

TYPE OF WORK :

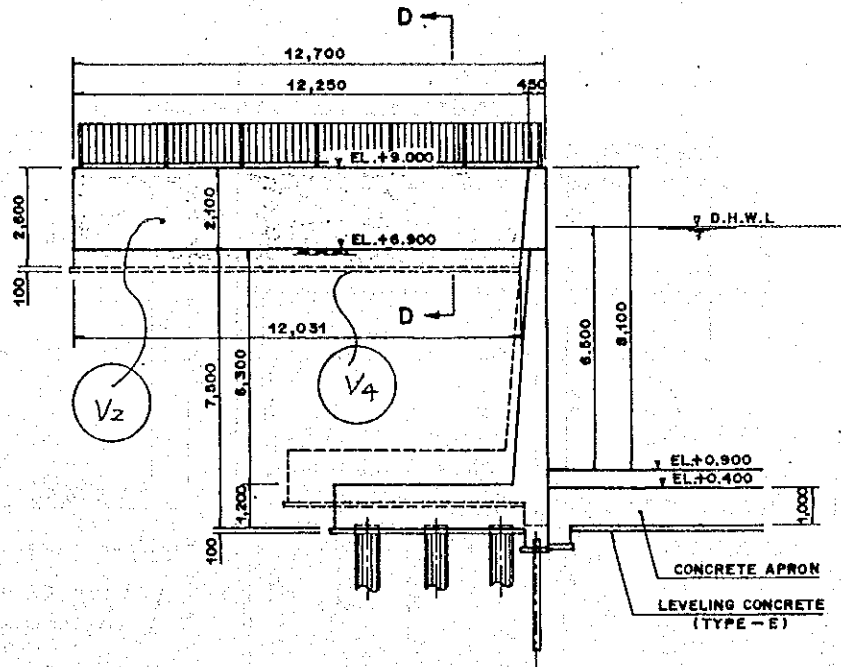
APPROACH WALL
EARTH RETAINING WALL

(1/2)

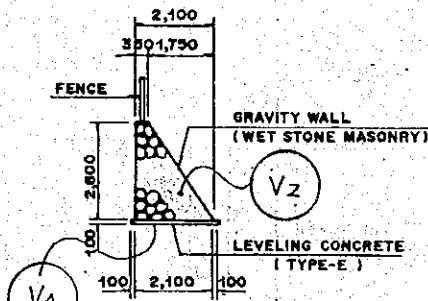
LOCATION :

DOWNSTREAM, RIGHT BANK

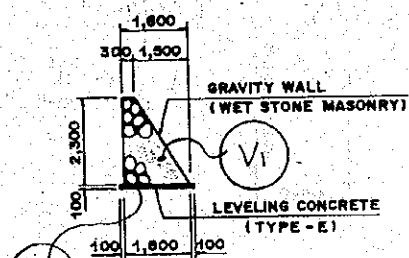
EXPLANATORY DRAWING



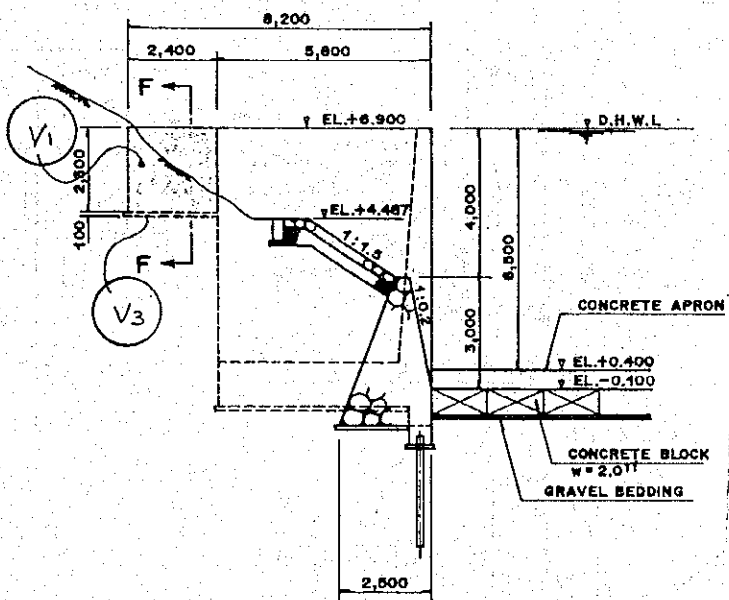
SECTION C-C
SCALE A



SECTION D-D
SCALE A



SECTION F-F
SCALE A



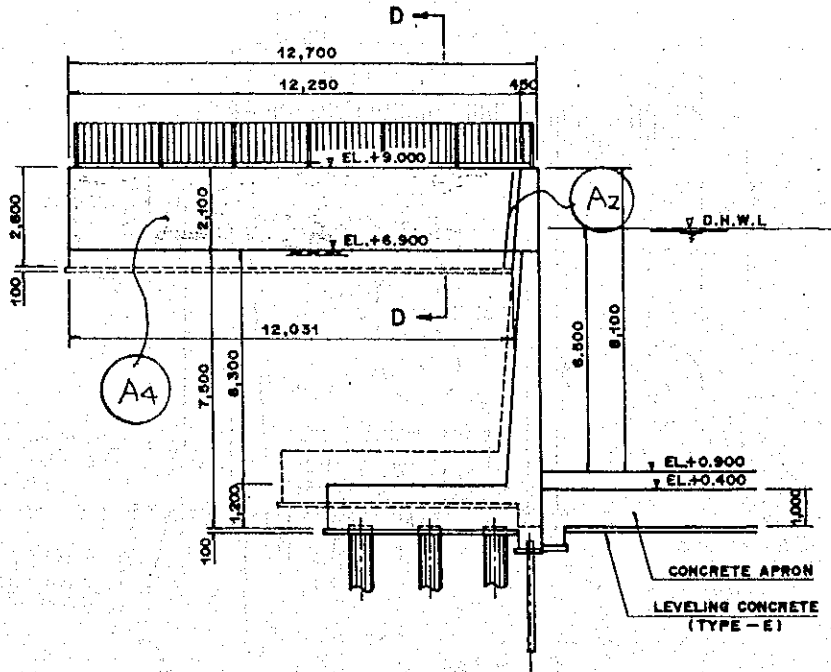
SECTION E-E
SCALE A

APPROACH WALL

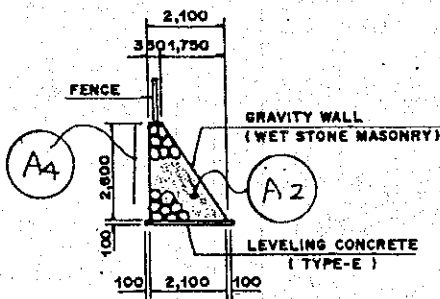
TYPE OF WORK : EARTH RETAINING WALL
 LOCATION : DOWNSTREAM, RIGHT BANK

(2/2)

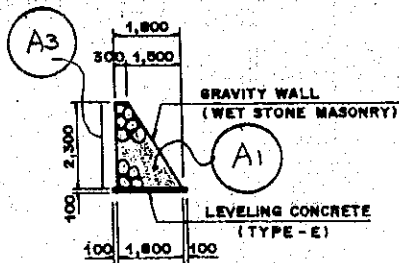
EXPLANATORY DRAWING



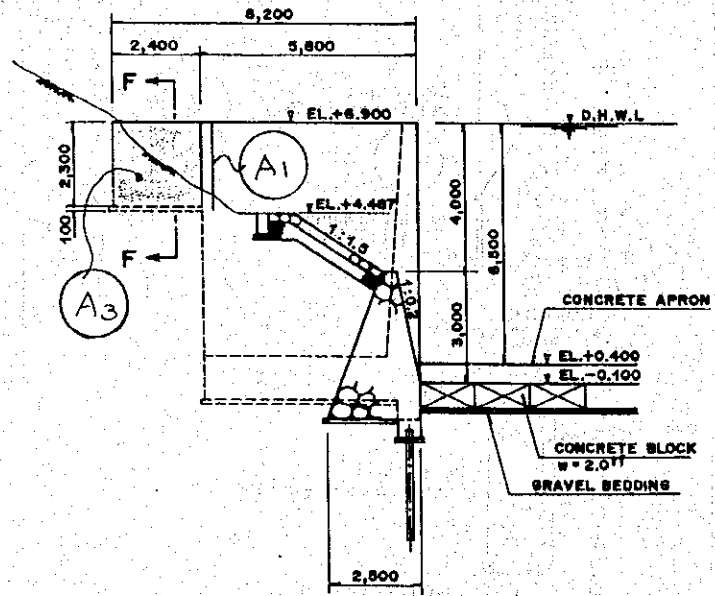
SECTION C-C
SCALE A



SECTION D-D
SCALE A



SECTION F-F
SCALE A



SECTION E-E
SCALE A

TYPE OF WORK : PILE HEAD TREATMENT
 LOCATION : APPROACH WALL
 : DOWN STREAM, RIGHT BANK

CALCULATION	RESULT
W - 1R	
PC PILE DIA 600 (A) n = 18 PILES	
LENGTH OF DESIGN : $L_1 = 10.50$ m	
SPARE PILE LENGTH : $L_2 = 1.00$ m	
ADOPTED PILE LENGTH	
$L = 10.50 + 1.00 = 11.50$ 12.00 m/pile	18 places
PILING	
N VALUE : $N = 22$ (Average)	
$D = 10.50$ m/pile	18 places
CONCRETE FILLING (TYPE-C1) n = 18 PILES	
$V = \frac{\pi}{4} \times 0.40^2 \times 1.30 = 0.163$ m ³ /pile	18 places
SUSPENDED FORM	
$A = \frac{\pi}{4} \times 0.40^2 = 0.126$ m ³ /pile	18 places
REINFORCING BAR	
D 19 (W = 2.23 kg/m)	
$W_1 = 10$ Bars $\times 1.95 \times 2.23 = 43.485$	
D 13 (W = 1.04 kg/m)	
$W_2 = 14$ Bars $\times 1.29 \times 1.04 = 18.782$	
TOTAL W = 62.267 kg.f/pile	18 places
CUTTING PILE HEAD	
Height of cutting : $h = 1.50$ m/pile	
$V = \frac{\pi}{4} \times (0.60^2 - 0.40^2) \times 1.50 = 0.236$ m ³ /pile	18 places

TYPE OF WORK : PILE HEAD TREATMENT
 LOCATION : APPROACH WALL
 : DOWN STREAM, RIGHT BANK

CALCULATION	RESULT
W - 2R	
PC PILE DIA 600 (A) n = 36 PILES	
LENGTH OF DESIGN : $L_1 = 11.00$ m	
SPARE PILE LENGTH : $L_2 = 1.00$ m	
ADOPTED PILE LENGTH	
$L = 11.00 + 1.00 = 12.00$ m/pile	36 places
PILING	
N VALUE : $N = 22$ (Average)	
$D = 11.00$ m/pile	36 places
CONCRETE FILLING (TYPE-C1) n = 36 PILES	
$V = \frac{\pi}{4} \times 0.40^2 \times 1.30 = 0.163$ m ³ /pile	36 places
SUSPENDED FORM	
$A = \frac{\pi}{4} \times 0.40^2 = 0.126$ m ³ /pile	36 places
REINFORCING BAR	
D 19 (W = 2.23 kg/m)	
$W_1 = 12 \text{ Bars} \times 1.95 \times 2.23 = 52.182$	
D 13 (W = 1.04 kg/m)	
$W_2 = 14 \text{ Bars} \times 1.29 \times 1.04 = 18.782$	
TOTAL W = 70.964 kg./pile	36 places
CUTTING PILE HEAD	
Height of cutting : $h = 1.00$ m/pile	
$V = \frac{\pi}{4} \times (0.60^2 - 0.40^2) \times 1.00 = 0.157$ m ³ /pile	36 places

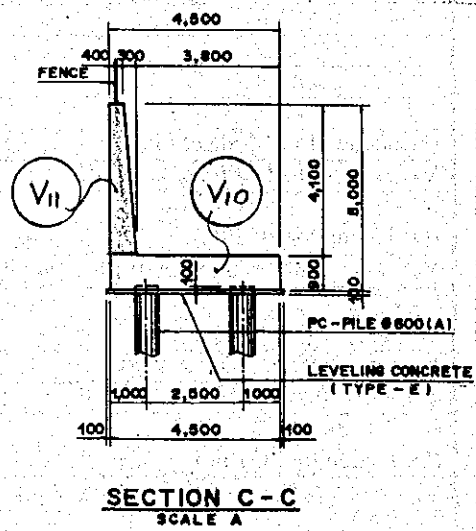
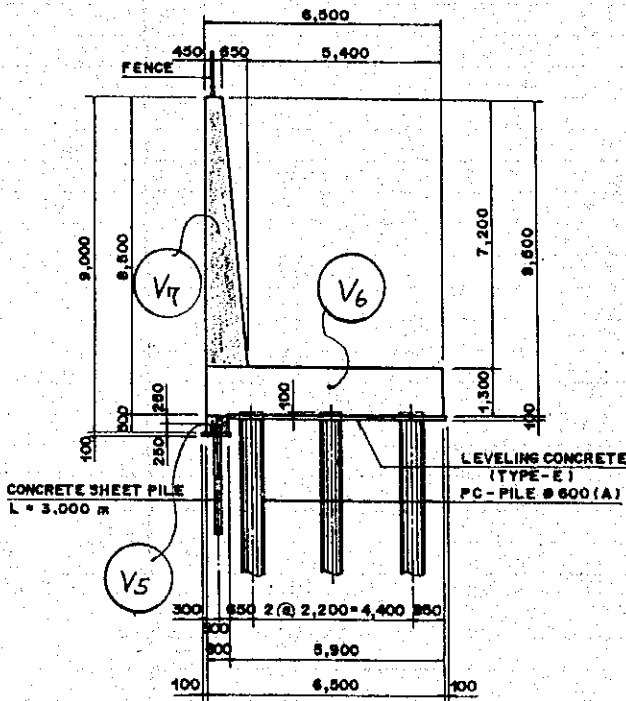
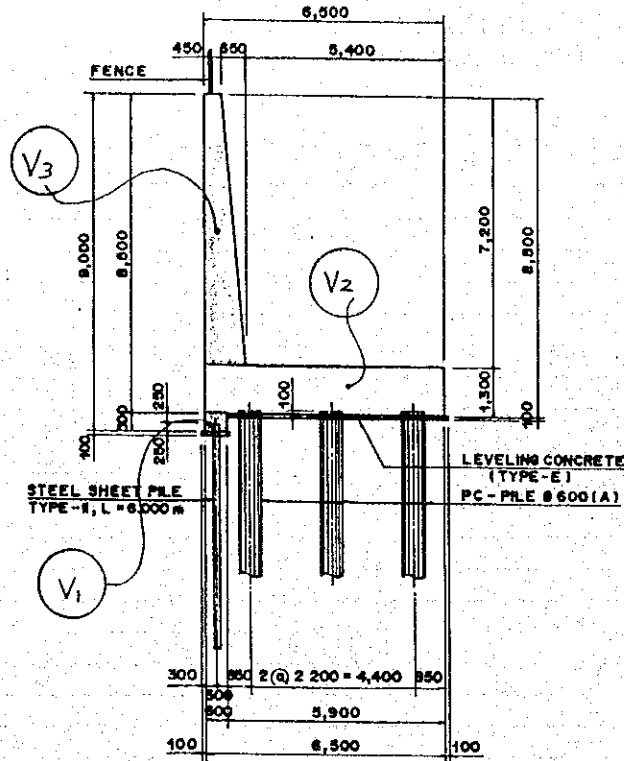
TYPE OF WORK : APPROACH WALL
 LOCATION : CONCRETE (TYPE - C1)
 : UPSTREAM, RIGHT BANK

CALCULATION		RESULT
(TYPE - C)		
W - 3R		
$V_1 = 0.60 \times 0.50 \times 7.50$	=	2.250
$V_2 = 1.30 \times 6.50 \times 7.50$	=	63.375
$V_3 = (0.45 + 1.10) \times \frac{1}{2} \times 7.20 \times 7.50$	=	41.850
(Deduction for PC Pile)		
$V_4 = (\pi/4 \times 0.60^2 \times 0.10 \times 15 \text{ piles})$	=	- 0.424
W - 4R		
$V_5 = 0.60 \times 0.50 \times 12.00$	=	3.600
$V_6 = 1.30 \times 6.50 \times 12.00$	=	101.400
$V_7 = (0.45 + 1.10) \times \frac{1}{2} \times 7.20 \times 12.00$	=	66.960
(Deduction for PC Sheet Pile)		
$V_8 = -(0.22 \times 0.25 \times 12.00)$	=	- 0.660
(Deduction for PC Pile)		
$V_9 = -(\pi/4 \times 0.60^2 \times 0.10 \times 24 \text{ piles})$	=	- 0.679
W - 5R		
$V_{10} = 0.90 \times 4.50 \times 9.00$	=	36.450
$V_{11} = (0.40 + 0.70) \times \frac{1}{2} \times 4.10 \times 9.00$	=	20.295
(Deduction for PC Pile)		
$V_{12} = -(\pi/4 \times 0.60^2 \times 0.10 \times 8 \text{ piles})$	=	- 0.226
TOTAL		= 334.191
		334.191 m ³

APPROACH WALL

TYPE OF WORK : CONCRETE (TYPE-C1)
 LOCATION : UPSTREAM, RIGHT BANK

EXPLANATORY DRAWING



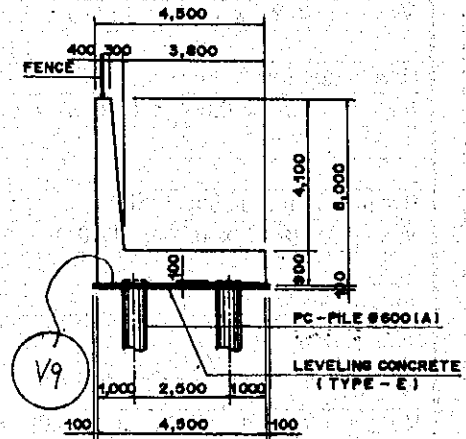
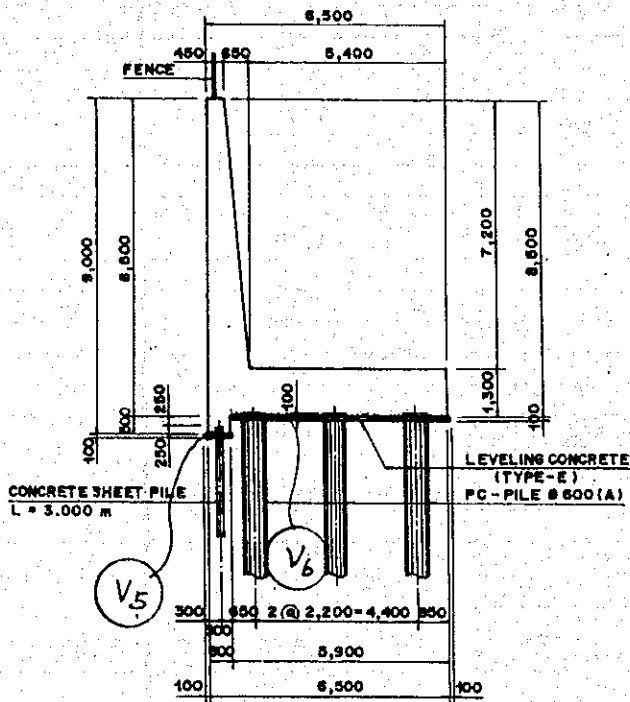
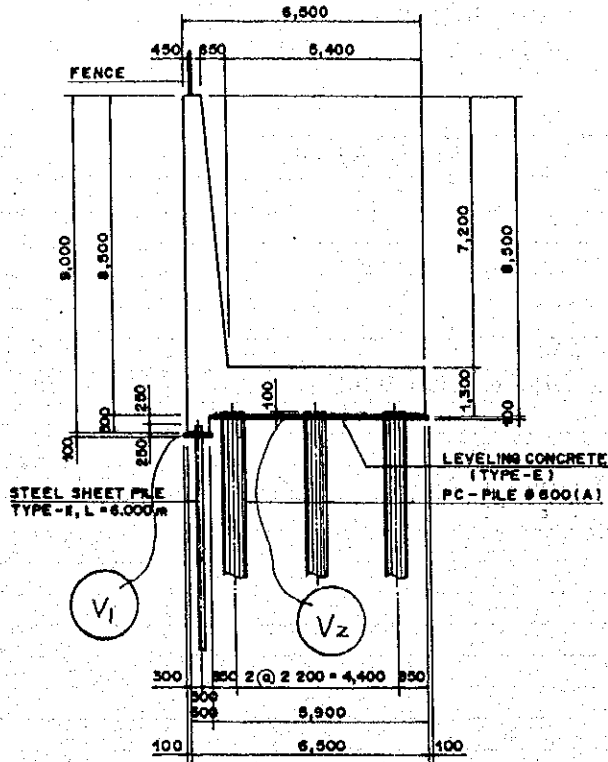
TYPE OF WORK : APPROACH WALL
 LOCATION : LEVELING CONCRETE
 : UPSTREAM, RIGHT BANK

CALCULATION		RESULT
(TYPE - E)		
W - 3R		
$V_1 = 0.80 \times 0.10 \times 7.50$	=	0.600
$V_2 = 0.10 \times (5.90 + 0.10) \times (7.50 + 0.10)$	=	4.560
(Deduction for PC Pile)		
$V_4 = (\pi/4 \times 0.60^2 \times 0.10 \times 15 \text{ piles})$	=	- 0.424
W - 4R		
$V_5 = 0.80 \times 0.10 \times (12.00 + 0.10)$	=	0.968
$V_6 = 0.10 \times (5.90 + 0.10) \times (12.00 + 0.10 \times 2)$	=	7.320
(Deduction for PC Pile)		
$V_7 = (0.22 \times 0.25 \times 12.00)$	=	- 0.660
(Deduction for PC Pile)		
$V_8 = (\pi/4 \times 0.60^2 \times 0.10 \times 24 \text{ piles})$	=	- 0.679
(Deduction for PC Sheet Pile)		
$V_8 = -(0.22 \times 0.25 \times 12.00)$	=	- 0.660
W - 5L		
$V_9 = (4.50 + 0.10 \times 2) \times (9.00 + 0.10 \times 2) \times 0.10$	=	4.324
(Deduction for PC Pile)		
$V_{10} = -(\pi/4 \times 0.60^2 \times 0.10 \times 8 \text{ piles})$	=	- 0.226
TOTAL		15.783
		15.783 m ³

APPROACH WALL

TYPE OF WORK : LEVELING CONCRETE
 LOCATION : UPSTREAM, RIGHT BANK

EXPLANATORY DRAWING



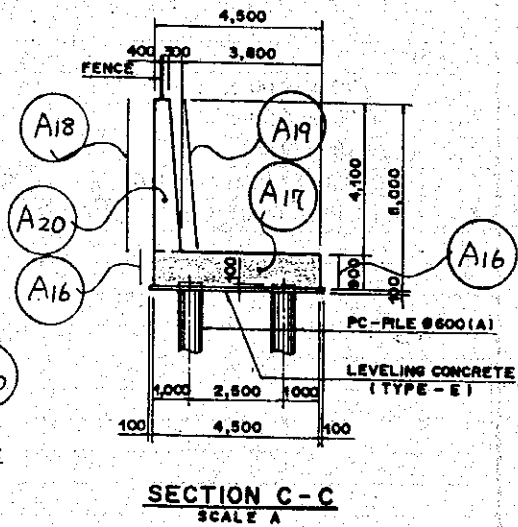
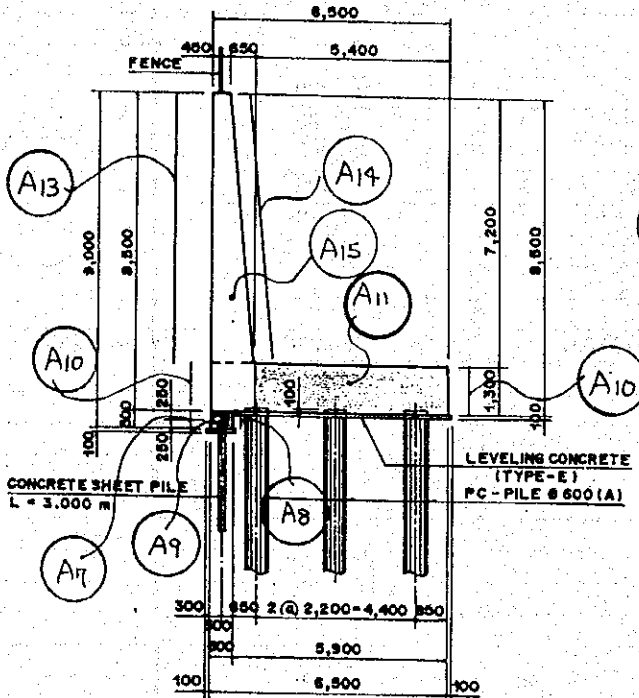
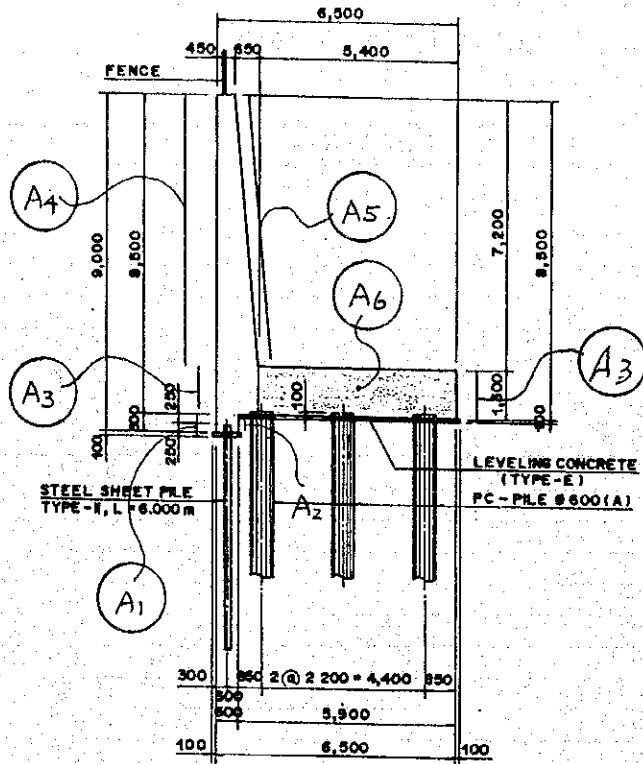
TYPE OF WORK : APPROACH WALL
 LOCATION : FORM
 : UPSTREAM, RIGHT BANK

CALCULATION		RESULT
(H ≥ 4.0 m)		
W - 3R		
A ₁ = 0.50 x 7.50	=	3.750
A ₂ = 0.40 x 7.50	=	3.000
A ₃ = 1.30 x 7.50 x 2	=	19.500
A ₄ = 7.20 x 7.50	=	54.000
A ₅ = $\sqrt{0.65^2 + 7.20^2} \times 7.50$	=	54.220
A ₆ = 5.40 x 1.30	=	7.020
W - 4R		
A ₇ = 0.50 x 12.00	=	6.000
A ₈ = 0.40 x 12.00	=	4.800
A ₉ = 0.50 x 0.60	=	0.300
A ₁₀ = 1.30 x 12.00 x 2	=	31.200
A ₁₁ = 5.40 x 1.30	=	7.020
A ₁₂ = 6.50 x 1.30	=	8.450
A ₁₃ = 7.20 x 12.00	=	86.400
A ₁₄ = $\sqrt{0.65^2 + 7.20^2} \times 12.00$	=	86.751
A ₁₅ = (0.45 + 1.10) x ½ x 7.20 x 2	=	11.160
W - 5R		
A ₁₆ = 0.90 x 9.00 x 2	=	16.200
A ₁₇ = 0.90 x 4.50 x 2	=	8.100
A ₁₈ = 4.10 x 9.00	=	36.900
A ₁₉ = $\sqrt{0.30^2 + 4.10^2} \times 9.00$	=	36.999
A ₂₀ = (0.40 + 0.70) x ½ x 4.10 x 2	=	4.510
TOTAL		= 486.280
		486.280 m ²

APPROACH WALL

TYPE OF WORK : FORM
 LOCATION : UPSTREAM, RIGHT BANK

EXPLANATORY DRAWING



TYPE OF WORK : APPROACH WALL
 LOCATION : FORM FOR LEVELING CONCRETE
 : UPSTREAM, RIGHT BANK

CALCULATION		RESULT
(H < 4.0 m)		
W - 3R		
$A_1 = 0.10 \times 7.50 \times 2$	= 1.500	
$A_2 = 0.10 \times (7.50 + 0.10) \times 2$	= 1.520	
$A_3 = 0.10 \times (5.90 + 0.10) \times 2$	= 1.200	
W - 4R		
$A_4 = 0.10 \times (12.00 \times 0.10) \times 2$	= 2.420	
$A_5 = 0.10 \times 0.80$	= 0.080	
$A_6 = 0.10 \times (12.00 + 0.10 \times 2) \times 2$	= 2.440	
$A_7 = 0.10 \times (5.90 + 0.10) \times 2$	= 1.200	
(Deduction for PC Sheet Pile)		
$A_8 = -(0.22 \times 0.10)$	= -0.022	
W - 5R		
$A_9 = 0.10 \times (9.00 + 0.10 \times 2) \times 2$	= 1.840	
$A_{10} = 0.10 \times (4.50 + 0.10 \times 2) \times 2$	= 0.940	
TOTAL		13.118 m ²
B = 200 mm		
W - 3R		
$L_1 = 8.500$		
W - 4R		
$L_2 = 8.500$		
W - 5R		
$L_3 = 5.000$		
TOTAL		22.000 m

APPROACH WALL

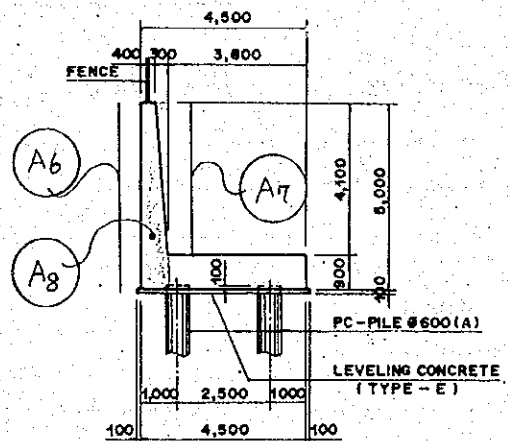
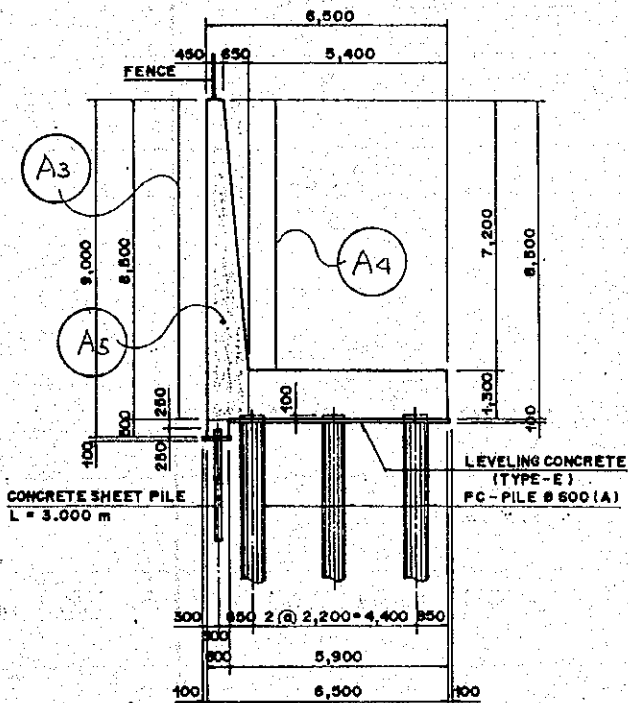
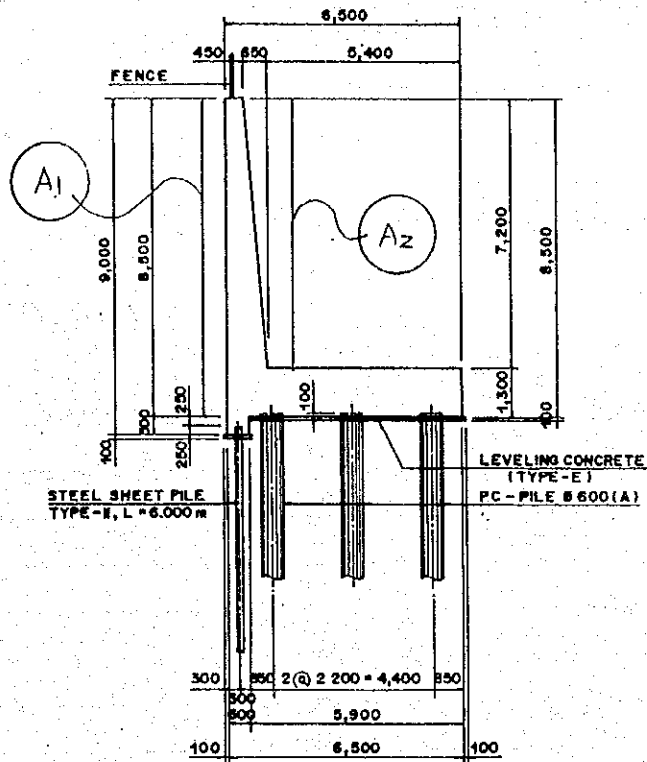
TYPE OF WORK :

SCAFFOLDING

LOCATION :

UPSTREAM, RIGHT BANK

EXPLANATORY DRAWING



TYPE OF WORK : APPROACH WALL
 : JOINT FILLER
 LOCATION : UPSTREAM, RIGHT BANK

CALCULATION	RESULT
$t=10$ (ELASTIC JOINT FILLER)	
(W-3R)	
$A_1 = (0.45 + 1.10) \times \frac{1}{2} \times 7.20 + (1.30 \times 6.50)$	
$= 14.030$	
$A_2 = 0.50 \times 0.60 \times 2$	
$= 0.600$	
$A_3 = (0.45 + 1.10) \times \frac{1}{2} \times 7.20 + (1.30 \times 1.10) = 7.010$	
(W-4R)	
$A_4 = (0.45 + 1.10) \times \frac{1}{2} \times 7.20 + (1.30 \times 1.10) = 7.010$	
$A_5 = 0.50 \times 0.60$	
$= 0.300$	
(Deduction for PC Sheet Pile)	
$A_6 = -(0.22 \times 0.25)$	
$= -0.055$	
(W-5R)	
$A_7 = (0.40 + 0.70) \times \frac{1}{2} \times 4.10 + (0.90 \times 0.70) = 2.885$	
TOTAL = 31.780	31.780 m²

APPROACH WALL

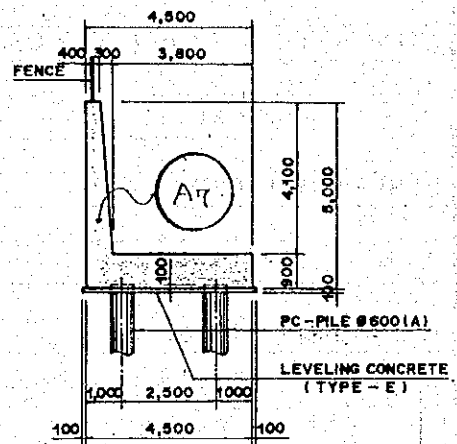
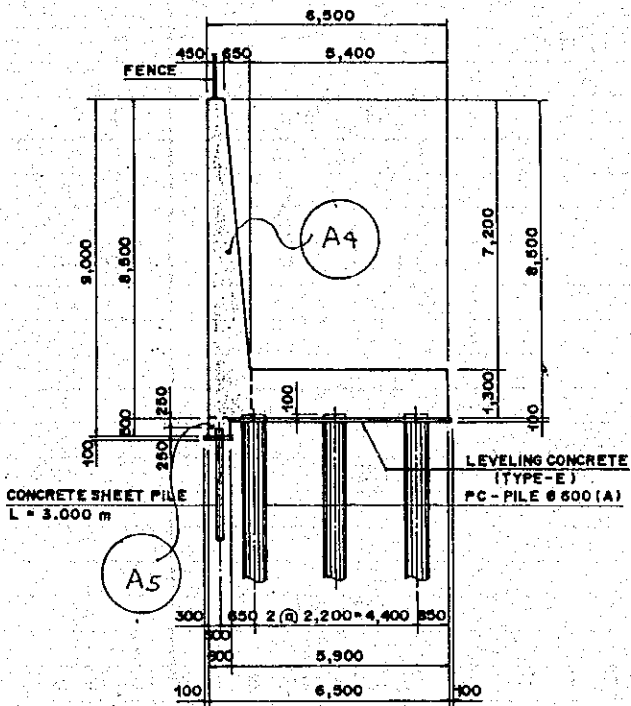
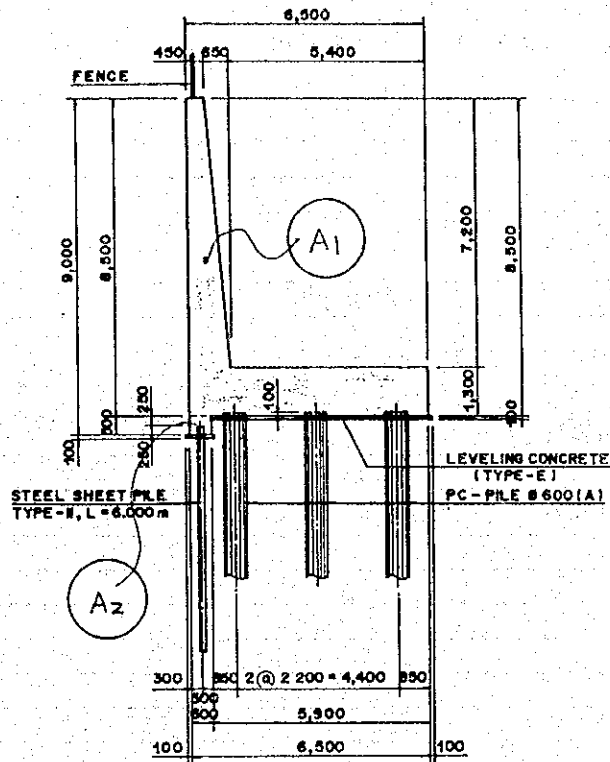
TYPE OF WORK :

JOINT FILLER

LOCATION :

UPSTREAM, RIGHT BANK

EXPLANATORY DRAWING



TYPE OF WORK : PILE HEAD TREATMENT
 LOCATION : APPROACH WALL
 : UP STREAM, RIGHT BANK

CALCULATION	RESULT
W - 3R	
PC PILE DIA 600 (A) n = 15 PILES	
LENGTH OF DESIGN : $L_1 = 11.60$ m/pile	
SPARE PILE LENGTH : $L_2 = 1.00$ m	
ADOPTED PILE LENGTH	
$L = 11.60 + 1.00 = 12.60$ 13.00 m/pile	15 places
JOINTING	
n = 1 place/pile	15 points
PILING	
N VALUE : N = 22 (Average)	
$D = 11.60$ m/pile	15 places
CONCRETE FILLING (TYPE-C1) n = 15 PILES	
$V = \frac{\pi}{4} \times 0.40^2 \times 1.30 = 0.163$ m ³ /pile	15 places
SUSPENDED FORM	
$A = \frac{\pi}{4} \times 0.40^2 = 0.126$ m ³ /pile	15 places
REINFORCING BAR	
D 19 (W = 2.23 kg/m)	
$W_1 = 12 \text{ Bars} \times 1.95 \times 2.23 = 52.182$	
D 13 (W = 1.04 kg/m)	
$W_1 = 14 \text{ Bars} \times 1.29 \times 1.04 = 18.782$	
TOTAL W = 70.964 kg.f/pile	15 places

TYPE OF WORK : PILE HEAD TREATMENT
 LOCATION : APPROACH WALL
 : UP STREAM, RIGHT BANK

CALCULATION	RESULT
CUTTING PILE HEAD	
Height of cutting : h = 1.40 m/pile	
$V = \frac{\pi}{4} \times (0.60^2 - 0.40^2) \times 1.40 = 0.220 \text{ m}^3/\text{pile}$	15 places
W - 4R	
PC PILE DIA 600 (A) n = 24 PILES	
ADOPTED PILE LENGTH	
L = 13.00 m/pile	24 places
JOINTING	
n = 1 place/pile	24 points
PILING	
N VALUE: N = 22 (Average)	
D = 11.60 m/pile	24 places
CONCRETE FILLING (TYPE-C1) n = 24 PILES	
V = 0.163 m ³ /pile	24 places
SUSPENDED FORM	
A = 0.126 m ³ /pile	24 places
REINFORCING BAR	
W = 70.964 kg.f/pile	24 places
CUTTING PILE HEAD	
Height of cutting : h = 1.40 m/pile	
V = 0.220 m ³ /pile	24 places

TYPE OF WORK : PILE HEAD TREATMENT
 LOCATION : APPROACH WALL
 : UP STREAM, RIGHT BANK

CALCULATION	RESULT
W - 5R	
PC PILE DIA 600 (A) n = 8 PILES	
LENGTH OF DESIGN : $L_1 = 6.50$ m/pile	
SPARE PILE LENGTH : $L_2 = 1.00$ m	
ADOPTED PILE LENGTH	
$L = 6.50 + 1.00 = 7.50$ 8.00 m/pile	8 places
PILING	
N VALUE : $N = 22$ (Average)	
$D = 6.50$ m/pile	8 places
CONCRETE FILLING (TYPE-C1) n = 8 PILES	
$V = \frac{\pi}{4} \times 0.40^2 \times 1.150$ = 0.145 m ³ /pile	8 places
SUSPENDED FORM	
$A = \frac{\pi}{4} \times 0.40^2$ = 0.126 m ³ /pile	8 places
REINFORCING BAR	
D 16 (W = 1.58 kg/m)	
$W_1 = 8 \text{ Bars} \times 1.65 \times 1.58$ = 20.856	
D 13 (W = 1.04 kg/m)	
$W_1 = 12 \text{ Bars} \times 1.25 \times 1.04$ = 15.600	
TOTAL W = 36.456 kg.f/pile	8 places
CUTTING PILE HEAD	
Height of cutting : $h = 1.50$ m/pile	
$V = \frac{\pi}{4} \times (0.60^2 - 0.40^2) \times 1.50$ = 0.236 m ³ /pile	8 places

3.3 Intake Structure

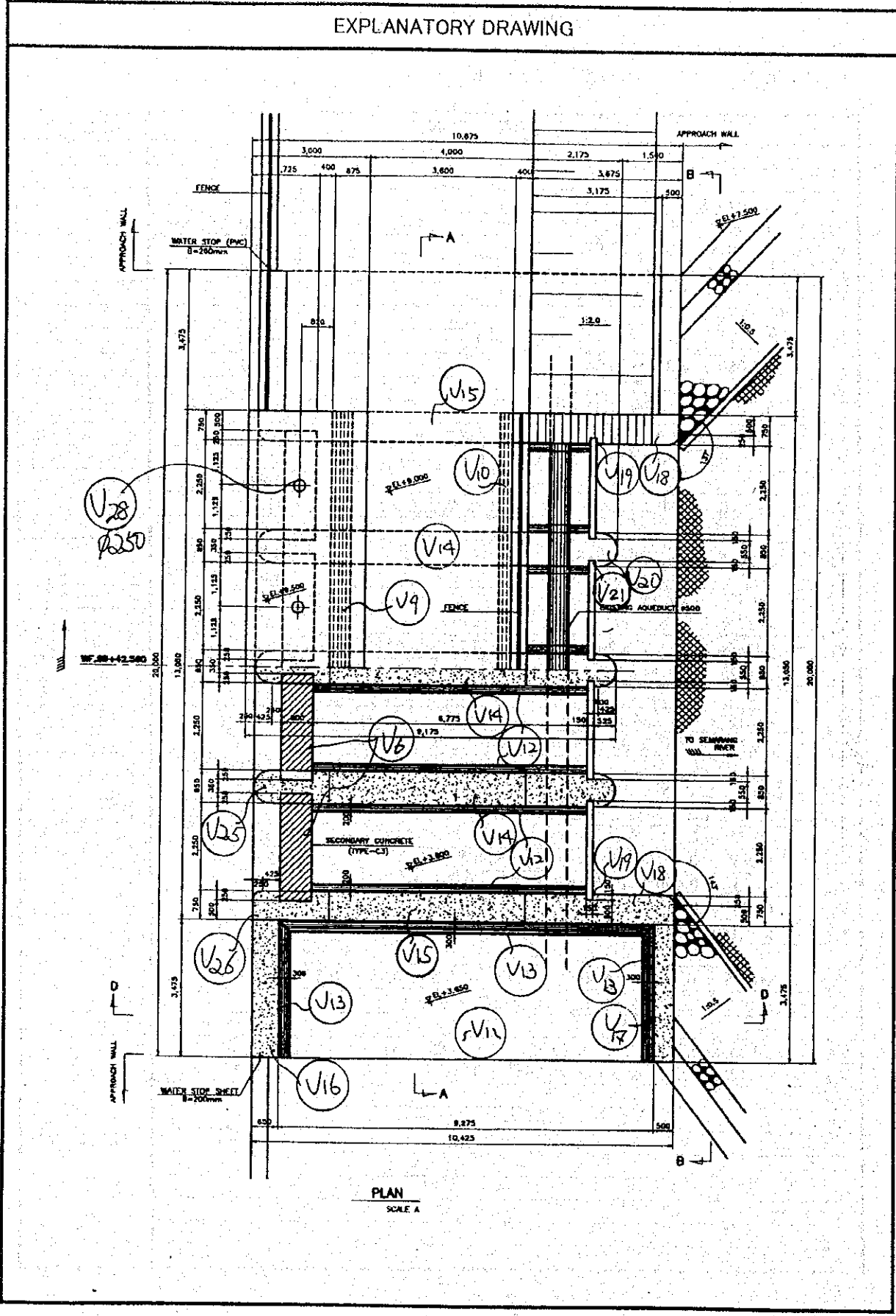
TYPE OF WORK : CONCRETE (TYPE - C1)
 LOCATION : INTAKE WEIR RIGHT

CALCULATION	RESULT
$V_1 = 10.675 \times 20.000 \times 0.85$	= 181.475
$V_2 = 0.500 \times 0.600 \times 20.000$	= 6.000
$V_3 = 2.950 \times 0.600 \times 20.000$	= 35.400
$V_4 = \frac{1}{2} \times 0.500 \times 0.500 \times 20.000$	= 2.500
$V_5 = -0.15 \times (2.25 + 0.15 \times 2) \times 0.15 \times 4$	= -0.230
$V_6 = -0.8 \times 0.2 \times 2.75 \times 4$	= -1.760
$V_7 = -(0.35 \div 2)^2 \times 3.142 \times (8 \times 5) \times 0.1$	= -0.385
$V_8 = 13.050 \times 4.875 \times 0.5$	= 31.809
$V_9 = \frac{1}{2} \times 0.5^{00} \times 0.5^{00} \times 13.05^0$	= 1.631
$V_{10} = \frac{1}{2} \times 0.3^{00} \times 0.3^{00} \times 13.05^0$	= 0.587
$V_{11} = 0.4^{00} \times 2.7^{00} \times 13.05^0$	= 14.094
$V_{12} = \frac{1}{2} \times 0.2^{00} \times 0.2^{00} \times (5.275 \times 8 + 6.775 \times 8)$	= 1.928
$V_{13} = \frac{1}{2} \times 0.3^{00} \times 0.3^{00} \times \{(9.275 - 0.2) \times 2 + (3.475 - 0.1) \times 4\}$	= 1.424
$V_{14} = 0.85 \times 2.000 \times 4.875 \times 3$	= 24.863
$V_{15} = 0.75^0 \times 2.000 \times 4.875 \times 2$	= 14.625
$V_{16} = 0.65^0 \times 5.20^{00} \times 3.475 \times 2$	= 23.491
$V_{17} = 0.5^{00} \times 3.7^{00} \times 3.475 \times 2$	= 12.858
$V_{18} = \{1/2 \times (3.700 + 5.200) \times 3.000 \times 0.75 + 0.675 \times 3.700 \times 0.75 - 0.25 \times 0.25 \times 3.7 + 0.25 \times 3.142 \div 4 \times 3.7\} \times 2$	= 23.672
$V_{19} = -0.15 \times 0.15 \times 4.413 \times 2$	= -0.199
$V_{20} = \{2.3^{00} \times 1.65^0 \times 0.85^0 + 0.425 \times 0.425 \times 3.142 \div 2 \times 2.3^{00}\}$	= 3.878
$V_{21} = -0.15 \times 0.15 \times 2.3^{00} \times 6$	= -0.311
$V_{22} = (3^{000} \times 0.5^{00} + \frac{1}{2} \times (1.2^{00} + 3^{000}) \times 0.15) \times 13.05$	= 23.686
$V_{23} = (0.75^0 \times 0.4^{00} + 0.5^{00} \times 0.8^{00}) \times 3.05^0 \times 2$	= 4.270
$V_{24} = (0.85^0 \times 0.400 + 0.35^0 \times 0.8^{00}) \times 3.05^0 \times 3$	= 5.673
$V_{25} = (0.85^0 \times 0.4^{00} + 0.35^0 \times 0.8^{00} + 0.25^0 \times 0.85^0 + 0.425 \times 0.425 \times 3.142 \div 2) \times 2.000 \times 3$	= 6.698
$V_{26} = \{(0.75 \times 0.4 + 0.5 \times 0.8) \times 2.0 + (0.425 \times 0.75 + 0.5 \times 0.25 + 0.25 \times 0.25 \times 3.142 \div 4) \times (5.05^0 + 1/3 \times 0.15^0)\} \times 2$	= 7.827
$V_{27} = -0.25 \times 0.25 \times 3.142 \times (0.75 \times 2 + 0.85 \times 3)$	= -0.795
$V_{28} = -0.125 \times 0.125 \times 3.142 \times 0.65^0 \times 4$	= -0.128
TOTAL	= 424.581
	424.581 m³

TYPE OF WORK : CONCRETE (Type-C1)
 LOCATION : INTAKE WEIR RIGHT

(1/2)

EXPLANATORY DRAWING

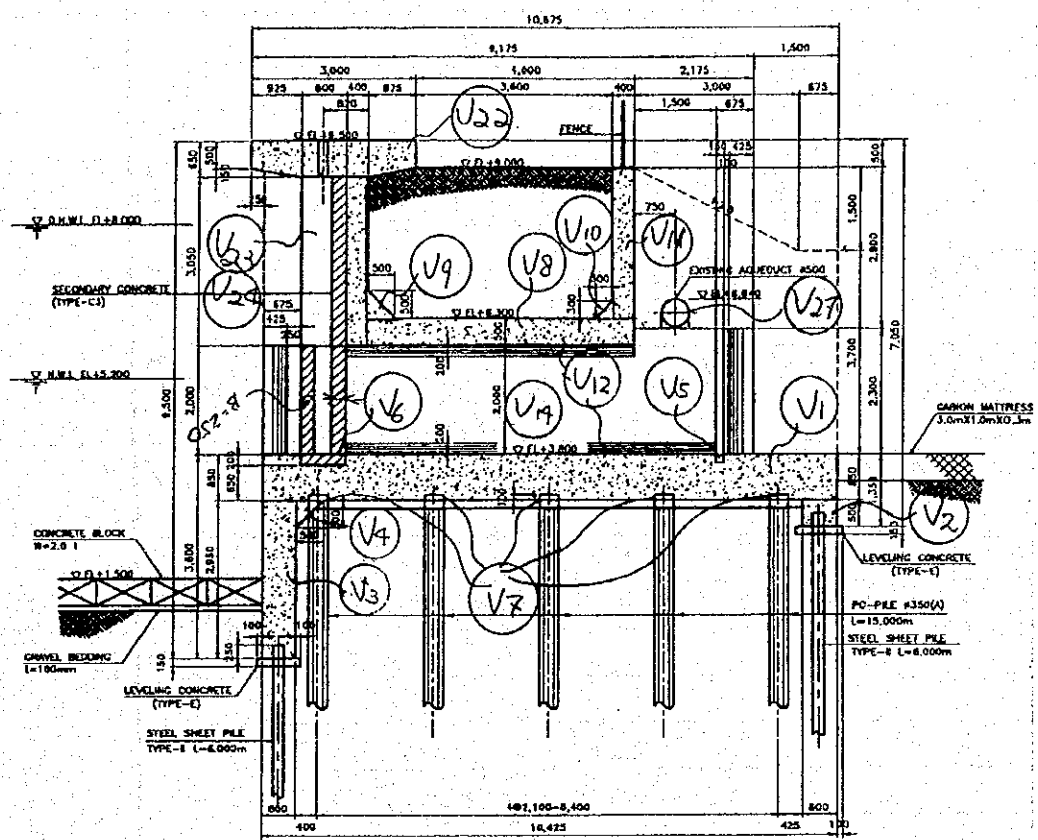


TYPE OF WORK : CONCRETE (Type-C1)

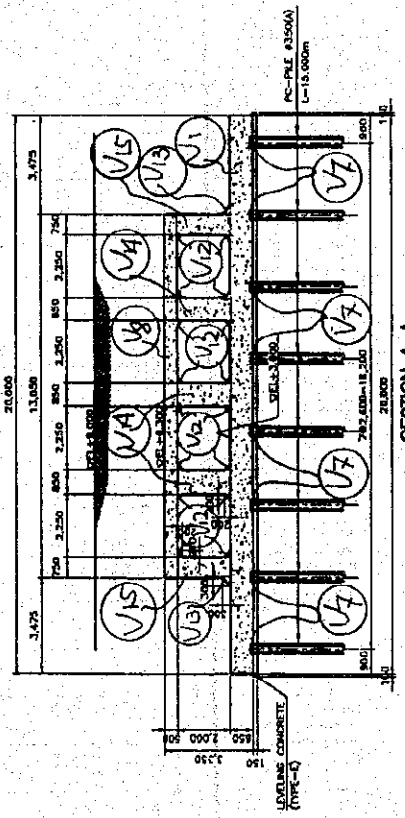
(2/2)

LOCATION : INTAKE WEIR RIGHT

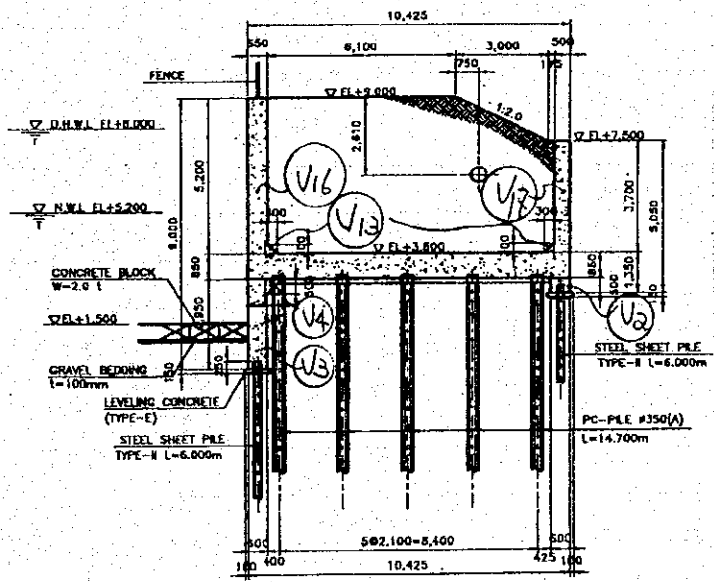
EXPLANATORY DRAWING



PROFILE
SCALE A



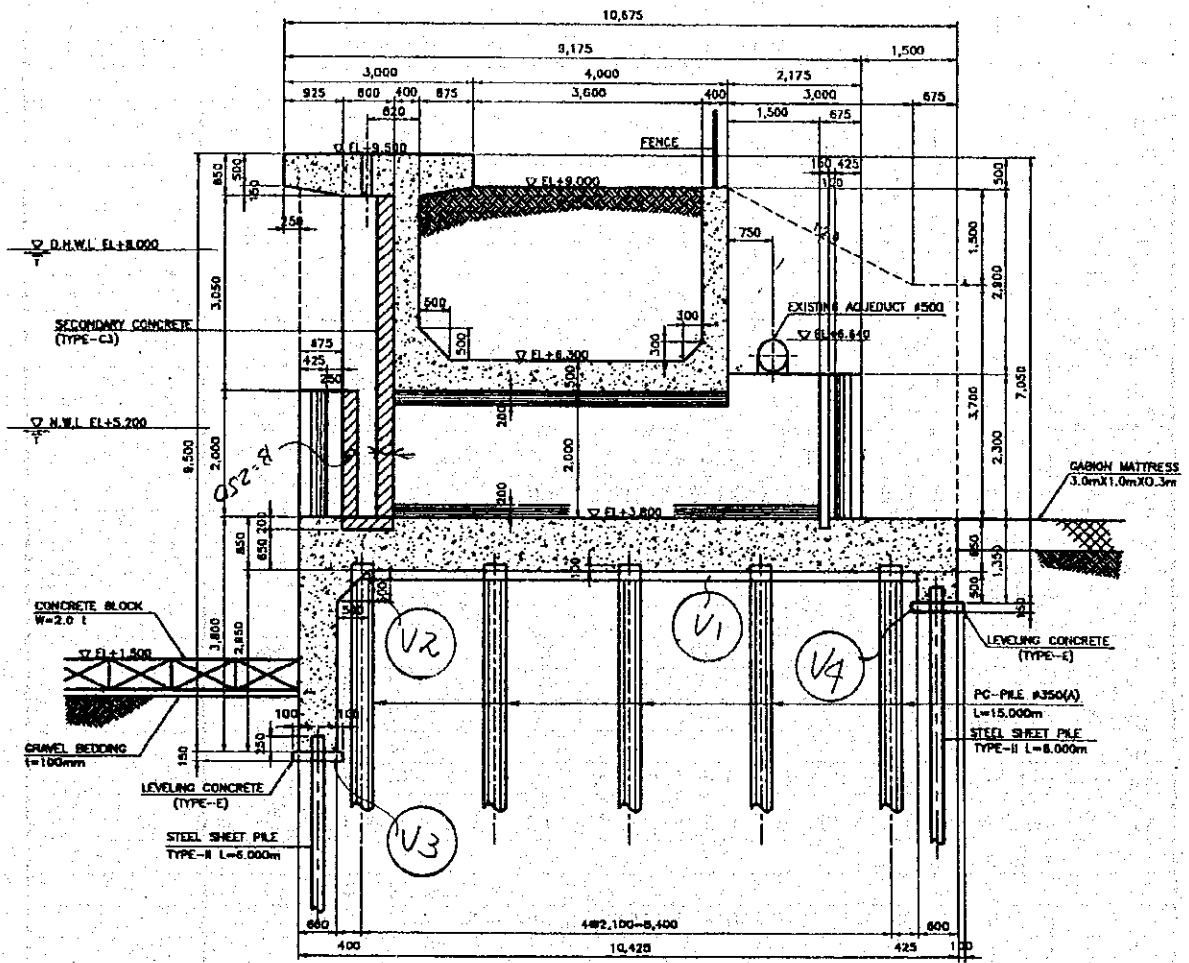
SECTION A-A
SCALE B



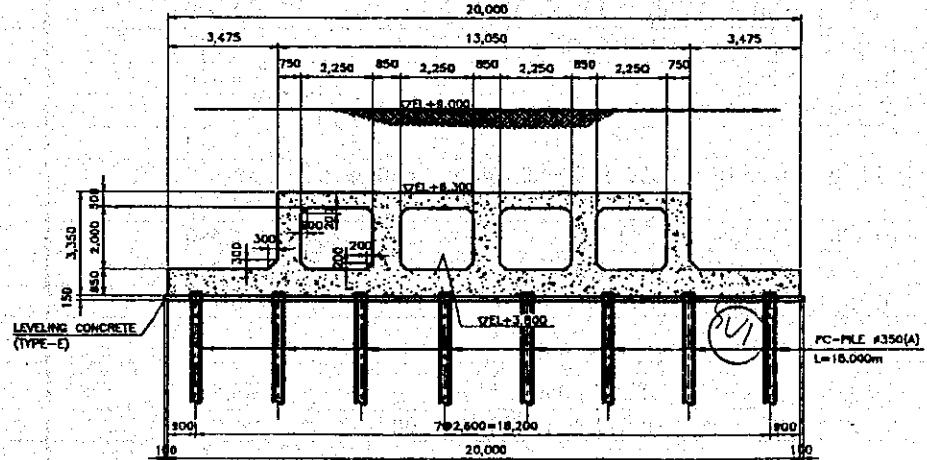
SECTION D-D
SCALE C

TYPE OF WORK : LEVELING CONCRETE
 LOCATION : INTAKE WEIR RIGHT

EXPLANATORY DRAWING



PROFILE
SCALE A



SECTION A-A
SCALE B

TYPE OF WORK : FORM (H ≥ 4.0 m)
 LOCATION : INTAKE WEIR RIGHT

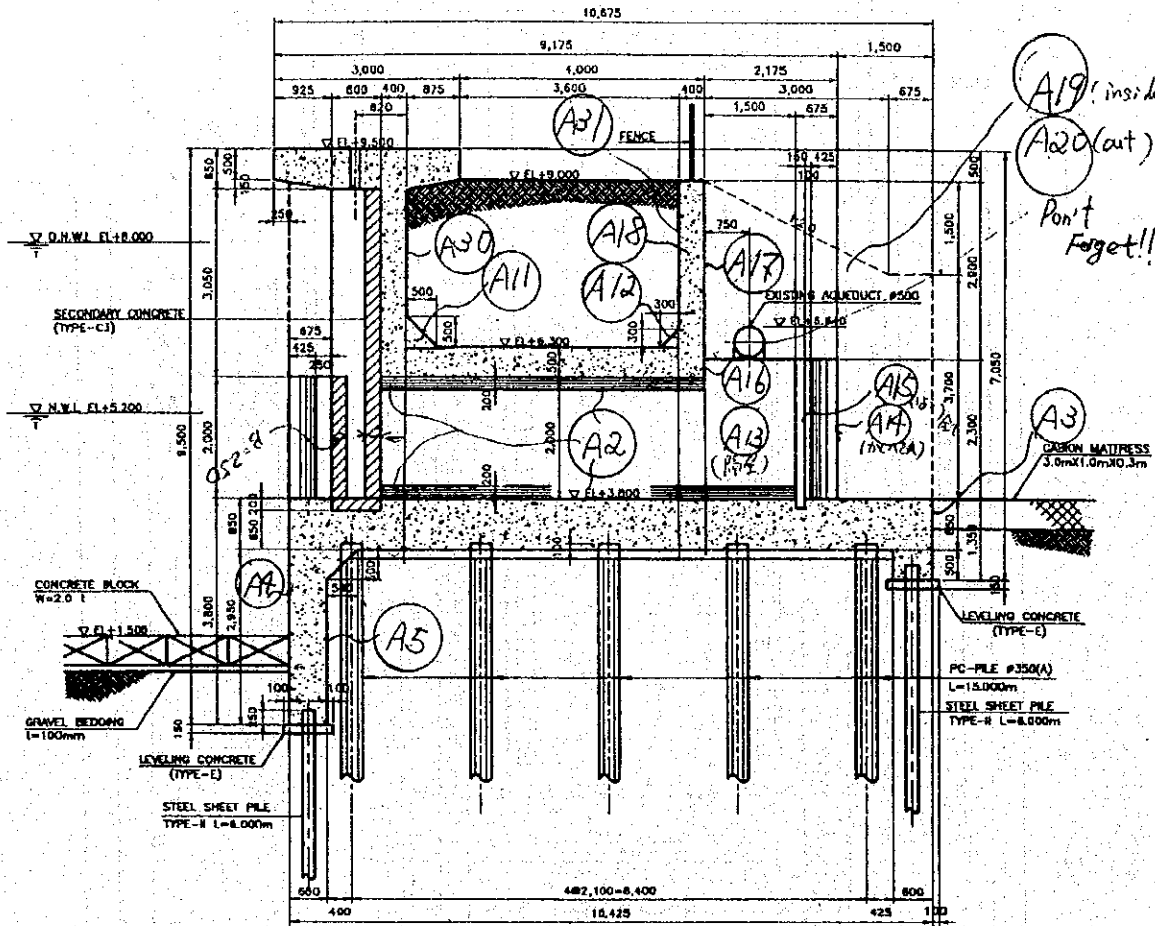
CALCULATION		RESULT
$A_4 = 3.8^{00} \times 20^{000}$	=	76.000
$A_5 = (2.45^0 + \sqrt{2} \times 0.5^{00}) \times 20^{000}$	=	63.142
$A_6 = 3.975 \times 5.200 \times 2$	=	41.340
$A_7 = (\sqrt{2} \times 0.300 + 4.900) \times 3.475 \times 2$	=	37.004
$A_8 = (1.35 \times 4.225 + 3.700 \times 3.975) \times 2$	=	40.823
$A_9 = (\sqrt{2} \times 0.300 + 3.400) \times 3.475 \times 2$	=	26.579
$A_{10} = (\sqrt{2} \times 0.300 + 2.200) \times 4.475 \times 2$	=	23.487
$A_{11} = \frac{1}{2} \times 0.500 \times 0.500 \times 2$	=	0.250
$A_{12} = \frac{1}{2} \times 0.300 \times 0.300 \times 2$	=	0.090
$A_{13} = (\sqrt{2} \times 0.200 + 2.100) \times 1.500 \times 6$	=	21.446
$A_{14} = (0.100 \times 2 + 2 \times 0.425 \times 3.142 \div 2) \times 2.300 \times 3$	=	10.594
$A_{15} = 0.150 \times 2 \times 2.450 \times 8 + 0.150 \times 2 \times 2.25 \times 4 + \frac{1}{2} \times 0.200 \times 0.200 \times 8$	=	8.740
$A_{16} = 0.300 \times 2.250 \times 4 + \frac{1}{2} \times 0.200 \times 0.200 \times 8$	=	2.860
$A_{17} = 2.900 \times 13.050$	=	37.845
$A_{18} = 0.4 \times 6.05 \times 2$	=	4.840
$A_{19} = (0.675 \times 3.700 + \frac{1}{2} \times (3.700 + 5.200)) \times 2$	=	30.124
$A_{20} = (\sqrt{2} \times 0.300 + 3.400) \times 0.175 + \{(\sqrt{2} \times 0.3 + 3.4) \times 2 + 1.5\} \times \frac{1}{2} \times 3.0\} \times 2$	=	28.784
$A_{21} = (0.650 \times 9.000 + \frac{1}{2} \times (0.850 + 1.150) \times 0.300 + \frac{1}{2} \times 0.500 \times 0.500) \times 2$	=	12.550
$A_{22} = (\sqrt{2} \times 0.30 + 4.750) \times 1.875 \times 2$	=	21.970
$A_{23} = 0.25 \times 2 \times 3.142 \div 4 \times 5.200 \times 2$	=	12.819
$A_{24} = 0.425 \times 5.200 \times 2$	=	4.420
$A_{25} = (0.250 \times 2 + 0.425 \times 2 \times 3.142 \div 2 + 0.250 \times 2)$	=	14.012
$A_{26} = (0.800 + 0.250) \times 5.050 \times 8 + 0.250 \times 5.050 \times 2$	=	44.945
$A_{27} = (0.800 + 2.750 \times 2) \times 0.200 \times 4$	=	5.680
$A_{28} = 0.25 \times 2 \times 3.142 \div 4 \times 3.700 \times 2$	=	2.906
$A_{29} = 0.125 \times 0.125 \times 3.142 \times 0.65 \times 4$	=	0.128
$A_{30} = (\sqrt{2} \times 0.500 + 2.050) \times 13.050$	=	35.980
$A_{31} = (\sqrt{2} \times 0.300 + 2.400) \times 13.050$	=	36.857
$A_{32} = 0.500 \times 5.05 + \frac{1}{2} \times (0.850 + 1.150) \times 0.300 \times 2$	=	3.125
TOTAL		649.338
		649.338 m ²

TYPE OF WORK : FORM (H ≥ 4.0m)

LOCATION : INTAKE WEIR RIGHT

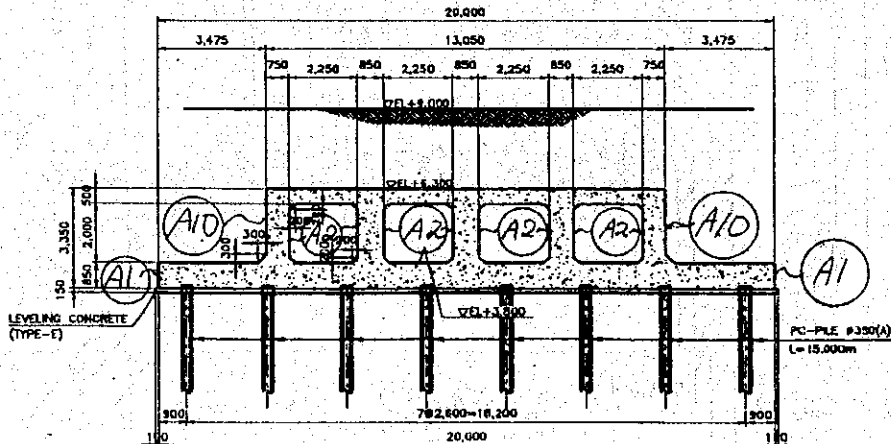
(2/2)

EXPLANATORY DRAWING



PROFILE

SCALE A



SECTION A-A

SCALE B

TYPE OF WORK :
 LOCATION : INTAKE WEIR RIGHT

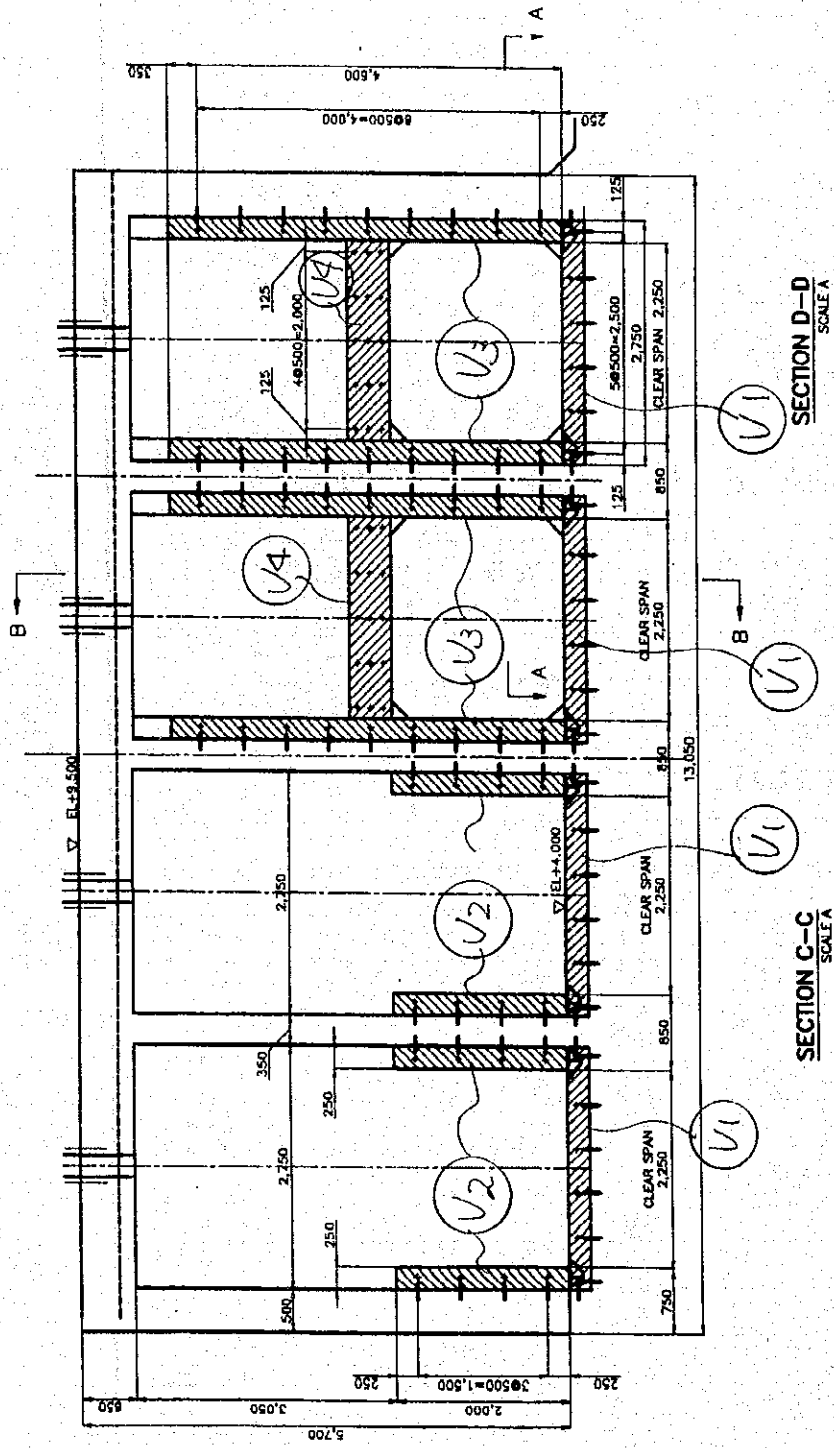
CALCULATION		RESULT
<input type="checkbox"/> FORM (H < 4.0 m)		<input type="checkbox"/>
$A_1 = 0.850 \times 8.675 \times 2$	= 14.748	
$A_2 = (\sqrt{2} \times 0.200 \times 2 + 1.6) \times 6.775 \times 2 \times 4$	= 117.380	
$A_3 = 1.35 \times 11.55$	= 15.593	
TOTAL		147.721 m ³
<input type="checkbox"/> FORMWORK FOR LEVELLING CONCRETE (H < 4.0 m)		
$A_1 = (10.425 - 0.6 \times 2 - 0.5) \times 0.15 \times 2$	= 2.612	
$A_2 = \frac{1}{2} \times 0.15 \times 0.15 \times 2$	= 0.023	
$A_3 = (0.8 \times 2 + 20.2 \times 2) \times 0.15$	= 6.300	
$A_4 = (0.8 \times 2 + 20.2 \times 2) \times 0.15$	= 6.300	
TOTAL		15.241 m ²

TYPE OF WORK :
 LOCATION : INTAKE WEIR RIGHT

CALCULATION		RESULT
□ SECONDARY CONCRETE & ANCHOR BAR		
(TYPE - C3)		
$V_1 = (2.25 + 0.25 \times 2) \times 0.8 \times 0.25 \times 4$	=	2.200
$V_2 = 2.0 \times 0.25 \times 0.285 \times 8$	=	1.140
$V_3 = 4.95 \times 0.25 \times 0.25 \times 8$	=	2.475
$V_4 = (0.5^{00} \times 0.22 + 0.175 \times 0.03) \times 2.25 \times 4$	=	1.037
TOTAL	=	6.852
		6.852 m ³
□ FORM FOR SECONDARY CONCRETE		
(H ≥ 4.0 m)		
$A_1 = 0.25 \times 4.95 \times 2 \times 4$	=	9.900
$A_2 = 0.25 \times 2.25 \times 4$	=	2.250
$A_3 = 0.25 \times 4.95 \times 2 \times 4$	=	9.900
$A_4 = 0.285 \times 2.00 \times 2 \times 4$	=	4.560
$A_5 = 0.25 \times 2.00 \times 2 \times 4$	=	4.000
$A_6 = 0.50 \times 2.25 \times 4$	=	4.500
TOTAL	=	35.110
		35.110 m ²
□ ANCHOR BAR (D19 , L = 300 mm/Bar)		
BOTTOM	$(6 \times 3 + 6 \times 2 + 4) \times 4$	= 136
TOP	$3 \times 5 \times 4$	= 60
FORMER SIDE	$4 \times 2 \times 2 \times 4$	= 64
LATTER SIDE	$9 \times 2 \times 2 \times 4$	= 144
n	= 404 Bars	
W	= 404 Bars x 0.30 x 2.23 kgf/m	= 270.276 kgf
		0.270 tf

TYPE OF WORK : SECONDARY CONCRETE & ANCHOR BAR
 LOCATION : INTAKE WEIR RIGHT

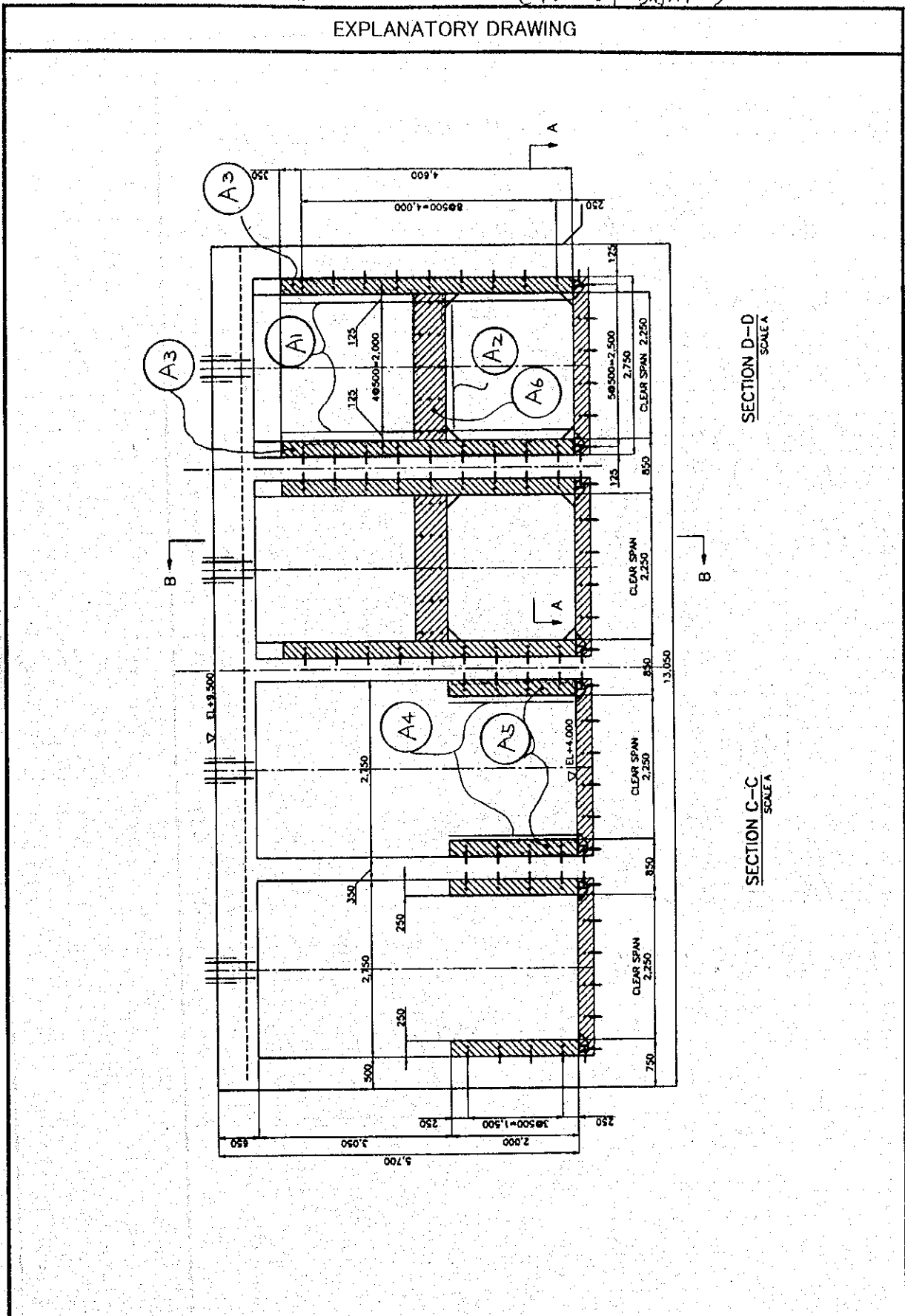
EXPLANATORY DRAWING



SECTION D-D
SCALE A

SECTION C-C
SCALE A

TYPE OF WORK : FORM FOR SECONDARY CONCRETE
 LOCATION : INTAKE STRUCTURE (RIGHT BANK)



SECTION D-D
SCALE A

SECTION C-C
SCALE A

TYPE OF WORK : SUPPORTING
 LOCATION : INTAKE WEIR RIGHT

CALCULATION		RESULT
$V_1 = 13.05^0 \times (0.25^0 \times 8.85^0 + \frac{1}{2} \times 0.25 \times (0.15 + 0.109)) =$	29.296	
$V_2 = \{ (0.25 \times 0.85 + 0.425 \times 0.425 \times 3.142 \div 2) \times 3 \}$ $\times (3.05 + \frac{1}{3} \times 0.109) = 1.4888 \times 3.0863 =$	4.595	
$V_3 = (0.675 \times 11.55 - 1.4888) \times (5.05 + \frac{1}{3} \times 0.109) =$	32.082	
$V_4 = 0.8^00 \times 2.75^0 \times (5.05 + 0.2^00) \times 4 =$	46.200	
$V_5 = \{ (2.05^0 + 2.200) \times 0.875 \times \frac{1}{2} + \frac{1}{2} \times (0.375$ $+ 0.875) \times 0.5^00 \} \times 13.05 =$	28.343	
$V_6 = (2.000 \times 2.250 - \frac{1}{2} \times 0.2^00 \times 0.2^00 \times 4) \times 5.275 \times 4 =$	93.262	
TOTAL	233.778	233.778 m ³
FORM OF TOP SLAB		
$A1(\text{CONT}) = 11.55 \times \sqrt{0.925^2 + 0.15^2} =$	10.823	
$A2(-\text{ditto}-) = 2.75 \times 0.8 \times 4 =$	8.800	
$A3(-\text{ditto}-) = 13.05 \times \sqrt{0.875^2 + 0.15^2} =$	11.585	
$A4(\text{Box}) = 1.85 \times 5.275 \times 4 =$	39.235	
TOTAL	70.243	m ²

TYPE OF WORK :

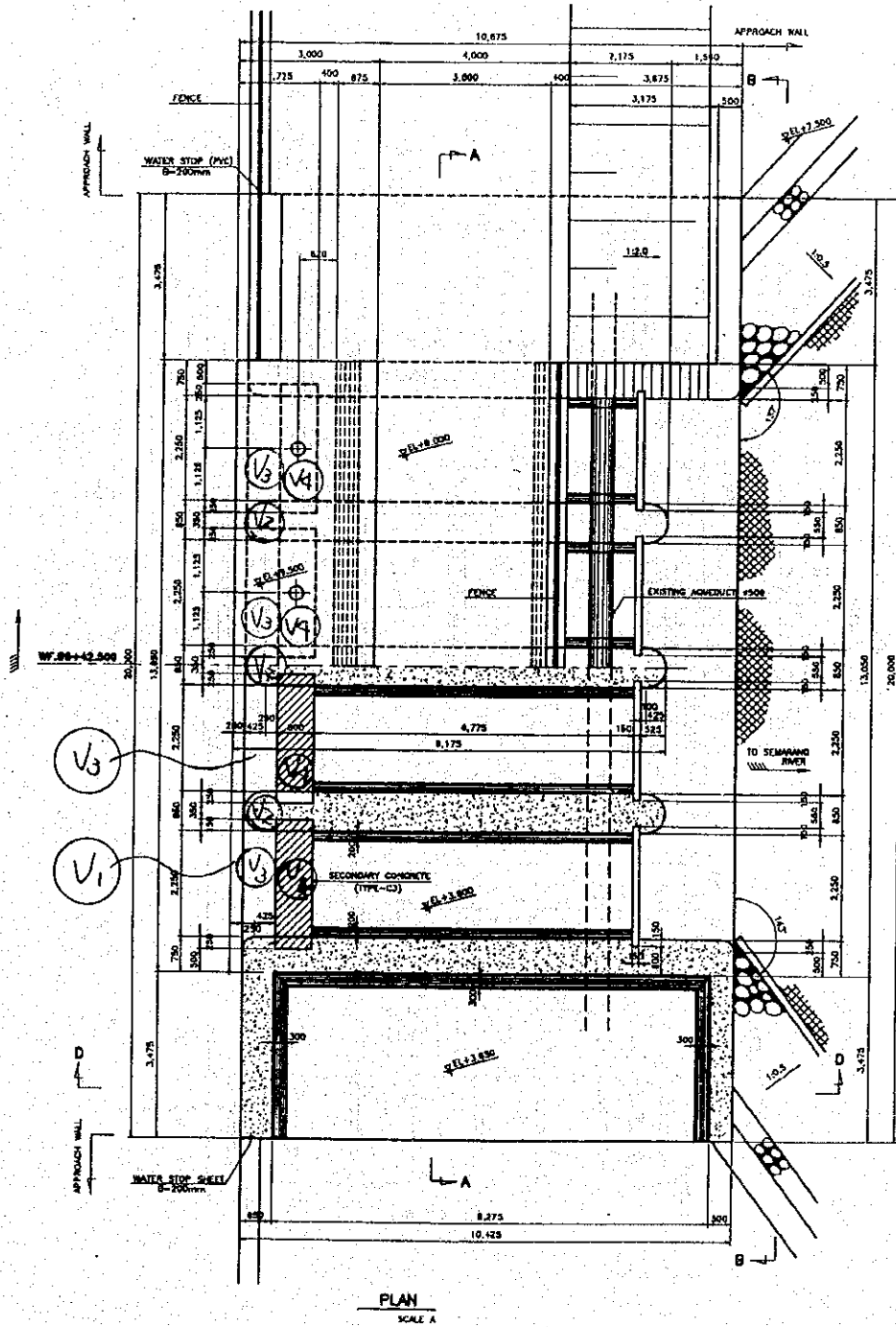
SUPPORTING

LOCATION :

INTAKE WEIR RIGHT

(1/2)

EXPLANATORY DRAWING



TYPE OF WORK :

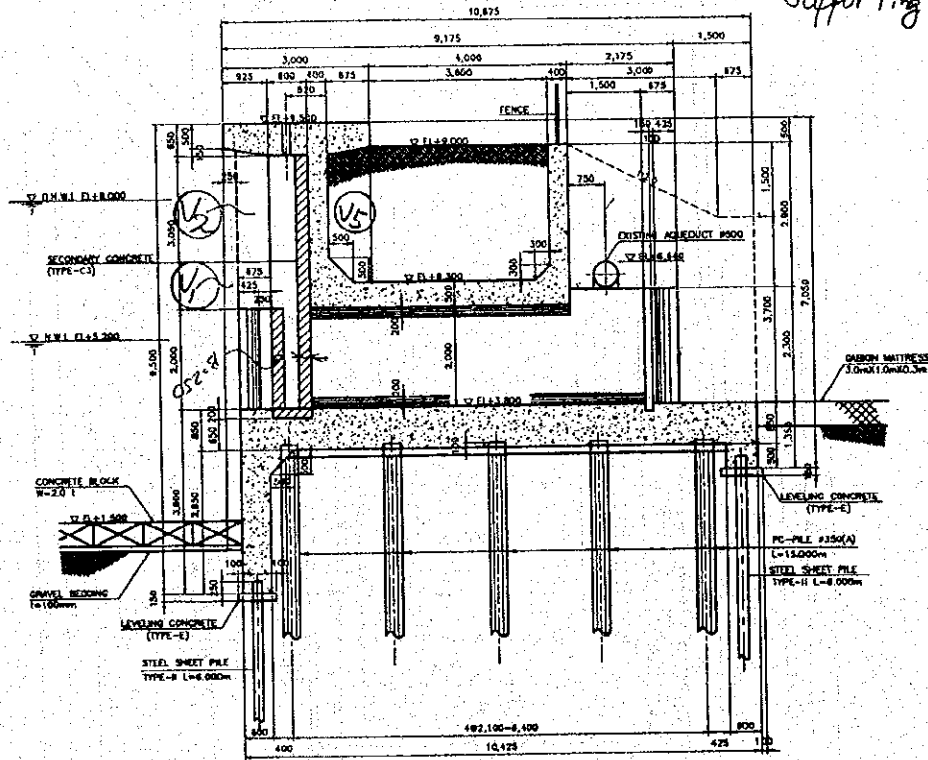
SUPPORTING

LOCATION :

INTAKE WEIR RIGHT

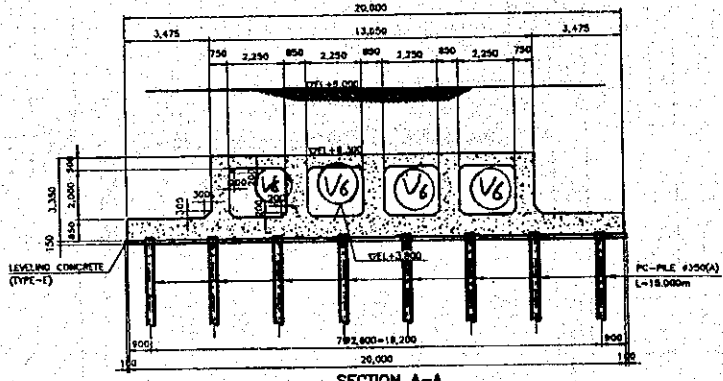
(2/2)

EXPLANATORY DRAWING



Supporting

PROFILE
SCALE A



SECTION A-A
SCALE B

TYPE OF WORK

: SCAFFOLDING

LOCATION

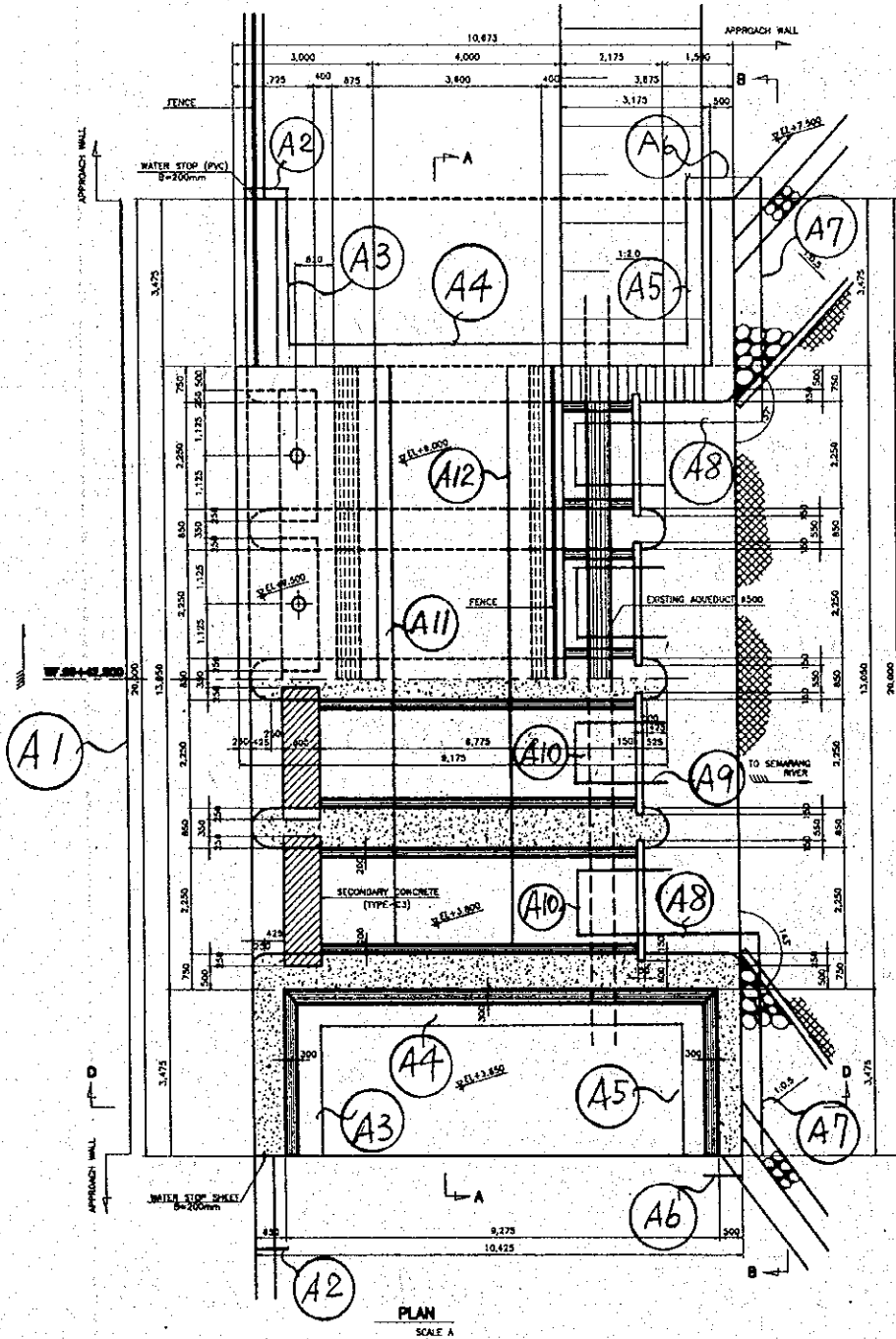
: INTAKE WEIR RIGHT

CALCULATION		RESULT
$A_1 = 9.5^{00} \times 20.000 =$	190.000	
$A_2 = 9.000 \times 0.65^0 \times 2 =$	11.700	
$A_3 = 5.2^{00} \times 3.475 \times 2 =$	36.140	
$A_4 = \{ 5.2^{00} \times 2.35^0 + 2.5^{00} \times 3.6^{00} + 5.2^{00} \times 0.4$ $+ \frac{1}{2} \times (3.7^{00} + 5.2^{00}) \times 3.000 + 0.075 \times 3.7^{00} \}$ $\times 2 =$	73.855	
$A_5 = 5.2^{00} \times 3.475 \times 2 =$	36.140	
$A_6 = 4.55^0 \times 0.6^{00} \times 2 =$	5.460	
$A_7 = 4.55^0 \times 4.225 \times 2 =$	38.448	
$A_8 = \{ 3.7^{00} \times 0.675 + \frac{1}{2} \times (3.7^{00} + 5.2^{00}) \times 3.000 \}$ $\times 2 =$	31.695	
$A_9 = 2.3^{00} \times 2.175 \times 6 =$	30.015	
$A_{10} = 5.2^{00} \times 2.25^0 \times 4 =$	46.80	
$A_{11} = 3.2^{00} \times 13.05^0 =$	41.760	
$A_{12} = 2.7^{00} \times 13.05^0 =$	35.235	
	TOTAL	577.248

TYPE OF WORK : SCAFFOLDING
 LOCATION : INTAKE WEIR RIGHT

(1/2)

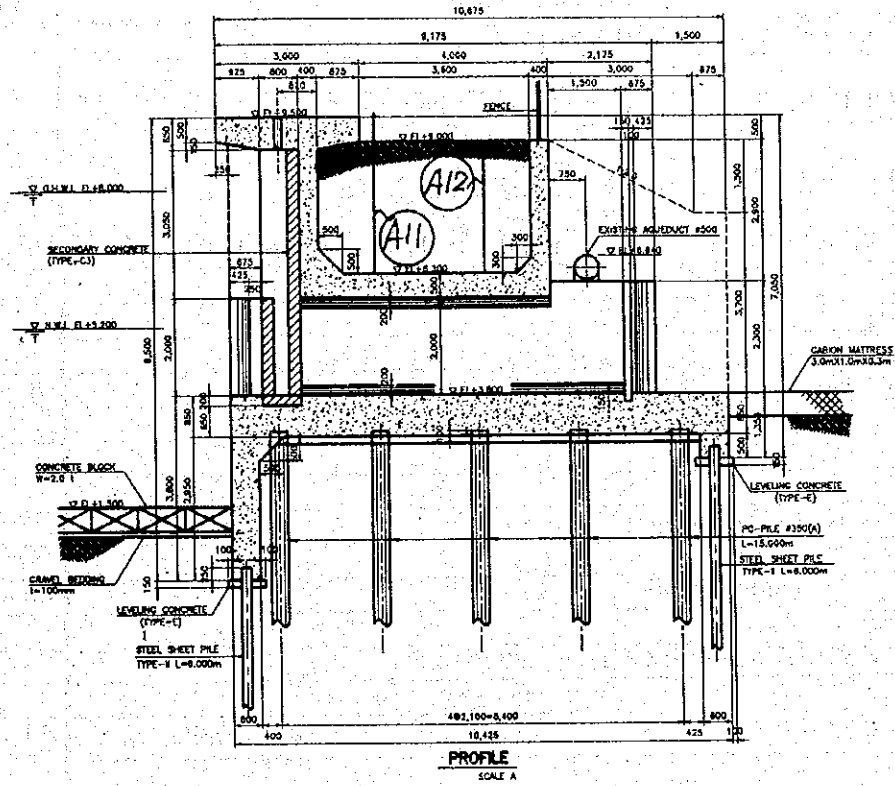
EXPLANATORY DRAWING



TYPE OF WORK : SCAFFOLDING
 LOCATION : INTAKE WEIR RIGHT

(2/2)

EXPLANATORY DRAWING



TYPE OF WORK : PILE HEAD TREATMENT
 LOCATION : INTAKE STRUCTURE
 : RIGHT BANK

CALCULATION	RESULT
PC PILE DIA 350 (A) n = 40 PILES	
LENGTH OF DESIGN : $L_1 = 14.00$ m/pile	
SPARE PILE LENGTH : $L_2 = 1.00$ m	
ADOPTED PILE LENGTH : $L = 14.00 + 1.00 = 15.00$ m/pile	40 places
JOINTING	
n = 1 place/pile	40 places
PILING	
N VALUE: N = 22 (Average)	
D = 14.00 m/pile	40 places
CONCRETE FILLING (TYPE-C1) n = 40 PILES	
$V = \frac{\pi}{4} \times 0.22^2 \times 0.55 = 0.021$ m ³ /pile	40 places
SUSPENDED FORM	
$A = \frac{\pi}{4} \times 0.22^2 = 0.038$ m ³ /pile	40 places
REINFORCING BAR	
D 13 (W = 1.04 kg/m)	
$W_1 = 6 \text{ Bars} \times 0.45 \times 1.04 = 2.808$	
$W_2 = 4 \text{ Bars} \times 0.74 \times 1.04 = 3.078$	
TOTAL W = 5.886 kg.f/pile	40 places
CUTTING PILE HEAD	
Height of cutting : h = 1.00 m/pile	
$V = \frac{\pi}{4} \times (0.35^2 - 0.40^2) \times 1.00 = 0.058$ m ³ /pile	40 places

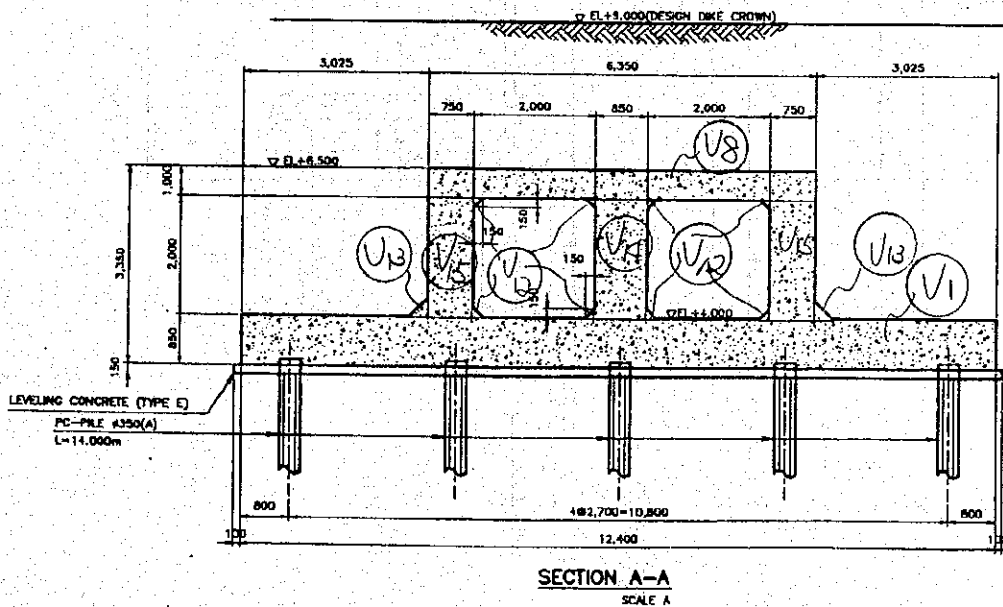
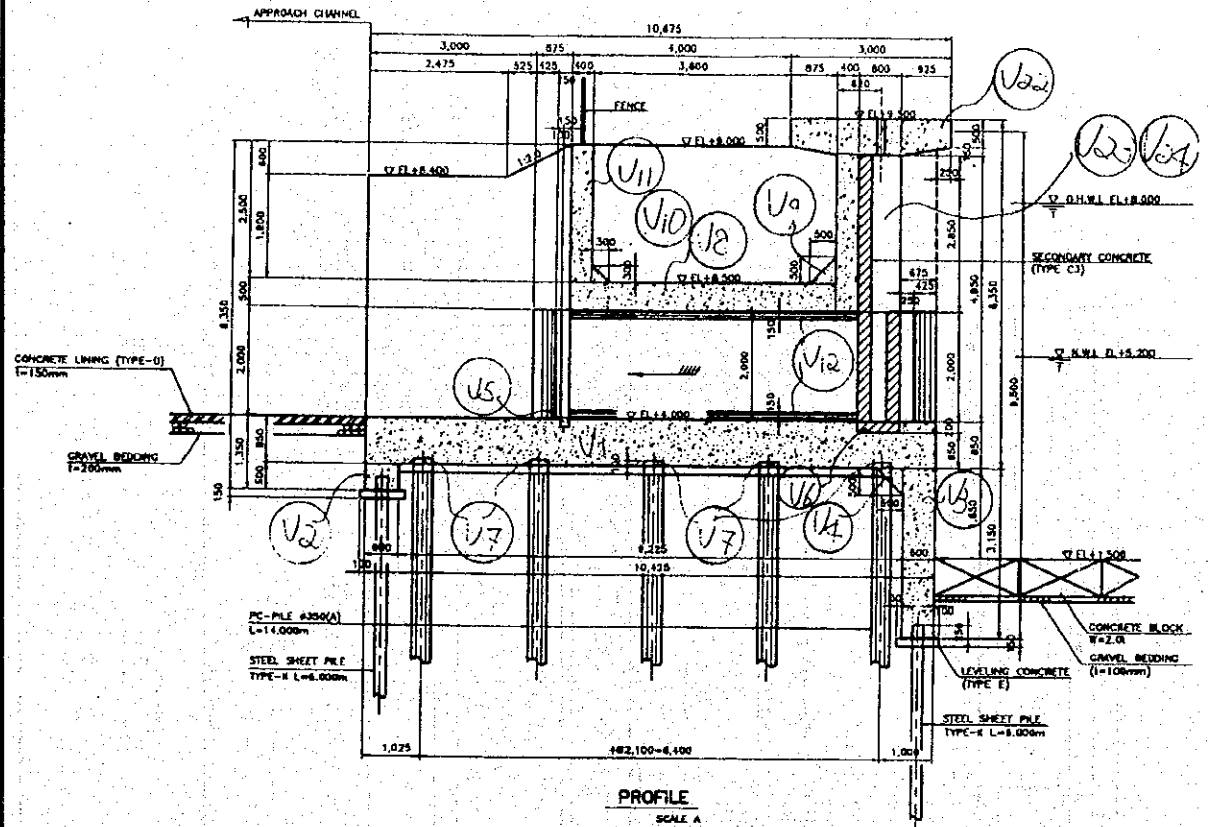
TYPE OF WORK :
 LOCATION : INTAKE WEIR LEFT

CALCULATION		RESULT
□ CONCRETE (TYPE - C1)		
$V_1 = 10.425 \times 12.4^{00} \times 0.85^0$	=	109.880
$V_2 = 0.500 \times 0.600 \times 12.400$	=	3.720
$V_3 = 0.600 \times 3.150 \times 12.400$	=	23.436
$V_4 = \frac{1}{2} \times 0.500 \times 0.500 \times 12.400$	=	1.55
$V_5 = -0.15 \times 0.15 \times 2.300 \times 2$	=	-0.104
$V_6 = -0.200 \times 0.800 \times 2.500 \times 2$	=	-0.800
$V_7 = -0.175 \times 0.175 \times 3.142 \times 0.1 \times 25$	=	-0.241
$V_8 = 0.5 \times 6.35 \times 4.875$	=	15.478
$V_9 = \frac{1}{2} \times 0.5^{00} \times 0.5^{00} \times 6.35^0$	=	0.794
$V_{10} = \frac{1}{2} \times 0.3^{00} \times 0.3^{00} \times 6.35^0$	=	0.286
$V_{11} = 0.4^{00} \times 2.5^{00} \times 6.35^0$	=	6.350
$V_{12} = \frac{1}{2} \times 0.15^{00} \times 0.15^{00} \times 5.275 \times 8$	=	0.475
$V_{13} = \frac{1}{2} \times 0.3^{00} \times 0.3^{00} \times (9.025 + 2.925 \times 2) \times 2$	=	1.339
$V_{14} = 0.85^0 \times 2.000 \times 4.875$	=	8.288
$V_{15} = 0.75^0 \times 2.000 \times 4.875 \times 2$	=	14.625
$V_{16} = 3.025 \times 0.65^0 \times 5.000 \times 2$	=	19.663
$V_{17} = 3.025 \times 0.55^0 \times 4.4^{00} \times 2$	=	14.641
$V_{18} = \{ \frac{1}{2} \times (4.4 + 5.0) \times 1.2 \times 0.75 + 4.4 \times 2.475 \times 0.75 - 0.25$ $\times 0.25 \times 4.4 + 0.25 \times 0.25 \times 3.142 \div 4 \times 4.4 \} \times 2$	=	24.677
$V_{19} = -0.15 \times 0.15 \times (5.0 - 0.075) \times 2$	=	-0.222
$V_{20} = (0.25 \times 0.85 + 0.425 \times 0.425 \times 3.142) \times 2$	=	1.560
$V_{21} = -0.15 \times 0.15 \times 20 \times 2$	=	-0.09
$V_{22} = (\frac{1}{2} \times (1.2 + 3.0) \times 0.15 + 0.5 \times 3.0) \times 6.350$	=	11.525
$V_{23} = (0.85^0 \times 0.400 + 0.8^{00} \times 0.35^0) \times 2.85^0$	=	1.767
$V_{24} = (0.75 \times 0.4 + 0.8 \times 0.5) \times 4.85 + (0.425 \times 0.75 + 0.25 \times 0.5^{00}$ $+ 0.25 \times 0.25 \times 3.142 \div 4)$	=	5.810
$V_{25} = (0.85 \times 0.4 + 0.8 \times 0.35^0 + 0.25^0 \times 0.85^0 + 0.425 \times 0.425$ $\times 3.142 \div 2) \times 2.0$	=	2.233
$V_{26} = -0.125 \times 0.125 \times 3.142 \times 0.65 \times 2.0$	=	-0.064
TOTAL	=	226.576
		226.576 m³
□ LEVELLING CONCRETE (TYPE - E)		
(TYPE - E)		
$V_1 = 8.975 \times 12.6 \times 0.15$	=	16.963
$V_2 = \frac{1}{2} \times 0.15 \times 0.15 \times 12.6$	=	0.142
$V_3 = 0.8 \times 12.6 \times 0.15$	=	1.512
$V_4 = 0.8 \times 12.6 \times 0.15$	=	1.512
$V_5 = -0.175 \times 0.175 \times 3.142 \times 0.15 \times 25$	=	-0.361
TOTAL	=	19.768
		19.768 m³

TYPE OF WORK : CONCRETE (TYPE-C1)
 LOCATION : INTAKE WEIR LEFT

(2/2)

EXPLANATORY DRAWING



TYPE OF WORK : FORM (H ≥ 4.0 m)
 LOCATION : INTAKE WEIR LEFT

CALCULATION		RESULT
$A_4 = (3.150 + 0.850) \times 12.400$	=	49.600
$A_5 = (\sqrt{2} \times 0.500 + 2.650) \times 12.400$	=	41.628
$A_6 = 5.000 \times 3.525 \times 2$	=	35.250
$A_7 = (\sqrt{2} \times 0.300 + 4.700) \times 3.025 \times 2$	=	31.002
$A_8 = 5.15 \times 3.775 \times 2$	=	38.883
$A_9 = (\sqrt{2} \times 0.300 + 4.100) \times 3.025 \times 2$	=	27.372
$A_{10} = (\sqrt{2} \times 0.300 + 2.200) \times 4.475 \times 2$	=	23.487
$A_{11} = \frac{1}{2} \times 0.5^{00} \times 0.5^{00} \times 2$	=	0.250
$A_{12} = \frac{1}{2} \times 0.3^{00} \times 0.3^{00} \times 2$	=	0.090
$A_{13} = \frac{1}{2} \times 0.3^{00} \times 0.3^{00} \times (9.025 + 2.925 \times 2) \times 2$	=	0.400
$A_{14} = 0.425 \times 2 \times 3.142 \div 2 \times 2.000$	=	2.671
$A_{15} = (0.150 \times 2 + 2.300 \times 2) \times 0.150 \times 2 + 0.150 \times 3 \times 2.000$ $\times 2 + 0.15 \times 3 \times 4.963 \times 2$	=	7.737
$A_{16} = \frac{1}{2} \times 0.150 \times 0.150 \times 4$	=	0.045
$A_{17} = 3.000 \times 6.300$	=	18.900
$A_{18} = (\sqrt{2} \times 0.3 + 4.700) \times 0.400 \times 2$	=	4.099
$A_{19} = (4.400 \times 2.475 + \frac{1}{2} \times (4.400 + 4.925) \times 1.05) \times 2$	=	31.571
$A_{20} = \{(\sqrt{2} \times 0.300 + 4.100) \times 1.975 + (\sqrt{2} \times 0.3 + 4.1) \times 2$ $+ 0.6) \times 1.200 \times \frac{1}{2}\}$	=	29.449
$A_{21} = (0.550 \times 5.250 + 0.500 \times 0.600 + \frac{1}{2} \times (0.85 + 1.15))$	=	6.975
$A_{22} = (\sqrt{2} \times 0.300 + 5.550) \times 1.475 + 0.500 \times 3.00 + \frac{1}{2} \times 0.15$ $\times 0.875 + \frac{1}{2} \times 0.146 \times 0.9000) \times 2$	=	20.887
$A_{23} = (0.25^0 \times 2 \times 3.142 \div 4 \times 4.850 \times 2$	=	3.810
$A_{24} = 0.425 \times 4.85 \times 2$	=	4.123
$A_{25} = (0.250 \times 2 + 0.425 \times 3.142 \div 2 + 0.25 \times 2) \times 2$	=	4.671
$A_{26} = ((0.250 + 0.800) \times 4 + 0.250 \times 2$	=	22.795
$A_{27} = (0.800 \times 2 + 2.500 \times 2) \times 0.200 \times 2$	=	2.640
$A_{28} = 0.125 \times 0.125 \times 3.142 \times 0.650 \times 2$	=	0.064
$A_{29} = (\sqrt{2} \times 0.500 + 1.85) \times 6.350$	=	16.238
$A_{30} = \{(\sqrt{2} \times 0.300 + 2.05) \times 6.350$	=	15.712
$A_{31} = (0.650 \times 5.85 + 0.600 \times 3.150 + \frac{1}{2} \times 0.300 \times 0.300 + \frac{1}{2} \times 0.500$ $\times 0.500 + 0.850 \times 0.300$	=	6.118
TOTAL		= 133.482