text{m}$ Unit Weight: $W = 1.58  \frac{149}{m}$ $W4 = 10 \times 0.929 \times 1.58 = 14.678$	67.370 kg
ROUND BAR 9/6 $n = 10$ , $L = 0.929  \text{m}$ Unit Weight: $W = 1.58  \frac{149}{m}$ $W4 = 10 \times 0.929 \times 1.58 = 14.678$	67.370 kg
ROUND BAR \$16 $n = 10$ , $l = 0.929  \text{m}$ Unit Weight: $W = 1.58  \frac{149}{m}$ $W4 = 10 \times 0.929 \times 1.58$ = 14.678	67.370 kg

MAINTENANCE STEPS

TYPE OF WORK

:_HANDRAIL

LOCATION GATE PIER



RESULT						,		:					12.8/4 19	>		1. S.				·	
CALCULATION		· STEEL PIPE \$50 (2,0 inch) t=2,3mm	n = 2, $L = 0.90  m$ Unit Weight: $W = 2.63  kg/s$		$W_1 = (Z \times 0.90 + Z \times 0.725) \times 2.63 = 8.598$		· ROUND BAR 9/6		n=3. L=0.90m Unit Weight: W=1.58 18/1		$Wz = 3 \times 0.90 \times 1.58$ = 4.266		TOTAL = 12.814			· PAÐ10CK					
TYPE OF WORK   MAINTENANCE STEPS	MAN. DUCK	LOCATION: CATE PIER				200		STOPPER (PAULOCK)	003	000'1		600 CO2			DETAIL OF TET / 2000   MORTAR FILLING	STEE PIPE #50 (2.0 INOH) I=2.3mm	COZDINATED)				

MAINTENANCE STEPS		
HORTAR FILLING	CALCULATION	RESULT
	V= {(0,20 × 0,20) - 1/4 × 0.075 2 } × 0.30 × 4	
	= 0.043	0.043 m3
1,750		
500 38300-900 350 C		
STEEL PIPE #50 (3.0		
ROUND BAR #16		
GTET, PIPE 375 (ALC INCH)		
(GALIAWIZED)		
0005,1 = 0.0000,1 = 0.0000,0 = 0.0000,0 = 0.0000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.000,0 = 0.0		
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Ladwi by other		
0051=0050		

CALCULATION RESULT  D = 6 x 4 = 24 Bars  D/3 : Unit Weight = 7.04 18/m  W = 24 x 0.30 x 7.04 = 7.488 7.488 8/m  D = 1.04 x 0.30 x 7.04 = 7.488 7.488 8/m  D = 1.04 x 0.30 x 7.04 = 7.488 7.488 8/m  D = 1.04 x 0.30 x 7.04 = 7.488 7.488 8/m  D = 1.04 x 0.30 x 7.04 = 7.488 8/m  D = 1.04 x 0.30 x 7.04 = 7.488 8/m  D = 1.04 x 0.30 x 7.04 = 7.488 8/m								in the control of the	一一一一一一一一一一一一一一一一一一一一一一一一一一一一一一一一一一一一一一																								- Company of the Comp	The state of the s																																															The state of the s												LIMP.	ww.	.Zmm	ww.C			
TYPE OF WORK: ANCHOR BAR LOCATION: GATE PIER  LOCATION: GANCHOR BAR LOCATION: GANCHOR BAR  STEL PIE 45 GO INCH 142.3mm  ROUND BAR # 16  CONORER TIPE - 05 GO INCH 142.3mm  CONORER TIPE - 05 GO	L=300mm		005 1-00F <b>@</b> 5	D 000	G				2. WWW002=7	CIT PORT INITION	TO STATE LANDS	ķ	ŀ	I	ŀ	ŀ	ŀ	Ì	Ì	が かんかい かんしょう かん カイス・アイ・ス・アイ・アイ・アイ・アイ・アイ・アイ・アイ・アイ・アイ・アイ・アイ・アイ・アイ・					CONCRETE TYPE-0)-7	1	- Z	•								09	765	0	2	2	0	000	000	ŌŚ	ŌŚ	) )	) )	300	200	, c	, c	200	200	200	200	200	2			0	0	0	C	0	300	000	1 0						\$ 7 T	\$   S   S	Soo	0														CHANGE AND A STATE				מילים	STEEL PIPE #75 (3.0	0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0			