

4. Geophysical Prospecting

Electric sounding at 103 exploration spots were carried out from March 1997 to April 1997. The method is Wenner alignment. The maximum electrode interval is 160 m for all the spots. The location of the spots is shown in Figure 4.1 “Location Map of Electric Sounding Spots and Geological Profiles Showing Deepest Resistivity”.

The data is given in Data Book. The results of analyses are drawn in the 11 profiles shown in Attachment-1. The core drilling results are also added into the profiles.

The resistivity boundary is shown in the 11 profiles by the long broken line. The resistivity boundaries are much lower than that of the geological boundaries that were known by the core drilling. Therefore, the geological boundaries were finally decided based on the drill logs through the course of interpretation for analysis results. The main causes of the discrepancy of the resistivity boundaries are inferred as follows.

- 1) The resistivity boundary not always reflects geological boundaries, because the geological boundaries in this area are unconformity. It is prospected that the upper portions of each geological unit such as Alluvium, Pleistocene, Pliocene formations and Bed rock is developed weathering, deteriorating and crack distributing by unconformity. Therefore, the resistivity boundary might be appeared below the geological unconformity boundary until reaching sound conditions.
- 2) The resistivity value by the analysis show an apparent value. It means that resistivity value is very much affected by the surrounding resistivity values in the shallower or neighboring zones.
- 3) Limit of analysis for similar resistivity layers and/or analysis error. An interpretation of core drilling data may be more reliable. The detail is described in Chapter 5.

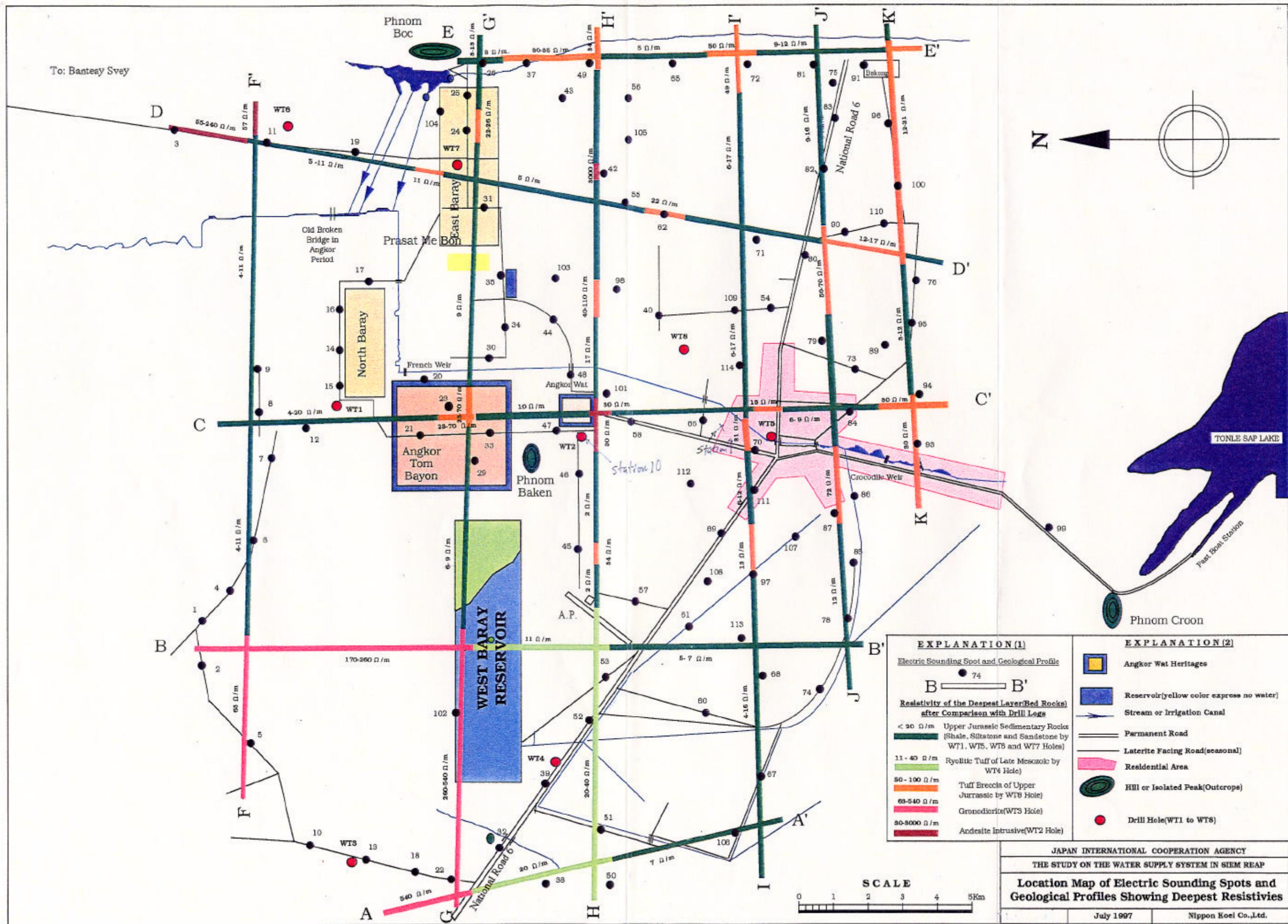


Figure 4.1 Location Map of Electric Sounding
 A3.3.1-9

5. Core Drilling for Geotechnical Investigations

Core drilling at 8 places was carried out as shown in Figure 4.1. Data of drill logs are shown in Attachment-1 for the WT1 to WT8 with the Standard Penetration Test (SPT or N value), electric logging and location of soil sampling point for test.

Geological profile of 10 places in total is arranged by referring the results of core drilling and the electric sounding. As a result, the 4 main geological units were clarified.

1) Alluvium Deposits

The layer is widely developed with the characteristic that coarse to medium sand is in the northeast area, and medium to fine sand is in the south area. Resistivity value reflects the above grain size condition with 50-3,000 ohm/m for the northeast area, and 50-1,100 ohm/m for the south area. Electric logging result of WT1 to WT8 is correlative with the electric sounding. A natural gamma logging of WT1 to WT8 shows that 4-25 cps in the north area and 4-20 cps in the south area. The thickness of the layer is 10-20 m. It becomes thin to the northward in general.

N value of the layer varies place by place. However, the value observed at WT1, WT2, WT3 and WT8 holes is rather high and widely ranging from 4 to 50. On the other hand, N value of WT4, WT5 and WT7 demonstrates rather smaller values of 2 to 22. It is inferred that the in southwestward, it might become more homogeneous deposits than the northeastward.

Figure 3.1 shows that the layer develops shallower in the north to eastward and deeper in the westward. The deepest parts can be traced near the West Baray. It is a buried valley trending from north to south direction, where the old river course of Siem Reap is located.

2) Pleistocene Deposits

The layer underlying the Alluvial deposits is coarse to medium sand (stone) with boulders in the north ward, and medium to fine sand (stone) with boulders in the south ward. Resistivities were examined by both methods of the electric sounding and logging, also reflect such contrast of the grain size distribution in the layer. Coarse to medium sand (stone) has resistivities ranging from 74 to 1,900 ohm/m. Medium to fine sand (stone) develops in the southward has resistivities of 20 to 400 ohm/m. Natural gamma measurement is 6-20 cps throughout this layer. But no distinguishable contrast is found.

N values of this layer ranges from 16 to 50 and/or more than 50 throughout the layer in all holes. Minimum value is 16 or 18 at WT4, WT7 and so on. Distribution of such small values demonstrates that the layer is composed of various materials from clay to coarse sand. Comparing with the value in alluvial deposits, this layer has 10 to 20 higher values of 10 to 20 than that.

Figure 3.2 shows the bottom of this layer. The map shows that the bottom of this layer is a distinguished hydrogeological boundary judged by pumping test in the wells.

3) Pliocene Formation

The formation is composed of homogeneous and impermeable clay stone. The drilling core sample is mostly cylindrical core. Resistivity ranges from 10 to 200 ohm/m by surface sounding, and from 5 to 20 ohm/m by logging. The core sample demonstrates that physical property, mechanical property and electric resistivity value might be uniform throughout this formation, except with the uppermost parts of weathering near boundary.

4) Bed Rock

The bed rock distribution is shown in Figure 3.3 by three dimension. The followings explain each bed rock conditions.

Shale, sandstone and silt stone

The rock is dark to reddish brown color. The rock is mostly very hard in fresh condition. However, the top parts near the upper boundary is decomposed or deteriorated to soil condition by weathering. The thickness of the weathered zone ranges from 8 m at WT1 and WT6 (shale), 5 m at WT5, and 3 m at WT7 (sandstone). Resistivity is mostly less than 10 ohm/m and 5 to 20 ohm/m by logging.

Ryolitic tuff

The rock is found out by WT4 in the depth from 71.7 to 80 m (hole bottom). Light gray to green color, heavily altered tuff. The surrounding area might be affected by granodiorite intrusion. Resistivity ranges from 11 to 40 ohm/m, and 20-30 ohm/m by logging.

Tuff breccia

Rock appears in the hole WT8 from the depth of 88.5 to 95 m (hole bottom). The rock is greenish to purplish brown tuff breccia with coarse matrix. Resistivity is 12-120 ohm/m, and 5-10 ohm/m by logging. It is prospected that a few zones of tuff breccia develop as shown in Figure 3.3.

Gronodiorite intrusive

Based on the facts from core sample of WT3 in the depth of 58 to 80 m (hole bottom), and extremely high resistivities of 63-540 ohm/m in the vicinity of West Baray, the wide distribution of granodiorite is estimated.

Andesite intrusive

Andesite intrusives are found from the hole WT2 in the depth of 77 to 95 m (hole bottom), and several electric sounding results. Resistivities vary from 30-3,000 ohm/m place by place.

6. Soil Laboratory Test

1) Laboratory Test for the 8 Core drilling in 1997

The result is summarized in Table 6.1 to Table 6.2 "Soil Laboratory Test in 1998-1999".

The elastic deformation coefficient from the unconfined compression result was recalculated by using the same pressure range of groundwater fluctuation. The estimated deformation coefficient is several to ten times larger value than that of the actual land reversible movements of 1.3 mm by the monitoring. The reason shall be explained as, i) the test was done under the unconfined condition, though the layer is confined, ii) the samples were released a confined stress by taking samples and loosen, and iii) the samples were disturbed in some degree, through the course of hydraulic pressing for sampling purpose.

The actual field records from the monitoring was used for the evaluation of land movement.

Table 6.1 Summary of Physical Properties (1/2)

Borehole No	Sample No	Depth(m)		Water Content (%)	Unit Weight (ton/m3)	Liquid Limit (%)	Plasticity Index (%)	Specific Gravity Gs	Grain Size Analysis(%)					Undrained Strength(ton/m2) UC	Modulus 50% Cy (ton/m2)	Colour	USCS	Soil Description
		from	to						Gravel		Sand Fine	Silt	Clay					
									Coa.	Med.								
WT-3	DB-1	18.30	18.80	11.5	2.2	34.8	27.5	2.57	0	0	7	36	57	21.7	987	Pale yellowish Brown	CL	Sandy CLAY
WT-3	DB-2	24.00	24.50	16.0	2.1	45.6	30.8	2.52	1	1	2	11	85	13.0	549	Pale yellowish Brown	CL	Sandy CLAY
WT-3	DB-3	27.20	27.50	12.2	2.2	35.9	22.1	2.63	0	0	16	47	36	6.7	1464	Pale yellowish Brown	CL	Clayey SAND
WT-3	DB-4	29.00	29.50	11.4	2.3	38.2	25.0	2.59	0	0	10	30	60	21.1	6012	Pale yellowish Brown	CL	Sandy CLAY
WT-3	DB-5	33.00	33.45	14.0	2.2	31.6	18.5	2.59	1	0	11	42	47	11.7	472	Pale yellowish Brown	SC	Clayey SAND
WT-3	DB-6	36.10	36.60	15.1	2.2	40.0	27.2	2.54	0	0	2	30	67	19.8	1083	Grayish Brown	CL	Sandy CLAY
WT-4	DB-1	9.40	9.60	16.5	2.1	40.6	21.2	2.63	3	1	10	29	57	6.2	579	Grayish Brown	CL	Sandy CLAY
WT-4	DB-2	18.40	18.80	12.0	2.2	31.1	18.1	2.56	1	0	9	40	51	9.0	1389	Grayish Brown	CL	Sandy CLAY
WT-4	DB-3	29.20	29.50	15.0	2.2	36.9	22.4	2.61	0	0	6	16	77	6.8	786	Grayish Brown	CL	Sandy CLAY
WT-4	DB-4	32.50	32.95	11.4	2.3	29.4	17.3	2.54	0	0	7	33	60	17.8	6346	Grayish Brown	CL	Sandy CLAY
WT-4	DB-5	36.75	37.00	14.9	2.1	49.3	35.0	2.59	0	0	2	17	80	47.4	7498	Grayish Brown	CL	Sandy CLAY
WT-4	DB-6	54.60	55.00	10.9	2.2	31.2	17.8	2.62	1	4	4	26	65	8.6	1809	Yellowish Gray	CL	Sandy CLAY
WT-4	DB-7	57.10	57.45	15.6	2.2	38.3	19.3	2.55	0	0	2	22	75	6.5	1284	Very light Gray	SC	Clayey SAND
WT-6	DB-1	7.20	7.50	11.2	2.3	24.9	11.3	2.65	0	0	17	42	41	3.9	297	Pale yellowish Brown	SC	Clayey SAND
WT-6	DB-2	16.20	16.60	10.9	2.3	24.2	14.5	2.58	1	0	19	38	42	8.9	728	Pale yellowish Brown	SC	Clayey SAND
WT-6	DB-3	42.60	42.90	7.0	2.1	33.7	20.6	2.55	1	0	10	35	55	39.6	9253	Pale yellowish Brown	CL	Sandy CLAY
WT-6	DB-4	56.50	56.88	26.7	2.0	58.0	27.6	2.66	0	0	1	1	98	8.1	902	Grayish Brown	CH	Sandy CLAY
WT-7	DB-1	13.60	14.00	12.1	2.3	27.3	16.5	2.56	0	0	18	43	38	7.0	744	Very light Gray	SC	Clayey SAND
WT-7	DB-2	19.00	19.60	12.8	2.3	27.8	17.0	2.56	1	0	12	42	45	13.2	305	Very light Gray	SC	Clayey SAND
WT-7	DB-3	23.00	23.40	14.0	2.2	33.3	18.8	2.62	0	0	11	48	41	10.8	346	Very light Gray	SC	Clayey SAND
WT-7	DB-4	27.15	27.50	11.1	2.2	29.7	19.9	2.58	0	0	10	40	50	11.3	2911	Very light Gray	SC	Clayey SAND
WT-7	DB-5	35.50	35.80	13.1	2.2	40.9	27.5	2.63	1	1	9	26	63	21.2	438	Pale yellowish Brown	CL	Sandy CLAY
WT-7	DB-6	46.20	46.75	11.5	2.3	35.5	22.2	2.60	0	0	9	35	56	38.6	668	Pale yellowish Brown	CL	Sandy CLAY
WT-7	DB-7	48.25	48.50	10.8	2.3	33.7	20.8	2.60	1	0	9	32	59	45.2	890	Pale yellowish Brown	CL	Sandy CLAY

A3.3.1-13

Table 6.1 Summary of Physical Properties (2/2)

Borehole No	Sample No	Depth(m)		Water Content (%)	Unit Weight (ton/m ³)	Liquid Limt (%)	Plasticity Index (%)	Specific Gravity Gs	Grain Size Analysis(%)				Undrained Strength(ton/m ²) UC	Modulus 50% Cy (ton/m ²)	Colour	USCS	Soil Description		
		from	to						Gravel		Sand	Silt						Clay	
									Coa.	Med.									Fine
WT-1	DB-1	21.70	22.00	7.7	2.02	NP	NP	2.57	0	1	12	46	41	+	+	Pale Yellowish Brown	SM	Silty Sand	
WT-1	DB-2	23.00	23.60	4.0	1.91	NP	NP	2.63	0	0	11	51	37		1.5	3.35	Pale Yellowish Brown	SM	Silty Sand
WT-1	DB-3	44.40	44.70	8.6	-	-	-	2.63	0	1	5	33	61	-	-	Pale Yellowish Brown	C	Sandy Clay	
WT-1	DB-4	46.00	46.30	9.4	2.20	40.5	27.9	2.64	1	1	8	33	56		4.3	772	Pale Yellowish Brown	CL	Sandy Clay
WT-1	DB-5	49.50	50.00	7.7	2.28	39.6	29.0	2.63	0	0	7	34	59		4.9	1477	Grayish Pink	CL	Sandy Clay
WT-1	DB-6	54.30	54.70	8.9	2.21	33.4	24.5	2.62	0	0	5	45	50		6.7	387	Pale Yellowish Brown	CL	Sandy Clay
WT-1	DB-7	62.00	62.30	8.4	2.21	27.6	17.0	2.65	0	0	3	27	70		4.9	1187	Grayish Brown	CL	Sandy Clay
WT-2	DB-1	6.85	7.00	13.1	2.15	33.4	20.2	2.57	0	1	15	38	45	+	+	Grayish Brown	SC	Clayey Sand	
WT-2	DB-2	15.85	16.00	1.2	-	21.8	13.0	2.55	0	0	16	51	32	-	-	Pale yellowish Brown	SC	Clayey Sand	
WT-2	DB-3	25.50	25.85	8.9	2.13	26.7	15.1	2.65	0	0	13	42	45		1.0	896	Very light Gray	SC	Clayey Sand
WT-2	DB-4	36.70	37.00	8.5	2.15	28.0	16.7	2.51	0	0	12	43	45		3.3	317	Very light Gray	SC	Clayey Sand
WT-2	DB-5	41.70	41.90	6.5	2.18	27.8	17.5	2.69	0	0	8	32	60		4.9	994	Pale Yellowish Brown	CL	Sandy Clay
WT-2	DB-6	48.15	48.60	7.9	2.24	33.7	17.6	2.58	0	0	5	18	77		5.8	2045	Pale Yellowish Brown	CL	Silty CLAY With Sand
WT-2	DB-7	50.20	50.55	9.4	2.28	34.9	21.6	2.58	0	0	7	34	59		7.2	1218	Grayish Brown	CL	Sandy CLAY
WT-2	DB-8	53.10	53.40	13.3	-	42.2	23.3	2.65	49	10	7	9	25	-	-	Grayish Brown	GC	Clayey Gravel With Sand	
WT-2	DB-9	63.10	63.55	11.8	2.04	40.5	21.6	2.54	0	0	1	11	88		4.8	457	Yellowish Gray	CL	Silty CLAY
WT-2	DB-10	70.70	70.90	7.9	2.18	29.1	15.1	2.58	0	0	24	47	28	+	+	Pale Yellowish Brown	SC	Clayey SAND	
WT-2	DB-11	75.00	75.40	11.6	1.99	66.3	42.8	2.53	36	3	3	5	52		2.6	466	Grayish Brown	CH	Silty CLAY
WT-5	DB-1	14.00	14.50	9.6	2.19	35.9	23.6	2.59	0	2	11	35	51		3.1	447	Pale Yellowish Brown	CL	Sandy CLAY
WT-5	DB-2	35.60	35.80	11.7	-	35.9	22.0	2.62	0	0	14	41	46	-	-	Grayish Brown	SC	Sandy CLAY	
WT-5	DB-3	46.70	47.00	6.4	2.22	38.5	27.6	2.64	0	0	2	38	60		7.6	1142	Pale Yellowish Brown	CL	Sandy CLAY
WT-5	DB-4	53.60	53.90	9.3	2.27	21.8	13.4	2.61	1	3	7	36	53	+	+	Very light Gray	CL	Sandy CLAY	
WT-5	DB-5	58.50	59.00	8.0	2.28	36.4	25.8	2.61	0	0	5	29	67		7.0	1392	Pale Yellowish Brown	CL	Sandy CLAY
WT-5	DB-6	67.00	67.50	5.3	-	40.3	28.0	2.61	0	0	2	28	71	-	-	Pale Yellowish Brown	CL	Sandy CLAY	
WT-5	DB-7	78.40	78.80	10.0	2.13	27.5	15.5	2.68	0	0	12	56	31	+	+	Pale Brown	CL	Clayey SAND	
WT-8	DB-1	9.40	9.80	9.0	2.13	30.9	17.3	2.65	0	2	18	43	38	+	+	Pale Yellowish Brown	SC	Clayey SAND	
WT-8	DB-2	18.40	18.70	9.6	-	-	-	2.62	0	1	12	48	39	-	-	Pale Yellowish Brown	SM	Silty CLAY	
WT-8	DB-3	45.50	45.90	8.0	2.23	35.3	21.5	2.64	0	0	16	39	45		5.9	923	Pale Yellowish Brown	SC	Clayey SAND
WT-8	DB-4	53.05	53.35	8.4	2.19	40.0	25.0	2.62	0	0	3	23	73		5.8	1106	Yellowish Gray	CL	Silty CLAY With Sand
WT-8	DB-5	59.60	59.90	8.2	-	45.5	27.7	2.69	4	2	8	25	62	-	-	Dark Yellowish Brown	CL	Sandy CLAY	
WT-8	DB-6	80.00	80.40	10.7	2.26	36.3	21.6	2.65	0	0	3	19	78		5.7	1060	Pale Yellowish Brown	CL	Silty CLAY With Sand
WT-8	DB-7	84.50	84.75	5.9	2.10	NP	NP	2.61	0	1	36	29	35	+	+	Grayish Brown	SM	Silty SAND	

Note : (*) ; Insufficient Soil for testing
 : (+) ; Sample Highly Disturbed

A3.3.1-14

Table 6.2 Soil Laboratory Test in 1998-1999(1/2)

Hole No	Sample No	Depth (m)	E50 including plasticity creep area (ton/m ²)	E50 in Elastic area (ton/m ²)	Poisson's Ratio	Coefficient of Volume Compressibility (m ² /kg)	Thickness (m)	Drawdown (m)	Specific Weight of Water (kg/m ³)	Settlement (mm)
WT-3	DB-2B	20.40-20.50	489	1525	0.3	4.87E-07	5	2.300	1000	5.60
WT-3	DB-3	27.20-27.50	1464	3250	0.3	2.29E-07	5	2.300	1000	2.63
WT-3	DB-4A	29.00-29.50	174	5700	0.3	1.30E-07	5	2.300	1000	1.50
WT-3	DB-4B	29.00-29.50	8177	11450	0.3	6.49E-08	5	2.300	1000	0.75
WT-3	DB-5A	33.00-33.45	472	850	0.3	8.74E-07	5	2.300	1000	10.05
WT-3	DB-5B	33.00-33.45	396	867	0.3	8.57E-07	5	2.300	1000	9.85
WT-3	DB-6A	36.10-36.60	1015	1577	0.3	4.71E-07	5	2.300	1000	5.42
WT-3	DB-6B	36.10-36.60	1150	19400	0.3	3.83E-08	5	2.300	1000	0.44
WT-4	DB-1	9.40-9.60	579	1367	0.3	5.43E-07	5	2.300	1000	6.25
WT-4	DB-2A	18.40-18.80	1463	2225	0.3	3.34E-07	5	2.300	1000	3.84
WT-4	DB-2B	18.40-18.80	1314	2308	0.3	3.22E-07	5	2.300	1000	3.70
WT-4	DB-3	29.20-29.50	786	1427	0.3	5.21E-07	5	2.300	1000	5.99
WT-4	DB-4A	32.50-32.95	8138	16250	0.3	4.57E-08	5	2.300	1000	0.53
WT-4	DB-4B	32.50-32.95	4553	7619	0.3	9.75E-08	5	2.300	1000	1.12
WT-4	DB-5	36.75-37.00	7498	23095	0.3	3.22E-08	5	2.300	1000	0.37
WT-4	DB-6A	54.60-55.00	2224	3926	0.3	1.89E-07	5	2.300	1000	2.18
WT-4	DB-6B	54.60-55.00	1393	1675	0.3	4.43E-07	5	2.300	1000	5.10
WT-4	DB-7A	57.10-57.45	1154	1541	0.3	4.82E-07	5	2.300	1000	5.54
WT-4	DB-7B	57.10-57.45	1414	1414	0.3	5.25E-07	5	2.300	1000	6.04
WT-5	DB-1	14.00-14.50	447	618	0.3	1.20E-06	5	2.300	1000	13.82
WT-5	DB-3A	46.70-47.00	1225	2286	0.3	3.25E-07	5	2.300	1000	3.74
WT-5	DB-3B	46.70-47.00	1059	1259	0.3	5.90E-07	5	2.300	1000	6.79
WT-5	DB-4	53.60-53.90	156	1864	0.3	3.99E-07	5	2.300	1000	4.58
WT-5	DB-5A	58.50-59.00	1819	1921	0.3	3.87E-07	5	2.300	1000	4.45

Table 6.2 Soil Laboratory Test in 1998-1999(2/2)

Hole No	Sample No	Depth (m)	E50 including plasticity creep area (ton/m ²)	E50 in Elastic area (ton/m ²)	Poisson's Ratio	Coefficient of Volume Compressibility (m ² /kg)	Thickness (m)	Drawdown (m)	Specific Weight of Water (kg/m ³)	Settlement (mm)
WT-7	DB-2B	19.00-19.60	428	1625	0.3	4.57E-07	5	2.300	1000	5.26
WT-7	DB-3	23.00-23.40	346	771	0.3	9.63E-07	5	2.300	1000	11.08
WT-7	DB-4A	27.15-27.50	2411	7667	0.3	9.69E-08	5	2.300	1000	1.11
WT-7	DB-4B	27.15-27.50	3411	7400	0.3	1.00E-07	5	2.300	1000	1.15
WT-7	DB-5A	35.50-35.80	401	1663	0.3	4.47E-07	5	2.300	1000	5.14
WT-7	DB-5B	35.50-35.80	475	1813	0.3	4.10E-07	5	2.300	1000	4.71
WT-7	DB-6A	46.20-46.75	618	1115	0.3	6.66E-07	5	2.300	1000	7.66
WT-7	DB-6B	46.20-46.75	718	1905	0.3	3.90E-07	5	2.300	1000	4.48
WT-7	DB-7	48.25-48.50	890	1500	0.3	4.95E-07	5	2.300	1000	5.70
WT-8	DB-1	9.40-9.80	362	383	0.3	1.94E-06	5	2.300	1000	22.31
WT-8	DB-3A	45.50-45.90	918	923	0.3	8.05E-07	5	2.300	1000	9.26
WT-8	DB-3B	45.50-45.90	927	927	0.3	8.01E-07	5	2.300	1000	9.22
WT-8	DB-4A	53.05-53.35	1220	1771	0.3	4.19E-07	5	2.300	1000	4.82
WT-8	DB-4B	53.05-53.35	992	1149	0.3	6.47E-07	5	2.300	1000	7.44
WT-8	DB-6	80.00-80.40	1060	1096	0.3	6.78E-07	5	2.300	1000	7.79
WT-8	DB-7	84.50-84.75	332	453	0.3	1.64E-06	5	2.300	1000	18.86

2) Additional 6 Consolidation Tests from the Surface Soil in 1998

Six additional consolidation tests were carried out in February 1998 because no soil sample was available to apply consolidation test from core drilling of that the former investigation stage. The 6 samples were taken by hand from the top artificial reclamation layer as shown in Figure 6.1. The test result is almost same as the Banteai Kdei conservation team. The results are shown in Table 6.3.

The calculation of deformation value was used a coefficient of volume compressibility (M_v). It is also same as the unconfined compression result as shown in Table 6.3. The 6 samples were collected from paddy field or clayey top soil by hand. Because the result is not likely representative underground condition, the actual field records from the monitoring were applied for the evaluation of land movement.

The monitoring records of land movement of LTb (In front of Angkor Wat) is compiled in Annex 3.4.1 “Hydrogeology”. The evaluation result of the influence to the Angkor Wat by the ground water exploitation is mentioned in Main Report, Chapter 4.2.4 “Groundwater”.

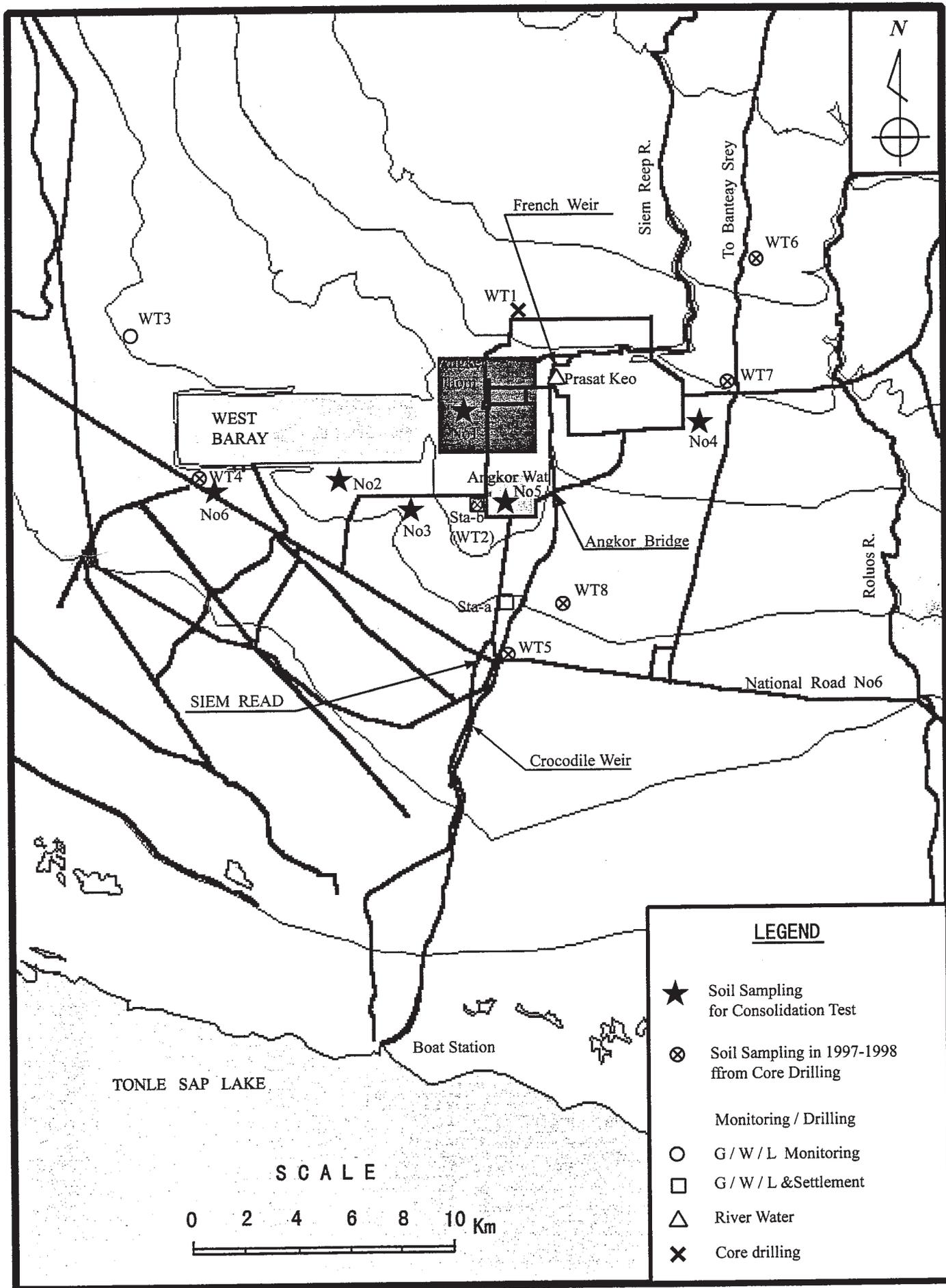


Figure 6.1 Location Map of Soil Sampling for Consolidation Test
A3.3.1-18

Table 6.3 Consolidation Test Results (1/3)

Location: Bayon Well No.87													
Sample No. 1													
Weight level	Vertical Stress (P) kg/cm ²	(dp) kg/cm ²	Consolidation (d) cm	Sample height(h) cm	Average height(h) cm	Compression Strain de %	Coefficient of compressibility cm ² /kg	of volume m ³ /kg	Thickness of layer m	Drawdown of groundwater m	Density of water (kg/m ³)	Consolidation settlement m	mm
0	0			19.000									
1	0.14	0.14	0.0170	18.983	18.992	0.0895	0.00639	6.394E-07	2.0	1.0	1000	0.00128	1.28
2	0.26	0.12	0.0410	18.942	18.963	0.2162	0.01802	1.802E-06	2.0	1.0	1000	0.00360	3.60
3	0.51	0.25	0.1100	18.832	18.887	0.5824	0.02330	2.330E-06	2.0	1.0	1000	0.00466	4.66
4	1.02	0.51	0.2880	18.544	18.688	1.5411	0.03022	3.022E-06	2.0	1.0	1000	0.00604	6.04
5	2.02	1.00	0.2960	18.248	18.396	1.6090	0.01609	1.609E-06	2.0	1.0	1000	0.00322	3.22
6	4.04	2.02	0.3140	17.934	18.091	1.7357	0.00859	8.592E-07	2.0	1.0	1000	0.00172	1.72
7	1.02	-3.02	-0.0240	17.958	17.946								
8	0.26	-0.76	-0.0210	17.979	17.969								

Location: Bayon Well No.87													
Sample No. 2													
Weight level	Vertical Stress (P) kg/cm ²	(dp) kg/cm ²	Consolidation (d) cm	Sample height(h) cm	Average height(h) cm	Compression Strain de %	Coefficient of compressibility cm ² /kg	of volume m ³ /kg	Thickness of layer m	Drawdown of groundwater m	Density of water (kg/m ³)	Consolidation settlement m	mm
0	0			19.000									
1	0.14	0.14	-0.057	19.057	19.029	-0.2996	-0.02140		0.5	1.0	1000	0.00000	0.00
2	0.26	0.12	0.012	19.045	19.051	0.0630	0.00525	5.249E-07	0.5	1.0	1000	0.00026	0.26
3	0.51	0.25	0.016	19.029	19.037	0.0840	0.00336	3.362E-07	0.5	1.0	1000	0.00017	0.17
4	1.02	0.51	0.023	19.006	19.018	0.1209	0.00237	2.371E-07	0.5	1.0	1000	0.00012	0.12
5	2.02	1	0.03	18.976	18.991	0.1580	0.00158	1.580E-07	0.5	1.0	1000	0.00008	0.08
6	4.04	2.02	0.03	18.946	18.961	0.1582	0.00078	7.833E-08	0.5	1.0	1000	0.00004	0.04
7	1.02	-3.02	-0.009	18.955	18.951								
8	0.26	-0.76	-0.015	18.970	18.963								

A3.3.1-19

Table 6.3 Consolidation Test Results (2/3)

Location: West of Angkor Wat(3k) Sample No.: 3

Weight level	Vertical Stress (P) kg/cm ²	(dp) kg/cm ²	Consolidation (d) cm	Sample height(h) cm	Average height(h) cm	Compression Strain de %	Coefficient of volume compressibility(mv) cm ² /kg	Thickness of layer m	Drawdown of groundwater m	Density of water (kg/m ³)	Consolidation settlement m	mm	
0	0			19.000									
1	0.14	0.14	-0.083	19.083	19.042	-0.4359	-0.03114	0.5	1.0	1000	0.00000	0.00	
2	0.26	0.12	0.056	19.027	19.055	0.2939	0.02449	2.449E-06	0.5	1.0	1000	0.00122	1.22
3	0.51	0.25	0.054	18.973	19.000	0.2842	0.01137	1.137E-06	0.5	1.0	1000	0.00057	0.57
4	1.02	0.51	0.096	18.877	18.925	0.5073	0.00995	9.946E-07	0.5	1.0	1000	0.00050	0.50
5	2.02	1.00	0.174	18.703	18.790	0.9260	0.00926	9.260E-07	0.5	1.0	1000	0.00046	0.46
6	4.04	2.02	0.422	18.281	18.492	2.2821	0.01130	1.130E-06	0.5	1.0	1000	0.00056	0.56
7	1.02	-3.02	0.01	18.271	18.276								
8	0.26	-0.76	-0.029	18.300	18.970								

Location: West of Angkor Wat(3k) Sample No.: 4

Weight level	Vertical Stress (P) kg/cm ²	(dp) kg/cm ²	Consolidation (d) cm	Sample height(h) cm	Average height(h) cm	Compression Strain de %	Coefficient of volume compressibility(mv) cm ² /kg	Thickness of layer m	Drawdown of groundwater m	Density of water (kg/m ³)	Consolidation settlement m	mm	
0	0			19.000									
1	0.14	0.14	-0.05	19.050	19.025	-0.2628	-0.01877	1.0	1.0	1000	0.00000	0.00	
2	0.26	0.12	0.051	18.999	19.025	0.2681	0.02234	2.234E-06	1.0	1.0	1000	0.00223	2.23
3	0.51	0.25	0.039	18.960	18.980	0.2055	0.00822	8.219E-07	1.0	1.0	1000	0.00082	0.82
4	1.02	0.51	0.071	18.889	18.925	0.3752	0.00736	7.356E-07	1.0	1.0	1000	0.00074	0.74
5	2.02	1.00	0.109	18.780	18.835	0.5787	0.00579	5.787E-07	1.0	1.0	1000	0.00058	0.58
6	4.04	2.02	0.228	18.552	18.666	1.2215	0.00605	6.047E-07	1.0	1.0	1000	0.00060	0.60
7	1.02	-3.02	-0.318	18.870	18.711								
8	0.26	-0.76	0.182	18.688	18.779								

A3.3.1-20

Table 6.3 Consolidation Test Results (3/3)

Location: WT4 Sample No.: 5													
Weight level	Vertical Stress (P) kg/cm ²	(dp) kg/cm ²	Consolidation (d) cm	Sample height(h) cm	Average height(h) cm	Compression Strain de %	Coefficient of volume compressibility(mv) cm ² /kg m ² /kg		Thickness of layer m	Drawdown of groundwater m	Density of water (kg/m ³)	Consolidation settlement m mm	
0	0	0.14	-0.065	19.000	19.033	-0.3415	-0.02439		1.0	1.0	1000	0.00000	0.00
1	0.14	0.12	0.065	19.065	19.033	0.3415	0.02846	2.846E-06	1.0	1.0	1000	0.00285	2.85
2	0.26	0.25	0.08	19.000	18.960	0.4219	0.01688	1.688E-06	1.0	1.0	1000	0.00169	1.69
3	0.51	0.51	0.109	18.920	18.866	0.5778	0.01133	1.133E-06	1.0	1.0	1000	0.00113	1.13
4	1.02	1.00	0.129	18.811	18.747	0.6881	0.00688	6.881E-07	1.0	1.0	1000	0.00069	0.69
5	2.02	2.02	0.175	18.682	18.595	0.9411	0.00466	4.659E-07	1.0	1.0	1000	0.00047	0.47
6	4.04	-3.02	-0.017	18.507	18.516								
7	1.02	-0.76	-0.04	18.524	18.544								
8	0.26			18.564									

Location: WT4 Sample No.: 6													
Weight level	Vertical Stress (P) kg/cm ²	(dp) kg/cm ²	Consolidation (d) cm	Sample height(h) cm	Average height(h) cm	Compression Strain de %	Coefficient of volume compressibility(mv) cm ² /kg m ² /kg		Thickness of layer m	Drawdown of groundwater m	Density of water (kg/m ³)	Consolidation settlement m mm	
0	0	0.14	-0.026	19.000	19.013	-0.1367	-0.00977		0.5	1.0	1000		
1	0.14	0.12	0.021	19.026	19.016	0.1104	0.00920	9.203E-07	0.5	1.0	1000	0.00046	0.46
2	0.26	0.25	0.04	19.005	18.985	0.2107	0.00843	8.428E-07	0.5	1.0	1000	0.00042	0.42
3	0.51	0.51	0.045	18.965	18.943	0.2376	0.00466	4.658E-07	0.5	1.0	1000	0.00023	0.23
4	1.02	1.00	0.044	18.920	18.898	0.2328	0.00233	2.328E-07	0.5	1.0	1000	0.00012	0.12
5	2.02	2.02	0.047	18.876	18.853	0.2493	0.00123	1.234E-07	0.5	1.0	1000	0.00006	0.06
6	4.04	-3.02	0	18.829	18.829								
7	1.02	-0.76	-0.02	18.829	18.839								
8	0.26			18.849									

A3.3.1-21