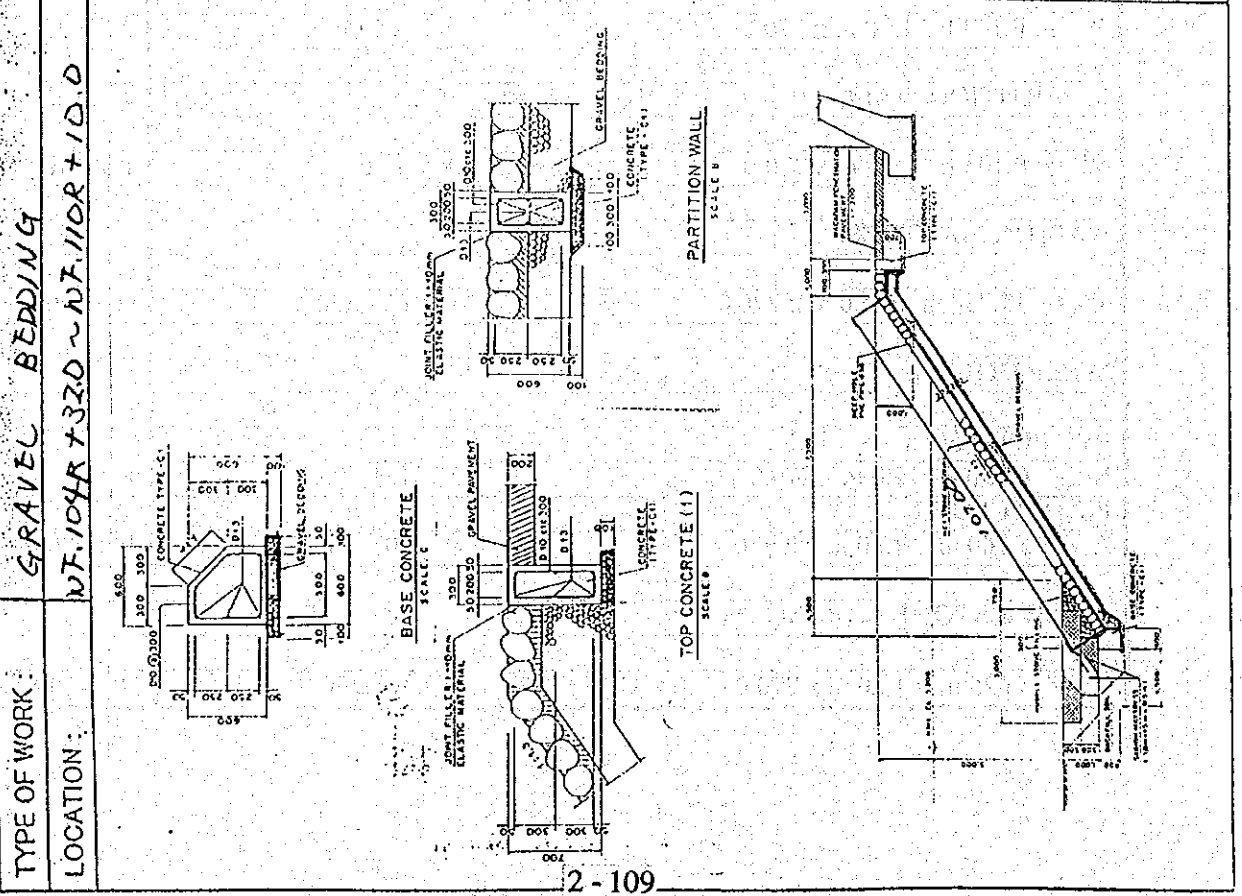


TYPE OF WORK : REVETMENT FOR SLOPE OF 1:1.5 (WET STONE MASONRY TYPE)  
 LOCATION : WF.104R + 32.0 ~ WF.110R + 10.0 m

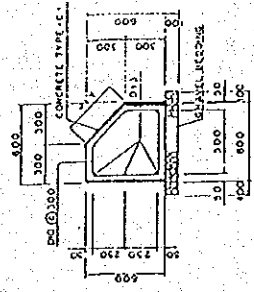
CALCULATION	RESULT
<b>STRUCTURAL EXCAVATION</b>	
$V = 176.850 \text{ m}^3 / 15.0 \text{ m} \times 270.00$	$= 3183.300$
	3183.300 m <sup>3</sup>
<b>BACKFILL WITH SELECTED SOIL</b>	
$V = 17.445 \text{ m}^3 / 15.0 \text{ m} \times 270.00$	$= 314.010$
	314.010 m <sup>3</sup>
<b>WET STONE MASONRY</b>	
$V = 50.789 \text{ m}^3 / 15.0 \text{ m} \times 270.0$	$= 914.202$
	914.202 m <sup>3</sup>
<b>CEMENT MORTAR POINTING</b>	
$A = 142.796 \text{ m}^2 / 15.0 \text{ m} \times 270.0$	$= 2570.328$
	2570.328 m <sup>2</sup>
<b>DEFORMED REINFORCING BAR</b>	
1. BASE CONCRETE	
$W = 0.151 \text{ tf} / 15.0 \text{ m} \times 270.0$	$= 2.718$
	2.718 tf
2. TOP CONCRETE	
$W = 0.143 \text{ tf} / 15.0 \text{ m} \times 270.0$	$= 2.574$
	2.574 tf
3. PARTITION WALL	
$W = 0.103 \text{ tf} / 15.0 \text{ m} \times 270.0$	$= 1.854$
	1.854 tf
<b>GABION MATTRESS</b>	
$V = 33.750 \text{ m}^3 / 15.0 \text{ m} \times 270.0$	$= 607.500$
	607.500 m <sup>3</sup>
<b>WEEP HOLE</b>	
PVC PIPE $\phi$ 50, n = 162	
$L = 6.750 \text{ m} / 15.0 \text{ m} \times 270.0$	$= 121.500$
	121.500 m
<b>FILTER CLOTH</b>	
$A = 5.760 \text{ m}^2 / 15.0 \text{ m} \times 270.0$	$= 103.680$
	103.680 m <sup>2</sup>
<b>RUBBLE STONE FILLING</b>	
$V = 5.625 \text{ m}^3 / 15.0 \text{ m} \times 270.0$	$= 101.250$
	101.250 m <sup>3</sup>

RETAINMENT FOR SLOPE OF 1:1.5 (WET STONE MASONRY TYPE)

TYPE OF WORK	CALCULATION	RESULT
GRAVEL BEDDING		
LOCATION: NT. 104R + 320 ~ NT. 110R + 10.0		
① RETAINMENT	$V = 49.409 \text{ m}^3 / 15.0 \text{ m} \times 270.0 \text{ m} = 889.362$	889.362 m <sup>3</sup>
② BASE CONCRETE	$V = 1.200 \text{ m}^3 / 15.0 \text{ m} \times 270.0 \text{ m} = 21.6$	21.600 m <sup>3</sup>
③ TOP CONCRETE	$V = 0.600 \text{ m}^3 / 15.0 \text{ m} \times 270.0 \text{ m} = 10.8$	10.800 m <sup>3</sup>
④ PARTITION WALL	$V = 0.422 \text{ m}^3 / 15.0 \text{ m} \times 270.0 \text{ m} = 7.596$	7.596 m <sup>3</sup>



REINFORCEMENT FOR SLOPE OF 1:1.5 (WET STONE MASONRY TYPE)

TYPE OF WORK: LOCATION:	CALCULATION	RESULT
CONCRETE FORM WT. 104R 732.0 ~ WT. 110R + 100		
	<p>CONCRETE</p> <p>① BASE CONCRETE  <math>3.150 \text{ m}^2 / 15.0 \text{ m} \times 270 \text{ m} = 56.700</math></p> <p>② TOP CONCRETE  <math>4.725 \text{ m}^2 / 15.0 \text{ m} \times 270 \text{ m} = 85.050</math></p> <p>③ PARTITION WALL  <math>2.073 \text{ m}^2 / 15.0 \text{ m} \times 270 \text{ m} = 37.314</math></p> <p>FORM</p> <p>① BASE CONCRETE  <math>21.210 \text{ m}^2 / 15.0 \text{ m} \times 270.0 \text{ m} = 381.78</math></p> <p>② TOP CONCRETE  <math>20.175 \text{ m}^2 / 15.0 \text{ m} \times 270.0 \text{ m} = 363.15</math></p> <p>③ PARTITION WALL  <math>13.820 \text{ m}^2 / 15.0 \text{ m} \times 270.0 \text{ m} = 248.76</math></p>	<p>56.700 m<sup>3</sup></p> <p>85.050 m<sup>3</sup></p> <p>37.314 m<sup>3</sup></p> <p>381.780 m<sup>2</sup></p> <p>363.150 m<sup>2</sup></p> <p>248.760 m<sup>2</sup></p>

BASE CONCRETE  
SCALE: C

TOP CONCRETE (1)  
SCALE: B

PARTITION WALL  
SCALE: C

REVEEMENT FOR SLOPE OF 1:1.5 (WET STONE MASONRY TYPE)

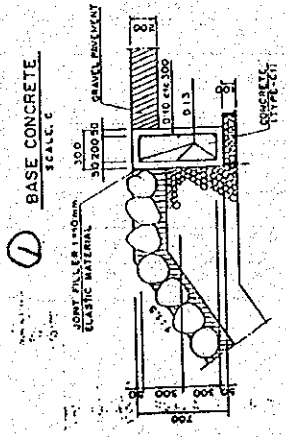
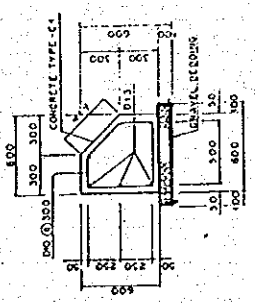
TYPE OF WORK :	JOINT FILLER	CALCULATION	RESULT
LOCATION :		<p>① TOP CONCRETE</p> $A = 4.620 \text{ m}^2 / 15.0 \text{ m} \times 270.0 \text{ m} = 83.160$ <p>② PARTITION WALL</p> $A = 3.455 \text{ m}^2 / 15.0 \text{ m} \times 270.0 \text{ m} = 62.19$ <p>③ BASE CONCRETE</p> $a = (0.3 + 0.6) \times 0.3 \div 2 + 0.3 \times 0.6$ $= 0.315 \text{ m}^2 / \text{piece}$ $A = 0.315 \times 270.0 \div 15 = 5.670$	<p>83.160m<sup>2</sup></p> <p>62.190m<sup>2</sup></p> <p>5.670m<sup>2</sup></p>

TYPE OF WORK : REVETMENT FOR SLOPE OF 1:1.5  
LOCATION : WF.176R ~ WF.180R (LOWER CHANNEL)

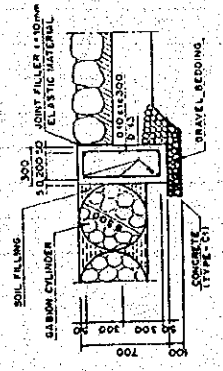
CALCULATION	RESULT
<b>STRUCTURAL EXCAVATION</b>	
$V = 153.450 \text{ m}^3 / 15.0 \text{ m} \times 175.00 \text{ m} = 1790.250$	1790.250 m <sup>3</sup>
<b>BACKFILL WITH SELECTED SOIL</b>	
$V = 21.690 \text{ m}^3 / 15.0 \text{ m} \times 175.00 \text{ m} = 253.050$	253.050 m <sup>3</sup>
<b>WET STONE MASONRY</b>	
$V = 43.027 \text{ m}^3 / 15.0 \text{ m} \times 175.00 \text{ m} = 501.982$	501.982 m <sup>3</sup>
<b>CEMENT MORTAR POINTING</b>	
$A = 102.900 \text{ m}^2 / 15.0 \text{ m} \times 175.00 \text{ m} = 1200.500$	1200.500 m <sup>2</sup>
<b>GABION MATTRESS</b>	
$V = 33.750 \text{ m}^3 / 15.0 \text{ m} \times 175.00 \text{ m} = 393.750$	393.750 m <sup>3</sup>
<b>RUBBLE STONE FILLING</b>	
$V = 5.625 \text{ m}^3 / 15.0 \text{ m} \times 175.00 \text{ m} = 65.625$	65.625 m <sup>3</sup>

REVEIEMENT FOR SLOPE OF 1:1.5

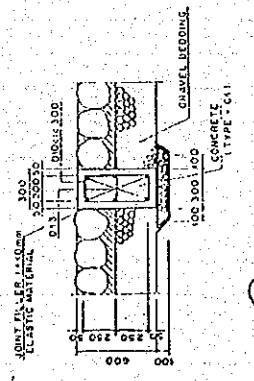
TYPE OF WORK:	CALCULATION		RESULT
GRAVEL BEDDING			
LOCATION:	WF. 176 F ~ WF. 180 R (LOWER CHANNEL)		
① REVEIEMENT	37.716 m <sup>3</sup> / 15.0 m x 175.0 = 440.02		440.020 m <sup>3</sup>
② BASE CONCRETE	1.200 m <sup>3</sup> / 15.0 m x 175.0 = 14.000		14.000 m <sup>3</sup>
③ TOP CONCRETE	0.600 m <sup>3</sup> / 15.0 m x 175.0 = 7.000		7.000 m <sup>3</sup>
④ PARTITION WALL	0.422 m <sup>3</sup> / 15.0 m x 175.0 = 4.923		4.923 m <sup>3</sup>
⑤ END WALL			
1) UPSTREAM	0.319 m <sup>3</sup> / place x 1 place = 0.319		
2) DOWN STREAM	0.393 m <sup>3</sup> / place x 1 place = 0.393		
	TOTAL		0.712 m <sup>3</sup>



③ TOP CONCRETE (11)  
SCALE 1/20



END WALL  
SCALE 1/20

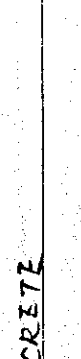
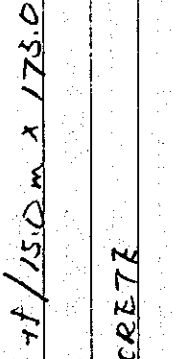
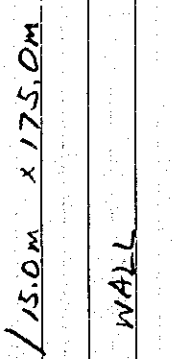
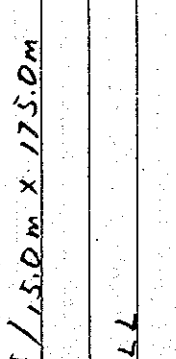


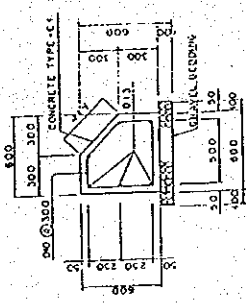
③ PARTITION WALL  
SCALE 1/20

RETAINMENT FOR SLOPE OF 1:1.5

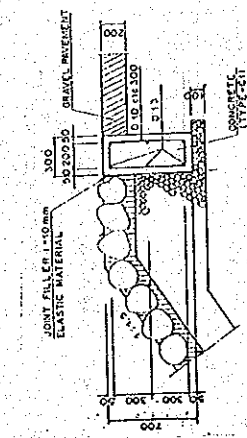
DEFORMED REINFORCING BARS

WF.176F ~ WF.176R (LOWER CHANNEL)

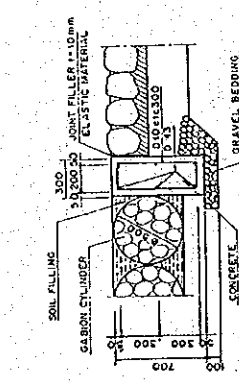
TYPE OF WORK :	CALCULATION	RESULT
LOCATION :		
 <p>① TOP CONCRETE (1) SCALE: 1:1</p>	<p>① BASE CONCRETE</p> <p><math>N = 0.151 \text{ tf} / 15.0 \text{ m} \times 175.0 \text{ m} = 1.762</math></p>	1.762 tf
 <p>② TOP CONCRETE</p>	<p><math>N = 0.143 \text{ tf} / 15.0 \text{ m} \times 175.0 \text{ m} = 1.668</math></p>	1.668 tf
 <p>③ PARTITION WALL SCALE: 1:1</p>	<p><math>N = 0.079 \text{ tf} / 15.0 \text{ m} \times 175.0 \text{ m} = 0.922</math></p>	0.922 tf
 <p>④ END WALL SCALE: 1:1</p>	<p><math>W = 0.080 \text{ tf} + 0.035 \text{ tf} = 0.115</math></p>	0.115 tf



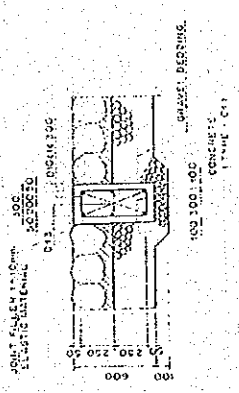
① TOP CONCRETE (1)  
SCALE: 1:1



② TOP CONCRETE  
SCALE: 1:1



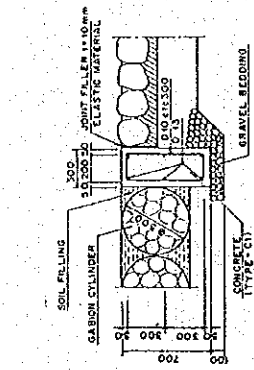
③ PARTITION WALL  
SCALE: 1:1



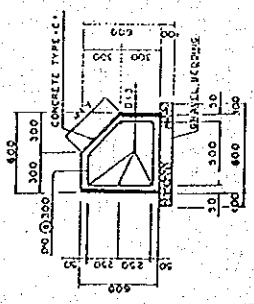
④ END WALL  
SCALE: 1:1

# RETAINMENT FOR SLOPE OF 1:1.5

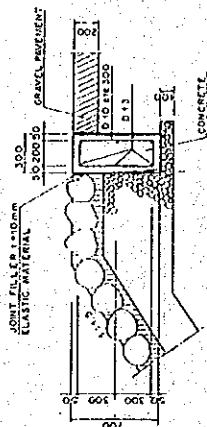
TYPE OF WORK:	CONCRETE, FORM	CALCULATION	RESULT
LOCATION:	WF. 1767 ~ WF. 1808 (LOWER CHANNEL)		
• CONCRETE			
① BASE CONCRETE			
		$4.725 \text{ m}^3 / 15.0 \text{ m} \times 175.0 = 55.125$	55.125 m <sup>3</sup>
② TOP CONCRETE			
		$3.150 \text{ m}^3 / 15.0 \text{ m} \times 175.0 = 36.750$	36.750 m <sup>3</sup>
③ PARTITION WALL			
		$1.553 \text{ m}^3 / 15.0 \text{ m} \times 175.0 = 18.118$	18.118 m <sup>3</sup>
④ END WALL			
		$1.762 \text{ m}^3 + 2,173 \text{ m}^3 = 3,935$	3,935 m <sup>3</sup>
• FORM			
① BASE CONCRETE			
		$20.175 \text{ m}^2 / 15.0 \text{ m} \times 175.0 = 235.375$	235.375 m <sup>2</sup>
② TOP CONCRETE			
		$21.120 \text{ m}^2 / 15.0 \text{ m} \times 175.0 = 246.4$	246.400 m <sup>2</sup>
③ PARTITION WALL			
		$10.350 \text{ m}^2 / 15.0 \text{ m} \times 175.0 = 120.92$	120.820 m <sup>2</sup>
④ END WALL			
		$10.060 \text{ m}^2 + 14.484 \text{ m}^2 = 24.544$	24.544 m <sup>2</sup>



END WALL  
SCALE 2



TOP CONCRETE (1)  
SCALE 1

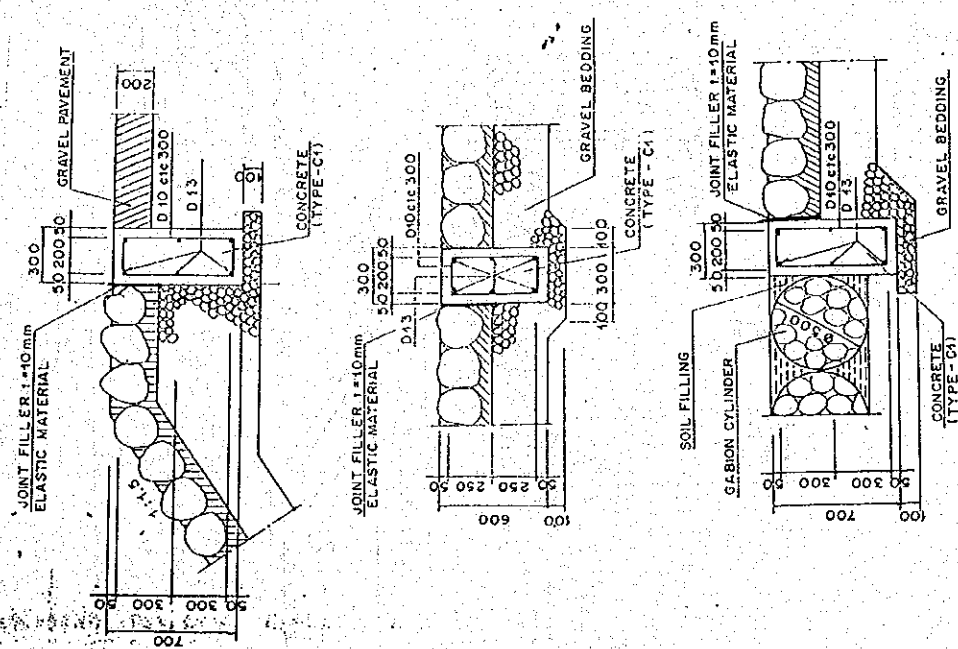


PARTITION WALL  
SCALE 1



REINFORCEMENT FOR SLOPE OF 1:1.5

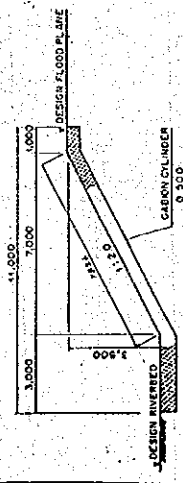
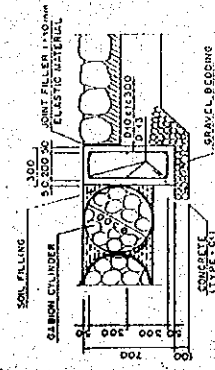
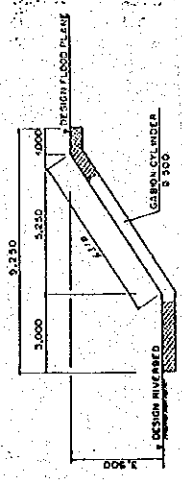
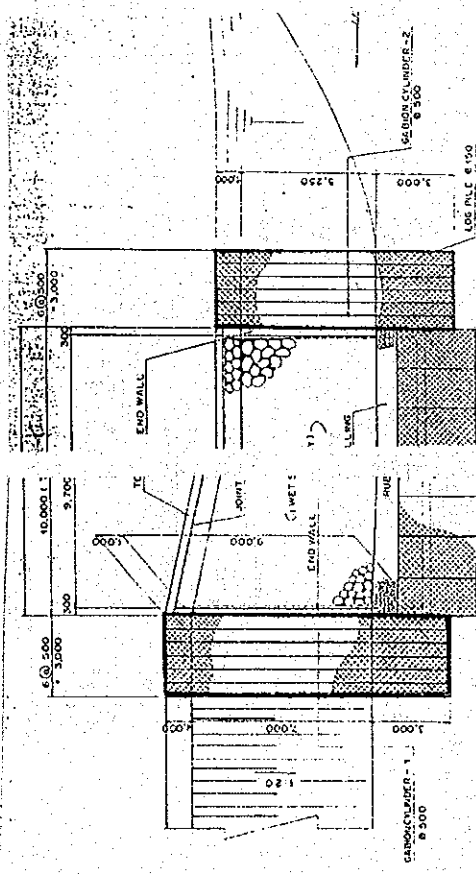
TYPE OF WORK :	CALCULATION	RESULT
JOINT FILLER		
LOCATION :	WF. 176 E ~ WF. 180 R (LOWER CHANNEL)	
① TOP CONCRETE	$A = 4.620 \text{ m}^2 / 15.0 \text{ m} \times 175.0 \text{ m} = 53.9$	33.900 m <sup>2</sup>
② BASE CONCRETE	$A = 0.315 \text{ m}^2 / 15.0 \text{ m} \times 175.0 \text{ m} = 3.675$	3.675 m <sup>2</sup>
③ PARTITION WALL		
④ END WALL	$A = 2.644 \text{ m}^2 / 15.0 \text{ m} \times 175.0 \text{ m} = 30.887$	30.847 m <sup>2</sup>
	$A = 2.517 \text{ m}^2 + 3.160 \text{ m}^2 = 5.677$	5.677 m <sup>2</sup>



END WALL  
SCALE 8

RETAINMENT FOR SLOPE OF 1:1.5  
 Gabion Cylinder Dia. 500mm  
 Galvanized and coated with PVC  
 WF. 1767 ~ WF. 18DR (LOWER CHANNEL)

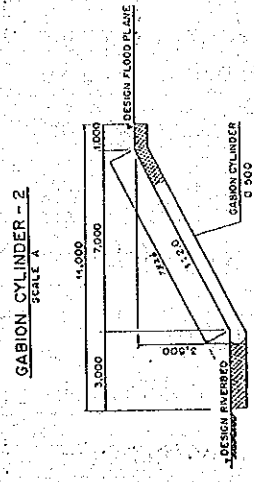
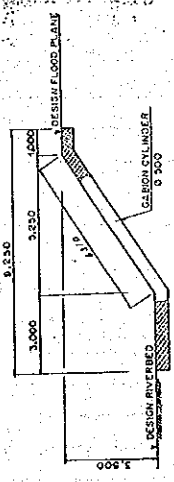
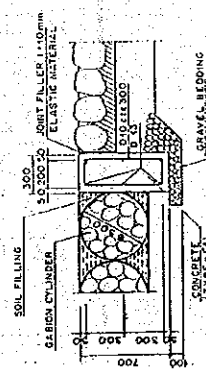
TYPE OF WORK :	LOCATION :	CALCULATION	RESULT
	1) Gabion Cylinder - 1	$L = 3.0 + 2.826 + 1.0$ $n = 6$ $L = 11.826 \times 6$ $a = \pi \times 0.5^2 \div 4$ $V = 70.956 \times 0.196$	$= 11.826 \text{ m}$ $= 70.956 \text{ m}$ $= 0.196 \text{ m}^2$ $= 13.907 \text{ m}^3$
	2) Gabion Cylinder - 2	$L = 3.0 + 6.310 + 1.0$ $n = 6$ $L = 10.310 \times 6$ $a = \pi \times 0.5^2 \div 4$ $V = 61.860 \times 0.196$	$= 10.310 \text{ m}$ $= 61.860 \text{ m}$ $= 0.196 \text{ m}^2$ $= 12.125 \text{ m}^3$
		<b>TOTAL</b>	<b>26.032 m<sup>3</sup></b>



GABION CYLINDER - 1  
SCALE: A

REVETMENT FOR SLOPE OF 1:1.5

TYPE OF WORK :	CALCULATION	RESULT
Soil Filling		
W.F. 176F ~ WF. 180R (LOWER CHANNEL)		
1) For Gabion Cylinder - 1		
	$V_1 = (3.0 + 7.826 + 1.0) \times 0.5 \times 3.0 = 17.739 \text{ m}^3$	
	$V_2 = 13.907 \text{ m}^3$	
	$\times V_2$ is volume of cylinder	
	$V = V_1 - V_2 = 17.739 - 13.907 = 3.832 \text{ m}^3$	3.832 m <sup>3</sup>
2) For Gabion Cylinder - 2		
	$V_1 = (3.0 + 6.810 + 1.0) \times 0.5 \times 3.0 = 15.465 \text{ m}^3$	15.465 m <sup>3</sup>
	$V_2 = 12.125 \text{ m}^3$	12.125 m <sup>3</sup>
	$V = V_1 - V_2 = 15.465 - 12.125 = 3.340 \text{ m}^3$	3.340 m <sup>3</sup>
	TOTAL	7.172 m <sup>3</sup>



## 2.3 Stone Facing Type

TYPE OF WORK : REVETMENT FOR SIDE SLOP OF 1:2 (STONE FACING)  
 LOCATION : WF.OR - 65.0 ~ WF.OR + 32.0

CALCULATION	RESULT
<b>LOCATION : WF.OR - 65.0 ~ WF.OR + 32.0</b>	
<b>☐ STRUCTURAL EXCAVATION</b>	
$V = \frac{514.147}{\text{BEDDING}} + \frac{1029.17}{\text{FACING}}$	$= 1543.32$
	<b>1543.32 m<sup>3</sup></b>
<b>☐ RUBBLE STONE BEDDING</b>	
$A_1 = (7.379 + 2.50 + 3.019) \times 0.40$	$= 5.159 \text{ m}^2$
$A_2 = (1.00 + 5.814) \times 0.40 \times 2$	$= 5.451 \text{ m}^2$
$V_1 = 5.159 \times 50.00$	$= 257.950$
$V_2 = 5.451 \times 47.00$	$= 514.147$
<b>TOTAL</b>	<b>= 514.147</b>
	<b>514.147 m<sup>3</sup></b>
<b>☐ STONE FACING</b>	
$A_1 = (2.00 + 7.379 + 2.50 + 3.019 + 2.00) \times 0.60$	$= 10.139 \text{ m}^2$
$A_2 = \{(1.60 + 5.159) \times 2 + 5.00\} \times 0.60$	$= 11.111 \text{ m}^2$
$V_1 = 10.139 \times 50.00$	$= 506.950$
$V_2 = 11.111 \times 47.00$	$= 522.217$
<b>TOTAL</b>	<b>= 1029.17</b>
	<b>1029.17 m<sup>3</sup></b>
<b>LOCATION : WF.4R + 16.0 ~ WF.5R + 51.0</b>	
<b>☐ STRUCTURAL EXCAVATION</b>	
$V = \frac{268.30}{\text{BEDDING}} + \frac{618.50}{\text{FACING}}$	$= 886.80$
	<b>886.80 m<sup>3</sup></b>
<b>☐ RUBBLE STONE BEDDING</b>	
$A = 1.118 \times 6.00 \times 0.40$	$= 2.683 \text{ m}^2$
$V = 2.683 \times 100.00$	$= 268.30$
	<b>268.30 m<sup>3</sup></b>
<b>☐ STONE FACING</b>	
$A = (2.00 + 6.708 + 1.00 + 0.60) \times 0.60$	$= 6.185 \text{ m}^2$
$V = 6.185 \times 100.00$	$= 618.50$
	<b>618.50 m<sup>3</sup></b>

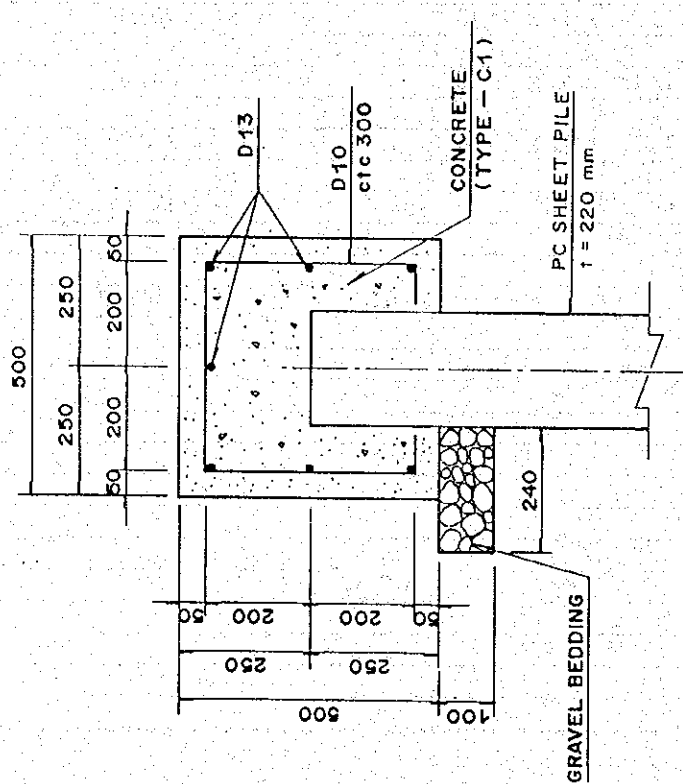
## 2.4 PC Sheet Pile Wall Type

TYPE OF WORK : PC SHEET PILE WALL TYPE REVETMENT  
 LOCATION : WF.31L + 32.8 ~ WF.38L + 25.1

CALCULATION	RESULT
<b>☑ STRUCTURAL EXCAVATION</b>	
$V_1 = \frac{1}{2} \times 0.31 \times 19.964$	= 3.09
$V_2 = 0.31 \times 53.47 \times \frac{1}{2}$	= 8.29
<b>TOTAL</b>	<b>= 11.38</b>
	<b>11.4 m<sup>3</sup></b>
<b>☑ BACKFILL WITH SELECTED SOIL</b>	
$V_1 = \frac{1}{2} \times 0.34 \text{ m}^2 \times 19.964$	= 3.39
$V_2 = (0.34 + 11.56) \times \frac{1}{2} \times 53.47$	= 318.15
$V_3 = (11.56 + 15.00) \times \frac{1}{2} \times 54.30$	= 721.15
$V_4 = (15.00 + 17.86) \times \frac{1}{2} \times 34.97$	= 722.43
$V_5 = (17.86 + 13.98) \times \frac{1}{2} \times 44.13$	= 702.55
$V_6 = (13.98 + 12.13) \times \frac{1}{2} \times 49.15$	= 641.65
$V_7 = (12.13 + 3.83) \times \frac{1}{2} \times 48.96$	= 390.70
$V_8 = \frac{1}{2} \times 3.83 \text{ m}^2 \times 24.942$	= 47.76
<b>TOTAL</b>	<b>= 3547.73</b>
	<b>3547.73 m<sup>3</sup></b>

PC SHEET PILE WALL TYPE REVETMENT

TYPE OF WORK :	CALCULATION	RESULT
COPING CONCRETE		
LOCATION : WF. 31L + 32.8 ~ WF. 38L + 25.1		
	<p>• CONCRETE (TYPE - C1)</p>	
	$V = 2.50 \frac{m^3}{10.00m} \times (96.0 + 8.50 \times 2 + 223.50) = 84.125$	84.125 m <sup>3</sup>
	<p>• GRAVEL BEDDING</p>	
	$V = 0.24 \frac{m^3}{10.00m} \times 336.50 = 8.076$	8.076 m <sup>3</sup>
	<p>• FORM (H &lt; 4.0m)</p>	
	$A = 11.65 \frac{m^2}{10.00m} \times 336.50 = 392.023$	392.023 m <sup>2</sup>
	<p>• REINFORCING BAR</p>	
	$W = 0.10 \frac{t^3}{10.00m} \times 336.50 = 3.365$	3.365 t
	<p>• JOINT FILLER</p>	
	$A = 0.25 \frac{m^2}{10.00m} \times 336.50 = 8.413$	8.413 m <sup>2</sup>



TYPE OF WORK : PC SHEET PILE WALL TYPE REVETMENT  
 LOCATION : WF.31L + 32.8 ~ WF.38L + 25.1

CALCULATION			RESULT
<b>☐ STRUCTURAL EXCAVATION</b>			
$V_1 = \frac{1}{2} \times 0.31 \times 19.964$	=	3.09	
$V_2 = 0.31 \times 53.47 \times \frac{1}{2}$	=	8.29	24.620 m <sup>3</sup>
TOTAL			11.4 m <sup>3</sup>
<b>☐ PC SHEET PILE</b>			
• PC SHEET PILE			
Length of pile L = 9.00 m / pile			
$L = 9.00 \text{ m / pile} \times (4.47 + 192 + 17 \times 2)$	=	6057.00	6057.00 m
• DRIVING			
Driving Length of pile L = 5.00 m (Average)			
$L = 5.00 \text{ m / pile} \times (447 + 192 + 17 \times 2)$	=	3365.00	3365.00 m
<b>☐ GABION MATTRESS</b>			
t = 300 with Soil Covering			
$V_1 = 1.75 \times 0.30 \times 203.50$	=	106.838	
$V_2 = 1.75 \times 0.30 \times 71.00$	=	37.275	
TOTAL			114.113 m <sup>3</sup>

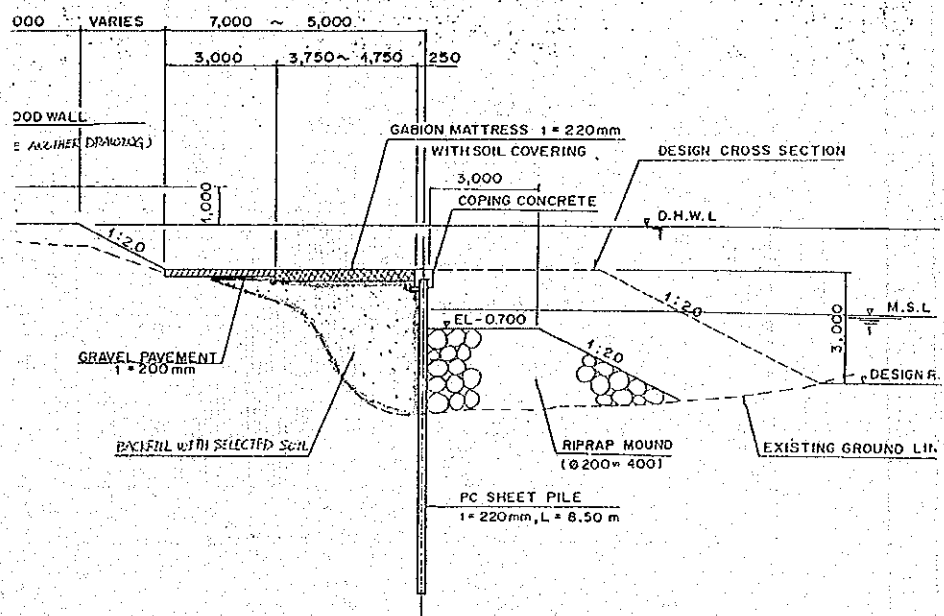
PC SHEET PILE WALL TYPE REVETMENT

TYPE OF WORK:	CALCULATION	RESULT
RIPRAP MOUND		
LOCATION: W.F. 312 + 32.8 ~ W.F. 381 + 25.1		
	$V_1 = \frac{1}{2} \times 7.10 \text{ m}^2 \times 19.964 = 70.87$	
	$V_2 = (7.10 + 10.59) \times \frac{1}{2} \times 53.47 = 472.94$	
	$V_3 = (10.59 + 10.00) \times \frac{1}{2} \times 54.30 = 559.02$	
	$V_4 = (10.00 + 10.74) \times \frac{1}{2} \times 43.97 = 455.97$	
	$V_5 = (10.74 + 10.74) \times \frac{1}{2} \times 44.13 = 473.96$	
	$V_6 = (10.74 + 8.51) \times \frac{1}{2} \times 49.15 = 473.07$	
	$V_7 = (8.51 + 4.69) \times \frac{1}{2} \times 48.96 = 323.74$	
	$V_8 = \frac{1}{2} \times 4.69 \text{ m}^2 \times 24.942 = 58.49$	
	TOTAL = 2887.46	2887.46 m <sup>3</sup>



PC SHEET PILE WALL TYPE REVETMENT

TYPE OF WORK:	BACKFILL WITH SELECTED SOIL	RESULT
LOCATION:	WF. 451 + 31.30 ~ WF. 511 + 29.75	
	$V_1 = \frac{1}{2} \times 2.30 \text{ m}^2 \times 20.00 = 23.00$	
	$V_2 = (2.3 + 8.1) \times \frac{1}{2} \times 47.14 = 245.13$	
	$V_3 = (8.1 + 12.19) \times \frac{1}{2} \times 51.54 = 522.87$	
	$V_4 = (12.19 + 8.38) \times \frac{1}{2} \times 51.74 = 532.15$	
	$V_5 = (8.38 + 10.99) \times \frac{1}{2} \times 48.62 = 470.88$	
	$V_6 = (10.99 + 3.66) \times \frac{1}{2} \times 48.21 = 353.14$	
	$V_7 = \frac{1}{2} \times 3.66 \text{ m}^2 \times 29.75 = 54.44$	
	TOTAL = 2201.61	2201.61 <sup>3</sup>

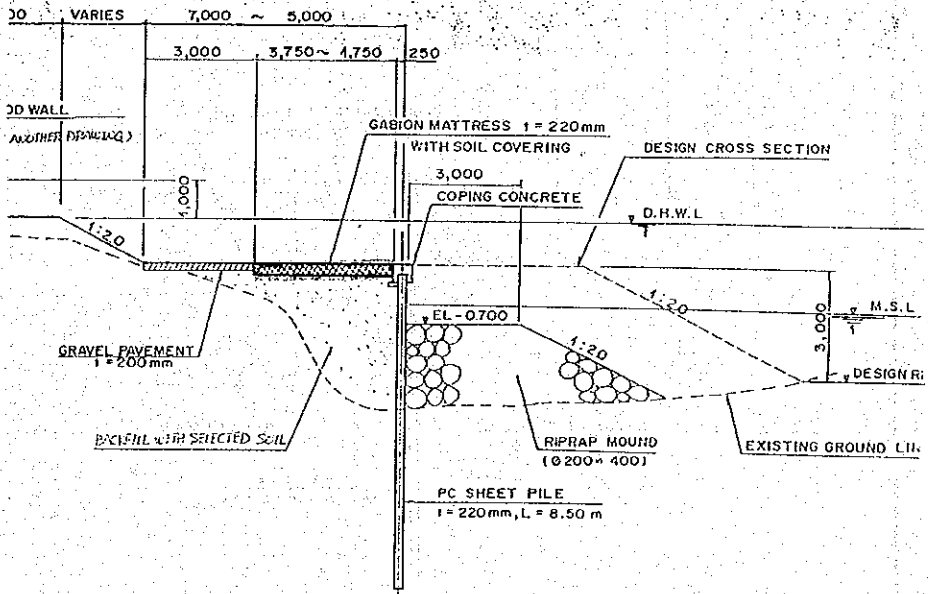


PC SHEET PILE WALL TYPE REVETMENT

TYPE OF WORK :	CALCULATION	RESULT
PC SHEET PILE		
LOCATION : WF. 452 + 31.30 ~ WF. 511 + 29.75		
	- PC SHEET PILE	
	Length of pile : L = 8.50 m / pile	
	$L = 8.50 \text{ m/pile} \times 594 \text{ piles} = 5049.00$	5049.00m
	- DRIVING	
	Driving length of pile : L = 5.00 m (Average)	
	$L = 5.00 \text{ m/pile} \times 594 \text{ piles} = 2970.00$	2970.00m

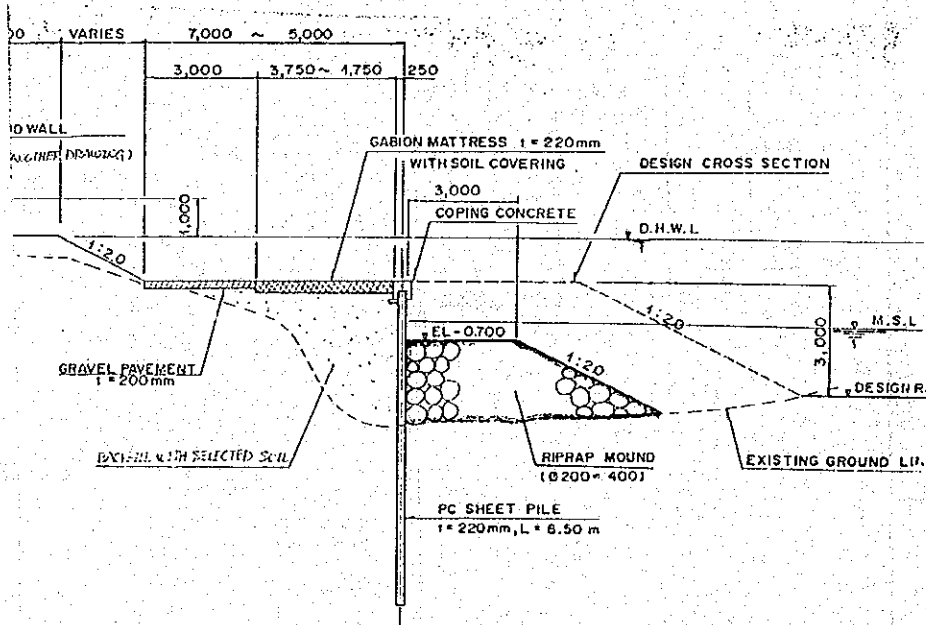
PC SHEET PILE WALL TYPE REVETMENT

TYPE OF WORK :	GABION MATTRESS		RESULT
LOCATION :	WF. 451 + 31.30 ~ WF. 511 + 29.75		
	t = 300 with Soil Covering		
	$V_1 = 3.75 \times 0.30 \times 240.00$		
	$= 270.000$		
	$V_2 = 1.75 \times 0.30 \times 57.00$		
	$= 29.070$		
	TOTAL = 299.070		299.070m



PC SHEET PILE WALL TYPE REVEMENT

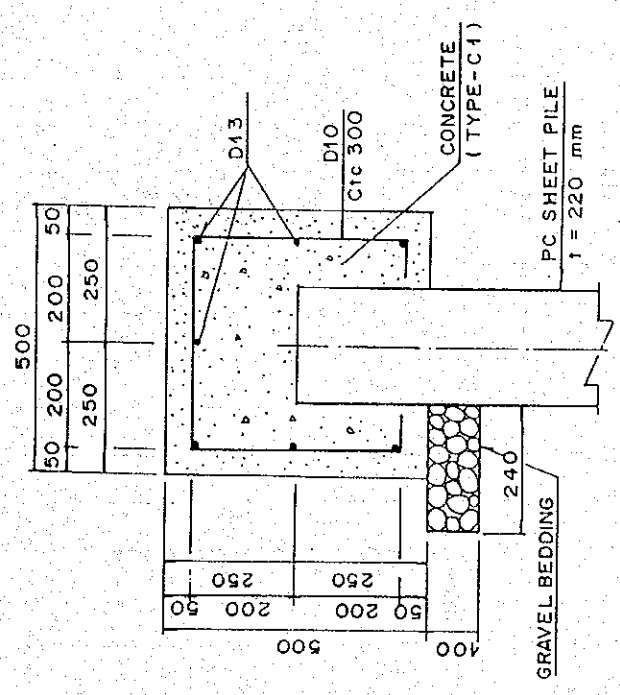
TYPE OF WORK:	CALCULATION	RESULT
RIPRAP MOUND		
LOCATION: WF. 452 + 31.30 ~ WF. 511 + 29.75		
	$V_1 = \frac{1}{2} \times 4.43 \text{ m}^2 \times 20.00 = 44.30$	
	$V_2 = (4.43 + 9.59) \times \frac{1}{2} \times 47.14 = 330.45$	
	$V_3 = (9.59 + 6.56) \times \frac{1}{2} \times 51.54 = 416.19$	
	$V_4 = (6.56 + 7.21) \times \frac{1}{2} \times 51.74 = 356.23$	
	$V_5 = (7.21 + 6.65) \times \frac{1}{2} \times 48.62 = 336.94$	
	$V_6 = (6.65 + 4.80) \times \frac{1}{2} \times 48.21 = 276.00$	
	$V_7 = \frac{1}{2} \times 4.80 \text{ m}^2 \times 29.75 = 71.40$	
	TOTAL = 1831.51	1831.51 m <sup>3</sup>



(1/2)

PC SHEET PILE WALL TYPE REVEITEMT

TYPE OF WORK:	COPING CONCRETE	CALCULATION	RESULT
LOCATION:	WF. 45L +3). 30 ~ WF. 51L +29.75		
• CONCRETE (TYPE - C1)			
$V = 0.50 \times 0.50 \times 10.00$		$= 2.500 \text{ m}^3 / 10.00 \text{ m}$	
$V' = 2.50 \text{ m}^3 / 10.00 \text{ m} \times 297.00$		$= 74.250$	74.250 m <sup>3</sup>
• GRAVEL BEDDING			
$V = 0.24 \times 0.10 \times 10.00$		$= 0.24 \text{ m}^3 / 10.00 \text{ m}$	
$V' = 0.24 \text{ m}^3 / 10.00 \text{ m} \times 297.00$		$= 7.128$	7.128 m <sup>3</sup>
• FORM (H < 4.0m)			
$A = (0.50 \times 2 + 0.14) \times 10.00 + 0.50 \times 0.50$			
$A' = 11.65 \text{ m}^2 / 10.00 \text{ m} \times 297.00$		$= 346.005$	346.005 m



TYPE OF WORK : PC SHEET PILE WALL TYPE REVETMENT  
 LOCATION : COPING CONCRETE  
 : WF. 45L + 31.30 ~ WF. 51L + 29.75

(2/2)

CALCULATION	RESULT
• REINFORCING BAR	
Ø13 (W = 1.04 kgf/m)	
$n_1 = 7 \text{ Bars}$	
$W_1 = (10.00 - 0.05 \times 2) \times 7 \text{ Bars} \times 1.04 = 72.072$	
Ø10 (W = 0.617 kgf/m)	
$n_2 = (10.00 - 0.05 \times 2) \div 0.30 + 1 = 34 \text{ Bars}$	
$L = 0.07 \times 2 + 0.40 \times 3 = 1.340 \text{ m/Bar}$	
$W_2 = 1.34 \text{ m/Bar} \times 34 \times 0.617 = 28.111$	
$W = 100.183 \text{ kgf} = 0.100 \text{ tf}/10.00\text{m}$	
$W' = 0.100 \text{ tf}/10.00\text{m} \times 297.00 = 2.970$	2.970 tf
• JOINT FILLER	
$A = 0.50 \times 0.50 = 0.250 \text{ m}^2/10.00\text{m}$	
$A' = 0.25 \text{ m}^2/10.00\text{m} \times 297.00 = 7.425$	7.425 m <sup>3</sup>

TYPE OF WORK : PC SHEET PILE WALL TYPE REVETMENT  
 LOCATION : WF.115L + 15.0 ~ WF.117L + 32.0

CALCULATION			RESULT
<b>☒ STRUCTURAL EXCAVATION</b>			
$A_1 = (0.50 + 0.90) \times \frac{1}{2} \times 0.40$	=	0.260 m <sup>2</sup>	
$A_2 = (1.70 + 2.85) \times \frac{1}{2} \times 1.30$	=	2.958 m <sup>2</sup>	
	A	= 3.218 m <sup>2</sup>	
$V = 3.218 \times 119.50$	=	384.55	384.55 m <sup>3</sup>
<b>☒ BACKFILL WITH SELECTED SOIL</b>			
$A_1 = (0.50 + 0.90) \times \frac{1}{2} \times 0.40$	=	0.280 m <sup>2</sup>	
$A_2 = (1.70 + 2.85) \times \frac{1}{2} \times 1.30 - (0.90 \times 0.70 + 0.90 \times 0.10)$	=	2.238 m <sup>2</sup>	
	A	= 2.518 m <sup>2</sup>	
$V = 2.518 \times 119.50$	=	300.90	300.90 m <sup>3</sup>
<b>☒ PC SHEET PILE</b>			
• PC SHEET PILE			
Length of pile in riverside	$L_1 = 8.50$ m		
Length of pile in land side	$L_2 = 5.00$ m		
$L = (8.50 + 5.00) \times 239$ sets	=	3226.50	3226.50 m
• DRIVING			
Driving length of pile in riverside	$L_3 = 4.20$ m (Average)		
Driving length of pile in land side	$L_4 = 5.00$ m		
$L = (4.20 + 5.00) \times 239$ sets	=	2198.80	2198.80 m
<b>☒ FIXING STEEL TIE ROD AND OTHERS</b>			
• STEEL TIE ROD ( $\varnothing 32$ , w = 6.31 kgf/m)			
n = 119.50 : 2.0	≈	60 place	
L = 6.00 + 0.75 x	=	7.50 m / place	
$W_1 = 7.50 \times 60 \times 6.3$	=	2839.50	
• STEEL CHANNEL (I - 150 x 75, W = 18.6 kgf/m)			
L = 119.50 x 2 x 2	=	478.00 m	
$W_2 = 478.00 \times 18.6$	=	8890.80	
• OTHERS			
$W_3 = 20$ kgf / place x 60 place	=	1200.00	
	TOTAL ( $W_1 + W_2 + W_3$ )	= 12930.30 kgf	
		= 12.930 tf	12.930 tf
<b>☒ RIPRAP MOUND</b>			
A = 10.50 m <sup>2</sup>			
$V = 10.50 \times (119.50 + 3.00 \times 2)$	=	1317.75	1317.8 m <sup>3</sup>

TYPE OF WORK : PC SHEET PILE WALL TYPE REVETMENT  
 LOCATION : WF.45L + 31.30 ~ WF.51L + 29.75

CALCULATION		RESULT
☐ COPING CONCRETE		
• CONCRETE(TYPE - C1)		
$V = 0.90 \times 0.70 \times 10.00$	=	6.300 m <sup>3</sup> /10.00 m
$V^1 = 6.300 \text{ m}^3/10.00 \text{ m} \times 297.00 \times 2$	=	150.570
		150.570 m <sup>3</sup>
• GRAVEL BEDDING		
$V = \{(0.24 \times 0.10) \times 2 + (10.00 \times 0.45)\} \times 10.00$	=	0.930 m <sup>3</sup> /10.00 m
$V^1 = 0.93 \text{ m}^3/10.00 \text{ m} \times 119.50$	=	11.114
		11.114 m <sup>3</sup>
• FORM (H < 4.0 m)		
$A = (0.90 \times 2 \times 2 + 0.35) \times 10.00 + (0.90 \times 0.70)$	=	40.130 m <sup>2</sup> /10.00 m
$A^1 = 40.13 \text{ m}^2/10.00 \text{ m} \times 119.50$	=	479.554
		479.554 m
• REINFORCING BAR		
D 13 (W = 1.04 kgf / m)		
$n_1 = 10 \text{ Bars}$		
$W_1 = (10.00 - 0.05 \times 2) \times 10 \text{ Bars} \times 1.04$	=	102.960
D 10 (W = 0.617 kgf / m)		
$n_1 = (10.00 - 0.05 \times 2) : 0.30 + 1$	=	34 Bars
$L = 0.25 + 0.80 \times 2 + 0.60 + 0.05$	=	2.500 m / Bar
$W_2 = 2.50 \text{ m} / \text{Bar} \times 34 \text{ Bars} \times 0.617$	=	52.445
$W = 155.405 \text{ kgf}$	=	0.155 tf / 10.00 m
$W^1 = 0.155 \text{ tf} / 10.00 \text{ m} \times 119.50 \times 2$	=	3.705
		3.705 tf
• JOINT FILTER		
$A = (0.90 \times 0.70) \times 2 - 0.65 \times 0.22 \times 2$	=	0.974 m <sup>2</sup> /10.00 m
$A^1 = 0.974 \text{ m}^2/10.00 \text{ m} \times 119.50$	=	11.639
		11.639 m <sup>3</sup>



## 2.5 Leaning Wall (Concrete Type)

TYPE OF WORK : LEANING WALL (CONCRETE TYPE)  
 LOCATION : WF.110L - 45.0 m ~ WF.111L + 15.0 m

CALCULATION	RESULT
<b>STRUCTURAL EXCAVATION</b>	
See Attached Table	
V = 2115.224	2115.224 m <sup>3</sup>
<b>BACKFILL WITH SELECTED SOIL</b>	
See Attached Table	
V = 462.726	462.726 m <sup>3</sup>
<b>BACKFILL WITH GRAVEL</b>	
See Attached Table	
V = 481.888	481.888 m <sup>3</sup>
<b>CONCRETE TYPE D</b>	
See Attached Table	
CONCRETE : V = 1015.096	1015.096 m <sup>3</sup>
FORM : A = 9940.880	9940.880 m <sup>2</sup>
<b>RUBBLE STONE BEDDING</b>	
V = 2.30 x 0.20 x 15.00 + 2.60 x 0.20 x 10.00 + 2.85 x 0.20 x 10.00 + 3.00 x 0.20 x 10.00 x 2.00 + 3.15 x 0.20 x 10.00 x 4.00 + 2.85 x 0.20 x 15.00 = 63.550	63.550 m <sup>3</sup>
<b>DEFORMED REINFORCING BARS</b>	
D 13 (1.04 kg.f/m)	
n = {(110.00 - 0.05 x 2) / 0.5 + 1.00} x 6.00 = 1325	
L = 1325 x 0.80 = 1060.000 m	
W = 1060.00 x 1.04 = 1102.400 kg.f	1102.40 kgf
<b>JOINT FILLER</b>	
See Attached Table	
A = 87.293	87.293 m <sup>2</sup>
<b>WEEP HOLE</b>	
PVC PIPE ø 50	
1. n <sub>1</sub> = 85.00 / 2.00 = 43	
L <sub>1</sub> = 0.80 x 43 = 34.400	

TYPE OF WORK : LEANING WALL (CONCRETE TYPE)  
 LOCATION : WF.110L - 45.0 m ~ WF.111L + 15.0 m

CALCULATION		RESULT
2. $n_2 = 110.00 / 2.00$	= 55	
$L_2 = 1.10 \times 55$	= 60.500	
3. $n_3 = 110.00 / 2.00$	= 55	
$L_3 = 1.50 \times 55$	= 82.500	
4. $n_4 = 110.00 / 2.00$	= 55	
$L_4 = 2.00 \times 55$	= 110.000	
<b>TOTAL L</b>		<b>287.400 m</b>
<b>FILTER CLOTH</b>		
$N = n_1 + n_2 + n_3 + n_4$	= 208 places	
$A = 0.640 \text{ m}^2 / \text{place} \times 208$	= 133.120	133.120 m <sup>2</sup>
<b>STEEL FENCE</b>		
$L = 110.117$		110.117 m
See "Form"		

LEANING WALL (CONCRETE TYPE)

TYPE OF WORK

: STRUCTURAL EXCAVATION

LOCATION

: WF.105R ~ WF.110R+22.0

CALCULATION																	RESULT
section	L1	L2	H1	A1	L3	L4	H2	A2	L5	L6	H3	A3	A1+A2+A3	ave	distance(m)	volume(m <sup>3</sup> )	
1	1.500	2.700	5.450	11.445	4.775	3.750	1.150	4.902	2.075	2.375	0.300	0.668	17.014	17.119	14.500	248.231	
2	1.500	2.700	5.550	11.655	4.775	3.750	1.150	4.902	2.075	2.375	0.300	0.668	17.224	17.224	10.000	172.244	
3	1.500	2.700	5.550	11.655	4.775	3.750	1.150	4.902	2.075	2.375	0.300	0.668	17.224	17.224	10.000	172.244	
4	1.500	2.700	5.550	11.655	4.775	3.750	1.150	4.902	2.075	2.375	0.300	0.668	17.224	17.224	10.000	172.244	
5	1.500	2.700	5.450	11.445	4.775	3.750	1.150	4.902	2.075	2.375	0.300	0.668	17.014	17.119	10.000	171.194	
6	1.500	2.700	5.450	11.445	4.775	3.750	1.150	4.902	2.075	2.375	0.300	0.668	17.014	17.014	10.000	170.144	
7	1.500	2.610	5.450	11.200	4.650	3.650	1.100	4.565	2.040	2.340	0.300	0.657	16.422	16.718	10.000	167.181	
8	1.500	2.610	5.350	10.994	4.650	3.650	1.100	4.565	2.040	2.340	0.300	0.657	16.216	16.319	10.000	163.190	
9	1.500	2.610	5.250	10.789	4.650	3.650	1.100	4.565	2.040	2.340	0.300	0.657	16.011	16.114	10.000	161.135	
10	1.500	2.520	5.150	10.352	4.475	3.500	1.050	4.187	1.955	2.255	0.300	0.632	15.170	15.590	10.000	155.903	
11	1.500	2.520	5.050	10.151	4.475	3.500	1.050	4.187	1.955	2.255	0.300	0.632	14.969	15.069	10.000	150.694	
12	1.500	2.520	4.950	9.950	4.475	3.500	1.050	4.187	1.955	2.255	0.300	0.632	14.768	14.868	10.000	148.684	
13	1.500	2.520	4.850	9.749	4.475	3.500	1.050	4.187	1.955	2.255	0.300	0.632	14.567	14.667	10.000	146.674	
14	1.500	2.430	4.650	9.137	4.300	3.350	1.000	3.825	1.870	2.170	0.300	0.606	13.568	14.068	10.000	140.676	
15	1.500	2.430	4.550	8.941	4.300	3.350	1.000	3.825	1.870	2.170	0.300	0.606	13.372	13.470	10.000	134.700	
16	1.500	2.430	4.450	8.744	4.300	3.350	1.000	3.825	1.870	2.170	0.300	0.606	13.175	13.274	10.000	132.735	
17	1.500	2.340	4.250	8.160	4.125	3.200	0.950	3.479	1.785	2.085	0.300	0.581	12.220	12.698	10.000	126.976	
18	1.500	2.340	4.050	7.776	4.125	3.200	0.950	3.479	1.785	2.085	0.300	0.581	11.836	12.028	10.000	120.279	
19	1.500	2.340	3.850	7.392	4.125	3.200	0.950	3.479	1.785	2.085	0.300	0.581	11.452	11.644	10.000	116.439	
20	1.500	2.250	3.650	6.844	3.950	3.050	0.900	3.150	1.700	2.000	0.300	0.555	10.549	11.000	10.000	110.003	
21	1.500	2.250	3.450	6.469	3.950	3.050	0.900	3.150	1.700	2.000	0.300	0.555	10.174	10.361	10.000	103.613	
22	1.500	2.340	3.550	6.816	4.125	3.200	0.950	3.479	1.785	2.085	0.300	0.581	10.876	10.525	10.000	105.248	
23	1.500	2.340	3.750	7.200	4.125	3.200	0.950	3.479	1.785	2.085	0.300	0.581	11.260	11.068	10.000	110.679	
24	1.500	2.340	3.950	7.584	4.125	3.200	0.950	3.479	1.785	2.085	0.300	0.581	11.644	11.452	10.000	114.519	
25	1.500	2.340	4.150	7.968	4.125	3.200	0.950	3.479	1.785	2.085	0.300	0.581	12.028	11.836	10.000	118.359	
26	1.500	2.430	4.250	8.351	4.300	3.350	1.000	3.825	1.870	2.170	0.300	0.606	12.782	12.405	10.000	124.051	
27	1.500	2.430	4.300	8.450	4.300	3.350	1.000	3.825	1.870	2.170	0.300	0.606	12.881	12.831	4.500	57.741	
																3643.532	

3643.532m<sup>3</sup>

TYPE OF WORK  
LOCATION

LEANING WALL (CONCRETE TYPE)  
: BACKFILL WITH SELECTED SOIL  
: WF. 105R ~ WF. 110R + 22.0

CALCULATION

RESULT

Table. Calculation for Backfill with Selected Soil

section	L1	L2	H1	A1	L3	L4	L5	L6	H3	A3	L7	L8	A4	A1+A2+A3+A4	ave	distance(m)	volume(m <sup>3</sup> )	
1	1.050	0.600	1.150	0.949	1.175	0.600	1.021	2.075	2.175	0.100	0.213	0.500	3.450	1.725	3.907	3.932	14.500	57.012
2	1.050	0.600	1.150	0.949	1.175	0.600	1.021	2.075	2.175	0.100	0.213	0.500	3.450	1.725	3.957	3.932	14.500	57.012
3	1.050	0.600	1.150	0.949	1.175	0.600	1.021	2.075	2.175	0.100	0.213	0.500	3.450	1.725	3.957	3.957	10.000	39.569
4	1.050	0.600	1.150	0.949	1.175	0.600	1.021	2.075	2.175	0.100	0.213	0.500	3.450	1.725	3.907	3.957	10.000	39.569
5	1.050	0.600	1.150	0.949	1.175	0.600	1.021	2.075	2.175	0.100	0.213	0.500	3.450	1.725	3.907	3.907	10.000	39.069
6	1.050	0.600	1.100	0.908	1.150	0.600	1.021	2.040	2.140	0.100	0.209	0.500	3.400	1.700	3.779	3.779	10.000	38.429
7	1.050	0.600	1.100	0.908	1.150	0.600	1.021	2.040	2.140	0.100	0.209	0.500	3.400	1.700	3.779	3.779	10.000	38.429
8	1.050	0.600	1.100	0.908	1.150	0.600	1.021	2.040	2.140	0.100	0.209	0.500	3.400	1.700	3.779	3.779	10.000	38.429
9	1.050	0.600	1.100	0.908	1.150	0.600	1.021	2.040	2.140	0.100	0.209	0.500	3.400	1.700	3.779	3.779	10.000	38.429
10	1.050	0.600	1.100	0.908	1.150	0.600	1.021	2.040	2.140	0.100	0.209	0.500	3.400	1.700	3.779	3.779	10.000	38.429
11	1.050	0.600	1.050	0.866	1.125	0.600	0.906	1.955	2.055	0.100	0.201	0.500	2.950	1.475	3.447	3.472	10.000	34.724
12	1.050	0.600	1.050	0.866	1.125	0.600	0.906	1.955	2.055	0.100	0.201	0.500	2.950	1.475	3.447	3.472	10.000	34.724
13	1.050	0.600	1.000	0.825	1.100	0.600	0.850	1.870	1.970	0.100	0.192	0.500	2.500	1.250	3.117	3.232	10.000	32.322
14	1.050	0.600	1.000	0.825	1.100	0.600	0.850	1.870	1.970	0.100	0.192	0.500	2.500	1.250	3.117	3.232	10.000	32.322
15	1.050	0.600	1.000	0.825	1.100	0.600	0.850	1.870	1.970	0.100	0.192	0.500	2.500	1.250	3.117	3.232	10.000	32.322
16	1.050	0.600	1.000	0.825	1.100	0.600	0.850	1.870	1.970	0.100	0.192	0.500	2.500	1.250	3.117	3.232	10.000	32.322
17	1.050	0.600	0.950	0.784	1.075	0.600	0.796	1.785	1.885	0.100	0.184	0.500	2.050	1.025	2.788	2.902	10.000	29.024
18	1.050	0.600	0.950	0.784	1.075	0.600	0.796	1.785	1.885	0.100	0.184	0.500	2.050	1.025	2.788	2.902	10.000	29.024
19	1.050	0.600	0.950	0.784	1.075	0.600	0.796	1.785	1.885	0.100	0.184	0.500	2.050	1.025	2.788	2.902	10.000	29.024
20	1.050	0.600	0.900	0.743	1.050	0.600	0.743	1.700	1.800	0.100	0.175	0.500	1.400	0.700	2.360	2.474	10.000	24.739
21	1.050	0.600	0.900	0.743	1.050	0.600	0.743	1.700	1.800	0.100	0.175	0.500	1.400	0.700	2.360	2.474	10.000	24.739
22	1.050	0.600	0.900	0.743	1.050	0.600	0.743	1.700	1.800	0.100	0.175	0.500	1.400	0.700	2.360	2.474	10.000	24.739
23	1.050	0.600	0.900	0.743	1.050	0.600	0.743	1.700	1.800	0.100	0.175	0.500	1.400	0.700	2.360	2.474	10.000	24.739
24	1.050	0.600	0.900	0.743	1.050	0.600	0.743	1.700	1.800	0.100	0.175	0.500	1.400	0.700	2.360	2.474	10.000	24.739
25	1.050	0.600	0.900	0.743	1.050	0.600	0.743	1.700	1.800	0.100	0.175	0.500	1.400	0.700	2.360	2.474	10.000	24.739
26	1.050	0.600	1.000	0.825	1.100	0.600	0.850	1.870	1.970	0.100	0.192	0.500	2.100	1.050	2.917	2.917	10.000	28.274
27	1.050	0.600	1.000	0.825	1.100	0.600	0.850	1.870	1.970	0.100	0.192	0.500	2.100	1.050	2.917	2.917	10.000	28.274
																		832.966

832.966 m<sup>3</sup>

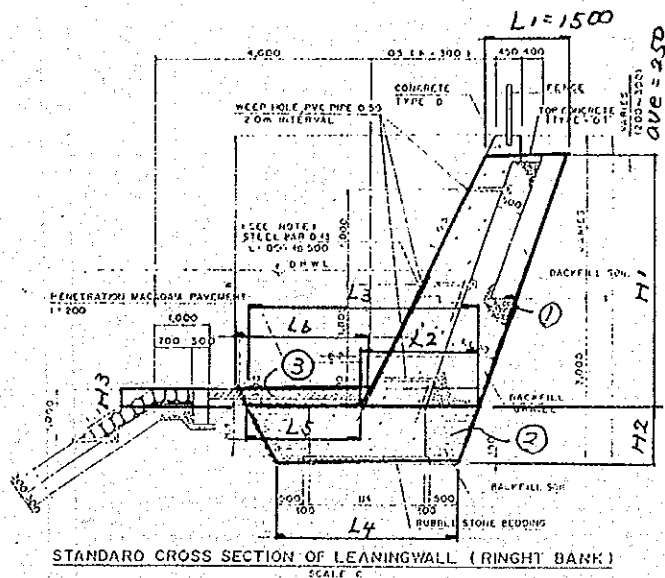
TYPE OF WORK

: LEANING WALL (CONCRETE TYPE)

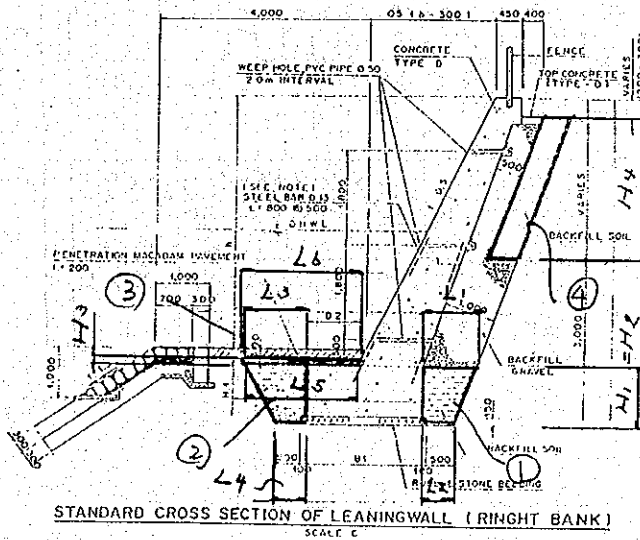
LOCATION

: WF. 105R ~ WF. 110R+22.0

EXPLANATORY DRAWING



FOR STRUCTURAL EXCAVATION



FOR BACKFILL WITH SELECTED SOIL

LEANING WALL (CONCRETE TYPE)

TYPE OF WORK : BACKFILL WITH GRAVEL

LOCATION : WF. 105R ~ WF. 110R + 22.0

CALCULATION

RESULT

Table. Calculation for Backfill with Gravel

section	L1	H1	A1	L2	H2	A2	A1+A2	ave	distance(m)	volume(m3)
1	0.500	3.450	1.725	1.050	2.000	2.100	3.825	3.825	14.500	55.825
2	0.500	3.550	1.775	1.050	2.000	2.100	3.875	3.875	10.000	38.750
3	0.500	3.550	1.775	1.050	2.000	2.100	3.875	3.875	10.000	38.750
4	0.500	3.550	1.775	1.050	2.000	2.100	3.875	3.875	10.000	38.750
5	0.500	3.450	1.725	1.050	2.000	2.100	3.825	3.825	10.000	38.500
6	0.500	3.450	1.725	1.050	2.000	2.100	3.825	3.825	10.000	38.250
7	0.500	3.400	1.700	1.050	2.050	2.153	3.853	3.853	10.000	38.388
8	0.500	3.300	1.650	1.050	2.050	2.153	3.803	3.803	10.000	38.275
9	0.500	3.200	1.600	1.050	2.050	2.153	3.753	3.778	10.000	37.775
10	0.500	3.050	1.525	1.050	2.100	2.205	3.730	3.741	10.000	37.413
11	0.500	2.950	1.475	1.050	2.100	2.205	3.680	3.705	10.000	37.050
12	0.500	2.850	1.425	1.050	2.100	2.205	3.630	3.655	10.000	36.550
13	0.500	2.750	1.375	1.050	2.100	2.205	3.580	3.605	10.000	36.050
14	0.500	2.500	1.250	1.050	2.150	2.258	3.508	3.544	10.000	35.438
15	0.500	2.400	1.200	1.050	2.150	2.258	3.458	3.483	10.000	34.825
16	0.500	2.300	1.150	1.050	2.150	2.258	3.408	3.433	10.000	34.325
17	0.500	2.050	1.025	1.050	2.200	2.310	3.335	3.371	10.000	33.713
18	0.500	1.850	0.925	1.050	2.200	2.310	3.235	3.285	10.000	32.850
19	0.500	1.650	0.825	1.050	2.200	2.310	3.135	3.185	10.000	31.850
20	0.500	1.400	0.700	1.050	2.250	2.363	3.063	3.099	10.000	30.988
21	0.500	1.200	0.600	1.050	2.250	2.363	2.963	3.013	10.000	30.125
22	0.500	1.350	0.675	1.050	2.200	2.310	2.985	2.974	10.000	29.738
23	0.500	1.550	0.775	1.050	2.200	2.310	3.085	3.035	10.000	30.350
24	0.500	1.750	0.875	1.050	2.200	2.310	3.185	3.135	10.000	31.350
25	0.500	1.950	0.975	1.050	2.200	2.310	3.285	3.235	10.000	32.350
26	0.500	2.100	1.050	1.050	2.150	2.258	3.308	3.296	10.000	32.963
27	0.500	2.150	1.075	1.050	2.150	2.258	3.333	3.320	4.500	14.940
										907.378

907.378 m<sup>3</sup>

LEANING WALL (CONCRETE TYPE)

TYPE OF WORK

: CONCRETE (TYPE-D)

LOCATION

: WF. 105R ~ WF. 110R+22.0

CALCULATION													RESULT	
Table Calculation for Concrete (Wall) Volume														
section	L1	L2	H1	A1	ave	distance(m)	V1(m3)	L3	H2	A2	distance(m)	V2(m3)	V1+V2(m3)	
1	0.450	1.650	3.700	5.985				2.550	1.000	2.550	14.500	36.975	36.975	
2	0.450	1.650	3.800	6.090	6.038	14.500	87.544	2.550	1.000	2.550	10.000	25.500	113.044	
3	0.450	1.650	3.800	6.090	6.090	10.000	60.900	2.550	1.000	2.550	10.000	25.500	86.400	
4	0.450	1.650	3.800	6.090	6.090	10.000	60.900	2.550	1.000	2.550	10.000	25.500	86.400	
5	0.450	1.650	3.700	5.985	6.038	10.000	60.375	2.550	1.000	2.550	10.000	25.500	85.875	
6	0.450	1.650	3.700	5.985	5.985	10.000	59.850	2.550	1.000	2.550	10.000	25.500	85.350	
7	0.450	1.560	3.700	5.729	5.857	10.000	58.568	2.450	0.950	2.328	10.000	23.275	81.843	
8	0.450	1.560	3.600	5.628	5.678	10.000	56.783	2.450	0.950	2.328	10.000	23.275	80.058	
9	0.450	1.560	3.500	5.528	5.578	10.000	55.778	2.450	0.950	2.328	10.000	23.275	79.053	
10	0.450	1.470	3.400	5.184	5.356	10.000	53.558	2.300	0.900	2.070	10.000	20.700	74.258	
11	0.450	1.470	3.300	5.088	5.136	10.000	51.360	2.300	0.900	2.070	10.000	20.700	72.060	
12	0.450	1.470	3.200	4.992	5.040	10.000	50.400	2.300	0.900	2.070	10.000	20.700	71.100	
13	0.450	1.470	3.100	4.896	4.944	10.000	49.440	2.300	0.900	2.070	10.000	20.700	70.140	
14	0.450	1.380	4.900	4.484	4.690	10.000	46.898	2.150	0.850	1.828	10.000	18.275	65.173	
15	0.450	1.380	4.800	4.392	4.438	10.000	44.378	2.150	0.850	1.828	10.000	18.275	62.653	
16	0.450	1.380	4.700	4.301	4.346	10.000	43.463	2.150	0.850	1.828	10.000	18.275	61.738	
17	0.450	1.290	4.500	3.915	4.108	10.000	41.078	2.000	0.800	1.600	10.000	16.000	57.078	
18	0.450	1.290	4.300	3.741	3.828	10.000	38.280	2.000	0.800	1.600	10.000	16.000	54.280	
19	0.450	1.290	4.100	3.567	3.654	10.000	36.540	2.000	0.800	1.600	10.000	16.000	52.540	
20	0.450	1.200	3.900	3.218	3.392	10.000	33.923	1.850	0.750	1.388	10.000	13.875	47.798	
21	0.450	1.200	3.700	3.053	3.135	10.000	31.350	1.850	0.750	1.388	10.000	13.875	45.225	
22	0.450	1.290	3.800	3.206	3.179	10.000	31.793	2.000	0.800	1.600	10.000	16.000	47.793	
23	0.450	1.290	4.000	3.480	3.393	10.000	33.930	2.000	0.800	1.600	10.000	16.000	49.930	
24	0.450	1.290	4.200	3.654	3.567	10.000	35.670	2.000	0.800	1.600	10.000	16.000	51.670	
25	0.450	1.290	4.400	3.828	3.741	10.000	37.410	2.000	0.800	1.600	10.000	16.000	53.410	
26	0.450	1.380	4.500	4.118	3.973	10.000	39.728	2.150	0.850	1.828	4.500	8.224	47.951	
27	0.450	1.380	4.550	4.163	4.140	4.500	18.632	2.150	0.850	1.828			18.632	
											total	519.899	1738.422	
Calculation for Concrete Top Cor														
block	distance	L	t	w	V(m3)									
L1	14.500	14.500	0.150	0.400	0.870									
L2	10.000	10.000	0.150	0.400	0.600									
L3	10.000	10.000	0.150	0.400	0.600									
L4	10.000	10.000	0.150	0.400	0.600									
L5	10.000	10.000	0.150	0.400	0.600									
L6	10.000	10.000	0.150	0.400	0.600									
L7	10.000	10.000	0.150	0.400	0.600									
L8	10.000	10.000	0.150	0.400	0.600									
L9	10.000	10.000	0.150	0.400	0.600									
L10	10.000	10.000	0.150	0.400	0.600									
L11	10.000	10.000	0.150	0.400	0.600									
L12	10.000	10.000	0.150	0.400	0.600									
L13	10.000	10.002	0.150	0.400	0.600									
L14	10.000	10.000	0.150	0.400	0.600									
L15	10.000	10.000	0.150	0.400	0.600									
L16	10.000	10.002	0.150	0.400	0.600									
L17	10.000	10.002	0.150	0.400	0.600									
L18	10.000	10.002	0.150	0.400	0.600									
L19	10.000	10.002	0.150	0.400	0.600									
L20	10.000	10.002	0.150	0.400	0.600									
L21	10.000	10.000	0.150	0.400	0.600									
L22	10.000	10.002	0.150	0.400	0.600									
L23	10.000	10.002	0.150	0.400	0.600									
L24	10.000	10.002	0.150	0.400	0.600									
L25	10.000	10.000	0.150	0.400	0.600									
L26	4.500	4.500	0.150	0.400	0.270									
					15.541									

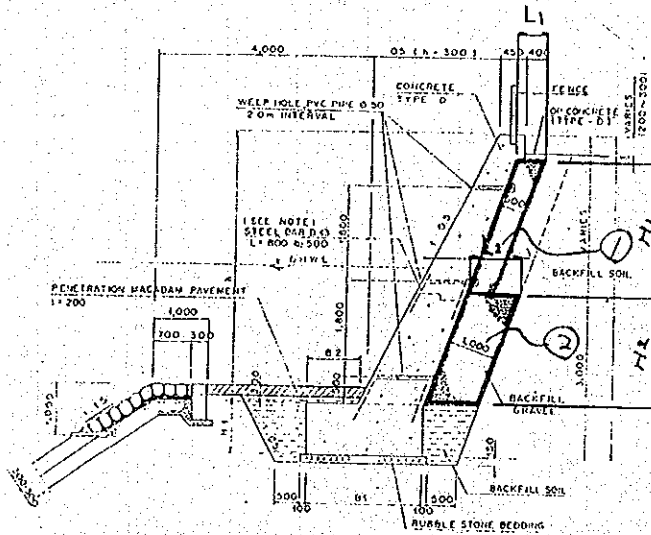
1738.422

TOTAL(m3)	
Wall	1738.422
Top Concrete	15.541
	1753.963

TYPE OF WORK : LEANING WALL (CONCRETE TYPE)

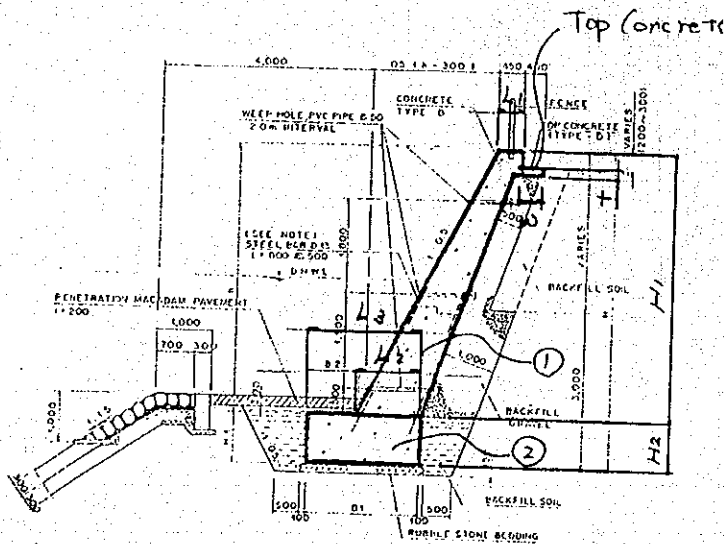
LOCATION : WF. 105R ~ WF. 110R+22.0

EXPLANATORY DRAWING



STANDARD CROSS SECTION OF LEANINGWALL (RIGHT BANK)  
SCALE C

FOR BACKFILL WITH GRAVEL



STANDARD CROSS SECTION OF LEANINGWALL (RIGHT BANK)  
SCALE C

FOR CONCRETE (TYPE-D)



TYPE OF WORK: RUBBLE STONE BEDDING

LOCATION:

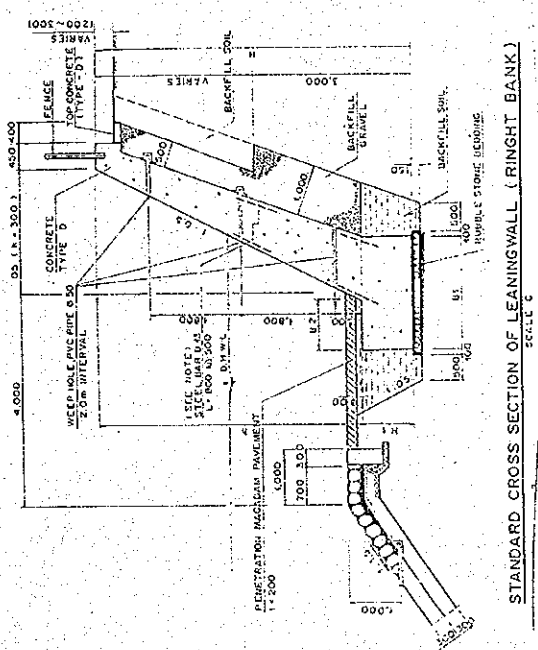
CALCULATION

See Attached Table

RESULT: 94.943m<sup>3</sup>

Table. Calculation for Rubble Stone Bedding

block	t	distance	V(m <sup>3</sup> )
L1	0.15	14.500	5.981
L2	0.15	10.000	4.125
L3	0.15	10.000	4.125
L4	0.15	10.000	4.125
L5	0.15	10.000	4.125
L6	0.15	10.000	4.125
L7	0.15	10.000	3.975
L8	0.15	10.000	3.975
L9	0.15	10.000	3.975
L10	0.15	10.000	3.750
L11	0.15	10.000	3.750
L12	0.15	10.000	3.750
L13	0.15	10.000	3.750
L14	0.15	10.000	3.525
L15	0.15	10.000	3.525
L16	0.15	10.000	3.525
L17	0.15	10.000	3.300
L18	0.15	10.000	3.300
L19	0.15	10.000	3.300
L20	0.15	10.000	3.075
L21	0.15	10.000	3.075
L22	0.15	10.000	3.300
L23	0.15	10.000	3.300
L24	0.15	10.000	3.300
L25	0.15	10.000	3.300
L26	0.15	4.500	1.586
			94.943



STANDARD CROSS SECTION OF LEANINGWALL (RING BANK) SCALE 1:200

TYPE OF WORK : **DEFORMED REINFORCING BARS**

LOCATION :

CALCULATION

B 13 (1.04 kg/m)

$L = 4.5 + 240.0 + 14.5 = 259.000$

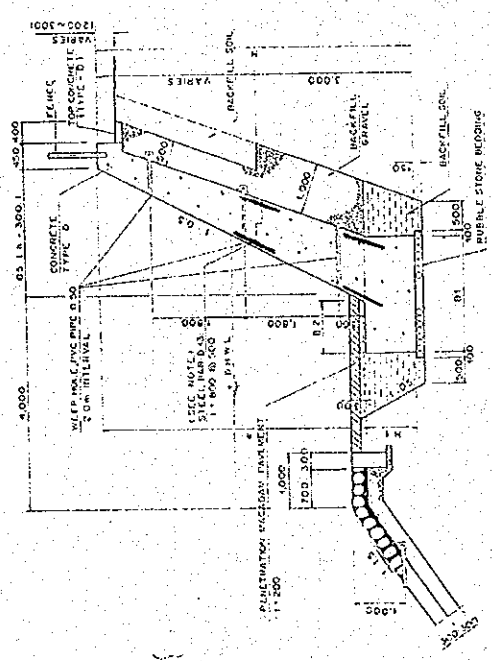
$n = \{ (259 - 0.05 \times 2) \div 0.5 + 1 \} \times 4 = 2075$

$L = 2075 \times 0.8 = 1660.0$

$W = 1660.0 \times 1.04 = 1726.4$

1726.4 kg

RESULT



STANDARD CROSS SECTION OF LEANINGWALL (RIGHT BANK) SCALE 5