

## 2.4 Distribution System

### 2.4.1 Hydraulic Network Analysis

From the distribution pumping station, water will be transmitted to the town through distribution trunk main which will be installed along the National Road No. 6. Hotels which are located along the National Road No. 6 will be supplied from branches on the trunk main.

According to the future land use plan, new hotel development project will take place in the northeast part of the town. Water supply system for the new hotel development area will be a separated system from the town water supply. Water for the new hotel development area will be available at the branch on the northern end of the distribution network and water will be supplied by bulk. Transmission pipeline from this branch to the new hotel area will be installed by hotel developers.

Water transmitted by the trunk main will be distributed to entire service area. Future distribution network model for the Stage 1 is established from the results of the hydraulic network analysis conducted in the Master Plan.

Planned Stage 1 distribution network is checked by computer network analysis. Network analysis is conducted based on the following condition.

- Basic water demand is hourly maximum water demand in year 2006
- C-value for new pipe is 120 and 110 for existing pipeline.
- Hourly peak factor is 1.2. (The peak factor is not applied for bulk water supply for the new hotel development area)
- Minimum residual pressure should be more than 15 m.
- In the case that existing pipe is not enough for future distribution, the existing pipe will be replaced by the new pipe and the existing pipe will be abandoned.

As the first step of the hydraulic network analysis, distribution network model was established as shown on Figure 2.4.2. As the second step, water demand in the service area is allocated to each node. Domestic water demands in 2006 for each village included in the service area in the Stage 1 by daily average, daily maximum and hourly maximum basis are shown in Annex 4.5.1. Hourly maximum water demand is used for the analysis. Number of nodes which included in respective village are calculated and ratio of demand is allocated as shown on Table 2.4.1. From hourly maximum water demand in each village and ratio allocated to each node, domestic water discharge from each node is calculated as shown on Table 2.4.2.

For non domestic water demand, basic water demands, daily average, daily maximum, and hourly maximum are as shown in Annex 4.5.1. Non domestic water discharge is calculated from the allocated ratio which is shown on Table

2.4.3 and water demand in each category and node discharge is calculated as shown on Table 2.4.4.

From the domestic water and non domestic water discharge, total water discharge at each node is calculated as shown on Table 2.4.5.

Using calculated water discharge and pipeline data, hydraulic analysis is executed and the results of the analysis is shown on Table 2.4.6.

Proposed future distribution network which was confirmed by its capacity and rationality is shown on Figure 2.4.1 and network diagram which shows pipe diameter and length is shown on Figure 2.4.2. Service area is separated by the Siem Reap river and eastern and western service area will be connected by two new pipe bridge.

Length of the proposed distribution network is as shown in Table 2.4.7.

**Table 2.4.7 Length of Distribution Pipeline by Diameter**

Diameter (mm)	Stage 1		Remaining Existing pipe (m)	Total (m)
	New Installation (m)	Replacement (m)		
500	7,450	-	-	7,450
450	710	-	-	710
400	-	166	-	166
350	-	254	-	254
300	-	230	-	230
250	360	900	509	1,769
200	2,630	92	354	3,076
150	765	1,169	1,964	3,898
100	1,860	3,499	3,005	8,364
75	3,250	-	-	3,250
<b>Total</b>	<b>17,025</b>	<b>6,310</b>	<b>5,832</b>	<b>29,167</b>

**Table 2.4.1 Domestic Water Demand Distribution to Each Node (Ratio in %)**

Node Number	Communes and Villages															
	Svay Dangkm					Sala Kamraeuk					Sla Kram					
	Svay Dangkm	Vihear Chen	Mundol1	Stung Thmey	Mundol2	Ta Phul	Saka Kanseng	Wat Bo	Vat Donmak	Sala Kamraeuk	Sla Kram	Banteay Chas	Boeung Donpa	Dok Po	Ta Vein	Chong Kaosu
00																
1								10.0			16.7					
2								10.0			16.7					
3								10.0								
4								10.0	6.3							
5									6.3							
6									6.3							
7									6.3							
8									6.3							
9								10.0			16.7					
10								10.0								
11								10.0								
12								10.0	6.3							
13									6.3							
14									6.3							
15									6.2							
16									6.2							
17									6.2							
18									6.2							
19					3.8											
20													14.2			
21													14.3			
22													14.3			
23											16.7					
24					3.8											
25					3.8											
26					3.8											
27					3.8											
28			6.2		3.8											
29			6.2													
30			6.3													
31			6.2													
32			6.2													
33			6.3													
34			6.3													
35	12.5	33.0	6.3													
36			6.2													
37			6.2													
38			6.3													
39			6.3													
40	12.5		6.3													
41	12.5		6.3													
42	12.5		6.2													
43	12.5		6.2		7.8											
44					7.8											
45					3.8											
46					7.7											
47					7.7											
48					3.8											
49					7.7											
50					3.8											
51					7.7	20.0										
52					3.8											
53						33.0										
54								6.2								
55													14.3			
56								10.0			16.5	33.0				
101																
102																
103																
104						20.0	33.0									
105						20.0	34.0									
106				10.0		20.0										
107				30.0	7.8	20.0										
108	12.5			30.0												
109	12.5			30.0	7.8											
110	12.5	33.0														
111		34.0														
112								6.2								
113								6.2								
114									100.0							
115								10.0	6.2							
116													14.3			
117													14.3			
118													14.3			
119											16.7					
120												33.0				
121												34.0				
122														34.0		34.0
123														33.0		33.0
124														33.0		33.0
Total	100.0	100.0	100.0	100.0	100.0	100.0	100.0	100.0	100.0	100.0	100.0	100.0	100.0	100.0	0.0	100.0



**Table 2.4.3 Non Domestic Water Demand Distribution to Each Node (Ratio in %)**

Node Number	Hotel Water Demand	Bulk Water Supply to New Hotel Area	Restaurant Water Demand	Special Use Water Demand
00				
1	3.3		1.25	
2			1.25	
3			1.25	
4	3.3		1.25	
5			1.25	
6	3.3		1.25	
7			1.25	
8			1.25	2.30
9			1.25	2.30
10			1.25	
11			1.25	
12	3.3		1.25	
13	3.3		1.25	
14			1.25	
15	3.3		1.25	
16			1.25	
17			1.25	
18			1.25	
19			1.25	2.35
20	3.3		1.25	2.30
21			1.25	2.30
22			1.25	
23			1.25	
24			1.25	2.30
25			1.25	2.30
26			1.25	2.35
27			1.25	2.35
28			1.25	2.35
29			1.25	2.35
30			1.25	2.35
31			1.25	2.35
32	3.3		1.25	2.35
33			1.25	2.35
34	3.3		1.25	
35			1.25	2.30
36			1.25	2.30
37	3.3		1.25	2.30
38			1.25	2.30
39			1.25	
40			1.25	
41	3.3		1.25	2.35
42	3.3		1.25	2.35
43	3.3		1.25	2.35
44	3.3		1.25	2.35
45			1.25	2.35
46	3.3		1.25	2.35
47	3.3		1.25	2.35
48			1.25	2.35
49	3.3		1.25	2.35
50			1.25	2.35
51	3.3		1.25	2.35
52			1.25	2.35
53	3.3		1.25	2.35
54			1.25	
55			1.25	2.30
56	3.3		1.25	
101	3.4		1.25	
102	3.4		1.25	
103	3.4	100.00	1.25	
104	3.4		1.25	
105	3.4		1.25	
106	3.4		1.25	
107	3.4		1.25	
108			1.25	
109			1.25	
110			1.25	
111			1.25	
112			1.25	
113			1.25	2.30
114			1.25	2.30
115			1.25	2.30
116			1.25	2.30
117			1.25	2.30
118	3.6		1.25	2.30
119			1.25	
120	3.3		1.25	2.30
121	3.3		1.25	2.30
122			1.25	2.30
123			1.25	2.30
124			1.25	
<b>Total</b>	<b>100.0</b>	<b>100.0</b>	<b>100.0</b>	<b>100.00</b>

**Table 2.4.4 Non Domestic Water Demand Distribution (Hourly Peak Demand Base in l/sec)**

Node Number	Hotel Water Demand	Bulk Water Supply to New Hotel Area	Restaurant Water Demand	Special Use Water Demand	Total
00	0.000	0.000	0.000	0.00	0.000
1	0.771	0.000	0.099	0.00	0.870
2	0.000	0.000	0.099	0.00	0.099
3	0.000	0.000	0.099	0.00	0.099
4	0.771	0.000	0.099	0.00	0.870
5	0.000	0.000	0.099	0.00	0.099
6	0.771	0.000	0.099	0.00	0.870
7	0.000	0.000	0.099	0.00	0.099
8	0.000	0.000	0.099	0.07	0.170
9	0.000	0.000	0.099	0.07	0.170
10	0.000	0.000	0.099	0.00	0.099
11	0.000	0.000	0.099	0.00	0.099
12	0.771	0.000	0.099	0.00	0.870
13	0.771	0.000	0.099	0.00	0.870
14	0.000	0.000	0.099	0.00	0.099
15	0.771	0.000	0.099	0.00	0.870
16	0.000	0.000	0.099	0.00	0.099
17	0.000	0.000	0.099	0.00	0.099
18	0.000	0.000	0.099	0.00	0.099
19	0.000	0.000	0.099	0.07	0.171
20	0.771	0.000	0.099	0.07	0.941
21	0.000	0.000	0.099	0.07	0.170
22	0.000	0.000	0.099	0.00	0.099
23	0.000	0.000	0.099	0.00	0.099
24	0.000	0.000	0.099	0.07	0.170
25	0.000	0.000	0.099	0.07	0.170
26	0.000	0.000	0.099	0.07	0.171
27	0.000	0.000	0.099	0.07	0.171
28	0.000	0.000	0.099	0.07	0.171
29	0.000	0.000	0.099	0.07	0.171
30	0.000	0.000	0.099	0.07	0.171
31	0.000	0.000	0.099	0.07	0.171
32	0.771	0.000	0.099	0.07	0.942
33	0.000	0.000	0.099	0.07	0.171
34	0.771	0.000	0.099	0.00	0.870
35	0.000	0.000	0.099	0.07	0.170
36	0.000	0.000	0.099	0.07	0.170
37	0.771	0.000	0.099	0.07	0.941
38	0.000	0.000	0.099	0.07	0.170
39	0.000	0.000	0.099	0.00	0.099
40	0.000	0.000	0.099	0.00	0.099
41	0.771	0.000	0.099	0.07	0.942
42	0.771	0.000	0.099	0.07	0.942
43	0.771	0.000	0.099	0.07	0.942
44	0.771	0.000	0.099	0.07	0.942
45	0.000	0.000	0.099	0.07	0.171
46	0.771	0.000	0.099	0.07	0.942
47	0.771	0.000	0.099	0.07	0.942
48	0.000	0.000	0.099	0.07	0.171
49	0.771	0.000	0.099	0.07	0.942
50	0.000	0.000	0.099	0.07	0.171
51	0.771	0.000	0.099	0.07	0.942
52	0.000	0.000	0.099	0.07	0.171
53	0.771	0.000	0.099	0.07	0.942
54	0.000	0.000	0.099	0.00	0.099
55	0.000	0.000	0.099	0.07	0.170
56	0.771	0.000	0.099	0.00	0.870
101	0.795	0.000	0.099	0.00	0.894
102	0.795	0.000	0.099	0.00	0.894
103	0.795	18.020	0.099	0.00	18.914
104	0.795	0.000	0.099	0.00	0.894
105	0.795	0.000	0.099	0.00	0.894
106	0.795	0.000	0.099	0.00	0.894
107	0.795	0.000	0.099	0.00	0.894
108	0.000	0.000	0.099	0.00	0.099
109	0.000	0.000	0.099	0.00	0.099
110	0.000	0.000	0.099	0.00	0.099
111	0.000	0.000	0.099	0.00	0.099
112	0.000	0.000	0.099	0.00	0.099
113	0.000	0.000	0.099	0.07	0.170
114	0.000	0.000	0.099	0.07	0.170
115	0.000	0.000	0.099	0.07	0.170
116	0.000	0.000	0.099	0.07	0.170
117	0.000	0.000	0.099	0.07	0.170
118	0.841	0.000	0.099	0.07	1.011
119	0.000	0.000	0.099	0.00	0.099
120	0.771	0.000	0.099	0.07	0.941
121	0.771	0.000	0.099	0.07	0.941
122	0.000	0.000	0.099	0.07	0.170
123	0.000	0.000	0.099	0.07	0.170
124	0.000	0.000	0.099	0.00	0.099
<b>Total</b>	<b>23.4</b>	<b>18.0</b>	<b>7.9</b>	<b>3.1</b>	<b>52.4</b>

**Table 2.4.5 Total Water Discharge at Each Node in Year 2010  
Hourly Peak Base in l/sec**

<b>Node Number</b>	<b>Water Discharge for Domestic Water Demand</b>	<b>Water Discharge for Non-Domestic Water Demand</b>	<b>Total Water Discharge at Each Node</b>
00	0.000	0.000	0.000
1	1.449	0.870	2.319
2	1.449	0.099	1.548
3	0.734	0.099	0.833
4	1.148	0.870	2.018
5	0.414	0.099	0.513
6	0.414	0.870	1.284
7	0.414	0.099	0.513
8	0.414	0.170	0.583
9	1.449	0.170	1.618
10	0.734	0.099	0.833
11	0.734	0.099	0.833
12	1.148	0.870	2.018
13	0.414	0.870	1.284
14	0.414	0.099	0.513
15	0.407	0.870	1.278
16	0.407	0.099	0.506
17	0.407	0.099	0.506
18	0.407	0.099	0.506
19	0.048	0.171	0.219
20	0.385	0.941	1.326
21	0.388	0.170	0.557
22	0.388	0.099	0.487
23	0.715	0.099	0.814
24	0.048	0.170	0.217
25	0.048	0.170	0.217
26	0.048	0.171	0.219
27	0.048	0.171	0.219
28	0.329	0.171	0.500
29	0.281	0.171	0.452
30	0.285	0.171	0.456
31	0.281	0.171	0.452
32	0.281	0.942	1.223
33	0.285	0.171	0.456
34	0.285	0.870	1.156
35	0.851	0.170	1.021
36	0.281	0.170	0.450
37	0.281	0.941	1.222
38	0.285	0.170	0.455
39	0.285	0.099	0.385
40	0.544	0.099	0.643
41	0.544	0.942	1.486
42	0.540	0.942	1.482
43	0.638	0.942	1.580
44	0.098	0.942	1.041
45	0.048	0.171	0.219
46	0.097	0.942	1.039
47	0.097	0.942	1.039
48	0.048	0.171	0.219
49	0.097	0.942	1.039
50	0.048	0.171	0.219
51	1.139	0.942	2.081
52	0.048	0.171	0.219
53	0.300	0.942	1.243
54	0.407	0.099	0.506
55	0.388	0.170	0.557
56	5.054	0.870	5.924
101	0.000	0.894	0.894
102	0.000	0.894	0.894
103	0.000	18.914	18.914
104	1.342	0.894	2.236
105	1.351	0.894	2.245
106	1.559	0.894	2.453
107	2.691	0.894	3.585
108	1.810	0.099	1.909
109	1.908	0.099	2.007
110	0.566	0.099	0.665
111	0.316	0.099	0.415
112	0.407	0.099	0.506
113	0.407	0.170	0.577
114	0.270	0.170	0.440
115	1.141	0.170	1.311
116	0.388	0.170	0.557
117	0.388	0.170	0.557
118	0.388	1.011	1.398
119	0.715	0.099	0.814
120	3.614	0.941	4.554
121	3.723	0.941	4.664
122	2.662	0.170	2.832
123	2.584	0.170	2.753
124	2.584	0.099	2.683
<b>Total</b>	<b>60.030</b>	<b>52.380</b>	<b>112.410</b>

**Table 2.4.6 Results of Hydraulic Analysis (1/4)**

Nos of nodes	83	NODE					
Nos of pipes	103	<b>NO</b>	<b>Type</b>	<b>Q</b> l/sec	<b>WL</b> m	<b>GL</b> m	<b>EH</b> m
		0	1	-112.410	49.000	14.0	35.000
		1	0	2.319	41.850	14.5	27.350
		2	0	5.082	41.792	14.8	26.992
		3	0	0.833	40.574	14.0	26.574
		4	0	2.018	40.236	13.7	26.536
		5	0	0.513	39.676	13.8	25.876
		6	0	1.284	39.466	13.7	25.766
		7	0	0.513	39.444	13.6	25.844
		8	0	0.583	39.433	13.5	25.933
		9	0	1.618	41.907	14.5	27.407
		10	0	0.833	40.755	14.3	26.455
		11	0	0.833	40.731	14.3	26.431
		12	0	2.018	40.315	14.0	26.315
		13	0	1.284	39.579	13.9	25.679
		14	0	0.513	39.570	13.9	25.670
		15	0	1.278	39.463	13.9	25.563
		16	0	0.506	39.447	13.9	25.547
		17	0	0.506	39.447	13.9	25.547
		18	0	0.506	39.351	13.5	25.851
		19	0	0.219	42.071	14.1	27.971
		20	0	1.326	41.794	14.1	27.694
		21	0	0.557	41.427	14.5	26.927
		22	0	0.487	41.432	14.5	26.932
		23	0	0.814	41.534	14.9	26.634
		24	0	0.217	42.073	14.2	27.873
		25	0	0.217	42.074	13.9	28.174
		26	0	0.219	42.076	13.9	28.176
		27	0	0.219	42.077	13.9	28.177
		28	0	0.500	39.443	14.0	25.443
		29	0	0.452	39.443	14.0	25.443
		30	0	0.456	39.446	14.1	25.346
		31	0	0.452	39.460	14.1	25.360
		32	0	1.223	39.471	14.2	25.271
		33	0	0.456	39.480	14.2	25.280
		34	0	1.156	39.453	14.1	25.353
		35	0	1.021	39.415	13.9	25.515
		36	0	0.450	39.467	14.1	25.367
		37	0	1.222	39.537	14.1	25.437
		38	0	0.455	39.533	14.1	25.433
		39	0	0.385	39.504	14.1	25.404
		40	0	0.643	39.513	14.1	25.413



**Table 2.4.6 Results of Hydraulic Analysis (2/4)**

NO	Type	Q l/sec	WL m	GL m	EH m
41	0	1.486	39.873	14.1	25.773
42	0	1.482	40.216	14.1	26.116
43	0	1.580	40.385	14.1	26.285
44	0	1.041	42.110	14.1	28.010
45	0	0.219	42.097	13.9	28.197
46	0	1.039	42.167	14.0	28.167
47	0	1.039	42.253	13.9	28.353
48	0	0.219	42.104	14.5	27.604
49	0	1.039	42.425	13.6	28.825
50	0	0.219	42.089	14.2	27.889
51	0	2.081	42.637	13.4	29.237
52	0	0.219	42.492	14.1	28.392
53	0	1.243	42.611	14.3	28.311
54	0	0.506	39.387	13.8	25.587
55	0	0.557	41.509	14.6	26.909
56	0	2.390	41.700	14.6	27.100
101	0	0.894	45.845	14.0	31.845
102	0	0.894	45.078	14.0	31.078
103	0	0.894	44.321	14.0	30.321
104	0	2.236	43.576	13.9	29.676
105	0	2.245	43.290	13.9	29.390
106	0	2.453	42.944	13.8	29.144
107	0	3.585	42.690	13.7	28.990
108	0	1.909	41.211	13.5	27.711
109	0	2.007	40.778	13.5	27.278
110	0	0.665	39.255	13.2	26.055
111	0	0.415	39.257	12.7	26.557
112	0	0.506	39.131	13.0	26.131
113	0	0.577	39.258	12.9	26.358
114	0	0.440	39.281	12.6	26.681
115	0	1.311	39.705	13.0	26.705
116	0	0.557	41.230	15.0	26.230
117	0	18.577	40.214	15.8	24.414
118	0	1.398	40.356	15.1	25.256
119	0	0.814	41.546	13.4	28.146
120	0	4.554	40.987	14.6	26.387
121	0	4.664	40.529	14.6	25.929
122	0	2.832	40.367	14.8	25.567
123	0	2.753	40.125	14.8	25.325
124	0	2.683	40.007	15.0	25.007
201	0	0.000	42.100	14.1	28.000
202	0	0.000	42.078	14.0	28.078

**Table 2.4.6 Results of Hydraulic Analysis (3/4)**

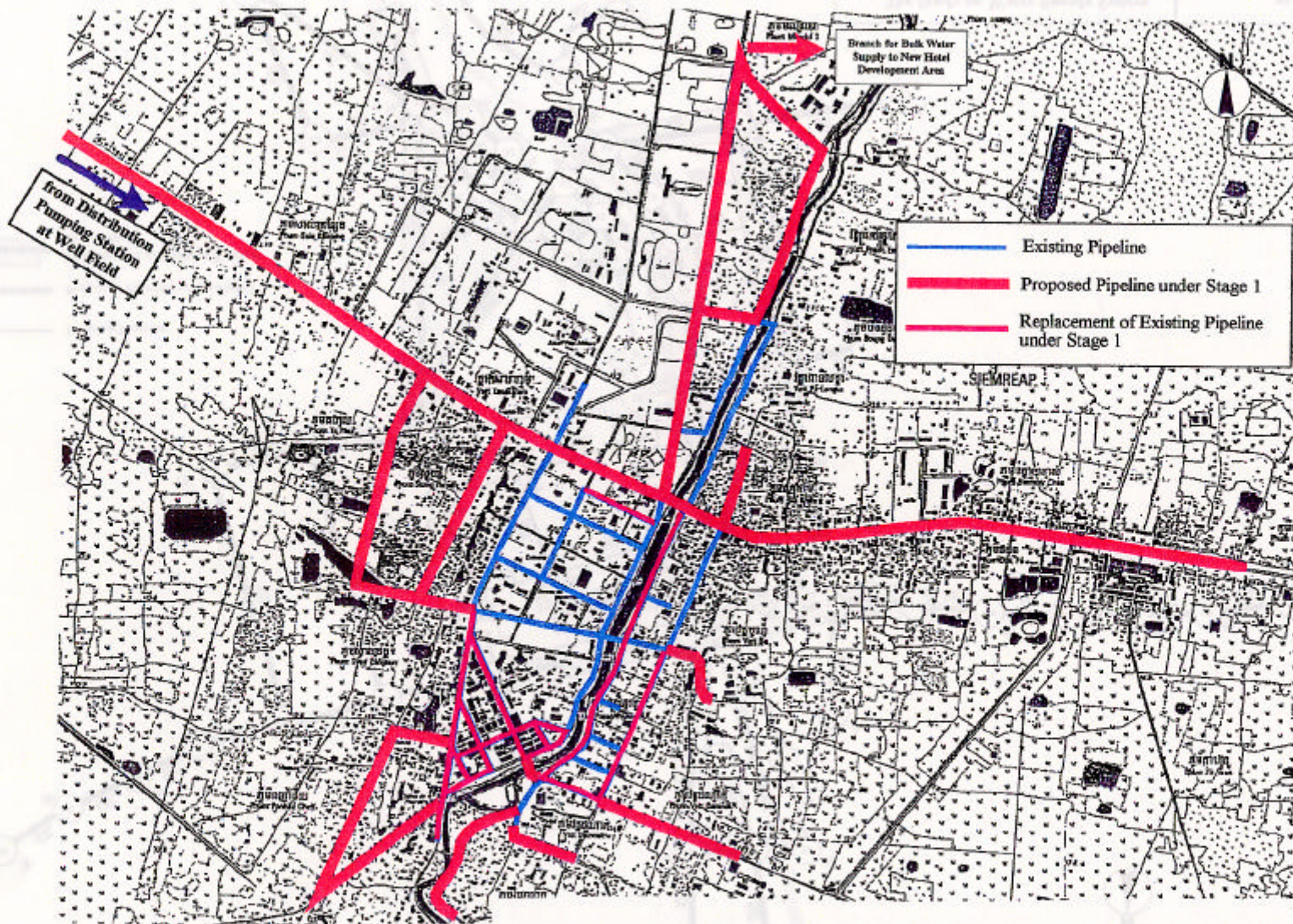
	NO(u)	NO(d)	Dia mm	Length m	C	Q l/sec	V m/sec	I o/oo
1	1	2	300	74	120	29.338	0.415	0.781
2	2	56	250	100	120	19.876	0.405	0.924
3	2	3	100	311	110	3.566	0.454	3.916
4	3	4	100	141	110	2.733	0.348	2.394
5	4	12	150	148	110	-3.512	-0.199	-0.528
6	12	10	150	170	120	-9.050	-0.512	-2.593
7	10	11	100	92	110	0.833	0.106	0.266
8	10	9	150	325	120	-10.716	-0.606	-3.544
9	9	1	300	64	120	31.657	0.448	0.899
10	9	19	300	92	120	-45.729	-0.647	-1.775
11	19	20	250	237	120	22.535	0.459	1.165
12	20	55	250	313	120	19.758	0.403	0.913
13	20	21	100	495	110	1.451	0.185	0.743
14	21	22	100	64	110	-0.437	-0.056	-0.081
15	22	23	100	318	110	-0.924	-0.118	-0.322
16	23	9	100	360	110	-1.737	-0.221	-1.036
17	19	24	250	99	110	-2.293	-0.047	-0.020
18	24	25	250	85	110	-2.093	-0.043	-0.017
19	25	26	250	120	110	-1.785	-0.036	-0.013
20	26	27	250	113	110	-2.004	-0.041	-0.016
21	27	45	150	265	110	-1.233	-0.070	-0.076
22	45	44	150	166	110	-1.206	-0.068	-0.073
23	44	46	150	127	110	-3.237	-0.183	-0.455
24	46	47	150	113	110	-4.276	-0.242	-0.761
25	47	49	150	92	110	-6.942	-0.393	-1.864
26	49	51	150	88	110	-7.981	-0.452	-2.414
27	51	53	150	336	110	1.243	0.070	0.077
28	51	52	400	166	120	66.408	0.528	0.872
29	52	19	350	254	120	66.189	0.688	1.661
30	24	50	100	262	120	-0.418	-0.053	-0.063
31	25	48	100	265	110	-0.525	-0.067	-0.113
32	48	47	100	163	110	-1.626	-0.207	-0.916
33	50	48	100	92	110	-0.637	-0.081	-0.162
34	48	45	100	237	110	0.245	0.031	0.028
35	27	202	250	92	110	-0.990	-0.020	-0.004
36	202	201	150	438	110	-0.990	-0.056	-0.051
37	201	44	150	191	110	-0.990	-0.056	-0.051
38	28	29	200	85	110	-0.500	-0.016	-0.004
39	29	30	200	269	110	-0.952	-0.030	-0.012
40	30	31	100	95	120	-0.645	-0.082	-0.141
41	31	36	100	92	120	-0.464	-0.059	-0.077
42	30	36	100	106	120	-0.764	-0.097	-0.193
43	31	32	100	85	120	-0.633	-0.081	-0.136
44	36	37	100	85	120	-1.678	-0.214	-0.826

**Table 2.4.6 Results of Hydraulic Analysis (4/4)**

	NO(u)	NO(d)	Dia mm	Length m	C	Q l/sec	V m/sec	I o/oo	
	45	32	37	150	95	120	-4.432	-0.251	-0.692
	46	37	42	150	360	120	-7.622	-0.431	-1.887
	47	37	38	100	113	120	0.291	0.037	0.032
	48	33	38	100	88	120	-1.419	-0.181	-0.607
	49	38	41	100	180	120	-2.620	-0.334	-1.886
	50	33	34	100	92	120	0.963	0.123	0.296
	51	38	39	100	88	120	1.037	0.132	0.340
	52	34	39	100	92	120	-1.348	-0.172	-0.551
	53	39	40	100	57	120	-0.695	-0.088	-0.162
	54	34	35	100	92	120	1.155	0.147	0.414
	55	35	40	100	155	120	-1.452	-0.185	-0.633
	56	40	41	100	170	120	-2.790	-0.355	-2.118
	57	41	42	150	219	120	-6.897	-0.390	-1.569
	58	42	43	200	92	120	-16.001	-0.509	-1.833
	59	35	54	100	304	120	0.506	0.064	0.090
	60	4	5	100	244	120	2.915	0.371	2.297
	61	5	6	100	131	120	2.402	0.306	1.605
	62	6	7	100	85	120	0.885	0.113	0.253
	63	7	8	100	99	120	0.578	0.074	0.115
	64	12	13	100	226	120	3.520	0.448	3.256
	65	13	14	100	78	110	0.513	0.065	0.108
	66	13	15	100	134	120	1.723	0.219	0.868
	67	6	15	100	127	110	0.233	0.030	0.025
	68	15	16	100	99	120	0.678	0.086	0.154
	69	16	7	100	152	110	0.206	0.026	0.020
	70	16	17	100	120	120	-0.035	-0.004	-0.001
	71	17	8	100	205	120	0.445	0.057	0.071
	72	17	18	100	110	110	1.590	0.202	0.879
	73	0	101	500	4,050	120	112.410	0.573	0.779
	74	101	102	500	1,000	120	111.516	0.568	0.768
	75	102	103	500	1,000	120	110.623	0.563	0.756
	76	103	104	500	1,000	120	109.729	0.559	0.745
	77	104	105	500	400	120	107.493	0.547	0.717
	78	105	106	450	300	120	105.248	0.662	1.152
	79	106	107	450	330	120	84.658	0.532	0.770
	80	107	51	450	80	120	77.713	0.489	0.657
	81	106	108	200	750	120	18.137	0.577	2.311
	82	107	109	100	640	120	3.360	0.428	2.988
	83	108	109	200	230	120	16.228	0.517	1.881
	84	109	43	200	180	120	17.581	0.560	2.182
	85	35	110	75	290	120	0.631	0.143	0.550
	86	110	111	75	550	120	-0.034	-0.008	-0.002
	87	35	111	75	540	120	0.449	0.102	0.293
	88	18	112	75	600	120	0.506	0.115	0.366
	89	18	113	75	200	120	0.577	0.131	0.466
	90	8	114	75	540	120	0.440	0.099	0.281
	91	32	17	150	95	120	2.576	0.146	0.254
	92	4	115	75	250	120	1.311	0.297	2.125
	93	55	21	100	200	120	1.146	0.146	0.408
	94	55	116	250	360	120	18.055	0.368	0.773
	95	116	117	200	470	120	17.498	0.557	2.163
	96	117	118	100	390	120	-1.079	-0.137	-0.365
	97	118	21	100	630	120	-2.477	-0.315	-1.700
	98	2	119	75	280	120	0.814	0.184	0.880
	99	56	120	200	330	120	17.486	0.557	2.160
	100	120	121	200	370	120	12.932	0.412	1.236
	101	121	122	200	300	120	8.268	0.263	0.540
	102	122	123	150	240	120	5.436	0.308	1.010
	103	123	124	150	430	120	2.683	0.152	0.273

End

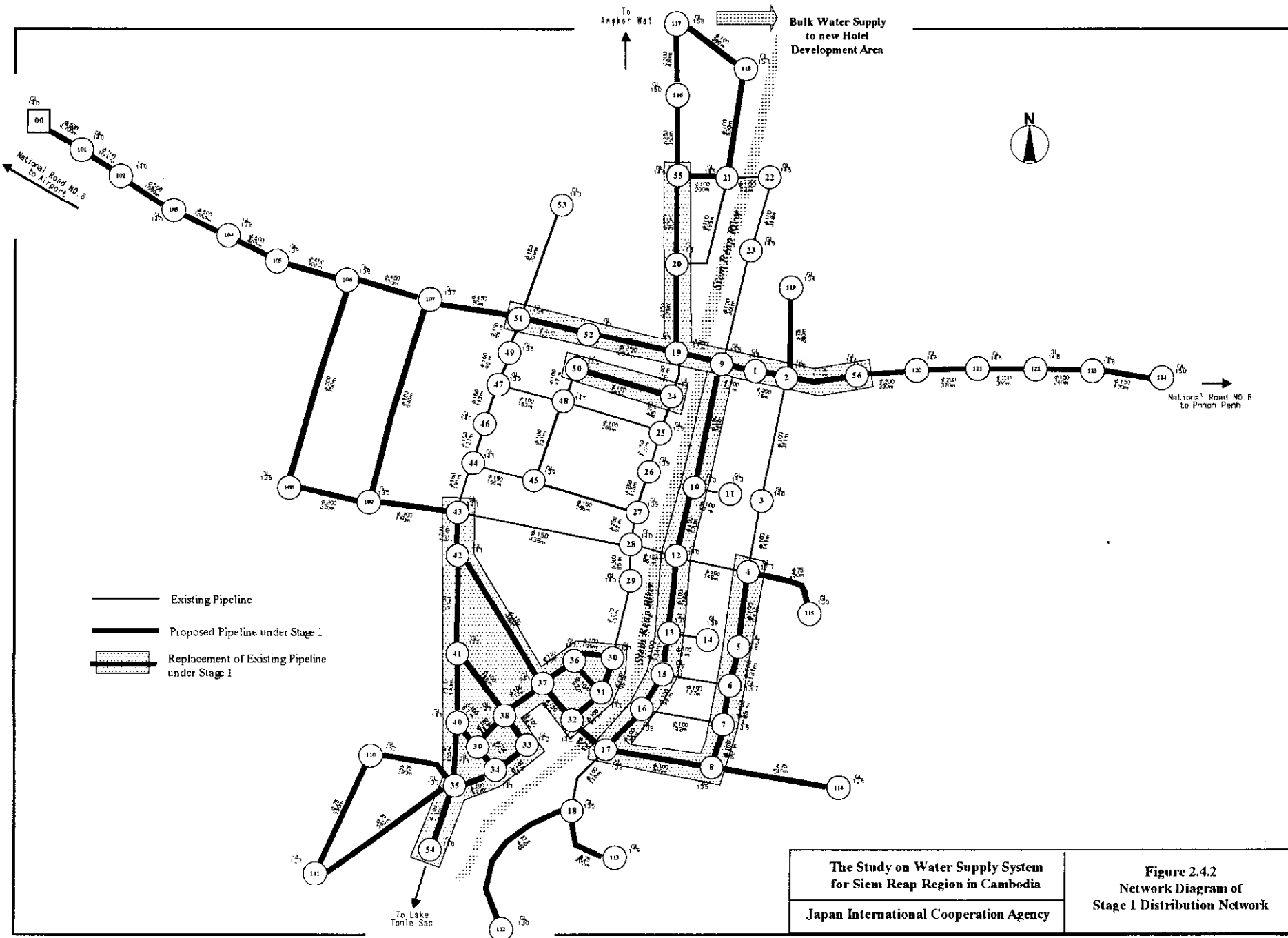




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Figure 2.4.1  
Stage 1 Distribution Network





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Figure 2.4.2  
Network Diagram of  
Stage 1 Distribution Network

Pipe material should be considered separately for the trunk main of which diameter is more than 200 mm and secondary main of which diameter is less than 150 mm.

For the trunk main, DIP is recommended from the following advantages.

- Easy installation work comparing with Steel Pipe (SP) under rainy weather condition and high groundwater level
- Quick and easier installation even by unskilled labor using push-on or mechanical joint.
- Easier maintenance and repair comparing with SP.
- High resistance against corrosion comparing with SP.

Even though DIP has disadvantages such as its weight, heavier than SP, DIP is a recommendable pipe material because of its high reliability.

For the secondary main, PVC or PE is recommendable. Recently, Siem Reap Waterworks started to use PVC for repairing ACP line. In Phnom Penh, PE is prevailing as a material for the secondary main. Table 2.4.8 shows the comparison of characteristics of the PVC and PE.

**Table 2.4.8 Comparison of PVC VS. PE**

<b>Description</b>	<b>Polyvinyl Chloride Pipe (PVC)</b>	<b>Polyethylene Pipe (PE)</b>
<b>Physical Characteristics</b>		
Material	Polyvinyl Chloride	Polyethylene
Specific Gravity	1.43 heavier than water	0.95 lighter than water
<b>Mechanical Characteristics</b>		
Tensile Strength	530 kgf/cm <sup>2</sup>	224 kgf/cm <sup>2</sup>
Tense Ratio at Break Point	50 – 150 %	more than 500 %
Bending Elasticity	3.0 x 10 <sup>4</sup> kgf/cm <sup>2</sup> rather hard	0.78 – 1.0 x 10 <sup>4</sup> kgf/cm <sup>2</sup> rather soft
Strength against Impact	not so strong	strong
<b>Temperature Range</b>	-5 C to 45 C	- 30 C to 40 C
<b>Combustion</b>	PVC will not burn by itself	PE will burn. Open flame shall be prohibited during pipe installation.
<b>Chemical Characteristics</b>		
Against Acid	strong	strong
Against Alkalinity	strong	strong
Against Organic Solvent	weak against some kind of organic solvent	weak against some kind of organic solvent

Description	Polyvinyl Chloride Pipe (PVC)	Polyethylene Pipe (PE)
<b>Available Joint Type</b>	Push-on Rubber Ring Joint Glue Joint (TS Joint) Mechanical Joint Flange Joint	Electric Fusion Joint Mechanical Joint Flange Joint  Electric Fusion Joint is the strongest against thrust force comparing with other joint types.
<b>Bend</b>	Bending of straight pipe by heating is not allowed. Fittings, such as bend pipes, shall be used for bend.	Straight pipe can be bend within allowable curvature without any fittings.
<b>Pipe Installation</b>	Easy because of its light weight.	Pipe weight is almost same as PVC. Number of fittings can be minimized because of its flexibility.
<b>Pipe Jointing Work</b>	Easy	Easy Required special equipment for Electric Fusion Jointing. Electric fusion joint will require longer time for cooling after heating comparing with PVC jointing works.
<b>Pipe Handling</b>	Care shall be taken against impact by keen object.	Open flame shall be prohibited near from the pipe.
<b>Underground Stress and Deflection</b>	Maximum stress : 38 kgf/cm <sup>2</sup> Deflection : 0.8 % under earth covering 1.2 m pipe diameter 200 mm	Maximum stress: 26.2kgf/cm <sup>2</sup> Deflection : 1.0 % under earth covering 1.2 m pipe diameter 200 mm

## 2.4.2 Cost Comparison of Distribution Trunk Mains

Since the distribution network was developed for Year 2010, pipeline has enough capacity for the water demand in the Stage 1, Year 2006. To avoid too big pre-investment, costs of the distribution trunk main, Node No. 00 to 19 on the Figure 2.4.2 along the National Road No. 6, are compared for two cases.

Case 1 : Distribution trunk main which has enough capacity for Year 2010 will be installed in the Stage 1 at once.

Case 2 : In the Stage 1, distribution trunk main which is required only for Year 2006 will be installed and additional trunk main will be installed in the Stage 2 to meet water demand in the target year.

Pipelines required and construction cost in present value for each case are shown on Table 2.4.9 and 2.4.10 respectively.

**Table 2.4.9 Distribution Trunk Main Required for Each Case**

Diameter (mm)	Case 1 (m)	Case 2	
		Stage 1 (m)	Stage 2 (m)
500	7,450	0	0
450	710	4,050	0
400	166	3,400	4,050
350	254	300	3,400
300	0	830	300
250	0	0	410
200	0	0	420
<b>Total</b>	<b>8,580</b>	<b>8,580</b>	<b>8,580</b>

**Table 2.4.10 Costs of Distribution Trunk Main Required for Each Case**

Diameter (mm)	Case 1 (US\$)	Case 2	
		Stage 1 (US\$)	Stage 2 (US\$)
500	4,842,500	0	0
450	407,067	2,322,000	0
400	76,222	1,561,167	1,859,625
350	106,045	125,250	1,419,500
300	0	309,867	112,000
250	0	0	123,000
200	0	0	104,650
<b>Total</b>	<b>5,431,833</b>	<b>4,318,283</b>	<b>3,514,125</b>

<b>Present Value in 1999</b>	<b>4,489,118</b>	<b>3,568,829</b>	<b>1,803,301</b>
<b>Total Case 1</b>	<b>4,489,118</b>	<b>Total Case 2</b>	<b>5,372,130</b>

Note : Disburse timing of Stage 1 is in Year 2001 and Stage 2 is in Year 2006  
Discount rate : 10% per annum

Comparing the present values of Case 1 and Case 2, value of Case 1 is smaller than that of the Case 2 as shown on table above.



Above comparison is made based on the condition that the Stage 2 will be implemented in Year 2006. However, implementation of the Stage 2 project is not yet fixed and there might be a possibility of the delay of the implementation. Considering the delay, break-even point of the present values of Case 1 and Case 2 is calculated. The result says that if the Stage 2 Project is implemented in Year 2014, present value of Case 1 and Case 2 will become same.

In other words, if the Stage 2 Project is implemented before Year 2014, Case 1 has advantage in aspect of investment cost.

Siem Reap Town is developing and situation of the town is changing drastically. Population is increasing and development of infrastructures to receive more tourist is on going. Timing of the implementation of the Stage 2 will be considered on schedule and not to delay until Year 2014. Therefore, the Case 1, even though it include pre-investment for the Stage 2, is recommended.

### 2.4.3 Meter District

It is recommended to introduce a Meter District System (MDS) for the proposed distribution network. By introducing the MDS, the Waterworks can evaluate the effectiveness of water supply or level of UFW in each meter district. Each meter district will be isolated by closing boundary valves. If a pipeline crossing the boundary can not be closed, district meter will be installed. Total water inflow will be measured continuously by several district meters. The Waterworks can calculated total water consumption by accumulating water bills in the respective district and can compare total water inflow and total water consumption.

Proposed MDS is shown on Figure 2.4.3. As shown on the figure, the whole distribution network is recommended to be divided into four meter districts and totally 6 district meters will be required for establishment of the MDS. The boundary of the meter district is set by considering topographic condition, easy operation, characteristics of consumers, and current service block system of the Waterworks.

Meter District I will contain less domestic connection, but a lot of non domestic connections, such as hotels along the National Road No. 6 and road to Angkor Wat and government offices located in the central part of the Town. This district contains many existing pipelines comparing with other district.

Meter District II will cover housing area in the western part of the Siem Reap River. Commercial area surrounding the old central market is also included in this district. Most of distribution pipelines in this district are newly installed or replaced.

Meter District III will cover the eastern side of the Siem Reap River and contain housing area and restaurants along the river. Main pipes will be replaced with

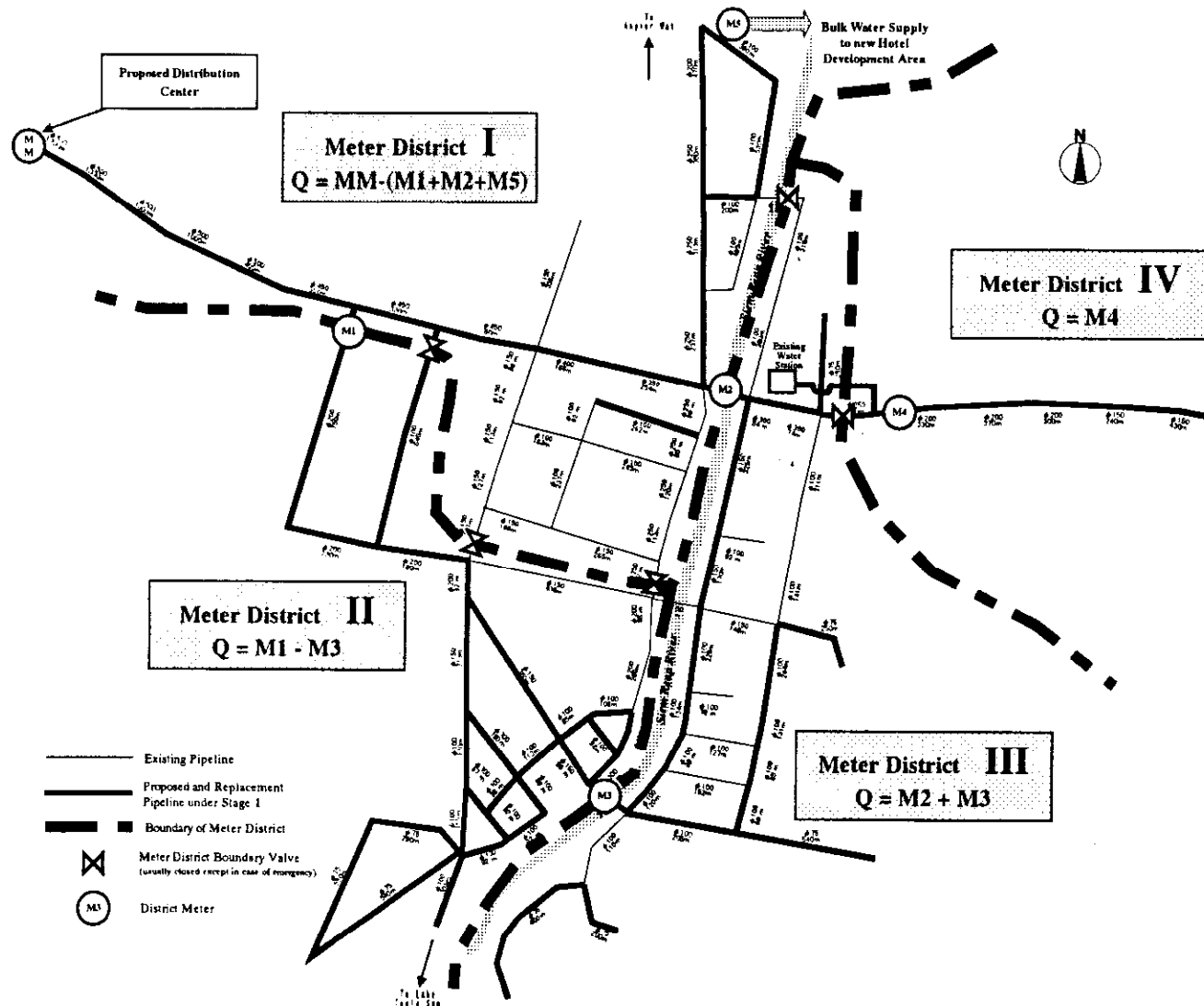
new pipe but several small size existing pipelines will be included.

Meter District IV will be completely separated from the other district and will be supplied exclusively from the existing Water Station. Necessary piping and valve arrangement for the Meter District IV is shown on Figure 2.4.4.

Estimated water consumption in each meter district during peak hour period in Year 2006 is as shown on Table 2.4.11. Layout of the District Meter is shown on Figure 2.4.5.

**Table 2.4.11 Estimated Water Consumption in Each Meter District**

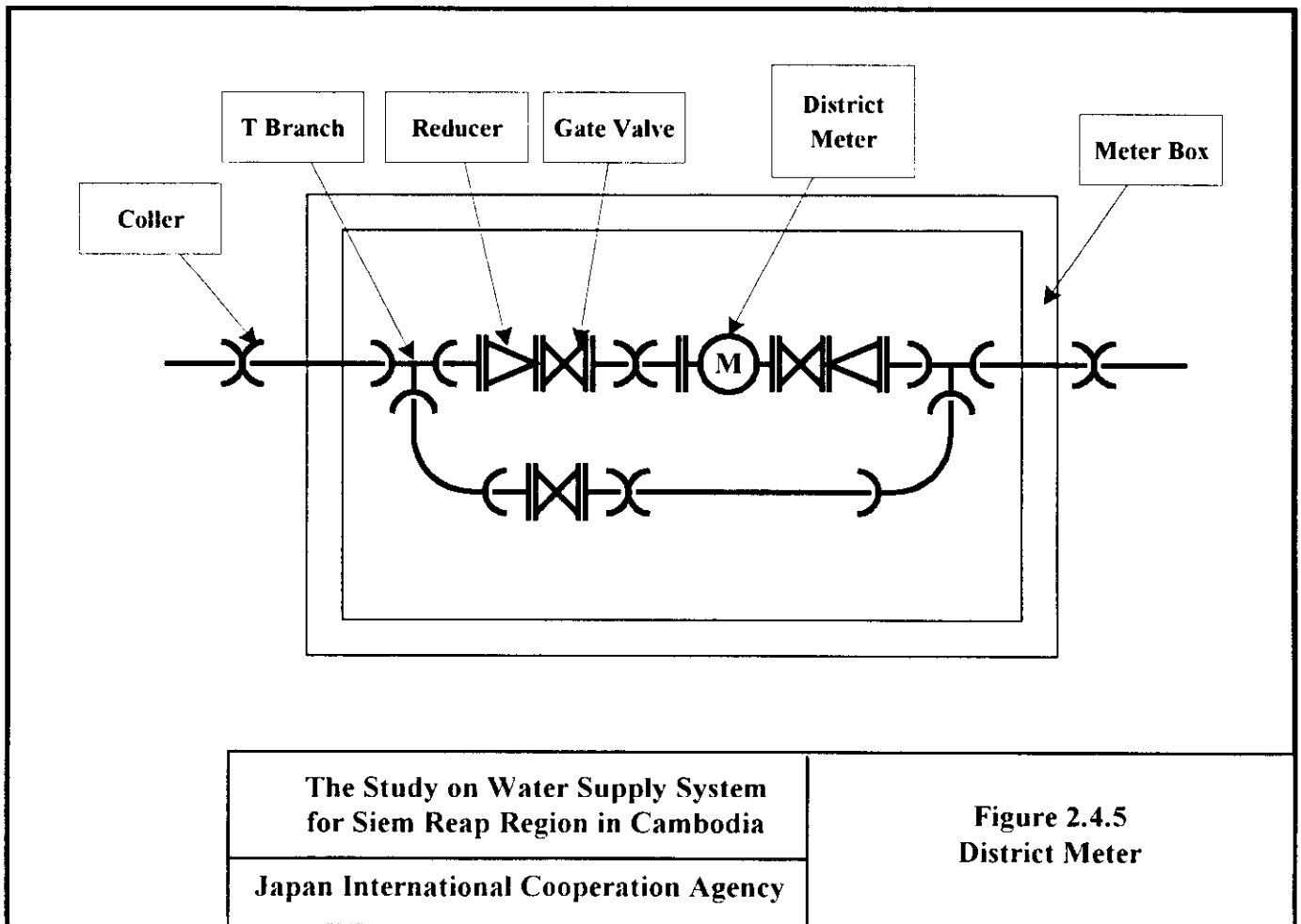
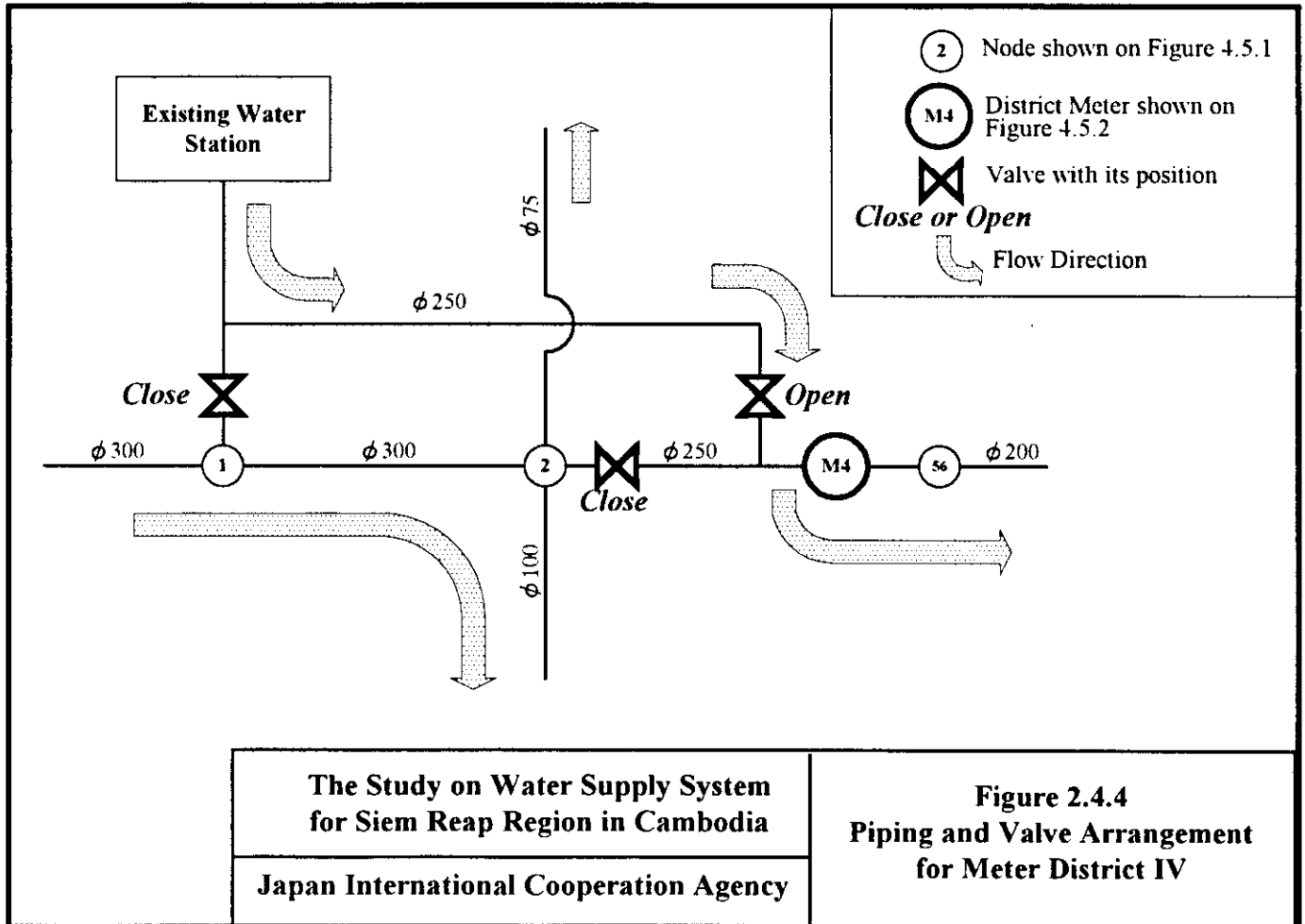
<b>Meter District</b>	<b>Water Consumption (Peak Hour) (l/sec)</b>
I	28.1
II	19.0
III	28.4
IV	19.9
Bulk Supply for Hotel Zone	17.0



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**Figure 2.4.3  
Proposed Meter District in Stage 1**



#### 2.4.4 Service Main

It is recommended that tapping for connection is allowed from distribution pipe of which diameter is smaller than 150 mm. It will require parallel pipe installation along with larger diameter, more than 200 mm, to install connection. Required length of service main will be calculated from the length of distribution trunk main with diameter more than 200 mm.

**Table 2.4.12 Length of Trunk Main Required Parallel Service Main**

Diameter (mm)	Existing (m)	Stage 1 (m)	Total (m)
450		710	710
400		166	166
350		254	254
300		230	230
250	509	1,260	1,769
200	354	2,722	3,076
<b>Total</b>	<b>863</b>	<b>5,342</b>	<b>6,205</b>

In the Table above, pipe diameter 500 mm is excluded because these pipe line will be installed along the National Road No. 6 and there will be no house connection in the area except hotel water supply. Hotel water supply will be branched by using T-fittings, therefore service main will not be required.

Total length of service main will be about 6,200 m and diameter will be 50 mm and 75 mm. Length of each diameter is assumed to be same and the length will be 3,100 m each.

#### 2.4.5 House Connection

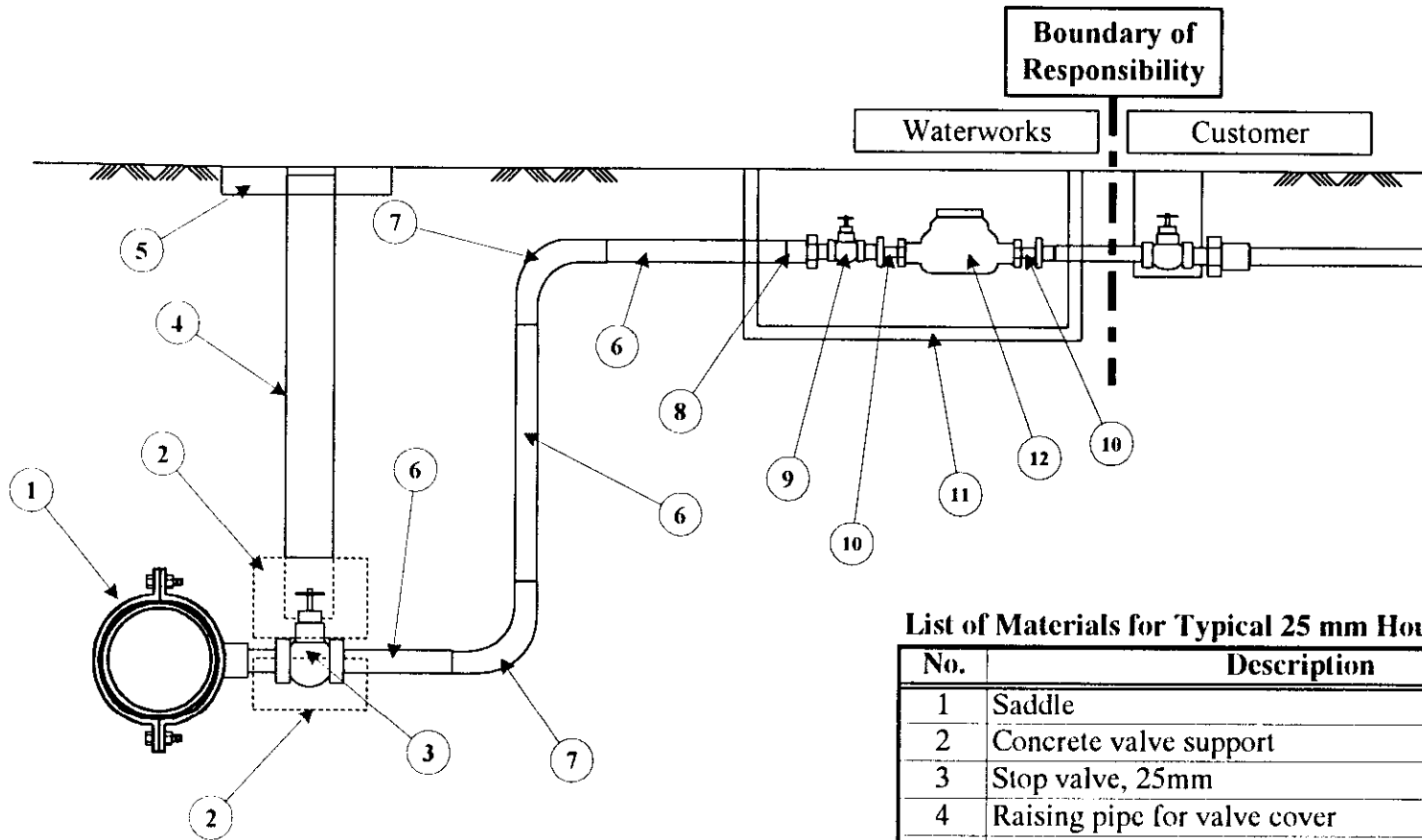
The Waterworks developed their standard installation of house connection as shown on Figure 2.4.6. House connection up to water meter is a property of the Waterworks and the Waterworks has a responsibility on this part. According to the agreement with customers, the Waterworks should repair any damage or leakage on the property of the Waterworks without any charge.

Pipe material of the house connection is PVC and the Waterworks has plan to change the material from PVC to PE like in Phnom Penh because of PE's flexibility. The Waterworks feels that development of standard installation of PE house connection is indispensable and that training for PE installation work is necessary.

Number of house connections will increase from the commencement of water supply service under the Stage 1 Project as shown on Table 2.4.13 and Figure 2.4.7.

**Table 2.4.13 Number of Connections**

<b>Year</b>	<b>House Connection (Domestic)</b>	<b>Connection for Hotels</b>	<b>Connection for Guest Houses</b>	<b>Total Number of Connections</b>	<b>Incremental Connection</b>
2002	1,640	21	83	1,744	1,354
2003	2,320	32	137	2,489	745
2004	3,067	43	204	3,314	826
2005	3,898	48	244	4,190	875
2006	4,475	50	272	4,797	607



**List of Materials for Typical 25 mm House Connection**

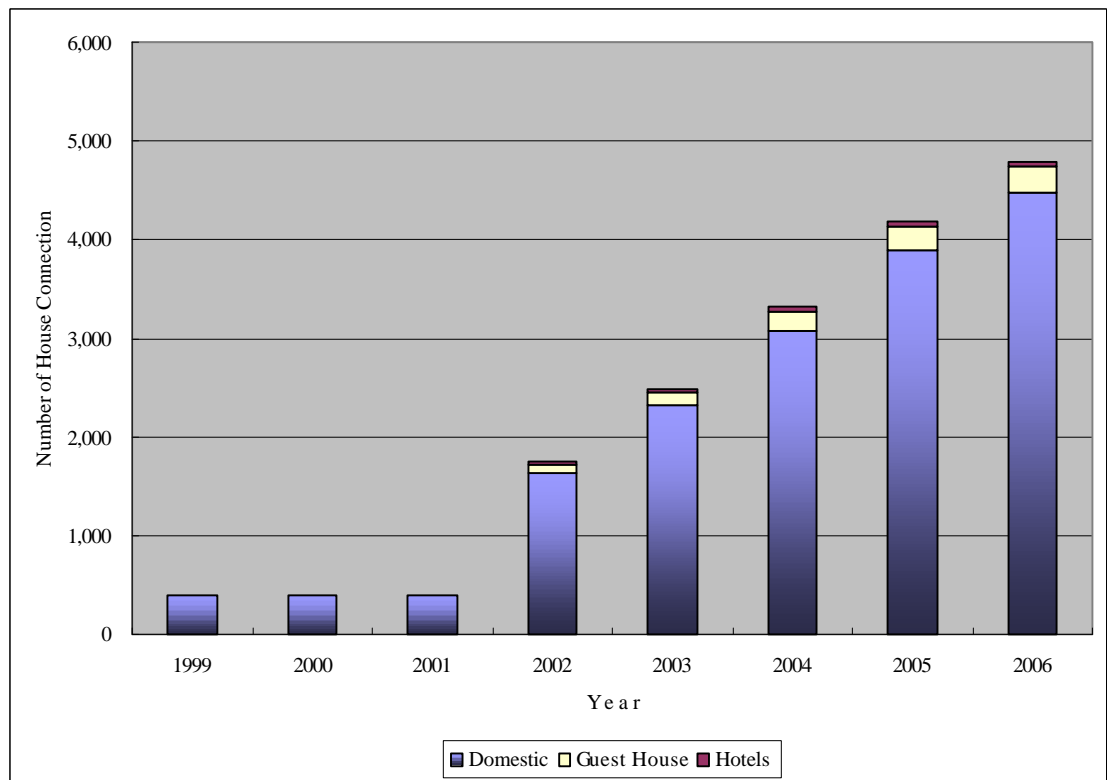
No.	Description
1	Saddle
2	Concrete valve support
3	Stop valve, 25mm
4	Raising pipe for valve cover
5	Valve cover (Concrete and cast iron)
6	PVC Straight Pipe, 25 mm
7	Elbow, 25 mm
8	Socket, 25 mm
9	Stop valve, 25mm
10	Union Coupling, 25 mm x 15 mm
11	Concrete made meter box with steel cover
12	Water meter, 15 mm

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**Figure 2.4.6**

**Typical House Connection**



**Figure 2.4.7 Number of House Connections**

There are 173 house connections, as of October 12, 1999, on pipeline which will be replaced. These house connections should be shifted to the new replaced pipe. According to the information of re-registration, 32 % of old customers have not registered yet. Therefore, number of the connection 173 may increase up to 254.

It is recommended to include procurement of materials and installation work required for these shifting connections to this Stage 1 Project.