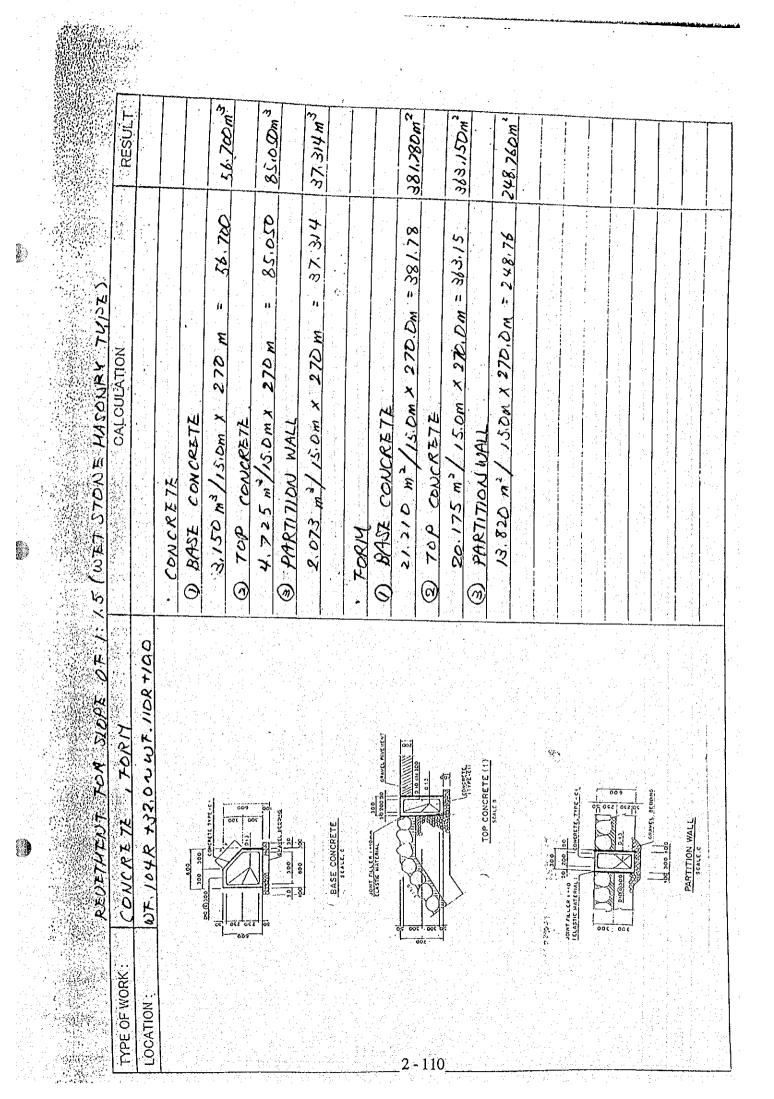
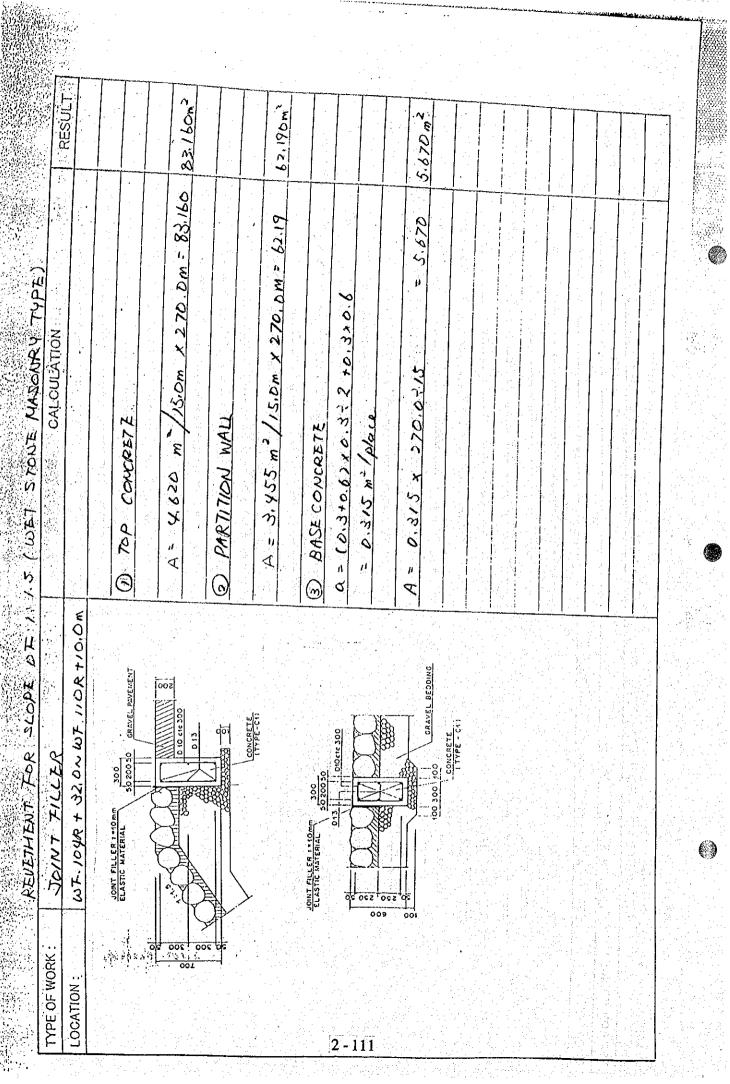
### TYPE OF WORK: REVETMENT FOR SLOPE OF 1:1.5 (WET STONE MASONRY TYPE)LOCATION: WF.104R + 32.0 ~ WF.110R + 10.0 m

CALCULATION	N	RESULT
STRUCTURAL EXCAVATION		
$V = 176.850 \text{ m}^3 / 15.0 \text{ m} \times 270.00$	= 3183.300	3183.300 m <sup>3</sup>
· · · · · · · · · · · · · · · · · · ·	5105.500	5105,500 m
BACKFILL WITH SELECTED SOIL		
$V = 17.445 \text{ m}^3 / 15.0 \text{ m} \ge 270.00$	= 314.010	314.010 m <sup>3</sup>
WET STONE MASONRY		
$V = 50.789 \text{ m}^3 / 15.0 \text{ m} \times 270.0$	= 914.202	914.202 m <sup>3</sup>
CEMENT MORTAR POINTING	<u>i Marine de la companya da serie da se</u> Filia da serie	
CEMENT MORTAR FOLTING		
$A = 142.796 \text{ m}^2 / 15.0 \text{ m} \times 270.0$	= 2570.328	2570.328 m <sup>2</sup>
DEFORMED REINFORCING BAR 1. BASE CONCRETE		
I.BASE CONCREIE	<u>e del del esta del francés de la compo</u> nda de la componentia de la componentia de la componentia de la componentia Componentia de la componentia de la comp	
$W = 0.151 \text{ tf} / 15.0 \text{ m} \times 270.0$	= 2.718	2.718 tf
2. TOP CONCRETE		
$W = 0.143 \text{ tf} / 15.0 \text{ m} \times 270.0$	= 2.574	2.574 tf
W = 0.145 H / 15.0 M X 270.0	2,J,T	2.574 u
3. PARTITION WALL		
	1054	1.054.48
$W = 0.103 \text{ tf} / 15.0 \text{ m} \times 270.0$	= 1.854	1.854 tf
GABION MATTRESS		
$V = 33.750 \text{ m}^3 / 15.0 \text{ m} \times 270.0$	= 607.500	607.500 m <sup>3</sup>
WEEP HOLE		
WEEPHOLE		
$PVC PIPE \emptyset 50, n = 162$		
$L = 6.750 \text{ m} / 15.0 \text{ m} \times 270.0$	= 121.500	121.500 m
FILTER CLOTH	100 000	100 (00) 7
$A = 5.760 \text{ m}^2 / 15.0 \text{ m x } 270.0$	= 103.680	103.680 m <sup>2</sup>
RUBBLE STONE FILLING		
$V = 5.625 \text{ m}^3 / 15.0 \text{ m} \times 270.0$	= 101.250	101.250 m <sup>3</sup>

	<sup>™</sup> ₹ 78	
RESULT 889.362 m	2 16.80 m	<u>∠</u> 596 <i>m «</i>
= 889.362	10.8	- 21.5%
74/2E	270.0 m ≥ 270.0 m =	270.0m
HASONRY CALCULATION	Som X	/ 15.0mx
	TOP CONCRET	<b>x x x x</b>
1. 1.5 ( WET 0. R.EVET V = 48.		<b>o</b>
REVETHENT FOR LOPE CR GRAVEL BEDUNG WF. 104A +320 ~ WF. 110.0	201 1 1 0 2000 201 1 1 1 0 2000 00 201 1 1 1 0 2000 00 1 1 1 1 0 2000 00 1 1 1 1 1 1 1 0 2000 00 1 1 1 1 1 1 1 0 2000 00 1 1 1 1 1 1 1 1 0 2000 00 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	
KENT FOR BEDUNG 320-NF.110		
REVETA GRAVEC WF.IO4A+	BASE CONCRETE	
LOCATION.		





## TYPE OF WORK: REVETMENT FOR SLOPE OF 1:1.5LOCATION: WF.176R ~ WF.180R (LOWER CHANNEL)

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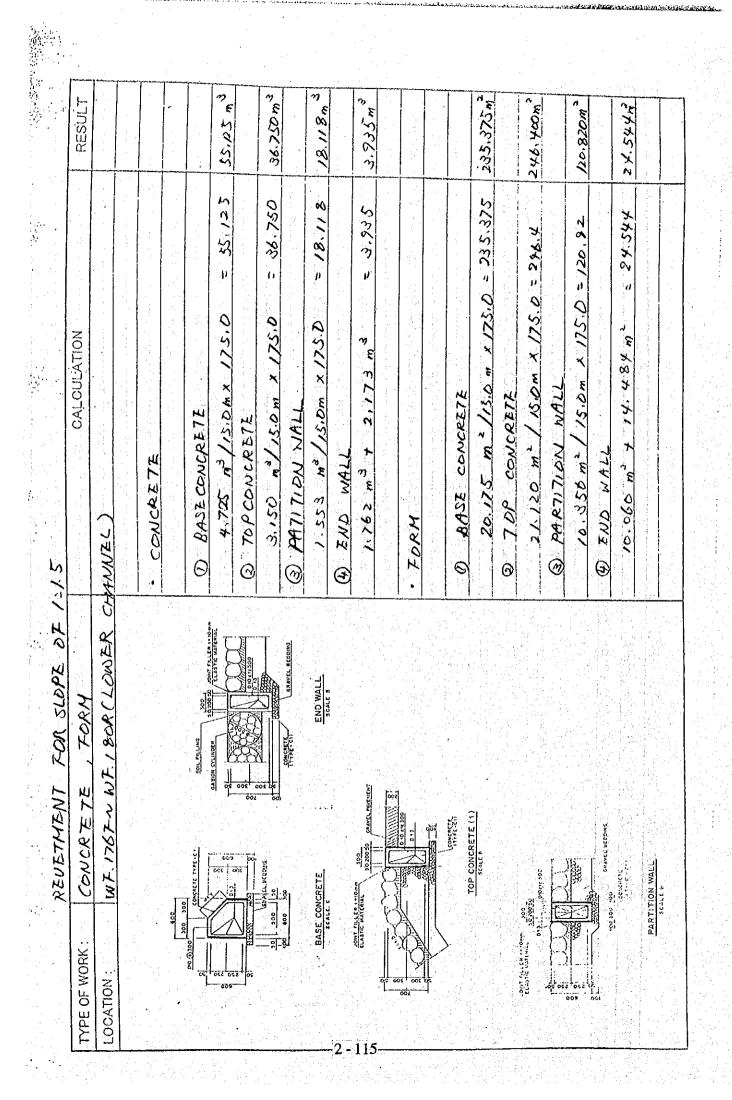
CALCULATION		RESULT
STRUCTURAL EXCAVATION		
$V = 153.450 \text{ m}^3 / 15.0 \text{ m} \times 175.00 \text{ m}$	= 1790.250	1790.250 m <sup>3</sup>
BACKFILL WITH SELECTED SOIL		
$V = 21.690 \text{ m}^3 / 15.0 \text{ m} \times 175.00 \text{ m}$	= 253.050	253.050 m <sup>3</sup>
WET STONE MASONRY		
$\overline{V} = 43.027 \mathrm{m}^3 / 15.0 \mathrm{m} \ge 175.00 \mathrm{m}$	= 501.982	501.982 m <sup>3</sup>
	- 301.982	J01.782 III
CEMENT MORTAR POINTING		
$A = 102.900 \text{ m}^2 / 15.0 \text{ m x } 175.00 \text{ m}$	= 1200.500	1200.500 m <sup>2</sup>
GABION MATTRESS		
GADION MAI I RESS	en al antes de la composición de la com En al composición de la composición de l	
$\overline{V} = 33.750 \text{ m}^3 / 15.0 \text{ m} \times 175.00 \text{ m}$	= 393.750	393.750 m <sup>3</sup>
RUBBLE STONE FILLING		
	66.606	
$V = 5.625 \text{ m}^3 / 15.0 \text{ m} \times 175.00 \text{ m}$	= 65.625	65.625 m <sup>3</sup>
$V = 5.625 \mathrm{m^3} / 15.0 \mathrm{m} \ge 175.00 \mathrm{m}$	= 65.625	65.625 m <sup>3</sup>
$V = 5.625 \mathrm{m^3} / 15.0 \mathrm{m} \ge 175.00 \mathrm{m}$	= 65.625	65.625 m³
$\frac{V = 5.625 \mathrm{m}^3 / 15.0 \mathrm{m} \ge 175.00 \mathrm{m}}{2}$	= 65.625	65.625 m <sup>3</sup>
$\frac{V = 5.625 \mathrm{m^3} / 15.0 \mathrm{m} \times 175.00 \mathrm{m}}{2}$	= 65.625	65.625 m <sup>3</sup>
$V = 5.625 \mathrm{m^3} / 15.0 \mathrm{m} \ge 175.00 \mathrm{m}$	= 65.625	65.625 m <sup>3</sup>
$V = 5.625 \mathrm{m^3} / 15.0 \mathrm{m} \ge 175.00 \mathrm{m}$	= 65.625	65.625 m <sup>3</sup>
V = 5.625 m <sup>3</sup> /15.0 m x 175.00 m	= 65.625	65.625 m <sup>3</sup>
V = 5.625 m <sup>3</sup> / 15.0 m x 175.00 m	= 65.625	65.625 m <sup>3</sup>
V = 5.625 m <sup>3</sup> /15.0 m x 175.00 m	= 65.625	65.625 m <sup>3</sup>

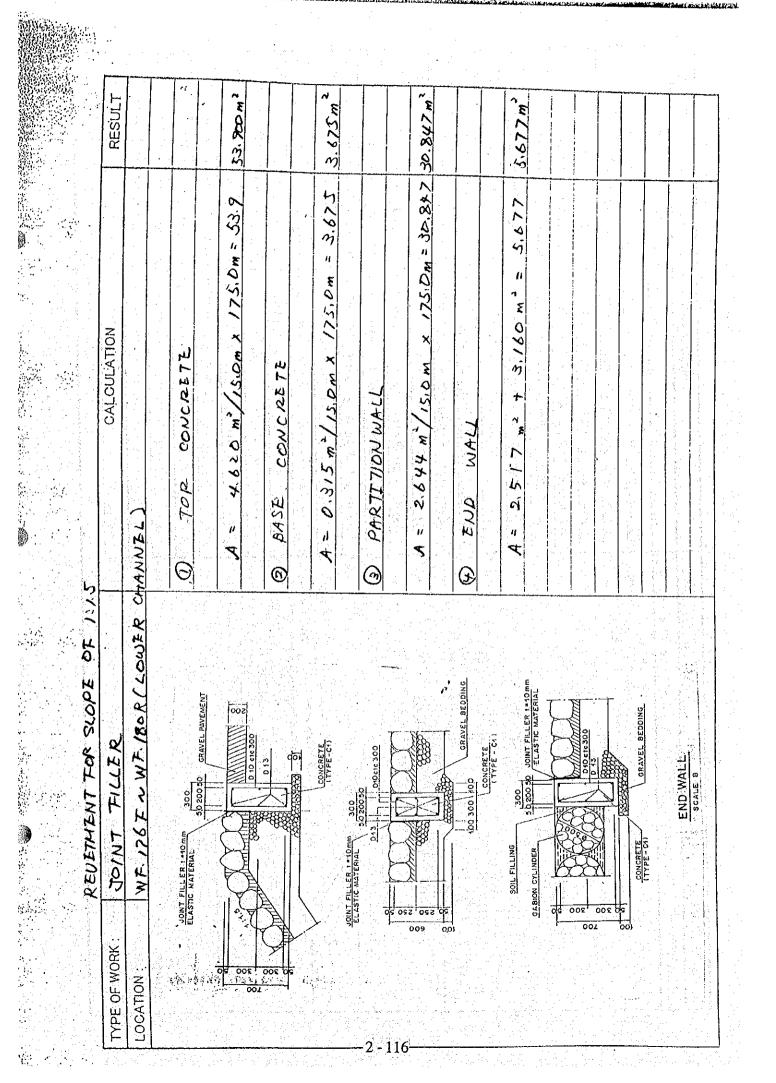
2 - 112

й( 1 440.020 14000 mg 7 000 m 0.712 mg 4 823 4 RESULT 0 20.04P = 4.923 - X 800 200 /1 = = 0.393 × 1 place = 0,319 . 70141 p 0.501 37, 7, 6 m 3/15.0m × 125.0 050 175.0 Na ce CALCULATION × 7 masi 115.20 0.319 m / place CONCRETE D.422 nº 15.0m PARTIZION WALL 2) DOWN STRFAM 0.393 mª/place TOP CONCRETE REVETMENT END WALL MAZARZAN (1 1.200 m 0.600 m3 BASE CHANNEL 6 ତ  $\odot$ 9 6 07 1225 00 3001400 CHAVEL BEOMG WF. 176 F~ WF. 180R ( LOWER PARTITION WALL 0105 20010 REVETMENT FOR SLOPE 300 2020050 GRAVEL BEDDING JOINT FR. ER 1150mm ିକ୍ଷ CRAVEL POUCHEN 1242-3445-04 PAYEL PEPOLIS D BASE CONCRETE TOP CONCRETE (1) 300 JOINT FILLER 1 1000 CONCRETE: 8 8 8 취활 DNAVEL BEDOING 300 300 \$00 600 800 END WALL JOHT FILLER 1 - 10mm 00 (0) 200 6 TYPE OF WORK : CONCRETE SOIL FRUME GA BION CYLINDER LOCATION 1 2 - 113

RESULT 166871 128217 0.922+5 0.115+5 173.0M = 1,762 = 1,468 - 0.922 = 0.115 x 175,0m 0.0 05 tf 15.0m × 175.0m 15.0 m x CALCULATION 15.0 M © TOP CONCRETE PARTITION WALL CONCRETE **+** 0.151 +F WALL 4 + 620 0 = N=0.1437 0.080 END BASE 1 2 3 0 2 WF. 176F ~ WF. 176R (LOWER CHANNEL) 1 9 REVETHENT FOR XOOK OF 1:1.5 JOINT FILLER 1- 10mm CHLTE TYPE-C+ × DEFORMED REINFORCING BARS מקור הנפטייה אלינים BASE CONCRETE CRAVEL BEDDING 00 000 000 000 END WALL 203 200 200 00 (0) 200 CONCRETE SOIL FILLING GABION CYLINDER ) 作() [2]望 ORAVEL PAVENENT 5175030 157780 3178000 307-005 005 C TOP-CONCRETE (1) CONCRETC. ም [1 PARTITION WALL 002 11 010 1 1 1 1 0 00 1 1 0 00 500 0 200 50 10011 51154 11 1000 300 JOINT FILLER 1-10 MM TYPE OF WORK : LOCATION : 2 - 114

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26.032 m n E 13.907 43 RESULT 12.125 = 12.125 mg =13.907 m3 10.310 M = 0.196m2 = 70.956 m = 0.195m2 = 61.860 m ž = 11.826 TOTAL 11 CALCULATION 0 2 0.196 ſ 61.860×0,196 1 = 3.0 + 7.826 + 1.0 ł Gabion Cylinder ) Gabion Cylinder -Ż トイ 6.310 0 = K×0.52 + 10.310 x 70,956 x 3 a= R × 0.5-X 9 c8 11 = 7 - 9 P -0 6 21 1 = 2 IJ ų ーと > 2 2 ្រុ WF.176F~WF.180R(LOWER CHANNEL REVETMENT FOR SLOPE OF 1:15 PVC JOINT FILLER I'TOMM 64.010N CYLINDER-2 GRAVEL BEOCHO Gabion Cylinder Dia. 500 mm Contranized and Courted with <u>) 000</u> LOG PLLC E 150 000 CONCRETE SOIL FILLING GABION CYLINDER END WAL DESIGN FLOOD PLANE £ Case Land CI WETS 10.000 1 Dist GABON CYL MDER GABION CYLINDER-1 CABION CYLINDER ŝ 8 5 ND V/A GABION CYLINDER - 2 5,250 7,000 6 0 500 9,250 800 J. **LYPE OF WORK:** 000 5 LOCATION : 000 B 500 RIVERBED 3,000 05.1 CCSIGN HIV DCSIGN R 006

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RESULT			3. 832 m²						3,340 m2	7.172 m <sup>3</sup>	•						
1:1.5 CALCULATION	1) For Gabron Cylinder - 1	$\sqrt{1} = (3.0 + 7.826 + 1.0) \times 0.5x3.0 = 17.739 \frac{1}{n^3}$ $\sqrt{2} = 13.907 \frac{1}{n^3}$	× 12 is volume of s/inder V= V, - Vz= 17.739-13.907 = 3.832 m3			2) For Gabin Cylinder -2	V= 13.0+6.310+1.0)×0.5 ×3.0 = 1.5.465 mg	1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1	V = V,-V2 = 15,465-12.125 = 3.340m3	The second s						a second a second second second second second second second a second of the second second second second second	
TYPE OF WORK SLOPE OF 1:11 LOCATION N. S. FILLING WELDOR I DWER CHANN	SOL FILLING SOL FILLING SOL FILLING SOLOTO JOINT FILLER FINOM			END WALL	化学学学校 化化学学学学学学学学学学学 化丁基苯乙基 计正确定置 计计算机 医外外外 计分子分子 计分子分子 化分子分子 化分子分子 化分子分子 化分子分子 化分子分子 化分子分子 化分子分子 化分子分子					化物料学 化分子分子分子 计分子分子 计分子分子 化合物合物 化合物合物合物合物合物合物合物合物合物合物合物合物合物合物合物合物	GABION CYCLINDER 2 TO THE PART OF THE P	0007. 0007. 0007.					

### 2.3 Stone Facing Type

TYPE OF WORK:REVETMENT FOR SIDE SLOP OF 1:2 (STONE FACING)LOCATION:WF.OR -- 65.0 ~ WF.OR + 32.0

		RESULT
CALCULATION		
LOCATION : WF.0R - 65.0 ~ WF.0R + 32.0		
STRUCTURAL EXCAVATION		
B SIRUCIURAL EXCAVATION		
V = 514.147 + 1029.17 =	1543.32	1543.32 m <sup>3</sup>
BEDDING FACING		
F RUBBLE STONE BEDDING		
$A_1 = (7.379 + 2.50 + 3.019) \times 0.40 =$	5.159 m <sup>2</sup>	
$A_2 = (1.00 + 5.814) \times 0.40 \times 2 =$	5.451 m <sup>2</sup>	
$V_1 = 5.159 \times 50.00 =$	257.950 514.147	
$V_2 = 5.451 \times 47.00$	514.147	for the state of the
TOTAL =	514.147	514.147 m <sup>3</sup>
	J14.147	<u> </u>
- CONTERACING		
<b>F</b> STONE FACING		
$A_1 = (2.00 + 7.379 + 2.50 + 3.019 + 2.00) \times 0.60 =$	10.139 m <sup>2</sup>	
	11.111 m <sup>2</sup>	
$A_2 = \{(1.60 + 5.159) \times 2 + 5.00\} \times 0.60 =$		
$V_1 = 10.139 \times 50.00 =$	506.950	
$V_2 = 11.111 \times 47.00 =$	522.217	
		en di la secolatega (
TOTAL =	1029.17	1029.17 m <sup>3</sup>
LOCATION : WF.4R + 16.0 ~ WF.5R + 51.0		
	· · · · · · · · · · · · · · · · · · ·	
STRUCTURAL EXCAVATION		
	007.00	886.80 m <sup>3</sup>
V = 268.30 + 618.50 =	886.80	000.0V III
BEDDING FACING		
<b>日 RUBBLE STONE BEDDING</b>		
$A = 1.118 \times 6.00 \times 0.40 =$	2,683 m <sup>2</sup>	
$A = 1.118 \times 6.00 \times 0.40$		
$V = 2.683 \times 100.00 =$	268.30	268.30 m <sup>3</sup>
E STONE FACING		
$A = (2.00 + 6.708 + 1.00 + 0.60) \times 0.60 =$	6.185 m <sup>2</sup>	
$V = 6.185 \times 100.00$ =	618.50	618.50 m <sup>3</sup>

TYPE OF WORK : LOCATION :

2.4 PC Sheet Pile Wall Type PC SHEET PILE WALL TYPE REVETMENT WF.31L+32.8~WF.38L+25.1

CA	LCULATION		RESULT
STRUCTURAL EXCAVATI	ON		
<u>en la composito de la composito</u>	, le conforma presidente de la constante de la Constante de la constante de la	ана станата на селото на селот На селото на	1997 - 1997 -
$V_1 = \frac{1}{2} \times 0.31 \times 19.964$		= 3.09	
v <sub>1</sub> - 72 x 0.31 x 19.904		- 5.09	
$V_2 = 0.31 \times 53.47 \times \frac{1}{2}$		= 8.29	
A takang seterahan ang akaranan			
	TOTAL	= 11.38	11.4 m <sup>3</sup>
			an ann an Araigh
<b>BACKFILL WITH SELECT</b>	ED SOIL		an a the set
$V_1 = \frac{1}{2} \times 0.34 \text{ m}^2 \times 19.964$		= 3.39	
$V_2 = (0.34 + 11.56) \times \frac{1}{2} \times \frac{1}{2}$	52 17	= 318.15	
$v_2 = (0.34 + 11.50) \times 72 \times 10^{-10}$	JJ;47		
$V_3 = (11.56 + 15.00) x \frac{1}{2} x$	54.30	= 721.15	
$V_4 = (15.00 + 17.86) \times \frac{1}{2} \times 10^{-10}$	34.97	= 722.43	
	44.10	= 702.55	
$V_5 = (17.86 + 13.98) \times \frac{1}{2} \times 10^{-10}$	44.13	= /02.55	
$V_6 = (13.98 + 12.13) \times \frac{1}{2} \times 12.13$	49.15	= 641.65	
$V_7 = (12.13 + 3.83) \times \frac{1}{2} \times 10^{-10}$	48.96	= 390.70	
$V_8 = \frac{1}{2} \times 3.83 \text{ m}^2 \times 24.942$		= 47.76	
			n <u>i sera ana a</u>
	TOTAL	= 3547.73	3547.73 m
a da an an an bha la sa an	and the second		s a star a star a
<u>i de la completa de la completa </u>	will of the start		
and the second			
			a da anti-
2014년 - 영양에서 대표가 승규는 불법을 제			

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<b>1</b>		ļ	]	°,			m	<b></b>		Ng	<b>.</b>			k.			<u> </u>		 T		<del></del>
RESULT				84.125 m <sup>3</sup>			8.076 m3			392.023m <sup>2</sup>				3.365tf				8.413 m <sup>2</sup>			
CALCULATION		·CONCRETE (TYPE-CI)		$V = 2.50 \frac{m^3}{10.00} = 8.50 \times 2 + 223.50 = 89.125$	-GRAVEL BEDDING		V = 0.24 m <sup>3</sup> /10.00m × 336.50 = 8.076		- FORM (H<4.0m)	A = 11.65 M/0.00m × 336.50 = 392.023		· REINFORCING BAR		W = 0.10 t / 0.00 m x 336.50 = 3.365		· JOINT FILLER		$A = 1.25^{m^2}/10.00 \text{ m} \times 336.50 = 8.413$			
COPING CONCRETE	WF. 312 +32.8 ~ WF. 382 +25. )				250	50 200			• • • • • • • • • • • • • • • • • • • •	010			$\frac{1}{11}$	\ <b>240</b>	NG t = 220 mm						
TYPE OF WORK :	LOCATION :							1.1	500	 00 0 90	52		<u>or</u>		GRAVEL BEDDING						

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#### TYPE OF WORK LOCATION

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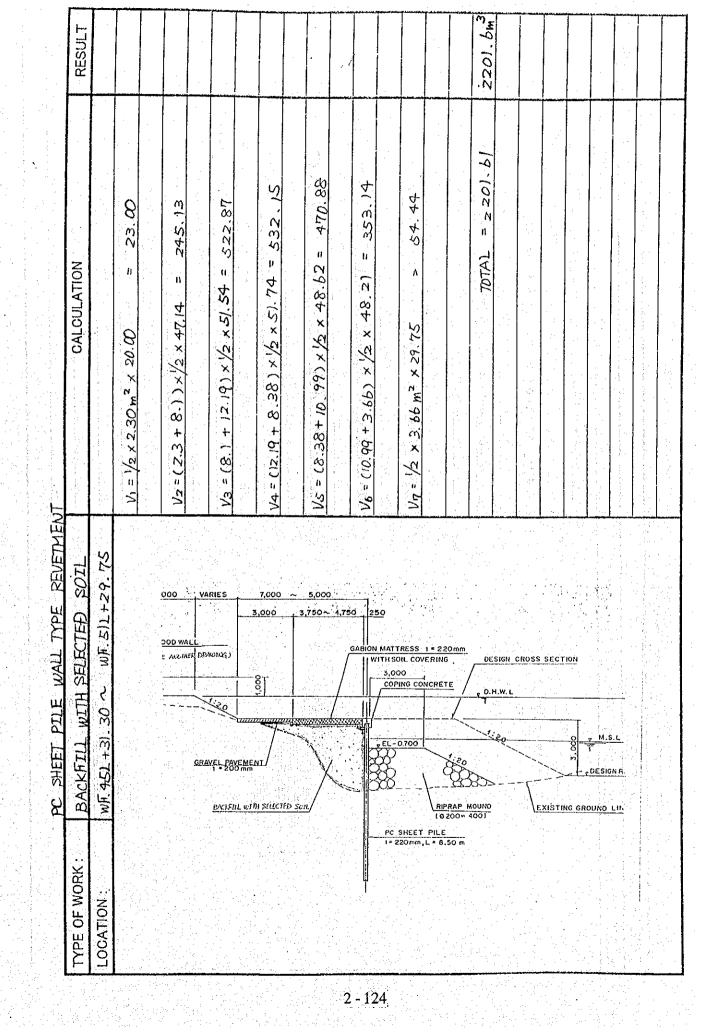
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#### PC SHEET PILE WALL TYPE REVETMENT WF.31L + 32.8 ~ WF.38L + 25.1

	CALCULATION	RESULT
5	STRUCTURAL EXCAVATION	
	$V_1 = \frac{1}{2} \times 0.31 \times 19.964 = 3.09$	
	$V_1 = \frac{1}{2} \times 0.31 \times 19.964 = 3.09$	
	$V_2 = 0.31 \times 53.47 \times \frac{1}{2} = 8.29$	24,620 m <sup>2</sup>
• .		21,020 11
	TOTAL = 11.38	11.4 m <sup>3</sup>
8	PC SHEET PILE	
	PC SHEET PILE	
	Length if pile $L = 9.00 \text{ m} / \text{pile}$	
	L = 9.00  m/pile x (4.47 + 192 + 17  x 2) = 6057.00	6057.00 m
	• DRIVING	
· · · ·		
<u> </u>	Driving Length if pile L = 5.00 m (Average)	
· · · ·	L = 5.00  m/pile x  (447 + 192 + 17  x  2) = 3365.00	3365.00 m
		5505.00 m
11.1		
E.	GABION MATTRESS	
11		
	t = 300 with Soil Covering	
<u>.</u>	$V_1 = 1.75 \times 0.30 \times 203.50 = 106.838$	
1.1	$V_1 = 1.75 \times 0.30 \times 203.50 = 106.838$	
	$V_2 = 1.75 \times 0.30 \times 71.00 = 37.275$	
		in a statistica. Na statistica
	TOTAL = 114.113	114.113 m <sup>3</sup>
· · · ·		
<u></u>		
	an a	
_	n <u>Benergen i de ser de la composition de</u> La Réferencia de la composition de la co	
а, <sup>с</sup>		
12.12		

<b> </b>	1	<u> </u>	960 <del>00</del> -7607,0	[ <b>```</b> ]	 							-	mε	 			-	1
RESULT							-	2					2887.46m				•	
CALCULATION		$\frac{1}{2} \times 7.10 \mathrm{m^2} \times 19.964 = 70.87$		$= (7,10 + 10.59) \times \frac{1}{2} \times 53.47 = 4.72.94$	= (10.59 + 10.00) ×1/2 × 54.30 = 559.02	14 = (10.00 + 10.74) × 1/2 × 43.97 = 455.97	Vs = (10.74 + 10.74) × 1/2 × 44. 13 = 473 96	-	V6 = (10.74 + 8.51) × 1/2 × 49.15 = 4.73.07	= (8.5) + 4.69) ×1/2 × 48.96 = 323.)4	= 1/2 × 4.69 m² × 24.942 = 58.49							
TYPE DE WORK DE DE DAD LADARD											8 <u>/</u>							

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5049.00m z970.00m RESULT 5.00 m (Average) 8 5049.00 2970. L = 8.50m / p; e 4 n CALCULATION Ŋ 5.00 m/bile × 594 piles ો 594 piles • Driving length of pile . PILE × bile 8.50 m/pile of SHEET - BELVING Length (1 2 H 4 PC SHEET PILE WALL TYPE REVETMENT . 75 .30~ W.F.S)L+29. UPSTREAM 267,250 WE 45-3130 DOWNSTREAM £L+0.932 PC SHEET PILE 00 181.0967 EL-0795 +1/800 1= 1/1,950 **H**lbr 8.0 3.026 WF. 452 +31 2 9.700 8, 25 D EL - 2.068 8.750 5,724 EL-2.057 5,750 PC SHEET PILE 1 \* 220mm, L\* / . .... - 1. - 3. 594@ 500 - 297,000 TYPE OF WORK : LOCATION :

2 - 125

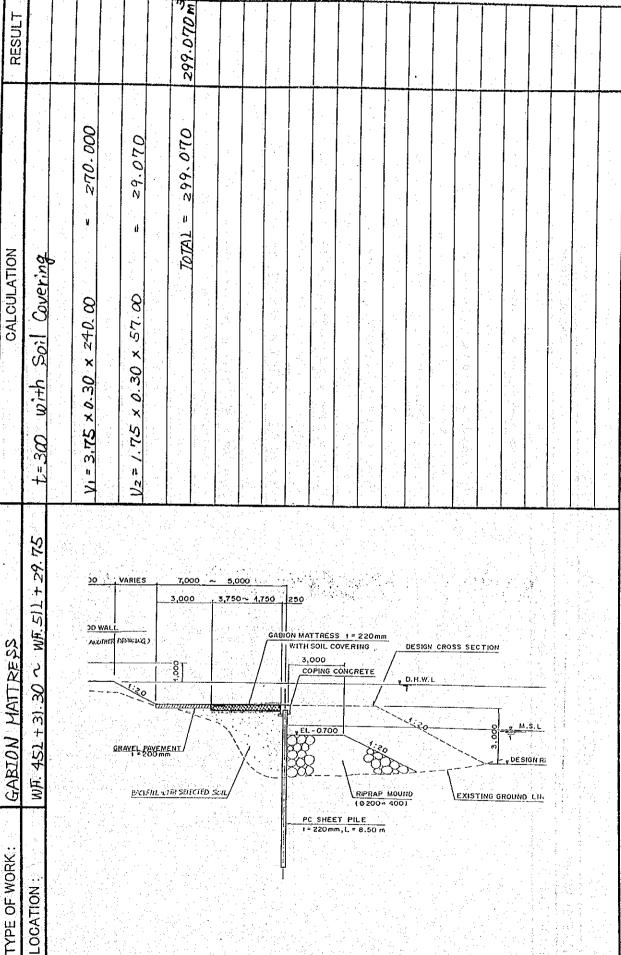
RESULT

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REVETMENT

PC SHEET PILE WALL TYPE

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RESULT													в 		1831.51 m <sup>3</sup>						
CALCULATION		$V_1 = \frac{1}{2} \times 4.43 \text{m}^2 \times 20.00 = 44.30$	V2=(4:43+9.59) × 1/2 × 47.14 = 330.45		$V_{3} = (9, 59 + 6.56) \times \frac{1}{2} \times 51.54 = 416.19$		V4=(6.56+7.21) × /2 × 51.74 = 356.23		VS=(7,2) + 6.65) × 12 × 48.62 = 336.94		V6 = (6.65 + 4.80) × 1/2 × 48.2) = 276.00		VT = 1/2 X 4.80 m2 × 29.75 = 71.40		TDTAL = /83, S)						
PC SHEET PILE WALL TYPE REVET RIPRAD MOUND	+		 0 0 0 0 0 0 0 0 0 0 0 0 0 0	IND-NUCZ	- %}	3.04 8 8 8 8 8 1 1 1 1 1 1 1 1 1 1 1 1 1 1		3.7	50~ 1 50~ 1	750						D.H.M T	<u>// // // // // // // // // // // // // </u>	CCTION Pri STING	 M <sub>1</sub> S.L	R	
OF WORK :	LOCATION :											C SHE * 2201	EET PH	6.50							

74. 250 m 7.128m3 346.005m RESULT (1/2)2.500 m3/0.00m 11.650 m2/0.00m 0.24 m3/10.00m 74.250 346.005 7.128 A= (0.50 × 2 + 0.14) × 10.00 + 0.50×0.50 u 11 Ħ 0 ıt CALCULATION = 2.50 m3/10.00 m × 297.00 n = 0.24 M3/0.00 × 297.00 = 11.65 m2/10.00m × 297.00 V=0.50 × 0.50 × 10.00 - CONCRETE (TYPE - CI) = 0.24×0.10 × 10.00 . BEDDING · FORM (H < 4.0m) GRAVEL R SHEET PILE WALL TYPE REVERMENT Ì Ì > > WF. 451 +3). 30 ~ WF. 5) L +29.75 CONCRETE PC SHEET PILE C1c 300 013 <u>0</u>10 t = 220 mm CONCRETE 50 250 200 500 200 250 COPING ċ 0 G 240 GRAVEL BEDDING Seo SRO TYPE OF WORK : 200 50 200 οĞ 009 001 LOCATION :

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OCATION : WF. 452 + 31.30 2		(2/2)
CALCULAT	ION	RESULT
· REINFORCING BAR		
DID the Lori haf		
Ð13 (W= 1.04 kg/m)		
n Prove		
$n_1 = 7 Bars$		
W1= (10.00-0.05×2) × 7 Bars × 1	.04 = 72.072	
D 10 (w = 0.617 + 8 fm)		
<u>, , , , , , , , , , , , , , , , , , , </u>		
$n_2 = (10.00 - 0.05 \times 2) + 0.30 + 1$	2 0 A D	
	<u>- 34 Bary</u>	
$L = 0.07 \times 2 + 0.40 \times 3 = 1.$	340 M/Bar	
制度 法法律保险法规 医结正病 医白色石		
$N^{2} = 1.34 \text{ m/Bar} \times 34 \times 0.617$	= 28 11 1	
W = 100.	183 kgf = 0.100 tf/10.00m	a
W'= 0.100 tf/10.00m × 297.00	= 2.970	2.970 tf
JOINT FILLER	a de la caractería de la caractería. A como de la como de la caractería de la como de la como de la como de la c	
<u>00+N) /1LLEX</u>	<u>a de la contra de la casa de la casa de</u> Estas de la casa de la c	
A = 0.50 × 0.50		
	= 0.250 m <sup>2</sup> /10.00 m	
$A' = 0.25 \frac{m^2}{10.00m} \times 297.00$	= 7.425	
	<u>, 725</u>	7.425 m <sup>3</sup>
	en e	

2 - 129

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## TYPE OF WORK:PC SHEET PILE WALL TYPE REVETMENTLOCATION:WF.115L + 15.0 ~ WF.117L + 32.0

CALCULATIO	NC		RESULT
STRUCTURAL EXCAVATION			
$A_1 = (0.50 + 0.90) \times \frac{1}{2} \times 0.40$	<u> </u>	0.260 m <sup>2</sup>	
$A_2 = (1.70 + 2.85) \times \frac{1}{2} \times 1.30$	<u> </u>	2.958 m <sup>2</sup>	
	A =	3.218 m <sup>2</sup>	
$V = 3.218 \times 119.50$		384.55	384.55 m <sup>3</sup>
A BACKFILL WITH SELECTED SOIL			
$A_1 = (0.50 + 0.90) \times \frac{1}{2} \times 0.40$	=	0.280 m <sup>2</sup>	
$A_2 = (1.70 \pm 2.85) \times \frac{1}{2} \times 1.30 - (0.90 \times 0.70)$	+ 0.90 x 0.10)		
	=	2.238 m <sup>2</sup>	
V 0.510 110.50	A =	2.518 m <sup>2</sup>	000.00
$Y = 2.518 \times 119.50$		300.90	300.90 m <sup>3</sup>
PC SHEET PILE			
		• • • • • • • • • • • • • • • • • • • •	
PC SHEET PILE	an a		
Length of pile in riverside	$L_1 = 8.50 m$		a de la competencia d
Length of pile in land side	$L_2 = 5.00 \text{ m}$	n general de la sector de la sec En anti-transport de la sector de	
$L = (8.50 + 5.00) \times 239$ sets	<u>, in a la la la set <del>a</del>n prin</u>	3226.50	3226.50 m
• DRIVING		ta (11) e este esta esta a la construcción de la construcción de la construcción de la construcción de la cons La construcción de la construcción d	
Driving length of pile in riverside $L_3 = 4$			
Driving length of pile in land side $L_4 = 5$	,00 m		
$L = (4.20 + 5.00) \times 239 \text{ sets}$	<u>in a l'in teach</u>	2198.80	2198.80 m
FIXING STEEL TIE ROD AND OTHE	RS		
a an	and the second	a de la companya de l	
<ul> <li>STEEL TIE ROD (Ø 32, w = 6.31 kgf/</li> </ul>	m)	n de la substantia de la seconda de la s Seconda de la seconda de la	
n = 119.50 ÷ 2.0	a here for i 😫 👘	60 place	
L = 6.00 + 0.75 x	etter en en en <b>≓</b> rtege	7.50 m / place	
$W_1 = 7.50 \times 60 \times 6.3$	<u> </u>	2839.50	and the second second
• STEEL CHANNEL $(I - 150 \times 75, W = 18)$	3.6 kgf / m)		
$L = 119.50 \times 2 \times 2$	<u>ar in literat</u> ion.	478.00 m	
$W_2 = 487.00 \times 18.6$	5 m sa na sa <b>⇒</b>	8890.80	
• OTHERS	an a	la de la companya de La companya de la comp	
$W_3 = 20 \text{ kgf} / \text{place x 60 place}$	n maalaan <del>di</del> kaa	1200.00	e de la composición d
	and a second second second	te server en en aller an l'han. Na h-server	
TOTAL $(W_1 + W_2 + W)$	3) =	12930.30 kgf	
	1999-1997 - <del>19</del> 94	12.930 tf	12.930 tf
	en e		
RIPRAP MOUND	Alexandra de la T	n a tana araa a tala a	
$A = 10.50 \text{ m}^2$	19 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	and the second	
$V = 10.50 \times (119.50 + 3.00 \times 2)$	=	1317.75	1317.8 m <sup>3</sup>
an an indexe. In familie yn 1997 wr seffer i'r blan ar seffer yn a'r ar	and a set of the second second second	والمراجع والمتحد والمتعيد والمتعاد والمتعاد والمتعاد والمتعاد والمتعاد والمتعاد والمتعاد والمتعاد والمتعاد والم	a state to a state a

## TYPE OF WORK:PC SHEET PILE WALL TYPE REVETMENTLOCATION:WF.45L + 31.30 ~ WF.51L + 29.75

CALCULATION		RESULT
COPING CONCRETE		
• CONCRETE (TYPE – C1)		
V = 0.90 x 0.70 x 10.00 =	6.300 m <sup>3</sup> /10.00 m	
$V^1 = 6.300 \text{ m}^3 / 10.00 \text{ m} \times 297.00 \times 2 =$	150.570	150.570 m <sup>3</sup>
GRAVEL BEDDING		
ORAVEL DEDDING		
$V = \{(0.24 \times 0.10) \times 2 + (10.00 \times 0.45)\} \times 10.00$	0.930 m <sup>3</sup> /10.00 m	
$V^1 = 0.93 \text{ m}^3/10.00 \text{ m} \times 119.50 =$	11.114	11.114 m <sup>3</sup>
• FORM (H < 4.0 m)		
$A = (0.90 \times 2 \times 2 + 0.35) \times 10.00 + (0.90 \times 0.70)$		a la ser que en el
	40.130m²/10.00 m	
$A^{1} = 40.13 \text{ m}^{2}/10.00 \text{ m} \text{ x } 119.50 =$	479.554	479.554 m
• REINFORCING BAR		
D 13 (W = 1.04 kgf/m)		
$n_1 = 10 Bars$	n an	
$W_1 = (10.00 - 0.05 \times 2) \times 10 \text{ Bars } \times 1.04 =$	102.960	
D 10 (W = $0.617 \text{ kgf/m}$ )	an a	s Na sang sa sa sa Tang sa sang sa sa sa sa
$n_1 = (10.00 - 0.05 \times 2) : 0.30 + 1 =$	34 Bars	
a an		
$L = 0.25 + 0.80 \times 2 + 0.60 + 0.05 =$	2.500 m / Bar	
W <sub>2</sub> = 2.50 m / Bar x 34 Bars x 0.617 =		
W = 155.405 kgf. =	0.155 tf / 10.00 m	
$W^{1} = 0.155 \text{ tf} / 10.00 \text{ m} \times 119.50 \times 2 =$	3.705	3.705 tf
• JOINT FILTER		
$A = (0.90 \times 0.70) \times 2 - 0.65 \times 0.22 \times 2 =$	0.974 m²/10.00 m	
$A^{1} = 0.974 \text{ m}^{2}/10.00 \text{ m} \text{ x } 119.50 =$	11.639	11.639 m <sup>3</sup>

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#### 2.5 Leaning Wall (Concrete Type) TYPE OF WORK : LEANING WALL (CONRETE TYPE) ; WF.110L - 45.0 m ~ WF.111L + 15.0 m

LOCATION

CALCULATION RESULT STRUCTURAL EXCAVATION See Attached Table 2115.224 m<sup>3</sup> V = 2115.224BACKFILL WITH SELECTED SOIL See Attached Table 462,726 m<sup>3</sup> V = 462.726**BACKFILL WITH GRAVEL** See Attached Table V = 481.888481.888 m<sup>3</sup> **CONCRETE TYPE D** See Attached Table 1015.096 m<sup>3</sup> CONCRETE : V = 1015.0969940.880 m<sup>2</sup> FORM : A = 9940.880**RUBBLE STONE BEDDING**  $= 2.30 \times 0.20 \times 15.00 + 2.60 \times 0.20 \times 10.00$ + 2.85 x 0.20 x 10.00 + 3.00 x 0.20 x 10.00 x 2.00 + 3.15 x 0.20 x 10.00 x 4.00 + 2.85 x 0.20 x 15.00 63.550 63.550 m<sup>3</sup> \_ **DEFORMED REINFORCING BARS** D 13 (1.04 kg. f/m) $= \{(110.00 - 0.05 \times 2) / 0.5 + 1.00\} \times 6.00$ 1325 = n  $= 1325 \times 0.80$ 1060.000 m L = -= 1102.400 kg.f 1102.40 kgf  $W = 1060.00 \times 1.04$ JOINT FILLER See Attached Table A = 87.293 87.293 m<sup>2</sup> WEEP HOLE **PVC PIPE ø 50** 1.  $\overline{n_1} = 85.00/2.00$ = 43  $L_1 = 0.80 \times 43$ 34,400 =

# TYPE OF WORK: LEANING WALL (CONRETE TYPE)LOCATION: WF.110L - 45.0 m ~ WF.111L + 15.0 m

	LCULATION	DECIN C
		RESULT
$\frac{2. n_2}{L_2} = \frac{110.00 / 2.00}{1.10 \times 55}$	= 55	
$L_2 - 1.10 \times 33$	= 60.500	
2 - 110.00./2.00		
$3. n_3 = 110.00 / 2.00$	= 55	
$L_3 = 1.50 \times 55$	= 82.500	
$4. n_4 = 110.00 / 2.00$		
$\frac{4. \ n_4}{L_4} = 2.00 \ x \ 55$	= 55	
1.4 - 2.00 x 55	= 110.000	
	TOTAL L = 287.400	007.400
n an an an Arrena an Arrena an Arrena an A	TOTAL L = 287.400	287.400 m
FILTER CLOTH	(1) The second s second second s second second s second second se second second se	
	n for a standard and a standard and An an	
$\overline{N} = n_1 + n_2 + n_3 + n_4$	= 208 places	
$A = 0.640 \text{ m}^2 / \text{place x 208}$	= 133.120	133.120 m <sup>2</sup>
	133-124	
STEEL FENCE		
L = 110.117		110.117 m
See "Form"		
e no novel prejegi Algebra de la estructura.		
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	ulio (1994) e u parte de trates a la	
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TYPE OF WORK

#### LEANING WALL (CONCRETE TYPE)

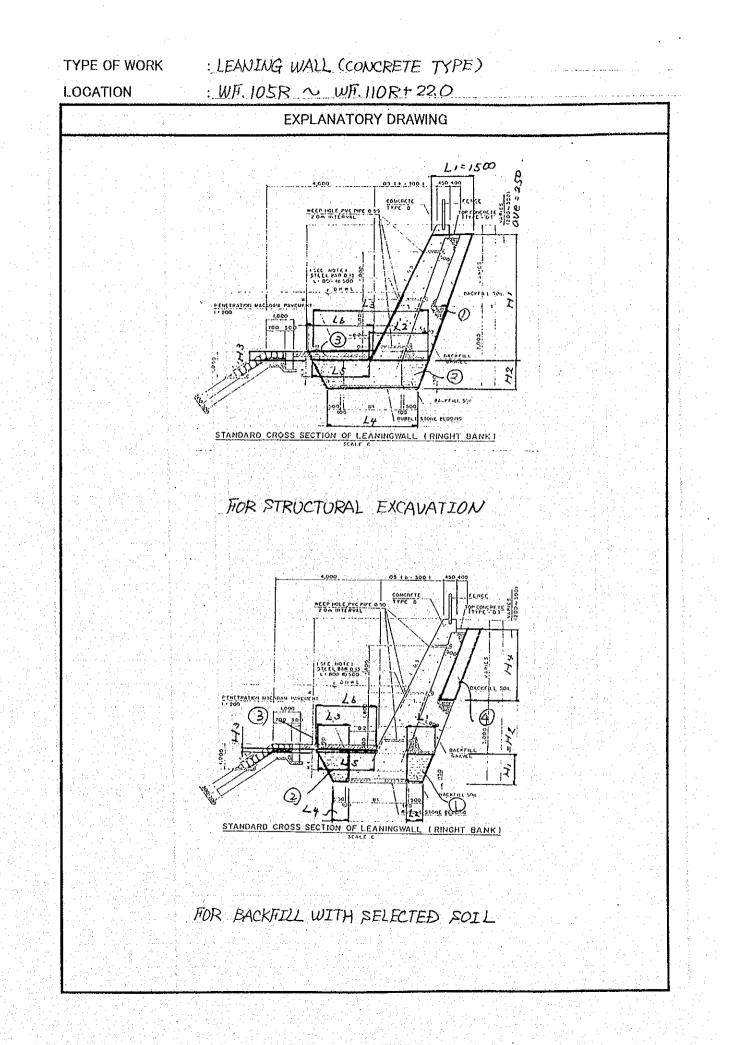
### STRUCTURAL EXCAUATION

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		2	E	17	7	2	Ŧ	181	2				5	20	8	ñ	22				3	ŝ	2	ŝ	5	Ŧ	222		
		volume	248,231	172.244	172.244	171.194	170.144	167.181	163.190	161.135	504.001	148 684	146.674	140.676	134.700	132.735	126.976	120.279	110 003	103 613	105.248	110.679	114.519	118.359	124.051	57.741	5 m		
			.	.								<u> </u>	_				1				1					_	~		
			14.500	10.000	10.000	10.000	10.000	10.000	10.000	10,000		10.000	10.000	10,000	10,000	10.000	10:000	10.000	10,000	000 01	10.000	10.000	0.000	10.000	10.000				
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			17.119	17.224	17.224	17 119	17 014	16.718	16.319	16 114	15 060	14 868	14.667	14 068	13 470	13.274	12.698	11 644	1000	10.361	10.525	11.068	11 452	11 836	12,405	1			
	- F	1-	17	7	5	   →	4	ន	<u>의</u> :	<u> </u>  2	20	000	57		2	2		20	। । १	2	2	0	1			-			 
	<u> 1+47-43</u>	17:014	17.224	17.224	17.224	17.014	17.014	16.422	16.216	10.011	14 969	14.768	14.567	13:568	133	13.175	12.220	11.630	10.549	10.174	10.876	11.260	11.644	12.0	12,782	2			
	+																												
		0.668	0.668	0.668	0.668	0.668	0.668	0.657	0.657	0 630	0.632	0.632	0.632	0.606	0.606	0.606	0.581	0.581	0.555	0.555	0.581	0.581	0.581	0.581	0.000	P)	1		
	13	S		0		0						0		Ĉ	ິ	°				0	0					×   .		<u> </u>	<u>la com</u>
<u>e de la composición de la com</u>	.	0.300	0.300	0.300	300	0.300	0.300	0.300	0.300	0.300	0.300	0.300	0.300	0.300.	0.300	0.300-	0.300	01200	0.300	0.300	0.300	0.300	0.300	0.300	0.200	3			
and a second	H		C	C		0						9		-						C	°	0							
	u	2.375	2.375	2.375	2.375	2.375	2.375	2,340	2 2 40	255	2.255	2.255	255.	170	2.170	2.170	C807	085	000	2.000	2.085	2.085	2.085	88	2.170	2			
	Excavation		2	7	7	2	2	~ 6	н (с	10	1	2	7	2	-	~	, c	*	Ŕ	2	N	2	ମ୍ବ	ri (	~ r	1			
		2.075	2.075	2.075	2.075	2.075	2.075	7.040	2.040	1.955	1.955	955	1:955	8	1.870	1.870	1 795	1.785	1.700	1.700	1.785	1.785	785	1.735	1.870				
	Structural L5		2	2	2	2			2	1	-	-	ី	-	- -	-  -	- -	-		1	~	-		-  -	- -				
	I for S	4 902	4.902	202	+ 902	4.902	4.902	1000	200 +	4.187	4.187	4.187	4.187	3 825	3.825	(72.5	1470	3.479	3.150	3.150	3 479	3.479	3.479	14/9	3 825				
	Calculation for		+	4	+	4	4	4 -	+ -	3	Ţ	4	ব	<b>m</b>	m e	~ (·	ĥ	6	3	m	e.	n l	n (	<u>,</u> , ,	'n				
	Calci	+	1.150	1.150	2	1.150	2	3		1.050	1.050	1.050	1.050	1.000	0001	1,000	0.050	0.950	0.900	0.900	0.950	0.950	0.950	0001	1.000				
	Table.		-	-	- -	-   ·		- -	- -	-		-				- 6	č	0	0	o	0	6		3	ľ				
	`		3.750	22	3.750	220	0000	1000		3.500	3.500	3.500	20	3.350	10000	000 8	8	3.200	3.050	3.050	3.200	007 5	002.5	3 6	3.350	i. U			
	3	3	'n	ε	e l	m r	n (	n e		ſ,	3	ĉ	e e		10	<u>,</u>	n m	6	3.0	3	с. С. (	2	7		2				
		775	775	13	Ê	113	0.05		520	175	175.	2	175	000		20	5	25	50	50	25	2	۹×	38	88				
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			\$55	3	S	<del>1</del> 45	2 C	20	68	152	51	9.950	9.749	151.2	8-744	8 I60	7.776	7.392	6.844	6.469	6.816	N07-1	1.068	815	8.450	1			
<u> </u>	AI	11,445	11.655	11.655	11:655	11.445	11 200	10.004	10.789	10.352	10.151	6	2	151.2	ċ	° «	1	7	6.8	6.4	9	1	10	8.351	8.4				
leter geboord. <del>Heren</del>	Ì	5.450	5.550	5.550	20	5.450	1024.5	5 350	5.250	5.150	5.050	4.950	4 850		3 5	4.750	8	50	3	20	2	35	2 ¢	20	8			· · · · · · · · · · · · · · · · · · ·	· · · · ·
	н	ŝ	5			ň		5	S	5.1	5.0	4	4 850	100.4	4 450	4	4.050	3.850	3.650	3.450	3.550	050 5	4 150	4 250	4.300				
	-	2.700	8	2,700	00/ 7	318	3 5	20	0	20	20	20	3			39	40	<del>6</del>	2	2				0	30				
	5	2.7	2.700	2	1	00/-7	2.700	2610	2.610	2.520	2.520	2.520	0707	124.20	7 430	2 340	2.340	2.340	2.250	0077	2.340	016.0	2.340	2.430	2.430				
	Ĩ	500	88	1.500	<u> </u>	000-1	38	88	88	8	8	8	318	315		88	8	8	81	3	3 2		38		l. Q		na – Priše Postava se postava		
·	<u> </u>	1.5	1.500	2	- -	1 200	1 500	1.500	1.500	1.500	1.500	1.500	000 T	007.1	1 500	1.500	1.500	1.500	1.500	<u> </u>	1 500	1 500	1.500	1.500	1.500				
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<u></u>													[](			10		-		1		<u></u>		নত	1	TILOOL
·	volume(m3)		57.012	20,202	39.319	39.069	38.429	37.540	35 88.2	34.724	34,224	33.724	32.322	074.05	29.024	27.379	26.379	24.735	23.489	24.879	25.879	26.879	28.27	832.966		
	volun						0				0	0.	0				0	0	2 2		2	0		2		
	distance(m)		14 50	10,000	10.00	10.00	10 00	10.000	200	10.01	10.00	10.00	10.000	0		10.01	10.000	10.00	10.000	10.0(	10.000	10.000	10.000	4.00C		
	cist:		8	5	1	5	<del>2</del>	3.754	2 688	3 472	8	72	3.232		10	1 8E	538	2 474	0755	2.488	2.588	2.688	2.827	2.930		
	ave										1			4												
<u> </u>	V3+A4	3.907	3.957	3.957	100.6	3.907	3.779	3.729	2107	3.447	3.397	3.347	3.117	3.067	2.017	2.688	2.588	2.360	2.438	2.538	2.638	2.738	2.917	2.942		
	+72+																									
		725	775	1.775	12	1.725	8	1.650	3 Ş	475	425	375	-250	ŝ		925	0.825	20	009.0	77.0	0.875	0.975	0.00	510		
	V																	2								
	1		3.55	3.550	24.5	345	3.400	3.30	07 F	6 6 6	285	2.75	2.500	24	200		1.650	1 400	5	1.550	1.750	1.950	2.100	51		
	F	0.50	0.500	0.50	002:0	0.500	0.500	0.500	000	300	0.500	0.500	0.500	0.50	88		0.500	0.500	0.500	0.500	0.500	0.500	0.50	0.50		
	17			٠ļ				0.209			17	1020	0.192	2	5	0184	0.184	0.175	0.175	1810	0.184	0:184	0.192	8	_	
	ctcd Sol	2		•		1.			·			Ľ														
	vilh Sele	0100	0.100	0.100	00100	200	0.18	010	8	010		0010	0.100	0,100	8	202	0.100	0.100	010	010	0.100	0.100	0.100	0.10		
	Calculation for Backfill with Selected	<u> </u>	2.175	175	2	c/ 1	19	140	2		ŝ	390	970	970	220	282.00	1 885	800	1 800	1 885	1.885	1.885	1.970	010		
	ion for E	3	ŀ		ŀ					1		1									1		·			
	Calculat	2015	10.2	2.07	2.07	207	2.040	2:040	2.04	1.95	5	10	1.870	1 87	1.87	80 P	1,28	1.700	21	CR/ 1	1.78	1.785	1.87	1.87		
	Table	10	15	1.021	1.021	1.02	150.0	0.963	0.963	906	0.50	0000	0.850	0.850	0.850	0.796	0.796	0.743	0.743	0.796	0.796	0.796	0.850	0.850		
<u> </u>		140 V2	3 S	1.150	5	1.150							1	2	000;1	0.950	0.950	0.900	0.900	0.950	0.950	0.950	1.000	1.000		
		2				1	•													-						
		2,50	0,600	0.60	0,600	0.60	0000	0.600	000	0.60		0000	8090	0.600	0.600	88	0.600	0.600	0.60	0.600	0,600	0.600	0.600	0.600		
		11	2/11	1175	1.175	1.175	1.150	150	1.150	1.125	1.125		001	1 100	1.100	1.075	2/0.1	1.050	1 050	1 075	c/01	075	1.100	1.100		
		<u>~</u>	<u> </u> .				:										т, I-,						<u>.</u>			
		<u></u>	0.949	0.949	0.94	0.949	0.04	0.908	0.908	0.866	0.86	0.800	0.825	0.82	0.825	0.78	0.784	0	0.743	0.784	0.784	0.784	0.825	0.825		
		_		1.150	2	1.150	120	1.10	1.100	1.050	02	000	202	8	1:000	0.950	0.950	860	0.900	0.950	0560	0.950	1.000	1.000		
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			(iii		55.825	8.750	38.750	38.500	38.250	38.388	38.275	7.775	37.413	37.050	36.550	36.050	5.438	4.825	34.325	3.713	2.850	31.850	0.988	221.0	29.738	0.350	350	2.350	32.963	14.940	907.378					. ·			<u>.</u>		
			volume(m3)		ŝ	м М	ю.	е,	ň	m	m   	in	ŝ	'n	ě	ň	ы М	ň 	ľ.	3	3	3	Ř	)   	15	1 N	31	33	33	1	606	<del>.</del>			-	: .					:
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			distance(m)		14.500	10.000	10.000	10.0	10.0	10.000	10.0	10.000	10.0	10.0	10.0	10.0	10.0	10.0	10.000	10.000	10:000	10,000		10.000	10,000	10,000	10,000	10.0	10.0	4.500						<u>.</u>		<del>.</del>		<u> </u>	<u>.</u>
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					3.850	3 875	3.875	3.850	3.825	3.839	3.828	3.778	3.741	3.705	3.655	3.605	3.544	3.483	3.433	3.371	3.285	3.185	3.099	3.013	2.974	3.035	3.135	3.235	3.296	3.320		·		 -					 	 • .	- <u></u>
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		Gravel	A1+A2.	3.825	3.875	3.87	3.875	3.825	3.82	3.853	3.803	3.753	3.73	3.680	3.63(	3.580	3.50	3.45	3.408	3.335	3.235	3,135	3.063	2.963	2.985	3.085	3.185	3.285	3.308	3.333					<u> </u>		· · · ·		· · · · ·		
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		Calculation for Backfill with	2	2.100	2.100	2.100	2,100	2.100	2.10	2.153	2.153	2.153	2:20	2.205	2.205	2.205	2.25	2.258	2.258	2.310	2.310	2.310	2.363	2.36	2.310	2.310	2.31	2.310	2.258	2.25		. * 	<u></u>				<u></u>				
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		tion fo	5	2.000	2,0	2,0	2.0	5	50	2.050	2.050	2.050	2.1(	2.100	2,100	2.100	2.150	2.1	2.150	2.200	2.200	2.200	2.250	2.2	2:2(	2.200	2.200	2.200	2.150	2.15					-		41 - 1 - 1 10 - 1 10	· ·			-
		alcula	H	050	50	020	1.050	1.050	50	50	0	1.050	1.050.	50	50	1.050	50	1.050	1.050	50	1.050	50	1.050	50		1.050	1.050	ខ្ល	1.050	50	_	-			i- I	<u>- 197</u> - 197 - 197			<u></u>		
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•		Ĥ		1.725	775	1.775	1 775	725	1.725	1.700	1.650	1.600	1.525	475	125	375	1 250	1.200	1.150	1.025	0.925	0.825	0.700	0.600	0.675	5	375	0.975	1.050	1.075	-		- -				12				
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·				3.450	550	3.550	3.550	3.450	3.450	400	3.300	3.200	3.050	950	850	750	<u>5</u> 0	<del>4</del> 0	2.300	80	850	1.650	1.400	1.200.	1.350	1.550	25	1.950	2.100	150				-				 	11 11 11 11 11 11 11 11 11 11 11 11 11		
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LEANING WALL (CONCRETE TYPE)

TYPE OF WORK

#### LEANING WALL (CONCRETE TYPE) CONCRETE (TYPE-D)

	TION						<u>W</u> 府 ILATIO		1 22	<u>. V</u>			R	ESULT	
	1. 	n en 1910 - Anna An			T	ble. Calculatio	n for Concrete			· · ·					
	soction	L1 0.450	L2	H1 5.700	A1 5.985	ave	distance(m)	V1(m3)	1.3 2.550	H2 1.000	A2 2.550	distance(m) 14.500	V2(m3) 36.975	V1+V2(m3) 36.975	•
,	2	0.450	1.650	5.800	6.090	6.038	14.500	87.544	2.550	1.000	2.550	10.000	25.500	113.044 86.400	
	3	0.450	1.650		6.070	6.090 6.090	10.000	60.900	2.550	1.000	2.550	10.000	25.500	86.400	
		0.450	1.650	5,700	5,985	6.038	10.000	60.375	2.550	1.000	2.550	10.000	25.500	85.875 85.350	
<b></b>	6	0.450	1.650 1.560		5.935 5.729	5.985 5.857	10.000	59.850 58.568	2.450	9.950	2.328	10.000	23.275	81.843	
	- 8	0.450	1.560		5.628	5.678 5.578	10.000	56,783 55,778	2.450	0.950	2.328	10.000	23.275	80.058 79.053	
	10	0.450	1.47	. 5,400	5.184	5.356	10.000	53.558	2.300	0.900	2.070	10.000	20,700	74.258	
	11 1::	0.450	1.470		5.088 4,992	5.136	10.000	51.360 50.400	2.300	0.900	2.070	10.000	20.700	72.060	
	13	0.450	1.470	5,100	4 896	4 944	10.000	49.440	2.300	0.900	2.070	10,000	20.700	70.140 65.173	
	14	0.450	1.38		4.484 4.392	4.690	10.000	46.898 44.378	2.150	0.850	1.828	10.000	18.275	62.653	1
	16	0.450	1.380		4 301	4,346	10.000	43.463	2.150	0.850	1.828		18.275	61.738 57.078	<u></u>
	17	0.450	1.290		3.915	4.108	10.000	38.280	2.000	0.800	1.600	10.000	16.000	54.280	1
	19	0,450	1.29		3.567	3.654	10.000	36.540	2.000	0.800	1.600		16.000	52.540 47,798	
	20 21	0.450	1.20	3.700	3.053	3,135	10.000	31.350	1.850	0.750	1.388	10.000	13.875	45.225	
	22 23	0.450	1.29		3.306 3.480	3.179 3.393	10.000	31.793 33.930	2.000	0.800			16.000	47.793 49.930	Ì
	24	0.450	1.29	4.200	3.654	3.567	10.000	35.670	2.000	0.800	1.600	10.000	16.000	51.670	
	<u>25</u> 25	0.450	1.29		3.828	3.741	10,000	37.410 39.728		0.800				53.410 47.951	
	27	0,450	1.38			4.140		18.632	2.150				519.899	18.632 1738.422	
<u> </u>	<u></u>	L	<b>1</b>	<mark>al di diana</mark> Ng		L	<u>15 - Carlon Sta</u> Al Color Status Sta	1 1218,523	<u> </u>	<u></u>	1.000	100.01	1	1 1122.744	Ť
i		n for Conci distance		07	lw .	V(m3)	1	<b>n</b>	1 A.	TOTA	L(m3)	7	1738.422	a da ser da s	
	block L1	14.500	14.50	0.150	_	0.870			1	Wall	1738.422				
<u> </u>	1.2	10.000	10.00			0.600		-	t in the second se	Top Concrete	15.541			na an taon An taona an taon	
	<u>L3</u>	10,000	10.00	0,150	0.400	0.600		1	11 A.	••••••••	1 11.5.703	ц			
1	15 16	10.000	10.00			0.600		-	د برد در هر در در						
	L7	10.000	10.00	0.150	0.400	0.600								$(f_1, g_2) \in f(g)$	<u> </u>
	L8 L9	10.000	10.00			0.600			ena poste de la consecue. No		$(1,1) \in \mathbb{R}^{n}$	et i se	i por d		-
· •	Lio	10.000	10.00	0.150	0.400	0.600		1		e statula					
	L11 L12	10.000	10.00	0.150	0.400	0.600									
	<u>L 3</u> L14	10.000	10.00			0.600		- · ·							Ĺ
	L15	10,000	10.00	0.150	0.400	0.600		1					· · ·		
<u> </u>	L16	10.000	10.00			0.600				an tha th	19 9 L		· · · ·	· · · · ·	· -
	L18	10,000	10.00	2 0.150	0.400	0.600								· · ·	1
÷	L19 L20	10.000	10.00			0.600						t signa i			
	L21 L22	10.000	10.00			0,600			the second			· · ·		· · · ·	_
	L23	10.000	10.00	2 0.150	0.400	0.600	$(g_{1}, g_{2}) \in [0, \infty)$				e fer etc. S			an an	
<u>.</u>	L24 L25	10.000				0,600					· · · ·	۰. · · ·			
<u> </u>	L.26	4.500				0.270			ala. Alamatika		An an An An An	ч.). Т	÷.,		-
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