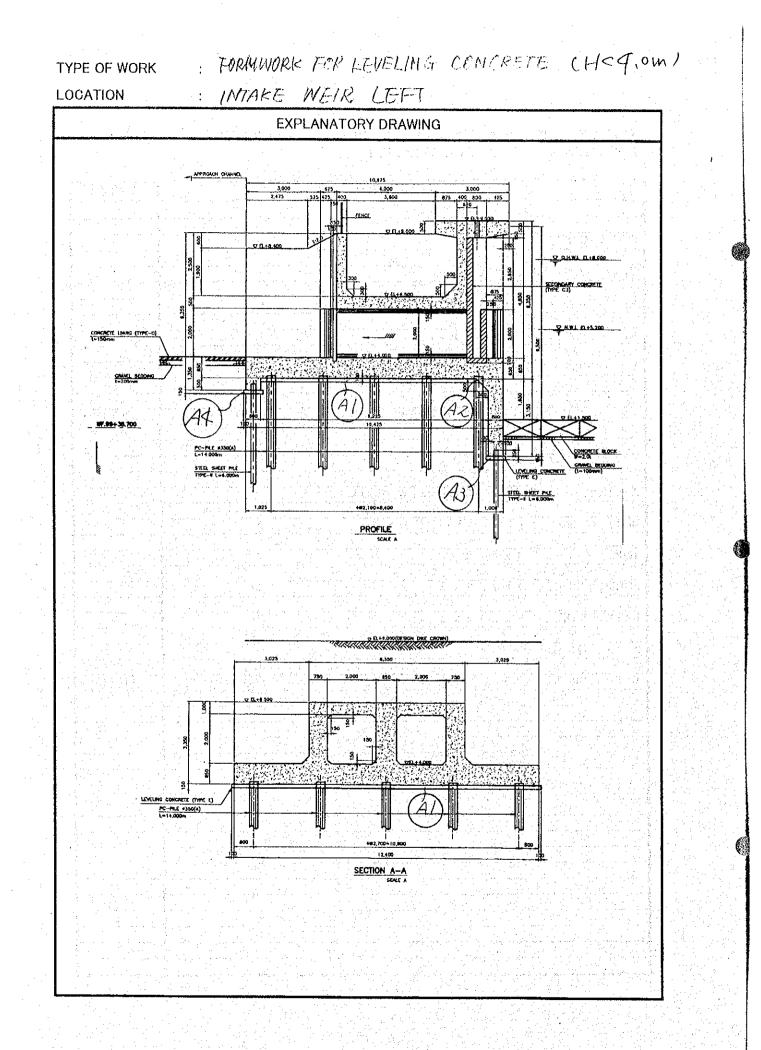


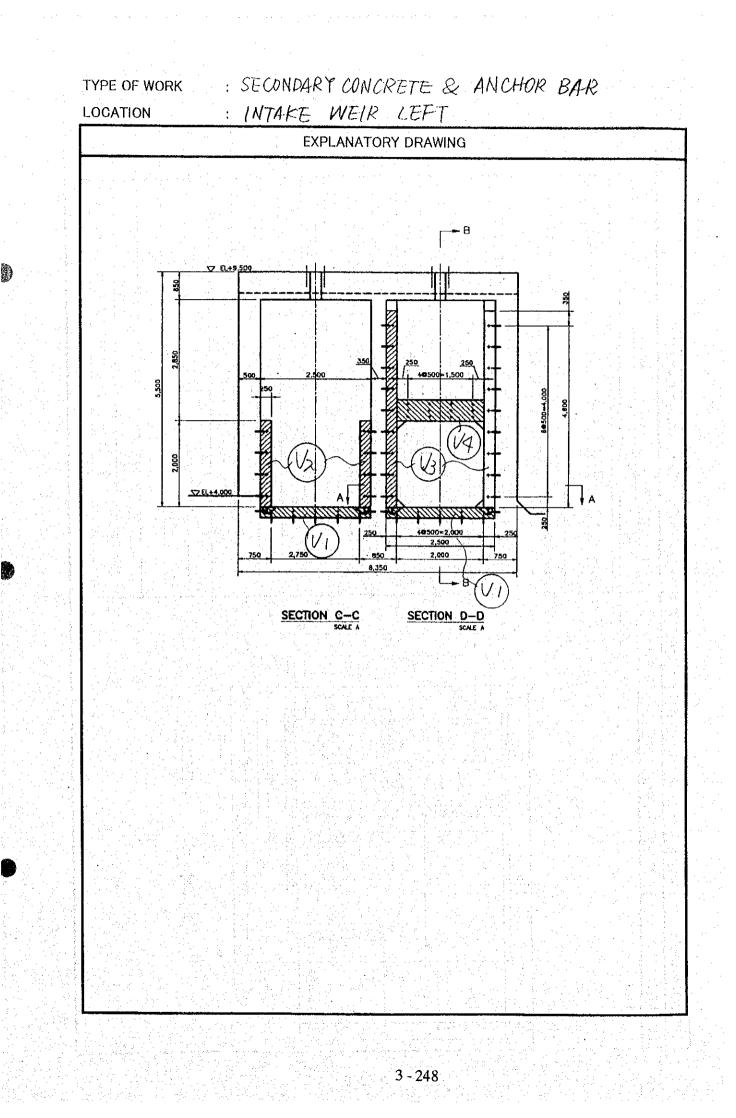
TYPE OF WORK :LOCATION :INTAKE WEIR RIGHT

CALCULATION	RESULT
D PODM (II < (A)	
\Box FORM (H < 4.0 m)	
$A_1 = 0.850 \times 8.675 \times 2 = 14.748$	
$A_2 = (\sqrt{2} \times 0.200 \times 2 + 1.6) \times 6.775 \times 2 \times 4 = 117.380$	
$A_2 = (\sqrt{2} \times 0.200 \times 2 + 1.6) \times 6.775 \times 2 \times 4 = 117.380$	
$A_3 = 1.35 \times 11.55 = 15.593$	
= 15.593	
TOTAL = 147.721	147.721 m ³
n de la construcción de la constru A secondario de la construcción de	
na senara de la contra la constante en esperan en entre a tradecimiente de la constante de constante de server A server se server en la constante de la constan	
□ FORMWORK FOR LEVELLING CONCRETE (H < 4.0 m)	
$A_1 = (10.425 - 0.6 \times 2 - 0.5) \times 0.15 \times 2 = 2.612$	
$A_2 = \frac{1}{2} \times 0.15 \times 0.15 \times 2 = 0.023$	
$\frac{A_2}{A_2} = \frac{1}{2} \times 0.15 \times 0.15 \times 2 = 0.023$	
$A_3 = (0.8 \times 2 + 20.2 \times 2) \times 0.15 = 6.300$	
$A_4 = (0.8 \times 2 + 20.2 \times 2) \times 0.15 = 6.300$	
	en al anna an a

TYPE OF WORK:SECONDARY CONCRETE & ANCHOR BARLOCATION:INTAKE WEIR LEFT

	RESULT
(TYPE – C3)	· . · · · · · · · · · · · · · · · · · ·
	· · · · · · · · · · · · · · · · · · ·
$V_1 = 2.5 \times 0.8 \times 0.25 \times 2 = 1.000$	
$V_2 = 0.25 \times 2.0 \times 0.285 \times 4 = 0.570$	<u> </u>
$V_2 = 0.25 \times 2.0 \times 0.285 \times 4 = 0.570$	
$V_3 = 0.25 \times 4.95 \times 0.25 \times 4$ = 1.238	
¥3 - 0.25 X 4.75 X 0.22 X 4	
$V_4 = (0.5 \times 0.22 + 0.175 \times 0.03) \times 2.0 \times 2 = 0.461$	
TOTAL = 3.269	3.269 m ³
ANCHOR BAR (D19, $L = 300$ mm/Bar, $W = 2.23$ kgf/m)	e la desta de la companya de la comp
BOTTOM $(5 \times 3 + 5 \times 2 + 4) \times 2 = 58$	
TOP $3 \times 4 \times 2 = 24$	
TOP $3 \times 4 \times 2$ = 24	
FORMER SIDE $4 \times 2 \times 2 \times 2 = 32$	en e
LATTER SIDE $9 \times 2 \times 2 \times 2 = 72$	
	e de la constante de la
n = 186 Bars	
W = 186 Bars x 0.30 x 2.23 kgf/m = 124.434 kgf	0.124 tf
	0112111

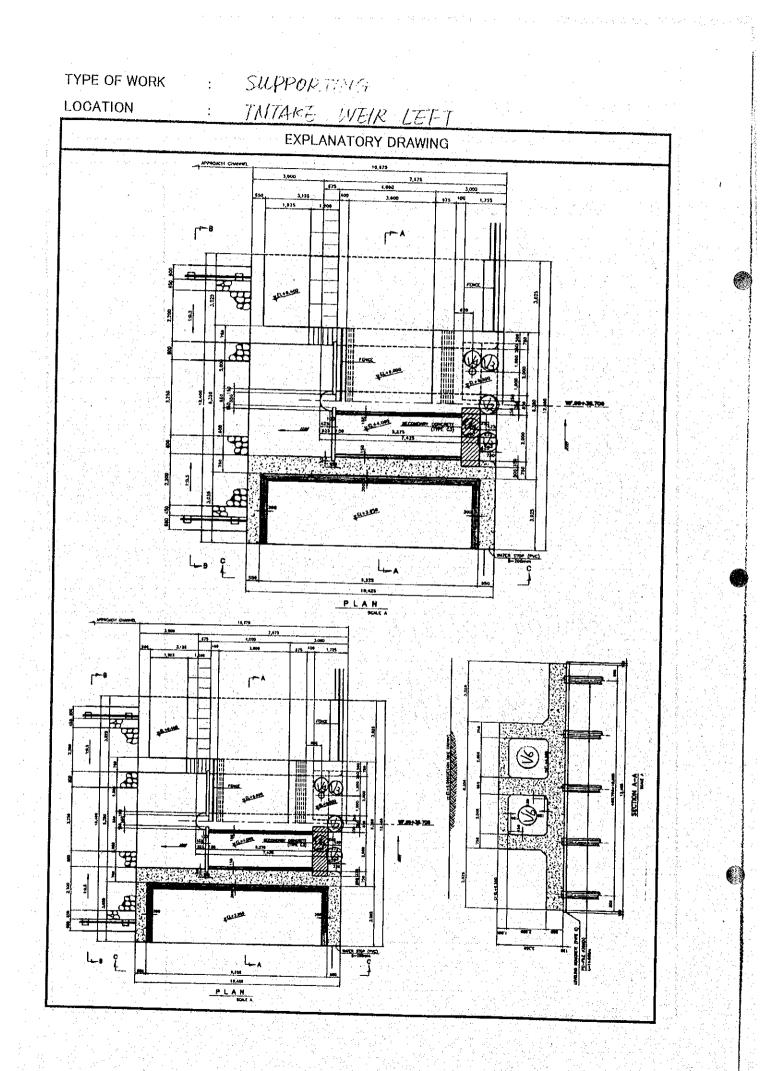




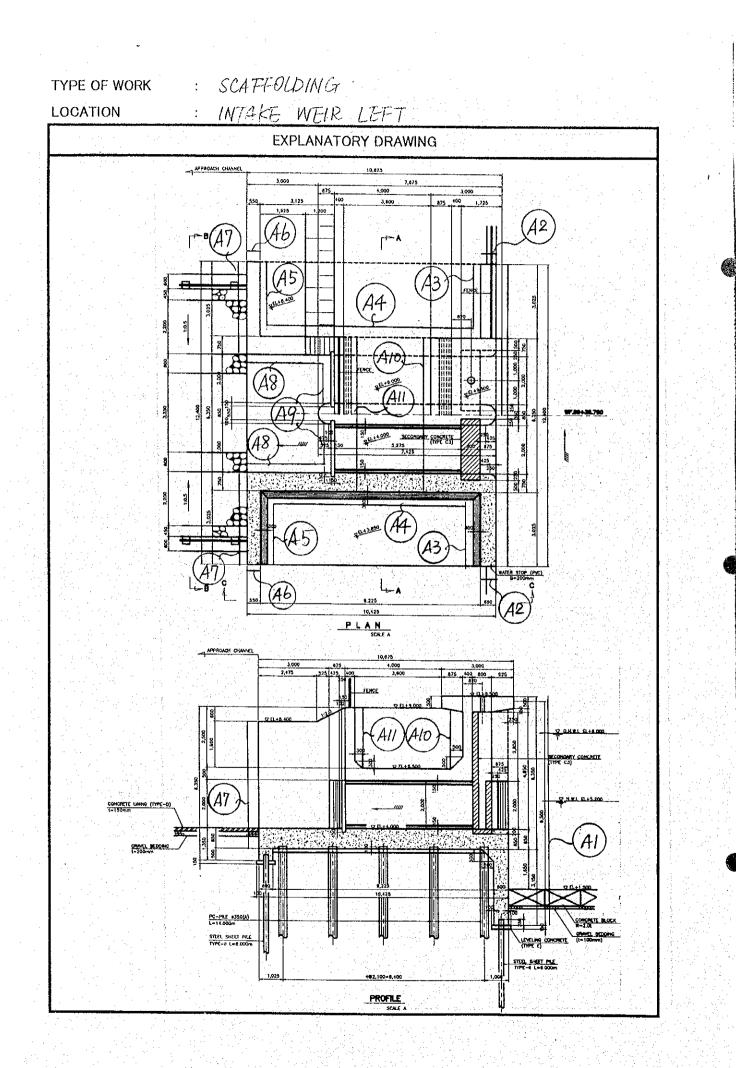
		$2 \times 2 = 4,950$	2×2 = 4,950	× 2	2 = 2,2 <i>SO</i>		x 2 x 2 = 3 280		× 2×2 = 2,000		70TAL = 19.555 19.555 mg							
HORM HOR SECUNDARI CONCRETE	INTAKE STRUCTURE (LEFT BANK) (H = 4.0m)	$A_{1} = 0.25 \times 4.95 \times 4.95 \times 10^{-10}$	人		+ + + + + + + + + + + + + + + + + + +	220 220 220 220 220 220 220 220 220 220	A5 = 0. 285 × 2.00	~ 2			<u></u>			(
	LOCATION					3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3	3		1 1 1 1 1 1 1 1 1 1	000°12	∑ £1+4.000	Ŧ	750					

OCATION : INTAKE WEIR LEF CALCULATION	l			Т
UALGULATION			+	RESULT
V () = () > MUQ DAW				
V1 = 6.35x (0.25°X8.85+1/2 x0.25x (0.15+6).109))=	- 14	255	
Vz= (0.25° × 0.85°+0.425×0.425×3.42:		:		
X (Z.85° + X3 × 0.109)= 0.4963 × 2	- 68-63 =	1	432	
U3 = (0.675 × 4.85° - 0.4963)×(4.85°+/3×	(0,109)=	13	572	
$4 = 0.8^{\infty} \times 2.5^{\infty} \times (4.85^{\circ} + 0.2^{\circ}) \times 2 =$		20	200	
<u>我们就是我的问题。</u> 我们就是我们的问题。				
15= { 1/2×(2.00+1.85°)×D.875+1/2×(D.375+	0.875)			
XD.500{ X 6.350=		1.2	680	
18= (2.000 x 2.000 - 0.15 X 0.15 X /2 X4) X 5.2	7(x)=	11	725	
		<u> </u>	$\cdot \alpha $	
	TOTAL	102	864	103.864 m
		14	<u>00</u> F	10.007
FORM OF TOP SLAB				
$A1(Cont) = 4.850 \times 10.925^{2} + 0.15^{2} =$		4	545	
		1	\mathcal{D}	
12 (Cont) = 2.500 × 0.8 × 2 = 3 (Cont) = 8.350 × 10.8752 + 0.152 =		 	427	
		 	$\frac{\omega_1}{\omega_1}$	
4 (Box) = 1.700 x5.275 XZ =		/7	935	
			720	
	TOTAL	(2)		
	<u> </u>		<u> // X</u>	
	androg y Digde andro Angles Charles and Angles Charles and			
		<u> </u> 		
	<u>an an a</u>	· · · ·		n an

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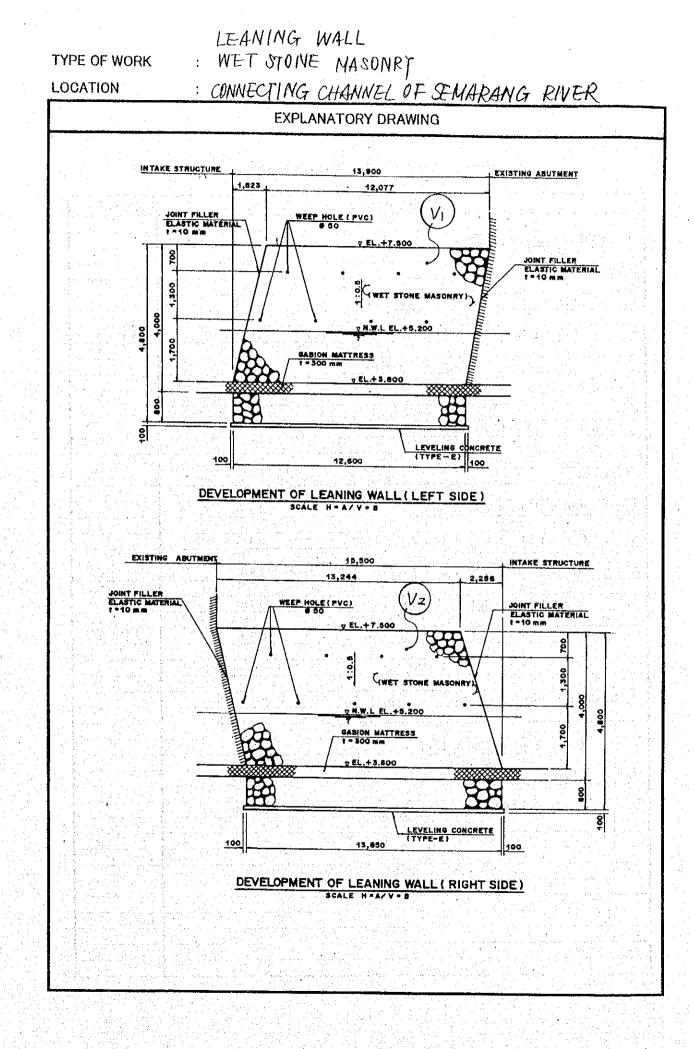
OCATION : INTAKE WEIR L CALCULATION			RESUL
A1 = 9.500 × 12.400 =	117	800	
Az= 9.000 x 0.65 x Z=		700	
Ay = 5.00 X 8.025 XZ=	(31)	250	
같다. 1948년 - 100년 - 2월 11일 - 12월 12일 (11)		200	
$A4 = \{5.5^{\infty} \times 2.35 + 2.5^{\infty} \times 3.6^{\infty} + 5.0^{\infty} + \frac{1}{2} \times (4.4^{\infty} + 5.0^{\infty}) \times 1.2^{\infty} + 4.4^{\infty} + \frac{1}{2} \times 1.4^{\infty} \times 1.2^{\infty} + \frac{1}{2} \times 1.4^{\infty} \times 1.2^{\infty} + \frac{1}{2} \times 1.4^{\infty} \times 1.2^{\infty} \times 1.2^{\infty} + \frac{1}{2} \times 1.2^{\infty} \times$	×0.400		
	x1.875}	())	
X		630	
$A_5 = 4.4^{\circ} \times 3.025 \times 2 =$	26	620	
A6= 5.25×0.60×Z=	6	300	
A7 = 5.25° XJ.775 XZ=	(39	633	
$\frac{11}{10} = 0.20 \text{ AU, } 10 \text{ AZ} =$	<u> </u>		in de la composition de la composition La composition de la c
A8 = (4.400 x 2.475+1/2 x (4.4+500	$) x/z^{\alpha \gamma} x z = UU$	060	
$\Lambda \rightarrow 600 \times 200 \times 2$		\sim	
$Aq = 5.000 \times 2.000 \times 2 =$	\mathcal{LO}	∞	
$A_{10} = 3.000 \times 6.35^{0} =$	19	250	
$A_{11} = 2.5^{00} \times 6.850 =$	15	875	
	TOTAL395.	923	325.92
		1	



TYPE OF WORK: PILE HEAD TREATMENTLOCATION: INTAKE STUCTURE: LEFT BANK

CALCULATION	RESULT
PC PILE DIA 350 (A) $n = 25$ PILES	
LENGTH OF DESIGN : $L_1 = 13.30 \text{ m/pile}$	
SPARE PILE LENGTH : $L_2 = 1.00 \text{ m}$	
n 1997 - Energia Angela, en 1997 - Seconda Angela, 1997 - Energia Angela, 1997 - Energia Angela, 1997 - Energia Energia Angela, energia energia energia energia energia de la composición de la composición de la composición d	26 -1
ADOPTED PILE LENGTH : $L = 13.30 + 1.00 = 14.30$ 15.00 m/pile	25 places
JOINTING	
n = 1 place/pile	0.5
	25 places
PILING	
NVALUE: N = 28 (Average)	
D = 13.30 m/pile	25 places
	pinces
CONCRETE FILLING (TYPE-C1) n = 25 PILES	
$V = \frac{\pi}{4} \times 0.22^2 \times 0.55$ = 0.021 m ³ /pile	25 places
SUSPENDED FORM	
A = $\frac{\pi}{4} \times 0.22^2$ = 0.038 m ³ /pile	25 places
	a de la compañía de l
REINFORCING BAR	
D 13 (W = 1.04 kg/m)	
$W_1 = 6 Bars x 0.45 x 1.04 = 2.808$	
$W_1 = 6 Bars x 0.45 x 1.04 = 2.808$	
$W_2 = 4 \text{ Bars x } 0.74 \text{ x } 1.04 = 3.078$	
$w_2 = 4 Dais x 0.74 x 1.04$	
TOTAL W = 5.886 kg.f/pile	25 places
	25 places
CUTTING PILE HEAD	
n negeri en de service de la construcción de la construcción de la construcción de la construcción de la constr En especie de la construcción de la	
Height of cutting : $h = 1.70 \text{ m/pile}$	
$V = \frac{\pi}{4} \times (0.35^2 - 0.40^2) \times 1.70 = 0.099 \text{ m}^3/\text{pile}$	25 places

YPE OF WORK	WET STONE MASONR)	r	
OCATION	: CONNECTING CHANNE		?TUFR
	CALCULATION		RESULT
A= (0.45 + 1.4	5) × 1/2 × 4.00 + 0.80 × 2.15	= 5.520 m ²	
$V_1 = (12, 60 +$	12.077) × 1/2 × 5.52	= 109	
V2 = (13,85	+ 13. 244) × 1/2 × 5.52	= 74. 779	
		TOTAL = 142.888	142.888 m ³
			<u> </u>
	n de la constante de la constan Constante de la constante de la		
	-		
		(1) A set of a set of a set of a set of a set of a set	
	(a) A set of the set of the first set of the set of		
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	and share from the formation of the second		
		an an an an ann an an an an an an an an	



TYPE OF WORK : LEANING WALL LOCATION :

: CONNECTING CHANNEL OF SEMARANG RIVER

	RESULT
= 2.961	
- 3.235	6.216 m ³
= 6.216	
<u>en der sins en ser der sins en s</u> der sins der statistische sins der sins en	
= 2.560	
= 0.470	
= 2.810	
= 0.470	
= 6310	6.310 m ²
0.510	<u> </u>
<u>a di anti di seconda di seconda di</u> seconda di seconda di seconda di seconda di seconda di seconda di seconda di Seconda di seconda di se	
= 2.450	
- 20,000	
= 33.190	
= 63.419	63.419 m ³
<u>a tanàna amin'ny dia kaominina dia kaominina dia kaominina dia kaominina dia kaominina dia kaominina dia kaomini</u> Jeografia	na serie de la composición de la compo Calencia de la composición de la composi
	= 3.255 $= 6.216$ $= 2.560$ $= 0.470$ $= 2.810$ $= 0.470$ $= 6.310$ $= 6.310$ $= 2.450$ $= 30.229$ $= 33.190$

TYPE OF WORK : LI LOCATION :

;

LEANING WALL

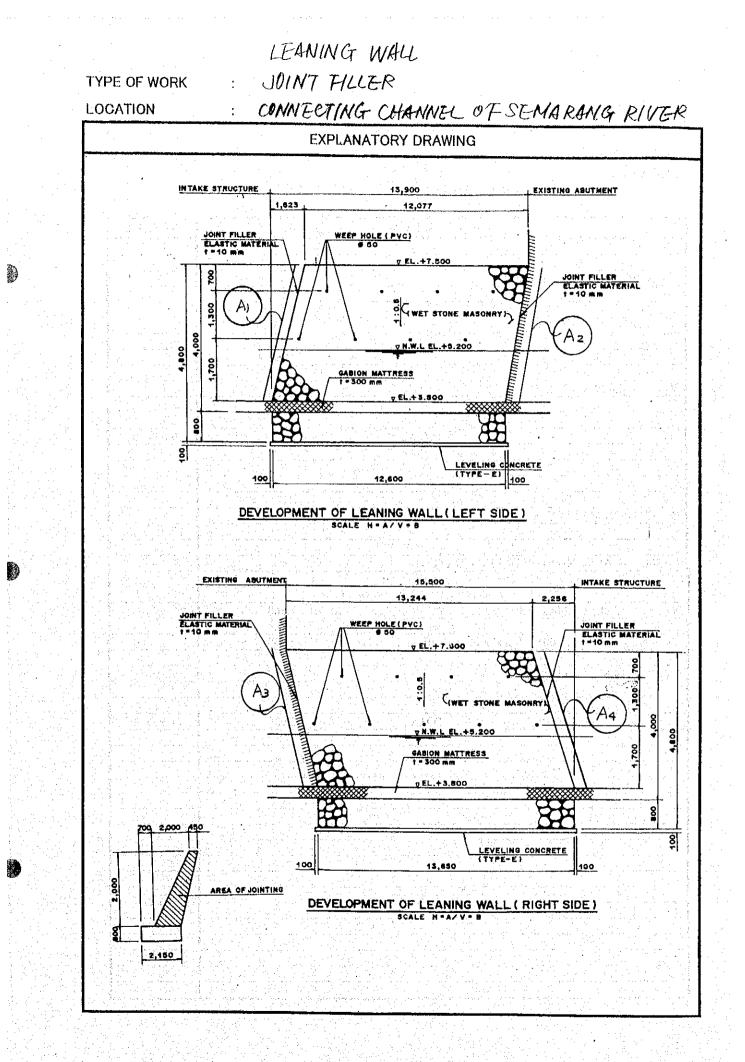
CONNECTING CHANNEL OF SEMARANG RIVER

		RESULT
<u> </u>	BACKFILL WITH SELECTED SOIL	e a construction M
A	$= 1.00 \times 1.20 = 1.200 \text{ m}^2$	
<u>A</u>	- 1.00 X 1.20	
V ₁	= 1.20 x 12.60 = 15.200	
V ₂	$= 1.20 \times 13.85 = 16.620$	
11 11		
	TOTAL = 31.740	31.740 m ³
	1월 1월 월 월일 19월 18일 - 11일 - 11일 <u>19월 19일</u> 19일 - 11일 - 11일 - 11일	
	BACKFILL SOIL	
À	$= 0.50 \times 2.40 = 1.200 \text{ m}^2$	
V ₁	= 12.00 + 12.077 = 14.492	
V	= 1.20 x 13.244 $=$ 15.893	
¥ 2	- 1,20 X 13,244	Contraction of the
	化二甲基乙酰基苯基乙酰基苯基乙酰基乙酰基 化乙酰基 网络小麦属植物 化乙酰氨酸 化乙酰乙酰氨基乙酰氨基乙酰氨基乙酰氨基乙酰氨	 Maging and second se second second se
	TOTAL = 30.385	30.385 m ³
	<u>TOTAL</u> = <u>30.385</u>	30.385 m ³
	TOTAL = 30.385	30.385 m ³
		<u>30,385 m³</u>
	TOTAL = 30.385 TOP CONCRETE	30.385 m ³
		30,385 m ³
	TOP CONCRETE	<u>30.385 m³</u>
		30.385 m ³
	TOP CONCRETE	30,385 m ³
	TOP CONCRETE CONCRETE (TYPE D)	30.385 m ³
	TOP CONCRETE CONCRETE (TYPE - D) = $0.60 \times 0.15 \times 12.077$ = 1.087 = $0.60 \times 0.15 \times 13.244$	30.385 m ³
• V ₁	TOP CONCRETE CONCRETE (TYPE - D) = $0.60 \times 0.15 \times 12.077$ = $0.60 \times 0.15 \times 13.244$ = 1.192	
• V ₁	TOP CONCRETE CONCRETE (TYPE - D) = $0.60 \times 0.15 \times 12.077$ = 1.087 = $0.60 \times 0.15 \times 13.244$	30.385 m ³
• V ₁	TOP CONCRETE CONCRETE (TYPE - D) = $0.60 \times 0.15 \times 12.077$ = $0.60 \times 0.15 \times 13.244$ = 1.192 TOTAL = 2.279	
• V ₁	TOP CONCRETE CONCRETE (TYPE - D) = $0.60 \times 0.15 \times 12.077$ = $0.60 \times 0.15 \times 13.244$ = 1.192 TOTAL = 2.279	
• V ₁	TOP CONCRETE CONCRETE (TYPE - D) = $0.60 \times 0.15 \times 12.077$ = $0.60 \times 0.15 \times 13.244$ = 1.192 TOTAL = 2.279	
• V ₁	TOP CONCRETE CONCRETE (TYPE - D) = $0.60 \times 0.15 \times 12.077$ = 1.087 = $0.60 \times 0.15 \times 13.244$ = 1.192 TOTAL TOTAL FORM FOR TOP CONCRETE (H < 4.0 m)	
• V ₁ V ₂	TOP CONCRETE CONCRETE (TYPE - D) = $0.60 \times 0.15 \times 12.077$ = 1.087 = $0.60 \times 0.15 \times 13.244$ = 1.192 TOTAL = 2.279 FORM FOR TOP CONCRETE (H < 4.0 m)	
• V ₁ V ₂	TOP CONCRETE CONCRETE (TYPE - D) = $0.60 \times 0.15 \times 12.077$ = 1.087 = $0.60 \times 0.15 \times 13.244$ = 1.192 TOTAL TOTAL FORM FOR TOP CONCRETE (H < 4.0 m)	
• V ₁ V ₂ •	TOP CONCRETE CONCRETE (TYPE - D) = $0.60 \times 0.15 \times 12.077$ = 1.087 = $0.60 \times 0.15 \times 13.244$ = 1.192 TOTAL = 2.279 FORM FOR TOP CONCRETE (H < 4.0 m)	2.279 m ³
• V ₁ V ₂ •	TOP CONCRETE CONCRETE (TYPE - D) = $0.60 \times 0.15 \times 12.077$ = 1.087 = $0.60 \times 0.15 \times 13.244$ = 1.192 TOTAL = 2.279 FORM FOR TOP CONCRETE (H < 4.0 m)	

TYPE OF WORK : LEANING WALL LOCATION :

: CONNECTING CHANNEL OF SEMARANG RIVER

	CALCULATION		RESULT
U WEEP HOLE	<u>na konstruktur (h. 1997)</u> 1995 - Andrea Statistik, serietak		
• PVC PIPE Ø 50			
		en e	
Upper Section			
$L_1 = 8 \text{ pipes x } 0.80$		- (100	
$L_1 = 8$ pipes x 0.80	and a star of the second star and the second star and the second star and the second star and the second star The second star and the second s	= 6.400	
Lower Section			
$L_2 = 9 \text{ pipes x } 1.10$		= 9.900	
	TOTAT	- 16 000	16.000
	TOTAL	= 16.300	16.300 m
• FILTER CLOTH			
A - 17 1 4		3	
A = 17 sheets x (0,	20 x 2 + 0.165 x 2 + 0.15		10.155
<u>in the second stread of the second streads of the</u>	<u>nde seudo do constante constante do</u> este A line Alexando este constante por est	13.165	13.165 m ²
SCAFFOLDING			
A 4470 (10 KO)	10 000		
$A_1 = 4.472 \text{ x} (12.600)$	$(1 + 12.077) \times \frac{1}{2}$	= 55.178	
$A_2 = 4.472 \times (13.24)$	4 + 13.85) x ¹ / ₂	= 60.582	
	TOTAL	= 115.760	115.760 m ²
D JOINT FILTER		<u>an an ann an Aonaichtean a</u> An an Aonaichtean an A	<u>, status supervises sono</u> San se se la segura de la seconda
t = 10, ELASTIC MATE	RIAL		
$A_1 = (0.45 + 1.45)$	(14 × 2.00 × 1.000		
$n_1 = (0.45 \pm 1.45)$	(REVISING)	= 2.322	
$A_2 = (0.45 + 1.45)$	< ½ x 2.00 x 1.404	= 2.668	<u>- 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1</u>
			X 與時期提出于
$A_3 = (0.45 + 1.45)$	<u>x ½ x 2.00 x 1.340</u>	= 2,546	a sha amala a sa ga a a mara ta ta ara
$A_4 = (0.45 + 1.45)$	<u>x ½ x 2.00 x 1.577</u>	= 2.996	
		2.770	
	TOTAL	= 10.532	10.532 m ²
	•		

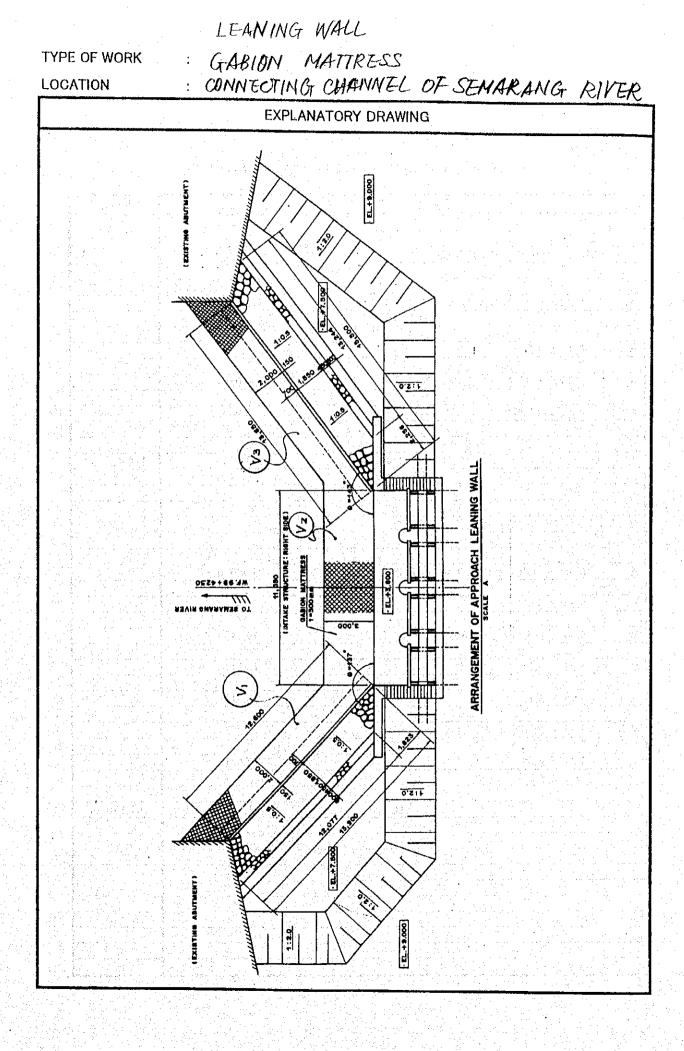


TYPE OF WORK :LEANING WALLLOCATION :::CONNECTING CHANNEL OF SEMARANG RIVER

CAL	CULATION		RESULT
	an a tha a th		
			· · · · · · · · · · · · · · · · · · ·
RUBBLE STONE FILLING			
	<u> </u>		
$A = \frac{1}{2} \times 0.30 \times 0.15$		$= 0.023 \text{ m}^2$	
$A = \frac{1}{2} \times 0.30 \times 0.15$		- 0.023 m	
$V_1 = 0.023 \times 12.60$		= 0.290	
$V_2 = 0.023 \times 13.85$		= 0.319	
	a she kara ya sa		
	TOTAL	= 0.609	0.609 m ³
an a			
anto ana amin'ny fivondrona dia mampiana amin'ny fivondrona dia mampiana amin'ny fivondrona dia mampiana dia m Ny INSEE dia mampiana amin'ny fivondrona dia mampiana dia mampiana dia mampiana dia mampiana dia mampiana dia ma			
(c) A set of the se			
GABION MATTRESS	and and a second se Second second	en ante en la construcción de la co Construcción de la construcción de l	
GADION MATTICOD			
in an	an a		and the start of the
t = 300 (GALVANIZED AND PVC C	OATED)		
	ta presidente de la composición de la c		that we had the
	and a star of the		
$V_1 = 12.60 \times 2.00 \times 0.30$		= 7.560	
	0.00	- 7.046	
$V_2 = (11.55 + 12.00) \times \frac{1}{2} \times 2.00$	<u>x 0.30</u>	= 7.065	
$V_3 = 13.85 \times 2.00 \times 0.30$		= 8.310	
	and a second strength of the second strength		
	TOTAL	= 22.935	22.935 m ³
		- Ali ang	
oth e chuir suit is freith Bolto, Alba			
	and an and a parameters. The second se	n provinske servinske provinske stationer for de servinske stationer. Referense servinske stationer stationer for de servinske stationer stationer stationer stationer stationer stati	
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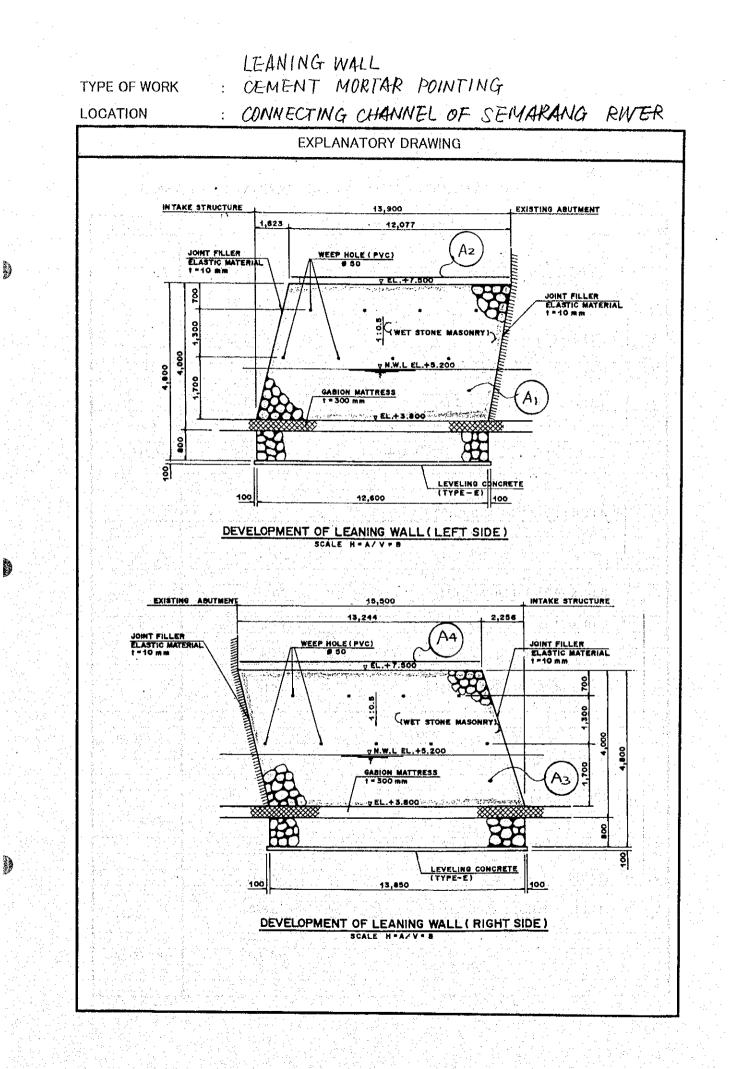
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TYPE OF WORK : LEANING WALL LOCATION :

:

CONNECTING CHANNEL OF SEMARANG RIVER

CALCULATION		RESULT
		an a
CEMENT MORTAR POINTING		
$A_1 = (12.60 + 12.077) \times \frac{1}{2} \times 3.70 \times 1.118 =$	51.039	
(REVISING)		
$A_2 = 0.45 \times 12.077$	5.425	
	- J.42J	
$A_3 = (13.85 + 13.244) \times \frac{1}{2} \times 3.70 \times 1.118 =$	56.039	
$A_4 = 0.45 \times 13.244$	5.960	
<u>그는 이 가지 않는 것 같아요. 그는 것은 것은 이 가지 않는 것 같은 것 않</u> 는 것 같이 있는 것 같이 있다. 그는 것 같은 것은 것 같아요. 이 같은 것 같은 것은 것 같은 것 같은 것 같은 것 같이 있다.	<u>na ang ang ang ang ang ang ang ang ang a</u>	and the second
TOTAL =	= 118.473	118.473 m ²
D WET STONE MACONEY		
WET STONE MASONRY	<u>n efficiente en en en entre de trans</u> 1997 - La Carlo Maria de La Carlo de La	and a second
A = $(0.45 + 1.40) \times \frac{1}{2} \times 4.40 + (0.85 \times 2.20)$ =	= 5.940 m ²	
$V_1 = 5.94 \times 4.65$	= 27.621	
$V_2 = 5.94 \times (15.119 + 11.663) \times \frac{1}{2} =$	= 79.543	
$V_3 = 5.94 \times 3.15$	- 18.711	
$V_4 = 5.94 \times 3.15$	10 711	
<u>v₄ − 3.94 x 3.13</u>	<u>= 18.711</u>	
$V_5 = 5.94 \times (0.589 + 4.045) \times \frac{1}{2}$	= 13.763	
$V_6 = 5.94 \times 4.65$	= 27.621	
가 있는 것은 가지 않는 것은 것을 가지 않는 것을 가지 않는 것을 가지 않는 것을 가지 않는 것을 했다. 기가 많은 것은	<u>en en la parte de la compactó de la comp</u> Compactó de la compactó de la compact	a da serie de la companya de la comp Nota de la companya d
TOTAL =	= 191.910	191.190 m ³
	<u>na se se respectatores se s</u> Se se se respectatores se se se	



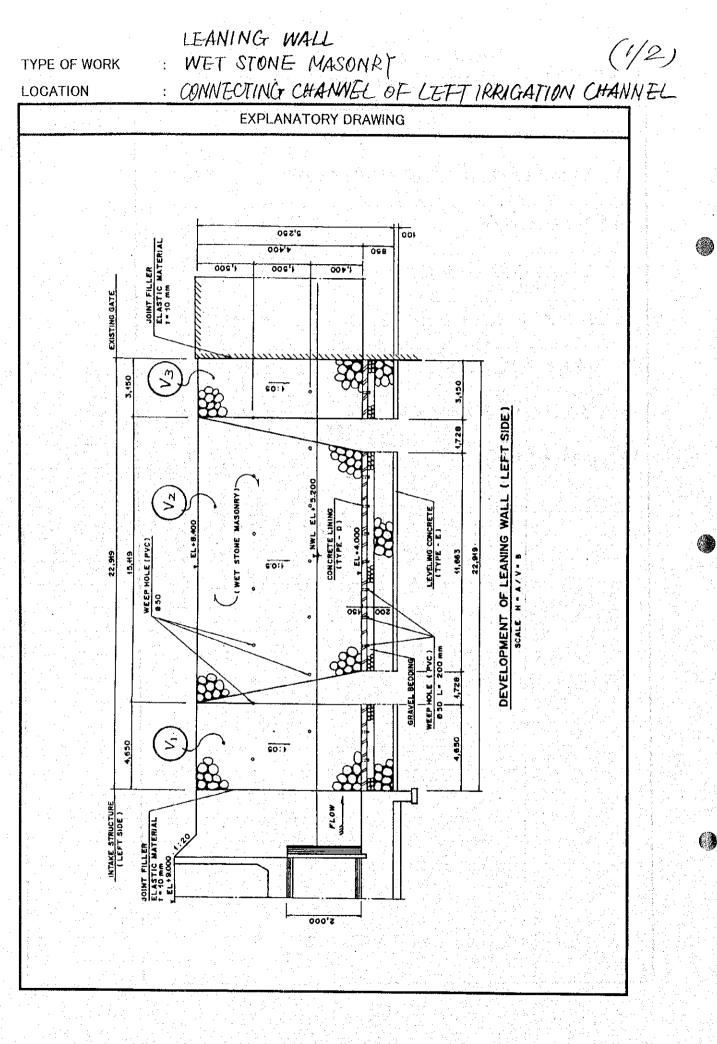
TYPE OF WORK:LEANING WALLLOCATION::::CONNECTING CHANNEL OF LEFT IRRIGATION CHANNEL

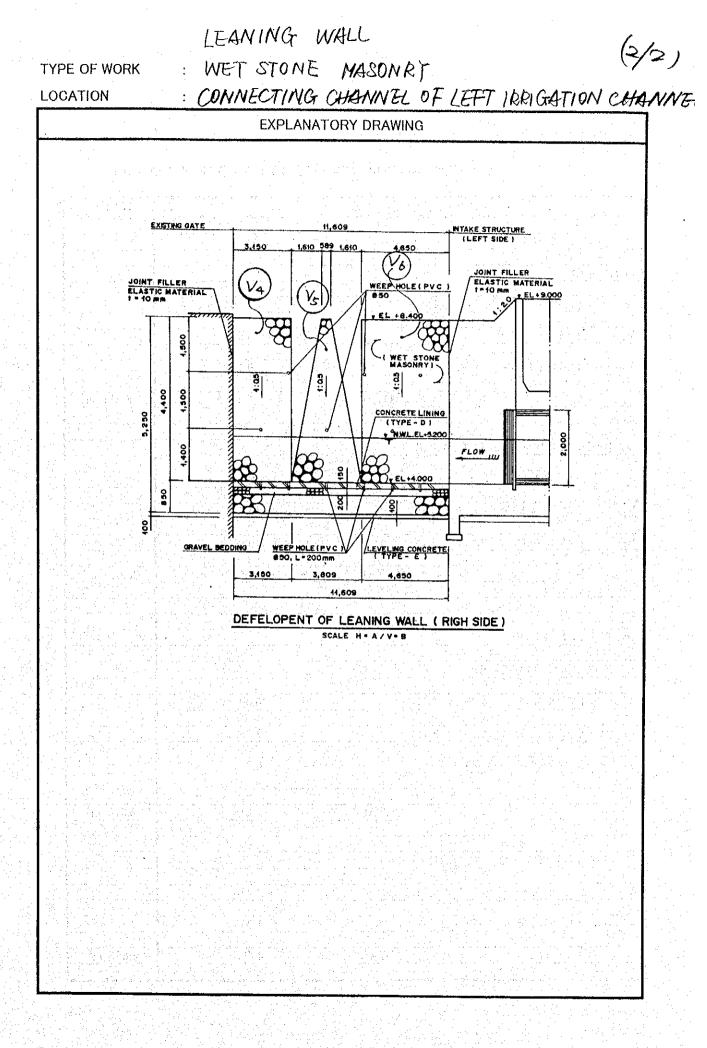
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$A = (0.45 + 1.90) \times \frac{1}{2} \times 4.40 + (0.85 \times 2.20) = 5.940 \text{ m}^2$ $V_1 = 5.94 \times 4.65 = 27.62$ $V_2 = 5.94 \times (15.119 + 11.663) \times \frac{1}{2} = 79.543$ $V_3 = 5.94 \times 3.15 = 18.711$ $V_4 = 5.94 \times (0.589 + 4.045) \times \frac{1}{2} = 13.763$ $V_6 = 5.94 \times 4.65 = 27.62$ $T07AL = 191.910 \qquad 191.910 \text{ m}$	CALC	<u>CHANNEL OF 1</u> ULATION			ULT
$V_{1} = 5.94 \times 4.65 = 27.62$ $V_{2} = 5.94 \times (.15.119 + 1).663) \times \frac{1}{2} = 79.543$ $V_{3} = 5.94 \times 3.15 = 18.711$ $V_{4} = 5.94 \times 3.15 = 18.711$ $V_{5} = 5.94 \times (.0.589 + 4.045) \times \frac{1}{2} = 13.763$ $V_{6} = 5.94 \times 4.65 = 27.621$ $ToTAL = 191.910 \qquad 191.910$				· · · · · · · · · · · · · · · · · · ·	
$V_{2} = 5.94 \times (15.119 + 11.663) \times \frac{1}{2} = 79.543$ $V_{3} = 5.94 \times 3.15 = 18.711$ $V_{4} = 5.94 \times (0.589 + 4.045) \times \frac{1}{2} = 13.763$ $V_{6} = 5.94 \times 4.65 = 27.621$ $ToTAL = 191.910 \qquad 191.910$	$A = (0.45 + 1.40) \times \frac{1}{2} \times 4.40$	+(0.85 × 2.20)	= 5.940 r	n ²	
$V_{2} = 5.94 \times (15.119 + 11.663) \times \frac{1}{2} = 79.543$ $V_{3} = 5.94 \times 3.15 = 18.711$ $V_{4} = 5.94 \times (0.589 + 4.045) \times \frac{1}{2} = 13.763$ $V_{6} = 5.94 \times 4.65 = 27.621$ $ToTAL = 191.910 \qquad 191.910$	1/1 = 594 × 815			· · · · · · · · · · · · · · · · · · ·	· · · · · ·
$\frac{V_{3} = 5.94 \times 3.15}{V_{4} = 5.94 \times 3.15} = 18.711$ $\frac{V_{5} = 5.94 \times (0.589 + 4.045) \times /2}{13.763} = 13.763$ $\frac{V_{6} = 5.94 \times 4.65}{707AL} = 191.910$ (91.910 m	<u> </u>		<u> </u>		
$V_{4} = 5.94 \times 3.15 = 18.711$ $V_{5} = 5.94 \times (0.589 + 4.045) \times 1/2 = 13.763$ $V_{6} = 5.94 \times 4.65 = 27.621$ $ToTAL = 191.910 \qquad 191.910 \qquad m$	$V_2 = 5.94 \times (15.119 + 11.663) \times$	12	= 79.543		
$V_{4} = 5.94 \times 3.15 = 18.711$ $V_{5} = 5.94 \times (0.589 + 4.045) \times 1/2 = 13.763$ $V_{6} = 5.94 \times 4.65 = 27.621$ $ToTAL = 191.910 \qquad 191.910 \qquad m$					
$\frac{\sqrt{5} = 5.94 \times (0.589 + 4.045) \times \frac{1}{2}}{\sqrt{6} = 5.94 \times 4.65} = 13.763$ $\frac{13.763}{2 = 27.62}$ $\frac{707AL}{2} = 191.910$ 191.910 m	$V_3 = 5.94 \times 3.15$		= 18.711		
$\frac{V_{5} = 5.94 \times (0.589 + 4.045) \times 1/2}{V_{6} = 5.94 \times 4.65} = 13.763$ $\frac{V_{6} = 5.94 \times 4.65}{T0TAL} = 191.910$ 191.910 m	Va - 100 - 215		(0 ml)		<u> </u>
$V_6 = 5.94 \times 4.65$ $> 27.62)$ TOTAL = 191.910 191.910 m	<u> </u>		<u>= /8, (/)</u>		
$V_6 = 5.94 \times 4.65$ $> 27.62)$ TOTAL = 191.910 191.910 m	Vs = 5.94 × (0.589 + 4.04	5) × 1/2	= 13.763		<u></u>
TOTAL = 191.910 191.910 m					
	V6 = 5.94 × 4.65		= 27.62)		
					1 12
		ToTA1 -			<u> </u>
- 「金山」には、「秋山」・「山」に、「「大大文大山」に加加した」「緑木大山」に「山山」に「山山」に「山山」に、「山山」に、「山山」に、「山山」に、「山山」に、「		<u>10162 -</u>	<u>111. 110</u>	/4/. 4/(<u>)</u> m
- 「金山」には、「秋山」・「山」に、「今天大学と「山山」指した」「緑玉子」に、「山山」に「山山」に、「山山」に、「山」に、「山」に、小山」に、「山」に、小山」に					
- 「金山」には、「秋山」・「山」に、「今天大学と「山山」指した」「緑玉子」に、「山山」に「山山」に、「山山」に、「山」に、「山」に、小山」に、「山」に、小山」に					
- 「金山」には、「秋山」・「山」に、「「大大文大山」に加加した」「緑木大山」に「山山」に「山山」に「山山」に、「山山」に、「山山」に、「山山」に、「山山」に、「					
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	[1] 같은 사람들의 비행이가 가지 않는 것을 가지 않는다. 1914년 - 1915년 - 1917년 -			<u></u>	1999 (1997)





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TYPE OF WORK LOCATION

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•

CONNECTING CHANNEL OF LEFT IRRIGATION CHANNEL

LEANING WALL

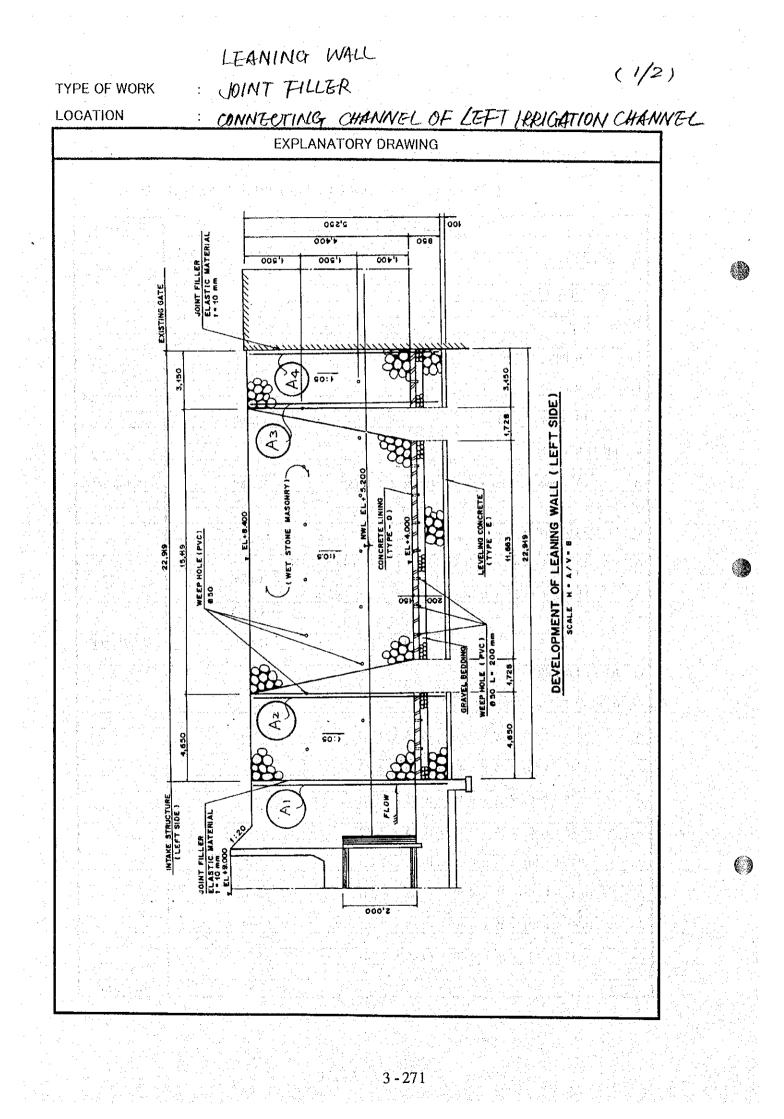
CAL	CULATION	RESULT
D BACKFILL SOIL		
(TYPE – E)		
$A = (0.50 \times 2.35) - 0.08 \times 0.15$	= 1.163	m²
$V_1 = 1.163 \times 22.919$	= 26.655	i <u>an l'international de la constante de la cons</u> tante de la constante de la constante de la constante de la const Constante de la constante de la
$V_2 = 1.163 \times 8.389$	= 9.756	
n de la companya de En esta de la companya	TOTAL = 36.411	36.411 m ²
BACKFILL WITH SELECTED SOI	La ser la se La ser la ser	
$A = 0.85 \times 1.00$	= 0.850	m ²
$V_1 = 0.85 \times (4.65 + 11.663 + 3.1)$	5) = 16.544	
$V_2 = 0.85 \times (4.65 + 4.045 + 3.1)$	5) = 10.068	<u>an an an Anna Anna Anna Anna</u> Tha Anna Anna Anna Anna Anna Anna Anna An
	TOTAL = 26.612	26.612 m ²
□ TOP CONCRETE		
• CONCRETE (TYPE – E)		
$A = 0.60 \times 0.15$	= 0.09 m ⁻	
$V_1 = 0.09 x (4.65 + 15.119 + 3.1)$	5) = 2.063	
$V_2 = 0.09 \times (4.65 + 0.589 + 3.15)$) = 0.755	
		an an Argentin an Argentin an Argentina an Argentina an Argentina an Argentina an Argentina an Argentina an Arg
	TOTAL = 2.818	2 010 -3
	<u>TOTAL</u> = 2.818	2.818 m ³
• FORM FOR TOP CONCRETE (H <	<u>4.0 m)</u>	
		e ale Taerde - Eren Arra e Leo. Teo Arra e Eren
$A_1 = 0.15 \times (4.65 + 15.119 + 3.1)$	5) = 3.438	
	5) = 3.438	
$A_1 = 0.15 \times (4.65 + 15.119 + 3.1)$	5) = 3.438	4.696 m ²

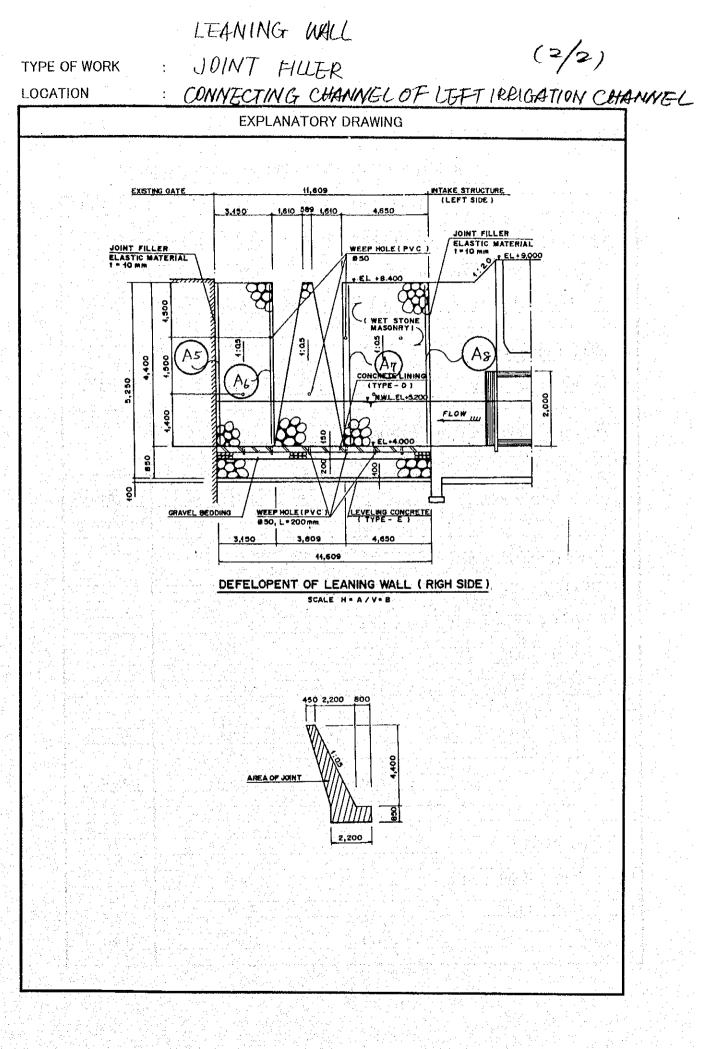
TYPE OF WORK : LEANING WALL LOCATION :

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: CONNECTING CHANNEL OF LEFT IRRIGATION CHANNEL

CALCULATION		RESULT
© WEEP HOLE		
• PVC PIPE Ø 50		
Upper Section		
$L_1 = 10 \text{ pipes x } 0.90 =$	9.000	
Lower Section		
$L_2 = 10 \text{ pipes x } 1.25 =$	12.500	
• FILTER CLOTH		
A = 20 sheets x $(0.20 \times 2 + 0.165 \times 2 + 0.15)^2$ =	15.488	15.488 m ²
		201100 m
$A_1 = 22.919 + (4.65 + 11.663 + 3.15) \times \frac{1}{2} \times 4.40 \times 1.118$		
	104.243	
$A_2 = 11.845 + (3.15 + 0.589 + 4.65) \times \frac{1}{2} \times 4.40 \times 1.118$		
	49.768	
TOTÁL =	154.011	154.011 m ²
		134,011 III
JOINT FILTER		
t = 10, ELASTIC MATERIAL		
$A_1 = (0.45 + 1.40) \times \frac{1}{2} \times 4.40 + (0.85 \times 2.20) =$	5.940	
$A_2 = (0.45 + 1.40) \times \frac{1}{2} \times 4.40 + (0.85 \times 2.20) =$	5.940	
$A_3 = (0.45 + 1.40) \times \frac{1}{2} \times 4.40 + (0.85 \times 2.20) =$	5.940	
<u>Λ3</u> (0. τ J + 1. τ U) Λ /2 Λ -τ. τ U Τ (0.0J Λ 2.20) -	<u></u>	
$A_4 = (0.45 + 1.40) \times \frac{1}{2} \times 4.40 + (0.85 \times 2.20) =$	5.940	
$A_5 = (0.45 + 1.40) \times \frac{1}{2} \times 4.40 + (0.85 \times 2.20) =$	5.940	
$A_6 = (0.45 + 1.40) \times \frac{1}{2} \times 4.40 + (0.85 \times 2.20) =$	5.940	
$A_7 = (0.45 + 1.40) \times \frac{1}{2} \times 4.40 + (0.85 \times 2.20) =$	5.940	
$A_{*} = (0.45 \pm 1.40) \times \frac{16}{3} \times \frac{440}{40} \pm (0.85 \times 2.20) =$	5.040	
$A_{3} = (0.45 + 1.40) \times \frac{1}{2} \times 4.40 + (0.85 \times 2.20) =$	5.940	
TOTAL =	47.520	47.520 m ²





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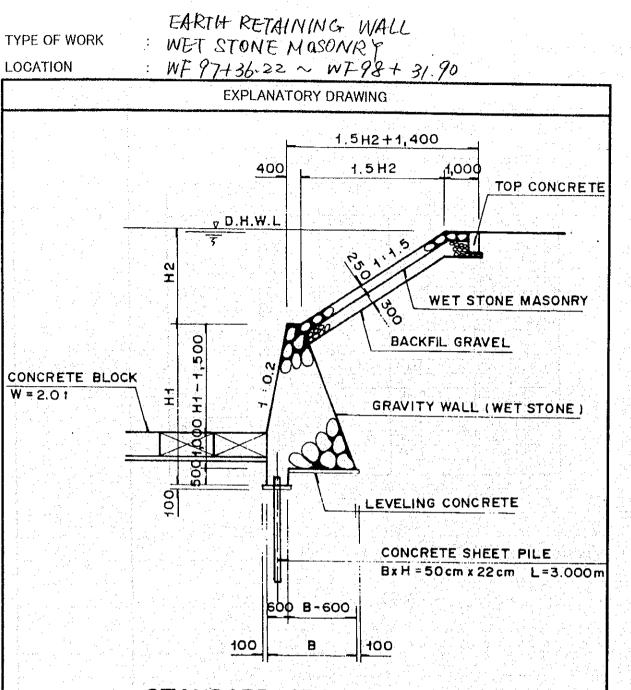
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TYPE OF WORK : LEANING WALL LOCATION : : CONNECTING CH

CONNECTING CHANNEL OF LEFT IRRIGATION CHANNEL

CALCULATION		RESULT
CHANNEL BED PROTECTION OF LEFT IRRIGAT	TION CHANNEL	
CONCRETE LINING (TYPE – D)		
$V_1 = 0.15 \times 3.25 \times (4.65 + 7.854 + 3.15) =$	7.631	7.361 m ²
$v_1 = 0.15 \times 5.25 \times (4.05 + 7.054 + 5.15)$	7.051	7.501
	Ale galera di Arraela e e	
GRAVEL BEDDING		
	10.176	10.126
$V_2 = 0.20 \times 3.25 \times (4.65 + 7.854 + 3.15) =$	- 10.175	10.175 m ³
• WEEP HOLE (PVC PIPE Ø 50, L = 200 mm)		
n = 16 points	la ang sang sang sang sang sang sang sang	
$L = 16 \text{ points } \times 0.20 =$	= 3.200	3.200 m
CEMENT MORTAR POINTING	Romente en la reconstruction de la construcción de la construcción de la construcción de la construcción de la Transmissione de la construcción de	and a second second Second second
$A_1 = 4.65 \times 4.40 \times 1.118$	= 22.874	
(REVISING)		
	= 65.873	
$A_3 = 3.15 \times 4.40 \times 1.118$	= 15.495	
$\frac{A_4}{A_5} = \frac{22.919 \times 0.45}{3.15 \times 4.40 \times 1.118}$	= 10.314 = 15.495	
$A_{6} = (0.589 + 4.045) \times \frac{1}{2} \times 4.40 \times 1.118$	= 11.389	
	= 22.874	
$A_8 = 3.15 \times 0.45$	= 1.418	
$A_9 = 0.589 \times 0.45$	= 0.265	-
$A_{10} = 4.65 \times 0.45$	= 2.093	
TOTAL	= 168.099	168.099 m ²
	ga af af a golf freide a trainig sa Talainn an trainig sa	
GUARD FENCE	<u>e de la construcción de la constru La construcción de la construcción d</u>	ere de la sere de la s La sere de la
$L_1 = (4.65 + 16.061 + 3.15)$	= 23.861	
$L_2 = (4.65 + 3.15)$	= 7.800	
Τ ΩΤΑΙ		21.661 m
TOTAL	= 31.661	<u>31.661 m</u>

LOCATION	<u> </u>	<u>F 97 + 36.</u>	$32^{\circ} \sim$	<u>wh 98 -</u>	+ 31.90	
			JLATION			RESUL
(WET S	STONE MASO	VRI	1 - 2 			·····
	EARTH RETAIN	ING WALL (W	ET STONE N	ASONRY)	· ·
			t det e			
	STATION NO.	PARTIAL DISTANCE (m)			WALL VOLUME (m ³) –	
	WF97R+36.320		6.400	<u>(117</u>	<u>(m)</u>	
	WF97R+41.320	7.099	6.400	6.400	45.434	
	WF98R+1.400	12.045	6.400	6.400	77.088	
	WF98R+11.900 WF98R+11.900	10.540	8.498 6.478	7.449	78.512	
	WF98R+21.900	10.040	8.300	7,389	74.183	
	WF98R+21.900		6.400	7.350		
	WF98R+31.900	10.040	6.400	6,400	64.256	
	WF98L+31.900	<u>tan ing kapang</u>	6 400	TOTAL	339.473	
	WF98L+21.900	10.000	6.400 8.300	<u>3.200</u> 7.350	73.500	
	WF98L+21.900		6.400	7.350	/0.000	
 	WF98L+11.900	10.000	8.498	7.449	74.490	
	WF98L+11.900 WF98L+1.400	10.500	<u>6.478</u> 6.400	7.488	07.007	
	WF97L+41.320	12.000	6.400	6.439 6.400	67.607 76.800	
	WF97L+36.320	7.071	6.400	6.400	45.254	
		an a		TOTAL	337.651 -	
(Đeduc	tion for PC	Sheet P	ile)			
		gage of the s				
	tion for PC (0.22×0.25	gage of the s			- <i>z</i> .737	
<u>Vı = -</u>	(0.22 × 0.25	<u>, × 49.764</u>	->		- 2.737	
<u>Vı = -</u>		<u>, × 49.764</u>	->			
<u>Vı = -</u>	(0.22 × 0.25	<u>, × 49.764</u>	->		- 2.737	
<u>Vı = -</u>	(0.22 × 0.25	<u>, × 49.764</u>	->		- 2.737	
<u>Vı = -</u>	(0.22 × 0.25	<u>, × 49.764</u>	->		- <i>z</i> .737 2.726	
<u>Vı = -</u>	(0.22 × 0.25	<u>, × 49.764</u>	->		- 2.737	67).661
<u>Vı = -</u>	(0.22 × 0.25	<u>, × 49.764</u>	->		- <i>z</i> .737 2.726	67).661
<u>Vı = -</u>	(0.22 × 0.25	<u>, × 49.764</u>	->		- <i>z</i> .737 2.726	677).661
<u>Vı = -</u>	(0.22 × 0.25	<u>, × 49.764</u>	->		- <i>z</i> .737 2.726	67).661
<u>Vı = -</u>	(0.22 × 0.25	<u>, × 49.764</u>	->		- <i>z</i> .737 2.726	67).661
<u>Vı = -</u>	(0.22 × 0.25	<u>, × 49.764</u>	->		- <i>z</i> .737 2.726	67).661
<u>Vı = -</u>	(0.22 × 0.25	<u>, × 49.764</u>	->		- <i>z.</i> 737 2.726	677).661
<u>Vı = -</u>	(0.22 × 0.25	<u>, × 49.764</u>	->		- <i>z.</i> 737 2.726	677).661
<u>Vı = -</u>	(0.22 × 0.25	<u>, × 49.764</u>	->		- <i>z.</i> 737 2.726	677).661
<u>Vı = -</u>	(0.22 × 0.25	<u>, × 49.764</u>	->		- <i>z.</i> 737 2.726	677).661
<u>Vı = -</u>	(0.22 × 0.25	<u>, × 49.764</u>	->		- <i>z.</i> 737 2.726	677).66)
<u>Vı = -</u>	(0.22 × 0.25	<u>, × 49.764</u>	->		- <i>z.</i> 737 2.726	677).661
<u>Vı = -</u>	(0.22 × 0.25	<u>, × 49.764</u>	->		- <i>z.</i> 737 2.726	677).661



STANDARD CROSS SECTION

SCALE A

TABLE OF DIMENSION

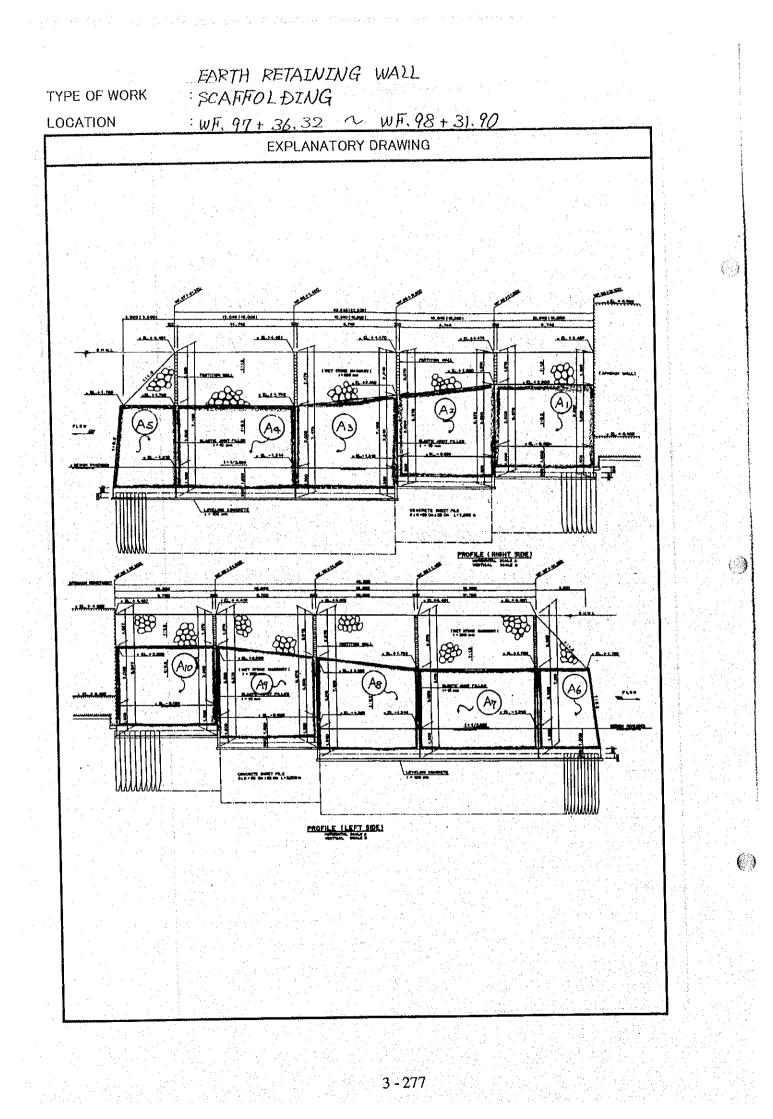
STATION	H H (m)	H2 (m)	B (m)	REMARK
WF. 97+41,320	4,500	2,669	2,500	
WF. 98+1, 400	4,500	2,675	2,500	
WF. 98+11,900	5,110	2,070	3,000	DOWN STREAM
WF. 98+11,900	4,500	2,070	2,500	UPSTREAM
WF. 98+21,900	5,000	1,579	3,000	DOWN STREAM
WF, 98 + 21,900	4,500	1,579	2,500	UPSTREAM
WF. 98+31,900	4,500	1,587	2,500	

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YPE OF WORK : SCAFFOLDING)	
OCATION : $wF. 97 + 36.32 \sim wF.9$	<u>8 + 31.90</u>	
CALCULATION		RESL
A) = 4.00 × 10.04 × 2	= 80.320	
$A_2 = (4.50 + 4.00) \times \frac{1}{2} \times 10.40 \times 2$	= 88.400	
A3 = (4.61 + 4.00) × 1/2 × 10.54 × 2	= 90.749	
14 = 4.00 × 12.045 × 2	= 96.360	
5= 4.00 × 7.098 × 2	= 56.784	
	<u> </u>	
46 = 4.00 × 7.07 × 2	= 56.560	
	50.00	
17 = 4.00 × 12.00 × 2	= 96.000	
	70,000	
18 = (4.00 + 4.610) × 1/2 × 10.50 × 2	= 90 405	
<u> </u>		an <mark>Christean a</mark> Tail an Anna Anna Anna Anna Anna Anna Anna
19 = (4.00 + 4.50) × 1/2 × 10.00 × 2	= 25 m	
410 = 4.00 × 10.00 × 2	= 80,000	
	<u> </u>	
70TA]_	= 820.578	820.57
<i>ivia</i> L	0~0.010	020.37
	<u>a an fairte a tar i far i s</u>	-
i 1999 - Canada Santa Santa Kanada sa kata s	n an	
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			<u>2 ~ W/H.</u> _ATION	98 + 31.90	· .	
		ALGUL	ATION			RES
(TYPE-E)				·	· · ·	· · ·
LEVELING CO	NORETE (TYPE	F)			• • • • • •	
					<u></u>	
STATION	PARTIAL	AREA		LEVELING CONCRE	TE	
NO. WF97R+36.320	DISTANCE (m)	(m ⁻) 0.270	(m ²)	VOLUME (m ³)	· · · · · · · · · · · · · · · · · · ·	
WF97R+41.320			0.270	1	917	
WF98R+1.400	12.045		0.270	3.	252	
WF98R+11.900 WF98R+11.900		0.320	0.295	3.	109	
WF98R+21.900	10,040	0.320	0.295	2	962	
WF98R+21.900		0.270	0.295			
WF98R+31.900	10.040	0.270	0.270 TOTAL		711	
WF98L+31.900	na series de la composición de la compo Na composición de la c	0.270	0.135	13.	<u>aoi</u>	
WF98L+21.900	10.000	0.320	0.295	2.	950	
WF98L+21.900 WF98L+11.900	10.000	0.270	0.295		ute de par	i Bulandi
WF98L+11.900	10.000	0.320	0.295	2.	950	
WF98L+1.400	10.500	0.270	0.270	2.	835	· · · · ·
WF97L+41.320 WF97L+36.320	12.000 7.071	0.270	0.270		240	
	7.071	0.270	0.270	1.	909	
(Deduction	for PC S		<u> </u>	13.	384	
		hee+	Pile)	<u>13.</u> =/.09<		
		hee+	Pile)			
$V_1 = -(0.22)$	2 × 0.10 ×	heet 49.7	Pile) 64)	= -1.09<		
	2 × 0.10 ×	heet 49.7	Pile) 64)	= -/.09<		
$V_1 = -(0.22)$	2 × 0.10 ×	heet 49.7	Pile) 64)	= -1.09<		
$V_1 = -(0.22)$	2 × 0.10 ×	heet 49.7	Pile) 64) 71)	= -1.09< = -1.091	5	
$V_1 = -(0.22)$	2 × 0.10 ×	heet 49.7	Pile) 64) 71)	= -1.09<	5	25.64
$V_1 = -(0.22)$	2 × 0.10 ×	heet 49.7	Pile) 64) 71)	= -1.09< = -1.091	5	25.64
$V_1 = -(0.22)$	2 × 0.10 ×	heet 49.7	Pile) 64) 71)	= -1.09< = -1.091	5	25.64
$V_1 = -(0.22)$	2 × 0.10 ×	heet 49.7	Pile) 64) 71)	= -1.09< = -1.091	5	25.64
$V_1 = -(0.22)$	2 × 0.10 ×	heet 49.7	Pile) 64) 71)	= -1.09< = -1.091	5	25.64
$V_1 = -(0.22)$	2 × 0.10 ×	heet 49.7	Pile) 64) 71)	= -1.09< = -1.091	5	25.64
$V_1 = -(0.22)$	2 × 0.10 ×	heet 49.7	Pile) 64) 71)	= -1.09< = -1.091	5	25.64
$V_1 = -(0.22)$	2 × 0.10 ×	heet 49.7	Pile) 64) 71)	= -1.09< = -1.091	5	25.64
$V_1 = -(0.22)$	2 × 0.10 ×	heet 49.7	Pile) 64) 71)	= -1.09< = -1.091	5	25.64
$V_1 = -(0.22)$	2 × 0.10 ×	heet 49.7	Pile) 64) 71)	= -1.09< = -1.091	5	25.64
$V_1 = -(0.22)$	2 × 0.10 ×	heet 49.7	Pile) 64) 71)	= -1.09< = -1.091	5	25.64

LOCATION : WF. 97 + 36.32 ~ WF. 98 + 31.90 CALCULATION	RESU
$(H < 9, O_m)$	
(RIGHT BANK)	
$A_1 = (9.74 + 0.10) \times 0.10 \times 4 = 3.93$	4
$A_2 = (0.80 \times 0.10) + (1.90 + 0.10) \times 0.10 = 0.280$	2
$A_3 = (9.74 + 0.10) \times 0.10 \times 4 = 3.936$	
$A_4 = (0.80 \times 0.10) + (2.40 + 0.10) \times 0.10 = 0.330$	
$As = (29.684 + 0.10) \times 0.10 \times 9 = 11.919$	
$A_{6} = (0.80 \times 0.10) + (2.40 + 0.10) \times 0.10 = 0.330$	
<u> </u>	
(LEFT BANK)	
$A_7 = (9.70 + 0.10) \times 0.10 \times 4 = 3.920$	
$A8 = (0.80 \times 0.10) + (1.90 + 0.10) \times 0.10 = 0.280$	
<u> </u>	
$Aq = (q, 70 + 0.10) \times 0.10 \times 4 = 3.920$	
$A_{10} = (0.80 \times 0.10) + (2.40 + 0.10) \times 0.10 = 0.330$	<u>la de la seconda seconda de la seconda de la seconda s</u>
$A_{11} = (29.571 + 0.10) \times 0.10 \times 4 = 11.868$	
$A_{12} = (0.80 \times 0.10) + (2.40 + 0.10) \times 0.10 = 0.330$	
	14 41,374

C	36, <u>32 ~ WF. 98 + 31.90</u> ALCULATION RESULT
t=10 CELASTIC MATER	<2712)
JOINT FILL	ER (t = 10, ELASTIC MATERIAL)
STATION	
NO.	
WF97R+36.3	20
WF97R+41.3 WF98R+1.40	20 6.400 0 6.400
WF98R+11.9	00 8,498
WF98R+11.9 WF98R+21.9	
WF98R+21.9	00 6.400
WF98R+31.9	00 6.400
<u> </u>	
WF98L+21.9	00, 8.300
WF98L+21.9 WF98L+11.9	
WF98L+11.9	00 6.478
WF98L+1.40	0 6.400
WF97L+41.3 WF97L+36.3	20 6.400 20 6.400
<u></u> <u></u> <u></u> <u></u> <u></u> <u></u> <u></u>	
	<u>en en la completa de la contra de</u> La contra de la contr
	$-\tau_{0}\tau_{0} = 0 \sigma \sigma$
	TOTAL = 97.752 97.752
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	<i>TOTAL = 97.752 97.752</i>
	<u>707AL = 97.752</u> 97.752
	<i>TOTAL = 97.752 97.752</i>
	707AL = 97.752 97.752

TYPE OF WORK

EARTH RETAINING WALL

LOCATION

: CEMENT MORTAR POINTING : WF. 97 + 36.32 ~ WF. 98 + 31.90

CALCULATION RESULT , ¹ CEMENT MORTAR POINTING STATION PARTIAL SLOPE AVERAGE AREA NO. DISTANCE (m) LENGTH (m) (m) (m^2) WF97R+36.320 3.059 WF97R+41.320 7.099 3.059 3,059 21.719 WF98R+1.400 12.045 3.059 3.059 36.851 WF98R+11.900 10.540 3.681 3.370 35.525 ÷ WF98R+11.900 3.110 WF98R+21.900 10.040 3.569 3.340 33.532 WF98R+21.900 3.059 WF98R+31.900 10.040 3.059 3.059 30.716 TOTAL 158.343 WF98L+31.900 3.059 WF98L+21.900 10.000 3.569 3.314 33.144 WF98L+21.900 3.059 WF98L+11.900 10.000 3.681 3.370 33.705 WF98L+11.900 3.110 WF98L+1.400 10.500 3.059 3.085 32.392 WF97L+41.320 12.000 3.059 3.059 36.713 WF97L+36.320 7.071 3.059 3.059 21.633 TOTAL 157.586 TOTAL = 315.929315.929 m2 ÷

LOCATION	: WF. 97+36.32 ~ WF. 98+31.90 CALCULATION	
· P/ CHEET		RES
<u> </u>	PILE (1=3.00 m/pile)	•
B= 47 665	+7.099 = 49.764 m	
<u> </u>	$\frac{1}{1.071} = \frac{47.704}{1000}$	
n = 49.764	$\div 0.50 = 99.53 \rightleftharpoons 100$ piles	
<u> </u>	$\frac{1}{100} \frac{1}{100} \frac{1}$	
$B_2 = 42500$	+ 7.071 = 49.571 m	
24	<u></u>	
$n_2 = 49.571$	÷ 0.50 = 99.14 ÷ 100 piles	
	$\frac{1}{1} \frac{1}{1} \frac{1}$	
L = (100 + 100)	$0) piles \times 3.00 = 600.00$) 600. a
· PILING		
· · · · · · · · · · · · · · · ·		
N Value	: N = 32 (Average)	
<u>N Value</u>	: N = 32 (Average)	
		S
	: N = 32 (Average) pile number : n = 200 pile,	<u>S</u>
Necessary	pile number : n = 200 pile,	
Necessary	pile number : n = 200 pile,	
Necessary	pile number : n = 200 pile,	
Necessary	pile number : n = 200 pile,	
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