DATA BOOK CHAPTER 9 WATER SUPPLY FACILITY

Annex IX -

Figure 1 Graphed Result of Flow Rater measurement at the Pumping Station (26 - 27 June, 1997)

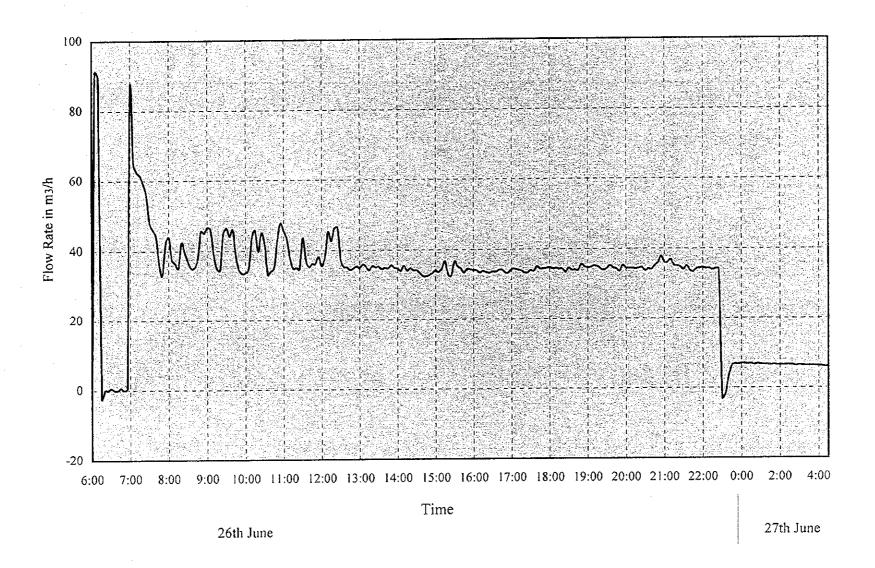
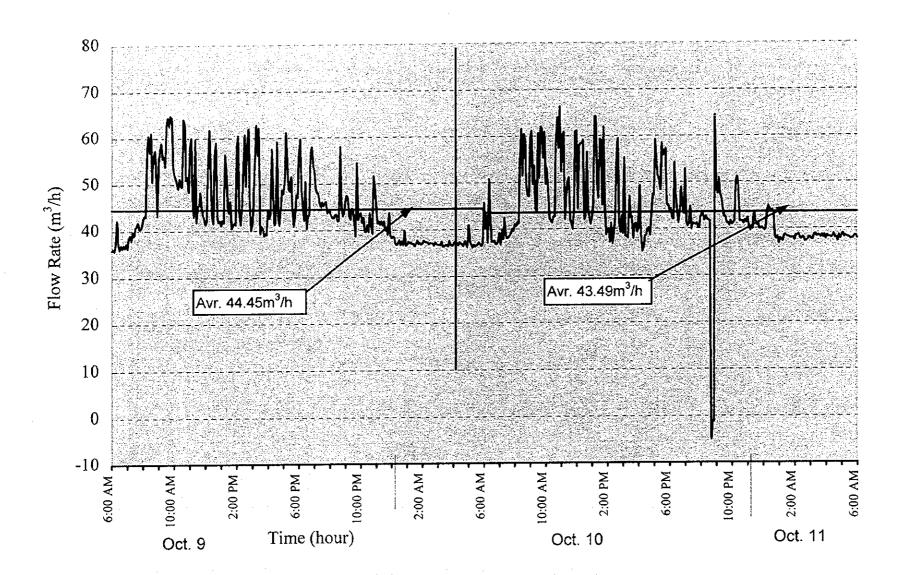


Figure 2 Graphed Reuslt of the Flow Rate Measurement at the Pumping Station (9 - 11 October, 1998)



DATA BOOK II MASTER PLAN

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Caluculation of Pipeline Network (Case 1)

[Caluculation Condition]

1 . Formula for Caluculation

Hazen-Williams Formula

 $h = I \times L$

 $I = 10.666 \times C^{-1.85} \times D^{-4.87} \times Q^{1.85}$

(Herein)

h: Friction loss (m)

L: Pipe length

I : hydraulic gradient (%)

C: Coefficient of flow rate condition (-) (C=90: Supposed the pipe condition in 25 years after installation)

D: Pipe diameter (mm)

Q: Flow rate (L/sec)

2 . Allowable range of In-put data and Out-put data

Minimum effective head	(m)	5.00
Maximum static head	(m)	75.00
Nominal diameter	(mm)	75~1000
Flow velocity	(m/sec)	0.0~5.0
Total Number of Nodes		38
Total Number of Pipelines		47

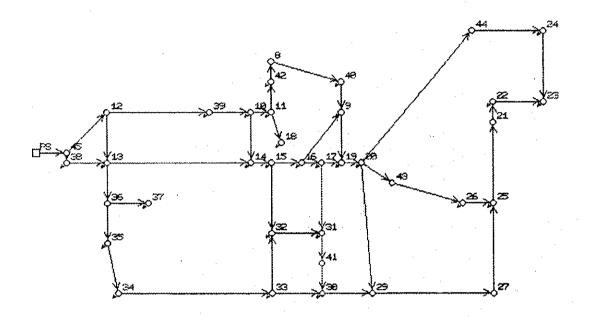
3. Result of Caluculation

The results of caluculation are shown in next 4 pages for Case 1.

(Herein)

Point	: Name of node	(-)
H	: Hydrodynamic head	(m)
G	: Ground Level	(m)
Е	: Effective head	(m)
S	: Static head	(m)
T	: Total distribution amount	(L/sec)
Line	: No. of Pipeline	(-)
В	: Start point	(-)
F	: End point	(-)
N	: Nominal diameter of pipe	(mm)
V	: Flow velocity	(m/sec)

A. Caluculation of pipe networ (case 1-1: For Low G-L Area)

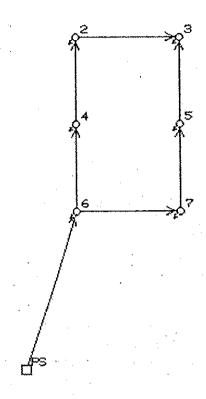


(2) Point data (PS: appointed the pumping station)

Point	Н	G	E	S	T
	(m)	(m)	(m)	(m)	(L/SEC)
38	2195.46	2165.00	30,46	33.50	1.150
13	2194.47	2165.00	29.47	33.50	0.680
12	2194.67	2170.00	24.67	28.50	1,670
36	2193.50	2160.00	33.50	38.50	0.000
37	2193.44	2160.00	33.44	38.50	2.020
39	2194.25	2170,00	24.25	28.50	1.220
10	2192.59	2165.00	27.59	33,50	0.980
42	2192.51	2165.00	27.51	33.50	0.560
- 18	2192.54	2165.00	27.54	33.50	0.160
- 14	2192.58	2165.00	27.58	33.50	1.320
15	2192.33	2165.00	27.33	33.50	2.060
32	2192.08	2145.00	47.08	53.50	2.560
11	2192.54	2165.00	27.54	33.50	0.490
16	2192.10	2165.00	27.10	33.50	4.890
17	2192.06	2165.00	27.06	33.50	1.930
9	2192.10	2165.00	27.10	33.50	0.800
40	2192.12	2170.00	22.12	28.50	0.120
31	2192.05	2145.00	47.05	53.50	2.480
19	2192.06	2165.00	27.06	33.50	0.460
20	2191.99	2165,00	26.99	33.50	0.420
43	2191.98	2165.00	26.98	33.50	0.000
33	2192.09	2140.00	52.09	58.50	0.600
41	2192.03	2140.00	52.03	58.50	0.000
30	2192.01	2140.00	52.01	58.50	2.100
29	2191.99	2150,00	46.99	48.50	0.000
34	2192.27	2140.00	52.27	58.50	2.020
35	2192.83	2150.00	42.83	48.50	2.020
27	2191.85	2145.00	46.85	53.50	0.000
26	2191,82	2160.00	31.82	38.50	0.060
25	2191.76	2160.00	31.76	38.50	1.810
21	2191.53	2170.00	21.53	28.50	0.000
22	2191.48	2170.00	21.48	28.50	1.810
23	2191.45	2170.00	21.45	28.50	1.810
24	2191.46	2175.00	16.46	23.50	1.810
45	2195.61	2165.00	30.61	33.50	0.000
PS	2198.50	2165.00	33.50	33.50	
8	2192.45	2170.00	22.45	28.50	
44	2191.52	2175.00	16.52	23.50	0.000

(3) Pipeline	data				•					
Line	В	F	N	D	С	L	Q	v	I h	
			(mm)	(mm)		(m)	(L/SEC)	(m/s)	(1/1000) (m)	
K1	PS	45	200	200	90	170.00	40.010	1.274	16,999 2.89	
K2	12	39	200	200	90	390.00	9.085	0.289	1.077 0.42	
K3	13	36	150	150	90	220.00	9.064	0.513	4.409 0.97	
K4	36	37	150	150	90	200.00	2.020	0.114	0.300 0.05	
K5	12	13	200	200	90	200.00	8.645	0.275	1.000 0.20	
K6	13	14	200	200	90	470.00	18.362	0.584	4.021 1.89	
K7	14	15	200	200	90	60.00	18.645	0.593	4.167 0.24	
K8	15	32	200	200	90	560.00	5.569	0.177	0.446 0.24	
K9	10	14	200	200	90	230.00	1.603	0.051	0.044 0.01	
K10	39	10	125	125	90	200.00	7.865	0.641	8.300 1.65	
K11	10	11	200	200	90	130.00	5.281	0.168	0.385 0.05	
K12	11	42	200	200	90	90.00	4.631	0.147	0.334 0.02	
K13	11	18	200	200	90	190.00	0.160	0.005	0.000 0.00	
K14	40	9	200	200	90	90.00	3.951	0.126	0.222 0.02	
K15	9	19	200	200	90	210.00	3.529	0.120	0.191 0.04	
K16	15	16	200	200	90	150.00	11.016	0.351	1.533 0.23	
K17	16	9	200	200	90	340.00	0.378	0.012	0.000 0.00	
K18	16	17	200	200	90	70.00	5.748	0.183	0.572 0.03	
K19	17	19	200	200	90	70.00	2.708	0.183	0.000 0.00	
K20	17	31	200	200	90	530.00	1.110	0.035	0.000 0.00	
K21	32	31	200	200	90	160.00	3.588	0.033	0.019 0.01	
K22	19	20	200	200	90	140.00	5.777	0.114	0.188 0.03	
K23	20	43	200	200	90	120.00	2.799	0.089	0.300 0.00	
K24	36	35	150	150	90	240.00	7.044	0.089	2.791 0.66	
K25	35	34	150	150	90	380.00	5.024	0.284	1.474 0.56	
K26	34	33	150	150	90	310.00	3.004	0.284	0.580 0.17	
K27	33	32	150	150	90	210.00	0.579	0.033	0.048 0.00	
K28	33	30	150	150	90	330.00	1.825	0.103	0.243 0.07	
K29	41	30	150	150	90	60.00	2.218	0.103	0.243 0.07	
K30	31	41	200	200	90	230.00	2.218	0.123	0.087 0.02	
K31	30	29	150	150	90	100.00	1.943	0.110	0.200 0.02	
K32	45	12	200	200	90	210.00	19.400	0.618	4.477 0.93	
K33	20	29	150	150	90	770.00	0.124	0.007		
K34	20	44	150	150	90	1200.00	2.435	0.138	0.392 0.46	
K35	27	25	150	150	90	310.00	2.067	0.117	0.291 0.08	
K36	43	26	150	150	90	310.00	2.799	0.158	0.516 0.15	
K37	29	27	150	150	90	500.00	2.067	0.117	0.280 0.14	
K38	26	25	150	150	90	130.00	2.739	0.155	0.462 0.06	
K39	44	24	150	150	90	150.00	2.435	0.138	0.400 0.05	
K40	25	21	150	150	90	400.00	2.995	0.169	0.575 0.22	
K41	24	23	150	150	90	380.00	0.625	0.035	0.026 0.01	
K42	22	23	150	150	90	230.00	1.185	0.067	0.020 0.01	
K43	21	22	150	150	90	90.00	2.995	0.169	0.131 0.02	
K44	45	38	200	200	90	30.00	20.611	0.109	5.005 0.15	
K45	38	13	200	200	90	220.00	19.461	0.619	4.500 0.98	
K46	42	8	150	150	90	60.00	4.071	0.019	1.001 0.06	
K47	8	40	150	150	90	330.00	4.071	0.230	0.999 0.33	
							1.071	0.230	U.J.J. U.J.J	-

B. Caluculation of pipe network (case 1-2: For High G·L Area)



(2) Point data	(PS: appointed	the pumpir	ng station)		
Poir	nt H	G	E	. S	T
	(m)	(m)	(m)	(m)	(L/SEC)
PS	2218.50	2165.00	53.50	53.50	-14.670
6	2210.95	2180.00	30.95	38.50	2.350
7	2210.16	2180.00	30.16	38.50	2.540
4	2210.11	2190.00	20.11	28.50	2.350
5	2209.83	2190.00	19.83	28.50	2.540
2	2209.85	2200.00	9.85	18.50	2.350
3	2209.80	2200.00	9.80	18 50	2.540

(3) Pipelir	ne data				•					
Line	В	F	N	D	C	L	Q .	V	I	h
			(mm)	(mm)		(m)	(L/SEC)	(m/s)	(1/1000)	(m)
K1	PS	6	150	150	90	700.00	14.670	0.830	10.786	7.552
K2	6	7	150	150	90	360.00	6.203	0.351	2.195	0.790
K3	7	5	150	150	90	400.00	3.663	0.207	0.825	0.331
K4	5	3	150	150	90	290.00	1.123	0.064	0.104	0.027
. K5	2	3	150	150	90	370.00	1.417	0.080	0.135	0.053
. K6	4	2	150	150	90	300.00	3.767	0.213	0.867	0.262
K7	6	4	150	150	90	390.00	6.117	0.346	2.153	0.834

Caluculation of Pipeline Network (Case 2)

[Caluculation Condition]

1 . Formula for Caluculation

Hazen-Williams Formula
$$\begin{split} &h=I\times L\\ &I=10.666\times C^{-1.85}\times D^{-4.87}\times Q^{1.85} \end{split}$$

(Herein)

h: Friction loss (m)

L: Pipe length

I: hydraulic gradient (%)

C: Coefficient of flow rate condition (-) (C=90: Supposed the pipe condition in 25 years after installation)

D : Pipe diameter (mm)

Q: Flow rate (L/sec)

2. Allowable range of In-put data and Out-put data

Minimum effective head	(m)	5.00
Maximum static head	(m)	75.00
Nominal diameter	(mm)	75~1000
Flow velocity	(m/sec)	$0.0 \sim 5.0$
Total Number of Nodes		38
Total Number of Pipelines		47

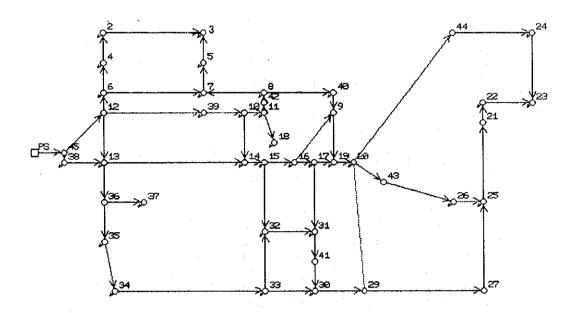
3. Result of Caluculation

The results of caluculation are shown in next 3 pages for Case 2.

(Herein)

: Name of node	(-)
: Hydrodynamic head	(m)
: Ground Level	(m)
: Hydrodynamic effective head	(m)
: Static head	(m)
: Total distribution amount	(L/sec)
: No.of Pipeline	(-)
: Start point	(-)
:End point	(-)
: Nominal diameter of pipe	(mm)
:Flow velocity	(m/sec)
	: Hydrodynamic head : Ground Level : Hydrodynamic effective head : Static head : Total distribution amount : No.of Pipeline : Start point : End point : Nominal diameter of pipe

A. Caluculation of pipe netwo (case 2)



(2) Point data (PS:appointed the pumping station)

Point	Н	G	E	S	T
	(m)	(m)	(m)	(m)	(L/SEC)
38	2213.10	2165.00	48.10	53.50	1.150
13	2211.46	2165.00	46.46	53.50	0.680
12	2211.52	2170.00	41.52	48.50	1.670
36	2210.39	2160.00	50.39	58.50	0.000
37	2210.34	2160.00	50.34	58.50	2.020
39	2211.06	2170.00	41.06	48.50	1.220
10	2209.24	2165.00	44.24	53.50	0.980
7	2208.76	2175.00	33.76	43.50	2.540
18	2209.15	2165.00	44.15	53.50	0.160
14	2209.24	2165.00	44.24	53.50	1.320
15	2208.98	2165.00	46.98	53.50	2.060
32	2208.73	2145.00	63.73	73.50	2.560
11	2209.15	2165.00	44.15	53.50	0.490
16	2208.73	2165.00	43.73	53.50	4.890
17	2208.70	2165.00	43.70	53.50	1.930
9	2208.73	2165.00	43.73	53.50	0.800
6	2209.16	2180.00	29.16	38.50	2.350
31	2208.70	2145.00	63.70	73.50	2.480
19	2208.69	2165.00	43.69	53.50	0.460
20	2208.63	2165.00	43.63	53.50	0.420
43	2208.61	2165.00	43.61	53.50	0.000
33	2208.75	2140.00	68.75	78.50	0.600
41	2208.68	2140.00	68.68	78.50	0.000
30	2208.66	2140.00	68.66	78.50	2.100
29	2208.63	2145.00	63.63	73.50	0.000
34	2208.98	2140.00	68.98	78.50	2.020
35	2209.64	2150.00	59.64	68.50	2.020
27	2208.49	2145.00	63.49	73.50	0.000
44	2208.16	2175.00	33.16	43.50	0.000
26	2208.46	2160.00	48.46	58.50	0.060
25	2208.40	2160.00	48.40	58.50	1.810
21	2208.17	2170.00	38.17	48.50	0.000
22	2208.12	2170.00	38.12	48.50	1.810
23	2208.09	2170.00	38.09	48.50	1.810
24	2208.11	2175.00	33.11	43.50	1.810
45	2213.35	2165.00	48.35	53.50	0.000
PS ·	2218.50	2165.00	53.50	53.50	-54.680
40	2208.74	2170.00	38.74	48.50	0.120
42	2209.1	2170.00	39.10	48.50	0.000
8	2208.96	2170.00	38.96	48.50	
4	2208.47		18.47	28.50	2.350
2	2208.28		8.28	18.50	
3	2208.26		8.26	18.50	2.540
5	2208.32		18.32	28.50	2.540

(3) Pipeline data

Line	В	F	N (mm)	D (mm)	С		L (m)	Q (L/SEC)	V (m/s)	I (1/1000)	h (m)
K1	PS	45	200	200		90	170.00	54.68	1.741	30.294	5.153
K2	12	39	200	200		90	390.00	9.49	0.302	1.179	0.463
K3	13	36	150	150		90	220.00	9.52	0.539	4.864	1.067
K4	36	37	150	150		90	200.00	2.02	0.114	0.249	0.055
K5	12	13	200	200		90	200.00	4.55	0.145	0.300	0.061
K6	13	14	200	200		90	470.00	20.01	0.637	4.723	2.219
K7	14	15	200	200		90	60.00	18.99	0.604	4.334	0.257
K8	15	32	200	200		90	560.00	5.57	0.177	0.446	0.248
K9	10	14	200	200		90	230.00	0.30	0.009	0.000	0.000
K10	39	10	125	125		90	200.00	8.27	0.674	9.100	1.816
K11	10	11	200	200		90 .	130.00	6.99	0.223	0.693	0.088
K12	42	8	150	150		90	60.00	6.34	0.359	2.336	0.137
K13	11	18	200	200		90	190.00	0.16	0.005	0.000	0.000
K14	11	42	200	200		90	90.00	6.34	0.202	0.553	0.051
K15	9	19	200	200		90	210.00	3.26	0.104	0.191	0.035
K16	15	16	200	200		90	150.00	11.36	0.361	1.667	0.248
K17	16	9	200	200		90	340.00	0.91	0.029	0.000	0.005
K18	16	17	200	200		90	70.00	5.55	0.177	0.429	0.031
K19	17	19	200	200		90	70.00	2.85	0.091	0.143	0.009
K20	17	31	200	200		90	530.00	0.78	0.025	0.000	0.006
K21	32	31	200	200		90	160.00	3.90	0.124	0.188	0.037
K22	19	20	200	200		90	140.00	5.65	0.180		0.064
K23	20	43	200	200	•	90	120.00	2.80	0.089		0.015
K24	36	35	150	150		90	240.00	7.50	0.425		0.749
K25	35	34	150	150		90	380.00	5.48	0.310		0.664
K26	34	33	150	150		90	310.00	3.46	0.196		0.231
K27	33	32	150	150	-	90	210.00	0.89	0.050	and the second s	0.013
K28	33	30	150	150		90	330.00	1.97	0.112		0.087
K29	41	30	150	150		90	60.00	2.20	0.124		0.019
K30	31	41	200	200		90	230.00		0.070		0.018 0.089
K31	27	25	150	150		90	310.00		0.117 0.887		1.829
K32	45	12	200	200		90 90	210.00 500.00		0.000		0.144
K33	29	27	150	150 150		90	100.00		0.117		0.029
K34	30	29	150 150	150		90	770.00		0.117		0.029
K35	29 43	20 26	150	150		90	310.00		0.000		0.156
K36	20	44	150	150		90	1200.00		0.058		0.467
K37 K38	26	25	150	150		90	130.00		0.138		0.063
K39	44	24	150	150		90	150.00		0.170		0.058
K40	25	21	150	150		90	400.00		0.035		0.228
K41	24	23	150	150		90	380.00		0.067		0.012
K42	22	23	150	150		90	230.00		0.170		0.024
K43	21	22	150	150		90	90.00		0.854		0.051
K44	45	38	200	200		90	30.00		0.817		0.243
K45	38	13	200	200		90	220.00		0.185		1.646
K46	8	40	150	150		90	330.00		0.100		0.221
K47	40	9	150	150		90	90.00		0.046		0.014
K48	2	3	150	150		90	370		0.098		0.019
K49	5	3	150	150		90	290		0.24		0.06
K50	7	5	150	150		90	400		0.23		0.439
K51	. 8	7	150	150		90	. 500		0.119		0.206
K52	6	7	150	150		90	360		0.26		0.399
K53	4	2	150			90	300		0.18		0.190
K54	6	4	150			90	390		0.31	7 1.769	0.689
K.55	12	6	150			90	310		0.71	1 7.613	2.362

Caluculation of Pipeline Network (Case 3)

[Caluculation Condition]

1 . Formula for Caluculation

Hazen-Williams Formula
$$\begin{split} h &= I \times L \\ I &= 10.666 \times C^{\text{-1.85}} \times D^{\text{-4.87}} \times O^{\text{1.85}} \end{split}$$

(Herein)

h: Friction loss (m)

L: Pipe length

I: hydraulic gradient (%)

C: Coefficient of flow rate condition (-)
(C=90: Supposed the pipe condition in 25 years after installation)

D : Pipe diameter (mm)

Q: Flow rate (L/sec)

2. Allowable range of In-put data and Out-put data

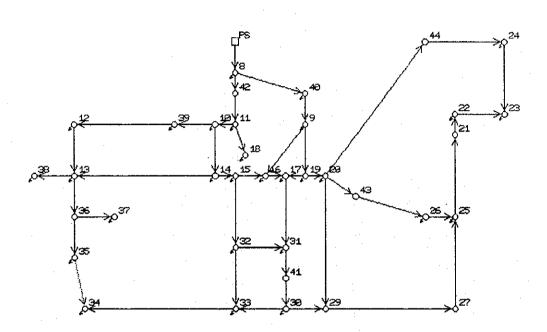
Minimum effective head	(m)	5.00
Maximum static head	(m)	75.00
Nominal diameter	(mm)	75~1000
Flow velocity	(m/sec)	0.0~5.0
Total Number of Nodes		37
Total Number of Pipelines		45

3 . Result of Caluculation

The results of caluculation are shown in next 5 pages for Case 3. (Herein)

Point	Mama afrada	7.3
Point	: Name of node	(-)
H	: Hydrodynamic head	(m)
G	: Ground Level	(m)
E	: Hydrodynamic effective head	(m)
S	: Static head	(m)
T	: Total distribution amount	(L/sec)
Line	: No.of Pipeline	(-)
В	:Start point	(~)
F	End point	(-)
N	: Nominal diameter of pipe	(mm)
V	:Flow velocity	(m/sec)

A. Caluculation of pipe netwo (case 3-1: For Low G·L Area)



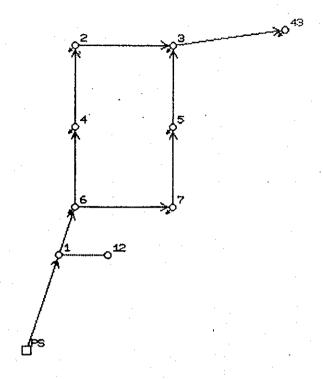
(2) Point data (PS:appointed the pumping station)

Point	Н	G	E	S	T
	(m)	(m)	(m)	(m)	(L/SEC)
38	2191.83	2165.00	26.83	33.50	1.150
13	2191.83	2165.00	26.83	33.50	0.680
12	2191.84	2170.00	21.84	28.50	1.670
36	2191.61	2160.00	31.61	38.50	0.000
37	2191.55	2160.00	31.55	38.50	2.020
39	2191.92	2170.00	21.92	28.50	1.220
10	2192.54	2165.00	27.54	33.50	0.980
42	2193.61	2165.00	28.61	33.50	0.000
18	2193.16	2165.00	28.16	33.50	0.160
14	2191.95	2165.00	26.95	33.50	1.320
15	2191.89	2165.00	26.89	33.50	2.060
32	2191.68	2145.00	46.68	53.50	2.560
11	2193.16	2165.00	28.16	33.50	0.490
16	2191.88	2165.00	26.88	33.50	4.890
17	2191.86	2165.00	26.86	33.50	1.930
9	2192.15	2165.00	27.15	33.50	0.800
40	2192.53	2170.00	22.53	28.50	0.120
- 31	2191.68	2145.00	46.68	53.50	2.480
19	2191.87	2165.00	26.87	33.50	0.460
20	2191.76	2165.00	26.76	33.50	0.420
43	2191.74	2165.00	26.74	33.50	0.000
33	2191.62	2140.00	51.62	58.50	
41	2191.66	2140.00	51.66	58.50	
30	2191.63	2140.00	51.63	58.50	
44	2191.26	2175.00	16.26	48.50	
34	2191.54	2140.00	51.54	58.50	
35	2191.54	2150.00	41.54	48.50	
29	2191.63	2145.00	46.63	53.50	
27	2191.53	2145.00	46.53	53.50	
26	2191.55	2160.00	31.55	38.50	
25	2191.47	2160.00	31.47	38.50	
21	2191.25	2170.00	21.25	28.50	
22	2191.21	2170.00	21.21	28.50	
23	2191.18	2170.00	21.18	28.50	
24	2191.20	2175.00	16.20	23.50	
8	2193.91	2170.00	23.91	28.50	
PS	2198.50	2208.50	-10.00	-10.00	-40.010

(3) Pipe	eline data									
Line	В	F	N	D	С	L	Q	V	· I	h
			(mm)	(mm)		(m)	(L/SEC)	(m/s)	(1/1000)	(m)
Κl	PS	8	250	250	90	800.00	40.010	0.815	5.738	4,590
K2	39	12	200	200	90	390.00	3.441	0.110	0.205	0.071
K3	13	36	150	150	90	220.00	4.091	0.231	1.000	0.224
K4	36	37	150	150	90	200.00	2.020	0.114	0.300	0.055
K5	12	13	200	200	90	200.00	1.771	0.056	0.050	0.011
K6	14	13	200	200	90	470.00	4.150	0,132	0.255	0.121
K7	14	15	200	200	90	60.00	8.914	0.284	1.001	0.063
K8	15	32	200	200	90	560.00	5.048	0.161	0.375	0.207
K9	10	14	200	200	90	230.00	14.384	0.458	2.566	0.589
K10	10	39	125	125	90	200.00	4.661	0.380	3.101	0.629
KII	11	10	200	200	90	130.00	20.025	0.637	4.768	0.614
K12	42	11	200	200	90	90.00	20.675	0.658	5.002	0.451
K13	11	18	200	200	90	190.00	0.160	0.005	0.000	0.000
K14	40	9	200	200	90	90.00	18.656	0.594	4.224	0.373
K15	9	19	200	200	90	210.00	10.220	0.325	1.332	0.286
K16	15	16	200	200	90	150.00	1.806	0.057	0.067	0.008
K17	9 .	16	200	200	90	340.00	7.636	0.243	0.794	0.270
K18	16	17	200	200	90	70.00	4.552	0.145	0.283	0.021
K19	19	17	200	200	90	70.00	2.169	0.069	0.143	0.005
K20	17	31	200	200	90	530.00	4.791	0.153	0.340	0.178
K21	32	31	200	200	90	160.00	0.393	0.013	0.000	0.001
K22	19	20	200	200	90	140.00	7.590	0.242	0.786	0.110
K23	20	43	200	200	90	120.00	3.115	0.099	0.167	0.018
K24	36	35	150	150	90	240.00	2.071	0.117	0.292	0.069
K25	35	34	150	150	90	380.00	0.051	0.003	0.000	0.000
K26	33	34	150	150	90	310.00	1.969	0.111	0.258	0.081
K27	32	33	150	150	90	210.00	2.095	0.119	0.285	0.062
K28	30	33	150	150	90	330.00		0.027	0.030	0.006
K29	41	30	150	150	90	60.00		0.153	0.500	0.028
K30	31	41	200	200	90	230.00		0.086	0.087	0.027
K31	20	44	150	150	9 0	1200.00	2.514	0.142	0.417	0.495
K32	8	42	200	200	90	60.00		0.658	4.997	0.301
K33	44	24	150	150	90	150.00		0.142	0.400	0.062
K34	30	29	150	150	90	100.00	0.130	0.007	0.000	0.000
K35	20	29	150	150	90	770.00	1.541	0.087	0.169	0.128
K36	43	26	150	150	. 90	310.00		0.176	0.613	0.190
K37	29	27	150	150	90	500.00		0.095	0.200	0.097
K38	26	25	150	150	90	130.00		0.173	0.616	0.077
K39	27	25	150	150	90,	310.00		0.095	0.194	0.060
K40	25	21	150	150	90	400.00		0.165	0.550	0.217
K41	24	23	150	150	90	380.00		0.040	0.053	0.015
K42	22	23	150	150	90	230.00		0.063	0.131	0.021
K43	21	22	150	150	90	90.00		0.165	0.445	0.049
K44	8	40	200	200	90	330.00		0.598	4.181	1.384
K45	13	38	200	200	90	220.00	1.150	0.037	0.000	0.005

B. Caluculation of pipe network

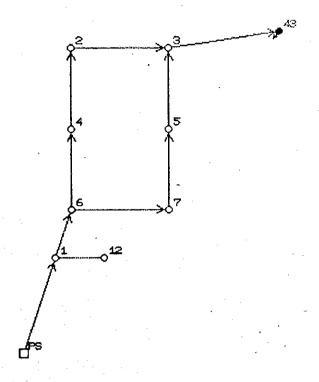
(case 3-2: For High G.L Area)



(2) Point data	(PS:appoin	ted the pun	nping statio	on)	•
Point	H	G	Е .	S	T
	(m)	(m)	(m)	(m)	(L/SEC)
PS	2228.50	2163.50	65.00	65.00	-24.770
6	2223.74	2180.00	43.74	48.50	2.350
. 7	2222.50	2180.00	42.5	48.50	2.540
4	2222.72	2190.00	32.72	38.50	2.350
5	2221.83	2190.00	32.83	38.50	2.540
2	2222.15	2200.00	22.15	28.50	2.350
3	2221.69	2200.00	21.69	28.50	2.540
43	2220.63	2212.00	8.63	16.50	10.100
1	2225.56	2170.00	55.56	58.50	0.000
12	2225.56	2170.00	55.56	58.50	0.000

(3) Pipe	line data			. •						
Line	В	F	N	D	C	L	Q	V .	I	h 👉
			(mm)	(mm)		(m)	(L/SEC)	(m/s)	(1/1000)	(m)
K1	1	6	200	200	. 90	260.00	24.770	0.788	7.000	1.821
K2	6	7	150	150	90	360.00	7.895	0.447	3.444	1.234
K3	7	5	150	150	90	400.00	5.355	0.303	1.675	0.669
K.4	5	3	150	150	90	290.00	2.815	0.159	0.483	0.148
K5	2	3	200	200	90	370.00	9.825	0.313	1.243	0.468
K6	4	2	200	200	90	300.00	12.175	0.388	1.900	0.565
K7	6	4	200	200	90	390.00	14.525	0.462	2.615	1.018
K8	3	43	200	200	90	790.00	10.100	0.321	1.342	1.053
K9	12	1	200	200	90	30.00	0.000	0.000L	0.000	0.000
K10	PS	1	200	200	90	420.00	24.770	0.788	7.000	2.942

B. Caluculation of pipe netw (case 3-2 night: For High G·L Area)

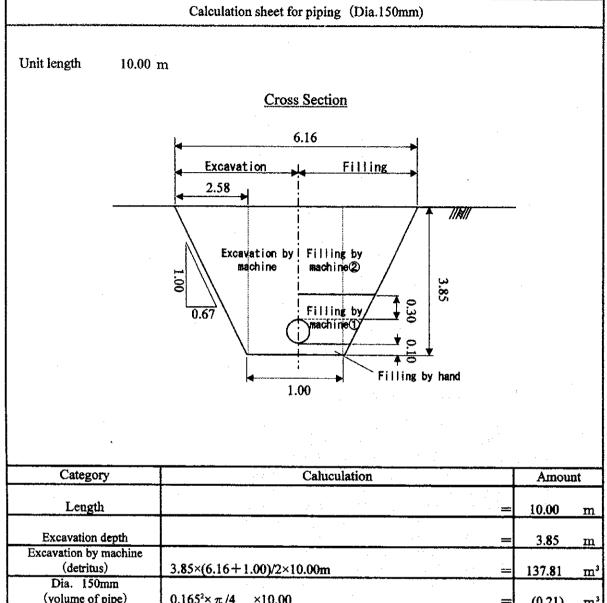


(2)	Point data	(PS:appoir	nted the pur	mping stati	on)	
	Point	. Н	G	E	S	T
		(m)	(m)	(m)	(m)	(L/SEC)
	PS	2228.50	2163.50	65.00	65.00	-24.770
	6	2227.59	2180.00	43.74	48.50	0.000
	, 7	2227.36	2180.00	42.49	48.50	0.000
	4	2227.34	2190.00	32.40	38.50	0.000
	5	2227.09	2190.00	31.11	38.50	0.000
	2	2227.14	2200.00	21.37	28.50	0.000
	3	2226.90	2200.00	20.10	28.50	0.000
	43	2225.85	2212.00	2.57L	16.50	24,770
	1	2227.94	2170.00	55.56	58.50	0.000
	12	2227.94	2170.00	55.56	58.50	0.000

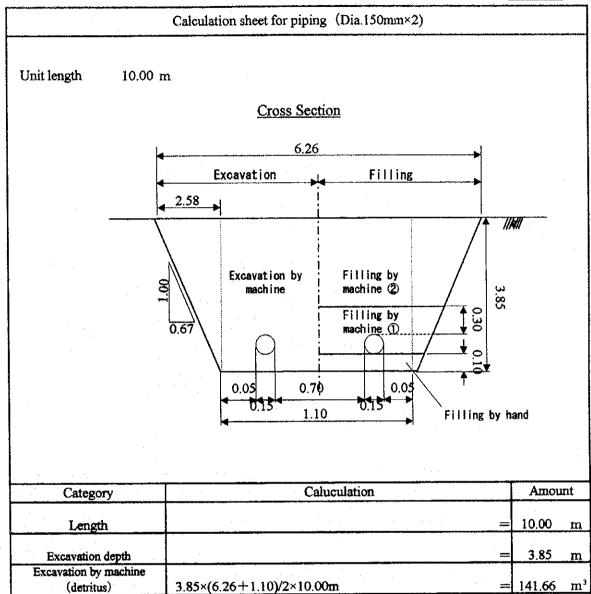
(3) Pip	eline data		* * * * * * * * * * * * * * * * * * * *					•		
Line	В	F	N :	D	C	L	Q	V	1	h
			(mm)	(mm)		(m)	(L/SEC)	(m/s)	(1/1000)	(m)
. K 1	1	6	200	200	90	260.00	24.770	0.788	7.000	1.821
K2	6	7	150	150	90	360.00	7.935	0.449	3.472	1.246
K3	7	5	150	150	90	400.00	7.935	0.449	3.450	1.385
K4	· 5	3	150	150	90	290.00	7.935	0.449	3.483	1.004
K5	2	3	200	200	90	370.00	16.835	0.536	3.432	1.269
-K6	. 4	2	200	200	90	300.00	16.835	0.536	3.433	1.029
K7	6	. 4	200	200	90	390.00	16.835	0.536	3.436	1.337
K8	3	43	200	200	90	790.00	24.770	0.788	7.000	5.534
K9	12	1	200	200	90	30.00	0.000	0.000L	0.000	0.000
K10	PS	1	200	200	90	420.00	24.770	0.788	7.000	2.942

Annex IX-3

PAGE



	.1	
Length		10.00 m
Excavation depth	=	3.85 m
Excavation by machine (detritus)	3.85×(6.16+1.00)/2×10.00m =	137.81 m
Dia. 150mm (volume of pipe)	$0.165^2 \times \pi/4 \times 10.00$	(0.21) m
Filling by hand (sand)	0.10×(1.00+1.13)/2×10.00m =	1.07 m
Filling by machine ① (including pipe volume)	$(0.55-0.10)\times(1.74+1.13)/2\times10.00$ m =	6.46 m
Filling by machine ① (excluding pipe volume)	(0.55-0.10)×(1.74+1.13)/2×10.00-0.21 =	6.25 m
Filling by machine ②	137.81-6.46-1.07	
· · · · · · · · · · · · · · · · · · ·		



Category	Caluculation	Amou	nt
Length	_	10.00	m
Excavation depth		3.85	m
Excavation by machine (detritus)	3.85×(6.26+1.10)/2×10.00m =	141.66	$\mathrm{m}^{\scriptscriptstyle 3}$
Dia. 150mm×2 (volume of pipe)	$0.165^2 \times \pi/4 \times 2 \times 10.00$	(0.21)	m³
Filling by hand (sand)	0.10×(1.23+1.10)/2×10.00m =	1.17	m^3
Filling by machine (1) (including pipe volume)	(0.55-0.10)×(1.84+1.23)/2×10.00m =	6.91	m³
Filling by machine ① (excluding pipe volume)	(0.55-0.10)×(1.84+1.23)/2×10.00-0.21 =	6.70	m³
Filling by machine ②	141.66-6.91-1.17	133.58	m³

Calculation sheet for piping (Dia.200mm) Unit length 10.00 m Cross Section Excavation Filling 2.61

Fillingk by machine ②

Filling by ymachine 1

1.00

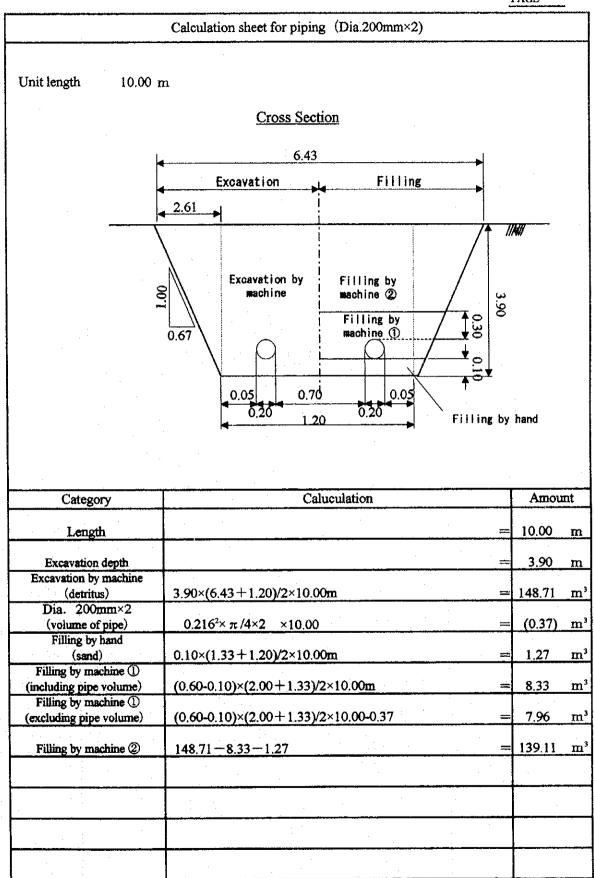
3.90

Filling by hand

Excavation by machine

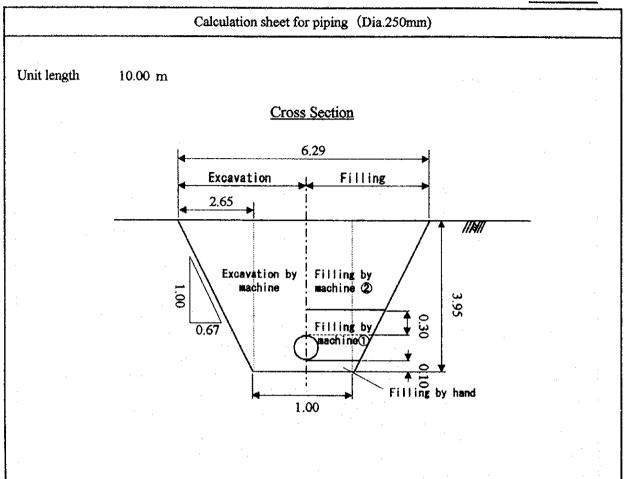
1.00

Category	Caluculation	Amou	nt
Length		10.00	n
Excavation depth	—	3.90	п
Excavation by machine (detritus)	3.90×(6.23+1.00)/2×10.00m =	140.91	n
Dia. 200mm (volume of pipe)	$0.216^2 \times \pi/4 \times 10.00$	(0.37)	n
Filling by hand (sand)	1.80×(1.00+1.13)/2×10.00m =		n
Filling by machine (1) (including pipe volume)	(0.60-0.10)×(1.80+1.13)/2×10.00m =	<u>.</u>	n
Filling by machine ① (excluding pipe volume)	(0.60-0.10)×(1.80+1.13)/2×10.00-0.37	6.96	: m
Filling by machine ②	140.91 - 7.33 - 1.07 =	132,51	1
			·



Annex IX-3

PAGE



Category	Caluculation	Amount		
Length		=	10.00	m
Excavation depth		_	3,95	m
Excavation by machine (detritus)	3.95×(6.29+1.00)/2×10.00m		144.04	m³
Dia. 250mm (volume of pipe)	$0.267^2 \times \pi/4 \times 10.00$		(0.56)	m³
Filling by hand (sand)	1.87×(1.00+1.13)/2×10.00m		1.07	m³
Filling by machine () (including pipe volume)	(0.65-0.10)×(1.87+1.13)/2×10.00m		8.25	m³
Filling by machine ① (excluding pipe volume)	(0.65-0.10)×(1.87+1.13)/2×10.00-0.56	_	7.69	m³
Filling by machine ②	144.04-8.25-1.07	==	134,72	m³
			.:	
			-	
				:

