

2-2. DESIGN OF STAIR CASE



Summary

2 staircases will be constructed on both ends of the Rupsa main bridge. These 4 (2+2) staircases are typical. Therefore analysis and design are carried out for a single staircase. A mathematical model is prepared for the staircase including its sub-structures (pile caps & piles).

The design is carried out following the standard specification for Buildings/Structure,

- Bangladesh National Building Code-1993 (BNBC)
- Building Code Requirements for Reinforced Concrete (ACI 318-89/95)
- American Society for Testing and Materials (ASTM)

Mathematical Modeling of Structure

Super-structure: Columns are considered as vertical member of 3.0m height starting at pile cap. Interior beams at landing levels are considered as two-nodded horizontal member starting from column and extend up to edge of landing. All loads from landing slab are superimposed on landing beams. The exterior landing beams are divided into two consecutive beams with one intermediate node. The flight is considered as a two-nodded member connecting at the intermediate nodes of exterior landing beams. The dead loads and live load are uniformly distributed over the flight beams.

Sub-structure: Sub-structures are started at pile cap level. Pile cap is considered as two nodded horizontal members connected to piles. Piles are considered as consecutive discrete two nodded vertical members of 1.0m length. Supports are provided through the substructure nodes with horizontal spring constants. The spring constant values are derived from the values of spring constants of the piles of the adjacent pier.

Loads

The following loads and forces are considered in the design:

Dead load: Dead load is consists of the weight of the structure. The following weights are considered in computing the dead loads.

Concrete	24 kN/cubic meter
Steel	7849 kN/cubic meter

Live load: The live load is considered as per BNBC/93 (5 kN/sq. meter)

Wind load: The calculation of wind loads conforms to sub-clause 2.4, chapter 2, part 6 of BNBC.

Seismic load: Seismic load is not considered since the proposed site falls within the lowest seismic risk zone of Bangladesh (fig 6.2.10 BNBC, 1993)

Load Combinations

R.C structural members is designed to have strength not less that that required to resist the most unfavorable effect of the combinations of the factored loads shown below.

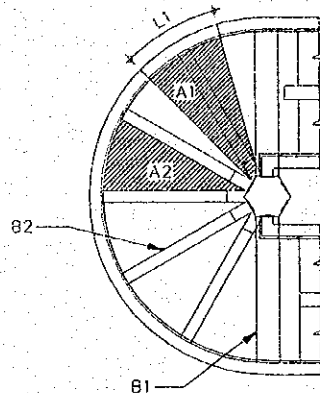
1. $1.4D + 1.7L$
2. $0.75(1.4D+1.7L+1.7W)$

(Ref. BNBC, Part 6, Sec. 2.7.5)

However for geotechnical design of foundation (eg. Number and length of ples) following load combination shall be considered.

1. D+L
2. D+L+W

Calculation of Superimposed Loads



Length of beam	=	3.80 m
Area of landing supported by each interior beam (A1)	=	3.26 m ²
Area of slab supported by each interior beam (A2)	=	2.39 m ²
Unit weight of concrete	=	24.00 kN/m ³
Live load	=	5.00 kN/m ²
Thickness of slab	=	0.30 m

Dead Load over Beams from Slab

Dead load of slab = 2.39 X 0.3 X 24	=	17.21 kN
Uniform dead load over the beam = 17.208/3.8	=	4.53 kN/m
Triangular distribution of load over beam (B2)	=	9.06 kN ▶ 0 kN
Triangular distribution of load over beam (B1)	=	4.53 kN ▶ 0 kN

Dead Load over Beams from Edge Wall

Length of edge beam segment (L1)	=	1.92 M
Width of edge beam	=	0.25 M
Depth of edge beam	=	0.45 M
Dead load from edge beam segment = 0.25X0.45X1.92X24	=	5.184 kN
Concreted load over beam (B2)	=	5.18 kN
Concreted load over beam (B1)	=	2.59 kN

Live Load over Beams from Slab

Load from landing = 3.26 X 5	=	16.30 kN
Uniform live load over the beam = 16.3/3.8	=	4.29 kN/m
Triangular distribution of load over beam (B2)	=	8.58 kN ▶ 0 kN
Triangular distribution of load over beam (B1)	=	4.29 kN ▶ 0 kN

Calculation of Basic Wind Speed

As per Bangladesh National Building Code (BNBC), 1993

Sustained wind pressure q_z

$$= C_c C_1 C_2 V_b^2$$

Where $C_c = 47.2 \times 10^{-6}$, a constant

$C_1 =$ structure importance co-efficient
 $= 0.8$, low risk structure (Table 6.2.9, BNBC)

$C_2 =$ height and exposure coefficient (Table 6.2.9, BNBC)
 $= 1.0$, for 10m average height & open country (B)

$V_b = 238$ km/h, base wind velocity

$$q_z = 2.14 \text{ kN/m}^2$$

Design Wind Pressure p_z

$$= C_G C_p C_2 q_z$$

Where $C_G =$ Gust co-efficient
 $= 1.4$ (Sec. 2.4.6.6)

$C_p = 1.3$ (Sec 2.4.6.7)

$$p_z = 1.4 \times 1.3 \times 2.14$$

$$= 3.8948 \text{ kN/m}^2$$

$$= 4.00 \text{ kN/m}^2 \text{ (say)}$$

Wind Load on Structure

Wind Load along X axis

Exposed height of flight $= 0.6$ m

Exposed height of railing (10% of actual height) $= 0.09$ m

Total exposed height of flight & railing $= 0.6 + 0.09$
 $= 0.69$ m

Considering average column diameter of 1.2 m

Length of flight $= 8 - 0.6 \times 2 = 6.8$ m

(actual length was not considered since some area is common to two consecutive flight)

Wind load from flight $= 18.8$ kN

Length of landing $= (3.8 - 0.6)$ m

$= 3.2$ m

Depth of landing $= (10\% \times 0.9 + 0.45 + (0.6 + 0.9)/2)$

$= 1.29$ m

Wind load from landing $= 16.6$ kN

Load at start of first flight $= 9.4$ kN

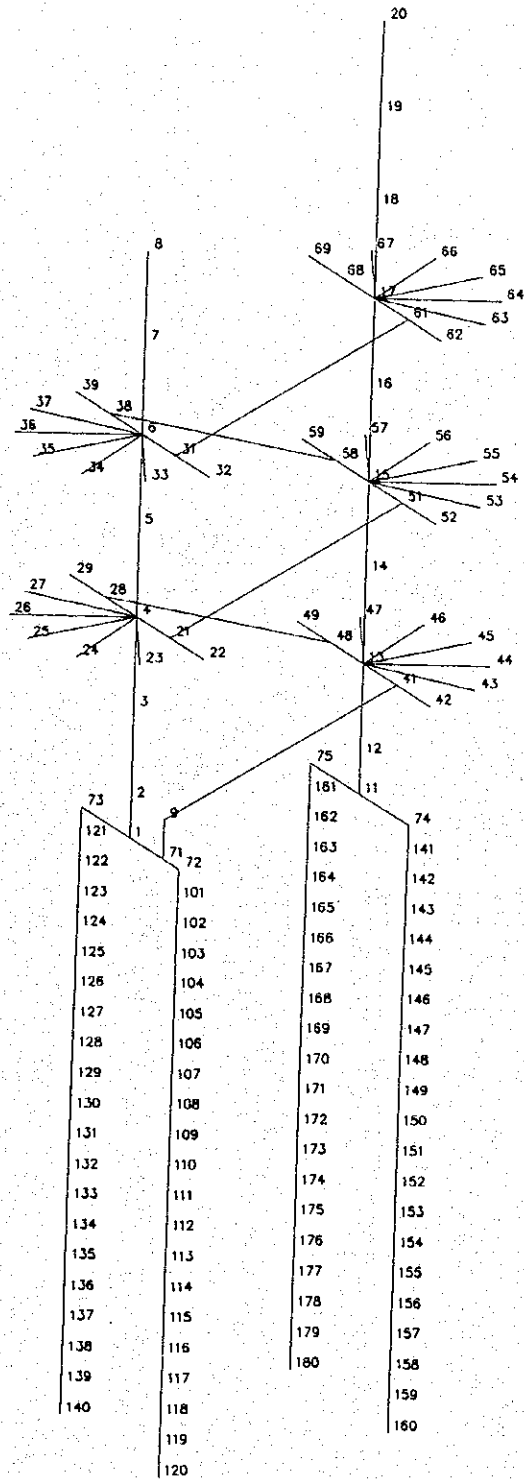
Load at junctions of intermediate flights $= 35.4$ kN

Load at end of last flight $= 26.0$ kN

The typical analysis and design output of the staircase is presented wherewith.

MEMBER	LOAD	JT	AXIAL	SHEAR-Y	SHEAR-Z	TORSION	MOM-Y	MOM-Z
		19	-14.57	.00	9.18	.00	-13.85	-.02
	10	18	35.56	-21.42	.00	.00	.29	-59.59
		19	-14.57	9.18	.00	.00	-.08	13.75
101	7	72	1646.30	-35.97	46.76	.00	-164.20	-204.32
		101	-1661.29	35.97	-46.76	.00	117.63	168.75
	8	72	2387.79	-52.70	67.88	.00	-238.18	-298.99
		101	-2408.78	52.70	-67.88	.00	170.56	246.84
	9	72	2285.98	-127.73	170.27	.00	-383.96	-428.82
		101	-2301.72	127.73	-170.27	.00	213.69	301.50
	10	72	2072.11	-281.84	64.17	.00	-205.98	-412.45
		101	-2087.85	281.84	-64.17	.00	141.99	130.56
121	7	73	1350.28	1.06	-38.16	.00	167.16	-123.65
		121	-1365.27	-1.06	38.16	.00	-129.23	125.05
	8	73	1954.37	1.52	-55.32	.00	242.56	-180.81
		121	-1975.35	-1.52	55.32	.00	-187.57	182.82
	9	73	739.25	70.97	91.72	.00	-75.99	-21.93
		121	-754.99	-70.97	-91.72	.00	-15.81	93.15
	10	73	1499.61	-65.68	-36.28	.00	186.04	22.80
		121	-1515.35	65.68	36.28	.00	-150.00	-88.47
141	7	74	1769.14	23.64	48.77	.00	-195.89	-82.99
		141	-1784.13	-23.64	-48.77	.00	147.41	107.12
	8	74	2564.48	34.69	70.73	.00	-284.12	-120.63
		141	-2585.46	-34.69	-70.73	.00	213.82	156.03
	9	74	2301.97	41.68	149.90	.00	-378.88	-94.97
		141	-2317.71	-41.68	-149.90	.00	229.16	137.24
	10	74	1549.92	6.61	30.08	.00	-135.23	174.95
		141	-1565.66	-6.61	-30.08	.00	105.46	-167.44
161	7	75	1716.86	11.27	-57.37	.00	218.22	-107.91
		161	-1731.85	-11.27	57.37	.00	-161.14	119.65
	8	75	2487.21	16.49	-83.29	.00	316.81	-157.30
		161	-2508.19	-16.49	83.29	.00	-233.93	174.45
	9	75	1166.31	15.08	44.81	.00	32.29	-148.76
		161	-1182.05	-15.08	-44.81	.00	-77.28	164.25
	10	75	1371.87	-130.07	-57.98	.00	208.16	-99.70
		161	-1387.61	130.07	57.98	.00	-150.50	-29.54

***** END OF LATEST ANALYSIS RESULT *****

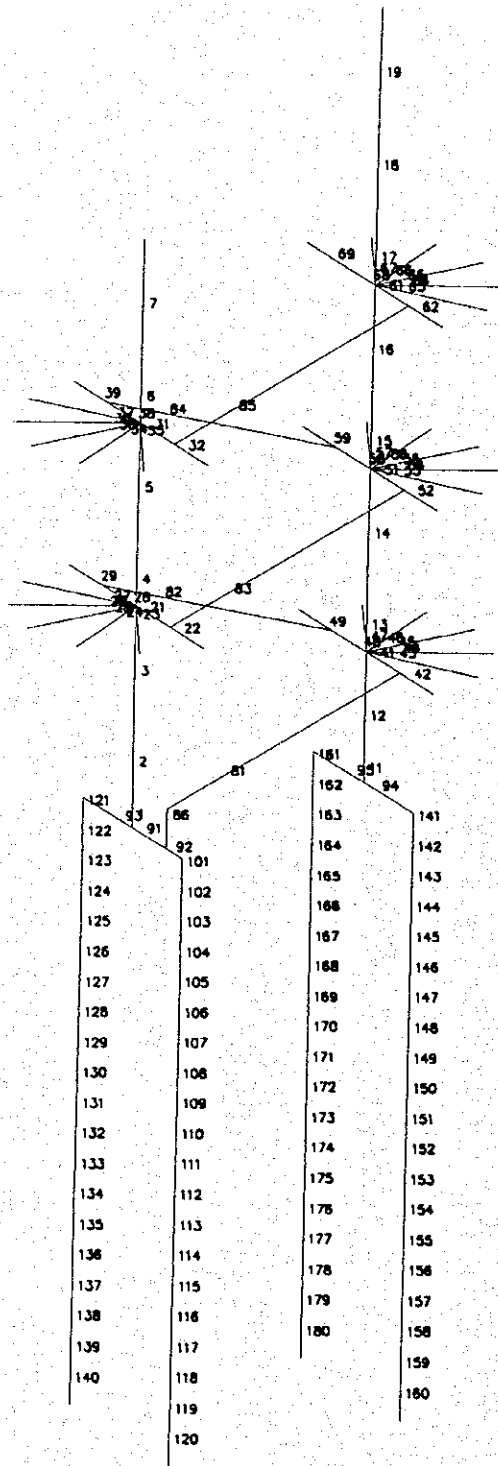


STAIRCASE (Node Number)

J=149,M=152

UNIT MET KNS

MN/ELEM



STAIRCASE (Member Number)

J=149,M=152

UNIT MET KNS


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*           S T A A D - III
*           Revision 22.3a
*           Proprietary Program of
*           Research Engineers, Inc.
*           Date=   FEB 29, 2000
*           Time=   15:52: 5
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*           USER ID: Development Design Consultants L
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1. STAAD SPACE RUPSA BRIDGE (ANALYSIS OF STAIRCASE)
2. UNIT KNS METER
3. PAGE EJE
4. JOINT COORDINATES
5. 1 0 0 0; 2 0 1.3 0 8 0 19.3 0; 9 0 1.3 1.9
6. 11 8 0 0; 12 8 1.3 0 20 8 25.3 0
7. *
8. 21 0 7.3 1.9; 22 0 7.3 3.8; 23 -1.9 7.3 3.291; 24 -3.291 7.3 1.9
9. 25 -3.8 7.3 0; 26 -3.291 7.3 -1.9; 27 -1.9 7.3 -3.291; 28 0 7.3 -1.9
10. 29 0 7.3 -3.8
11. 31 0 13.3 1.9; 32 0 13.3 3.8; 33 -1.9 13.3 3.291; 34 -3.291 13.3 1.9
12. 35 -3.8 13.3 0; 36 -3.291 13.3 -1.9; 37 -1.9 13.3 -3.291; 38 0 13.3 -1.9
13. 39 0 13.3 -3.8
14. 41 8 4.3 1.9; 42 8 4.3 3.8; 43 9.9 4.3 3.291; 44 11.291 4.3 1.9
15. 45 11.8 4.3 0; 46 11.291 4.3 -1.9; 47 9.9 4.3 -3.291; 48 8 4.3 -1.9
16. 49 8 4.3 -3.8
17. 51 8 10.3 1.9; 52 8 10.3 3.8; 53 9.9 10.3 3.291; 54 11.291 10.3 1.9
18. 55 11.8 10.3 0; 56 11.291 10.3 -1.9; 57 9.9 10.3 -3.291; 58 8 10.3 -1.9
19. 59 8 10.3 -3.8
20. 61 8 16.3 1.9; 62 8 16.3 3.8; 63 9.9 16.3 3.291; 64 11.291 16.3 1.9
21. 65 11.8 16.3 0; 66 11.291 16.3 -1.9; 67 9.9 16.3 -3.291; 68 8 16.3 -1.9
22. 69 8 16.3 -3.8
24. *PILECAP
25. 71 0 0 1.9; 72 0 0 2.8; 73 0 0 -2.8; 74 8 0 2.8; 75 8 0 -2.8
27. *PILE
28. 101 0 -1 2.8 120 0 -20.0 2.8
29. 121 0 -1 -2.8 140 0 -20.0 -2.8
30. 141 8 -1 2.8 160 8 -20.0 2.8
31. 161 8 -1 -2.8 180 8 -20.0 -2.8
33. MEMBER INCIDENCE
34. *COLUMN 1 TO 7; 11 TO 19
35. 1 1 2 7; 11 11 12 19
37. *BEAM 21 TO 29; 31 TO 39; 41 TO 49; 51 TO 59; 61 TO 69
38. 21 4 21; 22 21 22; 23 4 23; 24 4 24; 25 4 25; 26 4 26
39. 27 4 27; 28 4 28; 29 28 29
40. 31 6 31; 32 31 32; 33 6 33; 34 6 34; 35 6 35; 36 6 36
41. 37 6 37; 38 6 38; 39 38 39
42. 41 13 41; 42 41 42; 43 13 43; 44 13 44; 45 13 45; 46 13 46
43. 47 13 47; 48 13 48; 49 48 49
44. 51 15 51; 52 51 52; 53 15 53; 54 15 54; 55 15 55; 56 15 56
45. 57 15 57; 58 15 58; 59 58 59
46. 61 17 61; 62 61 62; 63 17 63; 64 17 64; 65 17 65; 66 17 66
47. 67 17 67; 68 17 68; 69 68 69
49. *FLIGHT 81 TO 86
50. 81 9 41; 82 28 48; 83 21 51; 84 38 58; 85 31 61
51. 86 9 71
53. *PILECAP 91 TO 95
54. 91 1 71; 92 71 72; 93 1 73; 94 11 74; 95 11 75
56. *PILE
57. 101 72 101; 102 101 102 120
58. 121 73 121; 122 121 122 140
59. 141 74 141; 142 141 142 160
60. 161 75 161; 162 161 162 180
62. MEMBER OFFSET
63. 1 11 START 0.70
64. 21 23 TO 28 START 0.35
65. 31 33 TO 38 START 0.30
66. 41 43 TO 48 START 0.45

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67. 51 53 TO 58 START 0.35
 68. 61 63 TO 68 START 0.30
 69. 86 END 0.70
 71. MEMBER PROPERETY
 72. *COLUMN
 73. 1 2 3 11 12 13 PRI YD 1.2
 74. 4 5 14 15 PRI YD 1.0
 75. 6 16 17 PRI YD 0.8
 76. 18 PRI YD 0.6
 77. 19 PRI YD 0.5
 78. 7 PRI YD 0.3
 80. *BEAM
 81. 21 28 31 38 41 48 51 58 61 68 PRI YD 1.20 ZD 0.55
 82. 22 29 32 39 42 49 52 59 62 69 PRI YD 0.95 ZD 0.55
 83. 23 TO 27 33 TO 37 43 TO 47 PRI YD 1.20 ZD 0.25
 84. 53 TO 57 63 TO 67 PRI YD 1.20 ZD 0.25
 86. *FLIGHT
 87. 81 TO 85 PRI YD 0.300 ZD 3.2
 88. 86 PRI YD 0.500 ZD 3.2
 90. *PILECAP
 91. 91 TO 95 PRI YD 1.40 ZD 2.2
 93. *PILE
 94. 101 TO 180 PRI YD 0.9
 96. CONSTANTS
 97. E CONCRETE ALL
 98. DEN CON ALL
 100. SUPPORT
 101. *72 TO 75 FIXED
 102. 101 TO 161 BY 20 FIXED BUT FY MX MY MZ KFX 15961 KFZ 11971
 103. 102 TO 162 BY 20 FIXED BUT FY MX MY MZ KFX 17474 KFZ 13105
 104. 103 TO 163 BY 20 FIXED BUT FY MX MY MZ KFX 18755 KFZ 14066
 105. 104 TO 164 BY 20 FIXED BUT FY MX MY MZ KFX 19896 KFZ 14922
 106. 105 TO 165 BY 20 FIXED BUT FY MX MY MZ KFX 20939 KFZ 15704
 107. 106 TO 166 BY 20 FIXED BUT FY MX MY MZ KFX 21908 KFZ 16431
 108. 107 TO 167 BY 20 FIXED BUT FY MX MY MZ KFX 61700 KFZ 46275
 109. 108 TO 168 BY 20 FIXED BUT FY MX MY MZ KFX 62563 KFZ 46922
 110. 109 TO 169 BY 20 FIXED BUT FY MX MY MZ KFX 63387 KFZ 47540
 111. 110 TO 170 BY 20 FIXED BUT FY MX MY MZ KFX 64177 KFZ 48133
 112. 111 TO 171 BY 20 FIXED BUT FY MX MY MZ KFX 64937 KFZ 48703
 113. 112 TO 172 BY 20 FIXED BUT FY MX MY MZ KFX 65672 KFZ 49254
 114. 113 TO 173 BY 20 FIXED BUT FY MX MY MZ KFX 66383 KFZ 49787
 115. 114 TO 174 BY 20 FIXED BUT FY MX MY MZ KFX 67073 KFZ 50305
 116. 115 TO 175 BY 20 FIXED BUT FY MX MY MZ KFX 67744 KFZ 50808
 117. 116 TO 176 BY 20 FIXED BUT FY MX MY MZ KFX 68399 KFZ 51299
 118. 117 TO 177 BY 20 FIXED BUT FY MX MY MZ KFX 69037 KFZ 51778
 119. 118 TO 178 BY 20 FIXED BUT FY MX MY MZ KFX 69662 KFZ 52247
 120. 119 TO 179 BY 20 FIXED BUT FY MX MY MZ KFX 70273 KFZ 52705
 121. 120 TO 180 BY 20 PINNED
 123. LOAD 1 : SELFWEIGHT
 124. SELF
 126. LOAD 2 : DEAD LOAD OVER FLIGHTS AND BEAMS
 127. MEM LOAD
 128. *OVER FLIGHTS FROM STEP+SIDEWALL+RAILING (4.47+3.6+10)
 129. 81 TO 85 UNI GY -18.07
 131. *DEDUCTION OF BEAM SELFWEIGHT
 132. 22 29 32 39 42 49 52 59 62 69 TRA GY 0.0 1.98
 133. 23 TO 27 33 TO 37 43 TO 47 TRA GY 0.0 1.80
 134. 53 TO 57 63 TO 67 TRA GY 0.0 1.80
 136. *OVER BEAMS FROM SLAB
 137. 21 28 31 38 41 48 51 58 61 68 TRA GY -0.0 -2.275
 138. 22 29 32 39 42 49 52 59 62 69 TRA GY -2.275 -4.550
 139. 23 TO 27 33 TO 37 43 TO 47 TRA GY -0.0 -9.100
 140. 53 TO 57 63 TO 67 TRA GY -0.0 -9.100
 142. *OVER BEAMS FROM SIDE WALL
 143. 22 29 32 39 42 49 52 59 62 69 CON GY -2.6
 144. 23 TO 27 33 TO 37 43 TO 47 CON GY -5.2
 145. 53 TO 57 63 TO 67 CON GY -5.2
 147. *OVER BEAMS FROM RAILING
 148. 22 29 32 39 42 49 52 59 62 69 CON GY -4.81
 149. 23 TO 27 33 TO 37 43 TO 47 CON GY -9.62

150. 53 TO 57 63 TO 67 CON GY -9.62
 152. LOAD 3 : FULL LIVE LOAD (5.0 KN/M^2)
 153. MEM LOAD
 154. *OVER FLIGHT
 155. 81 TO 85 UNI GY -13.5
 156. *OVER BEAMS
 157. 21 28 31 38 41 48 51 58 61 68 TRA GY -0.0 -2.15
 158. 22 29 32 39 42 49 52 59 62 69 TRA GY -2.15 -4.30
 159. 23 TO 27 33 TO 37 43 TO 47 TRA GY -0.0 -8.60
 160. 53 TO 57 63 TO 67 TRA GY -0.0 -8.60
 162. LOAD 4 : PARTIAL LIVE LOAD
 163. MEM LOAD
 164. *OVER FLIGHT
 165. 81 83 85 UNI GY -13.5
 166. *OVER BEAMS
 167. 21 31 41 51 61 TRA GY -0.0 -2.15
 168. 22 32 42 52 62 TRA GY -2.15 -4.30
 169. 23 TO 25 33 TO 35 43 TO 45 TRA GY -0.0 -8.60
 170. 53 TO 55 63 TO 65 TRA GY -0.0 -8.60
 172. LOAD 5 : WIND LOAD IN Z DIRECTION (WIND PRESSURE=4.0 KN/M^2)
 173. *EXP. HEIGHT = 0.6+10%0.9=0.69M
 174. *LOAD FROM FLIGHT=18.8, FROM LANDING=16.6
 175. JOINT LOAD
 176. 2 FZ 9.4
 177. 4 6 13 15 FZ 35.4
 178. 17 FZ 26.0
 179. MEM LOAD
 180. 2 3 12 13 UNI GZ 5.6
 181. 4 5 14 15 UNI GZ 4.8
 182. 6 16 17 UNI GZ 4.0
 183. 18 UNI GZ 3.2
 184. 19 UNI GZ 2.4
 185. 7 UNI GZ 1.2
 187. LOAD 6 : WIND LOAD IN X DIRECTION (WIND PRESSURE=4.0 KN/M^2)
 188. JOINT LOAD
 189. 12 FX -24.2
 190. 13 TO 17 FX -48.4
 191. MEM LOAD
 192. 12 13 UNI GX -5.6
 193. 14 15 UNI GX -4.8
 194. 16 17 UNI GX -4.0
 195. 18 UNI GX -3.2
 196. 19 UNI GX -2.4
 198. LOAD COMB 7 : DL+LL
 199. 1 1.0 2 1.0 3 1.0
 201. LOAD COMB 8 : 1.4DL+1.7LL
 202. 1 1.4 2 1.4 3 1.7
 204. LOAD COMB 9 : 0.75(1.4DL+1.7LL+1.7WL(Z))
 205. 1 1.05 2 1.05 4 1.275 5 1.275
 207. LOAD COMB 10 : 0.75(1.4DL+1.7LL+1.7WL(X))
 208. 1 1.05 2 1.05 4 1.275 6 1.275
 210. PDELTA ANALYSIS

PROBLEM STATISTICS

NUMBER OF JOINTS/MEMBER+ELEMENTS/SUPPORTS = 149/ 152/ 80
 ORIGINAL/FINAL BAND-WIDTH = 66/ 18
 TOTAL PRIMARY LOAD CASES = 6, TOTAL DEGREES OF FREEDOM = 882
 SIZE OF STIFFNESS MATRIX = 100548 DOUBLE PREC. WORDS
 REQRD/AVAIL. DISK SPACE = 12.84/ 231.8 MB, EXMEM = 1965.6 MB

++ Processing Element Stiffness Matrix. 15:52: 6
 ++ Processing Global Stiffness Matrix. 15:52: 6
 ++ Processing Triangular Factorization. 15:52: 6
 ++ Calculating Joint Displacements. 15:52: 6
 ++ Adjusting Displacements 15:52: 6
 ++ Calculating Member Forces. 15:52: 7

211. LOAD LIST 7 TO 10
 212. PRINT SUPPORT REACTION

SUPPORT REACTIONS		-UNIT	KNS	METE	STRUCTURE TYPE = SPACE		
JOINT	LOAD	FORCE-X	FORCE-Y	FORCE-Z	MOM-X	MOM-Y	MOM Z
101	7	-4.47	.00	-6.06	.00	.00	.00
	8	-6.50	.00	-8.81	.00	.00	.00
	9	17.47	.00	-43.24	.00	.00	.00
	10	98.28	.00	-10.23	.00	.00	.00
121	7	-14.43	.00	1.66	.00	.00	.00
	8	-21.08	.00	2.38	.00	.00	.00
	9	-38.80	.00	-36.06	.00	.00	.00
	10	36.16	.00	-1.10	.00	.00	.00
141	7	-21.50	.00	-3.87	.00	.00	.00
	8	-31.42	.00	-5.61	.00	.00	.00
	9	-32.10	.00	-34.00	.00	.00	.00
	10	15.87	.00	-.96	.00	.00	.00
161	7	-17.94	.00	5.77	.00	.00	.00
	8	-26.17	.00	8.38	.00	.00	.00
	9	-24.42	.00	-24.43	.00	.00	.00
	10	55.16	.00	7.04	.00	.00	.00
102	7	1.55	.00	-7.79	.00	.00	.00
	8	2.30	.00	-11.32	.00	.00	.00
	9	22.92	.00	-40.29	.00	.00	.00
	10	81.77	.00	-11.77	.00	.00	.00
122	7	-7.65	.00	4.51	.00	.00	.00
	8	-11.17	.00	6.52	.00	.00	.00
	9	-27.92	.00	-28.85	.00	.00	.00
	10	25.97	.00	2.78	.00	.00	.00
142	7	-13.79	.00	-6.75	.00	.00	.00
	8	-20.16	.00	-9.79	.00	.00	.00
	9	-21.27	.00	-33.18	.00	.00	.00
	10	7.55	.00	-3.36	.00	.00	.00
162	7	-10.59	.00	8.62	.00	.00	.00
	8	-15.45	.00	12.52	.00	.00	.00
	9	-14.38	.00	-17.92	.00	.00	.00
	10	42.82	.00	9.40	.00	.00	.00
103	7	5.14	.00	-8.05	.00	.00	.00
	8	7.53	.00	-11.69	.00	.00	.00
	9	23.51	.00	-33.89	.00	.00	.00
	10	61.32	.00	-11.46	.00	.00	.00
123	7	-2.46	.00	5.86	.00	.00	.00
	8	-3.59	.00	8.49	.00	.00	.00
	9	-17.65	.00	-20.97	.00	.00	.00
	10	16.38	.00	4.99	.00	.00	.00
143	7	-7.25	.00	-7.87	.00	.00	.00
	8	-10.62	.00	-11.41	.00	.00	.00
	9	-11.86	.00	-28.95	.00	.00	.00
	10	1.42	.00	-4.55	.00	.00	.00
163	7	-4.67	.00	9.52	.00	.00	.00
	8	-6.83	.00	13.82	.00	.00	.00
	9	-6.32	.00	-11.70	.00	.00	.00
	10	29.81	.00	9.88	.00	.00	.00
104	7	6.56	.00	-7.26	.00	.00	.00
	8	9.61	.00	-10.54	.00	.00	.00
	9	20.78	.00	-25.99	.00	.00	.00
	10	41.30	.00	-9.92	.00	.00	.00
124	7	.91	.00	5.99	.00	.00	.00
	8	1.34	.00	8.68	.00	.00	.00
	9	-9.32	.00	-13.70	.00	.00	.00
	10	8.62	.00	5.75	.00	.00	.00
144	7	-2.46	.00	-7.62	.00	.00	.00
	8	-3.62	.00	-11.05	.00	.00	.00
	9	-4.78	.00	-22.96	.00	.00	.00
	10	-2.36	.00	-4.73	.00	.00	.00
164	7	-.59	.00	8.94	.00	.00	.00
	8	-.87	.00	12.98	.00	.00	.00
	9	-.76	.00	-6.54	.00	.00	.00
	10	18.29	.00	9.01	.00	.00	.00

JOINT	LOAD	FORCE-X	FORCE-Y	FORCE-Z	MOM-X	MOM-Y	MOM Z
105	7	6.39	.00	-5.85	.00	.00	.00
	8	9.36	.00	-8.49	.00	.00	.00
	9	16.24	.00	-18.03	.00	.00	.00
	10	24.26	.00	-7.73	.00	.00	.00
125	7	2.63	.00	5.28	.00	.00	.00
	8	3.85	.00	7.65	.00	.00	.00
	9	-3.38	.00	-7.73	.00	.00	.00
	10	3.09	.00	5.43	.00	.00	.00
145	7	.53	.00	-6.48	.00	.00	.00
	8	.75	.00	-9.39	.00	.00	.00
	9	-.19	.00	-16.50	.00	.00	.00
	10	-4.09	.00	-4.22	.00	.00	.00
165	7	1.74	.00	7.43	.00	.00	.00
	8	2.54	.00	10.79	.00	.00	.00
	9	2.41	.00	-2.72	.00	.00	.00
	10	9.28	.00	7.32	.00	.00	.00
106	7	5.23	.00	-4.20	.00	.00	.00
	8	7.66	.00	-6.09	.00	.00	.00
	9	11.17	.00	-10.99	.00	.00	.00
	10	11.36	.00	-5.38	.00	.00	.00
126	7	3.08	.00	4.09	.00	.00	.00
	8	4.51	.00	5.94	.00	.00	.00
	9	.27	.00	-3.32	.00	.00	.00
	10	-.30	.00	4.44	.00	.00	.00
146	7	2.00	.00	-4.88	.00	.00	.00
	8	2.90	.00	-7.08	.00	.00	.00
	9	2.23	.00	-10.50	.00	.00	.00
	10	-4.30	.00	-3.30	.00	.00	.00
166	7	2.66	.00	5.49	.00	.00	.00
	8	3.88	.00	7.97	.00	.00	.00
	9	3.65	.00	-.23	.00	.00	.00
	10	3.06	.00	5.30	.00	.00	.00
107	7	9.90	.00	-7.10	.00	.00	.00
	8	14.48	.00	-10.30	.00	.00	.00
	9	17.62	.00	-14.69	.00	.00	.00
	10	7.46	.00	-8.73	.00	.00	.00
127	7	7.36	.00	7.53	.00	.00	.00
	8	10.77	.00	10.92	.00	.00	.00
	9	5.51	.00	-1.18	.00	.00	.00
	10	-5.22	.00	8.56	.00	.00	.00
147	7	6.35	.00	-8.70	.00	.00	.00
	8	9.25	.00	-12.61	.00	.00	.00
	9	8.14	.00	-15.07	.00	.00	.00
	10	-9.70	.00	-6.12	.00	.00	.00
167	7	7.07	.00	9.59	.00	.00	.00
	8	10.31	.00	13.92	.00	.00	.00
	9	9.68	.00	3.01	.00	.00	.00
	10	-1.69	.00	9.05	.00	.00	.00
108	7	5.75	.00	-3.58	.00	.00	.00
	8	8.41	.00	-5.18	.00	.00	.00
	9	7.81	.00	-4.22	.00	.00	.00
	10	-5.34	.00	-4.11	.00	.00	.00
128	7	5.34	.00	4.28	.00	.00	.00
	8	7.80	.00	6.22	.00	.00	.00
	9	6.65	.00	3.01	.00	.00	.00
	10	-6.25	.00	5.18	.00	.00	.00
148	7	5.47	.00	-4.75	.00	.00	.00
	8	7.97	.00	-6.89	.00	.00	.00
	9	7.44	.00	-5.41	.00	.00	.00
	10	-6.72	.00	-3.52	.00	.00	.00
168	7	5.51	.00	5.08	.00	.00	.00
	8	8.04	.00	7.37	.00	.00	.00
	9	7.53	.00	4.29	.00	.00	.00
	10	-6.14	.00	4.63	.00	.00	.00
109	7	2.65	.00	-1.19	.00	.00	.00
	8	3.88	.00	-1.73	.00	.00	.00
	9	1.65	.00	1.71	.00	.00	.00
	10	-10.27	.00	-1.09	.00	.00	.00

JOINT	LOAD	FORCE-X	FORCE-Y	FORCE-Z	MOM-X	MOM-Y	MOM Z
129	7	3.32	.00	1.92	.00	.00	.00
	8	4.85	.00	2.79	.00	.00	.00
	9	5.85	.00	4.54	.00	.00	.00
	10	-5.48	.00	2.59	.00	.00	.00
149	7	3.96	.00	-1.95	.00	.00	.00
	8	5.78	.00	-2.82	.00	.00	.00
	9	5.62	.00	.34	.00	.00	.00
	10	-3.98	.00	-1.61	.00	.00	.00
169	7	3.68	.00	1.94	.00	.00	.00
	8	5.37	.00	2.82	.00	.00	.00
	9	5.02	.00	4.16	.00	.00	.00
	10	-7.00	.00	1.61	.00	.00	.00
110	7	.67	.00	.19	.00	.00	.00
	8	.98	.00	.29	.00	.00	.00
	9	-1.56	.00	4.33	.00	.00	.00
	10	-10.50	.00	.59	.00	.00	.00
130	7	1.70	.00	.41	.00	.00	.00
	8	2.49	.00	.59	.00	.00	.00
	9	4.30	.00	4.48	.00	.00	.00
	10	-4.01	.00	.84	.00	.00	.00
150	7	2.45	.00	-.22	.00	.00	.00
	8	3.58	.00	-.31	.00	.00	.00
	9	3.64	.00	3.11	.00	.00	.00
	10	-1.90	.00	-.37	.00	.00	.00
170	7	2.08	.00	.05	.00	.00	.00
	8	3.04	.00	.07	.00	.00	.00
	9	2.83	.00	3.34	.00	.00	.00
	10	-5.98	.00	-.16	.00	.00	.00
111	7	-.39	.00	.84	.00	.00	.00
	8	-.57	.00	1.22	.00	.00	.00
	9	-2.73	.00	4.83	.00	.00	.00
	10	-8.47	.00	1.31	.00	.00	.00
131	7	.61	.00	-.41	.00	.00	.00
	8	.89	.00	-.59	.00	.00	.00
	9	2.70	.00	3.64	.00	.00	.00
	10	-2.52	.00	-.17	.00	.00	.00
151	7	1.26	.00	.67	.00	.00	.00
	8	1.84	.00	.98	.00	.00	.00
	9	1.98	.00	3.91	.00	.00	.00
	10	-.55	.00	.30	.00	.00	.00
171	7	.91	.00	-.89	.00	.00	.00
	8	1.33	.00	-1.30	.00	.00	.00
	9	1.24	.00	2.34	.00	.00	.00
	10	-4.30	.00	-1.01	.00	.00	.00
112	7	-.80	.00	1.01	.00	.00	.00
	8	-1.17	.00	1.46	.00	.00	.00
	9	-2.72	.00	4.20	.00	.00	.00
	10	-5.80	.00	1.43	.00	.00	.00
132	7	-.02	.00	-.74	.00	.00	.00
	8	-.04	.00	-1.07	.00	.00	.00
	9	1.41	.00	2.57	.00	.00	.00
	10	-1.31	.00	-.63	.00	.00	.00
152	7	.44	.00	.99	.00	.00	.00
	8	.65	.00	1.43	.00	.00	.00
	9	.80	.00	3.60	.00	.00	.00
	10	.19	.00	.58	.00	.00	.00
172	7	.19	.00	-1.19	.00	.00	.00
	8	.27	.00	-1.73	.00	.00	.00
	9	.25	.00	1.42	.00	.00	.00
	10	-2.64	.00	-1.24	.00	.00	.00
113	7	-.82	.00	.91	.00	.00	.00
	8	-1.21	.00	1.31	.00	.00	.00
	9	-2.16	.00	3.13	.00	.00	.00
	10	-3.38	.00	1.23	.00	.00	.00
133	7	-.31	.00	-.76	.00	.00	.00
	8	-.46	.00	-1.11	.00	.00	.00
	9	.52	.00	1.57	.00	.00	.00
	10	-.48	.00	-.75	.00	.00	.00

JOINT	LOAD	FORCE-X	FORCE-Y	FORCE-Z	MOM-X	MOM-Y	MOM Z
153	7	-.02	.00	.96	.00	.00	.00
	8	-.03	.00	1.40	.00	.00	.00
	9	.09	.00	2.79	.00	.00	.00
	10	.49	.00	.61	.00	.00	.00
173	7	-.19	.00	-1.12	.00	.00	.00
	8	-.27	.00	-1.63	.00	.00	.00
	9	-.26	.00	.71	.00	.00	.00
	10	-1.33	.00	-1.13	.00	.00	.00
114	7	-.67	.00	.69	.00	.00	.00
	8	-.98	.00	1.00	.00	.00	.00
	9	-1.45	.00	2.04	.00	.00	.00
	10	-1.56	.00	.91	.00	.00	.00
134	7	-.38	.00	-.64	.00	.00	.00
	8	-.56	.00	-.93	.00	.00	.00
	9	.00	.00	.80	.00	.00	.00
	10	.00	.00	-.67	.00	.00	.00
154	7	-.24	.00	.78	.00	.00	.00
	8	-.34	.00	1.13	.00	.00	.00
	9	-.26	.00	1.89	.00	.00	.00
	10	.53	.00	.51	.00	.00	.00
174	7	-.33	.00	-.89	.00	.00	.00
	8	-.47	.00	-1.29	.00	.00	.00
	9	-.45	.00	.23	.00	.00	.00
	10	-.45	.00	-.87	.00	.00	.00
115	7	-.46	.00	.47	.00	.00	.00
	8	-.68	.00	.67	.00	.00	.00
	9	-.84	.00	1.14	.00	.00	.00
	10	-.40	.00	.59	.00	.00	.00
135	7	-.34	.00	-.47	.00	.00	.00
	8	-.50	.00	-.67	.00	.00	.00
	9	-.24	.00	.28	.00	.00	.00
	10	.23	.00	-.51	.00	.00	.00
155	7	-.29	.00	.55	.00	.00	.00
	8	-.42	.00	.80	.00	.00	.00
	9	-.37	.00	1.11	.00	.00	.00
	10	.44	.00	.38	.00	.00	.00
175	7	-.32	.00	-.61	.00	.00	.00
	8	-.47	.00	-.89	.00	.00	.00
	9	-.44	.00	-.04	.00	.00	.00
	10	.05	.00	-.59	.00	.00	.00
116	7	-.28	.00	.28	.00	.00	.00
	8	-.40	.00	.40	.00	.00	.00
	9	-.39	.00	.51	.00	.00	.00
	10	.21	.00	.33	.00	.00	.00
136	7	-.25	.00	-.30	.00	.00	.00
	8	-.37	.00	-.44	.00	.00	.00
	9	-.30	.00	-.02	.00	.00	.00
	10	.28	.00	-.35	.00	.00	.00
156	7	-.26	.00	.34	.00	.00	.00
	8	-.37	.00	.50	.00	.00	.00
	9	-.35	.00	.55	.00	.00	.00
	10	.32	.00	.25	.00	.00	.00
176	7	-.26	.00	-.38	.00	.00	.00
	8	-.38	.00	-.55	.00	.00	.00
	9	-.35	.00	-.16	.00	.00	.00
	10	.27	.00	-.35	.00	.00	.00
117	7	-.14	.00	.14	.00	.00	.00
	8	-.21	.00	.21	.00	.00	.00
	9	-.12	.00	.15	.00	.00	.00
	10	.43	.00	.16	.00	.00	.00
137	7	-.16	.00	-.17	.00	.00	.00
	8	-.24	.00	-.25	.00	.00	.00
	9	-.27	.00	-.14	.00	.00	.00
	10	.25	.00	-.21	.00	.00	.00
157	7	-.19	.00	.19	.00	.00	.00
	8	-.28	.00	.28	.00	.00	.00
	9	-.27	.00	.20	.00	.00	.00
	10	.20	.00	.14	.00	.00	.00

JOINT	LOAD	FORCE-X	FORCE-Y	FORCE-Z	MOM-X	MOM-Y	MOM Z
177	7	-.18	.00	-.20	.00	.00	.00
	8	-.26	.00	-.29	.00	.00	.00
	9	-.24	.00	-.19	.00	.00	.00
	10	.31	.00	-.18	.00	.00	.00
118	7	-.06	.00	.06	.00	.00	.00
	8	-.09	.00	.09	.00	.00	.00
	9	.00	.00	-.02	.00	.00	.00
	10	.39	.00	.06	.00	.00	.00
138	7	-.09	.00	-.09	.00	.00	.00
	8	-.14	.00	-.13	.00	.00	.00
	9	-.19	.00	-.15	.00	.00	.00
	10	.18	.00	-.11	.00	.00	.00
158	7	-.12	.00	.09	.00	.00	.00
	8	-.17	.00	.13	.00	.00	.00
	9	-.17	.00	.03	.00	.00	.00
	10	.11	.00	.07	.00	.00	.00
178	7	-.11	.00	-.09	.00	.00	.00
	8	-.16	.00	-.14	.00	.00	.00
	9	-.15	.00	-.15	.00	.00	.00
	10	.24	.00	-.08	.00	.00	.00
119	7	-.02	.00	.02	.00	.00	.00
	8	-.03	.00	.03	.00	.00	.00
	9	.03	.00	-.04	.00	.00	.00
	10	.23	.00	.02	.00	.00	.00
139	7	-.04	.00	-.03	.00	.00	.00
	8	-.06	.00	-.05	.00	.00	.00
	9	-.10	.00	-.09	.00	.00	.00
	10	.09	.00	-.05	.00	.00	.00
159	7	-.06	.00	.03	.00	.00	.00
	8	-.08	.00	.05	.00	.00	.00
	9	-.08	.00	-.02	.00	.00	.00
	10	.05	.00	.03	.00	.00	.00
179	7	-.05	.00	-.03	.00	.00	.00
	8	-.07	.00	-.05	.00	.00	.00
	9	-.07	.00	-.08	.00	.00	.00
	10	.13	.00	-.03	.00	.00	.00
120	7	.22	1946.08	-.29	.00	.00	.00
	8	.33	2807.49	-.42	.00	.00	.00
	9	.49	2600.75	-.92	.00	.00	.00
	10	.55	2386.88	-.38	.00	.00	.00
140	7	.12	1650.06	.25	.00	.00	.00
	8	.18	2374.07	.37	.00	.00	.00
	9	-.01	1054.03	-.41	.00	.00	.00
	10	.01	1814.39	.26	.00	.00	.00
160	7	.07	2068.92	-.31	.00	.00	.00
	8	.11	2984.17	-.45	.00	.00	.00
	9	.08	2616.73	-.83	.00	.00	.00
	10	-.17	1864.69	-.20	.00	.00	.00
180	7	.10	2016.65	.36	.00	.00	.00
	8	.15	2906.90	.52	.00	.00	.00
	9	.14	1481.09	-.16	.00	.00	.00
	10	.17	1686.65	.36	.00	.00	.00

MEMBER END FORCES STRUCTURE TYPE = SPACE

ALL UNITS ARE -- KNS METE

MEMBER	LOAD	JT	AXIAL	SHEAR-Y	SHEAR-Z	TORSION	MOM-Y	MOM-Z
1	7	1	2012.87	1073.91	-6.16	34.59	-169.23	1528.20
		2	-1978.23	-1055.26	6.16	-34.59	178.18	43.47
	8	1	2931.16	1563.57	-8.97	52.13	-249.70	2226.86
		2	-2882.66	-1537.46	8.97	-52.13	262.73	62.20
	9	1	1974.07	1035.70	196.04	1089.02	-2488.52	1432.56
		2	-1937.70	-1016.12	-196.04	-1089.02	2198.87	81.99
	10	1	2342.37	1218.67	-11.48	345.55	-458.97	1486.41
		2	-2306.00	-1199.08	11.48	-345.55	475.73	298.85
2	7	2	2242.06	8.75	6.16	-54.02	173.28	43.47
		3	-2162.12	-8.75	-6.16	54.02	-190.99	-16.51
	8	2	3267.00	12.98	8.97	-78.66	256.04	62.20
		3	-3155.08	-12.98	-8.97	78.66	-281.84	-22.25
	9	2	2187.83	24.01	-184.05	-83.63	2452.35	81.99
		3	-2103.89	-24.01	162.63	83.63	-1931.17	-9.23
	10	2	2598.86	37.53	11.48	78.70	582.69	298.85
		3	-2514.92	-37.53	-11.48	-78.70	-615.97	-186.96
3	7	3	2162.12	8.75	6.16	-54.02	190.99	16.51
		4	-2082.18	-8.75	-6.16	54.02	-208.15	10.51
	8	3	3155.08	12.98	8.97	-78.66	281.84	22.25
		4	-3043.17	-12.98	-8.97	78.66	-306.86	17.77
	9	3	2103.89	24.01	-162.62	-83.63	1931.16	9.23
		4	-2019.95	-24.01	141.20	83.63	-1473.32	63.59
	10	3	2514.91	37.53	11.48	78.70	615.97	186.96
		4	-2430.97	-37.53	-11.48	-78.70	-648.36	-74.78
4	7	4	1091.69	-54	-6.05	-5.71	184.25	-5.43
		5	-1036.17	54	6.05	5.71	-165.00	4.16
	8	4	1590.52	38	-8.93	-8.22	271.28	-5.23
		5	-1512.80	-38	8.93	8.22	-242.88	6.89
	9	4	1069.89	-17.27	-119.10	20.93	1155.24	-82.77
		5	-1011.60	17.27	100.74	-20.93	-823.76	31.34
	10	4	1208.86	-34.01	-7.01	-58.95	419.73	-79.43
		5	-1150.57	34.01	7.01	58.95	-396.99	-22.60
5	7	5	1036.18	-54	-6.05	-5.71	165.00	-4.16
		6	-980.66	54	6.05	5.71	-145.29	2.89
	8	5	1512.81	38	-8.93	-8.22	242.88	-6.89
		6	-1435.09	-38	8.93	8.22	-213.82	8.53
	9	5	1011.61	-17.27	-100.74	20.93	823.77	-31.34
		6	-953.32	17.27	82.38	-20.93	-546.71	-20.10
	10	5	1150.58	-34.01	-7.01	-58.95	396.99	22.61
		6	-1092.29	34.01	7.01	58.95	-373.59	-124.63
6	7	6	40.53	.00	.00	.00	.09	.03
		7	-5.00	.00	.00	.00	-.01	.00
	8	6	56.74	.00	.00	.00	.13	.04
		7	-7.00	.00	.00	.00	-.01	.00
	9	6	42.55	.00	-19.89	.00	43.70	.03
		7	-5.25	.00	4.59	.00	-6.90	.00
	10	6	42.55	.00	.00	.00	.10	.03
		7	-5.25	.00	.00	.00	-.01	.00
7	7	7	5.00	.00	.00	.00	.01	.00
		8	.00	.00	.00	.00	.00	.00
	8	7	7.00	.00	.00	.00	.01	.00
		8	.00	.00	.00	.00	.00	.00
	9	7	5.25	.00	-4.59	.00	6.90	.00
		8	.00	.00	.00	.00	.00	.00
	10	7	5.25	.00	.00	.00	.01	.00
		8	.00	.00	.00	.00	.00	.00

STAIRCASE

MEMBER	LOAD	JT	AXIAL	SHEAR-Y	SHEAR-Z	TORSION	MOM-Y	MOM-Z	
11	7	11	2728.05	1429.30	-8.60	54.74	-162.12	1964.81	
		12	-2693.41	-1410.65	8.60	-54.74	174.71	131.67	
	8	11	3971.18	2080.20	-12.56	80.88	-239.28	2859.95	
		12	-3922.68	-2054.09	12.56	-80.88	257.68	192.01	
	9	11	2704.92	1392.03	194.71	1157.57	-2594.55	1885.34	
		12	-2668.55	-1372.44	-194.71	-1157.57	2306.87	155.39	
	10	11	2138.29	1291.63	-27.90	-48.92	-675.36	1821.59	
		12	-2101.92	-1272.04	27.90	48.92	716.36	69.97	
	12	7	12	3040.27	34.91	8.60	-34.64	179.78	131.67
			13	-2960.33	-34.91	-8.60	34.64	-204.74	-26.16
8		12	4427.66	51.18	12.56	-50.95	265.23	192.01	
		13	-4315.74	-51.18	-12.56	50.95	-301.72	-37.35	
9		12	3000.26	56.76	-194.71	-74.48	2579.94	155.39	
		13	-2916.32	-56.76	173.29	74.48	-2026.45	15.77	
10		12	2453.78	-92.60	27.90	-382.70	607.54	69.97	
		13	-2369.84	71.18	-27.90	382.70	-689.44	-312.41	
13		7	13	2029.31	-9.88	-.92	20.68	257.91	-58.27
			14	-1949.36	9.88	.92	-20.68	-253.94	29.18
	8	13	2951.77	-14.73	-1.27	30.04	379.68	-86.82	
		14	-2839.85	14.73	1.27	-30.04	-374.11	43.41	
	9	13	2044.14	-39.99	-183.29	62.96	1994.53	-205.69	
		14	-1960.20	39.99	161.87	-62.96	-1474.54	86.32	
	10	13	1780.34	-75.76	-3.88	-4.17	566.86	-37.28	
		14	-1696.41	54.34	3.88	4.17	-552.94	-156.32	
	14	7	14	1949.36	-9.88	-.92	20.68	253.94	-29.18
			15	-1893.84	9.88	.92	-20.68	-248.94	-.04
8		14	2839.85	-14.73	-1.27	30.04	374.11	-43.41	
		15	-2762.13	14.73	1.27	-30.04	-367.05	-.17	
9		14	1960.20	-39.99	-161.87	62.96	1474.54	-86.32	
		15	-1901.91	39.99	143.51	-62.96	-1012.67	-33.19	
10		14	1696.40	7.37	-3.88	-4.17	552.94	156.33	
		15	-1638.11	-25.73	3.88	4.17	-537.46	-105.48	
15		7	15	912.39	-63.73	8.50	4.53	304.15	-76.43
			16	-856.87	63.73	-8.50	-4.53	-327.80	-114.59
	8	15	1324.61	-94.20	12.51	6.98	448.40	-113.29	
		16	-1246.89	94.20	-12.51	-6.98	-483.29	-169.07	
	9	15	933.34	-70.43	-84.63	17.03	945.20	-110.98	
		16	-875.05	70.43	66.27	-17.03	-716.13	-100.12	
	10	15	864.38	-105.14	5.22	8.08	415.97	-137.94	
		16	-806.09	86.78	-5.22	-8.08	-428.78	-149.56	
	16	7	16	856.87	-63.73	8.50	4.53	327.80	114.59
			17	-821.34	63.73	-8.50	-4.53	-350.11	-305.02
8		16	1246.89	-94.20	12.51	6.98	483.29	169.07	
		17	-1197.14	94.20	-12.51	-6.98	-516.25	-450.59	
9		16	875.05	-70.43	-66.28	17.03	716.13	100.12	
		17	-837.74	70.43	50.98	-17.03	-536.02	-310.67	
10		16	806.09	-25.07	5.22	8.08	428.78	149.56	
		17	-768.78	9.77	-5.22	-8.08	-440.05	-200.90	
17		7	17	69.39	.00	.00	.00	.67	.20
			18	-33.86	.00	.00	.00	-.27	-.08
	8	17	97.15	.00	.00	.00	.94	.27	
		18	-47.41	.00	.00	.00	-.38	-.11	
	9	17	72.86	.00	-36.72	.00	147.58	.21	
		18	-35.56	.00	21.42	.00	-59.96	-.08	
	10	17	72.86	-36.72	.00	.00	.70	-146.67	
		18	-35.56	21.42	.00	.00	-.29	59.59	
	18	7	18	33.87	.00	.00	.00	.27	.08
			19	-13.88	.00	.00	.00	-.08	-.02
8		18	47.41	.00	.00	.00	.38	.11	
		19	-19.43	.00	.00	.00	-.11	-.03	
9		18	35.56	.00	-21.42	.00	59.95	.08	

214. PLOT BEN FILE
 215. PLOT FORCE FILE
 216. PLOT DISP FILE
 217. START CON DESIGN
 218. FC 30000
 219. TRACK 1
 220. CLEAR 0.05
 221. DESIGN COLUMN 1 TO 7 11 TO 18

C O L U M N N O . 1 D E S I G N R E S U L T S

FY - 413.7 FC - 30.0 MPA, CIRC SIZE 1200.0 MMS DIAMETER TIED

AREA OF STEEL REQUIRED = 16851.4 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
21 - 32 MM (EQUALLY SPACED)	1.494	9	STA	.700

C O L U M N I N T E R A C T I O N : M O M E N T A B O U T Z / Y - A X I S (K N - M E T)

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
35397.58	28318.07	13656.43	5549.58	406.4
M0	P-tens.	Des.Pn	Des.Mn	e/h
3255.17	-6988.57	2820.11	4102.00	.985

C O L U M N N O . 2 D E S I G N R E S U L T S

FY - 413.7 FC - 30.0 MPA, CIRC SIZE 1200.0 MMS DIAMETER TIED

AREA OF STEEL REQUIRED = 12101.4 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
39 - 20 MM (EQUALLY SPACED)	1.083	9	STA	.700

C O L U M N I N T E R A C T I O N : M O M E N T A B O U T Z / Y - A X I S (K N - M E T)

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
33595.46	26876.37	13642.51	5048.20	370.0
M0	P-tens.	Des.Pn	Des.Mn	e/h
2459.53	-5068.06	3125.47	3505.31	.374

C O L U M N N O . 3 D E S I G N R E S U L T S

FY - 413.7 FC - 30.0 MPA, CIRC SIZE 1200.0 MMS DIAMETER TIED

AREA OF STEEL REQUIRED = 11309.7 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
9 - 40 MM (EQUALLY SPACED)	1.000	7	STA	.700

C O L U M N I N T E R A C T I O N : M O M E N T A B O U T Z / Y - A X I S (K N - M E T)

PO	Pn max	P-bal.	M-bal.	e-bal. (MM)
33230.09	26584.07	13443.64	4921.88	366.1
MO	P-tens.	Des.Pn	Des.Mn	e/h
2255.03	-4678.69	3088.75	273.86	.030

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C O L U M N N O. 4 D E S I G N R E S U L T S

FY - 413.7 FC - 30.0 MPA, CIRC SIZE 1000.0 MMS DIAMETER TIED

AREA OF STEEL REQUIRED = 7853.9 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
40 - 16 MM (EQUALLY SPACED)	1.024	7	STA	.700

COLUMN INTERACTION: MOMENT ABOUT Z/Y -AXIS (KN-MET)

PO	Pn max	P-bal.	M-bal.	e-bal. (MM)
23150.34	18520.27	9341.11	2858.84	306.0
MO	P-tens.	Des.Pn	Des.Mn	e/h
1340.07	-3327.84	1559.55	263.33	.056

=====

C O L U M N N O. 5 D E S I G N R E S U L T S

FY - 413.7 FC - 30.0 MPA, CIRC SIZE 1000.0 MMS DIAMETER TIED

AREA OF STEEL REQUIRED = 7853.9 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
40 - 16 MM (EQUALLY SPACED)	1.024	7	STA	.700

COLUMN INTERACTION: MOMENT ABOUT Z/Y -AXIS (KN-MET)

PO	Pn max	P-bal.	M-bal.	e-bal. (MM)
23150.34	18520.27	9341.11	2858.84	306.0
MO	P-tens.	Des.Pn	Des.Mn	e/h
1340.07	-3327.84	1480.25	235.79	.053

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C O L U M N N O. 6 D E S I G N R E S U L T S

FY - 413.7 FC - 30.0 MPA, CIRC SIZE 800.0 MMS DIAMETER TIED

AREA OF STEEL REQUIRED = 5026.5 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 40 MM (EQUALLY SPACED)	1.000	7	STA	.700

COLUMN INTERACTION: MOMENT ABOUT Z/Y -AXIS (KN-MET)

PO	Pn max	P-bal.	M-bal.	e-bal. (MM)
14768.93	11815.14	5664.81	1403.19	247.7
MO	P-tens.	Des.Pn	Des.Mn	e/h
684.64	-2079.42	57.89	.14	.001

=====

C O L U M N N O. 7 D E S I G N R E S U L T S

FY - 413.7 FC - 30.0 MPA, CIRC SIZE 300.0 MMS DIAMETER TIED

AREA OF STEEL REQUIRED = 706.9 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
7 - 12 MM (EQUALLY SPACED)	1.120	7	STA	.700

COLUMN INTERACTION: MOMENT ABOUT Z/Y -AXIS (KN-MET)

PO	Pn max	P-bal.	M-bal.	e-bal. (MM)
2109.75	1687.80	647.60	67.90	104.8
MO	P-tens.	Des.Pn	Des.Mn	e/h
34.26	-327.45	7.14	.02	.001

=====

C O L U M N N O. 11 D E S I G N R E S U L T S

FY - 413.7 FC - 30.0 MPA, CIRC SIZE 1200.0 MMS DIAMETER TIED

AREA OF STEEL REQUIRED = 18434.8 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
23 - 32 MM (EQUALLY SPACED)	1.636	9	STA	.700

COLUMN INTERACTION: MOMENT ABOUT Z/Y -AXIS (KN-MET)

PO	Pn max	P-bal.	M-bal.	e-bal. (MM)
36022.13	28817.71	13731.45	5733.12	417.5
MO	P-tens.	Des.Pn	Des.Mn	e/h
3528.24	-7654.15	3864.17	4581.71	.803

=====

C O L U M N N O. 12 D E S I G N R E S U L T S

FY - 413.7 FC - 30.0 MPA, CIRC SIZE 1200.0 MMS DIAMETER TIED

AREA OF STEEL REQUIRED = 11309.7 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
9 - 40 MM (EQUALLY SPACED)	1.000	7	STA	.700

COLUMN INTERACTION: MOMENT ABOUT Z/Y -AXIS (KN-MET)

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
33230.09	26584.07	13443.64	4921.88	366.1
M0	P-tens.	Des.Pn	Des.Mn	e/h
2255.03	-4678.69	4343.24	318.34	.024

C O L U M N N O. 13 D E S I G N R E S U L T S

FY - 413.7 FC - 30.0 MPA, CIRC SIZE 1200.0 MMS DIAMETER TIED

AREA OF STEEL REQUIRED = 11309.7 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
9 - 40 MM (EQUALLY SPACED)	1.000	7	STA	.700

COLUMN INTERACTION: MOMENT ABOUT Z/Y -AXIS (KN-MET)

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
33230.09	26584.07	13443.64	4921.88	366.1
M0	P-tens.	Des.Pn	Des.Mn	e/h
2255.03	-4678.69	2899.01	377.73	.043

C O L U M N N O. 14 D E S I G N R E S U L T S

FY - 413.7 FC - 30.0 MPA, CIRC SIZE 1000.0 MMS DIAMETER TIED

AREA OF STEEL REQUIRED = 7853.9 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
40 - 16 MM (EQUALLY SPACED)	1.024	7	STA	.700

COLUMN INTERACTION: MOMENT ABOUT Z/Y -AXIS (KN-MET)

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
23150.34	18520.27	9341.11	2858.84	306.0
M0	P-tens.	Des.Pn	Des.Mn	e/h
1340.07	-3327.84	2784.80	365.16	.044

C O L U M N N O. 15 D E S I G N R E S U L T S

FY - 413.7 FC - 30.0 MPA, CIRC SIZE 1000.0 MMS DIAMETER TIED

AREA OF STEEL REQUIRED = 7853.9 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
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40 - 16 MM 1.024 7 STA .700
(EQUALLY SPACED)

COLUMN INTERACTION: MOMENT ABOUT Z/Y -AXIS (KN-MET)

PO	Pn max	P-bal.	M-bal.	e-bal. (MM)
23150.34	18520.27	9341.11	2858.84	306.0
MO	P-tens.	Des.Pn	Des.Mn	e/h
1340.07	-3327.84	1303.41	448.01	.115

=====

C O L U M N N O . 1 6 D E S I G N R E S U L T S

FY - 413.7 FC - 30.0 MPA, CIRC SIZE 800.0 MMS DIAMETER TIED

AREA OF STEEL REQUIRED = 6082.1 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
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31 - 16 MM (EQUALLY SPACED)	1.240	9	STA	.700
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COLUMN INTERACTION: MOMENT ABOUT Z/Y -AXIS (KN-MET)

PO	Pn max	P-bal.	M-bal.	e-bal. (MM)
15237.78	12190.23	5863.69	1524.02	259.9
MO	P-tens.	Des.Pn	Des.Mn	e/h
800.13	-2579.07	1250.07	1032.99	.275

=====

C O L U M N N O . 1 7 D E S I G N R E S U L T S

FY - 413.7 FC - 30.0 MPA, CIRC SIZE 800.0 MMS DIAMETER TIED

AREA OF STEEL REQUIRED = 5026.5 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
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4 - 40 MM (EQUALLY SPACED)	1.000	7	STA	.700
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COLUMN INTERACTION: MOMENT ABOUT Z/Y -AXIS (KN-MET)

PO	Pn max	P-bal.	M-bal.	e-bal. (MM)
14768.93	11815.14	5664.81	1403.19	247.7
MO	P-tens.	Des.Pn	Des.Mn	e/h
684.64	-2079.42	99.13	1.00	.003

=====

C O L U M N N O . 1 8 D E S I G N R E S U L T S

FY - 413.7 FC - 30.0 MPA, CIRC SIZE 600.0 MMS DIAMETER TIED

AREA OF STEEL REQUIRED = 2827.4 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
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26 - 12 MM 1.040 7 STA .700
(EQUALLY SPACED)

COLUMN INTERACTION: MOMENT ABOUT Z/Y -AXIS (KN-MET)

PO	Pn max	P-bal.	M-bal.	e-bal. (MM)
8351.22	6680.97	3166.67	600.70	189.7
MO	P-tens.	Des.Pn	Des.Mn	e/h
282.17	-1216.24	48.38	.40	.003

*****END OF COLUMN DESIGN RESULTS*****

222. DESIGN BEAM 21 TO 29 31 TO 39 41 TO 49 51 TO 59

B E A M N O . 2 1 D E S I G N R E S U L T S - F L E X U R E

LEN - 1932. MM FY - 414. FC - 30. MPA, SIZE - 550. X 1200. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	END
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1	73.	7 - 20MM	937.	1932.	NO	YES
---	-----	----------	------	-------	----	-----

CRITICAL POS MOMENT= 143.95 KN-MET AT 1932.MM, LOAD 10
REQD STEEL= 2067.MM2, ROW= .0033, ROWMX= .0228 ROWMN= .0033
MAX/MIN/ACTUAL BAR SPACING= 428./ 45./ 71. MMS
BASIC/REQD. DEVELOPMENT LENGTH = 450./ 592. MMS

2	1127.	7 - 20MM	0.	1932.	YES	YES
---	-------	----------	----	-------	-----	-----

CRITICAL NEG MOMENT= 825.53 KN-MET AT 0.MM, LOAD 8
REQD STEEL= 2067.MM2, ROW= .0033, ROWMX= .0228 ROWMN= .0033
MAX/MIN/ACTUAL BAR SPACING= 428./ 45./ 71. MMS
BASIC/REQD. DEVELOPMENT LENGTH = 450./ 819. MMS

B E A M N O . 2 1 D E S I G N R E S U L T S - S H E A R

AT START SUPPORT - Vu= 432.29 KNS Vc= 147.89 KNS Vs= 360.69 KNS
Tu= 372.48 KN-MET Ts= 310.8 KN-MET
Tc= 127.4 KN-MET

PROVIDE 12 MM BARS AT 81. MM C/C FOR 1932. MM
AT END SUPPORT - Vu= 438.69 KNS Vc= 149.93 KNS Vs= 366.19 KNS
Tu= 372.48 KN-MET Ts= 310.9 KN-MET
Tc= 127.3 KN-MET

PROVIDE 12 MM BARS AT 81. MM C/C FOR 1932. MM

RUPSA BRIDGE (ANALYSIS OF STAIRCASE) -- PAGE NO. 24

4J	1931X 549X 1199	21J
7No20 H1127. 0.TO 1932		
25*12c/c 81		25*12c/c 81
	7No20 H 73. 937.TO 1932	

0000000	0000000	0000000	0000000	0000000	0000000	0000000
7#20	7#20	7#20	7#20	7#20	7#20	7#20
			7#20	7#20	7#20	7#20
			0000000	0000000	0000000	0000000

=====

BEAM NO. 22 DESIGN RESULTS - FLEXURE

LEN - 1900. MM FY - 414. FC - 30. MPA, SIZE - 550. X 950. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	879.	9 - 16MM	0.	1900.	YES	YES

CRITICAL NEG MOMENT= 58.21 KN-MET AT 0. MM, LOAD 8

REQD STEEL= 1612. MM2, ROW= .0033, ROWMX= .0228 ROWMN= .0033

MAX/MIN/ACTUAL BAR SPACING= 432./ 41./ 54. MMS

BASIC/REQD. DEVELOPMENT LENGTH = 288./ 568. MMS

BEAM NO. 22 DESIGN RESULTS - SHEAR

AT START SUPPORT - Vu= 38.03 KNS Vc= 437.54 KNS Vs= .00 KNS
STIRRUPS ARE NOT REQUIRED.

AT END SUPPORT - Vu= 24.95 KNS Vc= 437.54 KNS Vs= .00 KNS
STIRRUPS ARE NOT REQUIRED.

21J 1899X 549X 949 22J

9No16 H 879. 0. TO 1900

000000000	000000000	000000000	000000000	000000000	000000000
9#16	9#16	9#16	9#16	9#16	9#16

=====

BEAM NO. 23 DESIGN RESULTS - FLEXURE

LEN - 3987. MM FY - 414. FC - 30. MPA, SIZE - 250. X 1200. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
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1      1127.      3 - 20MM      0.      3987.      YES YES
-----
CRITICAL NEG MOMENT= 251.61 KN-MET AT 0.MM, LOAD 8
REQD STEEL= 939.MM2, ROW= .0033, ROWMX=.0228 ROWMN= .0033
MAX/MIN/ACTUAL BAR SPACING= 128./ 45./ 64. MMS
BASIC/REQD. DEVELOPMENT LENGTH = 450./ 819. MMS
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BEAM NO. 23 DESIGN RESULTS - SHEAR

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AT START SUPPORT - Vu= 92.69 KNS Vc= 255.73 KNS Vs= .00 KNS
                    STIRRUPS ARE NOT REQUIRED.
AT END SUPPORT - Vu= 27.50 KNS Vc= 255.73 KNS Vs= .00 KNS
                    STIRRUPS ARE NOT REQUIRED.
    
```

4J	3986X 249X 1199					23J
3No20 H1127. 0.TO 3987						
0	3#20	3#20	3#20	3#20	3#20	3#2

BEAM NO. 24 DESIGN RESULTS - FLEXURE

LEN - 4107. MM FY - 414. FC - 30. MPA, SIZE - 250. X 1200. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	END
-------	-------------	----------	-----------	---------	------------	-----

```

1      1127.      3 - 20MM      0.      4107.      YES YES
-----
CRITICAL NEG MOMENT= 265.74 KN-MET AT 0.MM, LOAD 8
REQD STEEL= 939.MM2, ROW= .0033, ROWMX=.0228 ROWMN= .0033
MAX/MIN/ACTUAL BAR SPACING= 128./ 45./ 64. MMS
BASIC/REQD. DEVELOPMENT LENGTH = 450./ 819. MMS
-----
    
```

BEAM NO. 24 DESIGN RESULTS - SHEAR

```

AT START SUPPORT - Vu= 95.38 KNS Vc= 255.73 KNS Vs= .00 KNS
                    STIRRUPS ARE NOT REQUIRED.
AT END SUPPORT - Vu= 27.50 KNS Vc= 255.73 KNS Vs= .00 KNS
                    STIRRUPS ARE NOT REQUIRED.
    
```

RUPSA BRIDGE (ANALYSIS OF STAIRCASE)

-- PAGE NO. 27

4J 4106X 249X 1199 24J

3No20 H1127. 0.TO 4107

	ooo	ooo	ooo	ooo	ooo	ooo
0	3#20	3#20	3#20	3#20	3#20	3#2

=====

BEAM NO. 25 DESIGN RESULTS - FLEXURE

LEN - 4150. MM FY - 414. FC - 30. MPA, SIZE - 250. X 1200. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA END	
1	1127.	3 - 20MM	0.	4150.	YES	YES

CRITICAL NEG MOMENT= 270.90 KN-MET AT 0.MM, LOAD 8
REQD STEEL= 939.MM2, ROW= .0033, ROWMX= .0228 ROWMN= .0033
MAX/MIN/ACTUAL BAR SPACING= 128./ 45./ 64. MMS
BASIC/REQD. DEVELOPMENT LENGTH = 450./ 819. MMS

BEAM NO. 25 DESIGN RESULTS - SHEAR

AT START SUPPORT - Vu= 96.34 KNS Vc= 255.73 KNS Vs= .00 KNS
STIRRUPS ARE NOT REQUIRED.
AT END SUPPORT - Vu= 36.66 KNS Vc= 255.73 KNS Vs= .00 KNS
STIRRUPS ARE NOT REQUIRED.

4J 4149X 249X 1199 25J

3No20 H1127. 0.TO 4150

	ooo	ooo	ooo	ooo	ooo	ooo
0	3#20	3#20	3#20	3#20	3#20	3#2

BEAM NO. 26 DESIGN RESULTS - FLEXURE

LEN - 4107. MM FY - 414. FC - 30. MPA, SIZE - 250. X 1200. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA END
-------	----------------	----------	--------------	------------	-------------------

1	1127.	3 - 20MM	0.	4107.	YES YES
---	-------	----------	----	-------	---------

```

-----
| CRITICAL NEG MOMENT= 265.74 KN-MET AT 0. MM, LOAD 8 |
| REQD STEEL= 939.MM2, ROW= .0033, ROWMX= .0228 ROWMN= .0033 |
| MAX/MIN/ACTUAL BAR SPACING= 128./ 45./ 64. MMS |
| BASIC/REQD. DEVELOPMENT LENGTH = 450./ 819. MMS |
-----
    
```

BEAM NO. 26 DESIGN RESULTS - SHEAR

AT START SUPPORT - Vu= 95.38 KNS Vc= 255.73 KNS Vs= .00 KNS
STIRRUPS ARE NOT REQUIRED.

AT END SUPPORT - Vu= 16.23 KNS Vc= 255.73 KNS Vs= .00 KNS
STIRRUPS ARE NOT REQUIRED.

4J 4106X 249X 1199 26J

```

=====
| 3No20 H1127. 0.TO 4107 |
=====
    
```

0	0	0	0	0	0
3#20	3#20	3#20	3#20	3#20	3#20

BEAM NO. 27 DESIGN RESULTS - FLEXURE

LEN - 3987. MM FY - 414. FC - 30. MPA, SIZE - 250. X 1200. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA END
-------	----------------	----------	--------------	------------	-------------------

1	1127.	3 - 20MM	0.	3987.	YES YES
---	-------	----------	----	-------	---------

```

-----
| CRITICAL NEG MOMENT= 251.62 KN-MET AT 0. MM, LOAD 8 |
| REQD STEEL= 939.MM2, ROW= .0033, ROWMX= .0228 ROWMN= .0033 |
| MAX/MIN/ACTUAL BAR SPACING= 128./ 45./ 64. MMS |
| BASIC/REQD. DEVELOPMENT LENGTH = 450./ 819. MMS |
-----
    
```

BEAM NO. 27 DESIGN RESULTS - SHEAR

AT START SUPPORT - Vu= 92.69 KNS Vc= 255.73 KNS Vs= .00 KNS
 STIRRUPS ARE NOT REQUIRED.
 AT END SUPPORT - Vu= 16.23 KNS Vc= 255.73 KNS Vs= .00 KNS
 STIRRUPS ARE NOT REQUIRED.

4J	3986X 249X 1199	27J
3No20 H1127. 0.TO 3987		
3#20	3#20	3#20

=====

B E A M N O . 2 8 D E S I G N R E S U L T S - F L E X U R E

LEN - 1932. MM FY - 414. FC - 30. MPA, SIZE - 550. X 1200. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	END
1	73.	7 - 20MM	937.	1932.	NO	YES
CRITICAL POS MOMENT= 172.45 KN-MET AT 1932.MM, LOAD 10 REQD STEEL= 2067.MM2, ROW= .0033, ROWMX= .0228 ROWMN= .0033 MAX/MIN/ACTUAL BAR SPACING= 428./ 45./ 71. MMS BASIC/REQD. DEVELOPMENT LENGTH = 450./ 592. MMS						
2	1127.	7 - 20MM	0.	1932.	YES	YES
CRITICAL NEG MOMENT= 794.27 KN-MET AT 0.MM, LOAD 8 REQD STEEL= 2067.MM2, ROW= .0033, ROWMX= .0228 ROWMN= .0033 MAX/MIN/ACTUAL BAR SPACING= 428./ 45./ 71. MMS BASIC/REQD. DEVELOPMENT LENGTH = 450./ 819. MMS						

B E A M N O . 2 8 D E S I G N R E S U L T S - S H E A R

AT START SUPPORT - Vu= 419.88 KNS Vc= 136.98 KNS Vs= 357.00 KNS
 Tu= 392.67 KN-MET Ts= 333.9 KN-MET
 Tc= 128.1 KN-MET
 PROVIDE 12 MM BARS AT 77. MM C/C FOR 1932. MM

AT END SUPPORT - Vu= 428.42 KNS Vc= 139.59 KNS Vs= 364.43 KNS
 Tu= 392.67 KN-MET Ts= 334.0 KN-MET
 Tc= 127.9 KN-MET
 PROVIDE 12 MM BARS AT 77. MM C/C FOR 1932. MM

4J	1931X 549X 1199	28J
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						9#16	9#16
						000000000	000000000

=====

BEAM NO. 31 DESIGN RESULTS - FLEXURE

LEN - 1924. MM FY - 414. FC - 30. MPA, SIZE - 550. X 1200. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA END	
1	1125.	5 - 25MM	0.	1924.	YES	YES

```

-----
CRITICAL NEG MOMENT= 979.94 KN-MET AT 0.MM, LOAD 8
REQD STEEL= 2417.MM2, ROW= .0039, ROWMX= .0228 ROWMN= .0033
MAX/MIN/ACTUAL BAR SPACING= 423./ 50./ 106. MMS
BASIC/REQD. DEVELOPMENT LENGTH = 703./ 914. MMS
-----
    
```

BEAM NO. 31 DESIGN RESULTS - SHEAR

AT START SUPPORT - Vu= 384.69 KNS Vc= 108.17 KNS Vs= 344.40 KNS
 Tu= 460.92 KN-MET Ts= 412.6 KN-MET
 Tc= 129.6 KN-MET
 PROVIDE 12 MM BARS AT 66. MM C/C FOR 1924. MM

AT END SUPPORT - Vu= 392.41 KNS Vc= 110.26 KNS Vs= 351.40 KNS
 Tu= 460.92 KN-MET Ts= 412.7 KN-MET
 Tc= 129.5 KN-MET
 PROVIDE 12 MM BARS AT 66. MM C/C FOR 1924. MM

6J	1923X 549X 1199	31J
=====		
5No25 H1125.	0.TO 1924	
31*12c/c 66		31*12c/c 66

00000	00000	00000	00000	00000	00000	00000
5#25	5#25	5#25	5#25	5#25	5#25	5#25

=====

BEAM NO. 32 DESIGN RESULTS - FLEXURE

LEN - 1900. MM FY - 414. FC - 30. MPA, SIZE - 550. X 950. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA END	
-------	----------------	----------	--------------	------------	-------------------	--

```

1      879.      9 - 16MM      0.      1900.      YES YES
-----
| CRITICAL NEG MOMENT= 58.21 KN-MET AT 0.MM, LOAD 8 |
| REQD STEEL= 1612.MM2, ROW= .0033, ROWMX= .0228 ROWMN= .0033 |
| MAX/MIN/ACTUAL BAR SPACING= 432./ 41./ 54. MMS |
| BASIC/REQD. DEVELOPMENT LENGTH = 288./ 568. MMS |
-----

```

B E A M N O. 32 D E S I G N R E S U L T S - S H E A R

```

AT START SUPPORT - Vu= 38.04 KNS Vc= 437.54 KNS Vs= .00 KNS
                   STIRRUPS ARE NOT REQUIRED.
AT END SUPPORT - Vu= 24.95 KNS Vc= 437.54 KNS Vs= .00 KNS
                   STIRRUPS ARE NOT REQUIRED.

```

31J	1899X	549X	949	32J
9No16 H 879. 0.TO 1900				
9#16	9#16	9#16	9#16	9#16

B E A M N O. 33 D E S I G N R E S U L T S - F L E X U R E

LEN - 3959. MM FY - 414. FC - 30. MPA, SIZE - 250. X 1200. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	END
-------	-------------	----------	-----------	---------	------------	-----

```

1      1127.      3 - 20MM      0.      3959.      YES YES
-----
| CRITICAL NEG MOMENT= 248.38 KN-MET AT 0.MM, LOAD 8 |
| REQD STEEL= 939.MM2, ROW= .0033, ROWMX= .0228 ROWMN= .0033 |
| MAX/MIN/ACTUAL BAR SPACING= 128./ 45./ 54. MMS |
| BASIC/REQD. DEVELOPMENT LENGTH = 450./ 819. MMS |
-----

```

B E A M N O. 33 D E S I G N R E S U L T S - S H E A R

```

AT START SUPPORT - Vu= 92.07 KNS Vc= 255.73 KNS Vs= .00 KNS
                   STIRRUPS ARE NOT REQUIRED.
AT END SUPPORT - Vu= 36.66 KNS Vc= 255.73 KNS Vs= .00 KNS
                   STIRRUPS ARE NOT REQUIRED.

```

6J	3958X	249X	1199	33J
3No20 H1127. 0.TO 3959				

0	3#20	3#20	3#20	3#20	3#20	3#20

=====

BEAM NO. 34 DESIGN RESULTS - FLEXURE

LEN - 4063. MM FY - 414. FC - 30. MPA, SIZE - 250. X 1200. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	END
-------	-------------	----------	-----------	---------	------------	-----

1	1127.	3 - 20MM	0.	4063.	YES	YES
---	-------	----------	----	-------	-----	-----

CRITICAL NEG MOMENT=	260.50 KN-MET	AT	0. MM,	LOAD	8
REQD STEEL=	939. MM2,	ROW=	.0033,	ROWMX=	.0228
MAX/MIN/ACTUAL BAR SPACING=	128./	45./	64.	MMS	
BASIC/REQD. DEVELOPMENT LENGTH =	450./	819.	MMS		

BEAM NO. 34 DESIGN RESULTS - SHEAR

AT START SUPPORT - Vu= 94.39 KNS Vc= 255.73 KNS Vs= .00 KNS
STIRRUPS ARE NOT REQUIRED.

AT END SUPPORT - Vu= 27.50 KNS Vc= 255.73 KNS Vs= .00 KNS
STIRRUPS ARE NOT REQUIRED.

6J 4062X 249X 1199 34J

3No20	H1127.	0. TO	4063
-------	--------	-------	------

0	3#20	3#20	3#20	3#20	3#20	3#20

=====

BEAM NO. 35 DESIGN RESULTS - FLEXURE

LEN - 4100. MM FY - 414. FC - 30. MPA, SIZE - 250. X 1200. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
-------	-------------	----------	-----------	---------	------------	------------

1	1127.	3 - 20MM	0.	4100.	YES	YES
---	-------	----------	----	-------	-----	-----

```

-----
| CRITICAL NEG MOMENT= 264.92 KN-MET AT 0.MM, LOAD 8 |
| REQD STEEL= 939.MM2, ROW= .0033, ROWMX= .0228 ROWMN= .0033 |
| MAX/MIN/ACTUAL BAR SPACING= 128./ 45./ 64. MMS |
| BASIC/REQD. DEVELOPMENT LENGTH = 450./ 819. MMS |
-----

```

BEAM NO. 35 DESIGN RESULTS - SHEAR

AT START SUPPORT - Vu= 95.22 KNS Vc= 255.73 KNS Vs= .00 KNS
STIRRUPS ARE NOT REQUIRED.

AT END SUPPORT - Vu= 27.50 KNS Vc= 255.73 KNS Vs= .00 KNS
STIRRUPS ARE NOT REQUIRED.

6J	4099X 249X 1199	35J
3No20 H1127. 0.TO 4100		

000	000	000	000	000	000
3#20	3#20	3#20	3#20	3#20	3#2

BEAM NO. 36 DESIGN RESULTS - FLEXURE

LEN - 4063. MM FY - 414. FC - 30. MPA, SIZE - 250. X 1200. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
-------	-------------	----------	-----------	---------	------------	------------

1	1127.	3 - 20MM	0.	4063.	YES	YES
---	-------	----------	----	-------	-----	-----

```

-----
| CRITICAL NEG MOMENT= 260.50 KN-MET AT 0.MM, LOAD 8 |
| REQD STEEL= 939.MM2, ROW= .0033, ROWMX= .0228 ROWMN= .0033 |
| MAX/MIN/ACTUAL BAR SPACING= 128./ 45./ 64. MMS |
| BASIC/REQD. DEVELOPMENT LENGTH = 450./ 819. MMS |
-----

```

BEAM NO. 36 DESIGN RESULTS - SHEAR

AT START SUPPORT - Vu= 94.39 KNS Vc= 255.73 KNS Vs= .00 KNS
 STIRRUPS ARE NOT REQUIRED.
 AT END SUPPORT - Vu= 16.23 KNS Vc= 255.73 KNS Vs= .00 KNS
 STIRRUPS ARE NOT REQUIRED.

6J 4062X 249X 1199 36J
 =====
 3No20 H1127. 0.TO 4063
 =====

0	3#20	3#20	3#20	3#20	3#20	3#2

=====

BEAM NO. 37 DESIGN RESULTS - FLEXURE

LEN - 3959. MM FY - 414. FC - 30. MPA, SIZE - 250. X 1200. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	END
1	1127.	3 - 20MM	0.	3959.	YES	YES

 CRITICAL NEG MOMENT= 248.38 KN-MET AT 0.MM, LOAD 8
 REQD STEEL= 939.MM2, ROW= .0033, ROWMX= .0228 ROWMN= .0033
 MAX/MIN/ACTUAL BAR SPACING= 128./ 45./ 64. MMS
 BASIC/REQD. DEVELOPMENT LENGTH = 450./ 819. MMS

BEAM NO. 37 DESIGN RESULTS - SHEAR

AT START SUPPORT - Vu= 92.07 KNS Vc= 255.73 KNS Vs= .00 KNS
 STIRRUPS ARE NOT REQUIRED.
 AT END SUPPORT - Vu= 16.23 KNS Vc= 255.73 KNS Vs= .00 KNS
 STIRRUPS ARE NOT REQUIRED.

6J 3958X 249X 1199 37J
 =====
 3No20 H1127. 0.TO 3959
 =====

0	3#20	3#20	3#20	3#20	3#20	3#2



=====

BEAM NO. 38 DESIGN RESULTS - FLEXURE

LEN - 1924. MM FY - 414. FC - 30. MPA, SIZE - 550. X 1200. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	END
-------	-------------	----------	-----------	---------	------------	-----

1	73.	7 - 20MM	1091.	1924.	NO	YES
---	-----	----------	-------	-------	----	-----

CRITICAL POS MOMENT= 53.84 KN-MET AT 1924.MM, LOAD 10						
REQD STEEL= 2067.MM2, ROW= .0033, ROWMX= .0228 ROWMN= .0033						
MAX/MIN/ACTUAL BAR SPACING= 428./ 45./ 71. MMS						
BASIC/REQD. DEVELOPMENT LENGTH = 450./ 592. MMS						

2	1127.	7 - 20MM	0.	1924.	YES	YES
---	-------	----------	----	-------	-----	-----

CRITICAL NEG MOMENT= 772.84 KN-MET AT 0.MM, LOAD 8						
REQD STEEL= 2067.MM2, ROW= .0033, ROWMX= .0228 ROWMN= .0033						
MAX/MIN/ACTUAL BAR SPACING= 428./ 45./ 71. MMS						
BASIC/REQD. DEVELOPMENT LENGTH = 450./ 819. MMS						

BEAM NO. 38 DESIGN RESULTS - SHEAR

AT START SUPPORT - Vu= 386.95 KNS Vc= 123.87 KNS Vs= 331.36 KNS
 Tu= 402.45 KN-MET Ts= 344.6 KN-MET
 Tc= 128.8 KN-MET
 PROVIDE 12 MM BARS AT 77. MM C/C FOR 1924. MM

AT END SUPPORT - Vu= 394.67 KNS Vc= 126.22 KNS Vs= 338.10 KNS
 Tu= 402.45 KN-MET Ts= 344.8 KN-MET
 Tc= 128.7 KN-MET
 PROVIDE 12 MM BARS AT 76. MM C/C FOR 1924. MM

6J	1923X 549X 1199	38J
7No20 H1127.	0.TO 1924	
27*12c/c 77		27*12c/c 76
		7No20 H 73.1091.TO 1924

ooooooo	ooooooo	ooooooo	ooooooo	ooooooo	ooooooo
7#20	7#20	7#20	7#20	7#20	7#20
			7#20	7#20	7#20
			ooooooo	ooooooo	ooooooo

=====

BEAM NO. 39 DESIGN RESULTS - FLEXURE

LEN - 1900. MM FY - 414. FC - 30. MPA, SIZE - 550. X 950. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA END	
1	879.	9 - 16MM	0.	1900.	YES	YES
CRITICAL NEG MOMENT= 58.21 KN-MET AT 0.MM, LOAD 8 REQD STEEL= 1612.MM2, ROW= .0033, ROWMX= .0228 ROWMN= .0033 MAX/MIN/ACTUAL BAR SPACING= 432./ 41./ 54. MMS BASIC/REQD. DEVELOPMENT LENGTH = 288./ 568. MMS						

BEAM NO. 39 DESIGN RESULTS - SHEAR

AT START SUPPORT - Vu= 38.04 KNS Vc= 437.54 KNS Vs= .00 KNS
 STIRRUPS ARE NOT REQUIRED.
 AT END SUPPORT - Vu= 20.76 KNS Vc= 437.54 KNS Vs= .00 KNS
 STIRRUPS ARE NOT REQUIRED.

38J	1899X 549X 949	39J
9No16 H 879. 0.TO 1900		
9#16	9#16	9#16

BEAM NO. 41 DESIGN RESULTS - FLEXURE

LEN - 1953. MM FY - 414. FC - 30. MPA, SIZE - 550. X 1200. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA END	
1	73.	7 - 20MM	954.	1953.	NO	YES
CRITICAL POS MOMENT= 82.23 KN-MET AT 1953.MM, LOAD 9 REQD STEEL= 2067.MM2, ROW= .0033, ROWMX= .0228 ROWMN= .0033 MAX/MIN/ACTUAL BAR SPACING= 428./ 45./ 71. MMS BASIC/REQD. DEVELOPMENT LENGTH = 450./ 592. MMS						
2	1127.	7 - 20MM	0.	1953.	YES	YES
CRITICAL NEG MOMENT= 891.83 KN-MET AT 0.MM, LOAD 8 REQD STEEL= 2187.MM2, ROW= .0035, ROWMX= .0228 ROWMN= .0033 MAX/MIN/ACTUAL BAR SPACING= 428./ 45./ 71. MMS						

BASIC/REQD. DEVELOPMENT LENGTH = 450./ 819. MMS

BEAM NO. 41 DESIGN RESULTS - SHEAR

AT START SUPPORT - Vu= 391.69 KNS Vc= 140.33 KNS Vs= 320.48 KNS
 Tu= 356.99 KN-MET Ts= 292.1 KN-MET
 Tc= 127.9 KN-MET
 PROVIDE 12 MM BARS AT 88. MM C/C FOR 1953. MM

AT END SUPPORT - Vu= 399.71 KNS Vc= 143.02 KNS Vs= 327.23 KNS
 Tu= 356.99 KN-MET Ts= 292.3 KN-MET
 Tc= 127.7 KN-MET
 PROVIDE 12 MM BARS AT 87. MM C/C FOR 1953. MM

13J	1952X 549X 1199	41J
7No20 H1127. 0.TO 1953		
24*12c/c 88		24*12c/c 87
	7No20 H 173.1954.TO 1953	

7#20	7#20	7#20	7#20	7#20	7#20
			7#20	7#20	7#20

BEAM NO. 42 DESIGN RESULTS - FLEXURE

LEN - 1900. MM FY - 414. FC - 30. MPA, SIZE - 550. X 950. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
-------	-------------	----------	-----------	---------	------------	------------

1	879.	9 - 16MM	0.	1900.	YES	YES
---	------	----------	----	-------	-----	-----

CRITICAL NEG MOMENT= 58.21 KN-MET AT 0.MM, LOAD 8
 REQD STEEL= 1612.MM2, ROW= .0033, ROWMX= .0228 ROWMN= .0033
 MAX/MIN/ACTUAL BAR SPACING= 432./ 41./ 54. MMS
 BASIC/REQD. DEVELOPMENT LENGTH = 288./ 568. MMS

BEAM NO. 42 DESIGN RESULTS - SHEAR

AT START SUPPORT - Vu= 38.04 KNS Vc= 437.54 KNS Vs= .00 KNS
 STIRRUPS ARE NOT REQUIRED.
 AT END SUPPORT - Vu= 24.96 KNS Vc= 437.54 KNS Vs= .00 KNS
 STIRRUPS ARE NOT REQUIRED.

41J	1899X 549X 949	42J
9No16 H 879. 0.TO 1900		

00000000	00000000	00000000	00000000	00000000	00000000
9#16	9#16	9#16	9#16	9#16	9#16

=====

BEAM NO. 43 DESIGN RESULTS - FLEXURE

LEN - 3596. MM FY - 414. FC - 30. MPA, SIZE - 250. X 1200. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA END
1	1127.	3 - 20MM	0.	3596.	YES YES

CRITICAL NEG MOMENT= 208.41 KN-MET AT 0.MM, LOAD 8

REQD STEEL= 939.MM2, ROW= .0033, ROWMX= .0228 ROWMN= .0033

MAX/MIN/ACTUAL BAR SPACING= 128./ 45./ 64. MMS

BASIC/REQD. DEVELOPMENT LENGTH = 450./ 819. MMS

BEAM NO. 43 DESIGN RESULTS - SHEAR

AT START SUPPORT - Vu= 83.98 KNS Vc= 255.73 KNS Vs= .00 KNS
STIRRUPS ARE NOT REQUIRED.

AT END SUPPORT - Vu= 27.50 KNS Vc= 255.73 KNS Vs= .00 KNS
STIRRUPS ARE NOT REQUIRED.

13J	3596X 249X 1199	43J

3No20 H1127. 0.TO 3596		

000	000	000	000	000	000
3#20	3#20	3#20	3#20	3#20	3#2

=====

BEAM NO. 44 DESIGN RESULTS - FLEXURE

LEN - 3418. MM FY - 414. FC - 30. MPA, SIZE - 250. X 1200. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA END
-------	----------------	----------	--------------	------------	-------------------

1	1127.	3 - 20MM	0.	3418.	YES YES
---	-------	----------	----	-------	---------

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-----
| CRITICAL NEG MOMENT= 189.99 KN-MET AT 0.MM, LOAD 8 |
| REQD STEEL= 939.MM2, ROW= .0033, ROWMX= .0228 ROWMN= .0033 |
| MAX/MIN/ACTUAL BAR SPACING= 128./ 45./ 64. MMS |
| BASIC/REQD. DEVELOPMENT LENGTH = 450./ 819. MMS |
-----
    
```

B E A M N O . 4 4 D E S I G N R E S U L T S - S H E A R

AT START SUPPORT - Vu= 80.00 KNS Vc= 255.73 KNS Vs= .00 KNS
 STIRRUPS ARE NOT REQUIRED.
 AT END SUPPORT - Vu= 36.66 KNS Vc= 255.73 KNS Vs= .00 KNS
 STIRRUPS ARE NOT REQUIRED.

13J 3417X 249X 1199 44J

```

-----
| 3No20 H1127. 0.TO 3418 |
-----
    
```

0	0	0	0	0	0
3#20	3#20	3#20	3#20	3#20	3#2

B E A M N O . 4 5 D E S I G N R E S U L T S - F L E X U R E

LEN - 3350. MM FY - 414. FC - 30. MPA, SIZE - 250. X 1200. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA END
-------	----------------	----------	--------------	------------	-------------------

1	1127.	3 - 20MM	0.	3350.	YES YES
---	-------	----------	----	-------	---------

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-----
| CRITICAL NEG MOMENT= 183.22 KN-MET AT 0.MM, LOAD 8 |
| REQD STEEL= 939.MM2, ROW= .0033, ROWMX= .0228 ROWMN= .0033 |
| MAX/MIN/ACTUAL BAR SPACING= 128./ 45./ 64. MMS |
| BASIC/REQD. DEVELOPMENT LENGTH = 450./ 819. MMS |
-----
    
```

B E A M N O . 4 5 D E S I G N R E S U L T S - S H E A R

AT START SUPPORT - Vu= 79.66 KNS Vc= 255.73 KNS Vs= .00 KNS

STIRRUPS ARE NOT REQUIRED.
 AT END SUPPORT - Vu= 31.56 KNS Vc= 255.73 KNS Vs= .00 KNS
 STIRRUPS ARE NOT REQUIRED.

13J 3349X 249X 1199 45J
 =====
 3No20 H1127. 0.TO 3350
 =====

0	000 3#20	000 3#20	000 3#20	000 3#20	000 3#20	000 3#2

=====

B E A M N O . 4 6 D E S I G N R E S U L T S - F L E X U R E

LEN - 3418. MM FY - 414. FC - 30. MPA, SIZE - 250. X 1200. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	END
-------	-------------	----------	-----------	---------	------------	-----

1	1127.	3 - 20MM	0.	3418.	YES	YES
---	-------	----------	----	-------	-----	-----

CRITICAL NEG MOMENT=	189.99 KN-MET	AT	0. MM,	LOAD	8
REQD STEEL=	939.MM2,	ROW=	.0033,	ROWMX=	.0228
MAX/MIN/ACTUAL BAR SPACING=	128./	45./	64.	MMS	
BASIC/REQD. DEVELOPMENT LENGTH =	450./	819.	MMS		

B E A M N O . 4 6 D E S I G N R E S U L T S - S H E A R

AT START SUPPORT - Vu= 80.00 KNS Vc= 255.73 KNS Vs= .00 KNS

STIRRUPS ARE NOT REQUIRED.

AT END SUPPORT - Vu= 16.23 KNS Vc= 255.73 KNS Vs= .00 KNS

STIRRUPS ARE NOT REQUIRED.

13J 3417X 249X 1199 46J
 =====
 3No20 H1127. 0.TO 3418
 =====

0	000 3#20	000 3#20	000 3#20	000 3#20	000 3#20	000 3#2



=====

BEAM NO. 47 DESIGN RESULTS - FLEXURE

LEN - 3596. MM FY - 414. FC - 30. MPA, SIZE - 250. X 1200. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	END
-------	-------------	----------	-----------	---------	------------	-----

1	1127.	3 - 20MM	0.	3596.	YES	YES
---	-------	----------	----	-------	-----	-----

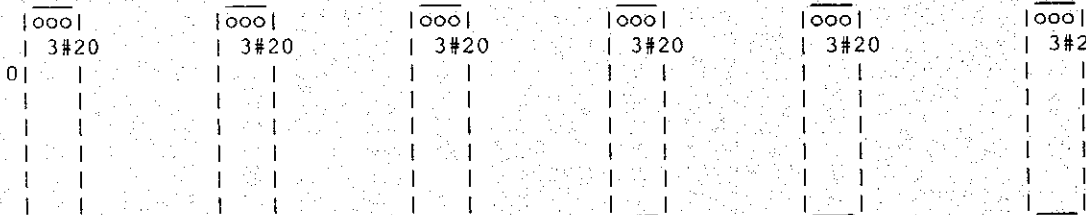
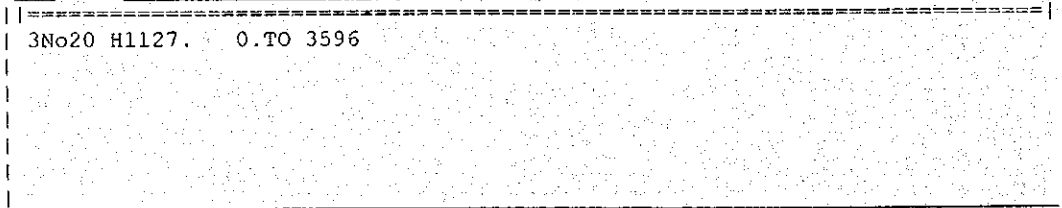
CRITICAL NEG MOMENT= 208.41 KN-MET AT 0.MM, LOAD 8						
REQD STEEL= 939.MM2, ROW= .0033, ROWMX= .0228 ROWMN= .0033						
MAX/MIN/ACTUAL BAR SPACING= 128./ 45./ 64. MMS						
BASIC/REQD. DEVELOPMENT LENGTH = 450./ 819. MMS						

BEAM NO. 47 DESIGN RESULTS - SHEAR

AT START SUPPORT - Vu= 83.98 KNS Vc= 255.73 KNS Vs= .00 KNS
STIRRUPS ARE NOT REQUIRED.

AT END SUPPORT - Vu= 16.23 KNS Vc= 255.73 KNS Vs= .00 KNS
STIRRUPS ARE NOT REQUIRED.

13J 3596X 249X 1199 47J



=====

BEAM NO. 48 DESIGN RESULTS - FLEXURE

LEN - 1953. MM FY - 414. FC - 30. MPA, SIZE - 550. X 1200. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	END
-------	-------------	----------	-----------	---------	------------	-----

1	73.	7 - 20MM	1279.	1953.	NO	YES
---	-----	----------	-------	-------	----	-----

CRITICAL POS MOMENT= 4.99 KN-MET AT 1953.MM, LOAD 9						
REQD STEEL= 2067.MM2, ROW= .0033, ROWMX= .0228 ROWMN= .0033						
MAX/MIN/ACTUAL BAR SPACING= 428./ 45./ 71. MMS						

```

| BASIC/REQD. DEVELOPMENT LENGTH = 450./ 592. MMS
|-----|
| 2      1125.      5 - 25MM      0.      1953.      YES YES
|-----|
| CRITICAL NEG MOMENT= 979.42 KN-MET AT 0.MM, LOAD 8
| REQD STEEL= 2415.MM2, ROW= .0039, ROWMX= .0228 ROWMN= .0033
| MAX/MIN/ACTUAL BAR SPACING= 423./ 50./ 106. MMS
| BASIC/REQD. DEVELOPMENT LENGTH = 703./ 914. MMS
|-----|
    
```

B E A M N O . 4 8 D E S I G N R E S U L T S - S H E A R

AT START SUPPORT - Vu= 426.61 KNS Vc= 140.41 KNS Vs= 361.48 KNS
 Tu= 388.57 KN-MET Ts= 329.2 KN-MET
 Tc= 127.9 KN-MET
 PROVIDE 12 MM BARS AT 78. MM C/C FOR 1953. MM

AT END SUPPORT - Vu= 434.63 KNS Vc= 142.89 KNS Vs= 368.45 KNS
 Tu= 388.57 KN-MET Ts= 329.4 KN-MET
 Tc= 127.7 KN-MET
 PROVIDE 12 MM BARS AT 77. MM C/C FOR 1953. MM

13J	1952X 549X 1199	48J
5No25 H1125.	0.TO 1953	
27*12c/c 78		27*12c/c 77
		7No20 H 73.1279.TO 1953

ooooo 5#25	ooooo 5#25	ooooo 5#25	ooooo 5#25	ooooo 5#25	ooooo 5#25
				7#20	7#20
				oooooooo	oooooooo

B E A M N O . 4 9 D E S I G N R E S U L T S - F L E X U R E

LEN - 1900. MM FY - 414. FC - 30. MPA, SIZE - 550. X 950. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	END
-------	-------------	----------	-----------	---------	------------	-----

```

| 1      71.      9 - 16MM      1432.      1900.      NO YES
|-----|
| CRITICAL POS MOMENT= .00 KN-MET AT 1900.MM, LOAD 9
| REQD STEEL= 1612.MM2, ROW= .0033, ROWMX= .0228 ROWMN= .0033
| MAX/MIN/ACTUAL BAR SPACING= 432./ 41./ 54. MMS
| BASIC/REQD. DEVELOPMENT LENGTH = 288./ 389. MMS
|-----|
    
```

```

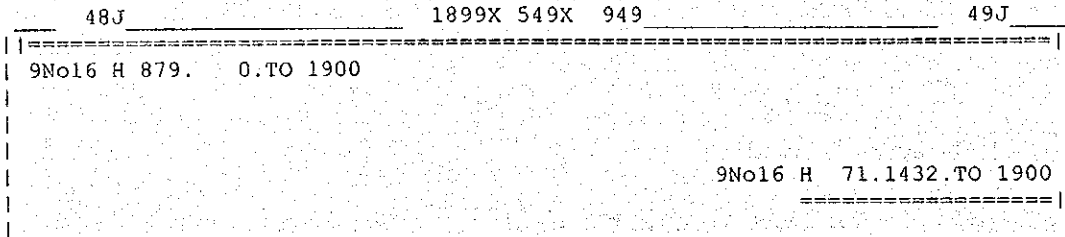
| 2      879.      9 - 16MM      0.      1900.      YES YES
|-----|
| CRITICAL NEG MOMENT= 58.21 KN-MET AT 0.MM, LOAD 8
| REQD STEEL= 1612.MM2, ROW= .0033, ROWMX= .0228 ROWMN= .0033
| MAX/MIN/ACTUAL BAR SPACING= 432./ 41./ 54. MMS
|-----|
    
```

BASIC/REQD. DEVELOPMENT LENGTH = 288./ 568. MMS

BEAM NO. 49 DESIGN RESULTS - SHEAR

AT START SUPPORT - Vu= 38.04 KNS Vc= 437.54 KNS Vs= .00 KNS
STIRRUPS ARE NOT REQUIRED.

AT END SUPPORT - Vu= 20.76 KNS Vc= 437.54 KNS Vs= .00 KNS
STIRRUPS ARE NOT REQUIRED.



9#16	9#16	9#16	9#16	9#16	9#16
				9#16	9#16
				9#16	9#16

BEAM NO. 51 DESIGN RESULTS - FLEXURE

LEN - 1932. MM FY - 414. FC - 30. MPA, SIZE - 550. X 1200. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	END
-------	-------------	----------	-----------	---------	------------	-----

1	73.	7 - 20MM	1098.	1932.	NO	YES
---	-----	----------	-------	-------	----	-----

CRITICAL POS MOMENT= 88.04 KN-MET AT 1932.MM, LOAD 9
REQD STEEL= 2067.MM2, ROW= .0033, ROWMX= .0228 ROWMN= .0033
MAX/MIN/ACTUAL BAR SPACING= 428./ 45./ 71. MMS
BASIC/REQD. DEVELOPMENT LENGTH = 450./ 592. MMS

2	1121.	3 - 32MM	0.	1932.	YES	YES
---	-------	----------	----	-------	-----	-----

CRITICAL NEG MOMENT= 903.01 KN-MET AT 0.MM, LOAD 8
REQD STEEL= 2228.MM2, ROW= .0036, ROWMX= .0228 ROWMN= .0033
MAX/MIN/ACTUAL BAR SPACING= 416./ 64./ 208. MMS
BASIC/REQD. DEVELOPMENT LENGTH = 1152./ 1498. MMS

BEAM NO. 51 DESIGN RESULTS - SHEAR

AT START SUPPORT - Vu= 424.91 KNS Vc= 144.10 KNS Vs= 355.78 KNS
Tu= 376.44 KN-MET Ts= 315.2 KN-MET
Tc= 127.7 KN-MET
PROVIDE 12 MM BARS AT 81. MM C/C FOR 1932. MM

AT END SUPPORT - Vu= 433.45 KNS Vc= 146.80 KNS Vs= 363.13 KNS
 Tu= 376.44 KN-MET Ts= 315.4 KN-MET
 Tc= 127.5 KN-MET
 PROVIDE 12 MM BARS AT 80. MM C/C FOR 1932. MM

15J	1931X 549X 1199	51J
3No32 H1121. 0.TO 1932		
25*12c/c 81		26*12c/c 80
	7No20 H 73.1098.TO 1932	

000	000	000	000	000	000	000
3#32	3#32	3#32	3#32	3#32	3#32	3#32
				7#20	7#20	7#20
				oooooo	oooooo	oooooo

BEAM NO. 52 DESIGN RESULTS - FLEXURE

LEN - 1900. MM FY - 414. FC - 30. MPA, SIZE - 550. X 950. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	END
1	879.	9 - 16MM	0.	1900.	YES	YES

CRITICAL NEG MOMENT= 58.21 KN-MET AT 0.MM, LOAD 8
 REQD STEEL= 1612.MM2, ROW= .0033, ROWMX= .0228 ROWMN= .0033
 MAX/MIN/ACTUAL BAR SPACING= 432./ 41./ 54. MMS
 BASIC/REQD. DEVELOPMENT LENGTH = 288./ 568. MMS

BEAM NO. 52 DESIGN RESULTS - SHEAR

AT START SUPPORT - Vu= 38.03 KNS Vc= 437.54 KNS Vs= .00 KNS
 STIRRUPS ARE NOT REQUIRED.
 AT END SUPPORT - Vu= 24.95 KNS Vc= 437.54 KNS Vs= .00 KNS
 STIRRUPS ARE NOT REQUIRED.

51J	1899X 549X 949	52J
9No16 H 879. 0.TO 1900		

oooooo	oooooo	oooooo	oooooo	oooooo	oooooo
9#16	9#16	9#16	9#16	9#16	9#16

--	--	--	--	--	--	--	--	--	--

=====

BEAM NO. 53 DESIGN RESULTS - FLEXURE

LEN - 3638. MM FY - 414. FC - 30. MPA, SIZE - 250. X 1200. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
-------	-------------	----------	-----------	---------	------------	------------

1	1127.	3 - 20MM	0.	3638.	YES	YES
---	-------	----------	----	-------	-----	-----

CRITICAL NEG MOMENT=		212.80 KN-MET	AT	0.MM,	LOAD	8
REQD STEEL=		939.MM2,	ROW=	.0033,	ROWMX=	.0228 ROWMN= .0033
MAX/MIN/ACTUAL BAR SPACING=		128./	45./	64.	MMS	
BASIC/REQD. DEVELOPMENT LENGTH =		450./	819.	MMS		

BEAM NO. 53 DESIGN RESULTS - SHEAR

AT START SUPPORT - Vu= 84.91 KNS Vc= 255.73 KNS Vs= .00 KNS
STIRRUPS ARE NOT REQUIRED.

AT END SUPPORT - Vu= 36.66 KNS Vc= 255.73 KNS Vs= .00 KNS
STIRRUPS ARE NOT REQUIRED.

15J 3637X 249X 1199 53J

3No20 H1127.		0.TO 3638
--------------	--	-----------

0	0	0	0	0	0
3#20	3#20	3#20	3#20	3#20	3#2

=====

BEAM NO. 54 DESIGN RESULTS - FLEXURE

LEN - 3501. MM FY - 414. FC - 30. MPA, SIZE - 250. X 1200. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
-------	-------------	----------	-----------	---------	------------	------------

1	1127.	3 - 20MM	0.	3501.	YES	YES
---	-------	----------	----	-------	-----	-----

CRITICAL NEG MOMENT=		198.51 KN-MET	AT	0.MM,	LOAD	8

```

REQD STEEL= 939.MM2, ROW= .0033, ROWMX= .0228 ROWMN= .0033
MAX/MIN/ACTUAL BAR SPACING= 128./ 45./ 64. MMS
BASIC/REQD. DEVELOPMENT LENGTH = 450./ 819. MMS
    
```

B E A M N O . 5 4 D E S I G N R E S U L T S - S H E A R

AT START SUPPORT - Vu= 81.86 KNS Vc= 255.73 KNS Vs= .00 KNS
 STIRRUPS ARE NOT REQUIRED.
 AT END SUPPORT - Vu= 27.50 KNS Vc= 255.73 KNS Vs= .00 KNS
 STIRRUPS ARE NOT REQUIRED.

```

15J 3501X 249X 1199 54J
-----
| 3No20 H1127. 0 TO 3501 |
|-----|
    
```

0	1000	2000	3000	4000	5000	6000
	3#20	3#20	3#20	3#20	3#20	3#20

B E A M N O . 5 5 D E S I G N R E S U L T S - F L E X U R E

LEN - 3450. MM FY - 414. FC - 30. MPA, SIZE - 250. X 1200. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR	
					STA	END

1	1127.	3 - 20MM	0.	3450.	YES	YES
---	-------	----------	----	-------	-----	-----

```

CRITICAL NEG MOMENT= 193.25 KN-MET AT 0.MM, LOAD 8
REQD STEEL= 939.MM2, ROW= .0033, ROWMX= .0228 ROWMN= .0033
MAX/MIN/ACTUAL BAR SPACING= 128./ 45./ 64. MMS
BASIC/REQD. DEVELOPMENT LENGTH = 450./ 819. MMS
    
```

B E A M N O . 5 5 D E S I G N R E S U L T S - S H E A R

AT START SUPPORT - Vu= 80.72 KNS Vc= 255.73 KNS Vs= .00 KNS
 STIRRUPS ARE NOT REQUIRED.
 AT END SUPPORT - Vu= 27.50 KNS Vc= 255.73 KNS Vs= .00 KNS
 STIRRUPS ARE NOT REQUIRED.

```

15J 3449X 249X 1199 55J
-----
| 3No20 H1127. 0 TO 3450 |
|-----|
    
```

0	3#20	3#20	3#20	3#20	3#20	3#20

=====

BEAM NO. 56 DESIGN RESULTS - FLEXURE

LEN - 3501. MM FY - 414. FC - 30. MPA, SIZE - 250. X 1200. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	1127.	3 - 20MM	0.	3501.	YES	YES

CRITICAL NEG MOMENT= 198.51 KN-MET AT 0.MM, LOAD 8
 REQD STEEL= 939.MM2, ROW= .0033, ROWMX= .0228 ROWMN= .0033
 MAX/MIN/ACTUAL BAR SPACING= 128./ 45./ 64. MMS
 BASIC/REQD. DEVELOPMENT LENGTH = 450./ 819. MMS

BEAM NO. 56 DESIGN RESULTS - SHEAR

AT START SUPPORT - Vu= 81.86 KNS Vc= 255.73 KNS Vs= .00 KNS
 STIRRUPS ARE NOT REQUIRED.
 AT END SUPPORT - Vu= 36.66 KNS Vc= 255.73 KNS Vs= .00 KNS
 STIRRUPS ARE NOT REQUIRED.

15J 3501X 249X 1199 56J

3No20 H1127.	0.TO 3501
--------------	-----------

0	3#20	3#20	3#20	3#20	3#20	3#20

=====

BEAM NO. 57 DESIGN RESULTS - FLEXURE

LEN - 3638. MM FY - 414. FC - 30. MPA, SIZE - 250. X 1200. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA END	
-------	-------------	----------	-----------	---------	----------------	--

1	1127.	3 - 20MM	0.	3638.	YES	YES
CRITICAL NEG MOMENT= 212.81 KN-MET AT 0.MM, LOAD 8 REQD STEEL= 939.MM2, ROW= .0033, ROWMX= .0228 ROWMN= .0033 MAX/MIN/ACTUAL BAR SPACING= 128./ 45./ 64. MMS BASIC/REQD. DEVELOPMENT LENGTH = 450./ 819. MMS						

BEAM NO. 57 DESIGN RESULTS - SHEAR

AT START SUPPORT - Vu= 84.91 KNS Vc= 255.73 KNS Vs= .00 KNS
 STIRRUPS ARE NOT REQUIRED.
 AT END SUPPORT - Vu= 16.23 KNS Vc= 255.73 KNS Vs= .00 KNS
 STIRRUPS ARE NOT REQUIRED.

15J	3637X 249X 1199				57J
3No20 H1127. 0.TO 3638					
0	3#20	3#20	3#20	3#20	3#2

BEAM NO. 58 DESIGN RESULTS - FLEXURE

LEN - 1932. MM FY - 414. FC - 30. MPA, SIZE - 550. X 1200. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA END	
-------	-------------	----------	-----------	---------	----------------	--

1	1125.	5 - 25MM	0.	1932.	YES	YES
CRITICAL NEG MOMENT= 988.47 KN-MET AT 0.MM, LOAD 8 REQD STEEL= 2438.MM2, ROW= .0039, ROWMX= .0228 ROWMN= .0033 MAX/MIN/ACTUAL BAR SPACING= 423./ 50./ 106. MMS BASIC/REQD. DEVELOPMENT LENGTH = 703./ 914. MMS						

BEAM NO. 58 DESIGN RESULTS - SHEAR

AT START SUPPORT - Vu= 459.13 KNS Vc= 149.39 KNS Vs= 390.77 KNS
 Tu= 391.34 KN-MET Ts= 333.1 KN-MET

Tc= 127.3 KN-MET
 PROVIDE 12 MM BARS AT 75. MM C/C FOR 1932. MM
 AT END SUPPORT - Vu= 467.68 KNS Vc= 151.97 KNS Vs= 398.24 KNS
 Tu= 391.34 KN-MET Ts= 333.2 KN-MET
 Tc= 127.2 KN-MET
 PROVIDE 12 MM BARS AT 75. MM C/C FOR 1932. MM

15J		1931X 549X 1199				58J	
5No25 H1125. 0.TO 1932							
27*12c/c 75			27*12c/c 75				
00000	00000	00000	00000	00000	00000	00000	
5#25	5#25	5#25	5#25	5#25	5#25	5#25	

=====

B E A M N O . 5 9 D E S I G N R E S U L T S - F L E X U R E

LEN - 1900. MM FY - 414. FC - 30. MPA, SIZE - 550. X 950. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	END
1	71.	9 - 16MM	1432.	1900.	NO	YES
CRITICAL POS MOMENT= .00 KN-MET AT 1900.MM, LOAD 9 REQD STEEL= 1612.MM2, ROW= .0033, ROWMX= .0228 ROWMN= .0033 MAX/MIN/ACTUAL BAR SPACING= 432./ 41./ 54. MMS BASIC/REQD. DEVELOPMENT LENGTH = 288./ 389. MMS						
2	879.	9 - 16MM	0.	1900.	YES	YES
CRITICAL NEG MOMENT= 58.21 KN-MET AT 0.MM, LOAD 8 REQD STEEL= 1612.MM2, ROW= .0033, ROWMX= .0228 ROWMN= .0033 MAX/MIN/ACTUAL BAR SPACING= 432./ 41./ 54. MMS BASIC/REQD. DEVELOPMENT LENGTH = 288./ 568. MMS						

B E A M N O . 5 9 D E S I G N R E S U L T S - S H E A R

AT START SUPPORT - Vu= 38.04 KNS Vc= 437.54 KNS Vs= .00 KNS
 STIRRUPS ARE NOT REQUIRED.
 AT END SUPPORT - Vu= 20.76 KNS Vc= 437.54 KNS Vs= .00 KNS
 STIRRUPS ARE NOT REQUIRED.

58J		1899X 549X 949			59J	
9No16 H 879. 0.TO 1900						

9No16 H 71.1432.TO 1900
=====

000000000	000000000	000000000	000000000	000000000	000000000
9#16	9#16	9#16	9#16	9#16	9#16
				9#16	9#16
				000000000	000000000

=====

BEAM NO. 81 DESIGN RESULTS - FLEXURE

LEN - 8544. MM FY - 414. FC - 30. MPA, SIZE - 3200. X 300. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	END
-------	-------------	----------	-----------	---------	------------	-----

1	69.	30 - 12MM	1458.	7442.	NO	NO
---	-----	-----------	-------	-------	----	----

CRITICAL POS MOMENT= 276.50 KN-MET AT 4272.MM, LOAD 8
 REQD STEEL= 3333.MM2, ROW= .0045, ROWMX= .0228 ROWMN= .0033
 MAX/MIN/ACTUAL BAR SPACING= 3086./ 37./ 106. MMS
 BASIC/REQD. DEVELOPMENT LENGTH = 162./ 322. MMS

2	229.	32 - 16MM	0.	2573.	YES	NO
---	------	-----------	----	-------	-----	----

CRITICAL NEG MOMENT= 507.89 KN-MET AT 0.MM, LOAD 10
 REQD STEEL= 6402.MM2, ROW= .0087, ROWMX= .0228 ROWMN= .0033
 MAX/MIN/ACTUAL BAR SPACING= 3082./ 41./ 99. MMS
 BASIC/REQD. DEVELOPMENT LENGTH = 288./ 437. MMS

3	231.	41 - 12MM	6437.	8544.	NO	YES
---	------	-----------	-------	-------	----	-----

CRITICAL NEG MOMENT= 375.04 KN-MET AT 8544.MM, LOAD 8
 REQD STEEL= 4630.MM2, ROW= .0063, ROWMX= .0228 ROWMN= .0033
 MAX/MIN/ACTUAL BAR SPACING= 3086./ 37./ 77. MMS
 BASIC/REQD. DEVELOPMENT LENGTH = 162./ 327. MMS

BEAM NO. 81 DESIGN RESULTS - SHEAR

AT START SUPPORT - Vu= 310.11 KNS Vc= 653.75 KNS Vs= .00 KNS

PROVIDE 12 MM BARS AT 85. MM C/C FOR 356. MM

AT END SUPPORT - Vu= 295.59 KNS Vc= 653.75 KNS Vs= .00 KNS

PROVIDE 12 MM BARS AT 85. MM C/C FOR 356. MM

9J 8543X3199X 299 41J

=====

32No16 H 229. 0.TO 2573	41No12 H 231.6437.TO 8544
6*12c/c 85	6*12c/c 85
30No12 H 69.1458.TO 7442	

=====

32#16		41#12
	30#12	

=====

B E A M N O . 8 3 D E S I G N R E S U L T S - F L E X U R E

LEN - 8544. MM FY - 414. FC - 30. MPA, SIZE - 3200. X 300. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	END
-------	-------------	----------	-----------	---------	------------	-----

1	73.	11 - 20MM	1237.	7663.	NO	NO
---	-----	-----------	-------	-------	----	----

CRITICAL POS MOMENT= 285.08 KN-MET AT 4272.MM, LOAD 8
 REQD STEEL= 3440.MM2, ROW= .0046, ROWMX= .0228 ROWMN= .0033
 MAX/MIN/ACTUAL BAR SPACING= 3078./ 45./ 308. MMS
 BASIC/REQD. DEVELOPMENT LENGTH = 450./ 543. MMS

2	231.	45 - 12MM	0.	1751.	YES	NO
---	------	-----------	----	-------	-----	----

CRITICAL NEG MOMENT= 411.57 KN-MET AT 0.MM, LOAD 8
 REQD STEEL= 5060.MM2, ROW= .0068, ROWMX= .0228 ROWMN= .0033
 MAX/MIN/ACTUAL BAR SPACING= 3086./ 37./ 70. MMS
 BASIC/REQD. DEVELOPMENT LENGTH = 162./ 327. MMS

3	231.	43 - 12MM	6437.	8544.	NO	YES
---	------	-----------	-------	-------	----	-----

CRITICAL NEG MOMENT= 383.75 KN-MET AT 8544.MM, LOAD 8
 REQD STEEL= 4850.MM2, ROW= .0067, ROWMX= .0228 ROWMN= .0033
 MAX/MIN/ACTUAL BAR SPACING= 3086./ 37./ 73. MMS
 BASIC/REQD. DEVELOPMENT LENGTH = 162./ 327. MMS

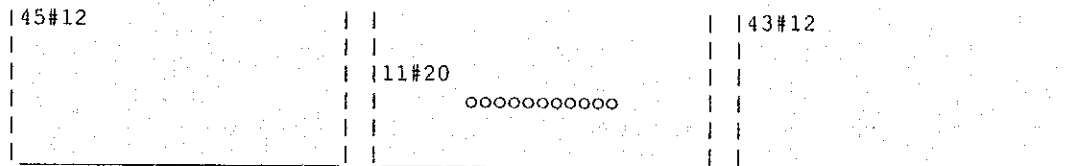
B E A M N O . 8 3 D E S I G N R E S U L T S - S H E A R

AT START SUPPORT - Vu= 306.10 KNS Vc= 653.75 KNS Vs= .00 KNS
 PROVIDE 12 MM BARS AT 85. MM C/C FOR 356. MM
 AT END SUPPORT - Vu= 299.61 KNS Vc= 653.75 KNS Vs= .00 KNS
 PROVIDE 12 MM BARS AT 85. MM C/C FOR 356. MM

21J 8543X3199X 299 51J

45No12 H 231. 0.TO 1751	43No12 H 231.6437.TO 8544
6*12c/c 85	6*12c/c 85
11No20 H 73.1237.TO 7663	

=====



=====

BEAM NO. 85 DESIGN RESULTS - FLEXURE

LEN - 8544. MM FY - 414. FC - 30. MPA, SIZE - 3200. X 300. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	END
-------	-------------	----------	-----------	---------	------------	-----

1	69.	27 - 12MM	1456.	7444.	NO	NO
---	-----	-----------	-------	-------	----	----

CRITICAL POS MOMENT= 251.53 KN-MET AT 8544. MM, LOAD 8
 REQD STEEL= 3021.MM2, ROW= .0041, ROWMX= .0228 ROWMN= .0033
 MAX/MIN/ACTUAL BAR SPACING= 3086./ 37./ 119. MMS
 BASIC/REQD. DEVELOPMENT LENGTH = 162./ 324. MMS

2	227.	17 - 20MM	0.	1970.	YES	NO
---	------	-----------	----	-------	-----	----

CRITICAL NEG MOMENT= 418.93 KN-MET AT 0. MM, LOAD 8
 REQD STEEL= 5325.MM2, ROW= .0074, ROWMX= .0228 ROWMN= .0033
 MAX/MIN/ACTUAL BAR SPACING= 3078./ 45./ 192. MMS
 BASIC/REQD. DEVELOPMENT LENGTH = 450./ 546. MMS

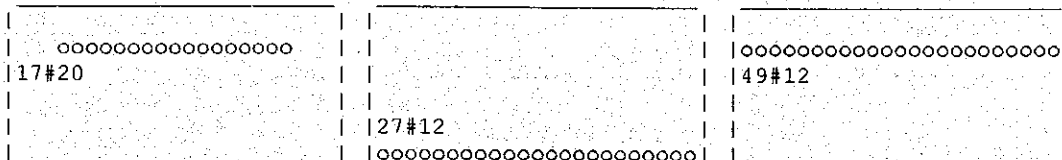
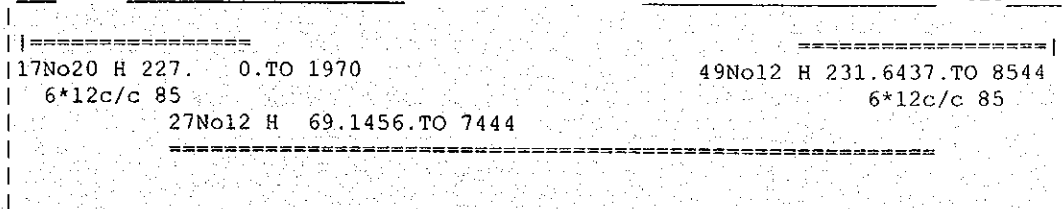
3	231.	49 - 12MM	6437.	8544.	NO	YES
---	------	-----------	-------	-------	----	-----

CRITICAL NEG MOMENT= 443.52 KN-MET AT 8544. MM, LOAD 8
 REQD STEEL= 5479.MM2, ROW= .0074, ROWMX= .0228 ROWMN= .0033
 MAX/MIN/ACTUAL BAR SPACING= 3086./ 37./ 64. MMS
 BASIC/REQD. DEVELOPMENT LENGTH = 162./ 327. MMS

BEAM NO. 85 DESIGN RESULTS - SHEAR

AT START SUPPORT - Vu= 299.97 KNS Vc= 653.75 KNS Vs= .00 KNS
 PROVIDE 12 MM BARS AT 85. MM C/C FOR 356. MM
 AT END SUPPORT - Vu= 305.74 KNS Vc= 653.75 KNS Vs= .00 KNS
 PROVIDE 12 MM BARS AT 85. MM C/C FOR 356. MM

31J 8543X3199X 299 61J



=====

BEAM NO. 91 DESIGN RESULTS - FLEXURE

LEN - 1900. MM FY - 414. FC - 30. MPA, SIZE - 2200. X 1400. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	END
-------	-------------	----------	-----------	---------	------------	-----

1	73.	34 - 20MM	0.	1900.	YES	YES
---	-----	-----------	----	-------	-----	-----

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-----
| CRITICAL POS MOMENT= 5075.99 KN-MET AT 0. MM, LOAD 8 |
| REQD STEEL= 10583. MM2, ROW= .0036, ROWMX= .0228 ROWMN= .0033 |
| MAX/MIN/ACTUAL BAR SPACING= 2078./ 45./ 63. MMS |
| BASIC/REQD. DEVELOPMENT LENGTH = 450./ 624. MMS |
-----
    
```

BEAM NO. 91 DESIGN RESULTS - SHEAR

AT START SUPPORT - Vu= 1804.17 KNS Vc= 2650.68 KNS Vs= .00 KNS

PROVIDE 12 MM BARS AT 123. MM C/C FOR 1900. MM

AT END SUPPORT - Vu= 1747.08 KNS Vc= 2650.68 KNS Vs= .00 KNS

PROVIDE 12 MM BARS AT 123. MM C/C FOR 1900. MM

1J 1899X2199X 1399 71J

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-----
| 17*12c/c123 17*12c/c123 |
| 34No20 H 73. 0.TO 1900 |
-----
    
```

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-----
| 34#20 34#20 34#20 |
| oooooooooooooooooooooo |
-----
    
```

=====

BEAM NO. 92 DESIGN RESULTS - FLEXURE

LEN - 900. MM FY - 414. FC - 30. MPA, SIZE - 2200. X 1400. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	END
-------	-------------	----------	-----------	---------	------------	-----

1	73.	31 - 20MM	0.	900.	YES	YES
---	-----	-----------	----	------	-----	-----

```

-----
| CRITICAL POS MOMENT= 1869.70 KN-MET AT 0. MM, LOAD 8 |
| REQD STEEL= 9733. MM2, ROW= .0033, ROWMX= .0228 ROWMN= .0033 |
| MAX/MIN/ACTUAL BAR SPACING= 2078./ 45./ 69. MMS |
-----
    
```

```

BASIC/REQD. DEVELOPMENT LENGTH = 450./ 630. MMS
-----
2      1327.    31 - 20MM      0.      900.      YES YES
-----
CRITICAL NEG MOMENT= 383.97 KN-MET AT 900.MM, LOAD 9
REQD STEEL= 9733.MM2, ROW= .0033, ROWMX= .0228 ROWMN= .0033
MAX/MIN/ACTUAL BAR SPACING= 2078./ 45./ 69. MMS
BASIC/REQD. DEVELOPMENT LENGTH = 450./ 819. MMS
-----
    
```

BEAM NO. 92 DESIGN RESULTS - SHEAR

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AT START SUPPORT - Vu= .04 KNS Vc= 2650.68 KNS Vs= .00 KNS
                   STIRRUPS ARE NOT REQUIRED.
AT END SUPPORT - Vu= .04 KNS Vc= 2650.68 KNS Vs= .00 KNS
                   STIRRUPS ARE NOT REQUIRED.
    
```

71J	899X2199X 1399	72J
31No20 H1327. 0.TO 900		
31No20 H 73. 0.TO 900		
31#20	31#20	31#20
31#20	31#20	31#20

BEAM NO. 93 DESIGN RESULTS - FLEXURE

LEN - 2800. MM FY - 414. FC - 30. MPA, SIZE - 2200. X 1400. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	END
1	73.	32 - 20MM	0.	2800.	YES	YES
<pre> CRITICAL POS MOMENT= 4831.42 KN-MET AT 0.MM, LOAD 8 REQD STEEL= 10042.MM2, ROW= .0034, ROWMX= .0228 ROWMN= .0033 MAX/MIN/ACTUAL BAR SPACING= 2078./ 45./ 67. MMS BASIC/REQD. DEVELOPMENT LENGTH = 450./ 629. MMS </pre>						
2	1327.	31 - 20MM	1864.	2800.	NO	YES
<pre> CRITICAL NEG MOMENT= 242.56 KN-MET AT 2800.MM, LOAD 8 REQD STEEL= 9733.MM2, ROW= .0033, ROWMX= .0228 ROWMN= .0033 MAX/MIN/ACTUAL BAR SPACING= 2078./ 45./ 69. MMS BASIC/REQD. DEVELOPMENT LENGTH = 450./ 819. MMS </pre>						

BEAM NO. 93 DESIGN RESULTS - SHEAR

AT START SUPPORT - Vu= 1804.47 KNS Vc= 2650.68 KNS Vs= .00 KNS
 PROVIDE 12 MM BARS AT 123. MM C/C FOR 2800. MM
 AT END SUPPORT - Vu= 1819.79 KNS Vc= 2650.68 KNS Vs= .00 KNS
 PROVIDE 12 MM BARS AT 123. MM C/C FOR 2800. MM

1J	2799X2199X 1399	73J
24*12c/c123		31No20 H1327.1864.TO 2800
32No20 H 73. 0.TO 2800		24*12c/c123
		31#20
32#20		32#20
		32#20

BEAM NO. 94 DESIGN RESULTS - FLEXURE

LEN - 2800. MM FY - 414. FC - 30. MPA, SIZE - 2200. X 1400. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	END
1	75.	28 - 25MM	0.	2800.	YES	YES
CRITICAL POS MOMENT= 6498.16 KN-MET AT 0.MM, LOAD 8 REQD STEEL= 13696.MM2, ROW= .0047, ROWMX= .0228 ROWMN= .0033 MAX/MIN/ACTUAL BAR SPACING= 2073./ 50./ 77. MMS BASIC/REQD. DEVELOPMENT LENGTH = 703./ 981. MMS						
2	1327.	31 - 20MM	1864.	2800.	NO	YES
CRITICAL NEG MOMENT= 378.88 KN-MET AT 2800.MM, LOAD 9 REQD STEEL= 9733.MM2, ROW= .0033, ROWMX= .0228 ROWMN= .0033 MAX/MIN/ACTUAL BAR SPACING= 2078./ 45./ 69. MMS BASIC/REQD. DEVELOPMENT LENGTH = 450./ 819. MMS						

BEAM NO. 94 DESIGN RESULTS - SHEAR

AT START SUPPORT - Vu= 2414.58 KNS Vc= 2650.68 KNS Vs= 190.00 KNS
 PROVIDE 12 MM BARS AT 123. MM C/C FOR 2800. MM
 AT END SUPPORT - Vu= 2429.90 KNS Vc= 2650.68 KNS Vs= 208.03 KNS
 PROVIDE 12 MM BARS AT 123. MM C/C FOR 2800. MM

11J	2799X2199X 1399	74J
		31No20 H1327.1864.TO 2800

42#20	42#20	42#20
oooooooooooooooooooooooooooo	oooooooooooooooooooooooooooo	oooooooooooooooooooooooooooo

*****END OF BEAM DESIGN*****

224. CLEAR 0.15
 225. DESIGN COLUMN 101 TO 108 141 TO 148 161 TO 168

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C O L U M N N O. 101 D E S I G N R E S U L T S

FY - 413.7 FC - 30.0 MPA, CIRC SIZE 900.0 MMS DIAMETER TIED

AREA OF STEEL REQUIRED = 6361.7 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
13 - 25 MM (EQUALLY SPACED)	1.003	7	STA	.700

COLUMN INTERACTION: MOMENT ABOUT Z/Y -AXIS (KN-MET)

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
18699.51	14959.61	5951.46	1811.19	304.3
M0	P-tens.	Des.Pn	Des.Mn	e/h
865.04	-2639.85	2373.27	374.47	.158

=====

C O L U M N N O. 102 D E S I G N R E S U L T S

FY - 413.7 FC - 30.0 MPA, CIRC SIZE 900.0 MMS DIAMETER TIED

AREA OF STEEL REQUIRED = 6361.7 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
13 - 25 MM (EQUALLY SPACED)	1.003	7	STA	.700

COLUMN INTERACTION: MOMENT ABOUT Z/Y -AXIS (KN-MET)

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
18699.51	14959.61	5951.46	1811.19	304.3
M0	P-tens.	Des.Pn	Des.Mn	e/h
865.04	-2639.85	2394.68	293.86	.123

=====

C O L U M N N O. 103 D E S I G N R E S U L T S

FY - 413.7 FC - 30.0 MPA, CIRC SIZE 900.0 MMS DIAMETER TIED

AREA OF STEEL REQUIRED = 6361.7 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
13 - 25 MM (EQUALLY SPACED)	1.003	7	STA	.700

COLUMN INTERACTION: MOMENT ABOUT Z/Y -AXIS (KN-MET)

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
18699.51	14959.61	5951.46	1811.19	304.3
M0	P-tens.	Des.Pn	Des.Mn	e/h
865.04	-2639.85	2416.10	214.06	.089

C O L U M N N O. 104 D E S I G N R E S U L T S

FY - 413.7 FC - 30.0 MPA, CIRC SIZE 900.0 MMS DIAMETER TIED

AREA OF STEEL REQUIRED = 6361.7 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
13 - 25 MM (EQUALLY SPACED)	1.003	7	STA	.700

COLUMN INTERACTION: MOMENT ABOUT Z/Y -AXIS (KN-MET)

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
18699.51	14959.61	5951.46	1811.19	304.3
M0	P-tens.	Des.Pn	Des.Mn	e/h
865.04	-2639.85	2437.51	142.88	.059

C O L U M N N O. 105 D E S I G N R E S U L T S

FY - 413.7 FC - 30.0 MPA, CIRC SIZE 900.0 MMS DIAMETER TIED

AREA OF STEEL REQUIRED = 6361.7 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
13 - 25 MM (EQUALLY SPACED)	1.003	7	STA	.700

COLUMN INTERACTION: MOMENT ABOUT Z/Y -AXIS (KN-MET)

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
18699.51	14959.61	5951.46	1811.19	304.3
M0	P-tens.	Des.Pn	Des.Mn	e/h
865.04	-2639.85	2458.92	84.64	.034

C O L U M N N O. 106 D E S I G N R E S U L T S

FY - 413.7 FC - 30.0 MPA, CIRC SIZE 900.0 MMS DIAMETER TIED

AREA OF STEEL REQUIRED = 6361.7 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
13 - 25 MM (EQUALLY SPACED)	1.003	7	STA	.700

COLUMN INTERACTION: MOMENT ABOUT Z/Y -AXIS (KN-MET)

PO	Pn max	P-bal.	M-bal.	e-bal. (MM)
18699.51	14959.61	5951.46	1811.19	304.3
M0	P-tens.	Des.Pn	Des.Mn	e/h
865.04	-2639.85	2480.34	41.29	.017

=====

C O L U M N N O. 107 D E S I G N R E S U L T S

FY - 413.7 FC - 30.0 MPA, CIRC SIZE 900.0 MMS DIAMETER TIED

AREA OF STEEL REQUIRED = 6361.7 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
13 - 25 MM (EQUALLY SPACED)	1.003	7	STA	.700

COLUMN INTERACTION: MOMENT ABOUT Z/Y -AXIS (KN-MET)

PO	Pn max	P-bal.	M-bal.	e-bal. (MM)
18699.51	14959.61	5951.46	1811.19	304.3
M0	P-tens.	Des.Pn	Des.Mn	e/h
865.04	-2639.85	2501.75	18.73	.007

=====

C O L U M N N O. 108 D E S I G N R E S U L T S

FY - 413.7 FC - 30.0 MPA, CIRC SIZE 900.0 MMS DIAMETER TIED

AREA OF STEEL REQUIRED = 6361.7 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
13 - 25 MM (EQUALLY SPACED)	1.003	7	STA	.700

COLUMN INTERACTION: MOMENT ABOUT Z/Y -AXIS (KN-MET)

PO	Pn max	P-bal.	M-bal.	e-bal. (MM)
18699.51	14959.61	5951.46	1811.19	304.3
M0	P-tens.	Des.Pn	Des.Mn	e/h
865.04	-2639.85	2523.16	27.90	.011

=====

C O L U M N N O. 141 D E S I G N R E S U L T S

FY - 413.7 FC - 30.0 MPA, CIRC SIZE 900.0 MMS DIAMETER TIED

AREA OF STEEL REQUIRED = 6361.7 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
13 - 25 MM (EQUALLY SPACED)	1.003	7	STA	.700

COLUMN INTERACTION: MOMENT ABOUT Z/Y -AXIS (KN-MET)

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
18699.51	14959.61	5951.46	1811.19	304.3
M0	P-tens.	Des.Pn	Des.Mn	e/h
865.04	-2639.85	2548.76	303.91	.119

=====

C O L U M N N O . 1 4 2 D E S I G N R E S U L T S

FY - 413.7 FC - 30.0 MPA, CIRC SIZE 900.0 MMS DIAMETER TIED

AREA OF STEEL REQUIRED = 6361.7 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
13 - 25 MM (EQUALLY SPACED)	1.003	7	STA	.700

COLUMN INTERACTION: MOMENT ABOUT Z/Y -AXIS (KN-MET)

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
18699.51	14959.61	5951.46	1811.19	304.3
M0	P-tens.	Des.Pn	Des.Mn	e/h
865.04	-2639.85	2570.16	260.32	.101

=====

C O L U M N N O . 1 4 3 D E S I G N R E S U L T S

FY - 413.7 FC - 30.0 MPA, CIRC SIZE 900.0 MMS DIAMETER TIED

AREA OF STEEL REQUIRED = 6361.7 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
13 - 25 MM (EQUALLY SPACED)	1.003	7	STA	.700

COLUMN INTERACTION: MOMENT ABOUT Z/Y -AXIS (KN-MET)

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
18699.51	14959.61	5951.46	1811.19	304.3
M0	P-tens.	Des.Pn	Des.Mn	e/h
865.04	-2639.85	2591.58	214.59	.083

=====
 C O L U M N N O . 1 4 4 D E S I G N R E S U L T S

FY - 413.7 FC - 30.0 MPA, CIRC SIZE 900.0 MMS DIAMETER TIED

AREA OF STEEL REQUIRED = 6361.7 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
13 - 25 MM (EQUALLY SPACED)	1.003	7	STA	.700

C O L U M N I N T E R A C T I O N : M O M E N T A B O U T Z / Y - A X I S (K N - M E T)

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
18699.51	14959.61	5951.46	1811.19	304.3
M0	P-tens.	Des.Pn	Des.Mn	e/h
865.04	-2639.85	2612.98	167.99	.064

 =====
 C O L U M N N O . 1 4 5 D E S I G N R E S U L T S

FY - 413.7 FC - 30.0 MPA, CIRC SIZE 900.0 MMS DIAMETER TIED

AREA OF STEEL REQUIRED = 6361.7 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
13 - 25 MM (EQUALLY SPACED)	1.003	7	STA	.700

C O L U M N I N T E R A C T I O N : M O M E N T A B O U T Z / Y - A X I S (K N - M E T)

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
18699.51	14959.61	5951.46	1811.19	304.3
M0	P-tens.	Des.Pn	Des.Mn	e/h
865.04	-2639.85	2634.40	123.77	.047

 =====
 C O L U M N N O . 1 4 6 D E S I G N R E S U L T S

FY - 413.7 FC - 30.0 MPA, CIRC SIZE 900.0 MMS DIAMETER TIED

AREA OF STEEL REQUIRED = 6361.7 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
13 - 25 MM (EQUALLY SPACED)	1.003	7	STA	.700

C O L U M N I N T E R A C T I O N : M O M E N T A B O U T Z / Y - A X I S (K N - M E T)

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
18699.51	14959.61	5951.46	1811.19	304.3
M0	P-tens.	Des.Pn	Des.Mn	e/h

865.04 -2639.85 2655.81 84.94 .032

=====

C O L U M N N O. 147 D E S I G N R E S U L T S

FY - 413.7 FC - 30.0 MPA, CIRC SIZE 900.0 MMS DIAMETER TIED

AREA OF STEEL REQUIRED = 6361.7 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

13 - 25 MM 1.003 7 STA .700
(EQUALLY SPACED)

COLUMN INTERACTION: MOMENT ABOUT Z/Y -AXIS (KN-MET)

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
18699.51	14959.61	5951.46	1811.19	304.3
M0	P-tens.	Des.Pn	Des.Mn	e/h
865.04	-2639.85	2677.22	54.09	.020

=====

C O L U M N N O. 148 D E S I G N R E S U L T S

FY - 413.7 FC - 30.0 MPA, CIRC SIZE 900.0 MMS DIAMETER TIED

AREA OF STEEL REQUIRED = 6361.7 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

13 - 25 MM 1.003 7 STA .700
(EQUALLY SPACED)

COLUMN INTERACTION: MOMENT ABOUT Z/Y -AXIS (KN-MET)

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
18699.51	14959.61	5951.46	1811.19	304.3
M0	P-tens.	Des.Pn	Des.Mn	e/h
865.04	-2639.85	2698.64	35.35	.013

=====

C O L U M N N O. 161 D E S I G N R E S U L T S

FY - 413.7 FC - 30.0 MPA, CIRC SIZE 900.0 MMS DIAMETER TIED

AREA OF STEEL REQUIRED = 6361.7 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

13 - 25 MM 1.003 7 STA .700
(EQUALLY SPACED)

COLUMN INTERACTION: MOMENT ABOUT Z/Y -AXIS (KN-MET)

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
18699.51	14959.61	5951.46	1811.19	304.3
M0	P-tens.	Des.Pn	Des.Mn	e/h
865.04	-2639.85	2474.07	347.77	.141

=====

C O L U M N N O. 162 D E S I G N R E S U L T S

FY - 413.7 FC - 30.0 MPA, CIRC SIZE 900.0 MMS DIAMETER TIED

AREA OF STEEL REQUIRED = 6361.7 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
13 - 25 MM (EQUALLY SPACED)	1.003	7	STA	.700

COLUMN INTERACTION: MOMENT ABOUT Z/Y -AXIS (KN-MET)

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
18699.51	14959.61	5951.46	1811.19	304.3
M0	P-tens.	Des.Pn	Des.Mn	e/h
865.04	-2639.85	2495.48	286.72	.115

=====

C O L U M N N O. 163 D E S I G N R E S U L T S

FY - 413.7 FC - 30.0 MPA, CIRC SIZE 900.0 MMS DIAMETER TIED

AREA OF STEEL REQUIRED = 6361.7 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
13 - 25 MM (EQUALLY SPACED)	1.003	7	STA	.700

COLUMN INTERACTION: MOMENT ABOUT Z/Y -AXIS (KN-MET)

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
18699.51	14959.61	5951.46	1811.19	304.3
M0	P-tens.	Des.Pn	Des.Mn	e/h
865.04	-2639.85	2516.90	225.30	.090

=====

C O L U M N N O. 164 D E S I G N R E S U L T S

FY - 413.7 FC - 30.0 MPA, CIRC SIZE 900.0 MMS DIAMETER TIED

AREA OF STEEL REQUIRED = 6361.7 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
13 - 25 MM (EQUALLY SPACED)	1.003	7	STA	.700

 COLUMN INTERACTION: MOMENT ABOUT Z/Y -AXIS (KN-MET)

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
18699.51	14959.61	5951.46	1811.19	304.3
M0	P-tens.	Des.Pn	Des.Mn	e/h
865.04	-2639.85	2538.31	167.39	.066

 =====

 COLUMN NO. 165 DESIGN RESULTS

FY - 413.7 FC - 30.0 MPA, CIRC SIZE 900.0 MMS DIAMETER TIED

AREA OF STEEL REQUIRED = 6361.7 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
13 - 25 MM (EQUALLY SPACED)	1.003	7	STA	.700

 COLUMN INTERACTION: MOMENT ABOUT Z/Y -AXIS (KN-MET)

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
18699.51	14959.61	5951.46	1811.19	304.3
M0	P-tens.	Des.Pn	Des.Mn	e/h
865.04	-2639.85	2559.73	116.59	.046

 =====

 COLUMN NO. 166 DESIGN RESULTS

FY - 413.7 FC - 30.0 MPA, CIRC SIZE 900.0 MMS DIAMETER TIED

AREA OF STEEL REQUIRED = 6361.7 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
13 - 25 MM (EQUALLY SPACED)	1.003	7	STA	.700

 COLUMN INTERACTION: MOMENT ABOUT Z/Y -AXIS (KN-MET)

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
18699.51	14959.61	5951.46	1811.19	304.3
M0	P-tens.	Des.Pn	Des.Mn	e/h
865.04	-2639.85	2581.14	75.46	.029

 =====

 COLUMN NO. 167 DESIGN RESULTS

FY - 413.7 FC - 30.0 MPA, CIRC SIZE 900.0 MMS DIAMETER TIED

AREA OF STEEL REQUIRED = 6361.7 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
13 - 25 MM (EQUALLY SPACED)	1.003	7	STA	.700

13 - 25 MM 1.003 7 STA .700
 (EQUALLY SPACED)

COLUMN INTERACTION: MOMENT ABOUT Z/Y -AXIS (KN-MET)

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
18699.51	14959.61	5951.46	1811.19	304.3
M0	P-tens.	Des.Pn	Des.Mn	e/h
865.04	-2639.85	2602.55	46.48	.018

C O L U M N N O . 1 6 8 D E S I G N R E S U L T S

FY - 413.7 FC - 30.0 MPA, CIRC SIZE 900.0 MMS DIAMETER TIED

AREA OF STEEL REQUIRED = 6361.7 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

13 - 25 MM 1.003 7 STA .700
 (EQUALLY SPACED)

COLUMN INTERACTION: MOMENT ABOUT Z/Y -AXIS (KN-MET)

P0	Pn max	P-bal.	M-bal.	e-bal. (MM)
18699.51	14959.61	5951.46	1811.19	304.3
M0	P-tens.	Des.Pn	Des.Mn	e/h
865.04	-2639.85	2623.96	34.64	.013

*****END OF COLUMN DESIGN RESULTS*****

226. END CON DESIGN
 227. FINISH

***** END OF STAAD-III *****

**** DATE= FEB 29,2000 TIME= 15:52:10 ****

 * For questions on STAAD-III, contact: *
 * Research Engineers, Inc at *
 * West Coast: Ph- (714) 974-2500 Fax- (714) 921-2543 *
 * East Coast: Ph- (508) 688-3626 Fax- (508) 685-7230 *

Bearing Capacity of a Pile: Bored into the soil layers as indicated by BH1BA1

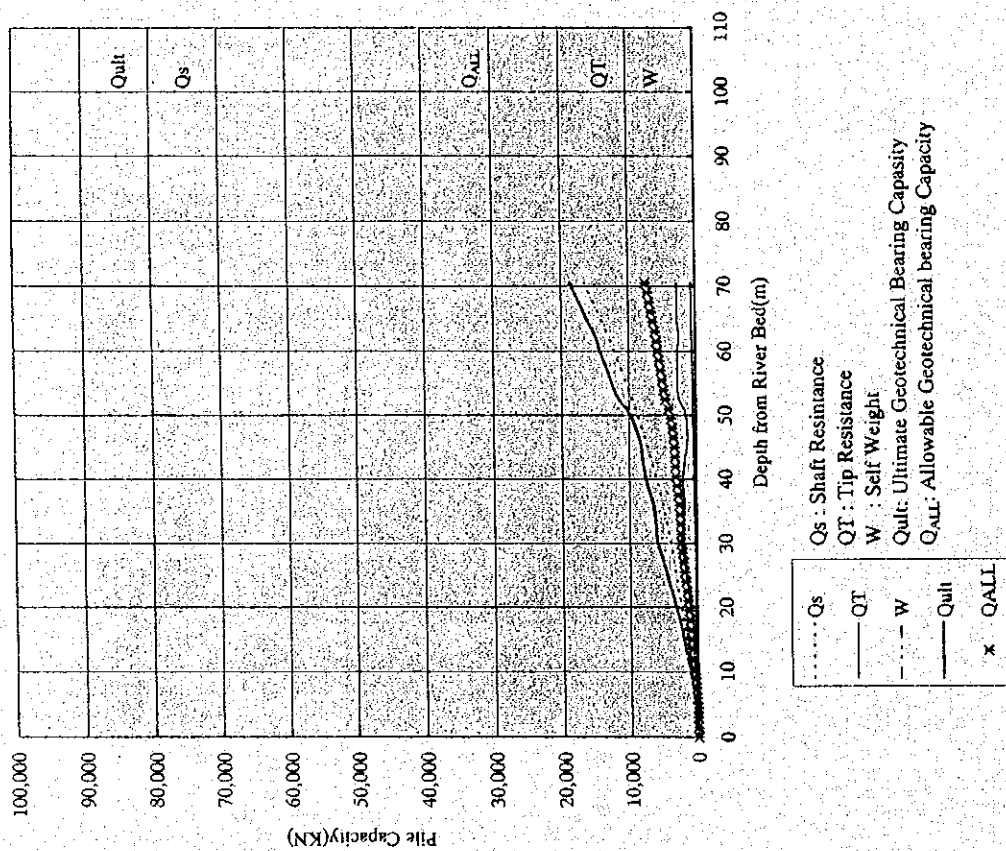
Diameter of pile = 0.90 m
Scour depth = 0.00 m
Safety Factor = 2.50

Depth	N	γ _s	γ _i	soil type	α _i	α _s	consistency	z _i	β _i	α _s	f _{si}	Q _{si}	Q _s	N _c	s _w	q _{tr}	q _{tr}	F _r	q _{tr}	Q _T	W	Q _{ULT}	Q _{ALL}
0.0	5	14.9	5.1	clayey silt	0.00	0.0	-	1.5	1.20	3.8	0.0	0.0	0.0	0.0	0.0	0.0	0.0	-	143.6	91.4	13.4	136.4	0.0
1.5	5	14.9	5.1	clayey silt	0.00	0.0	-	3.0	1.08	19.1	20.6	87.3	58.4	8.0	0.0	143.6	0.0	-	71.8	45.7	26.7	164.7	54.6
3.0	5	14.9	5.1	clayey silt	0.00	0.0	-	4.5	0.98	26.8	26.3	111.4	145.7	9.0	0.0	86.2	0.0	-	86.2	54.8	40.1	271.9	65.9
4.5	0	14.9	5.1	clayey silt	0.00	0.0	-	6.0	0.90	34.4	31.0	131.6	388.7	9.0	0.0	244.2	0.0	-	244.2	155.3	53.4	490.6	108.8
6.0	0	14.9	5.1	clayey silt	0.00	0.0	-	7.5	0.83	42.1	34.9	148.2	536.9	9.0	0.0	387.8	0.0	-	387.8	246.7	66.8	716.8	186.7
7.5	6	14.9	5.1	clayey silt	0.00	0.0	-	9.0	0.77	49.7	41.1	161.6	698.5	9.0	0.0	588.9	0.0	-	588.9	374.7	80.2	993.0	397.2
10.5	11	18.1	8.3	silty sand	0.00	0.0	-	10.5	0.71	62.2	44.0	186.6	885.1	9.0	0.0	746.9	0.0	-	746.9	475.2	93.5	1266.8	506.7
12.0	10	18.1	8.3	silty sand	0.00	0.0	-	12.0	0.65	74.6	48.7	206.7	1091.8	9.0	0.0	818.7	0.0	-	818.7	520.9	106.9	1505.7	602.3
13.5	14	18.1	8.3	silty sand	0.00	0.0	-	13.5	0.60	87.1	52.4	222.2	1313.9	9.0	0.0	976.8	0.0	-	976.8	621.4	120.2	1815.1	726.0
15.0	17	18.1	8.3	silty sand	0.00	0.0	-	15.0	0.55	99.5	55.1	233.9	1547.3	9.0	0.0	1062.9	0.0	-	1062.9	676.2	133.6	2089.9	836.0
16.5	16	18.1	8.3	silty sand	0.00	0.0	-	16.5	0.51	112.0	56.7	240.6	1788.0	9.0	0.0	1120.4	0.0	-	1120.4	712.8	147.0	2353.8	941.5
18.0	21	18.1	8.3	silty sand	0.00	0.0	-	18.0	0.46	124.4	57.6	244.1	2092.1	9.0	0.0	1177.8	0.0	-	1177.8	749.3	160.3	2621.1	1048.4
19.5	20	18.1	8.3	silty sand	0.00	0.0	-	19.5	0.42	136.9	57.5	243.9	2276.0	9.0	0.0	1450.8	0.0	-	1450.8	922.9	173.7	3025.3	1210.1
21.0	21	18.1	8.3	silty sand	0.00	0.0	-	21.0	0.38	149.3	56.7	240.3	2516.3	9.0	0.0	1738.0	0.0	-	1738.0	1105.7	187.0	3435.0	1374.0
22.5	20	18.1	8.3	silty sand	0.00	0.0	-	22.5	0.34	161.8	55.3	226.1	2749.7	9.0	0.0	1953.5	0.0	-	1953.5	1242.8	200.4	3792.0	1516.8
24.0	40	19.6	9.8	silty sand	0.00	0.0	-	24.0	0.30	176.5	53.3	226.1	2975.8	9.0	0.0	2312.6	0.0	-	2312.6	1471.2	213.8	4233.2	1693.3
25.5	40	19.6	9.8	silty sand	0.00	0.0	-	25.5	0.27	191.2	50.7	215.0	3190.8	9.0	0.0	2614.2	0.0	-	2614.2	1663.1	227.1	4626.8	1850.7
27.0	36	19.6	9.8	silty sand	0.00	0.0	-	27.0	0.25	205.9	51.5	218.3	3409.1	9.0	0.0	2887.2	0.0	-	2887.2	1836.7	240.5	5005.3	2002.1
28.5	45	19.6	9.8	silty sand	0.00	0.0	-	28.5	0.25	220.6	51.1	233.9	3642.9	9.0	0.0	3246.3	0.0	-	3246.3	2065.2	253.8	5454.3	2181.7
30.0	61	19.6	9.8	silty sand	0.00	0.0	-	30.0	0.25	235.3	58.8	249.5	3892.4	9.0	0.0	3662.8	0.0	-	3662.8	2330.2	267.2	5955.4	2382.2
31.5	59	19.6	9.8	silty sand	0.00	0.0	-	31.5	0.25	250.0	62.3	265.0	4157.4	9.0	0.0	3375.5	0.0	-	3375.5	2147.4	280.6	6024.3	2409.7
33.0	41	19.6	9.8	silty sand	0.00	0.0	-	33.0	0.25	264.7	66.2	280.6	4438.1	9.0	0.0	3102.6	0.0	-	3102.6	1973.8	293.9	6118.0	2447.2
34.5	74	19.6	9.8	silty sand	0.00	0.0	-	34.5	0.25	279.4	69.8	296.2	4734.3	9.0	0.0	2987.7	0.0	-	2987.7	1900.7	307.3	6327.7	2531.1
36.0	41	19.6	9.8	silty sand	0.00	0.0	-	36.0	0.25	294.1	73.5	311.8	5046.1	9.0	0.0	2714.8	0.0	-	2714.8	1727.1	320.6	6452.5	2581.0
37.5	40	19.6	9.8	silty sand	0.00	0.0	-	37.5	0.25	308.8	77.2	327.4	5375.5	9.0	0.0	2887.2	0.0	-	2887.2	1836.7	334.0	6876.2	2750.5
39.0	53	19.6	9.8	silty sand	0.00	0.0	-	39.0	0.25	323.5	80.9	343.0	5716.5	9.0	0.0	3088.3	0.0	-	3088.3	1964.7	347.4	7333.8	2933.5
40.5	55	19.6	9.8	silty sand	0.00	0.0	-	40.5	0.25	338.2	84.5	358.6	6075.0	9.0	0.0	3073.9	0.0	-	3073.9	1955.5	360.7	7669.8	3067.9
43.5	54	19.6	9.8	silty sand	0.00	0.0	-	43.5	0.25	352.9	88.2	374.1	6449.2	9.0	0.0	2829.7	0.0	-	2829.7	1800.2	374.1	7875.3	3150.1
45.0	52	19.6	9.8	silty sand	0.00	0.0	-	45.0	0.25	367.6	91.9	389.7	6838.9	9.0	0.0	2599.9	0.0	-	2599.9	1654.0	387.4	8105.5	3242.2
46.5	38	19.6	9.8	silty sand	0.00	0.0	-	46.5	0.25	382.3	95.6	405.3	7244.2	9.0	0.0	2140.2	0.0	-	2140.2	1361.6	400.8	8205.0	3282.0
48.0	37	19.6	9.8	silty sand	0.00	0.0	-	48.0	0.25	397.0	99.2	420.9	7665.1	9.0	0.0	2025.3	0.0	-	2025.3	1288.5	414.1	8539.4	3415.8
49.5	22	19.6	9.8	silty sand	0.00	0.0	-	49.5	0.25	411.7	102.9	436.5	8101.6	9.0	0.0	2082.8	0.0	-	2082.8	1325.0	427.5	8999.1	3599.7
51.0	44	19.6	9.8	silty sand	0.00	0.0	-	51.0	0.25	426.4	106.6	452.1	8553.7	9.0	0.0	2272.1	0.0	-	2272.1	1407.2	440.9	9520.1	3808.0
52.5	42	19.6	9.8	silty sand	0.00	0.0	-	52.5	0.25	441.1	110.3	467.7	9021.4	9.0	0.0	2757.9	0.0	-	2757.9	1754.5	454.2	10321.6	4128.7
54.0	46	19.6	9.8	silty sand	0.00	0.0	-	54.0	0.25	455.8	113.9	483.3	9504.6	9.0	0.0	3921.4	0.0	-	3921.4	2494.7	467.6	11531.7	4612.7
57.0	125	20.4	10.6	silty sand	0.00	0.0	-	55.5	0.25	470.5	117.6	498.8	10003.5	9.0	0.0	4280.5	0.0	-	4280.5	2723.1	480.9	12245.6	4898.3
55.5	60	20.4	10.6	silty sand	0.00	0.0	-	57.0	0.25	486.4	121.6	515.7	10519.2	9.0	0.0	4309.2	0.0	-	4309.2	2741.4	494.3	12766.3	5106.5
58.5	67	20.4	10.6	fine sand	0.00	0.0	-	58.5	0.25	502.3	125.6	532.6	11017.7	9.0	0.0	4309.2	0.0	-	4309.2	2741.4	507.7	13285.5	5314.2
60.0	87	20.4	10.6	fine sand	0.00	0.0	-	60.0	0.25	518.2	129.5	549.4	11601.1	9.0	0.0	4309.2	0.0	-	4309.2	2741.4	521.0	13821.5	5528.6
61.5	77	20.4	10.6	fine sand	0.00	0.0	-	61.5	0.25	534.1	133.3	566.3	12167.4	9.0	0.0	4309.2	0.0	-	4309.2	2741.4	534.4	14374.4	5749.8
63.0	74	20.4	10.6	fine sand	0.00	0.0	-	63.0	0.25	550.0	137.5	583.1	12730.5	9.0	0.0	3993.2	0.0	-	3993.2	2540.4	547.7	14745.2	5897.3
64.5	64	20.4	10.6	fine sand	0.00	0.0	-	63.0	0.25	565.9	141.5	600.0	13330.5	9.0	0.0	3978.8	0.0	-	3978.8	2531.2	561.1	15320.7	6128.3
66.0	63	20.4	10.6	fine sand	0.00	0.0	-	64.5	0.25	581.8	145.4	616.8	13967.4	9.0	0.0	4309.2	0.0	-	4309.2	2741.4	574.5	16134.3	6453.7
67.5	76	20.4	10.6	fine sand	0.00	0.0	-	66.0	0.25	597.7	149.4	633.7	14601.1	9.0	0.0	4309.2	0.0	-	4309.2	2741.4	587.8	16754.7	6701.9
69.0	134	20.4	10.6	fine sand	0.00	0.0	-	67.5	0.25	613.6	153.4	650.6	15251.7	9.0	0.0	4309.2	0.0	-	4309.2	2741.4	601.2	17391.9	6956.7
70.5	132	20.4	10.6	fine sand	0.00	0.0	-	69.0	0.25	629.5	157.4	667.4	15919.1	9.0	0.0	4309.2	0.0	-	4309.2	2741.4	614.5	18045.9	7218.4
70.5	132	20.4	10.6	fine sand	0.00	0.0	-	70.5	0.25	645.4	161.3	684.3	16603.4	9.0	0.0	4309.2	0.0	-	4309.2	2741.4	627.9	18716.9	7486.7

Geotechnical Ultimate Bearing Capacity of Bored Pile

Depth	Shaft Diameter D=0.9m			
	Qs	QT	W	Q _{ALL}
0.0	0.0	0.0	0.0	0.0
1.5	58.4	91.4	13.4	54.6
3.0	145.7	45.7	26.7	65.9
4.5	257.1	54.8	40.1	108.8
6.0	388.7	135.3	53.4	196.2
7.5	536.9	246.7	66.8	286.7
9.0	698.5	374.7	80.2	397.2
10.5	885.1	475.2	93.5	506.7
12.0	1091.8	570.9	106.9	602.3
13.5	1313.9	621.4	120.2	725.0
15.0	1547.3	676.2	133.6	836.0
16.5	1788.0	712.8	147.0	941.5
18.0	2032.1	749.3	160.3	1,048.4
19.5	2276.0	922.9	173.7	1,210.1
21.0	2516.3	1,105.7	187.0	1,374.0
22.5	2749.7	1,292.8	200.4	1,516.8
24.0	2975.8	1,471.2	213.8	1,693.3
25.5	3190.8	1,663.1	227.1	1,850.7
27.0	3409.1	1,836.7	240.5	2,002.1
28.5	3642.9	2,065.2	253.8	2,181.7
30.0	3892.4	2,302.2	267.2	2,382.2
31.5	4137.4	2,147.4	280.6	2,409.7
33.0	4438.1	1,973.8	293.9	2,447.2
34.5	4734.3	1,900.7	307.3	2,531.1
36.0	5046.1	1,727.1	320.6	2,581.0
37.5	5373.5	1,836.7	334.0	2,750.5
39.0	5716.5	1,964.7	347.4	2,935.5
40.5	6075.0	1,955.5	360.7	3,067.9
42.0	6449.2	1,800.2	374.1	3,150.1
43.5	6838.9	1,654.0	387.4	3,242.2
45.0	7244.2	1,561.6	400.8	3,282.0
46.5	7665.1	1,288.5	414.1	3,415.8
48.0	8101.6	1,325.0	427.5	3,599.7
49.5	8553.7	1,407.2	440.9	3,808.0
51.0	9021.4	1,754.5	454.2	4,128.7
52.5	9504.6	2,494.7	467.6	4,612.7
54.0	10003.5	2,723.1	480.9	4,898.3
55.5	10519.2	2,741.4	494.3	5,106.5
57.0	11051.7	2,741.4	507.7	5,314.2
58.5	11601.3	2,741.4	521.0	5,528.6
60.0	12167.4	2,741.4	534.4	5,749.8
61.5	12750.5	2,540.4	547.7	5,897.3
63.0	13350.5	2,531.2	561.1	6,128.3
64.5	13967.4	2,741.4	574.5	6,453.7
66.0	14601.1	2,741.4	587.8	6,701.9
67.5	15251.7	2,741.4	601.2	6,956.7
69.0	15919.1	2,741.4	614.5	7,218.4
70.5	16603.4	2,741.4	627.9	7,486.7

Ultimate Geotechnical Bearing Capacity of Bored Pile: BA1



Bearing Capacity of a Pile: Bored into the soil layers as indicated by BH1BA2

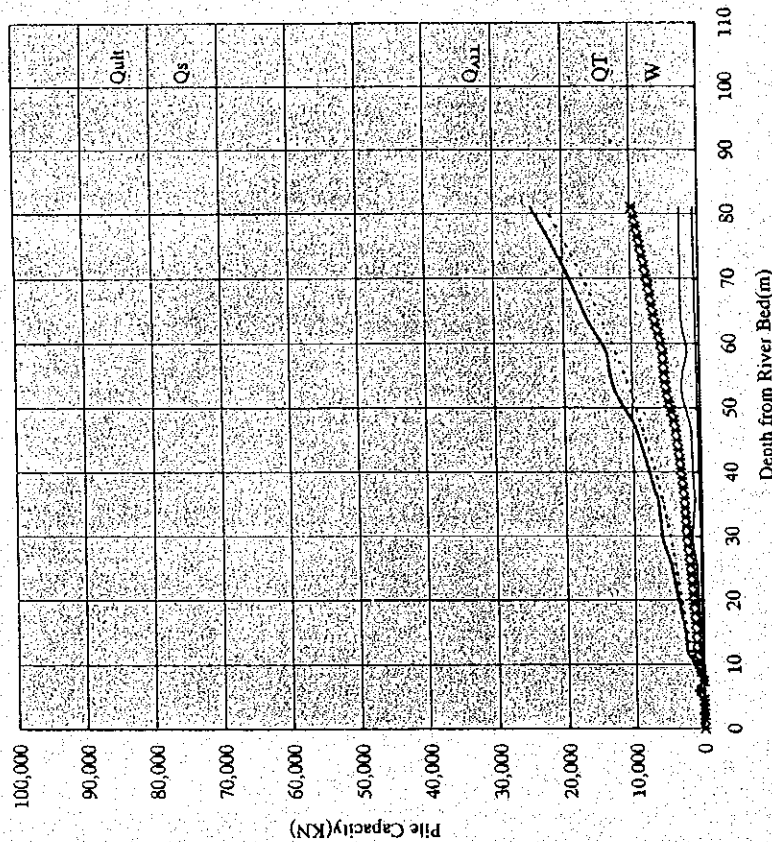
Diameter of pile = 0.90 m

Scour depth = 0.00 m

Safety Factor = 2.50

Depth	N	γ _s	γ _t	soil type	α _i	s _u	consistency	z _i	β _i	σ _u	f _c	Q _a	Q _c	N _c	s _u	q _T	a	b	F _L	q _{TR}	Q _{TR}	W	Q _{ULT}	Q _{ALL}	
0.0	14.9	5.1	clayey silt	0.00	0.0	0.0	0.0	0.0	1.20	3.8	0.0	0.0	0.0	0.0	0.0	0.0	-	-	-	0.0	0.0	0.0	0.0	0.0	
1.5	2	14.9	5.1	clayey silt	0.00	0.0	-	-	1.3	1.20	11.5	3.8	38.4	8.0	0.0	0.0	201.1	-	-	-	201.1	127.9	13.4	173.0	69.2
3.0	3	14.9	5.1	clayey silt	0.00	0.0	-	-	3.0	1.08	19.1	20.6	87.3	145.7	9.0	0.0	186.7	-	-	-	186.7	118.8	26.7	237.8	95.1
4.5	3	15.7	5.9	clayey silt	0.00	0.0	-	-	4.5	0.98	28.0	27.5	116.4	262.1	9.0	0.0	158.0	-	-	-	158.0	100.5	40.1	322.6	129.0
6.0	4	15.7	5.9	cohesive	0.55	35.0	soft	6.0	-	36.8	19.3	81.6	343.8	9.0	192.5	1653.8	0.015	1.500	0.139	1653.8	1052.1	53.4	1342.4	537.0	
7.5	1	14.9	5.1	cohesive	0.55	10.0	very soft	7.5	-	44.3	5.3	23.3	367.1	9.0	183.8	0.0	0.015	1.500	0.139	0.0	0.0	66.8	300.3	120.1	
9.0	3	15.7	5.9	clayey silt	0.00	0.0	-	-	9.0	0.77	53.3	40.9	173.3	540.4	9.0	0.0	1034.2	-	-	-	637.9	80.3	1118.2	447.3	
10.5	24	18.1	8.3	cohesive	0.55	725.0	very stiff	10.5	-	65.8	265.0	1123.9	1664.3	9.0	181.3	0.0	0.015	1.500	0.139	0.0	0.0	93.5	1570.8	628.3	
12.0	23	18.1	8.3	silty sand	0.00	0.0	-	-	12.0	0.65	78.2	51.1	216.6	1881.0	9.0	0.0	1091.7	-	-	-	694.5	106.9	2468.6	987.4	
13.5	22	18.1	8.3	silty sand	0.00	0.0	-	-	13.5	0.60	90.7	54.5	231.3	2112.3	9.0	0.0	962.4	-	-	-	962.4	120.2	2604.3	1041.7	
15.0	19	18.1	8.3	silty sand	0.00	0.0	-	-	15.0	0.55	103.1	57.0	241.8	2354.2	9.0	0.0	847.3	-	-	-	847.3	133.6	2759.7	1103.9	
16.5	12	18.1	8.3	silty sand	0.00	0.0	-	-	16.5	0.31	115.6	58.6	248.4	2602.3	9.0	0.0	833.1	-	-	-	833.1	530.0	147.0	2985.6	1194.2
18.0	14	18.1	8.3	silty sand	0.00	0.0	-	-	18.0	0.46	128.0	59.2	251.2	2853.7	9.0	0.0	933.7	-	-	-	933.7	594.0	160.3	3287.4	1314.9
19.5	14	18.1	8.3	silty sand	0.00	0.0	-	-	19.5	0.42	140.5	59.0	250.3	3104.1	9.0	0.0	1019.8	-	-	-	1019.8	648.8	173.7	3579.2	1431.7
21.0	18	18.1	8.3	silty sand	0.00	0.0	-	-	21.0	0.38	152.9	58.0	246.1	3350.2	9.0	0.0	1106.0	-	-	-	1106.0	703.6	187.0	3866.7	1546.7
22.5	19	18.1	8.3	silty sand	0.00	0.0	-	-	22.5	0.34	165.4	56.2	238.5	3588.7	9.0	0.0	1177.8	-	-	-	1177.8	749.3	200.4	4137.6	1653.0
24.0	20	18.1	8.3	silty sand	0.00	0.0	-	-	24.0	0.30	177.8	53.7	227.8	3816.5	9.0	0.0	1249.7	-	-	-	1249.7	795.0	213.8	4397.8	1759.1
25.5	20	18.1	8.3	silty sand	0.00	0.0	-	-	25.5	0.27	190.3	50.5	214.0	4030.5	9.0	0.0	1335.9	-	-	-	1335.9	849.8	227.1	4633.3	1861.3
27.0	23	18.1	8.3	silty sand	0.00	0.0	-	-	27.0	0.25	202.7	50.7	214.9	4245.5	9.0	0.0	1709.3	-	-	-	1709.3	1087.4	240.5	5092.4	2037.0
28.5	24	18.1	8.3	silty sand	0.00	0.0	-	-	28.5	0.25	215.2	53.8	228.1	4473.6	9.0	0.0	2125.9	-	-	-	2125.9	1352.4	263.8	5572.2	2228.9
30.0	26	18.1	8.3	silty sand	0.00	0.0	-	-	30.0	0.23	227.6	56.9	241.3	4715.0	9.0	0.0	2384.4	-	-	-	2384.4	1516.9	267.2	5964.7	2385.9
31.5	46	19.6	9.8	silty sand	0.00	0.0	-	-	31.5	0.25	242.3	60.6	256.9	4971.9	9.0	0.0	2140.2	-	-	-	2140.2	1264.5	280.6	6052.9	2421.2
33.0	52	19.6	9.8	silty sand	0.00	0.0	-	-	33.0	0.25	257.0	64.3	272.5	5244.4	9.0	0.0	1924.8	-	-	-	1924.8	1224.5	293.9	6175.0	2470.0
34.5	42	19.6	9.8	silty sand	0.00	0.0	-	-	34.5	0.25	271.7	67.9	288.1	5532.5	9.0	0.0	1723.7	-	-	-	1723.7	1096.6	307.3	6321.8	2528.7
36.0	31	19.6	9.8	silty sand	0.00	0.0	-	-	36.0	0.25	286.4	71.6	303.7	5836.2	9.0	0.0	1565.7	-	-	-	1565.7	996.0	320.6	6511.6	2604.7
37.5	31	19.6	9.8	silty sand	0.00	0.0	-	-	37.5	0.25	301.1	75.3	319.3	6155.5	9.0	0.0	1867.3	-	-	-	1867.3	1187.9	334.0	7009.5	2803.8
39.0	38	19.6	9.8	silty sand	0.00	0.0	-	-	39.0	0.25	315.8	79.0	334.9	6490.4	9.0	0.0	1939.1	-	-	-	1939.1	1233.6	347.4	7376.7	2950.7
40.5	31	19.6	9.8	silty sand	0.00	0.0	-	-	40.5	0.25	330.5	82.6	350.5	6840.8	9.0	0.0	1996.6	-	-	-	1996.6	1278.2	360.7	7750.3	3100.1
42.0	30	19.6	9.8	silty sand	0.00	0.0	-	-	42.0	0.25	345.2	86.3	366.0	7206.9	9.0	0.0	2025.3	-	-	-	2025.3	1288.5	374.1	8121.3	3248.5
43.5	36	19.6	9.8	silty sand	0.00	0.0	-	-	43.5	0.25	359.9	90.0	381.6	7588.5	9.0	0.0	2097.1	-	-	-	2097.1	1354.1	387.4	8535.2	3414.1
45.0	42	19.6	9.8	silty sand	0.00	0.0	-	-	45.0	0.25	374.6	93.7	397.2	7985.7	9.0	0.0	2169.0	-	-	-	2169.0	1379.8	400.8	8964.8	3589.9
46.5	33	19.6	9.8	silty sand	0.00	0.0	-	-	46.5	0.25	389.3	97.3	412.8	8398.5	9.0	0.0	2183.3	-	-	-	2183.3	1389.0	414.1	9373.3	3749.3
48.0	35	19.6	9.8	silty sand	0.00	0.0	-	-	48.0	0.25	404.0	101.0	428.4	8826.9	9.0	0.0	2556.8	-	-	-	2556.8	1626.6	427.5	10025.9	4010.4
49.5	41	19.6	9.8	silty sand	0.00	0.0	-	-	49.5	0.25	418.7	104.7	444.0	9270.9	9.0	0.0	3231.9	-	-	-	3231.9	2036.0	440.9	10886.0	4334.4
51.0	43	19.6	9.8	silty sand	0.00	0.0	-	-	51.0	0.25	433.4	108.4	459.6	9730.4	9.0	0.0	3863.9	-	-	-	3863.9	2458.1	454.2	11734.3	4693.7
52.5	59	19.6	9.8	silty sand	0.00	0.0	-	-	52.5	0.25	448.1	112.0	475.1	10205.5	9.0	0.0	4309.2	-	-	-	4309.2	2741.4	467.6	12479.4	4991.7
54.0	82	19.6	9.8	silty sand	0.00	0.0	-	-	54.0	0.25	462.8	115.7	490.7	10696.3	9.0	0.0	4309.2	-	-	-	4309.2	2741.4	480.9	12956.7	5182.7
55.5	85	20.4	10.6	silty sand	0.00	0.0	-	-	55.5	0.25	478.7	119.7	507.6	11203.9	9.0	0.0	4021.9	-	-	-	4021.9	2558.6	494.3	13268.2	5307.3
57.0	88	20.4	10.6	silty sand	0.00	0.0	-	-	57.0	0.25	494.6	123.7	524.1	11728.3	9.0	0.0	3433.0	-	-	-	3433.0	2184.0	507.7	13404.6	5361.8
58.5	68	20.4	10.6	silty sand	0.00	0.0	-	-	58.5	0.25	510.5	127.6	541.3	12269.6	9.0	0.0	3915.9	-	-	-	3915.9	1855.0	521.0	13603.6	5441.4
60.0	39	20.4	10.6	silty sand	0.00	0.0	-	-	60.0	0.25	526.4	131.6	558.2	12827.8	9.0	0.0	3102.6	-	-	-	3102.6	1973.8	534.4	14267.2	5706.9
61.5	44	20.4	10.6	silty sand	0.00	0.0	-	-	61.5	0.25	542.3	135.6	575.0	13402.8	9.0	0.0	3705.9	-	-	-	3705.9	2357.6	547.7	15212.6	6085.1
63.0	52	20.4	10.6	silty sand	0.00	0.0	-	-	63.0	0.25	558.2	139.6	591.9	13994.7	9.0	0.0	4108.1	-	-	-	4108.1	2613.5	561.1	16047.0	6418.8
64.5	81	20.4	10.6	silty sand	0.00	0.0	-	-	64.5	0.25	574.1	143.5	608.7	14603.4	9.0	0.0	4309.2	-	-	-	4309.2	2741.4	574.5	16770.3	6708.1
66.0	81	20.4	10.6	silty sand	0.00	0.0	-	-	66.0	0.25	590.0	147.5	625.6	15229.0	9.0	0.0	4309.2	-	-	-	4309.2	2741.4	587.8	17382.6	6933.0
67.5	72	20.4	10.6	silty sand	0.00	0.0	-	-	67.5	0.25	605.9	151.5	642.3	15871.5	9.0	0.0	4309.2	-	-	-	4309.2	2741.4	601.2	18011.7	7204.7
69.0	132	20.4	10.6	fine sand	0.00	0.0	-	-	69.0	0.25	621.8	155.5	659.3	16530.8	9.0	0.0	4309.2	-	-	-	4309.2	2741.4	614.5	18657.6	7463.1
70.5	84	20.4	10.6	fine sand	0.00	0.0	-	-	70.5	0.25	637.7	159.4	676.2	17206.9	9.0	0.0	4309.2	-	-	-	4309.2	2741.4	627.9	19320.4	7728.2
72.0	93	20.4	10.6	fine sand	0.00	0.0	-	-	72.0	0.25	653.6	163.4	693.0	17900.0	9.0	0.0	4309.2	-	-	-	4309.2	2741.4	641.3	20000.1	8000.0
73.5	112	20.4	10.6	fine sand	0.00	0.0	-	-	73.5	0.25	669.5	167.4	709.9	18609.9	9.0	0.0	4309.2	-	-	-	4309.2	2741.4	654.6	20696.6	8278.7
75.0	105	20.4	10.6	fine sand	0.00	0.0	-	-	75.0	0.25	685.4	171.4	726.7	19336.6	9.0	0.0	4309.2	-	-	-	4309.2	2741.4	668.0	21410.0	8564.0
76.5	112	20.4	10.6	fine sand	0.00	0.0	-	-	76.5	0.25	701.3	175.3	743.6	20080.2	9.0	0.0	4309.2	-	-	-	4309.2	2741.4	681.3	22140.3	8856.0
78.0	115	20.4	10.6	fine sand	0.00	0.0	-	-	78.0	0.25	717.2	179.3	760.5	20840.7	9.0	0.0	4309.2	-	-	-					

Ultimate Geotechnical Bearing Capacity of Bored Pile: BA2



Qs : Shaft Resistance
 QT : Tip Resistance
 W : Self Weight
 Qult : Ultimate Geotechnical Bearing Capacity
 QALL : Allowable bearing Capacity