

8. Hydrodynamic Modeling of Drainage Pipe System

The existing drainage pipe system in Ho Chi Minh City is large and complex networks. Furthermore the outlets of the drainage pipe system are affected by the tidal time varying water levels.

Thus, for consideration of the drainage pipe network and the hydraulic condition at the outlet, MOUSE, developed by Danish Hydraulic Institute (DHI), can be applied to modeling

(1) Modeling for the Drainage Areas in the Study Area

At the Master Plan stage, there are 3 main purposes for the modeling activity.

- 1) Construct the main drainage pipe network database
- 2) Assess the flow capacity of the main drainage pipes
- 3) Calibration against records of inundated areas during the 1994 flood

Consequently, the modeling shall be concentrated on the main drainage pipes having an equivalent or a larger area of cross-section than that of ϕ 600 mm pipe.

a) Boundary Conditions

Dry-weather flow

Unit wastewater discharge, which is 170 l/day/person for urban district and 140 l/day/person for rural district, is applied to the boundary condition for the calibration.

Rainfall

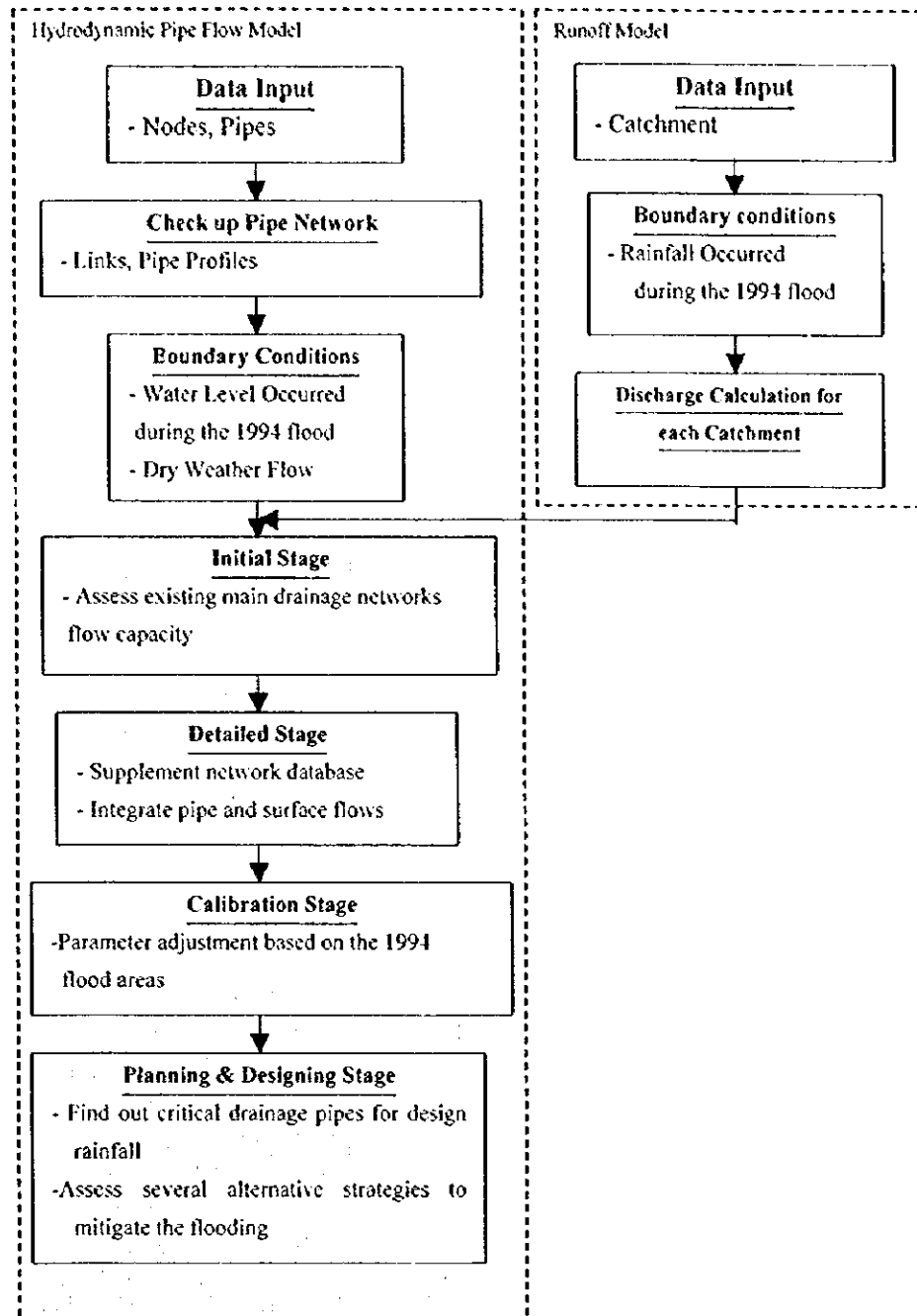
The rainfall time series occurred on June 28th, 1994 will be applied to the boundary condition for the calibration. The rainfall started at 7:00 on June 28, 1994 and continued for 24 hours.

Water Level at the Outlets

The water level at the Phu An station, corresponding to the rainfall time series, is applied to the boundary condition of the water level at the outlet into the Saigon River, the Tau Hu-Ben Nghe, the Doi-Te and the Nhieu Loc-Thi Nghe canals.

b) Procedure for Modeling

A general idea of modeling procedure is shown as the flow chart below.



Essentially, MOUSE consists of 2 components as shown:

- * MOUSE Runoff Model : Compute surface runoff for each catchment. The Catchment is linked to the specified network node (i.e. manhole) of the Pipe Flow Model.
- * MOUSE Hydrodynamic Pipe Flow Model : Compute the drainage network flows based on the runoff discharge calculated by the Runoff Model.

a) General Description of Modeling

Surface Runoff Model

The model computes the runoff discharge for each catchment. The resulting runoff from the catchment is computed on the basis of the specified time of concentration and the runoff coefficients.

Hydrodynamic Pipe Flow Model

The model is a computational tool for simulations of unsteady flows in pipe networks with alternating free surface and pressurized flow conditions. The computation is based on an implicit, finite difference numerical solution of basic 1-D, free surface flow equations (Saint Venant). The implemented algorithm provides efficient and accurate solutions in multiply connected, branched and looped pipe networks.

The computational scheme is applicable to vertically homogeneous flow conditions which occur in low-lying pipes, often pressurized main pipes, affected by the varying water level at the outlet.

Both subcritical and supercritical flows are treated by means of the numerical scheme which adapts according to the local flow conditions. Naturally, flow features such as backwater effects and surcharges are precisely simulated.

Moreover, a advanced technique using this model can simulate the street flooding when the overflow from the node surmounts the inflow capacity of the node.

b) Required Data for Modeling

Required data for modeling are shown in the table below.

Runoff Model	Hydrodynamic Pipe Flow Model	
Catchment	Node (i.e. manhole)	Pipe
- Area	- Bottom Elevation	- Linkage (upstream node and downstream node)
- Concentration Time	- Shape	- Cross-section
- Runoff Coefficient	- Dimension	- Upstream and Downstream Invert Elevation
	- Ground Elevation	- Manning Number

c) Boundary Conditions for Modeling

These models support the following time-variable boundary conditions for simulation:

Runoff Model	Hydrodynamic Pipe Flow Model
- A Rain Hydrograph	- Dry Weather Flow (wastewater from household)
	- Water Levels in the Outlets

9. CALCULATION ON STORAGE REQUIREMENT

Flood Plain Storage along Daihan Canal

Improvement plan of Daihan canal has been proposed such that the proposed canal sections can drain 5-year rain water in 24 hours. In such a case, inundation is expected to occur along the flood plain of Daihan canal. An analysis has been carried out on flood plain storage along Daihan canal for 5-year return period and is presented in Fig. C.9.1. Flood plain storage volume, V_s (m^3) along any reach is calculated as:

$$V_s = (V_i + V_f) - V_o$$

where: V_i = Inflow volume (m^3) from main channel of upper reach;
 V_f = Lateral inflow volume (m^3) from contributing sub-catchment along the reach in consideration and

V_o = Outflow volume (m^3) from main channel of the reach in consideration.

Average specific storage volume and area (considering a 30 cm inundation depth) are calculated to be $29,800 m^3/km^2$ and $99,300 m^2/km^2$ respectively.

Specific Storage Volume due to Urbanization

According to future (2020) landuse plan prepared by Urban Planning Institute of Ho Chi Minh City, rapid urbanization is expected in the North Eastern zone. To keep future runoff (under future landuse condition) same as present runoff (under existing land use condition), storage pond (non-structural measure through landuse regulation) is proposed for the rapidly urbanizing catchments. Required specific storage volume due to increase in urbanization has been estimated. Overland flow runoff hydrographs for 5 and 10 year return periods have been constructed for five landuse conditions for a unit catchment area of $1.0 km^2$ and are presented in Fig. C.9.2. Required specific storage volume ($m^3/s/km^2$) as a function of increase in runoff coefficient for four categories of urbanizations, with reference to agricultural land, have been calculated and the results are plotted as shown in Fig. C.9.3. For ease in application, two equations have been proposed for 5 and 10 year return periods :

$$\begin{aligned} V &= 76852 \times (\Delta C)^{0.76112} && \text{5-year return period} \\ V &= 85679 \times (\Delta C)^{0.76201} && \text{10-year return period} \end{aligned}$$

where: V = required specific storage volume ($m^3/s/km^2$) and
 ΔC = increase in runoff coefficient by urbanization.

10. STRENGTHENING MONITORING NETWORK ON METEO-HYDROLOGY

Proposed Rainfall Stations

In the Study area, there exists only one automatic type rainfall station at Tan Son Nhat where short duration rainfall data is available. There exist six more rainfall stations in and around the Study area which are of manual type and where, only daily rainfall data are available. This posed a great difficulty in performing a detailed analysis on rainfall distribution over the Study area that could support in carrying out a better runoff analysis and developing a better hydrodynamic model. To have a more uniform distribution with smaller representative areas, five new automatic type rainfall stations are proposed to be installed within the Study area. It is also proposed that the existing six rainfall stations shall be updated from manual to automatic type. Locations of the proposed rainfall stations are shown in Fig. C.10.1. It can be seen that areas of Thiessen

polygons with the proposed rainfall stations are much smaller and more uniformly distributed than those with the existing rainfall stations.

Proposed Water Level Stations

There exists no water level station along any of the canals in the Study area. This posed a great difficulty in carrying out a detailed analysis on discharge and water level along the canals as well setting up boundary conditions and calibration for hydrodynamic model development. To overcome this difficulty, nine new automatic type water level stations are proposed to be installed along the canals that best represents the hydrologic system. Among them, six water level stations are proposed in the Western canal system and three water level stations are proposed in the Eastern canal system. Locations of the proposed water level stations are shown in Fig. C.10.2.

Table C.1.1 INVENTORY ON RAINFALL STATIONS

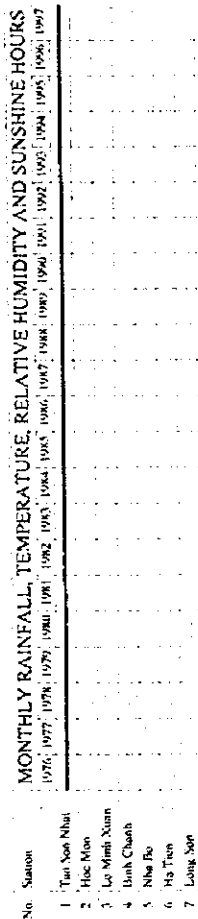
No.	Station		Date of Establishment	Elevation (EL. m)	Geographic Co-ordinates		Method of Measurement		Type of Data	
	Name	Location			Latitude	Longitude	Automatic	Manual	Short Duration	Daily
1	Tan Son Nhat (Tan Son Hoa)	Dist. Tan Binh	1915	9	10°48'	106°41'	X		X	
2	Hoc Mon	Dist. Hoc Mon	1973	10	10°53'	106°36'		X		X
3	Le Minh Xuan	Dist. Binh Chanh	1977		10°45'	106°34'		X		X
4	Binh Chanh	Dist. Binh Chanh	1977		10°40'	106°34'		X		X
5	Nha Be	Dist. Nha Be	1977	1	10°40'	106°44'		X		X
6	Ha Tien Cement Factory	Dist. 9	1963		10°50'	106°41'		X		X
7	Long Son	Dist. Thuan An	1980		10°53'	106°49'		X		X

Table C.1.2 INVENTORY ON WATER LEVEL STATIONS

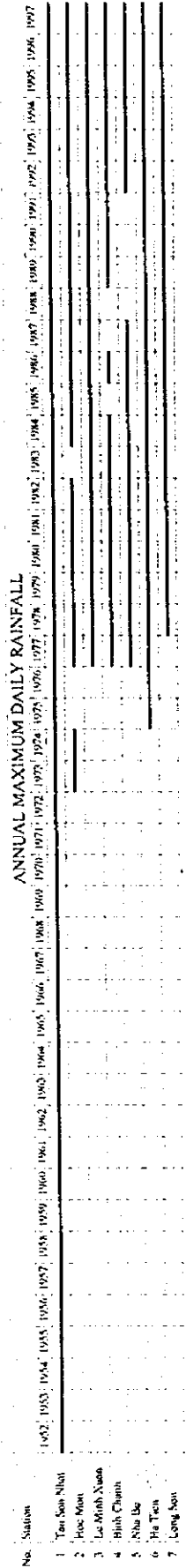
No.	Station Name	Location		Date of Establishment	Geographic Co-ordinates		Method of Measurement	
		River Name	Chainage (km)		Latitude	Longitude	Automatic	Manual
1	Thu Dau Mot	Saigon	107.78 km from East Sea Mouth or 40.41 km from Phu An station	1966	10°58'	106°39'		X
2	Phu An	Saigon	67.36 km from East Sea Mouth or 27.22 km from Nha Be station	1912	10°47'	106°43'	X	
3	Nha Be	Nha Be	40.14 km from East Sea Mouth	1977	10°40'	106°46'	X	
4	Bien Hoa	Dong Nai	88.57 km from East Sea Mouth or 48.44 km from Nha Be station	1960	10°56'	106°49'	X	
5	Ben Luc	East Vam Co (Vam Co Dong)	67.42 km from East Sea Mouth or 42.64 km from Phu An station	1909	10°38'	106°29'		X

Implementation agency of all stations : Southern Region Hydro Meteorological Center, Ho Chi Minh City

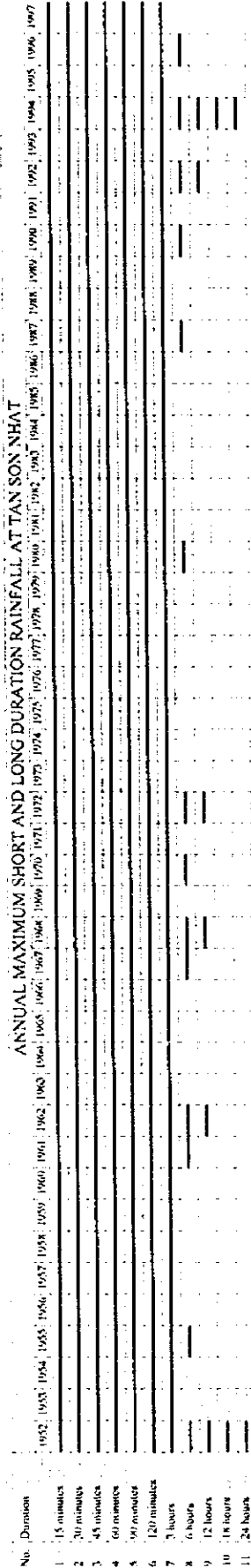
Table C.1.3 BAR CHART OF COLLECTED DATA ON RAINFALL



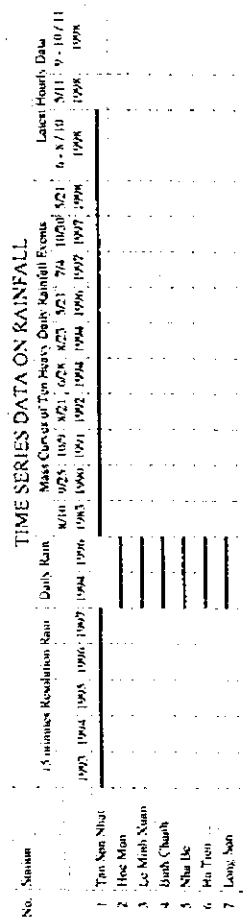
Data Source: Ho Chi Minh City Statistical Office



Data Source: Southern Regional Hydro-Meteorological Center, Ho Chi Minh City



Data Source: Southern Regional Hydro-Meteorological Center, Ho Chi Minh City



Data Source: Southern Regional Hydro-Meteorological Center, Ho Chi Minh City

Table C.3.1 ANNUAL MAXIMUM DAILY RAINFALLS

Unit : mm

Year	Tan Son Nhat		Hoc Mon		Le Minh Xuan		Binh Chanh		Nha Be		Ha Tien Cement Factory		Long Son	
	Amount	Date	Amount	Date	Amount	Date	Amount	Date	Amount	Date	Amount	Date	Amount	Date
1973	89.6	11/07	30.0	11/07										
1974	75.6	27/09	20.0	20/05										
1975	83.4	20/10										95.2	21/10	
1976	81.0	29/10										116.3	16/08	
1977	86.6	07/10	99.6	13/09	83.2	17/07	109.8	29/08	89.4	07/11		95.5	27/06	170.0
1978	176.9	10/10	134.7	22/04	130.4	06/10	144.3	15/09	90.5	11/04		158.5	09/10	81.0
1979	93.6	02/05	146.6	15/06	126.2	16/06	68.2	14/09	58.2	27/04		129.2	05/10	29/08
1980	94.5	18/05	85.5	07/05	88.0	18/05	75.0	01/08	83.5	19/05		120.5	15/05	175.0
1981	99.8	29/06	98.8	24/07	101.1	27/07	58.3	26/07	87.7	12/07		154.5	29/06	103.5
1982	110.8	08/09	88.8	07/09	82.7	07/04	52.4	04/04	46.5	14/06		168.1	08/09	114.0
1983	130.7	10/08			101.5	11/08	122.0	20/09	63.9	30/09		70.3	24/06	116.1
1984	65.7	25/07	80.6	08/10	82.0	12/07	66.3	12/10	71.4	26/07		96.0	06/09	84.0
1985	77.7	10/10	55.0	31/10	91.7	25/07			66.1	01/05		101.0	10/10	94.0
1986	92.5	23/06	83.0	08/10	84.0	24/06	100.5	17/06	90.1	11/05		100.6	13/09	146.0
1987	94.9	11/06	62.3	26/10	97.2	02/11			58.8	12/06		115.8	03/10	60.0
1988	63.6	07/11	79.9	01/11	87.6	25/11						108.0	03/10	70.0
1989	89.5	12/10	124.2	24/04	117.1	16/10	105.9	25/05				100.1	18/08	120.0
1990	84.3	29/09	70.8	25/09	99.3	08/11	74.6	17/08				46.8	20/07	115.0
1991	113.1	09/10	56.1	20/10	83.2	06/06	84.8	26/05				49.5	17/05	70.4
1992	99.9	21/08	104.7	11/06	92.9	21/06	96.7	23/05	129.3	21/08		98.8	18/07	68.0
1993	89.5	03/08	84.0	26/07	83.5	26/10	169.9	06/06	93.3	16/10		79.1	03/09	82.0
1994	162.2	28/06	137.0	04/09	62.3	18/10	184.5	28/06	196.1	28/06		124.6	28/06	159.0
1995	95.3	04/08	92.5	04/08	92.3	11/10	58.3	30/06	79.9	26/08		98.4	29/09	105.5
1996	94.6	21/05	93.5	14/10	94.0	14/10	62.3	14/10	79.3	26/09		67.4	21/05	104.5
1997	110.4	30/10	74.8	21/07	72.0	02/11	73.5	28/10	66.5	04/11		50.0	16/09	111.0
Maximum	176.9	10/10/78	146.6	15/06/79	130.4	06/10/78	184.5	28/06/94	196.1	28/06/94		168.1	08/09/82	175.0

Data Source : Southern Region Hydro Meteorological Center, Ho Chi Minh City
Daily : From 7:00 P.M. to 7:00 P.M.

Table C.3.2 ANNUAL MAXIMUM RAINFALLS AT TAN SON NHAT

Unit: mm

Year	Short Duration Rainfall							Long Duration Rainfall				Daily Rainfall		
	15	30	45	60	90	120	180 (min)		6	12	18	24	(compound rain)	
	(min)	(min)	(min)	(min)	(min)	(min)	Amount	Date	(hrs)	(hrs)	(hrs)	(hrs)	Amount	Date
1952	26.5	27.6	29.8	31.0	31.4	32.8	44.2	06/10	54.4	70.4	76.3	113.4	113.4	20/10
1953	20.0	31.0	42.2	45.8	52.7	56.3	58.8	-					106.5	28/05
1954	30.6	42.4	62.6	82.0	93.7	94.5	95.3	-					110.6	26/09
1955	26.0	38.0	36.8	45.2	50.2	55.6	60.8	06/11	72.1				87.8	06/11
1956	25.2	37.0	45.0	45.4	45.6	45.7	45.8	15/11					70.2	06/09
1957	28.0	36.0	48.0	53.8	57.4	64.4	68.6	22/09					71.6	22/09
1958	23.2	50.3	50.8	50.8	51.8	51.8	52.4	-					52.4	30/06
1959	40.3	50.0	60.0	70.0	79.0	79.0	79.0	30/06					87.0	16/08
1960	27.6	36.6	37.6	37.9	38.2	38.3	38.5	01/04					93.0	20/06
1961	28.0	40.6	50.6	51.0	53.8	55.0	55.0	06/07	68.5				72.0	25/07
1962	22.1	31.8	48.2	53.4	68.0	71.6	74.3	12/09	86.8	116.1			154.6	30/05
1963	25.7	44.0	48.0	55.5	59.4	68.2	73.0	19/06					86.9	19/06
1964	37.5	59.0	83.7	89.3	90.8	92.8	92.8	13/06					101.5	13/06
1965	34.0	49.0	64.6	68.1	75.0	76.5	78.3	30/04					82.5	27/06
1966	28.7	47.5	73.0	85.0	95.0	112.8	116.2	17/08					116.7	17/08
1967	35.0	52.0	72.5	80.0	89.3	90.0	91.5	03/05	100.6				107.5	03/05
1968	25.0	38.7	50.0	57.5	61.0	62.0	63.0	27/04	74.6	93.4			112.9	17/09
1969	30.7	51.4	62.1	77.0	80.3	80.3	80.3	26/10					81.5	26/10
1970	41.2	50.7	70.5	72.5	72.7	73.0	73.0	30/06	84.2				87.5	17/06
1971	30.0	52.0	74.0	78.7	86.0	87.5	88.0	31/05					91.8	31/05
1972	29.0	40.0	52.0	55.0	58.0	58.5	59.5	12/05	73.0	77.8			77.8	25/06
1973	45.0	74.0	81.0	81.8	82.0	82.0	82.0	01/10					89.6	11/07
1974	30.0	36.5	58.0	66.0	72.0	72.5	73.0	27/09					75.6	27/09
1975	25.8	32.1	41.0	54.5	66.0	71.3	80.5	20/10					83.4	20/10
1976	36.5	49.5	67.0	75.0	79.0	79.9	80.9	29/10					83.4	29/10
1977	31.0	38.6	61.0	61.0	65.6	65.6	67.8	13/09					86.6	07/10
1978	31.2	48.8	67.9	75.2	106.7	137.4	144.5	09/10					176.9	10/10
1979	34.0	51.5	85.0	92.0	92.0	93.0	93.2	04/05					93.6	02/05
1980	23.4	46.7	61.2	78.6	81.7	86.7	92.0	18/05	95.4				94.5	18/05
1981	34.2	71.2	76.1	114.3	116.1	116.1	116.9	09/07					99.8	29/06
1982	32.3	57.0	62.2	62.8	64.9	65.1	66.9	19/10					110.8	08/09
1983	27.2	47.7	96.3	99.9	117.0	118.6	127.3	10/08					130.7	10/08
1984	28.8	36.4	44.5	51.7	56.6	62.0	64.2	25/07					65.7	25/07
1985	29.4	42.0	50.5	55.7	60.4	64.4	72.7	09/10					77.7	10/10
1986	34.2	41.7	46.5	46.5	47.5	51.1	51.6	24/09					92.5	23/06
1987	44.6	68.0	83.1	83.7	88.6	91.9	93.5	11/06	94.3				94.9	11/06
1988	28.8	40.3	45.1	45.1	48.8	49.5	49.5	02/10					63.6	07/11
1989	19.4	36.0	48.0	48.0	61.0	61.3	73.2	12/10					89.5	12/10
1990	50.0	69.5	76.6	78.1	78.2	78.2	84.2	03/09	97.4				84.3	29/09
1991	37.8	60.9	68.3	68.3	109.8	112.6	112.7	09/10					113.1	09/10
1992	28.7	58.1	65.7	67.7	73.7	75.9	81.7	21/08	98.1	98.7			99.9	21/08
1993	35.4	52.7	59.8	64.5	65.4	65.9	69.0	03/08					89.5	03/08
1994	37.6	84.5	94.1	102.6	106.4	106.7	108.8	23/08	116.3	154.1	188.8	205.3	162.2	28/06
1995	37.6	65.6	73.5	75.0	76.0	86.5	88.2	06/08					95.3	04/08
1996	25.6	47.5	70.5	70.5	72.0	72.5	88.2	21/05	93.7				94.6	21/05
1997	29.5	57.4	77.5	86.0	98.3	99.5	111.0	04/07					110.4	30/10
Maximum	50.0	84.5	96.3	114.3	117.0	137.4	144.5	9/10/78	116.3	154.1	188.8	205.3	176.9	10/10/78

Short and long duration rainfall : Amount represents annual maximum rainfall for a single rainfall event.

Daily rainfall : Amount represents annual maximum rainfall for compound rainfall (single or multiple rainfall events on same day).

Data Source : Southern Region Hydro Meteorological Center, Ho Chi Minh City

Daily : From 7.00 P.M. to 7.00 P.M.

Table C.3.3 PROBABLE MAXIMUM RAINFALL DEPTHS AT TAN SON NHAT

Unit : mm

Time (minutes)	Probable Rainfall Depths for Different Return Periods												
	1-Yr.	1.5-Yr.	2-Yr.	3-Yr.	5-Yr.	10-Yr.	20-Yr.	25-Yr.	30-Yr.	50-Yr.	70-Yr.	100-Yr.	
15	18.82	27.70	30.06	32.80	35.86	39.70	43.39	44.56	45.51	48.16	49.90	51.74	
30	24.91	41.74	46.21	51.42	57.21	64.50	71.49	73.71	75.51	80.53	83.83	87.31	
45	31.94	53.15	58.78	65.34	72.64	81.82	90.63	93.42	95.69	102.03	106.18	110.57	
60	33.39	57.72	64.18	71.71	80.09	90.62	100.73	103.93	106.54	113.81	118.57	123.61	
90	34.92	62.64	70.00	78.57	88.12	100.12	111.63	115.28	118.25	126.53	131.96	137.70	
120	35.65	64.74	72.72	82.00	92.34	105.33	117.79	121.75	124.96	133.92	139.80	146.01	
180	37.12	67.75	75.88	85.36	95.91	109.17	121.88	125.92	129.20	138.34	144.34	150.68	
360	47.26	77.89	83.72	90.50	98.06	111.42	124.22	128.29	131.60	140.81	146.85	153.24	
daily	51.09	83.58	92.22	102.27	113.47	127.54	141.03	145.31	148.80	158.50	164.86	171.59	

Note: Calculated by applying Gumbel's distribution method using collected data from 1952 to 1997.

Data Source : Southern Region Hydro Meteorological Center, Ho Chi Minh City.

Table C.4.1 PROBABLE MAXIMUM WATER LEVELS BY STATION

Unit: EL. m (Mui Nai)

Station	Probable Maximum Water Level for Different Return Periods											
	10-Yr	15-Yr	2-Yr	3-Yr	5-Yr	10-Yr	20-Yr	25-Yr	30-Yr	50-Yr	70-Yr	100-Yr
Thu Dau Mot	1.23	1.27	1.28 (1.20)	1.29	1.31 (1.23)	1.32 (1.25)	1.34 (1.27)	1.35	1.35	1.36	1.37	1.38 (1.30)
Phu An	1.32	1.40	1.42 (1.35) <1.35>	1.45	1.48 (1.41) <1.41>	1.51 (1.44) <1.44>	1.54 (1.47) <1.46>	1.56	1.56	1.59	1.60	1.62 (1.57) <1.52>
Nha Be	1.40	1.47	1.48 (1.32)	1.50	1.53 (1.41)	1.56 (1.46)	1.58 (1.51)	1.59	1.60	1.62	1.63	1.65 (1.60)
Bien Hoa	1.30	1.55	1.62	1.69	1.78	1.89	1.99	2.02	2.05	2.12	2.17	2.22
Ben Luc	1.08	1.22	1.26	1.30	1.34	1.40	1.46	1.48	1.49	1.53	1.56	1.59

Table C.4.2 PROBABLE MINIMUM WATER LEVELS BY STATION

Unit: EL. m (Mui Nai)

Station	Probable Minimum Water Level for Different Return Periods											
	10-Yr	15-Yr	2-Yr	3-Yr	5-Yr	10-Yr	20-Yr	25-Yr	30-Yr	50-Yr	70-Yr	100-Yr
Thu Dau Mot	-1.76	-1.99	-2.05 (-2.18)	-2.12	-2.20 (-1.99)	-2.29 (-1.87)	-2.39 (-1.76)	-2.42	-2.44	-2.51	-2.56	-2.60 (-1.53)
Phu An	-1.80	-2.03	-2.09 (-2.23) <-2.16>	-2.15	-2.23 (-2.04) <-2.27>	-2.33 (-1.92) <-2.33>	-2.42 (-1.80) <-2.37>	-2.45	-2.48	-2.54	-2.59	-2.63 (-1.55) <-2.45>
Nha Be	-1.98	-2.17	-2.22 (-2.42)	-2.28	-2.35 (-2.29)	-2.43 (-2.15)	-2.51 (-2.06)	-2.54	-2.56	-2.61 (-1.87)	-2.65	-2.69
Bien Hoa	-1.35	-1.56	-1.61	-1.67	-1.74	-1.83	-1.92	-1.94	-1.96	-2.02	-2.06	-2.11
Ben Luc	-1.47	-1.56	-1.58	-1.61	-1.64	-1.67	-1.71	-1.72	-1.73	-1.75	-1.77	-1.79

1.48 : Calculated by the Study Team applying Gumbel's distribution method using collected data from 1960 to 1997, depending upon station.

Data Source : Southern Region Hydro Meteorological Center, Ho Chi Minh City.

(1.41) : The Hydrological Sub-Institute HCMC, 1990; Using Gumbel's method.

<1.41> : Asian Development Bank, 1998, Data ranges from 1960 - 1991.

Table C.4.3 REFERENCE WATER LEVELS BY STATION

Unit: EL. m (Mui Nai)

Station	Reference Water Levels					
	Recorded Historical Highest WL	High Water Level (HWL)	Design Flood Level (DFL)	Mean Water Level (MWL)	Low Water Level (LWL)	Recorded Historical Lowest WL
Thu Dau Mot	1.38	1.29	1.22	0.32	-2.08	-2.34
Phu An	1.54	1.43	1.31	0.21	-2.11	-2.41
Nha Be	1.67	1.49	1.36	0.17	-2.25	-2.57
Bien Hoa	2.02	1.65	1.50	0.66	-1.63	-1.87
Ben Luc	1.54	1.27	1.12	0.32	-1.59	-1.70

Note : Calculated by the Study Team using collected data from 1960 to 1997, depending upon station.

MWL : Data for Phu An ranges from 1993 to 1997 (hourly data) and for other stations, estimated from correlations of mean water levels.

Data Source : Southern Region Hydro Meteorological Center, Ho Chi Minh City.

Criteria for Reference Water Levels

- Highest WL : Recorded historical maximum water level
- HWL = High Water Level : Average of annual maximum water levels
- DFL = Design Flood Level : Average of monthly maximum water levels for the months August to November
- MWL = Mean Water Level : Average of daily mean water levels
- LWL = Low Water Level : Average of annual minimum water levels
- Lowest WL : Recorded historical minimum water level

Table C.4.4 PROBABLE MAXIMUM WATER LEVELS BY REACH

Unit: EL. m (Mui Nai)

Reach	Probable Maximum Water Level for Different Return Periods											
	1.0-Yr.	1.5-Yr.	2-Yr.	3-Yr.	5-Yr.	10-Yr.	20-Yr.	25-Yr.	30-Yr.	50-Yr.	70-Yr.	100-Yr.
Reach 1	1.33	1.42	1.44	1.47	1.50	1.53	1.57	1.58	1.59	1.62	1.63	1.65
Reach 2	1.38	1.48	1.51	1.55	1.58	1.63	1.67	1.69	1.70	1.73	1.75	1.77
Reach 3	1.32	1.53	1.58	1.65	1.72	1.81	1.89	1.92	1.94	2.00	2.04	2.09
Reach 4	1.33	1.42	1.44	1.47	1.50	1.53	1.57	1.58	1.59	1.62	1.63	1.65
Southern Boundaries	1.38	1.48	1.51	1.55	1.58	1.63	1.67	1.69	1.70	1.73	1.75	1.77

Table C.4.5 PROBABLE MINIMUM WATER LEVELS BY REACH

Unit: EL. m (Mui Nai)

Reach	Probable Minimum Water Level for Different Return Periods											
	1.0-Yr.	1.5-Yr.	2-Yr.	3-Yr.	5-Yr.	10-Yr.	20-Yr.	25-Yr.	30-Yr.	50-Yr.	70-Yr.	100-Yr.
Reach 1	-1.80	-2.02	-2.08	-2.15	-2.23	-2.32	-2.42	-2.45	-2.47	-2.54	-2.58	-2.63
Reach 2	-1.94	-2.14	-2.19	-2.26	-2.32	-2.41	-2.49	-2.52	-2.54	-2.60	-2.64	-2.68
Reach 3	-1.85	-2.06	-2.12	-2.19	-2.26	-2.35	-2.44	-2.47	-2.50	-2.56	-2.60	-2.65
Reach 4	-1.80	-2.02	-2.08	-2.15	-2.23	-2.32	-2.42	-2.45	-2.47	-2.54	-2.58	-2.63
Southern Boundaries	-1.94	-2.14	-2.19	-2.26	-2.32	-2.41	-2.49	-2.52	-2.54	-2.60	-2.64	-2.68

Table C.4.6 REFERENCE WATER LEVELS BY REACH

Unit: EL. m (Mui Nai)

Reach	Reference Water Levels					
	Recorded Historical Highest WL	High Water Level (HWL)	Design Flood Level (DFL)	Mean Water Level (MWL)	Low Water Level (LWL)	Recorded Historical Lowest WL
Reach 1	1.56	1.45	1.32	0.23	-2.11	-2.40
Reach 2	1.75	1.52	1.39	0.27	-2.12	-2.42
Reach 3	1.94	1.61	1.47	0.54	-1.78	-2.04
Reach 4	1.56	1.45	1.32	0.23	-2.11	-2.40
Southern Boundaries	1.75	1.52	1.39	0.27	-2.12	-2.42

Reach 1 : From confluence point of Rach Ba Hong with Song Saigon to confluence point of Kinh Te with Song Saigon (33.97 km)

Reach 2 : From confluence point of Kinh Te with Song Saigon to confluence point of Song Muong Chuoi with Song Nha Be (27.83 km).

Reach 3 : From confluence point of Song Saigon with Song Nha Be to (northern) confluence point of Song Tac with Song Dong Nai (26.33 km).

Reach 4 : From confluence point of Kinh Te (and Kinh Ben Nghe) with Song Saigon to confluence point of Rach Ba Goc with Song Ben Luc (15.00 km).

Southern Boundaries : The southern boundaries of Rach Can Giuoc, Rach Ba Lao etc. flowing towards the south.

Table C.6.1 BASIN AREAS BY ZONES, CATCHMENTS AND SUB-CATCHMENTS

Zone		Catchment		Sub-Catchment			
ID	Area (km ²)	ID	Area (km ²)	ID	Area (km ²)		
N	13618	N 1	1987	N11	333		
				N12	660		
				N13	425		
				N14	569		
		N 2	10757	N 2	10757	N21	1167
						N22	643
						N23	1124
						N24	532
						N25	1098
						N26	1119
						N27	743
						N28	1393
						N29	632
						N210	554
						N211	571
N212	489						
N213	689						
Na	875	Na	875				
C	10641	C 1	3167	C11	754		
				C12	355		
				C13	587		
				C14	758		
				C15	713		
		C 2	514	C 2	514		
		C 3	2022	C 3	2022	C31	586
						C32	472
						C33	377
						C34	588
		C 4	4150	C 4	4150	C41	184
						C42	288
						C43	341
						C44	311
						C45	524
C46	751						
C47	664						
C48	1086						
Ca	491	Ca	491				
Cb	128	Cb	128				
Cc	168	Cc	168				
W	7291	W 1	7291	W11	968		
				W12	514		
				W13	473		
				W14	862		
				W15	820		
				W16	320		
				W17	370		
				W18	604		
				W19	099		
				W110	027		
				W111	855		
				W112	1350		
S	8174	S 1	1433	S11	268		
				S12	404		
				S13	438		
				S14	196		
				S15	127		
		S 2	1566	S 2	1566	S21	240
						S22	196
						S23	285
						S24	845
		S 3	3151	S 3	3151	S31	217
						S32	232
						S33	479
						S34	575
						S35	431
						S36	434
S37	645						
S38	438						
S4	236					S4	236
S5	223					S5	223
Sa	346	Sa	346				
Sb	386	Sb	386				
Sc	533	Sc	533				
SE	11936	SE 1	198	SE1	198		
				SE2	260		
				SE3	192		
		SE 4	780	SE4	540		
				SE42	240		
		SE 5	383	SE5	383		
				SE6	510		
		SE 7	1458	SE7	839		
				SE72	620		
		SE 8	1133	SE8	277		
				SE82	856		
		SE 9	2141	SE9	746		
				SE92	626		
				SE93	738		
		SE 10	2488	SE10	757		
SE102	1143						
SE103	588						
SEa	367	SEa	367				
SEb	516	SEb	516				
SEc	182	SEc	182				
SEd	130	SEd	130				
SEe	277	SEe	277				
SEf	952	SEf	952				
NE	6491	NE 1	332	NE1	332		
				NE2	478		
				NE22	475		
				NE3	715		
		NE 4	265	NE4	265		
				NE5	3438		
		NE 5	3438	NE51	1012		
				NE52	472		
				NE53	316		
				NE54	1017		
				NE55	621		
		NEa	376	NEa	376		
		NEb	250	NEb	250		
		NEc	162	NEc	162		
						Total	

Measurement : GIS database
Scale = 1/10,000

Table C.6.2 EXISTING LAND USE AREAS BY SUB-CATCHMENT (1/2)

Zone	Catchment ID	Sub-Catchment		Landuse Area by Category (km ²)									Runoff Coefficient
		ID	Area (km ²)	Com	Ind	High R	Med R	Low R	Inst	Green	Agri	Water	
Northern Zone	N1	N1.1	3.33	0.00	0.13	0.00	0.22	0.00	0.00	0.00	2.96	0.01	0.44
		N1.2	6.60	0.00	0.16	0.00	1.79	0.00	0.00	0.00	4.60	0.05	0.50
		N1.3	4.25	0.00	0.00	0.00	0.37	0.00	0.00	0.00	3.82	0.06	0.43
		N1.4	5.69	0.00	0.00	0.00	0.22	0.00	0.00	0.00	5.26	0.22	0.43
	N2	N2.1	11.67	0.00	0.08	0.00	3.59	0.00	0.00	0.00	8.60	0.00	0.50
		N2.2	6.43	0.00	0.00	0.00	1.93	0.00	0.00	0.00	4.50	0.00	0.49
		N2.3	11.24	0.00	0.00	0.00	2.52	0.00	0.11	0.00	8.60	0.01	0.47
		N2.4	5.32	0.00	0.05	0.04	1.17	0.00	0.15	0.00	3.84	0.05	0.49
		N2.5	10.98	0.00	0.14	1.11	0.87	0.00	0.22	0.00	8.50	0.14	0.48
		N2.6	11.79	0.00	0.36	2.59	0.29	0.00	6.67	0.00	0.82	0.25	0.66
		N2.7	7.48	0.00	0.18	0.89	1.28	0.00	1.14	0.00	3.93	0.05	0.55
		N2.8	13.93	0.00	0.35	3.03	2.05	0.00	0.59	0.21	7.55	0.15	0.56
		N2.9	6.32	0.00	0.19	0.00	1.34	0.00	0.02	0.00	4.47	0.10	0.50
		N2.10	5.51	0.00	0.20	0.00	1.18	0.00	0.00	0.00	4.06	0.11	0.49
N2.11	5.71	0.00	0.01	1.36	0.24	0.00	0.64	0.00	2.89	0.57	0.59		
N2.12	4.89	0.00	0.03	0.58	0.52	0.00	0.01	0.00	3.52	0.23	0.51		
N2.13	6.89	0.05	0.07	1.64	0.75	0.00	1.51	0.00	3.21	0.26	0.58		
Na	Na	8.75	0.00	0.06	0.00	2.09	0.00	0.02	0.00	6.45	0.12	0.48	
Central Zone	C1	C1.1	7.54	0.06	0.28	5.61	0.00	0.00	1.45	0.06	0.00	0.08	0.76
		C1.2	3.55	0.11	0.01	2.34	0.00	0.00	0.70	0.08	0.00	0.10	0.75
		C1.3	5.87	0.36	0.09	4.13	0.00	0.00	1.12	0.12	0.00	0.03	0.76
		C1.4	7.58	0.23	0.36	4.49	0.00	0.00	1.69	0.56	0.14	0.12	0.72
		C1.5	7.13	0.05	0.18	5.79	0.00	0.00	0.50	0.24	0.06	0.30	0.78
	C2	C2	5.14	0.04	0.32	3.41	0.00	0.00	0.04	0.00	0.96	0.37	0.74
	C3	C3.1	5.85	0.00	0.63	4.88	0.00	0.00	0.09	0.00	0.20	0.07	0.80
		C3.2	4.72	0.00	0.49	2.82	0.10	0.00	0.09	0.21	0.83	0.20	0.72
		C3.3	3.77	0.00	0.15	1.98	0.00	0.00	0.07	0.00	1.24	0.33	0.69
		C3.4	5.88	0.00	0.16	0.27	0.70	0.00	0.00	0.00	4.54	0.21	0.49
	C4	C4.1	1.84	0.00	0.10	0.63	0.00	0.00	0.03	0.00	0.56	0.52	0.71
		C4.2	2.88	0.00	0.03	0.00	0.00	0.00	0.01	0.00	2.66	0.19	0.49
		C4.3	3.41	0.00	0.00	0.00	0.81	0.00	0.00	0.00	2.50	0.11	0.48
		C4.4	3.11	0.00	0.01	0.07	0.21	0.00	0.00	0.00	2.58	0.24	0.45
		C4.5	5.24	0.31	0.51	3.43	0.00	0.00	0.11	0.00	0.44	0.44	0.79
		C4.6	7.51	0.31	0.38	4.62	0.02	0.00	0.99	0.01	0.71	0.47	0.75
		C4.7	6.64	0.14	0.16	4.28	0.03	0.00	0.72	0.01	0.62	0.69	0.76
		C4.8	10.86	0.57	0.92	4.71	0.69	0.00	0.81	0.54	0.95	1.68	0.76
Ca	Ca	4.91	0.00	0.01	1.06	0.00	0.00	0.00	0.00	3.62	0.15	0.51	
Cb	Cb	1.28	0.00	0.32	0.72	0.00	0.00	0.08	0.00	0.08	0.08	0.80	
Cc	Cc	1.68	0.48	0.10	0.00	0.00	0.00	0.94	0.12	0.00	0.04	0.66	
Western Zone	W1	W1.1	9.68	0.01	0.27	3.96	0.76	0.00	0.17	0.00	4.44	0.06	0.61
		W1.2	5.14	0.00	0.00	0.00	0.50	0.00	0.00	0.00	4.58	0.06	0.44
		W1.3	4.73	0.00	0.00	0.00	0.12	0.00	0.00	0.00	4.52	0.09	0.42
		W1.4	8.62	0.00	0.10	0.02	1.88	0.00	0.00	0.00	6.60	0.03	0.47
		W1.5	8.20	0.00	0.04	0.00	0.68	0.00	0.00	0.00	7.37	0.14	0.44
		W1.6	3.20	0.00	0.34	0.03	0.84	0.00	0.02	0.00	1.93	0.03	0.54
		W1.7	3.70	0.00	0.12	0.60	0.04	0.00	0.00	0.00	2.84	0.10	0.50
		W1.8	6.04	0.00	0.01	0.00	0.56	0.00	0.00	0.00	5.33	0.13	0.44
		W1.9	0.99	0.00	0.06	0.06	0.05	0.00	0.00	0.00	0.72	0.10	0.53
		W1.10	0.27	0.00	0.00	0.09	0.00	0.00	0.00	0.00	0.05	0.13	0.82
		W1.11	8.85	0.00	0.00	0.46	1.35	0.00	0.00	0.00	6.75	0.30	0.49
		W1.12	13.50	0.00	0.00	0.00	1.21	0.00	0.03	0.00	11.64	0.63	0.46
Southern Zone	S1	S1.1	2.68	0.00	0.00	0.00	0.39	0.00	0.00	0.00	2.09	0.20	0.49
		S1.2	4.04	0.00	0.00	0.00	0.34	0.00	0.00	0.00	3.39	0.30	0.47
		S1.3	4.38	0.00	0.00	0.01	0.07	0.00	0.00	0.00	3.86	0.43	0.47
		S1.4	1.96	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.81	0.15	0.45
		S1.5	1.27	0.00	0.00	0.00	0.04	0.00	0.01	0.00	1.06	0.16	0.49
	S2	S2.1	2.40	0.00	0.00	0.00	0.26	0.00	0.00	0.00	1.88	0.26	0.50
		S2.2	1.96	0.00	0.00	0.03	0.40	0.00	0.00	0.00	1.41	0.14	0.50
		S2.3	2.85	0.00	0.00	0.00	0.14	0.00	0.60	0.00	2.33	0.38	0.49
S2.4	8.45	0.00	0.03	0.00	0.64	0.00	0.03	0.00	7.13	0.62	0.47		

Table C.6.2 EXISTING LAND USE AREAS BY SUB-CATCHMENT (2/2)

Zone	Catchment	Sub-Catchment		Land Use Area by Category (km ²)								Runoff Coefficient	
		ID	Area (km ²)	Com.	Ind.	High R.	Med R.	Low R.	Inst.	Green	Agri.		Water
Southern Zone	S3	S3.1	2.17	0.00	0.00	0.00	0.67	0.00	0.00	0.00	1.33	0.15	0.53
		S3.2	2.32	0.00	0.00	0.00	0.15	0.00	0.02	0.00	1.94	0.21	0.48
		S3.3	4.79	0.00	0.00	0.00	0.60	0.00	0.13	0.00	3.33	0.72	0.53
		S3.4	5.75	0.00	0.00	0.00	0.30	0.00	0.00	0.00	4.91	0.54	0.47
		S3.5	4.31	0.00	0.00	0.00	0.08	0.00	0.01	0.00	3.76	0.43	0.47
		S3.6	4.34	0.00	0.00	0.00	0.73	0.00	0.04	0.00	2.97	0.60	0.54
		S3.7	6.45	0.00	0.00	0.00	0.23	0.00	0.00	0.00	5.63	0.60	0.47
		S3.8	4.38	0.00	0.00	0.00	0.57	0.00	0.00	0.00	3.20	0.61	0.52
	S4	S4	2.56	0.00	0.81	0.00	0.60	0.00	0.00	0.00	0.82	0.14	0.68
	S5	S5	2.23	0.00	0.18	0.00	0.00	0.00	0.00	0.00	1.96	0.09	0.46
	Sa	Sa	3.46	0.00	2.03	0.00	0.24	0.00	0.00	0.00	1.04	0.16	0.74
	Sb	Sb	3.88	0.00	0.14	0.00	0.10	0.00	0.18	0.00	3.19	0.24	0.47
Sc	Sc	5.33	0.00	0.13	0.00	0.92	0.00	0.12	0.00	3.89	0.26	0.50	
North-Eastern Zone	NE1	NE1	3.32	0.00	0.01	0.00	0.33	0.00	0.00	0.00	2.73	0.05	0.46
	NE2	NE2.1	4.78	0.00	0.03	0.00	0.88	0.00	0.05	0.00	3.99	0.04	0.47
		NE2.2	4.73	0.00	0.00	0.00	1.01	0.00	0.00	0.00	3.66	0.07	0.47
	NE3	NE3	7.15	0.00	0.02	0.00	2.02	0.00	0.35	0.00	4.69	0.06	0.50
	NE4	NE4	2.65	0.00	0.18	0.00	1.04	0.00	0.28	0.00	1.14	0.01	0.57
	NE5	NE5.1	10.12	0.00	0.08	0.00	6.34	0.00	0.00	0.00	9.68	0.02	0.42
		NE5.2	4.72	0.00	0.16	0.00	0.58	0.00	0.00	0.00	3.94	0.03	0.46
		NE5.3	3.16	0.00	0.55	0.00	0.27	0.00	0.03	0.00	2.26	0.04	0.52
		NE5.4	10.17	0.00	0.23	0.00	1.24	0.00	0.47	0.00	8.20	0.03	0.46
		NE5.5	6.21	0.00	0.00	0.00	0.70	0.00	0.48	0.00	4.87	0.17	0.47
	NEa	NEa	3.76	0.00	0.03	0.00	0.14	0.00	0.02	0.00	3.42	0.15	0.44
	NEb	NEb	2.50	0.00	0.07	0.00	0.06	0.00	0.02	0.00	2.28	0.06	0.44
	NEc	NEc	1.62	0.00	0.49	0.00	0.00	0.00	0.00	0.00	0.83	0.30	0.66
South-Eastern Zone	SE1	SE1	1.98	0.00	0.04	0.00	0.18	0.00	0.00	0.00	1.68	0.08	0.46
	SE2	SE2	2.60	0.00	0.03	0.00	0.17	0.00	0.00	0.00	2.33	0.06	0.44
	SE3	SE3	1.92	0.00	0.01	0.00	0.13	0.00	0.00	0.00	1.60	0.19	0.48
	SE4	SE4.1	5.40	0.00	0.00	0.00	1.27	0.00	0.00	0.00	3.92	0.21	0.49
		SE4.2	2.40	0.00	0.00	0.00	0.27	0.00	0.00	0.00	1.98	0.75	0.47
	SE5	SE5	3.83	0.00	0.00	0.00	0.58	0.00	0.00	0.00	3.18	0.07	0.46
	SE6	SE6	5.70	0.00	0.03	0.00	0.64	0.00	0.08	0.00	4.24	0.33	0.46
	SE7	SE7.1	8.39	0.00	0.12	0.00	1.22	0.00	0.04	0.00	6.83	0.17	0.46
		SE7.2	6.20	0.00	0.19	0.00	0.74	0.00	0.12	0.00	4.79	0.36	0.49
	SE8	SE8.1	2.77	0.00	0.00	0.00	0.03	0.00	0.00	0.00	2.68	0.06	0.42
		SE8.2	8.56	0.00	0.00	0.00	1.03	0.00	0.00	0.00	7.25	0.28	0.46
	SE9	SE9.1	7.36	0.00	0.02	0.00	1.20	0.00	0.36	0.00	5.76	0.12	0.47
		SE9.2	6.26	0.00	0.00	0.00	0.32	0.00	0.00	0.00	5.73	0.22	0.44
		SE9.3	7.38	0.00	0.00	0.00	0.62	0.00	0.00	0.00	6.51	0.25	0.45
	SE10	SE10.1	7.57	0.00	0.00	0.00	0.00	0.00	0.08	0.00	6.68	0.81	0.47
		SE10.2	11.43	0.00	0.00	0.00	0.62	0.00	0.00	0.00	9.85	0.96	0.47
		SE10.3	5.88	0.00	0.00	0.00	0.28	0.00	0.00	0.00	5.17	0.43	0.46
	SEa	SEa	3.67	0.00	0.00	0.00	1.09	0.00	0.00	0.00	1.83	0.75	0.61
	SEb	SEb	5.16	0.00	0.00	0.00	1.00	0.00	0.00	0.00	3.99	0.17	0.48
SEc	SEc	1.82	0.00	0.00	0.00	0.16	0.00	0.00	0.00	1.62	0.03	0.44	
SEd	SEd	1.50	0.00	0.07	0.00	0.00	0.00	0.37	0.00	0.69	0.17	0.56	
SEe	SEe	2.77	0.00	0.00	0.00	0.00	0.00	0.00	0.00	2.51	0.26	0.46	
SEf	SEf	9.52	0.00	0.00	0.00	0.42	0.00	0.00	0.00	8.99	0.12	0.42	
		Total (km ²)	581.51	2.72	14.02	71.98	60.83	0.00	24.12	2.14	380.66	25.04	
		Percent (%)	100.00	0.47	2.41	12.38	10.46	0.00	4.15	0.37	65.46	4.31	

Com. : Commercial Areas Inst. : Institutional Areas
 Ind. : Industrial Areas Green : Green Spaces
 High R. : High Residential Areas Agri. : Agricultural Areas
 Med R. : Medium Residential Areas Water : Water Bodies
 Low R. : Low Residential Areas

Data Source : Urban Planning Institute, Ho Chi Minh City (UPI, HCMC)
 Calculation : GIS database : Inner City : Scale = 1/10,000
 Outer City : Scale = 1/25,000

Table C.6.3 FUTURE (2020) LAND USE AREAS BY SUB-CATCHMENT (1/2)

Zone	Catchment ID	Sub-Catchment		Landuse Area by Category (km ²)										Runoff Coefficient
		ID	Area (km ²)	Com.	Ind.	High R.	Med R.	Low R.	Inst.	Green	Agri.	Water		
Northern Zone	N1	N1.1	3.33	0.00	0.90	0.00	0.12	0.06	0.00	1.51	0.72	0.01	0.51	
		N1.2	6.60	0.00	2.07	0.00	0.64	0.23	0.26	2.18	1.17	0.03	0.57	
		N1.3	4.25	0.00	0.00	0.00	0.88	0.09	0.07	1.94	1.20	0.06	0.43	
		N1.4	5.69	0.00	0.00	0.00	1.52	0.00	0.00	1.09	2.87	0.22	0.43	
	N2	N2.1	11.67	0.00	0.07	0.00	2.58	1.85	0.00	2.87	4.31	0.00	0.43	
		N2.2	6.43	0.00	0.00	0.00	0.41	0.65	0.13	0.19	5.05	0.00	0.41	
		N2.3	11.24	0.00	1.83	1.54	0.67	0.40	0.08	2.03	4.68	0.01	0.54	
		N2.4	5.32	0.00	1.00	1.82	0.70	0.00	0.00	1.74	0.00	0.03	0.64	
		N2.5	10.98	0.00	0.08	4.39	2.30	0.58	0.18	2.13	1.19	0.14	0.63	
		N2.6	11.19	0.00	0.15	3.15	0.00	0.00	7.14	0.49	0.00	0.25	0.66	
		N2.7	7.48	0.00	0.00	3.74	0.29	0.65	1.70	1.04	0.00	0.03	0.66	
		N2.8	13.93	0.00	0.00	6.49	2.25	1.86	1.18	1.98	0.00	0.13	0.67	
		N2.9	6.32	0.00	0.27	0.00	1.66	0.70	0.26	3.32	0.00	0.10	0.49	
		N2.10	5.54	0.00	1.39	0.00	1.52	0.00	0.00	2.52	0.00	0.11	0.57	
		N2.11	5.71	0.00	0.00	2.29	1.70	0.00	0.25	0.91	0.00	0.03	0.57	
N2.12	4.89	0.00	0.00	1.23	2.03	0.00	0.11	1.24	0.03	0.23	0.63			
N2.13	6.89	0.00	0.00	5.15	0.23	0.00	0.16	1.09	0.00	0.26	0.72			
Na	Na	8.75	0.00	0.23	0.00	4.02	0.00	0.43	3.91	0.00	0.12	0.52		
Central Zone	C1	C1.1	7.54	0.00	0.00	5.96	0.00	0.00	1.47	0.04	0.00	0.03	0.76	
		C1.2	3.53	0.00	0.00	3.16	0.00	0.00	0.00	0.29	0.00	0.10	0.77	
		C1.3	5.87	0.00	0.00	5.58	0.00	0.00	0.11	0.13	0.00	0.03	0.79	
		C1.4	7.58	0.00	0.00	5.61	0.00	0.00	1.09	0.76	0.00	0.12	0.72	
		C1.5	7.13	0.00	0.00	6.15	0.00	0.00	0.46	0.21	0.00	0.30	0.78	
	C2	C2	5.14	0.00	0.00	4.47	0.00	0.00	0.00	0.30	0.00	0.37	0.78	
		C2.1	5.86	0.00	0.00	5.64	0.00	0.00	0.00	0.15	0.00	0.07	0.79	
	C3	C3.2	4.72	0.00	0.00	3.36	0.00	0.00	0.00	1.16	0.00	0.20	0.69	
		C3.3	3.77	0.00	0.00	3.42	0.00	0.00	0.02	0.00	0.00	0.33	0.82	
		C3.4	5.88	0.00	0.03	4.18	0.00	0.00	0.00	1.40	0.04	0.21	0.69	
		C3.1	1.84	0.00	0.00	1.31	0.00	0.00	0.00	0.01	0.00	0.52	0.85	
	C4	C4.2	2.88	0.00	0.00	1.15	0.93	0.00	0.00	0.61	0.00	0.19	0.62	
		C4.3	3.41	0.00	0.00	0.00	2.56	0.00	0.00	0.74	0.00	0.11	0.53	
		C4.4	3.11	0.00	0.00	0.04	0.62	0.00	0.71	0.96	0.53	0.24	0.67	
		C4.5	5.24	0.00	0.00	2.77	0.00	0.00	2.03	0.00	0.00	0.44	0.74	
		C4.6	7.51	0.00	0.00	3.24	0.01	0.00	3.07	0.71	0.00	0.47	0.68	
		C4.7	6.64	0.00	0.00	3.10	0.00	0.00	2.75	0.10	0.00	0.69	0.73	
		C4.8	10.88	0.00	0.36	3.67	0.53	0.29	3.28	1.03	0.00	1.68	0.72	
C4.1		4.91	0.00	0.00	2.03	0.00	0.00	0.07	2.65	0.00	0.15	0.53		
Cb	Cb	1.28	0.00	0.00	0.75	0.00	0.00	0.00	0.42	0.04	0.08	0.64		
Cc	Cc	1.68	0.00	0.00	0.00	0.00	0.00	1.53	0.11	0.00	0.04	0.59		
Western Zone	W1	W1.1	9.68	0.00	0.03	6.07	0.58	0.28	0.38	2.26	0.01	0.06	0.66	
		W1.2	5.14	0.00	0.00	0.00	0.60	0.41	0.00	0.19	3.58	0.66	0.45	
		W1.3	4.73	0.00	0.00	0.00	0.00	0.08	0.00	0.00	4.56	0.09	0.41	
		W1.4	8.62	0.00	0.08	1.93	1.61	0.62	0.15	4.13	0.07	0.03	0.52	
		W1.5	8.20	0.00	2.64	0.85	0.16	0.84	0.99	0.98	1.62	0.11	0.65	
		W1.6	3.20	0.00	0.16	1.99	0.33	0.00	0.01	0.69	0.00	0.03	0.69	
		W1.7	3.70	0.00	0.92	0.00	1.69	0.45	0.03	0.51	0.01	0.10	0.69	
		W1.8	6.04	0.00	0.00	0.00	0.49	0.21	0.00	0.00	5.20	0.13	0.44	
		W1.9	0.99	0.00	0.73	0.00	0.00	0.00	0.00	0.15	0.00	0.10	0.82	
		W1.10	0.27	0.00	0.13	0.00	0.00	0.00	0.00	0.02	0.00	0.13	0.91	
		W1.11	8.85	0.00	1.04	1.08	1.25	0.07	0.87	2.34	1.92	0.30	0.56	
		W1.12	13.50	0.00	0.96	0.39	1.48	0.69	0.37	1.75	7.21	0.63	0.51	
Southern Zone	S1	S1.1	2.68	0.00	0.25	0.00	0.92	0.00	0.00	0.66	0.66	0.20	0.57	
		S1.2	4.04	0.00	0.00	0.00	0.99	0.06	0.00	1.06	1.62	0.30	0.50	
		S1.3	4.38	0.00	0.43	0.00	0.85	0.00	0.21	1.26	1.19	0.43	0.55	
		S1.4	1.96	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.81	0.15	0.45	
		S1.5	1.27	0.00	0.00	0.00	0.00	0.00	0.00	0.05	1.06	0.16	0.47	
	S2	S2.1	2.40	0.00	0.00	0.00	0.58	0.07	0.00	1.49	0.00	0.26	0.48	
		S2.2	1.96	0.00	0.00	0.00	0.85	0.00	0.51	0.49	0.00	0.11	0.59	
		S2.3	2.85	0.00	0.00	0.00	0.57	0.00	0.00	1.32	0.58	0.38	0.49	
S2.4	8.45	0.00	0.92	0.00	1.55	0.07	0.00	0.61	4.67	0.62	0.55			

Table C.6.3 FUTURE (2020) LAND USE AREAS BY SUB-CATCHMENT (2/2)

Zone	Catchment	Sub-Catchment		Landuse Area by Category (km ²)										Runoff Coefficient
		ID	Area (km ²)	Com.	Ind	High R.	Med R.	Low R.	Inst	Green	Agri.	Water		
Southern Zone	S3	S31	217	0.00	0.00	0.79	0.59	0.42	0.12	0.10	0.00	0.13	0.71	
		S32	232	0.00	0.00	1.73	0.00	0.00	0.00	0.38	0.00	0.21	0.74	
		S33	479	0.00	0.00	1.77	0.62	1.56	0.00	0.11	0.00	0.72	0.74	
		S34	575	0.00	0.00	0.38	2.13	0.01	0.22	2.47	0.00	0.54	0.56	
		S35	431	0.00	0.00	1.00	1.65	0.07	0.15	1.58	0.00	0.45	0.60	
		S36	434	0.00	0.00	0.00	2.33	0.33	0.00	1.08	0.00	0.60	0.63	
		S37	645	0.00	0.00	0.00	2.75	0.00	0.05	3.05	0.00	0.60	0.54	
		S38	438	0.00	0.00	0.00	2.33	0.31	0.23	0.90	0.00	0.61	0.63	
	S4	S4	236	0.00	1.04	0.26	0.00	0.00	0.09	0.84	0.00	0.14	0.67	
	S5	S5	223	0.00	0.61	0.49	0.00	0.00	0.00	1.04	0.00	0.09	0.60	
	Sa	Sa	348	0.00	2.75	0.00	0.00	0.00	0.00	0.55	0.00	0.16	0.81	
	Sb	Sb	385	0.00	0.92	0.47	0.00	0.00	0.23	2.00	0.00	0.24	0.57	
	Sc	Sc	533	0.00	1.13	0.00	0.27	1.40	0.00	2.26	0.00	0.25	0.56	
North-Eastern Zone	NE1	NE1	332	0.00	0.47	0.00	1.46	0.00	0.00	1.34	0.00	0.05	0.57	
	NE2	NE21	478	0.00	0.00	0.00	3.70	0.73	0.28	0.04	0.00	0.04	0.68	
		NE22	475	0.00	0.00	0.26	2.68	0.77	0.09	0.85	0.02	0.07	0.62	
	NE3	NE3	715	0.00	0.17	1.35	1.63	0.49	2.62	0.82	0.01	0.06	0.64	
	NE4	NE4	265	0.00	0.00	1.76	0.31	0.16	0.25	0.17	0.00	0.01	0.73	
	NE5	NE51	1012	0.00	1.09	0.00	0.00	0.80	4.89	3.32	0.00	0.02	0.53	
		NE52	472	0.00	1.98	0.00	0.17	0.65	0.32	1.57	0.00	0.03	0.63	
		NE53	316	0.00	1.71	0.72	0.00	0.00	0.44	0.25	0.00	0.04	0.79	
		NE54	1017	0.67	0.88	0.23	1.34	1.07	1.97	3.97	0.00	0.03	0.59	
		NE55	621	0.00	1.43	0.00	0.36	0.35	0.40	3.52	0.00	0.17	0.52	
	NEa	NEa	376	0.00	0.21	0.00	1.32	0.00	0.44	1.65	0.00	0.15	0.54	
	NEb	NEb	250	0.00	0.00	0.00	0.81	0.00	0.21	1.41	0.00	0.06	0.47	
	NEc	NEc	162	0.00	0.47	0.25	0.00	0.22	0.00	0.37	0.00	0.30	0.72	
South-Eastern Zone	SE1	SE1	198	0.00	0.00	0.25	0.00	0.00	0.73	0.92	0.00	0.08	0.50	
	SE2	SE2	260	0.00	0.00	1.71	0.00	0.00	0.00	0.82	0.00	0.06	0.65	
	SE3	SE3	192	0.00	0.00	1.57	0.00	0.00	0.10	0.05	0.00	0.19	0.80	
	SE4	SE41	540	0.00	0.00	0.00	1.12	0.09	0.29	3.70	0.00	0.21	0.43	
		SE42	240	0.00	0.00	0.41	0.00	0.00	0.32	1.52	0.00	0.15	0.47	
	SE5	SE5	383	0.00	0.19	2.31	0.00	0.00	0.01	1.25	0.00	0.07	0.65	
	SE6	SE6	510	0.00	3.79	0.00	0.20	0.36	0.00	0.63	0.00	0.13	0.80	
	SE7	SE71	839	0.75	0.00	0.00	3.76	0.68	0.00	3.02	0.00	0.17	0.56	
		SE72	620	0.00	0.02	0.02	1.77	0.99	0.37	2.67	0.00	0.36	0.52	
	SE8	SE81	277	0.00	0.00	0.00	1.03	0.00	0.00	1.70	0.00	0.06	0.46	
		SE82	856	0.00	2.28	0.00	1.00	0.40	0.08	4.52	0.00	0.28	0.55	
	SE9	SE91	746	0.63	1.10	1.69	1.06	0.35	0.50	2.01	0.00	0.12	0.65	
		SE92	626	0.00	0.00	1.29	0.50	0.17	0.41	3.67	0.00	0.22	0.49	
		SE93	738	0.00	0.00	0.00	3.52	0.00	0.23	3.37	0.00	0.25	0.52	
	SE10	SE101	757	0.00	0.00	0.00	0.85	0.00	0.18	5.72	0.00	0.81	0.43	
		SE102	1143	0.00	0.49	2.69	1.70	0.03	0.59	4.55	0.01	0.96	0.58	
		SE103	588	0.00	0.00	0.00	0.35	0.26	0.23	4.61	0.00	0.43	0.40	
	SEa	SEa	367	0.00	0.00	2.25	0.00	0.00	0.13	0.53	0.00	0.75	0.76	
	SEb	SEb	516	0.00	0.00	0.53	0.00	0.00	3.45	0.93	0.00	0.17	0.58	
	SEc	SEc	182	0.00	0.00	0.00	0.00	0.00	0.00	1.79	0.00	0.03	0.31	
SEd	SEd	130	0.00	0.49	0.00	0.00	0.00	0.00	0.64	0.00	0.17	0.62		
SEe	SEe	277	0.00	0.00	0.00	0.00	0.00	0.00	2.51	0.00	0.26	0.37		
SET	SET	952	0.00	0.00	0.00	1.18	0.29	0.45	7.43	0.00	0.12	0.38		
		Total (km ²)	581.51	2.06	39.91	133.92	86.59	24.19	53.16	158.67	57.99	25.04		
		Percent (%)	100.00	0.35	6.86	23.03	14.89	4.16	9.14	27.29	9.97	4.31		

Com : Commercial Areas Inst : Institutional Areas
 Ind : Industrial Areas Green : Green Spaces
 High R. : High Residential Areas Agri. : Agricultural Areas
 Med R. : Medium Residential Areas Water : Water Bodies
 Low R. : Low Residential Areas

Data Source : Urban Planning Institute, Ho Chi Minh City (UPI, HCMC)
 Calculation : GIS database : Inner City : Scale = 1/25,000
 Outer City : Scale = 1/25,000

Table C.6.4 CRITERIA FOR RUNOFF COEFFICIENT BY LAND USE CATEGORY

Item No.	Main Landuse Group (grouped by JICA Study Team)	Sub-Group for Landuse Category (as in original GIS database by UPI, HCMC)		Run off Coefficient		
		Existing Landuse	Future Landuse (2020)	Normal Range	Hydrological Sub-Institute, HCMC*	Applied by JICA Study Team
1	Commercial Areas	Services, Commerce	Commerce, Services	0.70 - 0.90	0.80	0.80
		Industrial Area	Industrial Area (Existing)			
2	Industrial Areas	Ware House	Industrial Area (Planned) Ware House	0.70 - 0.90	0.90	0.90
			Garbage Treatment Site (Planned)			
3	High Residential Areas	Residential (Inner City)	Inner City	0.70 - 0.90	0.80	0.80
			Suburban Residential Town (Planned)			
4	Medium Residential Areas	Residential (Suburban)	Suburban Residential Area (Planned)	0.50 - 0.70	0.60	0.70
			Existing Residential Area (Suburban)			
5	Low Residential Areas	Administrative Office	City Center	0.30 - 0.50	0.40	0.60
		Education	District Center			
6	Institutional Areas	Health Care	Center of an Area	0.40 - 0.60	0.50	0.60
		Social Culture, Sports	Culture, Education			
7	Green Spaces	Military	Tourism	0.10 - 0.30	0.30	0.30
		Preservative Works	Military			
8	Agricultural Areas	Religion		0.10 - 0.60	0.15	0.40
		Transportation Stations				
9	Water Bodies	Public Works		0.10 - 0.60	0.15	1.00
		Technical Works				
		Green Park	Green Park (Planned)			
		Cemetery	Ecological Forestry			
		Agriculture	Agriculture			
		River, Canal	River, Canal			

* : "Research the drainage capacity and measure for the inner area of Ho Chi Minh City"; Hydrological Sub-Institute of HCMC; 1989

Table C.6.5 URBANIZATION AND RUNOFF COEFFICIENTS

Zone	Catchment	Existing Landuse Area (km ²)				Future (2020) Landuse Area (km ²)				Runoff Coefficient							
		ID	Area (km ²)	Urbanized		Non-Urbanized		Urbanized		Non-Urbanized		Catchment		Zone			
				Catchment	Zone	Catchment	Zone	Catchment	Zone	Catchment	Zone	Existing	Future	Existing	Future		
N	136.18	N1	19.87	2.88	16.98	6.85	13.01	0.45	0.51	0.52	0.58	0.52	0.58				
		N2	107.57	41.54	46.60	66.03	89.58	68.81	80.35					38.75	55.83	0.53	0.60
		Na	8.75	2.17	6.58	4.68	4.06	0.48	0.52								
C	106.41	C1	31.67	29.26	1.91	29.59	2.07	0.75	0.76	0.70	0.72	0.70	0.72				
		C2	5.14	3.81	1.33	4.47	0.67	0.74	0.78								
		C3	20.22	12.41	7.81	16.67	3.56	0.67	0.74								
		C4	41.50	25.61	75.38	15.89	31.03	22.45	87.56					9.05	18.85	0.70	0.70
		Ca	4.91	1.14	3.77	2.10	2.81	0.51	0.53								
		Cb	1.28	1.13	0.16	0.75	0.54	0.80	0.64								
		Cc	1.68	1.52	0.16	1.53	0.16	0.66	0.59								
W	72.91	W1	72.91	14.39	14.39	58.52	58.52	33.62	33.62	39.29	39.29	0.48	0.56				
S	81.71	S1	14.33	0.87	13.46	3.71	10.62	0.47	0.52	0.50	0.59	0.50	0.59				
		S2	15.66	1.53	14.13	5.12	10.54	0.48	0.53								
		S3	34.51	3.54	30.97	20.06	13.55	0.50	0.63								
		S4	2.36	1.40	0.96	1.39	0.97	0.68	0.67								
		S5	2.23	0.15	2.05	1.10	1.13	0.46	0.60								
		Sa	3.46	2.26	1.20	2.75	0.71	0.74	0.83								
		Sb	3.86	0.42	3.44	1.62	2.24	0.47	0.57								
		Sc	5.33	1.17	4.15	2.81	2.52	0.50	0.56								
NE	64.91	NE1	3.32	0.54	2.78	1.93	1.39	0.46	0.57	0.47	0.59	0.47	0.59				
		NE2	9.53	1.97	7.56	8.51	1.62	0.47	0.63								
		NE3	7.13	2.39	4.75	6.25	0.89	0.50	0.64								
		NE4	2.65	1.50	1.15	2.48	0.18	0.57	0.73								
		NE5	34.38	5.13	29.25	21.46	12.92	0.45	0.57								
		Nea	3.76	0.20	3.57	1.97	1.79	0.44	0.54								
		NEb	2.50	0.16	2.34	1.02	1.47	0.44	0.47								
		NEc	1.62	0.49	1.13	0.95	0.67	0.66	0.72								
SE	119.36	SE1	1.99	0.22	1.75	0.98	1.00	0.46	0.50	0.46	0.53	0.46	0.53				
		SE2	2.60	0.20	2.39	1.71	0.88	0.44	0.65								
		SE3	1.92	0.13	1.78	1.68	0.24	0.48	0.80								
		SE4	7.80	1.55	6.25	2.23	5.57	0.49	0.44								
		SE5	3.83	0.58	3.25	2.51	1.32	0.46	0.65								
		SE6	5.10	0.74	4.36	4.35	0.76	0.46	0.80								
		SE7	14.58	2.43	12.15	8.36	6.22	0.48	0.55								
		SE8	11.33	1.06	10.27	4.76	5.57	0.45	0.53								
		SE9	21.11	2.52	18.59	11.46	9.64	0.45	0.56								
		SE10	24.88	0.98	23.89	7.40	17.48	0.46	0.49								
		SEa	3.67	1.69	2.57	2.39	1.28	0.61	0.76								
		SEb	5.16	1.00	4.16	4.01	1.15	0.48	0.58								
		SEc	1.82	0.16	1.66	0.00	1.82	0.44	0.31								
		SEd	1.30	0.44	0.86	0.49	0.81	0.56	0.62								
		SEe	2.77	0.00	2.77	0.00	2.77	0.46	0.37								
		SEf	9.52	0.42	9.11	1.92	7.60	0.42	0.38								
Total Area (km ²)		581.51	173.66	407.85	339.81	241.70											
Percent (%)		100.00	29.86	70.14	58.44	41.56											

Urbanized Sub-Total of commercial, industrial, residential and institutional areas
 Non-Urbanized Sub-Total of green spaces, agricultural areas and water bodies

Data Source: Urban Planning Institute, Ho Chi Minh City (UPI, HCMC)
 Measurement: GIS database Inner City => Scale = 1/10,000 (for existing landuse) and 1/25,000 (for future landuse)
 Outer City => Scale = 1/25,000 (for both existing and future landuse)

Table C.6.6 PEAK RUNOFF CALCULATION FOR OUTFLOW FROM DAIHAN CANAL (ALTERNATIVE 1)

Drainage Area	Catchment ID	Sub-Catchment ID	Area (km ²)	Runoff Point		C	L _s (km)	T _c (min)	V _r (m/s) by Segment		L _s (km)	Cumulative T _r (min)		Cumulative I _a (mm)		I for 5-yr (mm/hr)		I for 10-yr (mm/hr)		f		Q _p for 5-yr (m ³ /s)		Q _p for 10-yr (m ³ /s)		
				Ex	Put				Ex	Put		Ex	Put	Ex	Put	Ex	Put	Ex	Put	Ex	Put	Ex	Put	Ex	Put	Ex
Northern Zone	N.2.1	N.2.A	11.07	0.48	3.47	100	0.70	1.68	2.65	23	124	46.0	31.5	0.96	69	77										
	N.2.2	N.2.B	18.10	0.47	0.70	100	0.70	1.68	2.65	63	163	35.4	39.4	0.94	78	87										
	N.2.3	N.2.C	29.31	0.50	0.68	100	0.68	4.09	6.74	177	277	20.7	22.6	0.91	83	83										
	N.2.4	N.2.D1	34.65	0.52	0.40	193	0.40	2.49	9.23	281	381	15.3	16.7	0.90	74	83										
	N.2.5	N.2.D2	10.98	0.63	3.33	193	0.30	3.34	6.23	186	379	15.4	16.8	0.96	28	31										
	N.2.6	N.2.E	45.63	0.35	0.35	100	0.35	1.56	10.79	281	381	15.3	16.7	0.88	91	101										
	N.2.7	N.2.F	56.82	0.57	0.57	100	0.35	2.25	13.04	355	455	13.0	14.2	0.86	99	108										
	N.2.8	N.2.G1	64.30	0.58	0.40	100	0.35	4.37	17.41	462	562	10.7	11.7	0.84	99	108										
	N.2.9	N.2.G2	78.22	0.59	0.49	123	0.30	3.10	3.10	670	770	8.1	8.9	0.83	99	108										
	N.2.10	N.2.G3	11.86	0.53	2.16	123	0.35	2.49	5.59	172	295	19.5	21.3	0.98	16	18										
	N.2.11	N.2.H	90.09	0.59	0.59	100	0.35	1.57	18.98	745	845	7.4	8.2	0.81	99	108										
	N.2.12	N.2.I	95.79	0.59	0.59	100	0.35	2.50	21.48	864	964	6.6	7.3	0.81	99	108										
	N.2.13	N.2.J	107.57	0.60	0.60	100	0.35	2.61	24.09	988	1088	5.9	6.6	0.81	99	108										
Western Zone	W.1.1	W.1.A	9.68	0.66	3.00	66	0.50	3.60	87	154	37.5	41.8	0.97	65	72											
	W.1.2	W.1.B	14.81	0.59	0.40	100	0.40	2.05	4.65	172	239	23.9	26.1	0.95	65	72										
	W.1.3	W.1.C1	19.55	0.55	0.40	100	0.40	4.11	8.76	343	411	14.3	15.6	0.94	65	72										
	W.1.4	W.1.C2	8.62	0.52	3.20	88	0.30	2.20	2.20	122	211	27.2	29.7	0.97	33	36										
	W.1.5	W.1.C	28.16	0.54	0.54	100	0.30	2.20	8.76	343	411	14.3	15.6	0.92	65	72										
	W.1.6	W.1.D1	36.36	0.57	0.69	52	0.40	4.49	13.25	531	598	10.1	11.1	0.90	65	72										
	W.1.7	W.1.D	39.56	0.58	0.69	52	0.30	2.07	3.07	115	167	34.7	38.7	0.99	21	23										
	W.1.8	W.1.E1	43.26	0.58	0.58	100	0.40	2.72	15.97	644	711	8.7	9.5	0.88	65	72										
	W.1.9	W.1.E2	6.04	0.44	0.44	74	0.30	4.90	4.90	322	346	16.8	18.3	0.98	12	13										
	W.1.10	W.1.E3	7.02	0.50	0.50	100	0.40	1.36	6.26	325	402	14.5	15.9	0.98	14	15										
	W.1.11	W.1.F	50.28	0.57	0.57	100	0.40	1.18	17.15	693	711	8.7	9.5	0.87	65	72										
	W.1.12	W.1.G	59.41	0.57	0.57	100	0.40	1.85	18.99	770	837	7.5	8.3	0.85	65	72										

C = Runoff coefficient, L_s = Inlet length, T_c = Inlet time, V_r = Flow velocity, L_r = Flow length, T_r = Flow time
 T_c = Time of concentration; I = Rainfall intensity; f = Areal reduction factor; Q_p = Peak runoff

Note: It is assumed that outflow from each Daihan will be discharged into each Tham Luong - each Ben Cat system.

Table C.6.7 PEAK RUNOFF CALCULATION FOR OUTFLOW FROM DAIHAN CANAL (ALTERNATIVE 2)

Drainage Area	Catchment ID	Sub-Catchment ID	Area (km ²)	Runoff Point ID	Area (km ²)	C	L _i (km)	T _i (min)	V _i (m/s) by Segment		L _i (km)	Cumulative T _i (min)		f _o - f _{o-1} (mm/hr)		f _o	Q _p for Surf (m ³ /s)	Q _p for Ex (m ³ /s)	Q _p for In (m ³ /s)	
									Ex	Fut		Ex	Fut	Ex	Fut					Ex
Northern Zone	N.2	N.2.6	11.19	N.2.E	11.19	0.66	2.10	65	1.00	1.00	3.00	60	45.6	51.0	0.96	90	90	0	100	
		N.2.7	7.48	N.2.F	18.66	0.66	2.25	65	0.35	0.35	3.65	167	24.7	26.9	0.94	90	90	0	100	
		N.2.8	13.03	N.2.G	32.59	0.66	4.37	10.21	440	0.35	0.35	4.72	375	19.4	14.6	0.91	90	90	0	100
		N.2.9	6.32	N.2.G5	6.32	0.49	2.16	123	0.30	0.30	3.10	172	29.5	19.3	0.98	16	16	0	18	
		N.2.10	5.54	N.2.G5	11.86	0.53	2.49	130	0.35	0.35	5.59	291	41.3	14.2	0.96	24	24	0	26	
		N.2.11	5.71	N.2.H	44.45	0.63	10.21	440	0.35	0.35	10.21	375	19.4	14.6	0.98	90	90	0	100	
		N.2.12	4.89	N.2.I	50.16	0.64	11.79	450	0.35	0.35	11.79	450	11.6	11.6	0.87	90	90	0	100	
		N.2.13	6.86	N.2.J	55.05	0.64	14.28	509	0.35	0.35	14.28	509	9.6	9.6	0.86	90	90	0	100	
		N.2.14	6.86	N.2.J	61.94	0.65	16.39	601	0.35	0.35	16.39	601	8.2	8.2	0.85	90	90	0	100	
		N.2.15	11.67	N.2.A	11.67	0.48	3.47	100	0.70	0.70	0.97	23	124	46.9	51.5	0.96	92	92	0	77
		N.2.16	6.43	N.2.B	18.10	0.47	1.68	63	0.70	0.70	2.65	63	163	35.4	39.4	0.94	78	78	0	87
		N.2.17	11.24	N.2.C	29.33	0.50	4.09	6.74	0.60	0.60	6.74	177	277	20.7	22.6	0.91	78	78	0	87
N.2.18	5.32	N.2.D	34.65	0.52	2.49	9.23	0.40	0.40	2.49	381	15.3	15.3	0.90	78	78	0	87			
N.2.19	10.98	N.2.D2	45.63	0.55	3.34	12.58	0.30	0.30	3.34	567	19.6	19.6	0.88	78	78	0	87			
Western Zone	W.1	W.1.1	9.68	W.1.A	9.68	0.66	3.00	68	0.30	0.30	2.60	87	154	37.5	41.8	0.97	65	65	0	72
		W.1.2	5.14	W.1.B	55.31	0.57	0.56	13.07	0.30	0.30	0.56	483	10.4	11.3	0.96	78	78	0	87	
		W.1.3	4.73	W.1.C	65.18	0.55	4.11	19.23	0.40	0.40	2.05	668	9.2	10.1	0.95	78	78	0	87	
		W.1.4	8.82	W.1.C2	8.62	0.52	3.20	21.1	0.30	0.30	2.20	122	211	7.5	8.2	0.94	78	78	0	87
		W.1.5	8.20	W.1.D1	81.99	0.55	4.49	23.73	0.40	0.40	4.49	740	7.5	7.5	0.94	78	78	0	87	
		W.1.6	3.20	W.1.D2	3.20	0.60	1.80	52	0.30	0.30	2.07	115	167	34.7	29.7	0.97	33	33	0	36
		W.1.7	3.70	W.1.E	83.89	0.56	5.56	23.71	0.40	0.40	5.56	740	640	7.5	8.2	0.93	78	78	0	87
		W.1.8	6.04	W.1.E2	6.04	0.44	1.22	74	0.30	0.30	4.90	372	9.9	9.9	0.92	78	78	0	87	
		W.1.9	0.99	W.1.E3	7.02	0.50	1.36	402	0.40	0.40	4.00	329	14.5	14.5	0.98	14	14	0	15	
		W.1.10	0.27	W.1.F	95.91	0.56	26.44	1040	0.40	0.40	26.44	1040	140	140	0.81	78	78	0	87	
		W.1.11	8.85	W.1.G	105.04	0.56	29.47	1166	0.40	0.40	1.85	1093	5.5	5.5	0.81	78	78	0	87	
		W.1.12	13.50	W.1.H	118.54	0.56	31.22	1230	0.40	0.40	1.76	1330	5.0	5.0	0.81	78	78	0	87	

C = Runoff coefficient; L_i = Inlet length; T_i = Inlet time; V_i = Flow velocity; L_f = Flow length; T_f = Flow time
 T_i = Time of concentration; I = Rainfall intensity; f = Areal reduction factor; Q_p = Peak runoff
 Note: It is assumed that outflow from Kaoh Onhan will be discharged into Rach Tham Luong - Rach Ben Ca system.

Table C.6.8 PEAK RUNOFF CALCULATION BY RATIONAL METHOD (1/2)

Drainage Area	ID	Sub-Catchment	Remot. Elev. (m)	Area (ha)	ID	Area (ha)	L _i (km)	T _i (min)	V _i (m/s)	Y _i (m)	L _f (km)	Cumulative Tr (min)		L _f (km)		Cumulative		L _f (km)	T _f (min)	V _f (m/s)	Y _f (m)	I for U _{av} (mm/hr)		I for S _{av} (mm/hr)		Q _p for I _{av} (m ³ /s)					
												Ex	Inlet	Ex	Inlet	Ex	Inlet					Ex	Inlet	Ex	Inlet	Ex	Inlet	Ex	Inlet	Ex	Inlet
Northern Zone	N.1*	N.1.1	3.35	N.1.1A	3.35	0.84	0.51	1.35	35	3.0	0.31	0.33	1.07	5.1	1.14	10.1	49.3	54.9	53.2	61.5	0.94	20	25	23	29	24	25	23	29		
		N.1.2	6.60	N.1.2B	6.60	1.48	0.55	0.30	0.35	2.78	21.1	143	235	21.1	24.5	23.5	26.6	19.7	26	26	26.6	0.97	26	26	30	36	30	36	36		
		N.1.3	4.25	N.1.3C	4.25	0.47	0.52	0.45	0.50	2.04	34.1	301	17.0	19.1	18.5	20.8	10.96	30	30	30	30	10.96	30	30	32	40	30	32	40		
		N.1.4	3.69	N.1.4D	3.69	0.46	0.51	0.45	0.50	3.61	41.6	372	47.5	42.1	15.9	13.6	15.2	10.4	30	30	30	30	15.2	10.4	30	37	32	41	30	41	
	N.2**	N.2.1	11.67	N.2.1A	11.67	0.50	0.48	3.47	100	100	0.70	0.97	0.97	23	124	124	46.0	44.8	33.5	51.5	0.94	69	69	69	77	77	69	77	77		
		N.2.2	6.93	N.2.2B	6.93	0.50	0.47	1.66	63	63	0.70	0.70	2.65	63	163	35.4	35.4	39.4	39.4	0.94	78	78	78	87	87	78	87	87			
		N.2.3	11.24	N.2.3C	11.24	0.60	0.50	4.09	6.24	170	0.77	2.79	2.79	20.6	20.7	27.1	20.6	20.7	27.1	27.1	0.91	78	78	78	87	87	78	87	87		
		N.2.4	5.32	N.2.4D	5.32	0.49	0.52	2.49	9.23	290	0.31	3.00	3.00	381	381	381	15.1	15.1	16.3	16.3	0.94	78	78	78	87	87	78	87	87		
		N.2.5	10.98	N.2.5E	10.98	0.48	0.63	5.52	249	193	0.23	0.30	3.34	242	186	491	381	12.1	15.3	13.2	16.1	0.96	17	17	17	26	17	26	17	26	
		N.2.6	11.19	N.2.6F	11.19	0.52	0.59	4.55	9.23	290	0.31	3.00	3.00	381	381	381	15.1	15.1	16.3	16.3	0.94	78	78	78	87	87	78	87	87		
		N.2.7	7.48	N.2.7G	7.48	0.53	0.59	2.25	13.04	490	0.62	3.90	3.90	562	562	562	10.2	10.7	11.2	11.7	0.94	88	88	88	96	88	96	88	96		
		N.2.8	13.91	N.2.8H	13.91	0.53	0.59	4.37	17.41	724	0.70	4.21	4.21	724	724	724	7.6	8.1	8.4	8.9	0.83	38	38	38	46	38	46	38	46		
		N.2.9	6.32	N.2.9I	6.32	0.50	0.49	2.16	123	123	0.30	0.30	3.10	172	172	172	19.5	19.5	21.3	21.3	0.98	16	16	16	18	16	18	16	18		
		N.2.10	5.54	N.2.10J	5.54	0.50	0.55	2.49	17.41	724	0.70	4.21	4.21	724	724	724	7.6	8.1	8.4	8.9	0.83	38	38	38	46	38	46	38	46		
Central Zone	C.1	N.2.11	5.71	N.2.11A	5.71	0.53	0.59	1.57	16.98	808	0.45	2.59	21.48	942	866	1042	6.2	6.6	6.9	7.3	0.81	88	88	88	96	88	96	88	96		
		N.2.12	4.89	N.2.12B	4.89	0.53	0.59	2.59	21.48	942	0.45	2.59	21.48	942	866	1042	6.2	6.6	6.9	7.3	0.81	88	88	88	96	88	96	88	96		
		N.2.13	6.89	N.2.13C	6.89	0.53	0.60	2.61	24.76	1083	0.31	0.35	2.61	1083	1083	1083	5.5	5.9	6.1	6.6	0.81	88	88	88	96	88	96	88	96		
		N.2.14	7.38	N.2.14D	7.38	0.53	0.76	2.61	24.76	1083	0.31	0.35	2.61	1083	1083	1083	5.5	5.9	6.1	6.6	0.81	88	88	88	96	88	96	88	96		
	C.2	C.2.1	7.38	C.2.1A	7.38	0.53	0.76	2.61	24.76	1083	0.31	0.35	2.61	1083	1083	1083	5.5	5.9	6.1	6.6	0.81	88	88	88	96	88	96	88	96		
		C.2.2	3.55	C.2.2B	3.55	0.70	0.76	2.14	2.46	124	0.66	0.70	2.14	2.46	124	124	44.2	44.2	49.4	49.4	0.96	104	104	104	117	104	117	104	117		
		C.2.3	5.87	C.2.3C	5.87	0.76	0.71	1.77	4.06	97	0.99	0.70	1.77	4.06	97	97	33.6	33.6	37.8	37.8	0.95	114	114	114	130	114	130	114	130		
		C.2.4	7.58	C.2.4D	7.58	0.75	0.76	2.23	6.78	156	0.69	0.70	2.23	6.78	156	156	25.1	25.1	27.3	27.3	0.93	118	118	118	134	118	134	118	134		
	C.3	C.3.1	5.14	C.3.1A	5.14	0.74	0.74	1.43	31	29	0.33	0.40	2.69	92	87	121	161	46.4	48.5	51.9	54.3	0.98	49	49	53	54	49	53	49	53	
		C.3.2	3.86	C.3.2B	3.86	0.60	0.79	1.80	45	45	1.40	1.40	2.26	2.26	72	72	72	71.1	79.8	79.8	81.0	0.98	90	90	90	101	90	101	90	101	
		C.3.3	4.72	C.3.3C	4.72	0.76	0.74	1.20	1.20	1.20	1.97	4.24	34	34	100	100	55.4	55.4	62.1	62.1	0.97	117	117	117	131	117	131	117	131		
		C.3.4	3.77	C.3.4D	3.77	0.74	0.76	1.43	31	29	0.33	0.40	2.69	92	87	121	161	46.4	48.5	51.9	54.3	0.98	49	49	53	54	49	53	49	53	
	C.4	C.4.1	1.84	C.4.1A	1.84	0.54	0.55	0.59	1.7	15	0.35	0.40	3.34	161	130	178	154	32.6	37.4	36.2	41.7	0.99	12	12	12	14	12	14	12	14	
		C.4.2	2.88	C.4.2B	2.88	0.45	0.67	1.60	131	88	0.26	0.40	3.33	210	139	342	227	17.6	25.2	18.5	27.5	0.99	6	6	6	7	6	7	6	7	
C.4.3		3.41	C.4.3C	3.41	0.49	0.62	1.51	105	86	0.31	0.40	2.56	136	107	241	166	23.8	30.1	25.9	33.0	0.99	11	11	11	12	11	12	11	12		
C.4.4		3.11	C.4.4D	3.11	0.48	0.58	1.46	105	86	0.38	0.40	3.18	144	107	241	166	23.8	30.1	25.9	33.0	0.99	11	11	11	12	11	12	11	12		
Western Zone	W.1	C.4.5	3.24	C.4.5A	3.24	0.61	0.71	2.14	2.14	2.14	2.14	2.14	2.14	2.14	2.14	2.14	2.14	2.14	2.14	2.14	2.14	2.14	2.14	2.14	2.14	2.14	2.14	2.14	2.14	2.14	
		C.4.6	7.51	C.4.6B	7.51	0.66	0.71	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46
		C.4.7	6.64	C.4.7C	6.64	0.67	0.71	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46
		C.4.8	10.86	C.4.8D	10.86	0.67	0.71	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46	1.46
	W.1	W.1.1	9.68	W.1.1A	9.68	0.61	0.68	3.00	73	66	0.46	0.50	2.80	95	87	168	154	34.5	37.5	34.8	41.8	0.97	55	55	55	65	55	65	55	65	
		W.1.2	3.14	W.1.2B	3.14	0.55	0.59	1.81	18.8	18.8	0.37	0.40	2.05	186	172	290	234	22.1	23.9	24.0	26.1	0.95	55	55	55	65	55	65	55	65	
		W.1.3	4.75	W.1.3C	4.75	0.52	0.55	1.51	15.5	15.5	0.38	0.40	4.11	8.76	368	343	441	411	13.4	14.3	14.0	15.6	0.94	55	55	55	65	55	65	55	65
		W.1.4	8.62	W.1.4D	8.62	0.47	0.52	3.20	97	88	0.27	0.30	2.20	135	122	231	211	24.7	27.2	27.0	29.7	0.97	55	55	55	65	55	65	55	65	
		W.1.5	8.20	W.1.5E	8.20	0.50	0.54	1.80	64	52	0.35	0.40	4.49	13.25	585	531	658	598	9.3	10.1	10.2	11.1	0.90	55	55	55	65	55	65	55	65
		W.1.6	3.20	W.1.6F	3.20	0.46	0.69	1.80	64	52	0.24	0.30	2.07	146	115	210	167	27.3	34.7	29.8	38.7	0.99	13	13	13	14	13	14	13	14	
		W.1.7	3.70	W.1.7G	3.70	0.49	0.56	1.80	64	52	0.34	0.40	2.72	179	144	292	211	7.9	8.7	8.7	9.5	0.98	55	55	55	65	55	65	55	65	
		W.1.8	6.04	W.1.8H	6.04	0.44	0.44	1.22	74	74	0.30	0.30	4.90	4.90	274	348	366	16.7	16.8	18.2	18.3	0.98	12	12	12	13	12	13	12	13	
		W.1.9	0.99	W.1.9I	0.99	0.45	0.50																								

Table C.6.8 PEAK RUNOFF CALCULATION BY RATIONAL METHOD (2/2)

Drainage Area (Acres)	Catchment ID	Sub-catchment ID	Runoff (mm)	Area (Acres)	C (mm)	L (ft)	T _r (min)	V _r (m/s)	Segment	L (km)		Cumulative		1 for V _r (min/hr)		1 for V _r (mm/hr)		Q _p for V _r (m ³ /s)				
										Ex	In	Ex	In	Ex	In	Ex	In	Ex	In			
S.1	S.1	S.1.1	2.08	1.0	0.57	0.41	35	0.34	0.40	2.54	2.54	124	106	158	131	36.5	41.8	40.7	46.7	17	18	
		S.1.2	4.04	1.1	0.48	0.43	274	0.36	0.40	5.06	7.60	239	211	274	242	21.0	23.7	22.8	25.8	18	20	
		S.1.3	4.38	1.1	0.47	0.45	0.78	0.34	0.40	4.00	8.60	196	167	230	203	22.6	26.3	24.6	28.7	13	14	
		S.1.4	1.96	1.1	0.46	0.42	0.32	0.36	0.40	6.07	12.67	350	255	350	255	16.6	19.0	18.0	20.7	14	19	
		S.1.5	1.27	1.1	0.47	0.45	0.45	0.36	0.40	7.46	15.13	301	232	301	232	14.9	16.9	16.0	20.7	34	29	
		S.1.6	2.40	1.1	0.48	0.43	1.22	0.40	0.40	2.02	4.04	84	74	158	131	36.5	41.8	40.7	46.7	17	18	
		S.1.7	1.96	1.1	0.48	0.45	1.17	0.36	0.40	1.90	3.80	93	79	158	131	36.5	41.8	40.7	46.7	17	18	
		S.1.8	2.85	1.1	0.50	0.48	0.52	0.39	0.40	2.05	4.05	93	84	175	158	33.1	38.6	36.6	40.8	10	14	
		S.1.9	8.85	1.1	0.48	0.43	1.56	0.39	0.40	3.51	7.01	197	147	240	178	23.6	26.3	24.6	28.7	22	26	
		S.1.10	2.17	1.1	0.53	0.51	1.02	0.30	0.40	6.92	13.84	314	288	356	339	14.6	16.2	16.1	17.6	29	29	
S.2	S.2	S.2.1	2.32	1.1	0.50	0.47	26	0.28	0.40	1.57	1.57	65	65	163	123	35.4	40.1	39.5	43.3	13	13	
		S.2.2	4.49	1.1	0.49	0.47	1.20	0.28	0.40	3.96	5.53	181	136	272	198	18.7	21.5	20.3	22.8	15	15	
		S.2.3	4.79	1.1	0.51	0.49	1.20	0.28	0.40	3.12	6.24	181	136	272	198	21.1	24.0	22.0	24.9	15	15	
		S.2.4	5.75	1.1	0.51	0.49	1.43	0.34	0.40	3.58	7.16	178	149	264	202	19.7	22.6	21.5	24.5	12	12	
		S.2.5	4.31	1.1	0.50	0.47	0.65	0.30	0.40	3.90	7.80	166	142	244	184	18.4	21.3	20.3	23.2	14	14	
		S.2.6	4.34	1.1	0.49	0.46	0.65	0.31	0.40	6.39	12.78	376	275	452	335	13.1	14.3	14.3	16.9	34	37	
		S.2.7	6.45	1.1	0.47	0.44	2.48	0.35	0.40	9.22	18.44	517	364	594	442	10.2	11.3	11.2	14.5	37	42	
		S.2.8	30.13	1.1	0.49	0.43	0.63	0.32	0.40	3.41	6.82	164	142	244	184	18.4	21.3	20.3	23.2	14	14	
		S.2.9	4.38	1.1	0.50	0.43	0.63	0.32	0.40	11.05	22.10	644	485	720	543	8.8	11.1	11.2	14.5	34	34	
		S.2.10	2.33	1.1	0.48	0.46	0.65	0.31	0.40	1.86	3.72	80	77	150	116	38.3	43.1	42.2	45.2	19	21	
S.3	S.3	S.3.1	3.78	1.1	0.46	0.43	21	0.32	0.40	3.38	3.38	186	186	459	211	170	27.1	34.0	29.6	37.9	11	12
		S.3.2	4.78	1.1	0.47	0.46	2.40	0.27	0.40	2.57	5.95	107	249	172	23.0	33.6	25.1	37.4	14	18		
		S.3.3	4.75	1.1	0.47	0.45	0.65	0.29	0.40	4.76	9.51	282	186	376	246	15.5	17.7	16.9	23.7	19	20	
		S.3.4	7.13	1.1	0.50	0.48	2.26	0.31	0.40	3.14	6.28	166	160	262	161	23.6	25.8	25.7	29.0	33	36	
		S.3.5	10.12	1.1	0.47	0.43	1.80	0.33	0.40	2.17	4.34	141	91	180	143	32.3	40.2	35.9	44.9	14	15	
		S.3.6	4.72	1.1	0.46	0.43	1.80	0.33	0.40	3.35	6.60	34	19	69	55	73.4	85.6	82.4	96.0	83	93	
		S.3.7	17.94	1.1	0.45	0.40	0.60	0.29	0.40	2.34	4.68	13	10	62	46	79.5	95.0	89.2	106.8	47	53	
		S.3.8	14.94	1.1	0.45	0.40	0.60	0.29	0.40	3.05	6.10	24	19	69	55	73.4	85.6	82.4	96.0	83	93	
		S.3.9	10.17	1.1	0.45	0.40	0.60	0.29	0.40	5.05	10.10	39	30	85	66	63.2	75.7	71.0	85.0	124	139	
		S.3.10	6.21	1.1	0.45	0.40	0.60	0.29	0.40	7.88	15.76	100	77	166	133	39.5	49.6	44.1	55.6	133	149	
S.4	S.4	S.4.1	7.57	1.1	0.47	0.45	2.00	0.32	0.40	4.44	12.32	133	262	376	268	15.4	19.3	18.3	21.0	16	16	
		S.4.2	11.43	1.1	0.47	0.45	2.00	0.32	0.40	3.62	6.02	151	151	265	205	21.6	23.6	23.6	26.6	19	19	
		S.4.3	19.00	1.1	0.47	0.45	2.00	0.32	0.40	4.08	8.16	321	454	435	435	12.0	13.5	14.2	14.7	30	35	
		S.4.4	53.36	1.1	0.46	0.43	0.51	0.34	0.40	12.32	24.64	340	321	654	435	13.0	15.5	14.2	14.7	133	161	
		S.4.5	59.26	1.1	0.46	0.43	0.51	0.34	0.40	2.41	4.82	422	503	438	438	11.8	12.9	12.9	14.1	133	161	
		S.4.6	59.26	1.1	0.46	0.43	0.51	0.34	0.40	2.22	4.44	105	105	138	126	41.6	44.7	44.8	50.0	10	12	
		S.4.7	2.60	1.1	0.48	0.45	0.51	0.37	0.40	2.08	4.16	27	46	70	118	24.1	27.9	27.9	30.5	11	12	
		S.4.8	1.95	1.1	0.48	0.45	0.51	0.37	0.40	2.50	5.00	172	104	356	205	18.7	21.6	20.5	23.4	4	5	
		S.4.9	5.40	1.1	0.48	0.45	0.51	0.37	0.40	3.41	6.82	182	200	200	246	20.6	23.5	21.3	24.2	18	20	
		S.4.10	7.40	1.1	0.47	0.44	0.44	0.30	0.40	2.05	4.10	227	285	285	202	20.2	22.9	21.9	24.0	19	21	
S.5	S.5	S.5.1	3.83	1.1	0.46	0.43	2.34	0.28	0.40	1.11	1.11	65	65	147	103	21.7	23.6	23.6	26.6	11	11	
		S.5.2	5.00	1.1	0.46	0.43	1.43	0.25	0.40	4.39	8.78	319	183	456	265	13.0	15.8	14.1	17.0	27	27	
		S.5.3	8.39	1.1	0.46	0.43	1.80	0.29	0.40	1.50	3.00	51	42	211	191	24.8	26.9	27.0	30.2	26	28	
		S.5.4	14.58	1.1	0.48	0.45	0.51	0.35	0.40	3.20	6.40	704	175	364	325	15.2	17.8	16.5	19.4	28	30	
		S.5.5	2.77	1.1	0.42	0.40	0.40	0.36	0.40	1.95	3.90	90	81	180	164	31.8	35.3	34.9	39.3	10	12	
		S.5.6	8.56	1.1	0.43	0.41	0.51	0.34	0.40	4.12	8.24	292	253	383	336	15.2	17.2	16.6	18.8	21	22	
		S.5.7	2.40	1.1	0.47	0.43	0.40	0.32	0.40	2.47	4.94	142	103	201	147	24.5	26.5	26.5	29.5	51	51	
		S.5.8	15.73	1.1	0.45	0.42	0.57	0.32	0.40	4.24	8.48	365	279	428	324	13.9	17.8	15.1	19.4	27	30	
		S.5.9	2.38	1.1	0.45	0.43	0.56	0.32	0.40	2.83	5.66	510	307	570	442	10.6	12.3	11.6	14.6	27	30	
		S.5.10	123.43																			

C = Runoff coefficient; L_i = Inlet length; T_r = Inlet time; V_r = Flow velocity; L_r = Flow length; T_r = Flow time; T_c = Time of concentration
 I = Rainfall intensity; I_r = Annual reduction factor; Q_p = Peak runoff

Table C.6.9 PEAK RUNOFF ALONG BEN DA-BA HONG AND THAM LUONG-BEN CAT CANALS WITHOUT INUNDATION

Drainage Area	Catchment ID	Sub-Catchment ID	Catchment Area (km ²)	Runoff Point ID	Area (km ²)	C	L (km)	T _r (min)	V _r (m/s) by Segment		L _r (km)		Cumulative T _r (min)		I (for 10-yr (mm/hr))		Q _p for 5-yr (m ³ /s)		Q _p for 10-yr (m ³ /s)						
									Ex	Fut	Ex	Fut	Ex	Fut	Ex	Fut	Ex	Fut	Ex	Fut	Ex	Fut			
N.1	N.1	N.1.1	3.33	N.1.A	3.33	0.44	0.51	1.53	50	50	1.07	1.07	14	12	62	73.3	79.3	82.2	89.0	99	33				
		N.1.2	6.60	N.1.B	9.93	0.48	0.53		0.70	0.80	2.78	3.85	80	70	136	170	42.2	47.4	47.1	53.0	97	54	70		
		N.1.3	4.25	N.1.C	14.17	0.47	0.52		0.45	0.50	2.04	5.89	155	138	211	183	27.1	30.0	29.6	33.5	96	54	70		
		N.1.4	5.69	N.1.D	19.87	0.46	0.51		0.45	0.50	3.61	9.50	289	258	345	308	16.8	18.7	18.3	20.4	94	54	70		
		N.2.1	11.67	N.2.A	11.67	0.50	0.48	3.47	100	100	1.50	1.50	0.97	1.1	11	11	111	111	30.5	56.6	56.6	69.6	96	76	85
		N.2.2	6.43	N.2.B	18.10	0.50	0.47		1.50	1.50	1.68	2.65	29	29	130	130	44.0	44.0	49.2	49.2	94	97	97		
		N.2.3	11.24	N.2.C	29.33	0.49	0.50		1.18	1.20	4.09	6.74	87	86	183	187	30.5	30.7	33.4	33.7	91	111	114		
		N.2.4	5.32	N.2.D1	34.65	0.49	0.52		0.17	0.40	2.49	9.23	198	190	299	290	19.3	19.3	21.0	21.6	90	111	114		
		N.2.5	10.98	N.2.D2	10.98	0.48	0.52	3.52	249	193	0.23	3.34	242	166	491	379	12.1	15.4	13.2	16.8	96	17	28		
		N.2.6	11.19	N.2.E	56.82	0.52	0.57		0.32	0.35	1.56	9.23	242	190	491	379	12.1	15.4	13.2	16.8	96	65	91		
		N.2.7	7.48	N.2.F	64.30	0.52	0.58		0.32	0.35	2.25	13.04	442	271	542	472	11.1	12.6	12.1	13.7	84	97	122		
		N.2.8	13.93	N.2.G1	78.22	0.53	0.59		0.31	0.35	4.37	17.41	674	579	776	680	8.0	9.0	8.8	9.9	83	97	122		
		N.2.9	6.32	N.2.G2	6.32	0.50	0.49	2.16	123	123	0.30	3.10	172	172	295	295	19.5	19.5	21.3	21.3	98	16	16		
		N.2.10	5.54	N.2.G3	11.86	0.50	0.53		0.33	0.35	2.49	5.59	208	201	421	413	13.9	14.2	15.2	15.5	96	22	24		
N.2.11	5.71	N.2.H	90.09	0.52	0.59		0.31	0.35	1.57	17.41	674	579	776	680	8.0	9.0	8.8	9.9	81	97	122				
N.2.12	4.89	N.2.I	65.79	0.53	0.59		0.31	0.35	2.50	18.98	760	654	860	755	7.3	8.2	8.1	9.0	81	97	122				
N.2.13	6.89	N.2.J	100.68	0.53	0.59		0.31	0.35	2.61	21.48	894	775	994	874	6.4	7.2	7.1	8.0	81	97	122				
			107.57	0.53	0.60				24.09	103.5	897	897	1135	998	5.7	6.4	6.4	7.1	81	97	122				

C = Runoff coefficient, L_r = Inlet length, T_r = Inlet time, V_r = Flow velocity, L_r = Flow length, T_r = Time of concentration, I = Rainfall intensity, f = Areal reduction factor, Q_p = Peak runoff

Table C.6.10 DESIGN DISCHARGES OF THE CANALS BY RATIONAL METHOD (1/2)

Drainage Area	Catchment ID	Area (km ²)	ID	Canal Name	Runoff Area		Kunoff Coefficient	Segment Flow		Time of Concentration (minutes)	Rainfall Intensity		Areal Reduction Factor	Design Discharge 5-Yr. R.P. (m ³ /s)	Design Discharge 10-Yr. R.P. (m ³ /s)
					ID	Area (km ²)		Length (km)	Velocity (m/s)		5-Yr. R.P. (mm/hr)	10-Yr. R.P. (mm/hr)			
Northern Zone	N.1	19.87	C-N.1	Kach Ben Da - Rach Ba Hong *	N.1.A	3.33	0.31	1.07	0.35	101	54.9	61.5	0.99	25	36
					N.1.B	9.93	0.55	2.76	0.35	233	24.5	26.8	0.97	36	37
					N.1.C	14.17	0.52	2.04	0.35	301	19.1	20.8	0.96	37	37
					N.1.D	19.87	0.51	3.61	0.36	421	13.9	15.2	0.94	37	37
					N.2.A	11.67	0.48	0.97	0.70	124	46.0	51.5	0.96	69	69
	N.2	107.57	C-N.2	Rach Dai Han **	N.2.B	16.10	0.47	1.68	0.70	163	35.4	39.4	0.94	78	78
					N.2.C	20.35	0.50	4.09	0.60	277	30.7	32.6	0.91	78	78
					N.2.D	34.65	0.52	2.46	0.40	381	15.3	16.7	0.90	78	78
					N.2.E	10.98	0.30	3.34	0.30	370	13.4	16.8	0.96	28	28
					N.2.F	36.82	0.37	1.56	0.35	453	13.0	14.2	0.86	99	108
Central Zone	C.1	31.67	C-C.1	Kach Nhim Loo - Rach Thi Nigte	N.2.G	64.30	0.58	2.25	0.35	562	10.7	11.7	0.84	96	108
					N.2.H	78.22	0.59	4.37	0.35	770	8.1	8.9	0.83	96	108
					N.2.I	6.32	0.49	3.10	0.20	295	19.5	21.3	0.98	16	16
					N.2.J	11.86	0.53	2.49	0.35	413	14.2	15.5	0.96	24	24
					N.2.K	95.79	0.59	1.57	0.35	645	7.4	8.2	0.81	99	108
	C.2	5.14	C-C.2	Kach Cau Son - Kach Tau Yam Tar	N.2.L	100.68	0.59	2.50	0.35	964	6.6	7.3	0.81	99	108
					N.2.M	107.57	0.60	2.61	0.35	1088	5.9	6.6	0.81	99	108
					N.2.N	7.54	0.76	0.55	1.50	76	62.2	75.4	0.98	104	117
					N.2.O	11.06	0.76	2.14	0.70	129	44.2	49.4	0.96	104	117
					N.2.P	16.96	0.77	1.77	0.70	171	33.8	37.6	0.95	116	129
C.3	20.22	C-C.3	Rach Tan Hoa - Rach Lo Gom	N.2.Q	24.54	0.76	2.32	0.70	227	25.2	27.5	0.93	121	131	
				N.2.R	31.67	0.76	2.15	0.70	278	20.7	22.5	0.91	126	137	
				N.2.S	5.14	0.78	2.09	0.40	116	48.5	54.3	0.93	117	131	
				N.2.T	5.86	0.79	2.26	1.40	72	71.1	79.8	0.98	90	90	
				N.2.U	10.58	0.74	1.97	1.20	100	55.4	62.1	0.97	55.4	62.1	
C.4	41.20	C-C.4	Kach Ruo Nguia	N.2.V	14.35	0.76	2.84	0.60	176	32.5	36.1	0.95	117	117	
				N.2.W	3.88	0.58	1.50	0.40	135	42.6	47.6	0.98	47	47	
				N.2.X	6.73	0.61	3.74	0.40	154	37.4	41.7	0.99	16	18	
				N.2.Y	1.84	0.67	3.35	0.40	227	25.2	27.5	0.90	131	131	
				N.2.Z	3.41	0.62	2.56	0.60	190	30.1	33.0	0.96	18	18	
W.1	72.91	C-W.1	Kach Chua - Rach Nuoc Lun ***	N.2.AA	6.52	0.58	3.18	0.40	344	16.8	18.3	0.98	18	18	
				N.2.AB	36.71	0.71	2.02	0.40	369	15.8	17.2	0.90	117	131	
				N.2.AC	44.22	0.71	1.40	0.40	427	13.8	15.0	0.88	117	131	
				N.2.AD	50.86	0.71	1.97	0.40	509	11.7	12.8	0.87	117	131	
				N.2.AE	61.73	0.71	4.25	0.40	687	8.9	9.8	0.85	117	131	
Western Zone	W.1	43.26	C-W.1	Kach Chua - Rach Nuoc Lun ***	N.2.AF	9.68	0.68	2.60	0.30	154	37.5	41.8	0.97	65	65
					N.2.AG	14.81	0.59	2.05	0.40	239	23.9	26.1	0.95	65	72
					N.2.AH	19.55	0.55	4.11	0.40	411	14.3	15.6	0.94	65	72
					N.2.AI	8.62	0.52	2.20	0.30	311	27.2	29.7	0.97	33	33
					N.2.AJ	30.36	0.57	4.49	0.40	598	10.1	11.1	0.90	65	72
W.1	43.26	C-W.2	Rach Nhim	N.2.AK	3.20	0.69	2.07	0.30	167	34.7	38.7	0.99	21	21	
				N.2.AL	43.26	0.58	2.72	0.40	711	8.7	9.5	0.88	65	72	
				N.2.AM	43.26	0.58	4.90	0.30	346	16.8	18.3	0.98	12	12	
				N.2.AN	6.04	0.44	4.90	0.30	402	14.5	15.9	0.98	14	15	
				N.2.AO	7.02	0.50	1.36	0.40	402	14.5	15.9	0.98	14	15	
W.1	50.55	C-W.3	Kach Cai Thong - Kach Ba Coc	N.2.AP	50.55	0.57	1.18	0.40	760	8.2	9.0	0.87	65	72	
				N.2.AQ	50.55	0.57	1.85	0.40	837	7.3	8.3	0.85	65	72	
				N.2.AR	59.41	0.57	1.85	0.40	837	7.3	8.3	0.85	65	72	
				N.2.AS	72.91	0.56	1.76	0.40	910	7.0	7.7	0.83	65	72	
				N.2.AT	72.91	0.56	1.76	0.40	910	7.0	7.7	0.83	65	72	

* Canal improvement plan for Rach Ben Da - Ba Hong has been proposed considering inundation in the upper two reaches. The design discharges shown in this table represent discharges under inundation condition.
 ** Canal improvement plan for Rach Dai Han has been proposed considering inundation. Design (5-year) discharges through main channel at runoff points N.2.A, N.2.B, N.2.C and N.2.D are 7, 11, 19 and 24 m³/s respectively.
 *** The design discharges along Rach Dai Han, as shown in this table represent total discharges through main channel and flood plain at different runoff points, under inundation condition.
 **** Canal improvement plan for Kach Chua has been proposed considering inundation. Design (5-year) discharges through main channel for lengths of 0.35 and 3.11 km along reaches W.1.B and W.1.C1 are 12 and 14 m³/s respectively. The design discharges along Kach Chua (runoff points W.1.B and W.1.C1), as shown in this table represent total discharges through main channel and flood plain, under inundation condition.

Table C.6.10 DESIGN DISCHARGES OF THE CANALS BY RATIONAL METHOD (2/2)

Drainage Area	Catchment ID	Area (km ²)	Canal Name	Runoff Point ID	Area (km ²)	Kunoff Coefficient	Segment's Flow Length (km)	Segment's Flow Velocity (m/s)	Time of Concentration (minutes)	Rainfall Intensity 3-Yr. R.P. (mm/hr)	Rainfall Intensity 10-Yr. R.P. (mm/hr)	Areal Reduction Factor	Design Discharge	
													5-Yr. R.P. (m ³ /s)	10-Yr. R.P. (m ³ /s)
Southern Zone	S.1	14.33	C-S.1	Rach Ba Lao	S.1.A	2.68	0.37	2.54	0.40	131	41.8	46.7	0.95	181
			S.1.B	6.72	0.33	2.51	0.40	242	23.7	25.8	0.98	23		
			S.1.B.2	4.38	0.55	2.17	0.40	217	26.3	28.7	0.90	17		
			S.1.B.3	6.33	0.32	2.06	0.40	303	19.0	20.7	0.98	17		
			S.1.C	14.33	0.52	1.39	0.40	342	16.9	18.4	0.95	34		
	C-S.3	Rach Ong Lon - Kim Cay Kho	S.2.A	2.40	0.48	2.02	0.40	158	36.6	40.8	0.99	12		
	C-S.4	Rach Ong Bay	S.2.B	1.96	0.59	1.90	0.40	150	38.4	42.9	0.99	12		
	C-S.5	Rach Ong Lon - Kim Cay Kho	S.2.C	7.21	0.52	1.51	0.40	218	26.2	28.6	0.98	26		
	C-S.6	Rach Ong Lon - Kim Cay Kho	S.2.C	15.66	0.53	3.39	0.40	359	16.2	17.6	0.95	36		
	C-S.5	Rach Roi - Rach Tom - Song Muong Choui	S.3.A	2.17	0.71	1.57	0.40	123	46.1	51.5	0.99	20		
C-S.6	Rach Thay Tieu	S.3.B	4.49	0.73	2.43	0.40	223	25.5	27.8	0.99	23			
C-S.7	Rach Dia	S.3.B.2	4.79	0.74	3.12	0.40	196	29.0	31.7	0.98	24			
C-S.5	Rach Roi - Rach Tom - Song Muong Choui	S.3.C	9.75	0.65	2.56	0.40	299	19.7	21.5	0.96	17			
C-S.8	Phuoc Khum River	S.3.D	19.33	0.65	2.60	0.40	333	17.4	18.9	0.94	57			
C-S.5	Rach Roi - Rach Tom - Song Muong Choui	S.3.D.2	6.45	0.65	2.63	0.40	442	13.3	14.5	0.93	57			
C-S.5	Rach Roi - Rach Tom - Song Muong Choui	S.3.E	34.51	0.63	2.81	0.40	259	22.2	24.1	0.98	21			
S.4	2.36	C-S.9	Rach Cau Kim	S.4.A	2.36	0.67	1.92	0.40	133	41.1	48.2	0.99	19	
S.5	2.23	C-S.10	Rach AP3 Phu My	S.5.A	2.23	0.60	1.86	0.40	116	48.5	54.5	0.99	18	
North-Eastern Zone	NE.1	3.52	C-NE.1	Rach Ong Dau	NE.1.A	3.23	0.57	3.38	0.40	170	34.0	37.9	0.99	18
	NE.2	9.53	C-NE.2	Rach Go Dau	NE.2.A	4.78	0.68	2.57	0.40	172	33.6	37.4	0.98	30
	NE.3	7.15	C-NE.3	Rach Thu Duc	NE.3.A	7.15	0.65	2.20	0.40	264	21.7	23.7	0.97	36
	NE.4	2.65	C-NE.4	Rach Trong Tho	NE.4.A	2.65	0.73	3.14	0.40	161	35.8	39.9	0.98	44
	NE.5	34.38	C-NE.5	Rach Nhum - Rach Cau - Rach Go Cong*	NE.5.A	10.12	0.42	3.35	0.33	63	0.0	82.4	0.97	83
	NE.5.A.2	4.72	0.46	2.54	0.40	65	0.0	89.2	0.98	47				
	NE.5.B	17.99	0.45	1.70	1.84	85	65.2	71.0	0.94	133				
	NE.5.C	28.17	0.45	2.63	0.77	146	39.5	44.1	0.92	133				
	NE.5.D	34.38	0.45	4.44	0.52	178	15.4	16.8	0.90	133				
	NE.5.E	34.38	0.45	2.32	0.40	128	44.7	50.0	0.99	12				
South-Eastern Zone	SE.1	1.98	C-SE.1	Rach Binh Khaich	SE.1.A	1.98	0.50	2.08	0.40	118	48.1	53.8	0.99	22
	SE.2	2.60	C-SE.2	Rach Ca Tre Nho	SE.2.A	2.60	0.65	2.30	0.40	205	27.4	30.5	0.99	12
	SE.3	1.92	C-SE.3	Rach Da Do	SE.3.A	1.92	0.80	2.00	0.40	200	24.9	28.9	0.98	18
	SE.4	7.80	C-SE.4	Rach Gieng Ong To	SE.4.A	5.40	0.43	3.41	0.40	200	24.9	28.9	0.97	19
	SE.5	3.83	C-SE.5	Rach Muong	SE.5.A	3.83	0.44	2.05	0.40	285	20.2	21.9	0.97	19
	SE.6	5.10	C-SE.6	Rach Ky Ha	SE.6.A	3.10	0.50	4.39	0.40	263	21.8	23.2	0.98	24
	SE.7	14.58	C-SE.7	Rach Chieu - Rach Kinh Ong Hong	SE.7.A	8.39	0.56	1.90	0.40	191	30.0	32.8	0.97	38
	SE.8	11.33	C-SE.8	Rach Ong Cay - Rach Ba Cau - Rach Ong Kieu	SE.8.A	2.77	0.40	1.95	0.40	164	35.3	39.3	0.99	12
	SE.9	21.11	C-SE.9	Rach Tin - Rach Ong Nheu	SE.9.A	7.46	0.65	2.47	0.40	147	39.1	43.6	0.98	31
	SE.10	24.88	C-SE.10	The River	SE.10.A	7.57	0.65	3.02	0.40	265	21.6	23.0	0.98	19
SE.10.B	19.00	0.52	4.08	0.00	435	13.5	14.7	0.94	35					
SE.10.C	59.26	0.54	2.41	0.40	458	12.9	14.1	0.85	133	149				

* Canal improvement plan for Rach Go Cong has been proposed considering construction of on-site storage ponds to reduce peak runoff due to rapid urbanization such that discharges under existing landuse condition can be kept. The design discharges along Rach Go Cong shown in this table represent discharges under existing landuse condition.

Table C.7.2 RESULTS OF HYDRODYNAMIC MODELING (1/2)

Canal System	Drainage Area	Catchment ID	Runoff Point	Design Discharge by Rational Formula	Proposed Bank Level (EL m)	Scenario 1: Constant Water Level Boundary Condition				Scenario 2: Dynamic Water Level Boundary Condition											
						5-Year Return Period		10-Year Return Period		5-Year Return Period		10-Year Return Period									
						W.L. (EL m)	Free Board (m)	Discharge (m ³ /s)	W.L. (EL m)	Free Board (m)	Discharge (m ³ /s)	W.L. (EL m)	Free Board (m)	Discharge (m ³ /s)	W.L. (EL m)	Free Board (m)	Discharge (m ³ /s)				
Northern Zone		N.2	N.2.D2	10.98	2.02	1.65	0.37	27	-8	1.63	0.39	29	-5	1.52	0.50	57	-31	1.52	0.50	62	-35
				56.82	99	1.64	0.35	70		1.63	0.36	80		1.52	0.47	82	-42	1.51	0.48	90	-50
				64.30	99	1.63	0.31	80		1.61	0.33	92		1.52	0.42	94	-63	1.51	0.43	100	-72
				78.22	99	1.58	0.27	85		1.56	0.29	98		1.48	0.37	134	-95	1.47	0.38	150	-109
				6.32	16	1.60	0.30	11		1.59	0.31	12		1.51	0.39	151	-19	1.51	0.39	17	-21
				11.86	24	1.58	0.27	23		1.56	0.29	26		1.48	0.37	29	-20	1.47	0.38	32	-34
				95.79	99	1.53	0.29	124		1.52	0.31	141		1.46	0.36	168	-127	1.45	0.38	190	-166
				100.68	99	1.44	0.33	137		1.43	0.34	155		1.40	0.37	184	-147	1.39	0.38	208	-166
				107.57	99	1.32	0.40	156		1.32	0.40	177		1.30	0.40	209	-108	1.32	0.40	236	-186
				5.86	90	2.05	0.71	72		2.17	0.59	81		1.62	1.14	69	-15	1.77	0.99	77	-15
				10.58	117	1.58	0.39	112		1.62	0.35	125		1.40	0.57	168	-27	1.41	0.56	117	-27
				14.35	117	1.43	0.40	134		1.44	0.39	150		1.38	0.43	112	-63	1.38	0.45	129	-62
5.88	47	1.43	0.40	35		1.44	0.39	40		1.38	0.45	35	-18	1.38	0.45	39	-18				
Central Zone		C.3	C.3.A	1.84	16	1.42	0.41	66		1.42	0.41	65		1.38	0.45	45	-56	1.38	0.45	45	-61
				2.88	13	1.43	0.40	39		1.44	0.39	39		1.38	0.45	34	-52	1.38	0.45	34	-59
				3.41	18	1.42	0.41	18		1.42	0.41	18		1.38	0.45	17	-39	1.38	0.45	18	-40
				6.52	18	1.42	0.47	12		1.43	0.46	13		1.40	0.49	20	-24	1.41	0.48	20	-24
				36.71	117	1.42	0.41	21		1.42	0.41	24		1.38	0.45	44	-56	1.38	0.45	44	-56
				44.22	117	1.40	0.40	48		1.40	0.40	51		1.35	0.45	171	-253	1.36	0.44	181	-254
				50.86	117	1.42	0.38	51		1.43	0.38	54		1.37	0.43	40	-46	1.37	0.43	41	-46
				61.73	117	1.39	0.41	176		1.39	0.40	181		1.34	0.45	208	-301	1.34	0.45	216	-302
				72	131	1.30	0.40	45		1.30	0.39	47		1.35	0.43	44	-60	1.35	0.43	45	-60
				72	131	1.37	0.41	217		1.37	0.41	226		1.34	0.43	236	-346	1.34	0.43	246	-347
				72	131	1.36	0.40	56		1.36	0.40	58		1.34	0.42	59	-45	1.34	0.42	59	-45
				72	131	1.32	0.40	383		1.32	0.40	394		1.32	0.40	491	-587	1.32	0.40	491	-585
Western Zone		W.1	W.1.A	9.68	65	1.63	0.45	60		1.62	0.46	51		1.47	0.61	43	-12	1.48	0.60	49	-12
				14.81	65	1.61	0.61	60		1.61	0.61	64		1.45	0.49	73	-29	1.48	0.48	82	-37
				19.55	72	1.53	0.45	46		1.53	0.45	46		1.40	0.58	85	-65	1.42	0.56	98	-79
				36.36	65	1.46	0.45	67		1.46	0.45	77		1.38	0.53	116	-69	1.39	0.53	133	-107
				43.26	65	1.41	0.46	80		1.42	0.45	92		1.38	0.50	133	-119	1.38	0.49	151	-129
				7.02	14	1.41	0.46	8		1.42	0.45	9		1.38	0.50	27	-34	1.38	0.49	27	-35
				50.55	65	1.41	0.46	89		1.42	0.46	102		1.37	0.50	175	-185	1.38	0.50	194	-194
				59.41	65	1.40	0.43	104		1.41	0.42	130		1.38	0.46	279	-450	1.38	0.46	279	-450
				72.91	65	1.39	0.40	112		1.39	0.40	130		1.39	0.40	324	-492	1.39	0.40	339	-494
				72	130	1.32	0.40	383		1.32	0.40	394		1.32	0.40	491	-587	1.32	0.40	491	-585
				72	130	1.32	0.40	383		1.32	0.40	383		1.32	0.40	383	-492	1.32	0.40	383	-492

Note: A +ve discharge indicates flow direction from HD simulation is the same as assumed flow direction, as shown in Fig. C.37 and vice versa.

A -ve free board at point W.1.B represents depth of inundation along the flood plain of Kinh Chua.

The discharges at runoff points NE.5.B, NE.5.C, NE.5.D and SE.10.C are for existing landuse condition.

The shaded portion represents designed condition.

Table C.7.2 RESULTS OF HYDRODYNAMIC MODELING (2/2)

Canal System	Drainage Area	Catchment ID	Runoff Point	Design Discharge by Rational Formula	Proposed Bank Level (E.L. m)	Scenario 1: Constant Water Level Boundary Condition				Scenario 2: Dynamic Water Level Boundary Condition										
						W.L. (E.L. m)	Free Board (m)	Discharge (m ³ /s)	10-Year Return Period Discharge (m ³ /s)	W.L. (E.L. m)	Free Board (m)	Discharge (m ³ /s)	10-Year Return Period Discharge (m ³ /s)							
Western Canal System	Southern Zone	S.1	S.1.A	2.08	18	20	1.40	0.45	11	12	1.39	0.46	18	-27	1.40	0.49	19	-27		
			S.1.B1	6.72	23	25	1.39	0.42	25	26	1	1.39	0.42	55	-85	1.39	0.42	55	-85	
			S.1.B2	4.34	17	19	1.40	0.44	-21	-21	1	1.38	0.46	22	-23	1.38	0.46	23	-35	
			S.1.B3	6.33	17	19	1.39	0.42	4	-21	1	1.39	0.42	40	42	1.39	0.42	42	-59	
		S.1.C	14.33	34	37	1.39	0.40	24	-25	1	1.39	0.40	210	-325	1.39	0.40	213	-325		
		S.2	S.2	S.2.A1	2.40	12	13	1.37	0.48	-171	-171	1	1.37	0.50	57	-160	1.37	0.50	57	-159
		S.2.A2		1.96	12	14	1.39	0.48	8	1	-8	1.37	0.50	9	-19	1.37	0.50	9	-19	
		S.2.B		7.21	29	29	1.39	0.45	-24	-24	1	1.37	0.47	57	-60	1.37	0.47	58	-99	
		S.2.C		15.66	36	39	1.39	0.40	-52	-52	1	1.39	0.40	109	-173	1.39	0.40	111	-173	
		S.3	S.3	S.3.A	2.17	20	22	1.42	0.54	13	14	1	1.40	0.55	13	-17	1.40	0.55	15	-17
		S.3.B1		4.48	23	25	1.40	0.51	23	26	1	1.38	0.53	38	-56	1.38	0.53	39	-56	
		S.3.B2		4.79	28	31	1.40	0.51	20	22	1	1.38	0.53	42	-55	1.38	0.53	43	-55	
S.3.B3	5.75	17		19	1.40	0.51	-103	-103	1	1.38	0.53	102	-181	1.38	0.53	103	-181			
S.3.C	19.33	57		62	1.40	0.47	103	103	1	1.38	0.49	188	-316	1.38	0.49	189	-314			
S.3.D2	23.68	57		62	1.39	0.44	-23	-23	1	1.38	0.45	77	-137	1.38	0.45	79	-137			
S.3.E	34.51	64		69	1.39	0.40	-125	-125	1	1.39	0.40	383	-651	1.39	0.40	391	-648			
S.3.F	17.92	33		35	1.60	0.45	114	127	1	1.50	0.55	114	127	1.50	0.55	127	127			
NE Zone	NE Zone	NE.5	NE.5.B	17.92	133	140	1.48	0.42	165	165	1	1.48	0.40	167	-2	1.48	0.40	167	-2	
		NE.5.C	28.17	169	176	1.48	0.42	163	170	1	1.47	0.42	170	-3	1.47	0.42	170	-3		
		NE.5.D1	34.38	183	189	1.48	0.42	163	170	1	1.48	0.42	170	-3	1.48	0.42	170	-3		
		NE.5.A	5.40	18	20	1.39	0.53	2	4	-13	1.39	0.53	4	-13	1.38	0.54	31	-49		
		NE.5.B	7.80	19	21	1.39	0.48	9	11	-13	1.39	0.48	11	-13	1.39	0.48	11	-13		
		NE.5.C	8.39	26	28	1.39	0.54	63	63	0.54	1.39	0.54	63	63	1.38	0.55	77	46		
		NE.5.D	14.58	38	42	1.32	0.55	123	126	0.55	1.32	0.55	164	-136	1.32	0.55	169	-145		
		NE.5.E	2.77	12	14	1.46	0.49	-24	-24	0.49	1.46	0.49	28	-25	1.44	0.51	27	-24		
		NE.5.F	11.33	27	30	1.47	0.40	3	6	0.40	1.47	0.40	21	-31	1.47	0.40	22	-31		
		NE.5.G	7.46	51	57	1.48	0.48	33	37	0.48	1.48	0.48	55	-7	1.48	0.48	59	-14		
South-Eastern Zone	South-Eastern Zone	SE.7	SE.7.A	8.39	26	28	1.47	0.44	-32	-32	1	1.48	0.44	47	-69	1.47	0.44	48	-76	
		SE.7.B	14.58	38	42	1.47	0.44	19	-31	0.40	1.47	0.40	25	-31	1.47	0.40	26	-31		
		SE.7.C	21.11	51	57	1.47	0.40	19	-31	0.40	1.47	0.40	25	-31	1.47	0.40	26	-31		
		SE.7.D	7.57	19	21	1.48	0.48	8	8	0.48	1.48	0.48	8	-54	1.48	0.48	8	-54		
		SE.7.E	19.00	35	38	1.48	0.42	1	-55	0.42	1.47	0.42	3	-57	1.47	0.42	3	-57		
		SE.7.F	59.26	133	140	1.47	0.40	165	170	0.40	1.47	0.40	170	-25	1.47	0.40	171	-25		
Eastern Canal System	Eastern Canal System	SE.8	SE.8.A	2.77	12	14	1.46	0.49	-24	-24	1	1.46	0.49	28	-25	1.44	0.51	27	-24	
		SE.8.B	11.33	27	30	1.47	0.40	3	6	0.40	1.47	0.40	21	-31	1.47	0.40	22	-31		
		SE.8.C	7.46	51	57	1.48	0.48	33	37	0.48	1.48	0.48	55	-7	1.48	0.48	59	-14		
		SE.8.D	13.73	51	57	1.47	0.44	-32	-32	0.44	1.47	0.44	47	-69	1.47	0.44	48	-76		
		SE.8.E	21.11	51	57	1.47	0.40	19	-31	0.40	1.47	0.40	25	-31	1.47	0.40	26	-31		
		SE.8.F	7.57	19	21	1.48	0.48	8	8	0.48	1.48	0.48	8	-54	1.48	0.48	8	-54		
		SE.8.G	19.00	35	38	1.48	0.42	1	-55	0.42	1.47	0.42	3	-57	1.47	0.42	3	-57		
		SE.8.H	59.26	133	140	1.47	0.40	165	170	0.40	1.47	0.40	170	-25	1.47	0.40	171	-25		
		SE.8.I	7.57	19	21	1.48	0.48	8	8	0.48	1.48	0.48	8	-54	1.48	0.48	8	-54		
		SE.8.J	19.00	35	38	1.48	0.42	1	-55	0.42	1.47	0.42	3	-57	1.47	0.42	3	-57		

Note: A +ve discharge indicates flow direction from HD simulation is the same as assumed flow direction, as shown in Fig. C.37 and vice versa.
 A -ve free board at point W.1.B represents depth of inundation along the flood plain of Kimh Chua.
 The discharges at runoff points NE.5.B, NE.5.C, NE.5.D and SE.7.D are for existing landuse condition.
 The shaded portion represents designed condition.

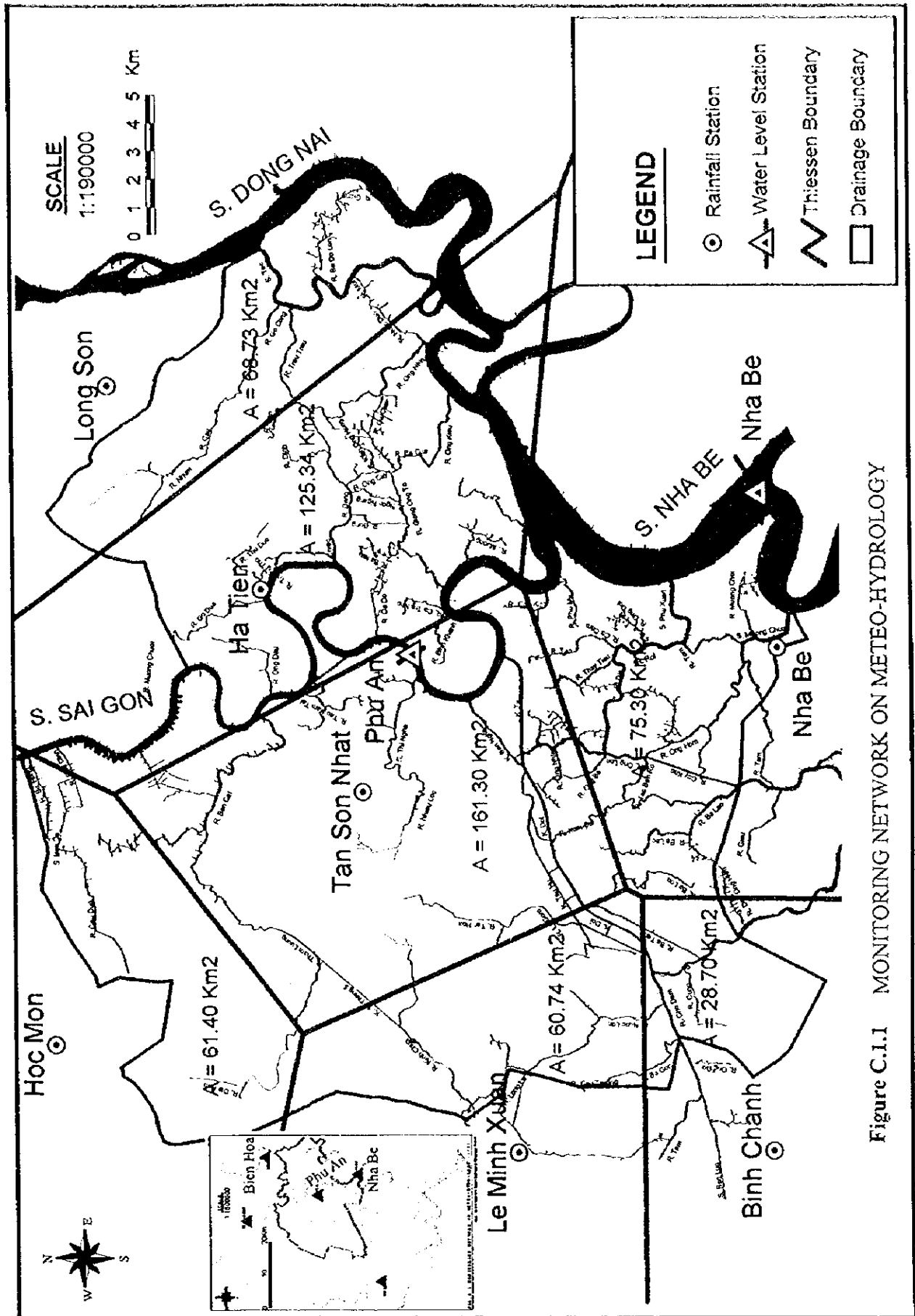
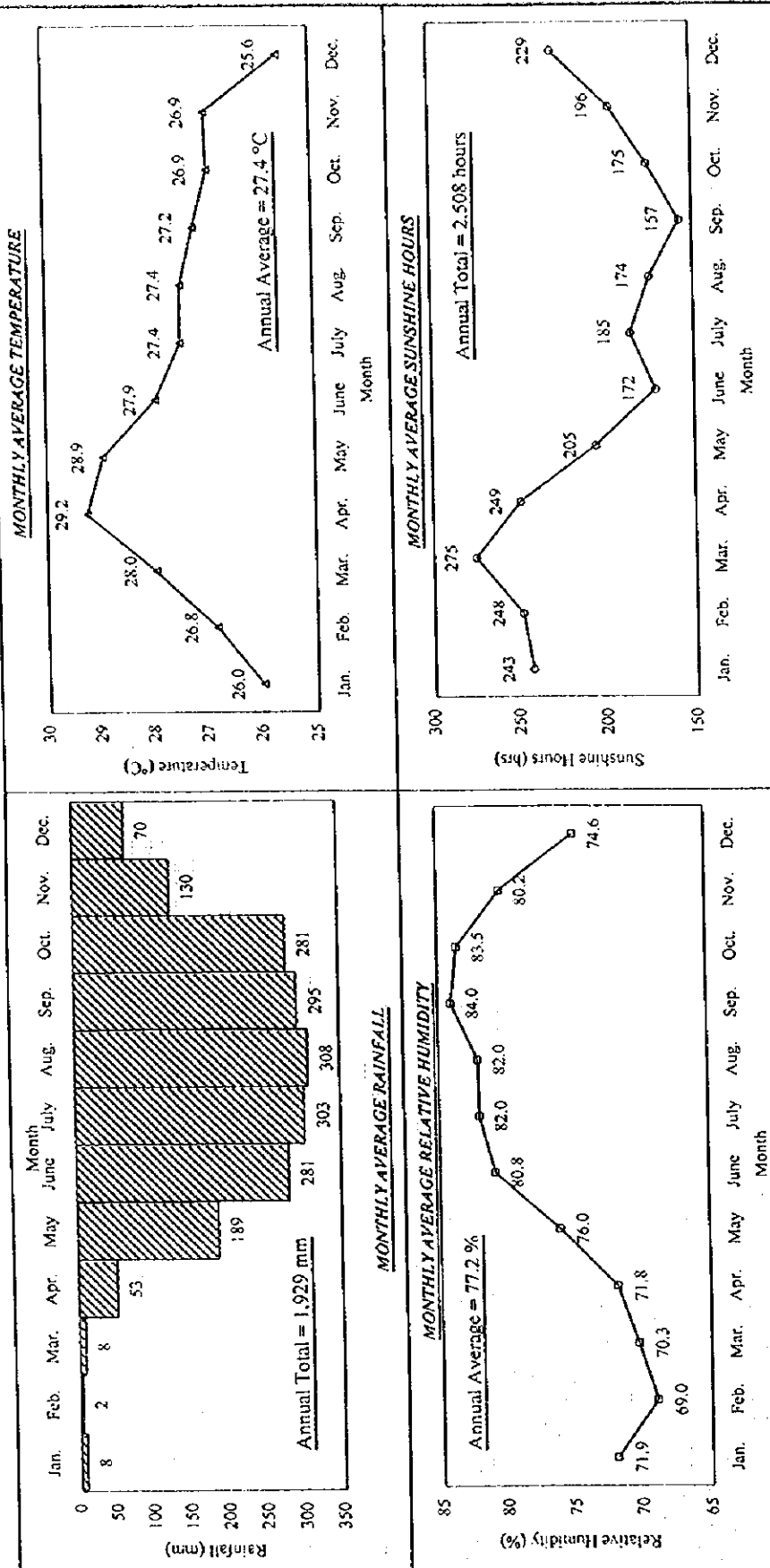


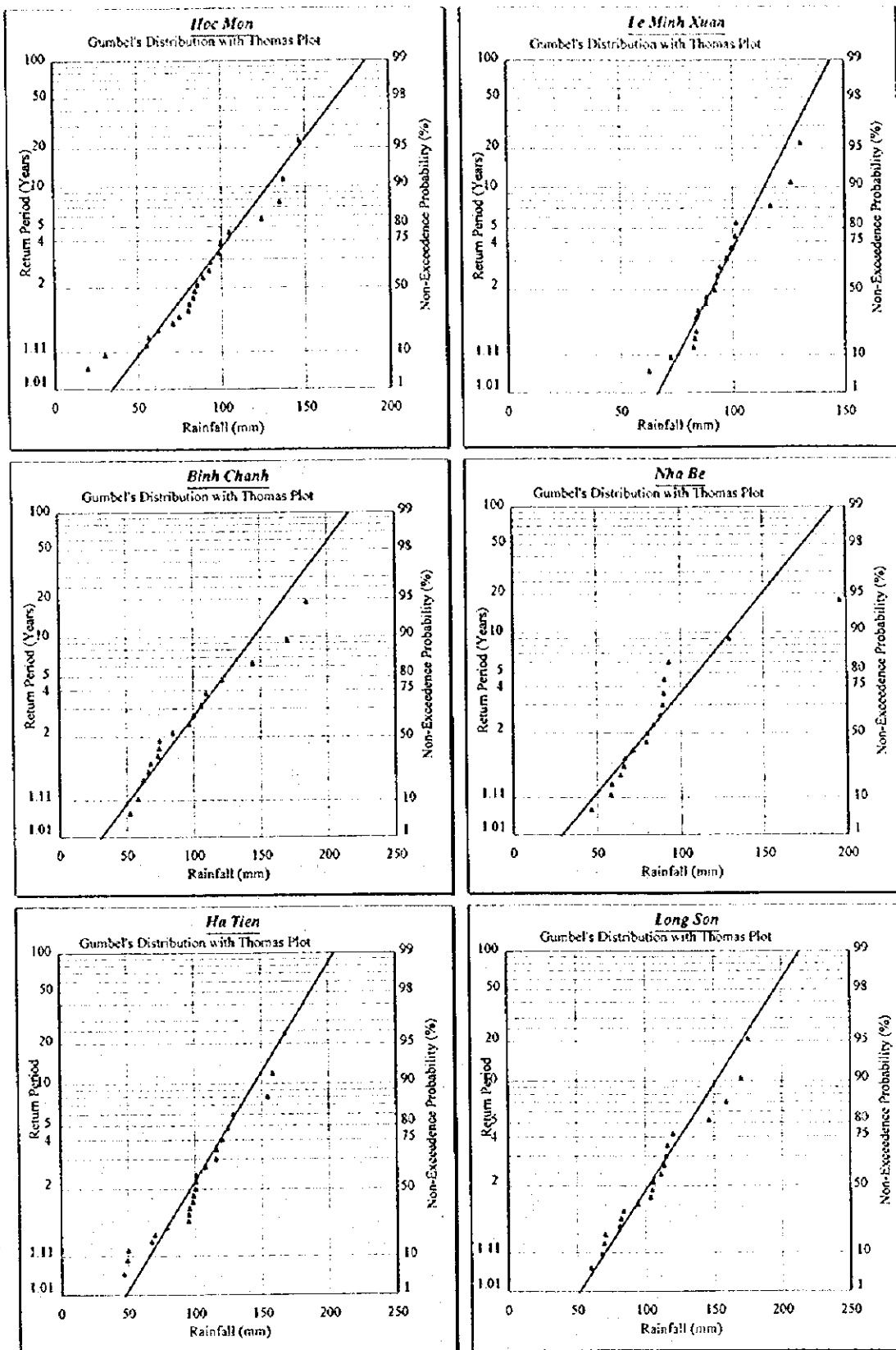
Figure C.I.1 MONITORING NETWORK ON METEO-HYDROLOGY



Data : From 1976 to 1997 at Tan Son Nhat

Data Source : Ho Chi Minh City Statistical Office

Figure C.2.1 GENERAL METEOROLOGICAL CONDITION AT TAN SON NHAT STATION



Data: 1973 - 1997

Figure C.3.1 PROBABILITY PLOTS OF DAILY RAINFALLS AT SIX STATIONS

Unit: mm

Station	Probable Maximum Daily Rainfall for Different Return Periods											
	1-Yr	1.5-Yr	2-Yr	3-Yr	5-Yr	10-Yr	20-Yr	25-Yr	30-Yr	50-Yr	70-Yr	100-Yr
Tan Son Nhat	51.05	83.58	92.22	102.27	113.47	127.54	141.03	145.31	148.80	158.56	164.66	171.59
Hoc Mon	28.84	69.82	81.25	94.55	109.36	127.97	145.87	151.49	156.09	168.93	177.35	185.25
Le Minh Xuan	62.42	84.84	90.29	97.10	104.68	114.22	123.36	126.26	128.62	135.19	139.50	144.06
Binh Chanh	22.03	74.52	88.47	104.71	122.80	145.53	167.33	173.24	179.87	195.55	205.83	216.70
Nha Be	21.18	67.32	79.70	94.01	109.94	129.96	149.17	155.26	160.21	174.02	183.08	192.65
Ha Tien	40.33	83.73	96.53	110.26	125.55	143.77	163.20	169.03	173.80	187.06	195.75	204.94
Long Son	41.16	89.78	101.91	116.02	131.73	151.49	170.44	176.45	181.31	194.96	203.90	213.34
Basin Mean	41.27	80.66	91.13	103.31	116.69	133.94	150.30	157.45	159.71	171.47	179.18	187.34

Probable Rainfalls: Calculated applying Gumbel's distribution method
 Basin Mean: Estimated applying Thiessen Polygon method
 Data: 1952 - 1997 for Tan Son Nhat and 1973 - 1997 for other stations

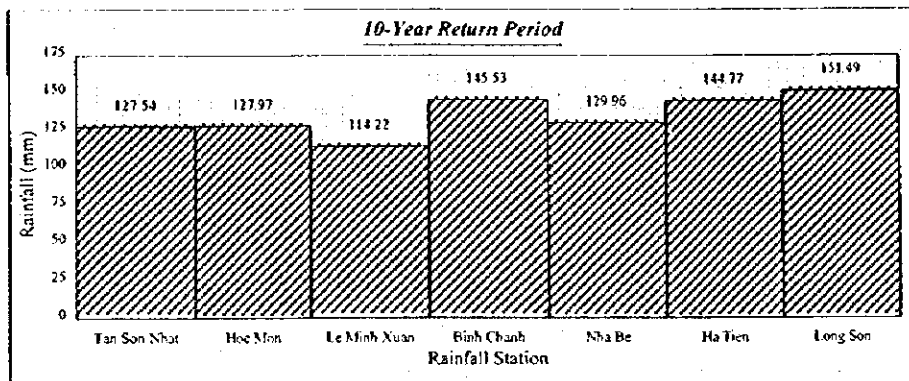
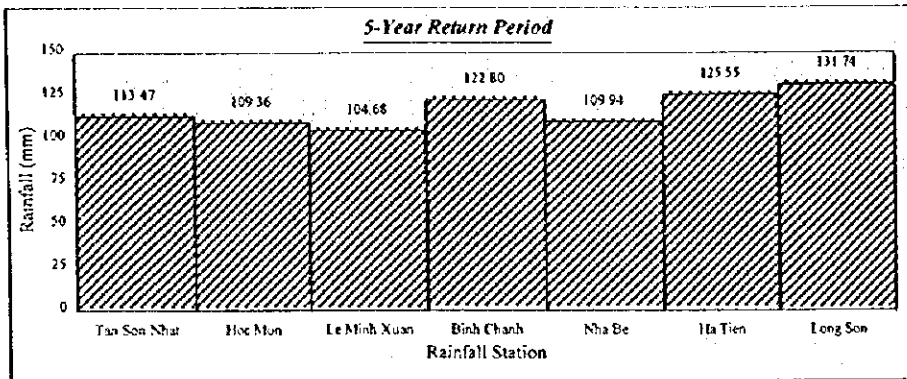
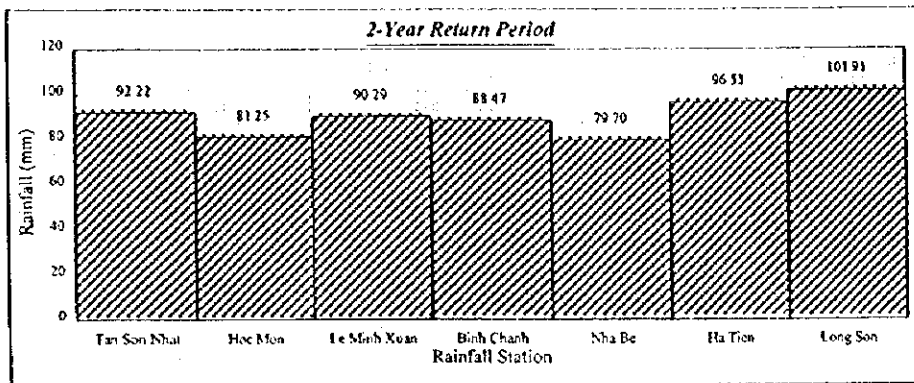
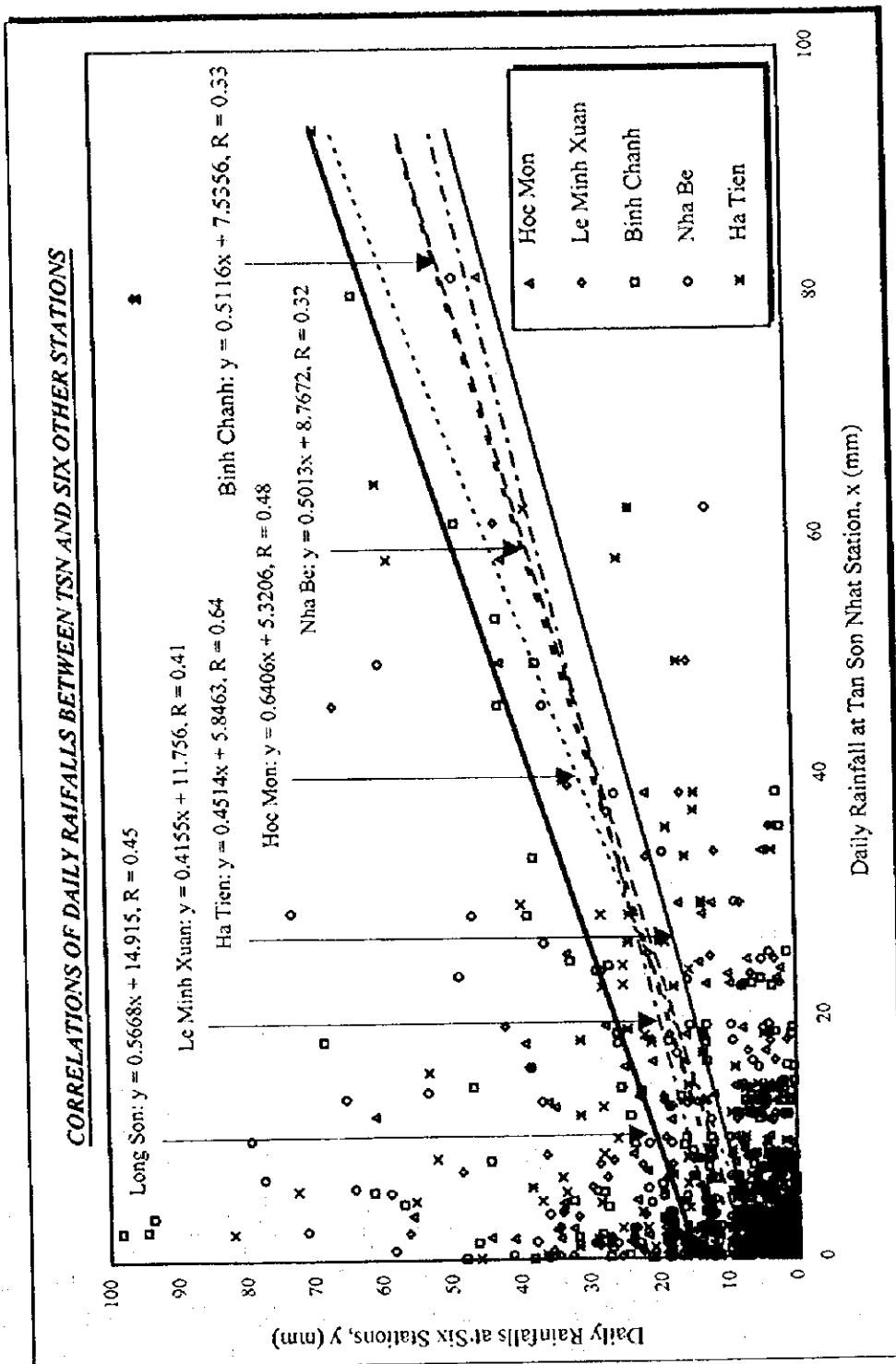
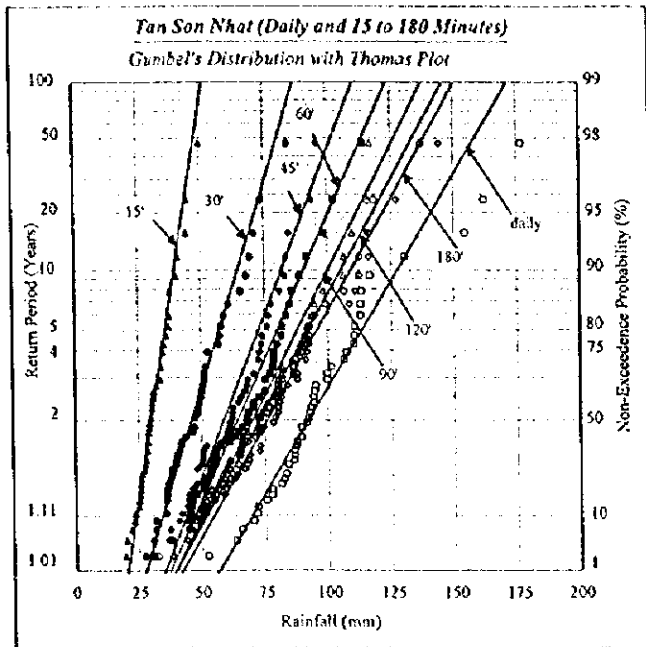


Figure C.3.2 PROBABLE MAXIMUM DAILY RAINFALLS

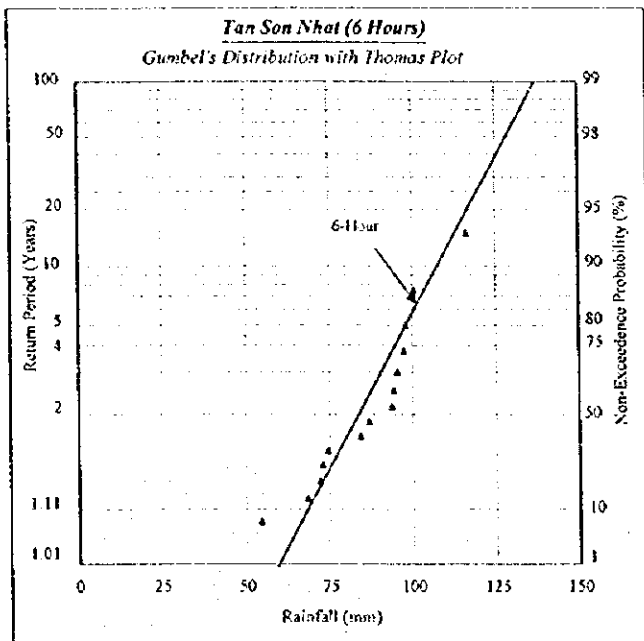


Data : Daily rainfalls for two years - 1994 and 1996.

Figure C.3.3 CORRELATIONS ON DAILY RAIFALLS BETWEEN TAN SON NHAT AND SIX OTHER STATIONS



Data : 1952 - 1997 = 46 years



Data : 1952, 1955, 1961-1962, 1967-1968, 1970, 1972, 1980, 1987, 1990, 1992, 1994 and 1996 = 14 years

Figure C.3.4 PROBABILITY PLOTS OF MAXIMUM RAINFALLS AT TAN SON NHAT

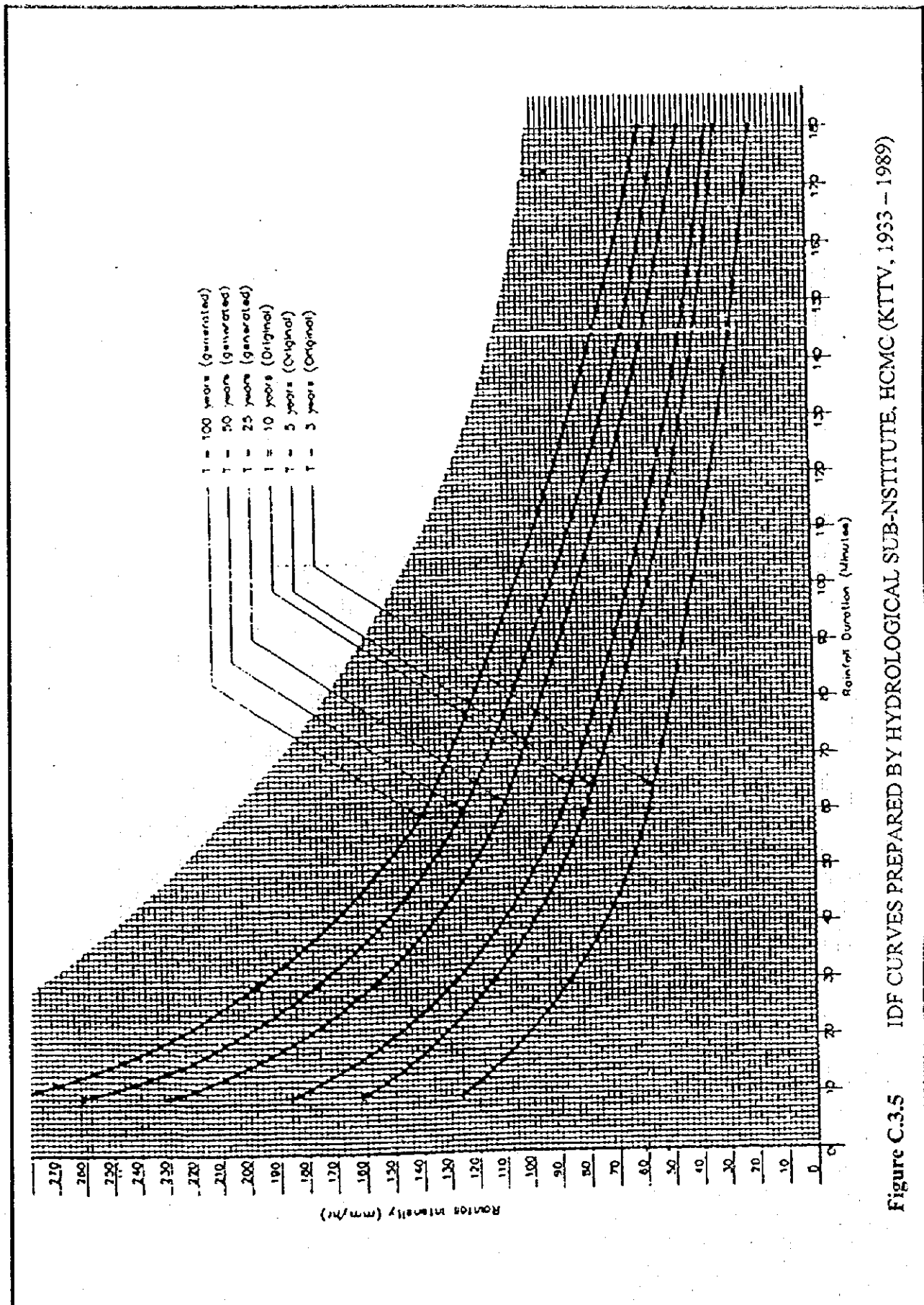


Figure C.3.5 IDF CURVES PREPARED BY HYDROLOGICAL SUB-NSTITUTE, HCMC (KTTV, 1953 - 1989)

RAINFALL INTENSITY - DURATION - FREQUENCY CURVE OBSERVED VALUES
OF HO CHI MINH CITY, 1953 - 1989

(BY MINISTRY OF THE CONSTRUCTION, HANOI JM 1985L)

FORMULA : $Q = \frac{\mu \cdot V \cdot q \cdot F}{1000} \text{ (M}^3\text{/S)}$

- WHERE :
- μ : RAINFALL DISTRIBUTIVE COEFFICIENT
($\mu = 1$ WHEN $F < 300 \text{ HA}$)
 - V : RUNOFF COEFFICIENT
 - Q : RAINFALL INTENSITY (LITERS/SECOND, HECTARE)
 - F : AREA OF THE WATERSHED (HECTARES)

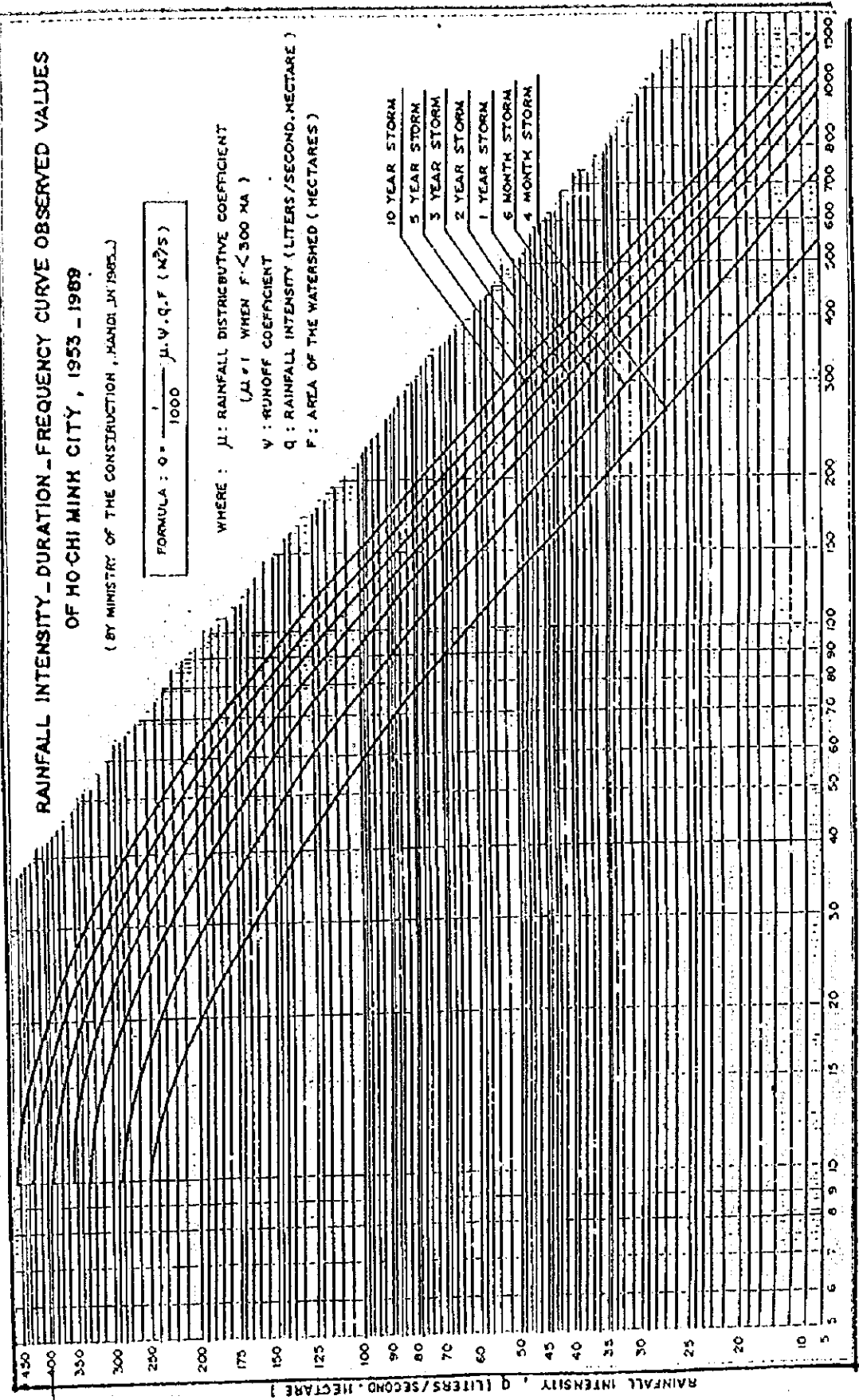
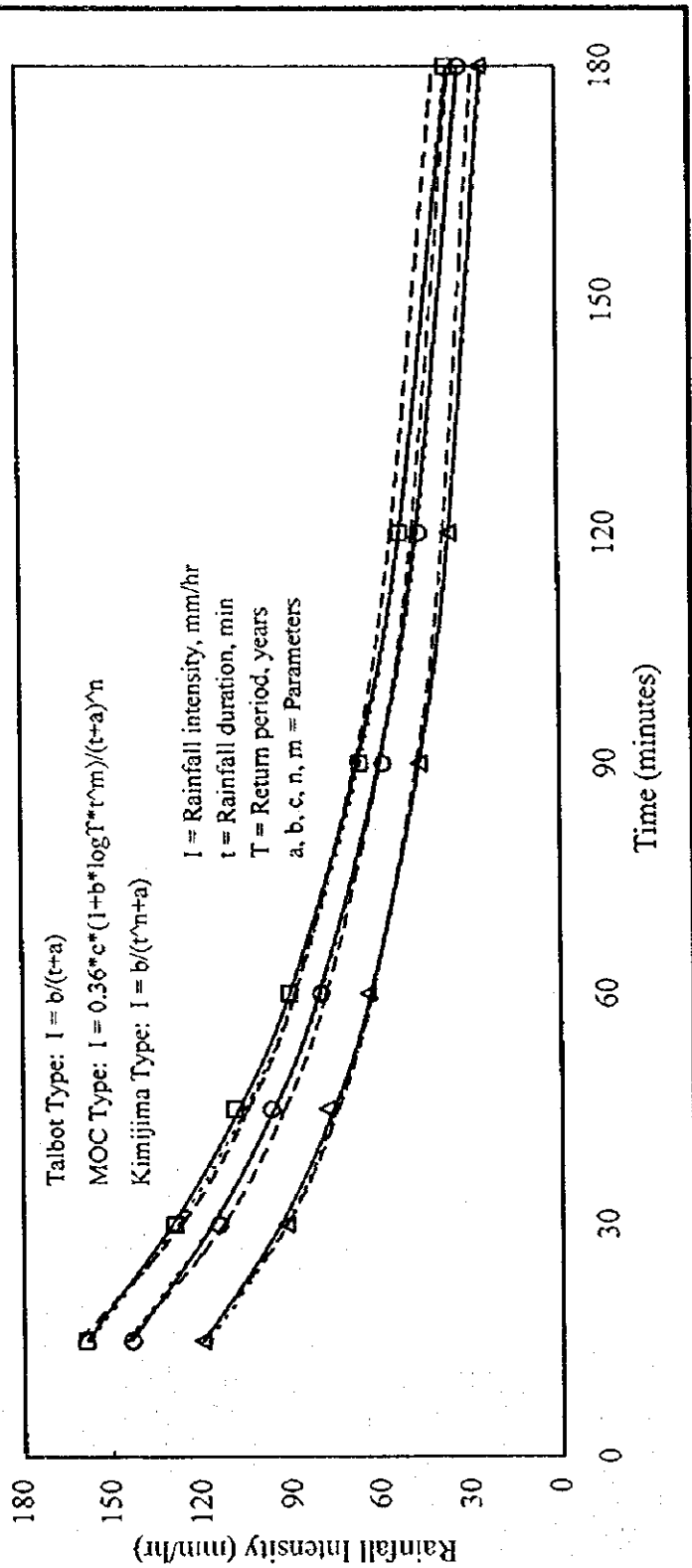


Figure C.3.6 IDF CURVES PREPARED BY MINISTRY OF CONSTRUCTION, HANOI (MOC 1953 - 1983)

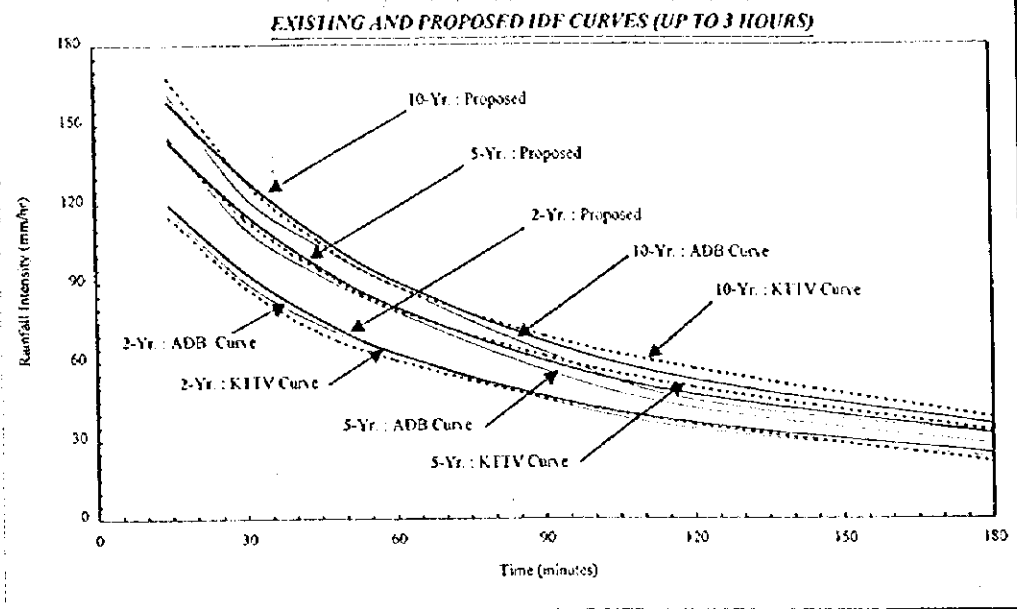
COMPARISON AMONG THREE EQUATIONS FOR IDF CURVES



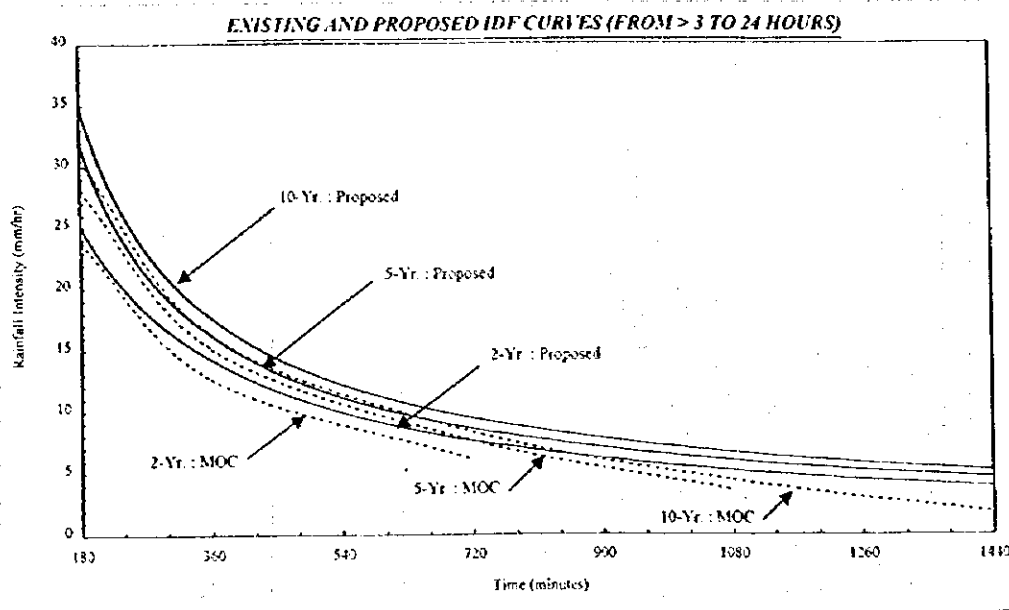
Note : Three types of equations have been compared to find the best fit IDF curve.
 The values of the points are the calculated probable maximum rainfall intensities using Gumbel's method.

Data : 1952 - 1997

Figure C.3.7 COMPARISON AMONG THREE TYPES OF EQUATIONS FOR IDF CURVES AT TSN



Note: Comparison has been made (for 2-Yr, 5-Yr and 10-Yr rainfall intensities) among Proposed IDF curves (1952 - 1997 data), fitted by Kimijima type equation with Existing IDF curves (1933 - 1939 data) of Hydrological Sub-Institute, HCMC (KTIV) and Existing IDF curves (1956 - 1994 data) of Asian Development Bank's Report (ADB)



Note: Comparison has been made (for 2-Yr, 5-Yr and 10-Yr rainfall intensities) between Proposed IDF curves (1952 - 1997 data), fitted by Kimijima type equation and Existing IDF curves (1953 - 1983 data) of Ministry of Construction, Hanoi (MOC)

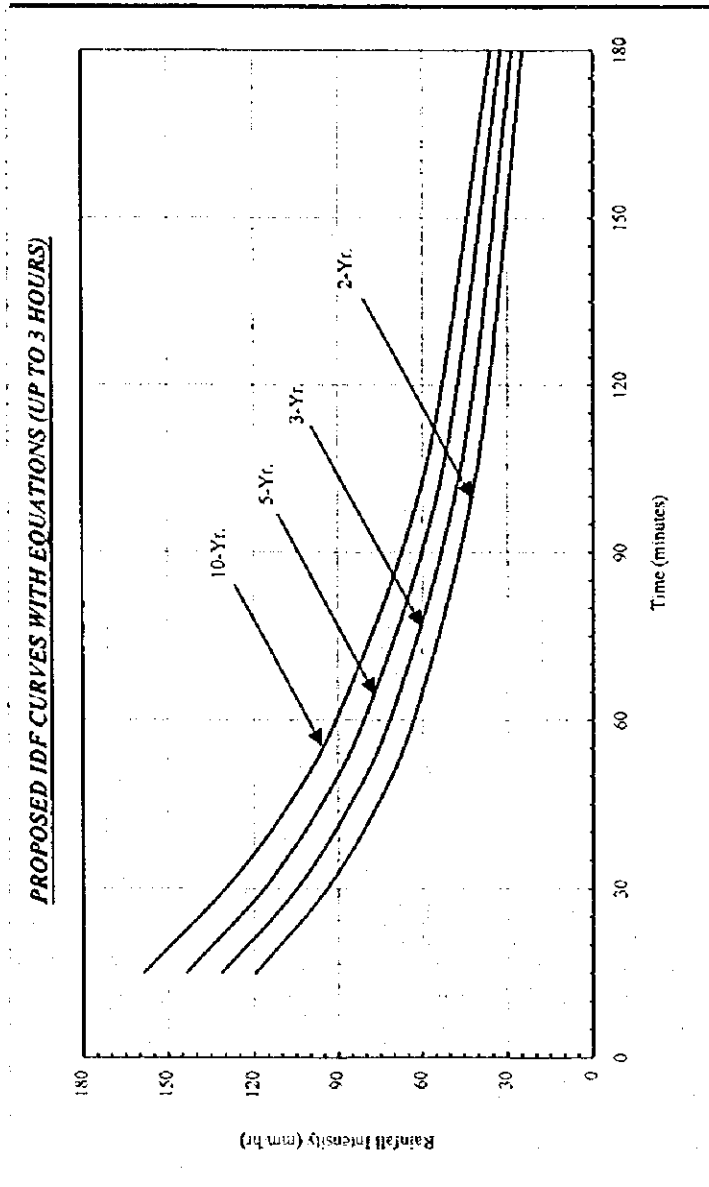
Figure C.3.8 COMPARISON BETWEEN EXISTING AND PROPOSED IDF CURVES

Proposed Equations of IDF Curves

Kimijima Type : $I = \frac{b}{t^{n+a}}$

I = Rainfall Intensity, mm/hr
t = Duration, minutes

Return Period (Years)	Parameter		
	b	n	a
1	3,055	1.04	24
1.5	10,633	1.15	74
2	13,567	1.18	89
3	17,439	1.20	107
5	22,294	1.22	128
10	29,125	1.25	154
20	36,410	1.27	179
25	38,836	1.28	186
30	40,861	1.28	192
50	46,733	1.29	209
70	50,683	1.30	220
100	54,976	1.31	231



Data : 1952 - 1997

Figure C.3.9 PROPOSED IDF CURVES WITH EQUATIONS AT TSN (UP TO 3 HOURS)

Proposed Equations of IDF Curves

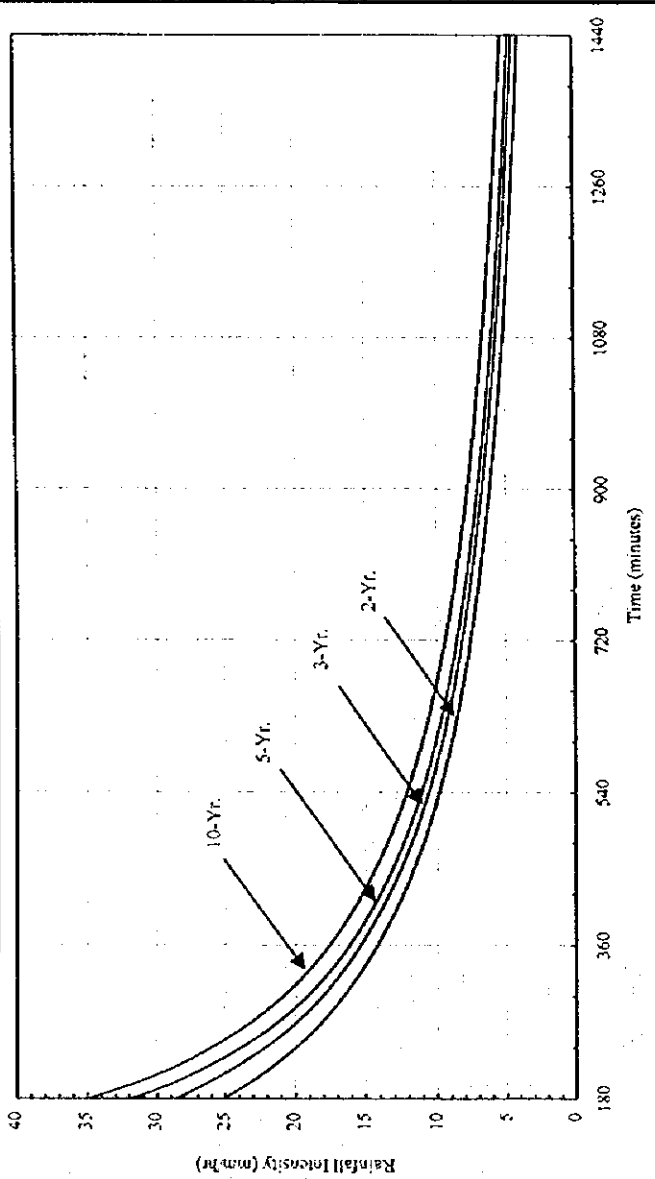
Kimijima Type : $I = \frac{b}{t^{n+a}}$

I = Rainfall Intensity, mm/hr

t = Duration, minutes

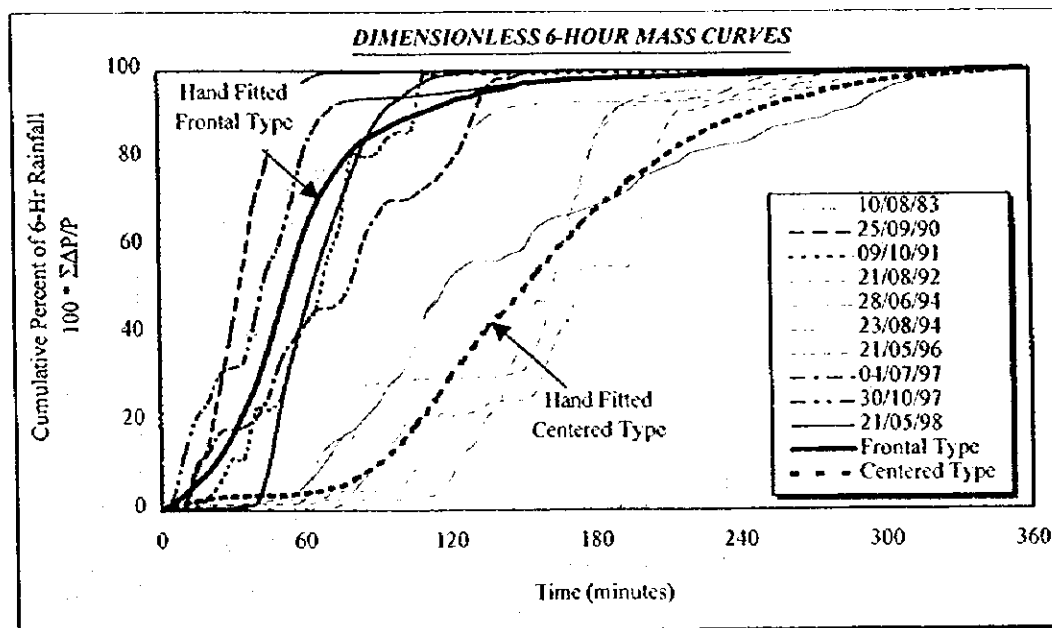
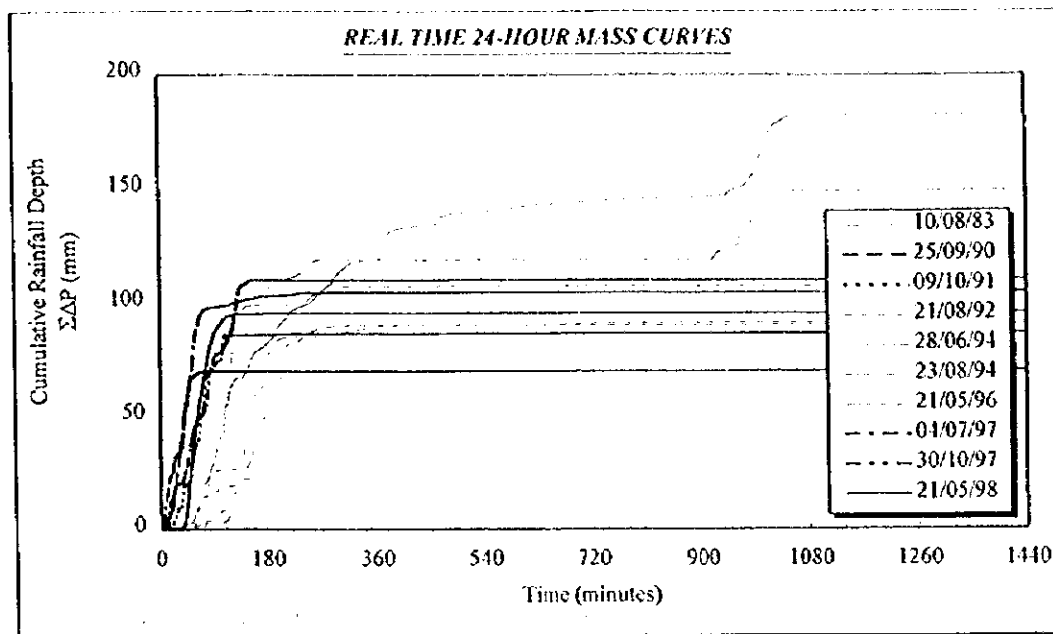
Return Period (Years)	Parameter		
	b	n	a
1	515,400	1.68	35.184
1.5	7,372	1.05	94
2	5,858	1.00	53
3	3,269	0.91	1
5	2,024	0.84	-15
10	1,669	0.80	-16
20	1,229	0.75	-17
25	1,305	0.75	-17
30	1,073	0.72	-17
50	886	0.69	-17
70	872	0.68	-16
100	875	0.68	-16

PROPOSED IDF CURVES WITH EQUATIONS (FROM > 3 TO 24 HOURS)



Data : 1952 - 1997

Figure C.3.10 PROPOSED IDF CURVES WITH EQUATIONS AT TSN (FROM MORE THAN 3 TO 24 HOURS)

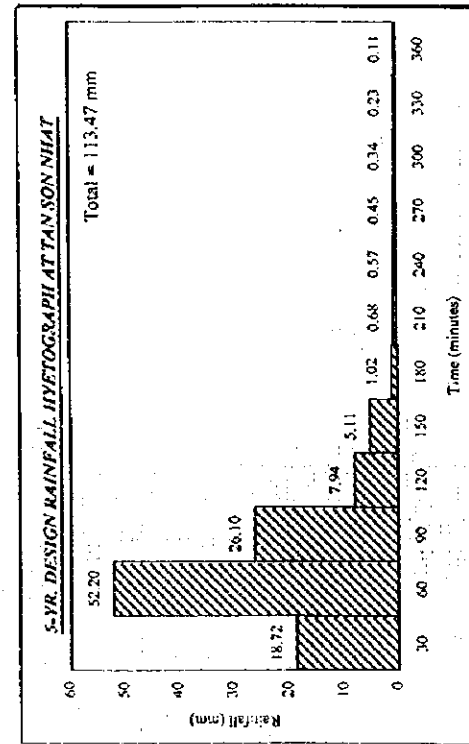
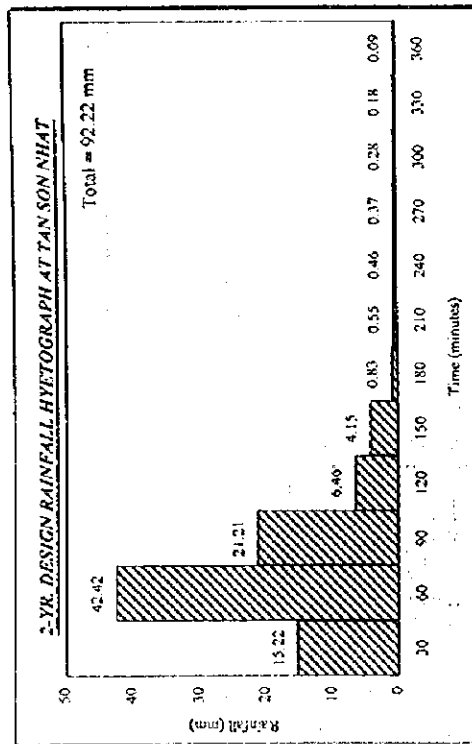
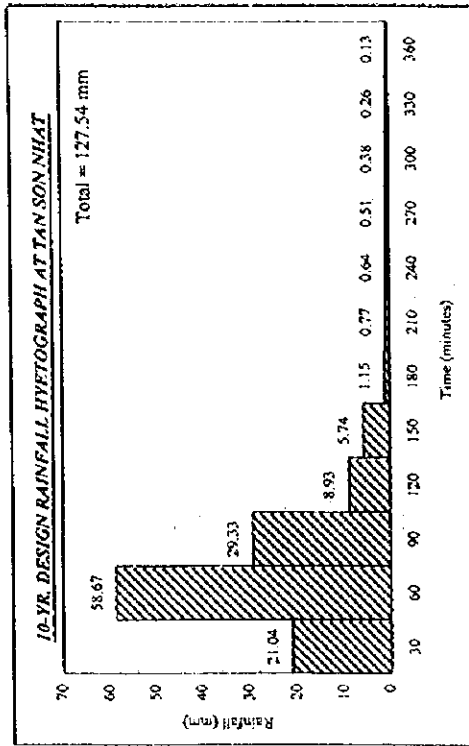
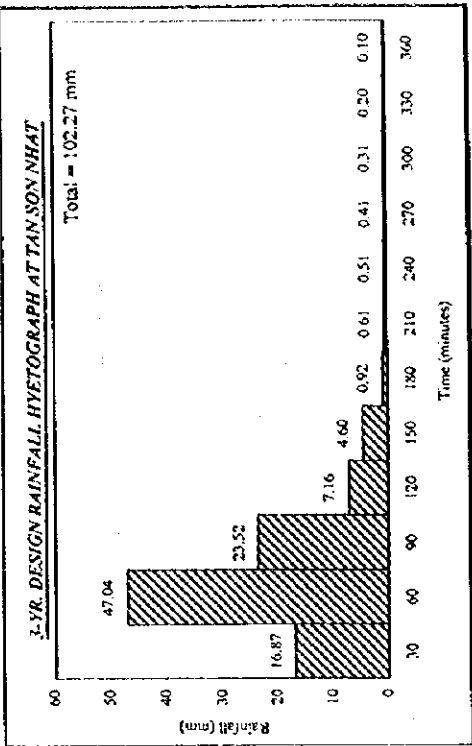


Nomenclature : P = Total rainfall depth (mm) in 6 hours
 $\Sigma\Delta P$ = Cumulative rainfall depth (mm) at time, t

Note : The ten heavy rain events have daily rainfall depths of about 100 mm to more than 100 mm with return periods varying from 5-year to 10-year.

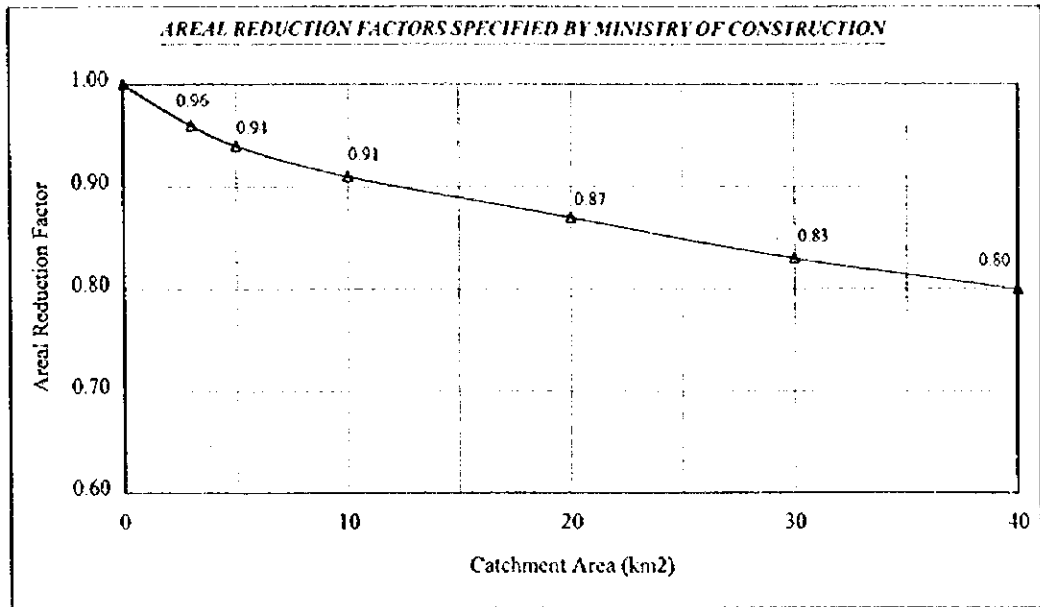
Data Source : Southern Region Hydro Meteorological Center, Ho Chi Minh City

Figure C.3.11 MASS CURVES OF TEN HEAVY RAINALL EVENTS (1983 - 1998)

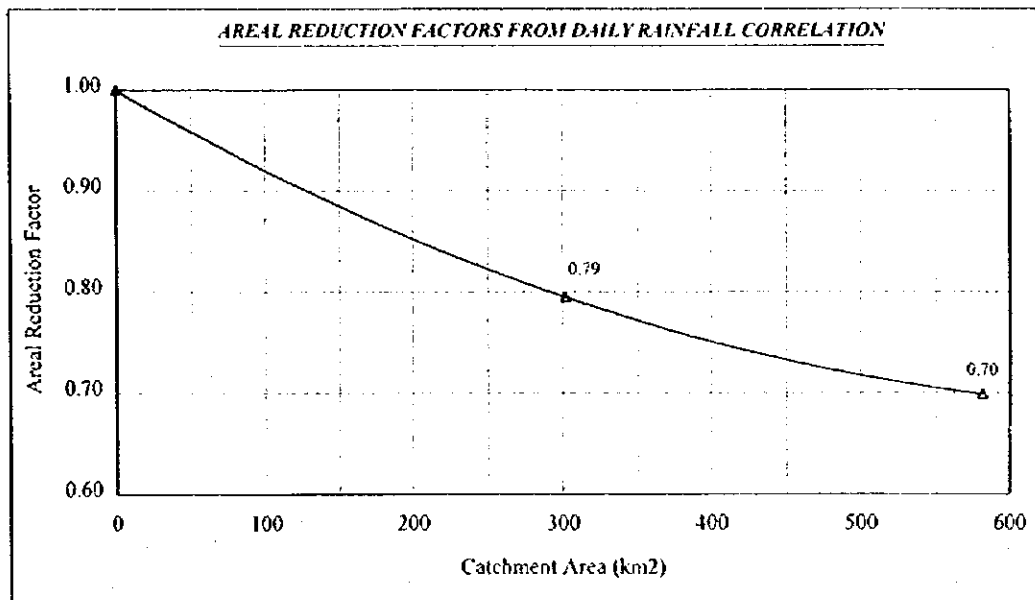


Note: The amount of rainfalls represent daily total probable maximum rainfalls as calculated from Gumbel's distribution method. The duration (6 hours) and pattern (frontal type) of the design rainfall hyetographs have been selected based on mass curve analysis on ten heavy rain events during the period of 1993 to 1998.

Figure C.3.12 PROPOSED DESIGN RAINFALL HYETOGRAPHS AT TAN SON NHAT



Note: Ministry of Construction (MOC) only specifies the values, not the smooth line.



Note: Investigated by the JICA Study Team using Thiessen Polygons of seven stations and daily rainfall correlations between Tan Son Nhat and other six stations based on 1994 & 1996 rainfall data.

Figure C.3.13 AREAL REDUCTION FACTORS SPECIFIED BY MOC AND CALCULATED FROM DAILY RAINFALL CORRELATIONS BY JICA

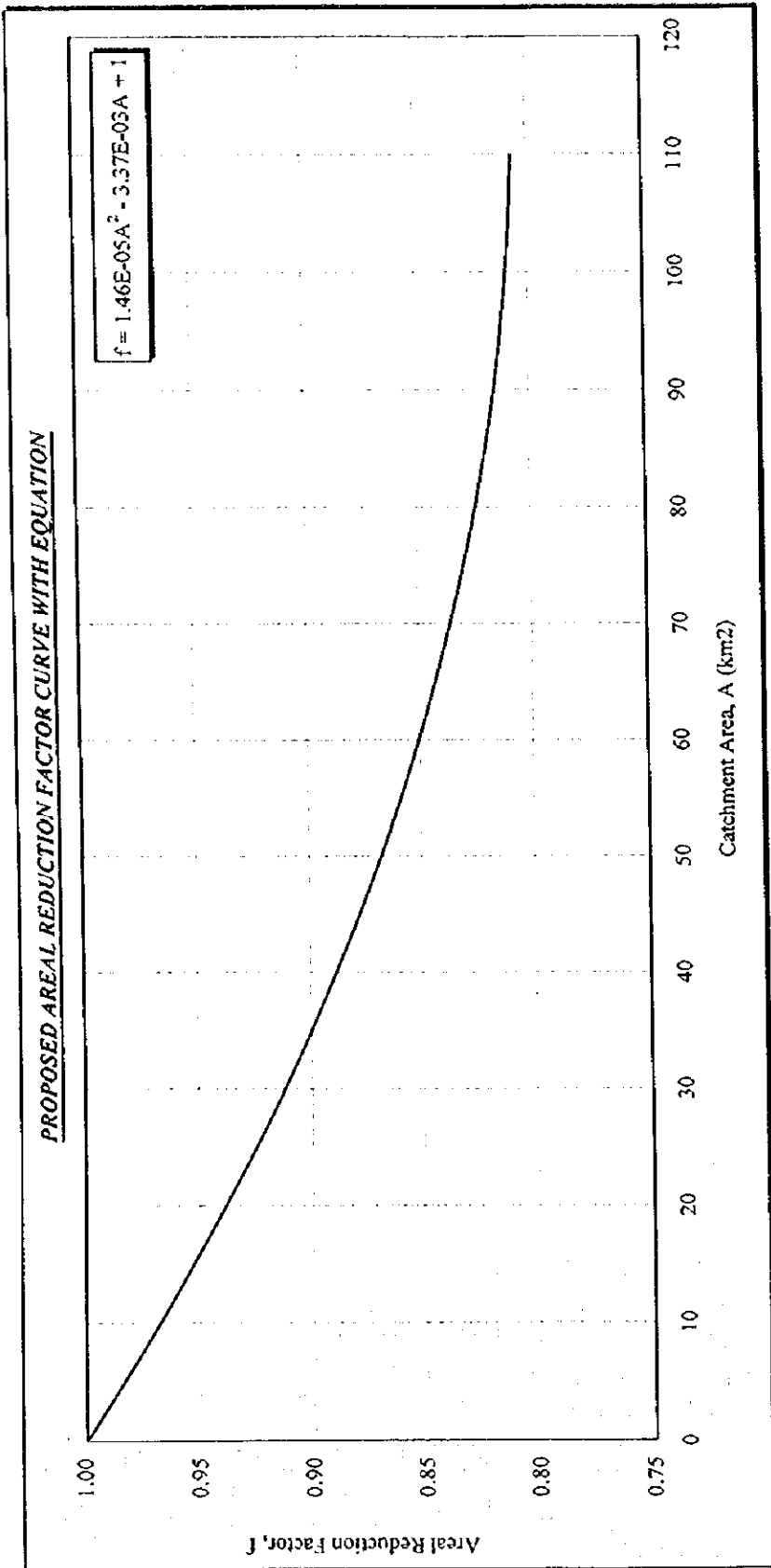
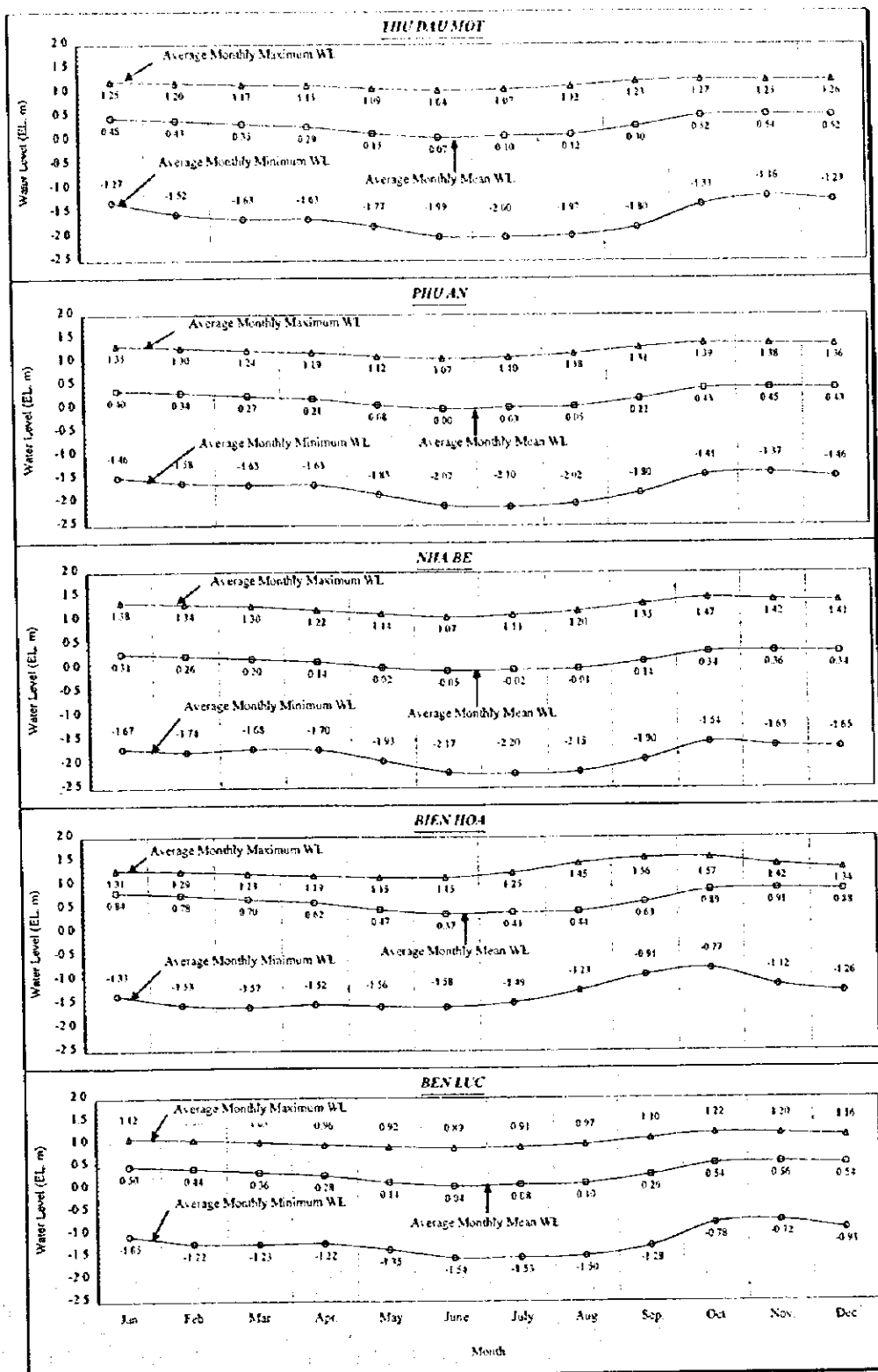
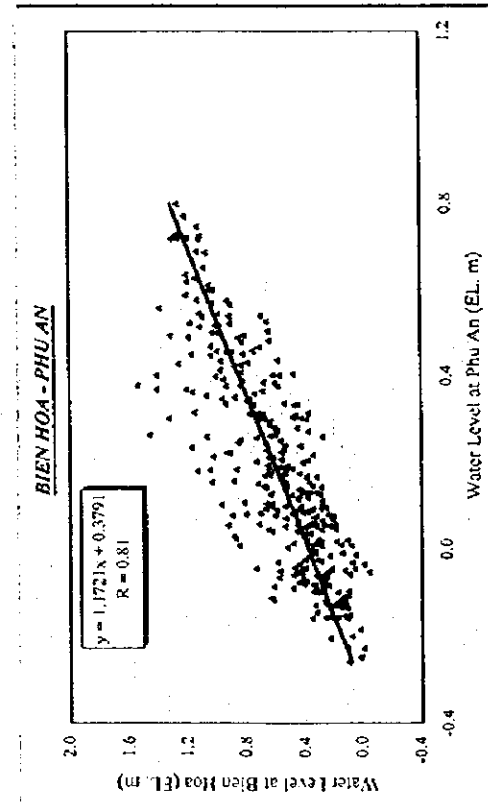
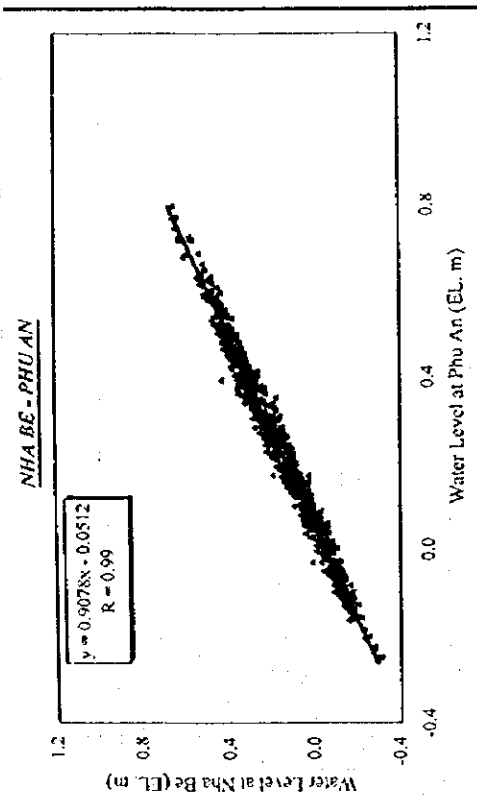
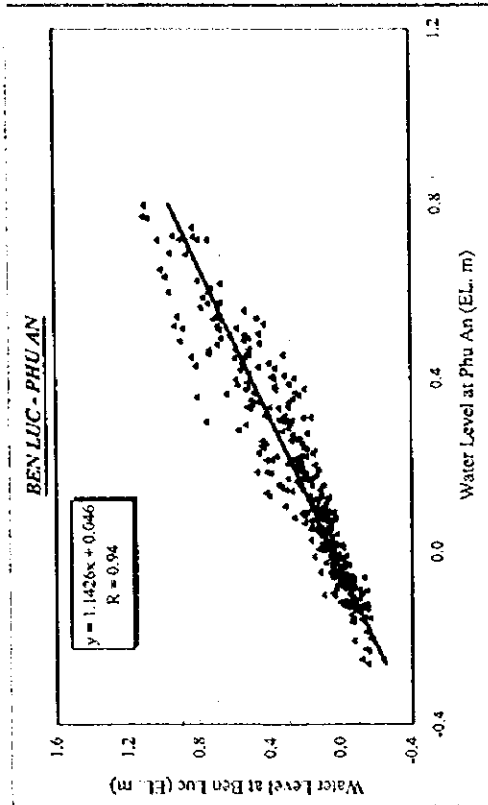
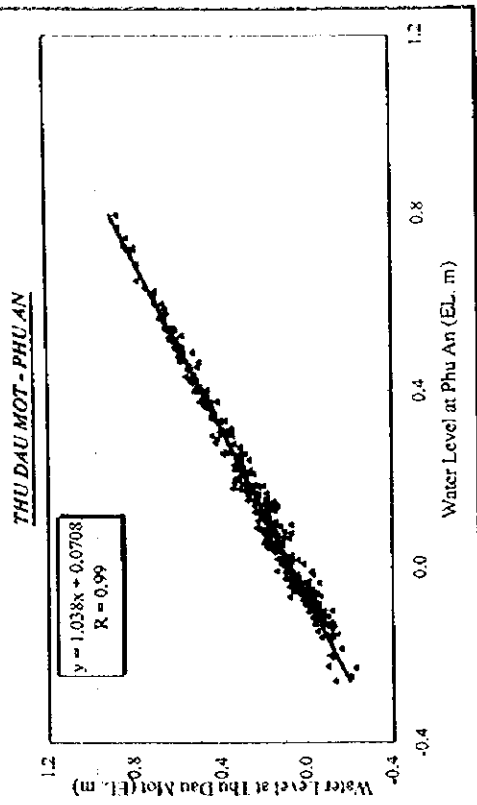


Figure C.3.14 PROPOSED AREAL REDUCTION FACTOR CURVE WITH EQUATION



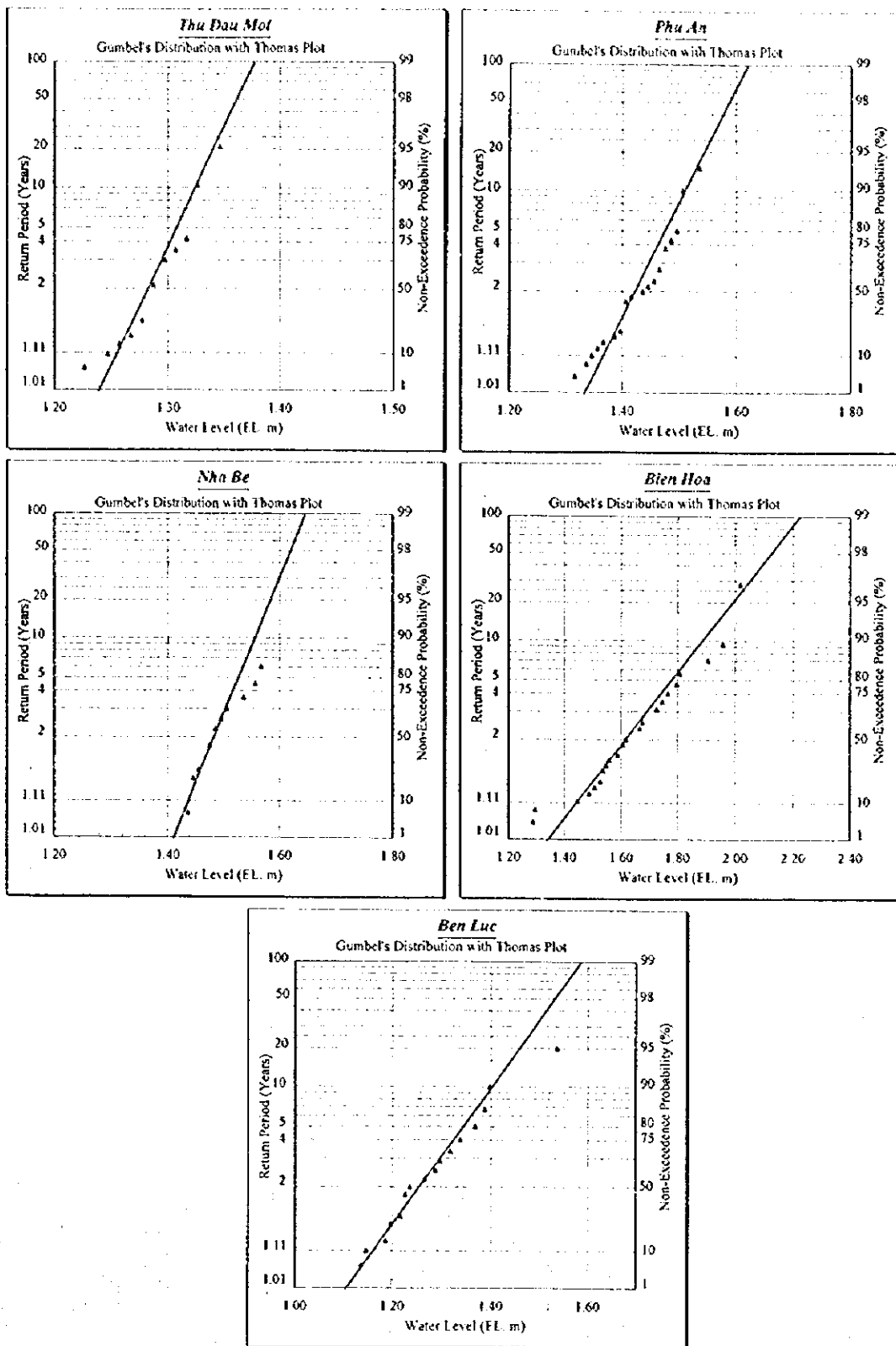
Monthly maximum and minimum water levels: Data ranges from 1960 to 1997, depending upon the station
 Monthly mean water level: Data for Phua An ranges from 1993 to 1997 (hourly data) and for other stations, estimated from correlations of mean water levels

Figure C.4.1 MONTHLY MAXIMUM, MINIMUM AND MEAN WATER LEVELS



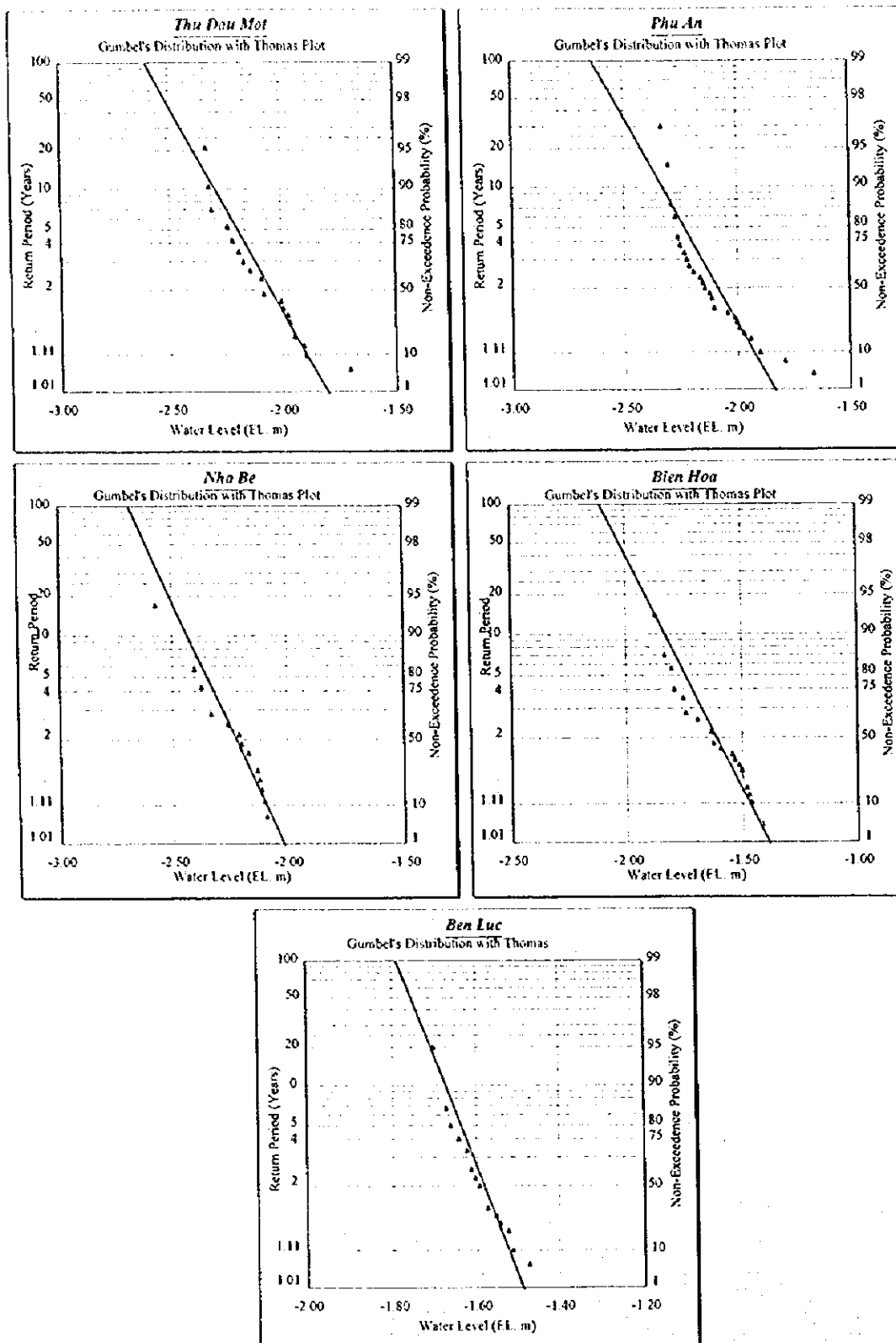
Data : Daily mean water levels for different months during the period 1990 to 1997.

Figure C.4.2 CORRELATIONS ON DAILY MEAN WATER LEVELS BETWEEN PHU AN AND OTHER STATIONS



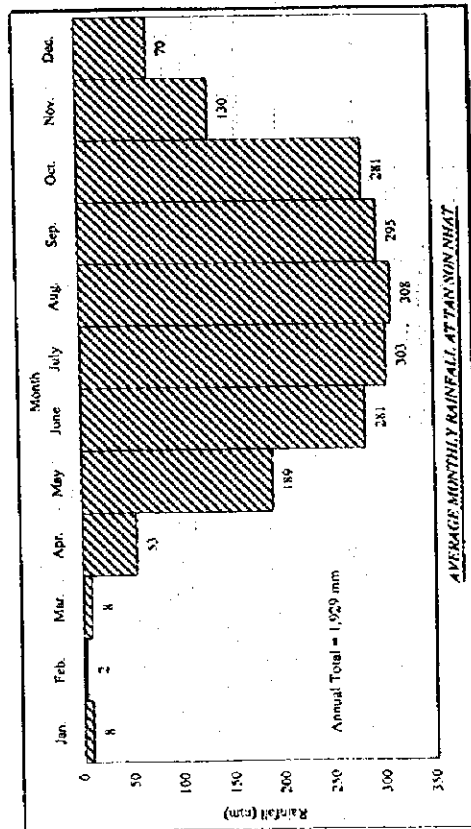
Data : Thu Dau Mot => 1966-1997; Phu An => 1960-1997; Nha Be => 1977-1997; Bien Hoa => 1960-1997 and Ben Luc => 1962-199

Figure C.4.3 PROBABILITY PLOTS OF MAXIMUM WATER LEVELS

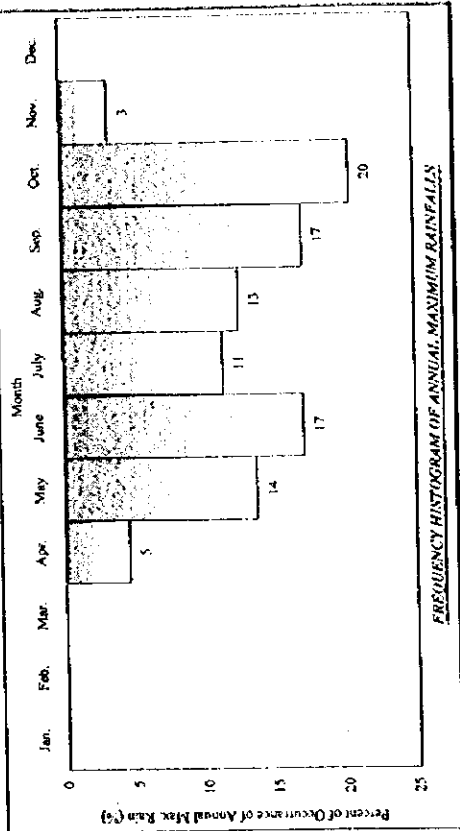


Data : Thu Dau Mot => 1966-1997, Phu An => 1960-1997, Nha Be => 1977-1997, Bien Hoa => 1960-1997 and Ben Luc => 1962-199

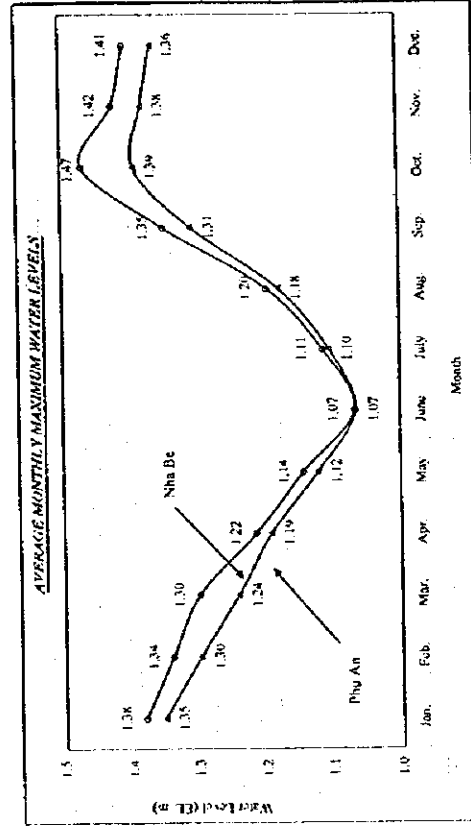
Figure C.4.4 PROBABILITY PLOTS OF MINIMUM WATER LEVELS



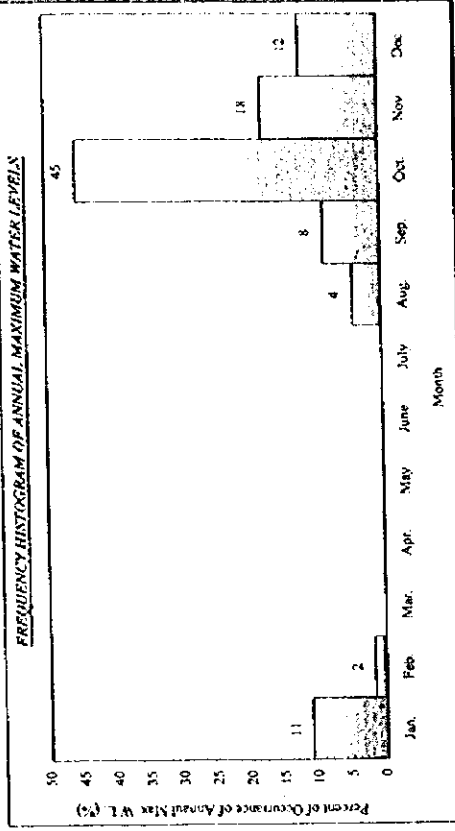
Data : From 1976 - 1997 at Tan Son Nhat.



Date : Frequency of occurrences (1952 - 1997) of annual maximum daily rainfalls at Tan Son Nhat, Hoc Mon, Le Minh Xuan, Binh Chanh, Nha Be, Ha Tien Cement Factory and Long Son.



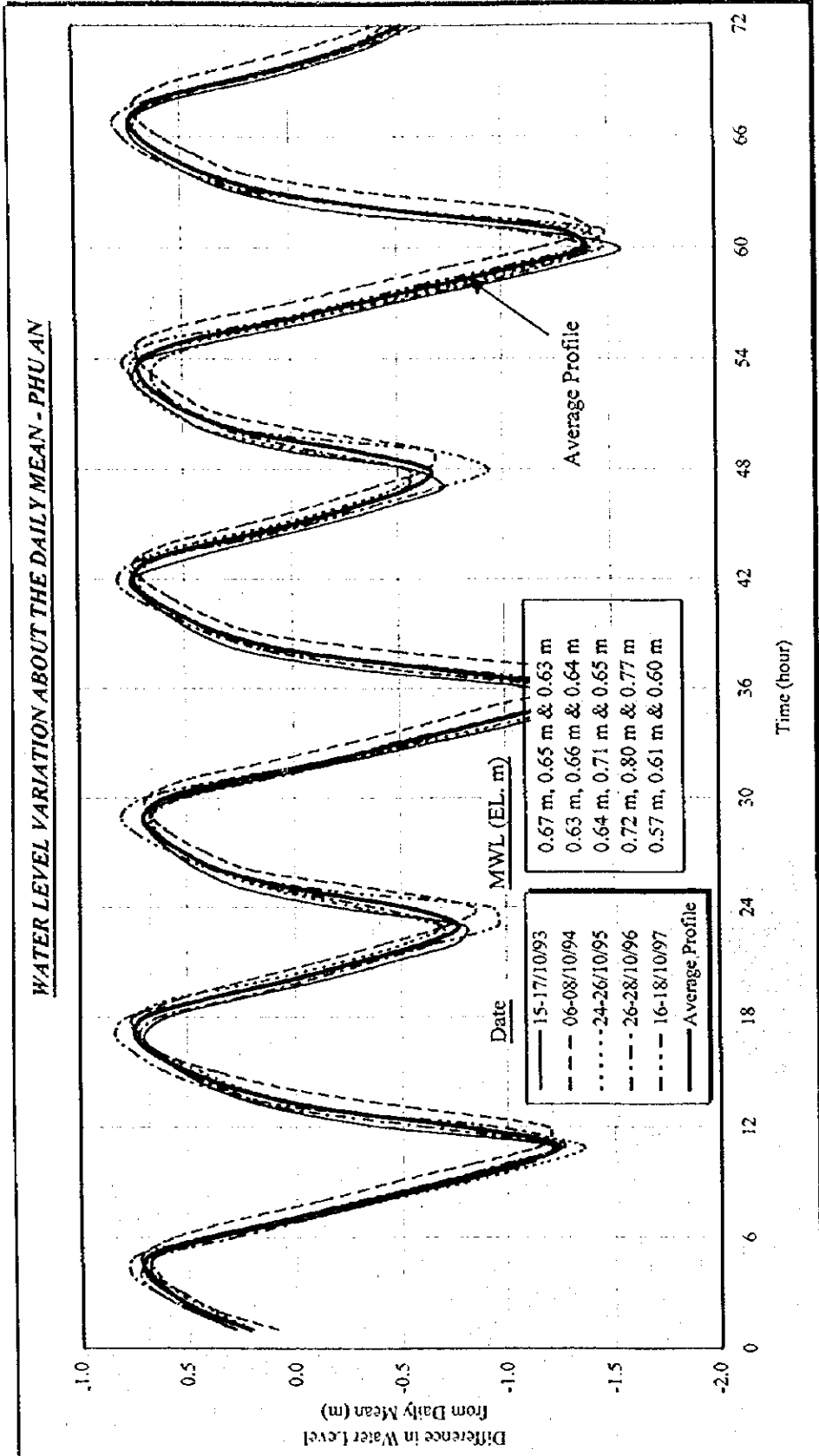
Data : From 1960 - 1997, depending upon station.



Data : Frequency of occurrences (1960 - 1997, depending upon station) of annual maximum water levels at Phu An, Nha Be, Bien Hoa and Ben Luc.

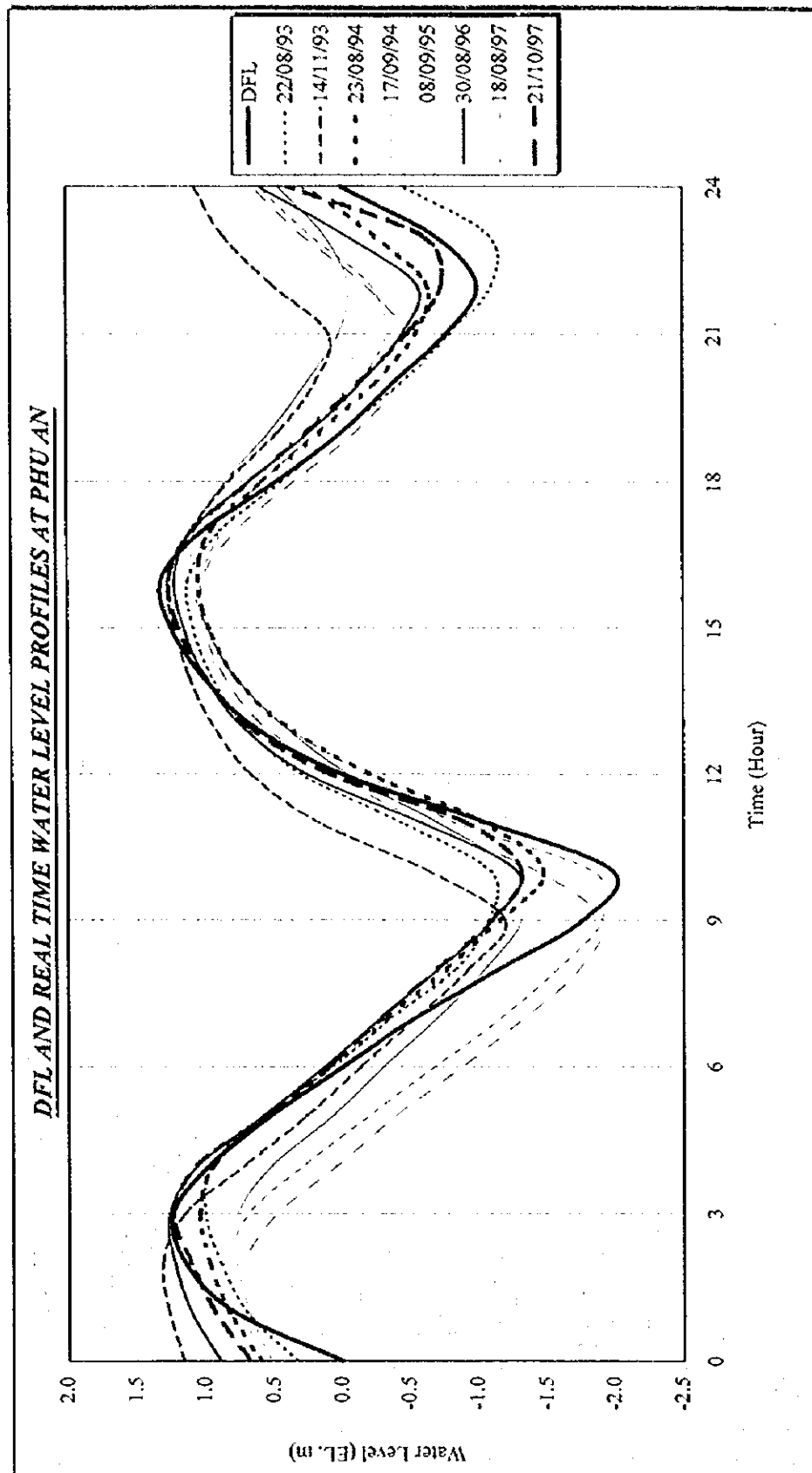
Figure C.4.5 MONTHLY RAINFALL AND MAXIMUM WATER LEVELS AND FREQUENCY HISTOGRAMS

WATER LEVEL VARIATION ABOUT THE DAILY MEAN - PHU AN



Data : The dates correspond to maximum water levels in October for the recent five years.

Figure C.4.6 VARIATION IN WATER LEVEL ABOUT THE DAILY MEAN AT PHU AN



Note: The real time water level profiles have been selected from the months of August to November during the period 1993 to 1997. The maximum water levels of the profiles are close to the maximum value of Design Flood Level.

Figure C-4.7 COMPARISON OF 24-HOUR DFL WITH REAL TIME WATER LEVEL PROFILES AT PHU AN

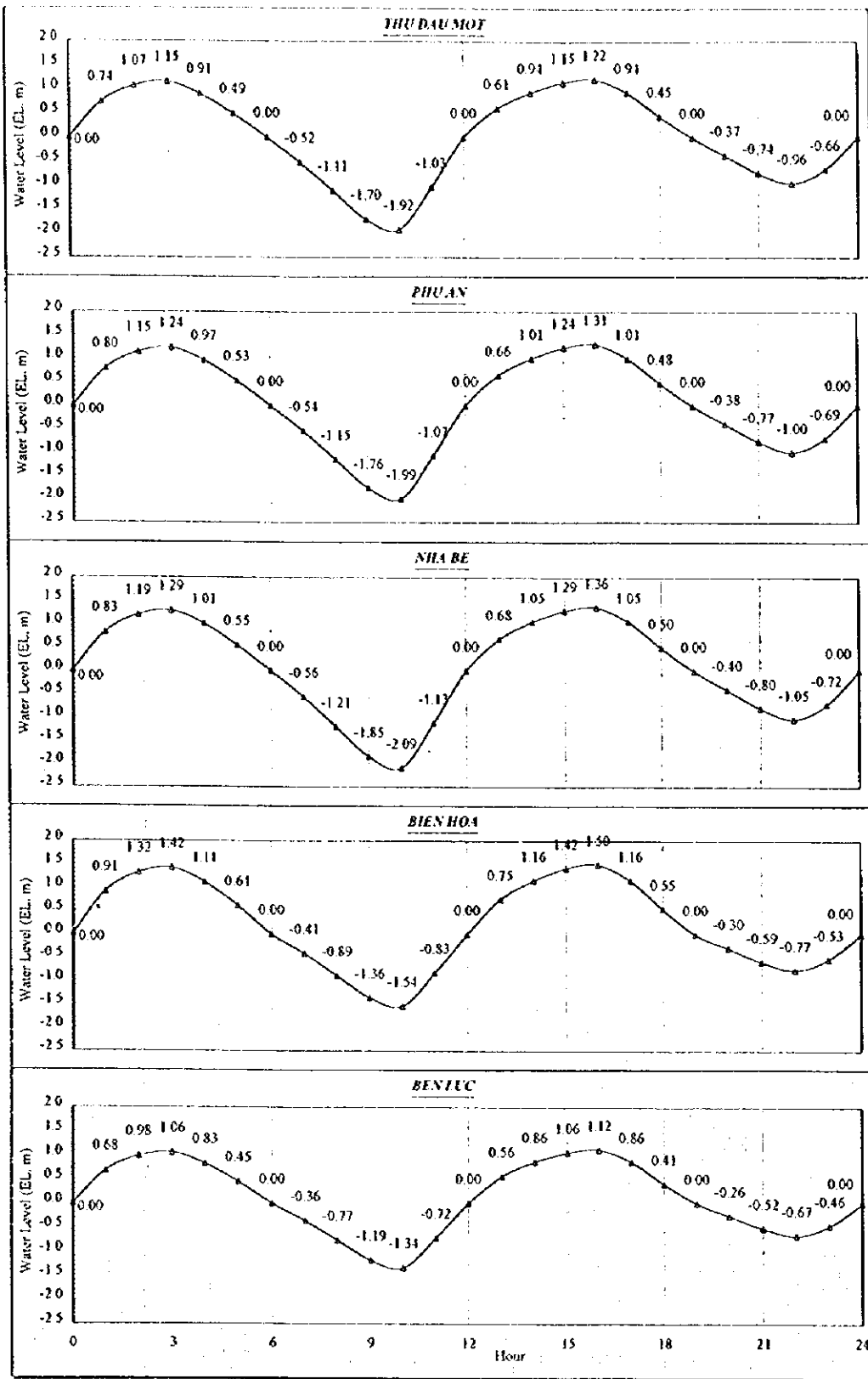


Figure C.4.8 24-HOUR DESIGN FLOOD LEVEL (DFL) BY STATION

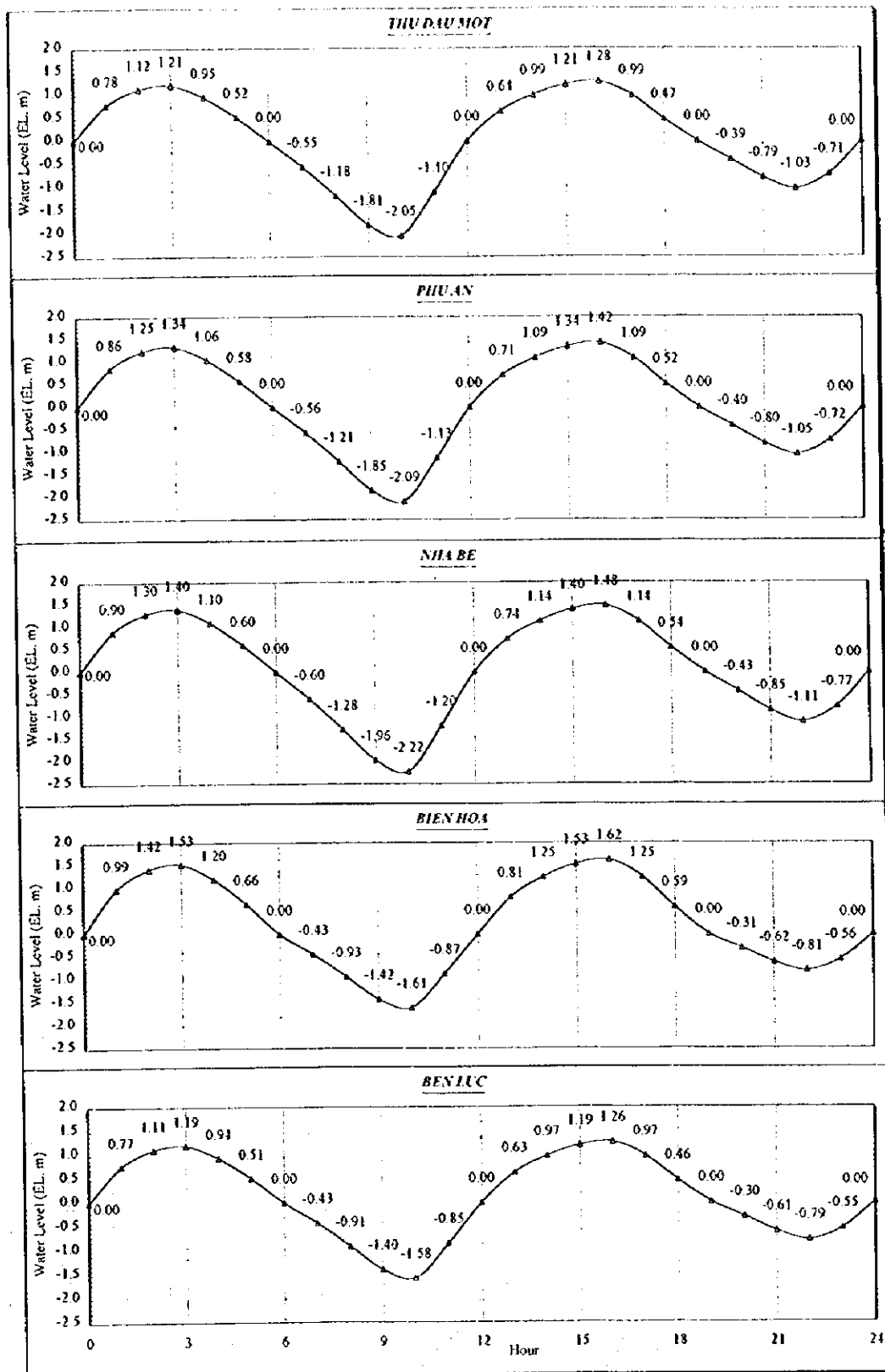


Figure C.49 2-YEAR WATER LEVEL PROFILE BY STATION

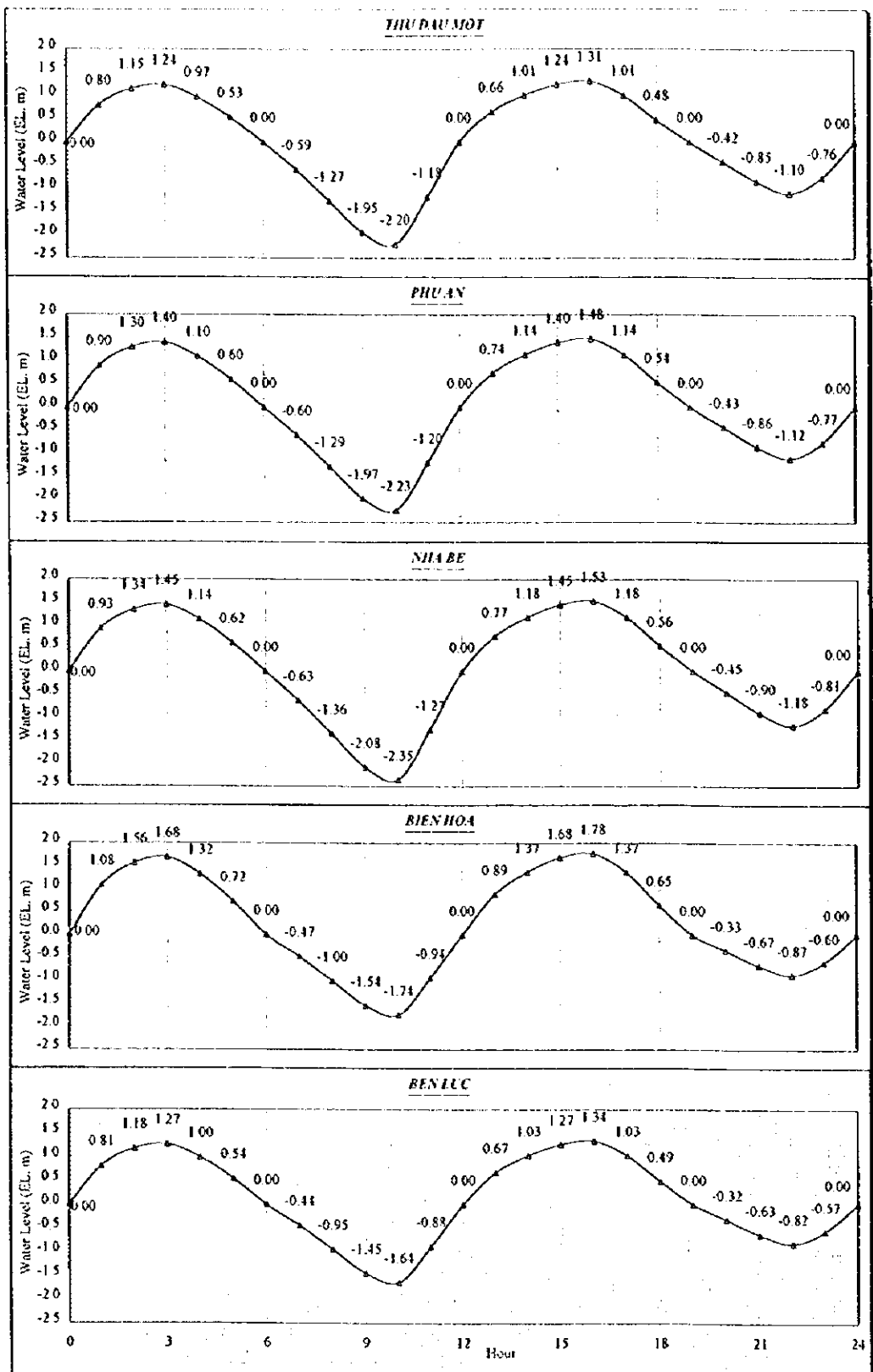


Figure C.4.10 5-YEAR WATER LEVEL PROFILE BY STATION

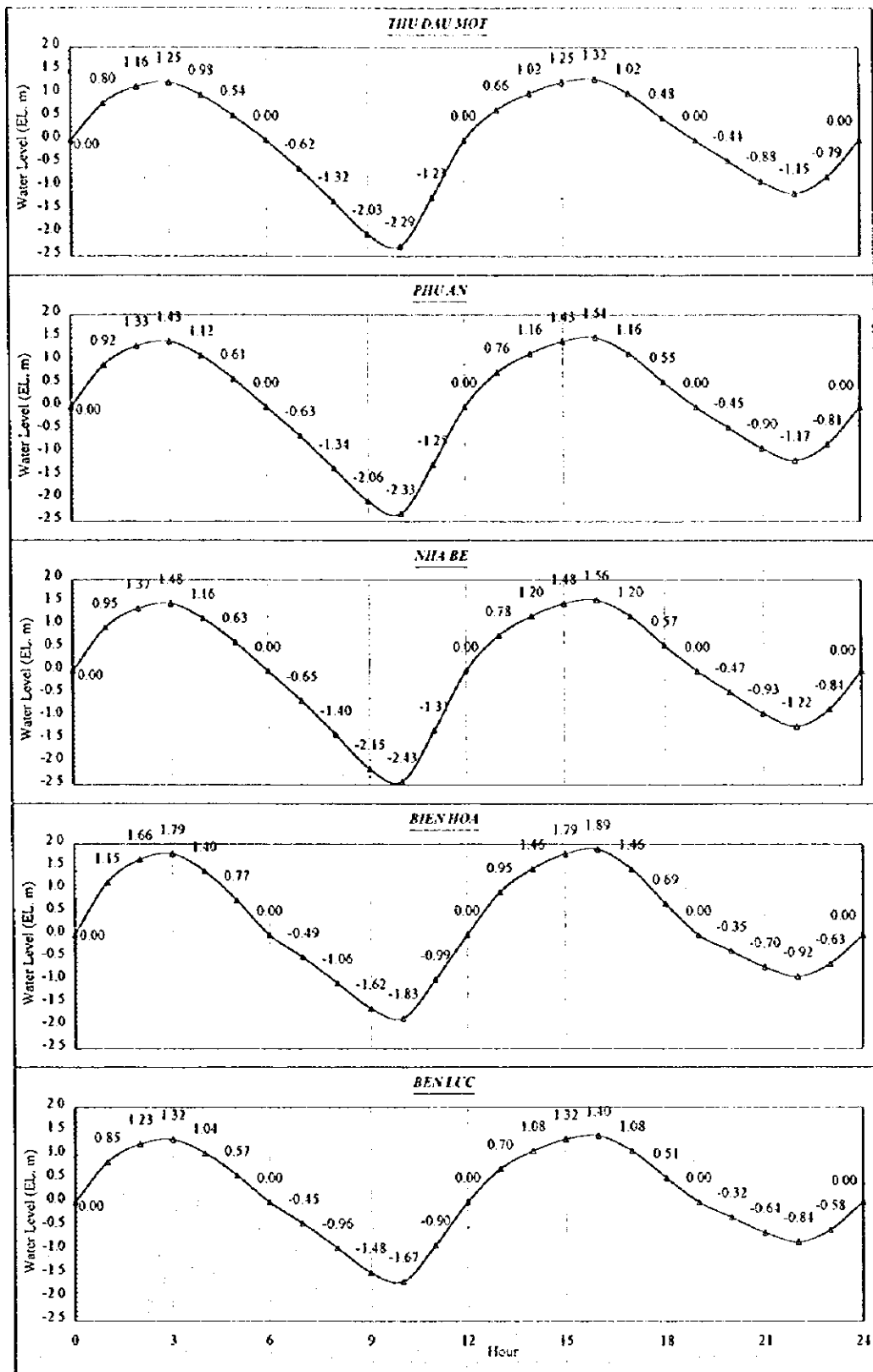


Figure C.4.11 10-YEAR WATER LEVEL PROFILE BY STATION

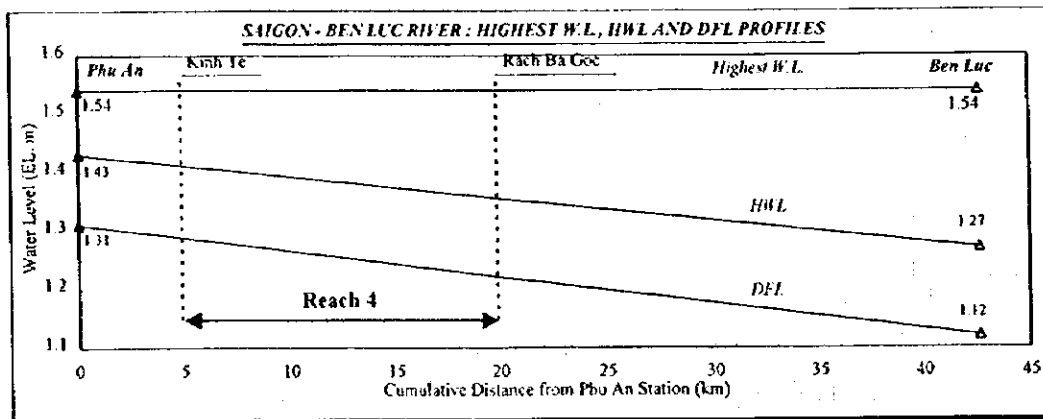
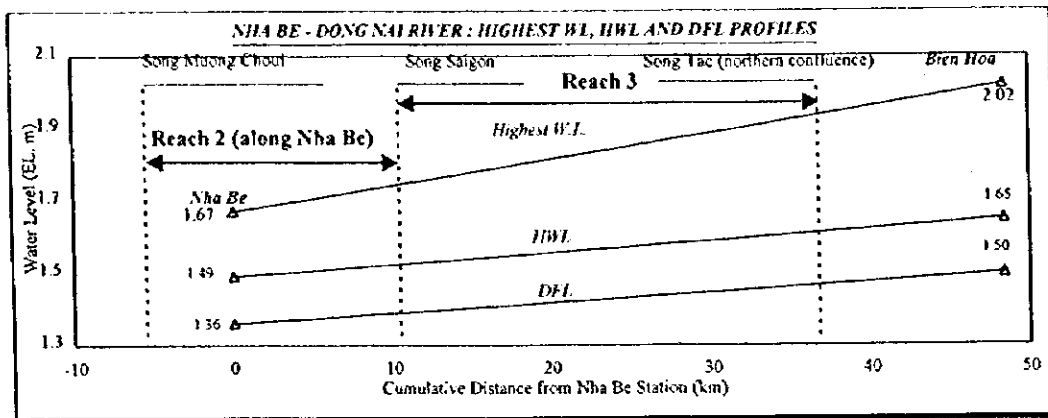
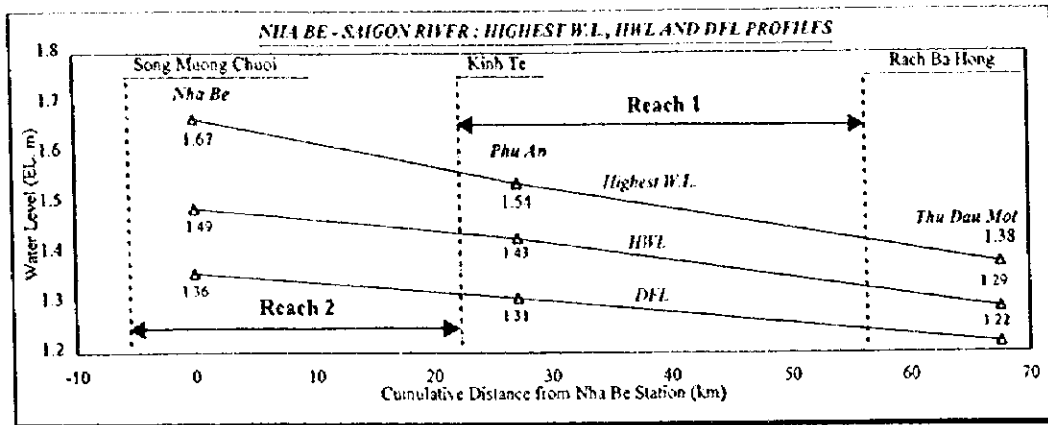
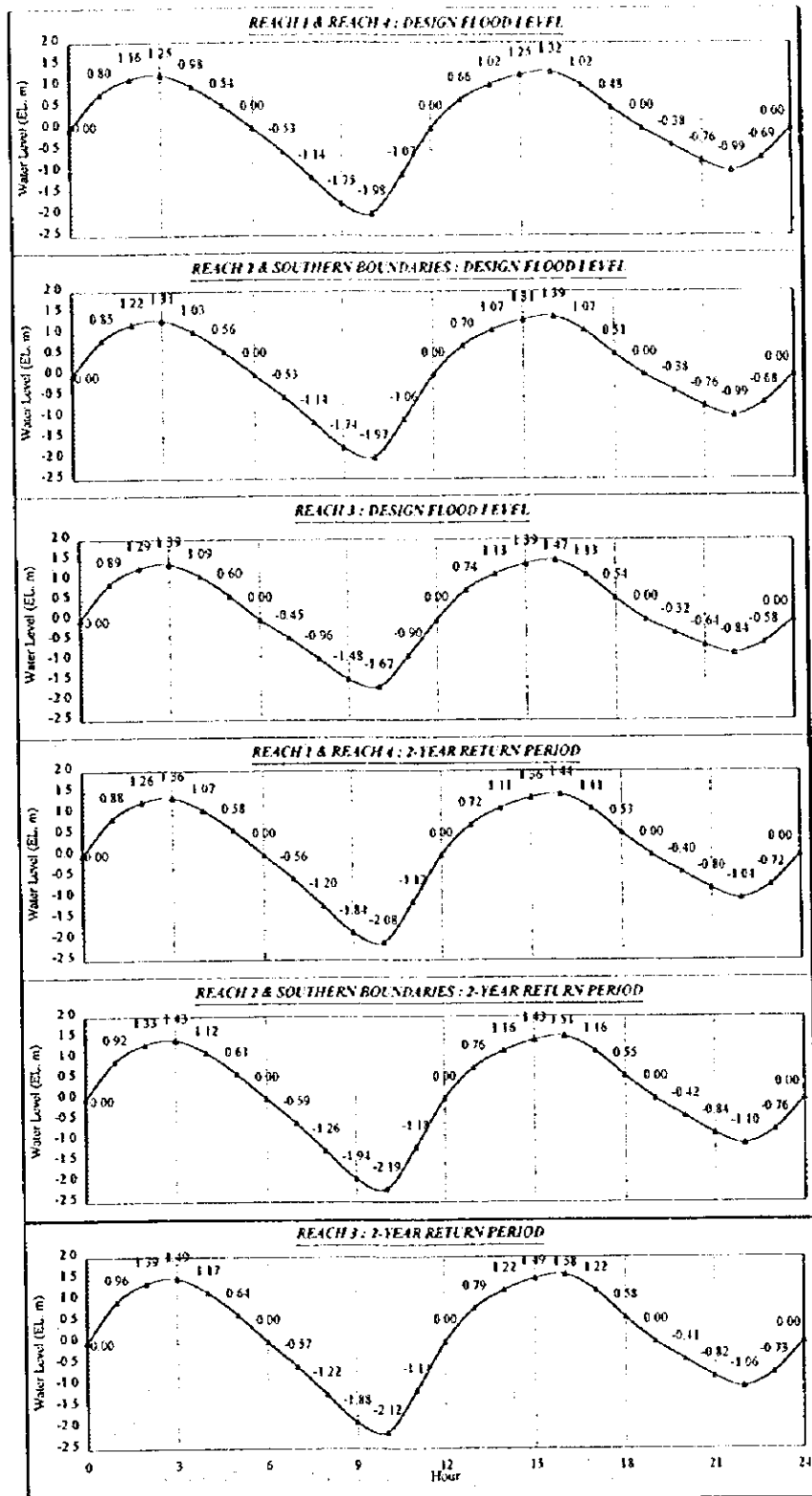
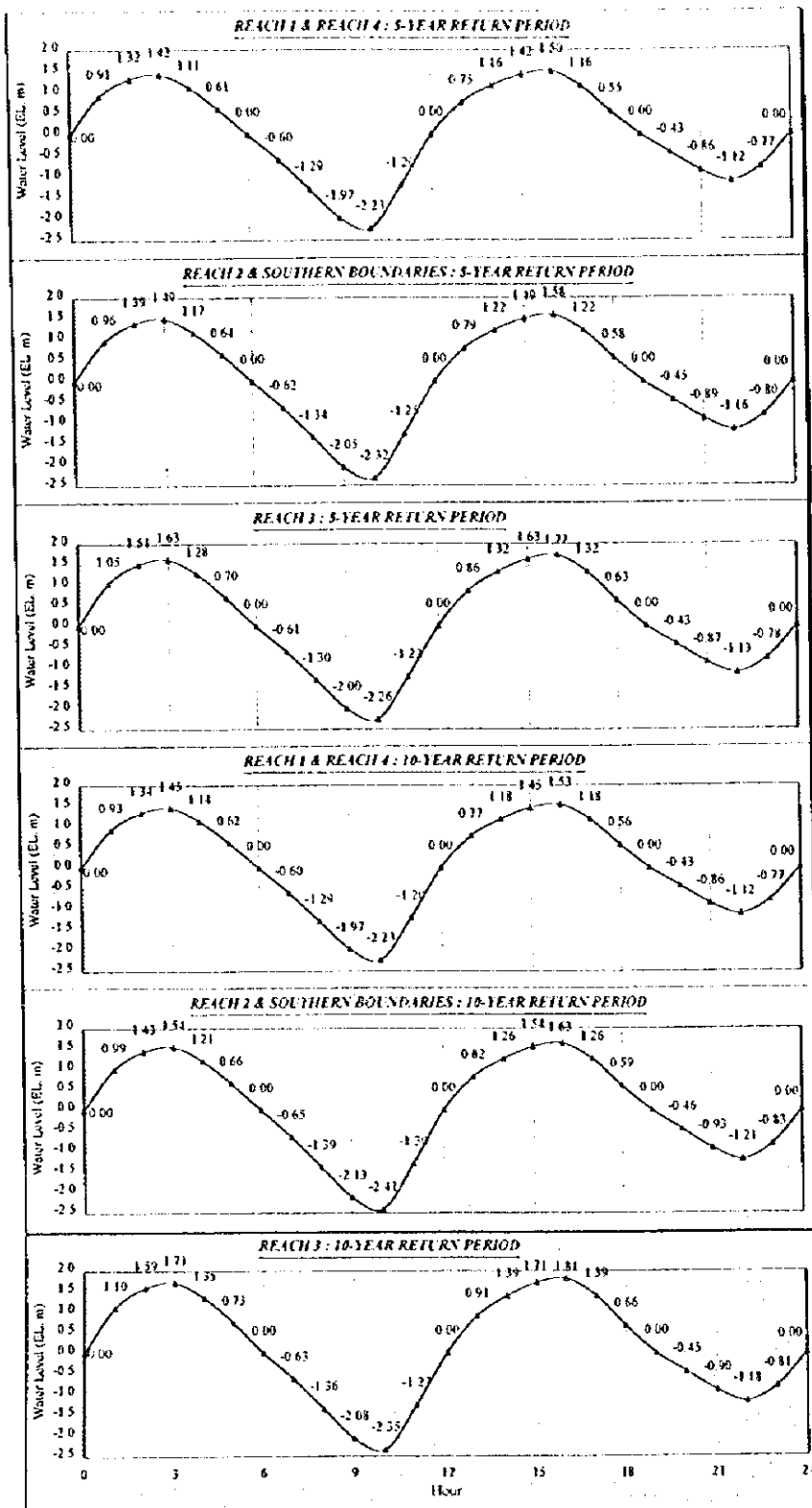


Figure C.4.12 LONGITUDINAL WATER LEVEL PROFILES



Reach 1 : From confluence point of Rach Ba Hong with Song Saigon to confluence point of Kinh Te with Song Saigon (33.97 km)
 Reach 2 : From confluence point of Kinh Te with Song Saigon to confluence point of Song Muong Chacoi with Song Nha Be (27.83 km)
 Reach 3 : From confluence point of Song Saigon with Song Nha Be to (northern) confluence point of Song Tac with Song Dong Nai (26.33 km)
 Reach 4 : From confluence point of Kinh Te with Song Saigon to confluence point of Rach Ba Goc with Song Bon Tue (15.65 km)
 Southern Boundaries : The southern boundaries of Rach Can Giuoc and Rach Ba Lao flowing towards the south

Figure C.4.13 24-HOUR WATER LEVEL PROFILE BY REACH (1/2)



Reach 1 - From confluence point of Rach Ba Hong with Song Saigon to confluence point of Kinh Te with Song Saigon (33.97 km)
 Reach 2 - From confluence point of Kinh Te with Song Saigon to confluence point of Song Maong Chud with Song Nha Be (27.83 km)
 Reach 3 - From confluence point of Song Saigon with Song Nha Be to (northern) confluence point of Song Tac with Song Dong Nai (26.33 km)
 Reach 4 - From confluence point of Kinh Te with Song Saigon to confluence point of Rach Ba Goc with Song Bun Luc (15.65 km)
 Southern Boundaries - The southern boundaries of Rach Can Giuoc and Rach Ba Lao flowing towards the south

Figure C.4.13 24-HOUR WATER LEVEL PROFILE BY REACH (2/2)