

2.6 HYDROGEOLOGICAL CONDITION

2.6.1 General

(1) Outline of the study

After the establishment of Altai City in 1940, the first well was constructed in 1956 and many wells were drilled during 1960's and 1980's. Various hydrogeological studies, including drilling of test wells and observation wells, also have been carried out in Sukhiin Hooloi, Kharzat, and Olon Nur areas. In addition, summarizing the result of these past studies, the Study Team carried out the following work during the Study.

- Collection and analysis of the existing data.
- Hydrogeological field survey.
- Measurement of groundwater levels (the work comprises periodic measurement of the observation wells and continuous recording by the water level recorders installed on some wells).
- Analysis of the pumping test data of the JICA test wells.
- Water quality analysis of the JICA test wells and the selected wells.

(2) General information on the wells

A number of test wells, production wells and exploration boreholes were drilled in the Study Area in the past. Most wells are, however, not in use or abandoned now. A well inventory for the Study Area was established as shown in Table 2.6.1, which includes 99 deep boreholes and the 10 test wells drilled for this Study. Locations of the wells are shown in Figure 2.6.1. The drilled depth ranges from 12 meters to 200 meters. The details of these boreholes are described in the following sections. Table 2.6.2 shows the summarized hydrogeological description in the area.

2.6.2 Aquifer Type

There are two types of hydrogeological units that are expected to be an aquifer, namely basement rocks (Fissured Aquifer) and unconsolidated deposits of Tertiary and Quaternary Formation (Granular Aquifer).

(1) Aquifers in the basement rocks

Proterozoic and Paleozoic formations are mainly composed of metamorphic, carbonate and granitic rocks which are compact, hard and practically impermeable with poor porosity. These formations are considered to be the basement rock from the hydrogeological point of view. Many geological logs of these formations, however, show the development of joints and fractures formed by weathering, tectonic movements, etc. Moreover, the results of the field reconnaissance and interpretation of topographic map, aerial-photographs and satellite images point out the existence of lineaments that correspond to fault and fractured zones in the mountain area.

This sort of fractured zones form fissure aquifers. It was reported that the pumping yield of wells drilled in the fracture zones ranges from 1.6 to 14.4liters/sec.

(2) Aquifers in the Unconsolidated deposits

The Deposits forming aquifers consist of Tertiary and Quaternary formations.

Quaternary formation is mainly composed of permeable deposits of alluvial fan, talus, and recent river. Alluvial aquifer is distributed in the area of rather thick quaternary deposits consisting of sand, and gravel with clay. Judging from the geological logs of boreholes, results of pumping test, and other surveys, this alluvial aquifer is classified into unconfined aquifer. The potential of the groundwater varies depending on the thickness of the quaternary deposits, particle size of deposits and precipitation, etc. The thickness of Quaternary deposit is shown as an isopach map in Figure 2.6.2. The thickness varies from 2 to 70 meters.

2.6.3 Aquifer Characteristics

(1) General

From the geologic and geomorphologic viewpoint, the Study Area can be divided into two main areas. They are Sukhyn Khooley, and Kharzat-Olon Nuur.

Sukhyn Hooloy in the north of Altai City and Kharzat-Olon Nuur in the south of the City are separated by the basement rocks of Sertengyn mountain range distributed from east to west in the Study Area. Kharzat and Olon Nuur can be separated by a fault inferred by this study. The distribution of the three aquifers are shown in Figure 2.6.3. The hydrological constants for each area are also shown in the table below.

Area		Depth (m)	Pumping Rate (liter/sec.)	Specific Capacity (m ³ /day/m)	Transmissivity (m ² /day/m)	Hydraulic Conductivity (m/day)
Sukhyn Khooley	Max	193	10.4	187.2	307.1	14.7
	Min	28.2	0.2	1.8	1.3	0
	Average	94.8	3.3	40.9	66.1	3.3
Kharzat	Max*	200.3	17	864	1055	21
	Min	12	0.01	0.33	1.3	0.01
	Average*	66.7	3.3	72.2	109	3
Olon Nuur	Max*	175	7	454	590	16
	Min	46	0.05	1.32	3	0.11
	Average*	104.2	2.7	61.2	119.8	3.4

*:Extraordinarily high values were removed.

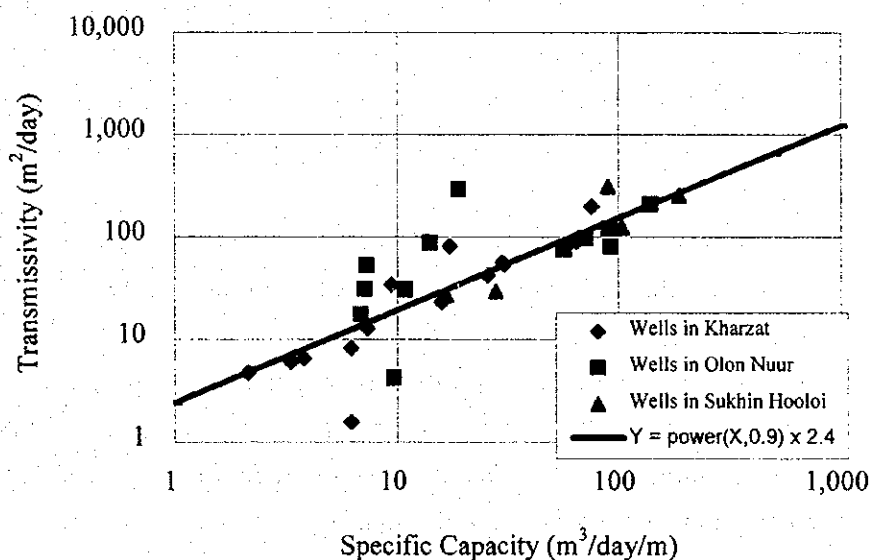
In Kharzat, the fractured basement rocks are covered with comparatively thin quaternary sediments. An aquifer occurs both in the fractured rocks and in the overlying sediments as a single hydrologic unit.

The quaternary sediments are thick and overlay the Neogene deposits in Olon Nuur. An aquifer occurs in the quaternary sediments.

The Isopach Map of the main aquifer, Figure 2.6.3, was drawn based on the collected water level data, the result of the study, and the isopach of the Quaternary deposit.

For Kharzat and Olon Nuur, distribution maps of transmissivity and permeability coefficient, Figure 2.6.4 and 2.6.5, were drawn. These maps were based on the collected data and the estimated figures obtained by the correlation between specific capacity and transmissivity shown below.

Correlation between Sc and T in the area



(2) Altai City Area and Kharzat

The previous studies by the Mongolian side revealed that the groundwater in Kharzat is stored both in Quaternary deposits and the permeable fractured rocks. Although the groundwater storage formation is heterogeneous, the recorded continuous water level fluctuations indicate that the body of Quaternary deposits and the fractured rocks can be considered a single aquifer system of one hydrologic unit.

Although many boreholes and wells were drilled in the city area and Kharzat, most boreholes and wells are not in use at present. Four (4) wells in Kharzat are working as production wells operated by Altai City Public Service Department. Pumping yield of boreholes drilled for the past studies varies widely from 0.01 to 12.4 liters/sec.

In this JICA Study, Some deep test wells were drilled in and around Altai City.

Four test wells (A-1, A-2, A-3, and A-4) were drilled for the purpose of reaching a fissured aquifer. A-1 is located on the western edge of Altai City, the depth of which is 200 m. A-2 was constructed about 5 km to the northeast of the Altai City, the depth of which is 193 m. A-3 was drilled until a depth of 150 m in Khadaasan Valley. A-4 is located in the northeast of Altai City.

As described already in the section of the result of pumping test, these wells can yield only limited quantity and the quality of water is poor except for A-4.

(3) Olon Nuur

It is reported that about 20 boreholes were drilled for the past studies. However, none of them is in use at present. Pumping rate of these wells ranged from 1.2 to 7.1 liters/sec. The average rate was 2.51 liters/sec. Specific capacity varied widely from 1 to 93 m³/day/m. The range of hydraulic conductivity in the area was from 0.1 to 16.3 m/day. Higher values occurred generally in the area bordering Kharzat, where the thickness of quaternary deposits is expected to be more than 50 meters.

During the Study, two test wells, B-5 and B-6, were constructed in the area. The pumping tests of the both wells showed a good result with high yield.

Figure 2.6.6 is the result of the geophysical logs of the two wells. The curves of resistivity, gamma, and SPR show almost similar pattern in a part where the most productive layers may exist. This suggests that the productive layer spreads continuously with some extent. The distribution map of transmissivity (refer to Figure 2.6.4) may indicate the extent of the aquifer.

Neogene deposits that underlay Quaternary deposits are not considered an exploitable aquifer.

(4) Sukhyn Hooloy

Aquifers occur sporadically in Quaternary deposits. Pumping yield of the boreholes ranged from 0.18 to 10.4liters/sec ($15.6\text{m}^3/\text{day}$ - $899\text{m}^3/\text{day}$). The mean pumping rate was 3.59liters/sec. The borehole (No.7) yielded the maximum volume in the area. In addition, it showed the maximum specific capacity in the area, $187\text{m}^3/\text{day}/\text{m}$. Other boreholes (No.6, 8, and 9) around No.7 borehole showed comparatively high specific capacities as well, (90 to $104\text{m}^3/\text{day}/\text{m}$). According to the geological logs of these boreholes, thick quaternary deposits are distributed over the entire area. Hydraulic conductivity ranges from 4.57 to 8.3m/day in this area. The groundwater potential seems to be high in this limited part and low in the other parts because of thin alluvial sediments and thick impermeable clay stratum.

The JICA test wells (B-2, B-3, and B-4) were constructed in the eastern end of the area, northeast of Altai City. However, underlying Quaternary deposits, which were not so thick or permeable as they were expected. According to the result of the pumping tests, transmissivity ranges from 1 to $5\text{m}^3/\text{day}/\text{m}$ and hydraulic conductivity ranges from 0.04 to 0.21 m/day in the area. The values indicate the aquifer has very low productivity.

2.6.4 Groundwater Level

(1) General

Groundwater table fluctuates depending on natural and artificial conditions as listed below.

The natural condition	The artificial condition
seasonal and daily change in precipitation evaporation level of surface water atmospheric pressure	pumping rate from wells construction work and others.

Fluctuation of groundwater table in unconfined aquifer represents the change of water volume in the aquifer. Therefore, continuous measurement of groundwater level is indispensable for groundwater development and management. Groundwater levels of most wells in the Study Area, however, have not been measured since the wells were constructed.

The Study Team installed the automatic water level recorders provided by JICA on selected wells to observe continuous water level fluctuations. In addition, with a portable water level gauge provided by JICA, the Study Team measured periodically water levels of 17 wells including the wells where the water level recorders were installed. The names and locations of these observation wells are shown in Figure 2.6.7. The measurements of water levels were carried out from July 1997 to October 1998 in cooperation with the Groundwater Development Company in Gobi Altai province.

The depth to water map expressed in meter below ground level, Figure 2.6.8, and the water level contour expressed in meter above sea level, Figure 2.6.9, were drawn on the basis of the collected data including the previous studies.

(2) Water Level Fluctuation

The result of the water level measurement is shown in Table 2.6.3 and Figure 2.6.10(1) ~ (4).

To graph the result, the observation wells were divided into three groups as follows;

Group A: Relatively deep wells in the area from Altai city to Olon Nuur. No.1, 4, and 5 belong to this group. Well depth ranges from 30 to 70 meters.

Group B: Shallower wells in the above area and the northeast of Altai city that is the downstream of streams from Kharzat and Olon Nuur. This group comprises No.2, 6, 10, and some dug wells in the area. The depth ranges from 1 to 6.6 meters.

Group C: Wells in Sukhyn Khooley. The depth ranges from 1.1 to 4.5 meters.

Figure 2.6.10 (1), (2), and (3) are the hydrograph of Group A, B, and C, respectively. Figure 2.6.10 (4) was prepared to compare these three groups and natural factors, rainfall and temperature.

The wells that belong to the same group have generally a similar tendency of water level fluctuation as shown in the figures. However, Figure 2.6.10 (4) indicates that

there is a difference in tendency between the wells of Group A, which are relatively deep, and the shallower wells of Group B and C.

The water levels of Group B and C start going down soon after the end of rainy season, which was July in 1997 and 1998. Meanwhile the levels of Group A continue to rise until the end of October. From the beginning of February, the levels of Group B wells commence to increase while the levels of Group A keep decreasing until the middle of April. The details are as follows;

The groups of shallower wells, B and C, seem to be influenced considerably by two natural factors, of rainfall and temperature. For example, the hydrograph of No.6 named water supply well (depth is 6.6 meters) shows that the water level rises in a few days after rainfall reaches more than seven millimeters per day. The trend of the running average lines of daily temperature agrees generally with the fluctuation of the well No.6 except rainy periods as shown in Figure 2.6.10 (4). Around the middle of February, maximum ground temperature rises above the freezing point, and then the water level begins to go up.

The ground temperature affects evaporation from the ground and the freezing of the ground also affects the water flow under ground. The characteristics of water level fluctuation of each group are considered to reflect these two factors.

2.6.5 Groundwater Quality

Water quality analysis of the eight existing wells and the ten test wells was conducted. The result is described in detail in Section 2.8. This section deals with the stiff diagrams, which show similarities and differences between the collected groundwater samples.

Figure 2.6.11 (1) is the stiff diagrams of the test wells. The diagrams show clearly the difference between the wells in the north of Altai City and the wells in the south of the city, near Olon Nuur. The former are A-1, A-2, A-3, A-4, B-1, B-2, B-3, and B-4; the latter are B-5 and B-6.

The water quality of A-1 to A-4 and B-1 to B-4 is characterized by very high value of sulfate ion. This group also shows higher values of magnesium and calcium ion. These wells have been drilled into the consolidated basement rocks.

B-5 and B-6 are located near Olon Nuur. The stiff diagrams of the two wells are similar to the one of Kharzat Intake Well but magnesium content is a little higher than Kharzat (refer to Figure 2.6.11 (2)).

Figure 2.6.11 (2) shows the water quality of the wells selected by the Study Team. All of them have higher value of bicarbonate ion than sulfate ion. Six out of eight wells are shallow dug wells and two other wells are in the south of Altai. The higher bicarbonate type probably indicates that the water stays shorter period in underground than the higher sulfate type. In this sense the groundwater in Kharzat and Olon Nuur may be comparatively fresh.

The shape of the diagrams resembles each other in the area from Altai City to Kharzat, that is, Park Well, Kharzat Intake Well, and School Well. The diagram size of the shallowest School Well is slightly larger than the those of the others, which means the total concentration of major ions is higher.

Figure 2.6.12, the map plotted with the stiff diagrams, shows differences between groundwater in Sukhyn Hooloy and Kharzat.

2.6.6 Existing Groundwater Utilization

In the Study area, 13 wells are in use at present as shown below:

Owner	Number of well	Remarks
APSD	4	at Kharzat water source
Power station	1	about 4km from Kharzat water source
Veterinary hospital	1	about 4km from Kharzat water source
City planning office	1	about 4km from Kharzat water source
Private company	2	Altai camel, Entum
Unknown	4	in Sukhyn Hooloy, little consumption, for livestock

APSD: Altai City Public Service Department

The existing groundwater utilization was surveyed as a part of the water supply facility study. The result is described in detail in Section 2.9. The capacity of intake pumps installed in the production wells is 25 m³/hour x (80 m – 100 m). Two production wells are pumped up by turns or simultaneously. The total yield of Kharzat production wells is about 960 m³/day in an average and 1,150 m³/day in a maximum at present. Other wells are not located in Kharzat water source and their date were lost.

2.6.7 Recharge Volume

The average annual groundwater flow through an aquifer can be quantified by

considering the disposition of the rain that falls on the recharge area. The recharge volume to Kharzat and Olon Nuur is estimated in this section.

Figure 2.6.13 shows the catchment area of Kharzat and Olon Nuur.

Area	Catchment area	Annual Precipitation	> 8mm/day in a year	Recharged precipitation	Recharged volume
Kharzat	about 70km ²	181.6 mm	34 mm	14% of total 25.4mm/year	4,870 m ³ /day
Olon Nuur	about 80km ²	181.6 mm	34 mm	14% of total 25.4mm/year	5,567 m ³ /day

The area of Kharzat is about 70 km². The annual average precipitation is reported at 181.6 mm. About 64 % of precipitation concentrate during rainy season from June to August. The result of the continuous water level observation indicates that rainfall affects water level if it is over eight millimeters per day. It probably means that rainfall of seven millimeters or less flow out from ground surface.

In 1998, the total of rainfall that is over eight millimeters per day was 34.4 mm. The value is 25 % of the total rainfall in the rainy season of the same year. (The total precipitation during the rainy season usually) accounts for 64 % of annual precipitation. Therefore, 16 %, or 0.64 multiplied by 0.25, of annual precipitation is the roughly estimated recharge to underground. Two percent of precipitation is considered to become interflow and to move laterally above water table. Consequently, 14 % of annual precipitation, or 25.4 mm, recharges to the aquifer. On these assumptions the estimated total recharge volume becomes 1,778,000 m³/year, or 4,870 m³/day in Kharzat area.

The area of Olon Nuur is about 80km². The similar estimation as described above can be applied to this area. The estimated total recharge volume is calculated to be 2,032,000 m³/year, or 5,567 m³/day in Olon Nuur area.

2.6.8 Groundwater Potential

(1) General

1) Alluvial Aquifer

The result of the hydrogeological study described above indicates that the Sukhyn Khooloy is less feasible for the development of groundwater resources in terms of both quality and quantity.

The consideration of water budget of Kharzat area suggests that it is possible to further

exploit the area although the area already produces about 1,150 m³/day in a maximum presently. In addition, this study revealed that a productive and usable aquifer occurs also in Olon Nuur. This aquifer may be exploited after the yield of Kharzat aquifer reaches the limit in future. At present, the aquifer in Kharzat is the most practicable to develop for future water demand in Altai City.

2) Fissure Aquifer

The productivity of A-4 well is excellent, but its quality is a problem as described already. Therefore, the aquifer developed by A-4 cannot be expected to be usable at the moment. The fissure aquifer in the study area is generally less feasible than alluvial aquifers because of the poor water quality. The water quality of A-3 is better than the others, but still some items are over the Mongolian water quality standard. Consequently these fissure aquifers are less practicable compared with Kharzat and Olon Nuur aquifer.

3) Alternative Water Resources

A large amount of water may become necessary to meet water demand in the future, after Kharzat and Olon Nuur aquifer is fully developed. In such a case, Zavkhan River will be considered as the new source of water supply. This issue is discussed in chapter 3.

(2) Priority Site to be developed

The result of the hydrogeological study revealed the following points about the aquifers in the area.

- Water quality of the aquifer in Skhyn Hooloy is generally poor and some items exceed greatly the Mongolian drinking water standard.
- Yield of the aquifer in Skhin Hooloi is not expected to be exploitable.
- Water quality of the fissure aquifer is also poor.
- Yield of the fissure aquifers varies widely.
- Water quality of the aquifer located in Olon Nuur, which is investigated by B-5 and B-6 in the Study, is near the Mongolian standard.
- Yield and productivity of B-5 and B-6 is higher than that of the other test wells.
- Water quality of Kharzat aquifer is the best in the area.
- Kharzat aquifer produces about 1,150 m³/day in a maximum at present though the recharge volume is estimated about 4,870 m³/day.

These points suggest that the first thing to be considered is to make the best use of Kharzat aquifer. The Olon Nuur aquifer whose potential was confirmed by B-5 and B-6 in this Study should be developed after Kharzat aquifer is fully exploited.

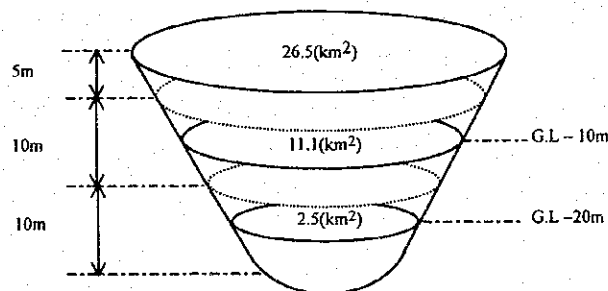
(3) Groundwater Storage in Kharzat and Olon Nuur

Based on the Isopach map of the aquifer, the volume of groundwater storage can be roughly calculated as follows;

1) Kharzat

Assuming the aquifer is simply shaped as shown below,
The area of each layer of the aquifer will be as follows.

Depth	0	10	20	(GL-m)
Thickness	5	10	10	(meters)
The area	26.5	11.1	2.5	(km ²)



The volume of the whole aquifer can be calculated by the following simplified expression.

$$26.5 \times 10^6 \times 5 + (11.1 + 2.5) \times 10^6 \times 10 = 268.5 \times 10^6 \text{ (m}^3\text{)}$$

Effective porosity of the aquifer material is estimate at 0.1~0.15. Then, the groundwater volume stored in the aquifer is;

$$268.5 \times 10^6 \times (0.1 \sim 0.15) = 26.9 \sim 40.3 \times 10^6 \text{ (m}^3\text{)}$$

This volume covers the amount extracted by a continuous pumping for more than 49 years when the daily discharge is 1500m^3 .

2) Olon Nuur

In the same way,

The area of each layer of the aquifer;

A thickness	0	10	20	30	40	(meters)
The area	29.4	11.8	8.5	3.1	0.13	(km^2)

The volume of the whole aquifer is calculated to be,
 $38.2 \times 10^6 \sim 57.3 \times 10^6 (\text{m}^3)$

(4) Groundwater Potential of Kharzat Water Resources

The following simple equation is representing amount of groundwater flow through an aquifer;

$$\text{Recharge to the aquifer} - \text{Discharge to the aquifer} = \text{Change in groundwater storage}$$

In the Kharzat area, the estimated groundwater storage is about $27 \sim 40 \times 10^6 \text{ m}^3$ and the estimated recharge volume is $4,870 \text{ m}^3/\text{day}$ on annual average. The record of the water level fluctuations indicates that the recharge to the aquifer occurs mostly in rainy season. Although the pumping from the aquifer reduces the groundwater storage in winter, the storage has a large surplus and recovers in the next rainy season.

The present yield from the aquifer is $1,150 \text{ m}^3/\text{day}$ in a maximum. Therefore, more than $3,000 \text{ m}^3/\text{day}$ is considered still available for water supply. However, when the development plan is implemented newly, it should be avoided that a large amount of water is pumped up from only one production well. It may cause local reduction of water level and the deterioration of water quality.

2.6.9 Program for Groundwater Utilization

(1) General

According to the water budget estimation in the previous section, more than $3,000 \text{ m}^3/\text{day}$ is still available from the aquifer within natural recharge. So long as the extraction is kept less than natural recharge, the points to be paid attention to are pumping operation and location of wells in a well field. A pumping well should be

located not too close to each other and should be operated not to cause excessively low pumping water level. When pumping wells are too close, interference creates additional drawdowns with corresponding higher pumping lifts. A decline in the pumping water level leads to an increase in the pumping head and reduced discharge.

(2) Program for Groundwater Utilization

In the Master Plan, four intake(production) wells in Kharzat have been planned to pump up 600-800 m³/day with a drawdown of 4~6 meters. According to the collected data, specific capacity of the aquifer ranges from 104 to 432 m³/day/m. Based on the relation between specific capacity and transmissivity, it is estimated that the value of transmissivity ranges from 157 to 565 m²/day.

The optimum pumping yield for a well is examined with Theis non-equilibrium equation on the basis of the following assumed figures;

- 1) Transmissivity is 360 m²/day that is the mean value in the area.
- 2) Storage coefficient of the aquifer that is semi-confined mixed of alluvial aquifer and fissure aquifer is 0.005.

The above-assumed figures can be applied first to the present condition as described below;

Assume that there are two intake wells which are 100m apart working simultaneously. When the annual average of pumping rate of the wells is 480 m³/day, the drawdown in one cased well with the inside diameter of 0.2 m is calculated to be 2.39 m after 365 days of pumping. The interference drawdown caused by the other intake well is calculated to be 0.92 m. Therefore the total drawdown in the intake well will be 3.31 m.

The figure can be considered reasonable compared with the original planned figures by the Mongolian side. The drawdown in the aquifer should not exceed six meters (ideally four meters). If the actual drawdown is more than the estimated drawdown, it is possible that the well performance has been deteriorated by some factors.

The following table shows the estimated drawdown in a well located in the center of the well field when it is surrounded by one to four wells respectively. The table also shows the projected water demand in 2005 and 2015. The conditions for the calculation are,

- the pumping duration is 3650 days
- the wells with the inside diameter of 0.2m are located in a straight line and the one in the center is focused on
- the wells are operated simultaneously with the same pumping rate

When the wells are operated, the drawdown should be kept around 4m while satisfying the projected water demand. At present there are four intake wells including one that is under repair. The table indicates that the operation of three wells is sufficient for water demand in 2005. Therefore, one well can be used as a spare. In 2015, if the intake wells are operated attentively with a water level monitoring, four wells are adequate for the water demand. In that case, it is recommended that a spare well should be constructed anew.

Year		1997-98	2005	2015	2015
Water demand (average, m ³ /day)		960	1140	1500	2140
Total number of wells					
2	Pumping rate / well (m ³ /day)	1500	570x2	750x2	1070x2
	DD* (Pumping)	-2.63 m	-3.12 m	-4.11 m	-5.87 m
	DD* (Interference)	-1.17 m	-1.38 m	-1.82 m	-2.6 m
	Total drawdown of the center well	-3.80 m	-4.50 m	-5.93 m	-8.47 m
3	Pumping rate / well (m ³ /day)		380x3	500x3	714x3
	DD* (Pumping)		-2.08 m	-2.74 m	-3.91 m
	DD* (Interference)		-0.92x2 m	-1.21x2 m	-1.73x2 m
	Total drawdown of the center well		-3.92 m	-5.16 m	-7.37 m
4	Pumping rate / well (m ³ /day)			375x4	535x4
	DD* (Pumping)			-2.06 m	-2.93 m
	DD* (Interference)			-0.91x2 m	-(1.30x2+1.03) m
	Total drawdown of the center well			-4.67 m	-6.56 m
5	Pumping rate / well (m ³ /day)			300x5	428x5
	DD* (Pumping)			-1.64 m	-2.35 m
	DD* (Interference)			(-0.73-0.64)x2 m	-(1.04+0.91)x2
	Total drawdown of the center well			-4.38 m	-6.25 m

DD; drawdown Well location; every 100 m on a straight line
Pumping duration: 3650days

The calculation in the above table is based on the Theis non-equilibrium equation. When two or more wells are mutually interfering, the general rule is that the total drawdown is the sum of the individual drawdowns.

Table 2.6.1 Well Inventory (1/4)

Map No.	Code No.	Area	Coordination		Elevation (m)	Depth (m)	Screen (m)	Pumping Rate (Q/liter/sec)	S.W.L (GL-m)	Drawdown (m)	Specific Capacity (m ³ /day/m)	Transmissivity (m ² /day)	Thickness of permeable formation (m)	Permeability coefficient (m/day)	Stratigraphy (m)			Geologic Column	Construction Year	
			Coordinate X	Coordinate Y											Quaternary Deposit	Neogene Deposits	Consolidated Rocks			
1	20	Sukhyn Kholoi	96° 13' 55"	46° 30' 50"	2155	28.2		2.41	2.72	10.96	19	(34) *		4.1	0-3.0	3.0-28.2		○	1981	
2	3		96° 11' 40"	46° 29' 48"	2200	64.0		3.40	0.10	10.40	28	29		34.1	0.9	0-34.2	34.2-64.0		○	1984
3	1644		96° 10' 00"	46° 29' 00"	2172	76.0		3.60	6.5	25.0	12	(23) *				0-3.0	3.0-76.0			
4	2		96° 13' 40"	46° 28' 48"	2072	84.0		1.20	5.40	6.20	17	27		9.3	2.89	0-14.7	14.7-82.0	82.0-84.0	○	1969
5	102		96° 08' 44"	46° 27' 45"	2164	69.3			Dry					17.8		0-17.8		17.8-69.3		
6	8		96° 09' 25"	46° 26' 50"	2111	131.0		6.00	4.20	5.00	104	122		26.8	4.6	0-31.0	31.0-129.6	129.6-131.0	○	1984
7	107		96° 10' 16"	46° 26' 28"	2110	102.0		10.40	1.10	4.80	187	251		45.6	5.5	0-45.6	45.6-102.0			1980
8	124		96° 11' 24"	46° 26' 04"	2118	170.0		2.00	46.5	1.92	90	307		37.0	8.3	0-37.0	37.0-170.0			
9	106		96° 12' 00"	46° 25' 37"	2140	44.8		4.10	3.2	3.50	101	(153) *		10.4	(14.7) *	0-10.4		10.4-44.8		
10	56		96° 19' 32"	46° 26' 23"	2012	35.1		0.18	22.2	7.4	2	(5) *				0-3.0		3.0-35.1		1982
11	11*		96° 17' 10"	46° 25' 21"	2005	96.7		2.61	+0.2	35.20	6	(13) *		6.5	(2.0) *	0-6.5	6.5-75.4	75.4-96.65	○	1064
12	1619*		96° 10' 30"	46° 23' 30"	2228	40.0		0.50	16.00							0-7.0		7.0-40.0	○	1985
13	10*		96° 10' 40"	46° 22' 03"	2296	102.0		0.01								0-37.0		37.0-102.0	○	1964
14	1		96° 11' 54"	46° 20' 39"	2315	63.8		0.22	+1.5	39.3	0			30.5		0-30.5		30.5-63.8		1964
15	2		96° 12' 58"	46° 21' 05"	2247	128.2		1.53	60.0	1.13	117	(174) *		57.5	(3.0) *	0-46.0		46.0-128.2		1964
16	3881		96° 13' 50"	46° 22' 00"	2190	45.0		0.1	14.0	26.0	0			10		0-10.0		10.0-45.0		
17	4*		96° 13' 56"	46° 21' 29"	2200	97.4		1.24	43.60	9.80	11	(21) *		28.8	(0.7) *	0-28.8	28.8-57.5	57.5-97.4	○	1964
18	333*		96° 14' 00"	46° 22' 00"	2185	82.0		1.25	50.0							0-7.0	7.0-45.0	45.0-82.0	○	
19	3731		96° 14' 15"	46° 19' 55"	2263	57.0								14.0		0-43.0		43.0-57.0		
20	1618*		96° 14' 30"	46° 21' 15"	2198	90.0		0.60	44.30					49.5		0-49.5	49.5-70.0	70.0-90		1980
21	3*		96° 14' 34"	46° 21' 46"	2178	29.3										0-29.0		29.0-29.3		1964
22	9*		96° 14' 36"	46° 21' 03"	2200	117.1		1.60	44.0	3.35	41	(68) *				0-50.0	50.0-70.0	70.0-117.1	○	1981
23	87		96° 14' 42"	46° 21' 07"	2197	49.1		1.88	21.9	9.5	17	(31) *				0-33.8		33.8-49.1		
24	88		96° 14' 46"	46° 21' 04"	2198	57.7		0.35	23.9	0.1	233	(324) *				0-8.0	8.0-29.6	29.6-57.7		
25	89		96° 14' 50"	46° 21' 15"	2195	41.0		0.40	19.9	0.2	150	(218) *				0-4.0	4.0-37.1	37.1-41.0		1981
26	3875		96° 14' 52"	46° 22' 25"	2168	70.0		0.20	17.0	53.0	0			20.0		0-20.0		20.0-70.0		
27	85a		96° 14' 58"	46° 20' 55"	2204	69.8			40.6							0-47.5	47.5-55.5	55.5-69.8		

Note: Code No. with and without * correspond to well to use groundwater and well for hydrogeological study, respectively (-)*: Estimated value

Table 2.6.1 Well Inventory (2/4)

Map No.	Code No.	Area	Coordination		Elevation (m)	Depth (m)	Screen (m)	Pumping Rate (Q)(liter/sec)	S.W.L.(GL-m)	Drawdown(m)	Specific Capacity (m ³ /day/m)	Transmissivity (m ² /day)	Thickness of permeable formation (m)	Permeability coefficient (m/day)	Stratigraphy (m)			Geologic Column	Construction Year
			Coordinate X	Coordinate Y											Quaternary Deposit	Neogene Deposits	Consolidated Rocks		
28	1628*		96° 15' 00"	46° 22' 00"	2168	24.0		1.00	8.60	3.00	29	(49) *			0-3.0		3.0-24.0	○	
29	1078*		96° 15' 00"	46° 20' 50"	2206	12.0		2.00	4.00	1.00	173	(248) *	12.0	(20.6) *	0-4.0		4.0-12.0	○	1974
30	3018*		96° 15' 08"	46° 21' 45"	2170	70.0	25.0-40.0	1.17	0.04	26.52	4	7	46.6	0.14	0-23.4		23.4-70.0		
31	1630		96° 15' 08"	46° 21' 45"	2170	65.0									0-25.0		25.0-65.0		
32	3001*		96° 15' 10"	46° 22' 40"	2163	50.0		3.60	2.00	2.75	113	(169) *			0-2.0		2.0-50.0	○	1990
33	2824*		96° 15' 10"	46° 22' 10"	2166	50.0		8.00	9.00	6.20	111	(167) *			0-11.0		11.0-50.0	○	1964
34	1650*		96° 15' 10"	46° 22' 10"	2166	83.0	10.7-20.0	1.50	10.65	24.35	5	(11) *	(12.5?)		0-4.0(12.5?)	4.0(12.5?)	4.0(12.5?)	○	1970
35	1656*		96° 15' 15"	46° 22' 30"	2165	42.0		8.00	6.80	4.30	161	(232) *	>42.0		0-22.0		22.0-42.0	○	
36	136*		96° 15' 19"	46° 22' 01"	2165	90.0	75-80,82-87	7.10	9.00	20.00	31	(52) *							
37	136a*		96° 15' 21"	46° 21' 57"	2165	90.0	49.3-59.5	3.60	9.25	12.75	24	(43) *							
38	2812*		96° 15' 30"	46° 22' 20"	2164	45.0		0.80	6.00	10.00	7	(14) *	23.0	(0.6) *	0-8.0		8.0-45.0	○	1983
39	1676*		96° 15' 30"	46° 22' 10"	2164	50.0		1.00	3.00	23.50	4	(8) *	7.0	(1.1) *	0-3.0		3.0-50.0	○	
40	1677*		96° 15' 45"	46° 23' 00"	2160	38.8		1.00	3.0	22.0	4	(8) *	9.0	(0.9) *	0-9.0		9.0-38.8	○	
41	1677*		96° 15' 45"	46° 23' 00"	2160	53.0		0.75	3.00	14.00	5	(10) *	19.0	(0.5) *	0-9.0		9.0-53.0	○	1990
42	94*		96° 15' 55"	46° 22' 30"	2155	60.0		1.50	4.00	1.00	130	(191) *			0-8.0		8.0-60.0	○	1990
43	1675*		96° 16' 00"	46° 22' 30"	2152	62.0		2.60	3.00	13.00	17	(31) *			0-4.5	4.5-7.5	7.5-62.0	○	1975
44	38*		96° 15' 40"	46° 22' 30"	2161	20.0		1.40	4.00	10.00	12	(23) *	>20.0		0-2.0		2.0-20.0	○	1975
45	8*		96° 15' 30"	46° 20' 40"	2197	64.0									0-18.0	18.0-60.0	60.0-64.0	○	1964
46	93a		96° 15' 33"	46° 20' 57"	2190	39.1		0.17	26.2	0.2	73	(115) *			0-30.7	30.7-36.8	36.8-39.1	○	1990
47	5*		96° 15' 35"	46° 21' 25"	2178	60.0		2.30	6.85	13.90	14	(26) *	13.0	(2.0) *	0-13.0		13.0-60.0	○	
48	51		96° 15' 35"	46° 21' 09"	2185	55.8		1.42	26.28	16.79	7	13	29.5	0.43	0-30.3	30.3-38.5	38.5-55.8	○	
49	688*		96° 16' 00"	46° 21' 00"	2181	60.0		5.50	7.95	0.10	4752	(4891) *	13.0	376.2) *	0-7.0	7.0-13.0	13.0-60.0	○	
50	55		96° 16' 00"	46° 20' 28"	2195	70.2		2.80	33.70	9.34	26	42	19.5	2.14	0-23.6	23.6-50.7	50.7-70.2	○	
51	4		96° 16' 28"	46° 21' 11"	2162	70.8		3.20	8.40	17.32	16	23	57.6	0.4	0-13.2		13.2-70.8	○	
52	1626*		96° 16' 30"	46° 20' 30"	2185	25.0			3.50	1.70			12.0		0-12.0		12.0-25.0	○	
53	6*		96° 16' 39"	46° 21' 25"	2152	75.8		4.20	7.40	8.45	43	(71) *			0-5.0		5.0-75.8	○	1964
54	3		96° 16' 44"	46° 21' 05"	2155	70.0		1.17	7.54	26.98	4	(8) *	23.4	(0.3) *	0-23.4		23.4-70.0	○	

Note: Code No. with and without * correspond to well to use groundwater and well for hydrogeological study, respective (-): Estimated value

Table 2.6.1 Well Inventory (3/4)

Map No.	Code No.	Area	Coordination		Elevation (m)	Depth (m)	Screen (m)	Pumping Rate (Q)(liter/sec)	S.W.L (GL-m)	Drawdown(m)	Specific Capacity (m ³ /day/m)	Transmissivity (m ² /day)	Thickness of permeable formation (m)	Permeability coefficient (m/day)	Stratigraphy (m)			Geologic Column	Construction Year
			Coordinate X	Coordinate Y											Quaternary Deposit	Neogene Deposits	Consolidated Rocks		
55	103		96° 16' 55"	46° 20' 13"	2190	121.0									0-17.8	17.8-80.8	80.8-121.0		1980
56	16		96° 17' 15"	46° 20' 07"	2185	41.8									0-21.0	21.0-30.6	30.6-41.8		
57	20		96° 17' 20"	46° 20' 30"	2164	76.0									0-34.0	34.0-47.0	47.0-76.0	○	
58	12		96° 17' 22"	46° 21' 14"	2142	41.0									0-5.0		5.0-41.0	○	
59	7*		96° 17' 25"	46° 20' 47"	2150	74.0									0-15.0		15.0-74.0	○	1975
60	21		96° 17' 26"	46° 20' 42"	2153	48.0									0-36.0		36.0-48.0	○	
61	4924		96° 17' 28"	46° 20' 55"	2148	62.0									0-14.0		14.0-62.0	○	
62	100		96° 17' 30"	46° 20' 50"	2149	73.6									0-14.0		14.0-73.6	○	
63	90		96° 17' 30"	46° 20' 03"	2186	74.0									0-32.1	32.1-47.6	47.6-74.0	○	1975
64	694*		96° 17' 38"	46° 21' 10"	2142	46.0									0-15.0		15.0-46.0	○	1974
65	8761		96° 17' 44"	46° 20' 39"	2151	79.8									0-14.1		14.1-79.8	○	1980
66	45		96° 17' 50"	46° 20' 46"	2148	76.4									0-22.1		22.1-76.4	○	1975
67	23		96° 17' 51"	46° 20' 27"	2161	75.8									0-9.0	9.0-45.4	45.4-75.75	○	
68	31		96° 17' 52"	46° 21' 20"	2137	57.7									0-8.2		8.2-57.7	○	
69	35		96° 17' 55"	46° 22' 05"	2110	59.0									0-4.4		4.4-59.0	○	
70	54		96° 18' 57"	46° 21' 28"	2125	33.0									0-3.5		3.5-33.0	○	1964
71	41		96° 18' 51"	46° 21' 02"	2142	79.8									0-8.0		8.0-79.8	○	1980
72	40a		96° 19' 20"	46° 20' 45"	2149	40.6									0-22.6		22.6-40.6	○	1981
73	42		96° 20' 06"	46° 20' 20"	2162	94.7									0-25.9	25.9-94.7		○	1990
74	39a		96° 18' 45"	46° 20' 19"	2157	73.0									0-28.0		28.0-73.0	○	
75	123		96° 18' 39"	46° 20' 15"	2158	90.0									0-43.0	43.0-66.0	66.0-90.0		
76	44b		96° 19' 49"	46° 19' 46"	2158	60.0									0-60.0				
77	117		96° 18' 37"	46° 19' 30"	2201	79.0									0-60.0	60.0-79.0			
78	8818		96° 19' 00"	46° 19' 02"	2235	46.0									0-17.0		17.0-46.0		
79	32		96° 19' 56"	46° 18' 18"	2245	147.0									0-32.9	32.9-147.0			1981
80	1620*		96° 18' 00"	46° 18' 00"	2302	102.0									0-90.0		90.0-102.0	○	
81	33		96° 20' 45"	46° 19' 50"	2164	106.0									0-19.2	19.2-67.2	67.2-106.0	○	1990

Note: Code No. with and without * correspond to well to use groundwater and well for hydrogeological study, respectively (-): Estimated value

Table 2.6.1 Well Inventory (4/4)

Map No.	Code No.	Area	Coordination		Elevation (m)	Depth (m)	Screen (m)	Pumping Rate (l/iter/sec)	S.W.L (GL-m)	Drawdown(m)	Specific Capacity (m ³ /day/m)	Transmissivity (m ² /day)	Thickness of permeable formation (m)	Permeability coefficient (m/day)	Stratigraphy (m)			Geologic Column	Construction Year	
			Coordinate X	Coordinate Y											Quaternary Deposit	Neogene Deposits	Consolidated Rocks			
82	115		96° 22' 00"	46° 19' 46"	2155	101.4	16.2-68.2	1.20	5.17	31.51	3	0	3.0	0.1	0-3.0	3.0-63.6	63.6-101.4			
83	34		96° 21' 30"	46° 19' 39"	2160	55.9		2.00	11.27	15.83	11	30	14.1	2.16	0-10.7	10.7-41.8	41.8-56.0	○	1980	
84	25		96° 20' 21"	46° 19' 30"	2190	175.0		2.70	28.56	2.57	91	120	20.0	6.01	0-48.4	48.4-130.0	130.0-175.0	○		
85	26		96° 20' 48"	46° 19' 22"	2190	120.0		5.00	27.46	6.06	71	97	33.9	2.85	0-34.8	34.8-120.0		○	1982	
86	111		96° 20' 48"	46° 19' 20"	2185	82.2	15.8-60.7	2.50	15.85	31.65	7	17	60.2	0.3	0-60.7	60.7-82.2				
87	259		96° 20' 36"	46° 19' 02"	2201	80.0		1.50	45.0	10.0	13	0			0-47.0	47.0-				
88	122		96° 20' 37"	46° 19' 00"	2188	120.5	25.8-63.5	7.10	26.78	6.63	93	80	58.2	1.4	0-58.2	58.2-120.5			1982	
89	27		96° 21' 16"	46° 18' 51"	2200	150.0		3.10	27.91	4.67	57	75	36.9	2.03	0-42.6	42.6-150.0		○	1964	
90	121		96° 22' 08"	46° 18' 47"	2188	100.0	22.4-55.0	1.53	27.00	13.66	10	4	7.0	0.6	0-7.0	7.0-83.5	83.5-100.0		1982	
91	105		96° 20' 17"	46° 18' 45"	2215	100.0	29.4-77.2	1.58	40.85	19.20	7	31	70.5	0.4	0-70.5	70.5-100.0				
92	29		96° 20' 16"	46° 18' 41"	2217	64.4	36.0-48.0								0-64.4				1982	
93	46		96° 22' 01"	46° 18' 30"	2201	54.0	23.0-35.0	2.1	16.1	0.40	454	(590) *			0-30.0	30.0-54.0				
94	28		96° 20' 46"	46° 18' 26"	2227	140.0														
95	112		96° 21' 05"	46° 18' 22"	2225	100.0														1982
96	72		96° 20' 10"	46° 18' 20"	2240	165.5		0.05	34.6	2.9	1	(3) *					54.8-100.0			
97	47		96° 23' 09"	46° 17' 53"	2220	145.0		3.17	16.84	14.32	19	288	17.7	16.3	0-30.0	30.0-54.8	50.0-165.5	○	1990	
98	50p		96° 24' 03"	46° 17' 25"	2225	150.0		2.50	11.52	15.33	14	87	31.4	2.77	0-17.7	17.7-134.8	134.8-145.0		1975	
99	118		96° 23' 26"	46° 17' 13"	2250	80.0	21.0-30.0	1.22	20.80	14.50	7	52	25.6	2.04	0-31.4	31.4-150.0				
100	A1		96° 14' 50"	46° 22' 19"	2165	200.3	14-194*	3.33	11.12	73.47	4	1	(190) **	0.01	0-4		4-200.3		1998	
101	A2		96° 18' 19"	46° 24' 19"	2060	193.0	91-187*	0.99	2.60	5.0	17	10	(60) **	0.17	0-5.5		5.5-193		1998	
102	A3		96° 11' 39"	46° 24' 29"	2150	150.3	12-144*	9.90	3.90	60.13	14	7	(140) **	0.05	0-4		4-150.3		1998	
103	A4		96° 16' 42"	46° 22' 50"	2120	160.2	16-154*	16.67	4.16	11.49	125	205	(150) **	1.37	0-4		4-160.2		1998	
104	B1		96° 14' 17"	46° 22' 10"	2175	56.2	8-50*	1.23	20.15	12.38	9	3	(34) **	0.09	0-10		10-56.2		1998	
105	B2		96° 18' 12"	46° 25' 36"	2030	73.6	31-61*	0.50	11.67	10.94	4	5	(24) **	0.21	0-8	8-36	36-73.6		1998	
106	B3		96° 18' 26"	46° 24' 55"	2050	131.0	76-118*	0.67	25.22	32.55	2	1	(30) **	0.04	0-45		45-130		1998	
107	B4		96° 19' 38"	46° 26' 04"	2020	41.6	5-41*	1.23	4.2	10.5	10	4	(35) **	0.11	0-4		4-41.6		1998	
108	B5		96° 19' 01"	46° 20' 24"	2157	80.0	26-74*	6.7	3.22	19.23	30	39	(30) **	1.31	0-35	35-80			1998	
109	B6		96° 20' 45"	46° 19' 11"	2190	120.0	24-114*	10.08	24.01	1.04	838	31392	(42) **	747	0-60	60-120			1998	

Note: Code No. with and without * correspond to well to use groundwater and well for hydrogeological study , respectively (-):* : Estimated value (-)**: Length of the open hole part
 *:Top-Bottom;Screen pipes are partially installed.

Table 2.6.2 Hydrogeological Description of Formations in the Study Area

Era / Period	System	Maximum thickness (m)	Explanation	
CENOZOIC QUATERNARY	Upper Quaternary Recent river deposits.	>20	Distributed in river and stream bed. Mainly sandy loam and loam. And sand, clay, gravel. A scattered and limited aquifer in Sukhin Khooloi.	
	QIV			
	Middle and Upper Quaternary Fan and talus deposits	>100	Distributed widely in the area. Gravel, sandy gravel, sand. Most exploitable aquifer occurs in Olon Nuul, where the deposit is thick.	
	QII-III			
TERTIARY	Neogene System	>100	Mainly covered by Quaternary sediments in the area. Unconsolidated generally. Very low productive aquifer in Sukhin Hooloi and Olon Nuul. Reddish clay with sand and gravel.	
	N2at			
PALEOZOIC	Lower and Middle Devonian series	>1000	Distributed in the limited area. Sandstone and conglomerate. No aquifer in Study Area.	
	D1-2			
	CAMBRIAN Lower and Middle	Tsagaan Olom Series	>1000	Distributed in the south end of Study Area. Mainly carbonate rocks.
	E1-2			
PRECAMBRIAN PROTEROZOIC	Vend Series Khantaishir Series		Limited distribution in the mountain range on the south of Study Area.	
	Vht			
	Upper-Lower Riphean Series		Distributed in the southwest mountain range of Altai City. Mainly Dunite and Serpentine, Peridotite. Springs occur in places along faults.	
	R1-3gb			
	Gobi Altai and Ulaantolgoy Series	2950	Distributed widely in the northwest and central mountainous region in Study Area. Fractured aquifers occur in Kharzat and Sukhin Hooloi.	
	PR1 PR1am			
PROTEROZOIC	Intrusive Rock of Riphean Series		Limited distribution in the eastern area.	
	γ R2-3			

Table 2.6.3 The Result of Water Level Observation

WLR No.	Well Name	Depth (MP-m)	M.P. (GL+m)	24-Jun	7-Jul	14-Jul	19-Jul	22-Jul	13-Aug	27-Sep	5-Nov	5-Dec	4-Jan	15-Jan	14-Feb	9-Mar	10-Apr	1-May	19-May	5-Jun	30-Jun	1-Aug	31-Aug. 1-Sep.	26,27-Oct.	
No.1	Intake Well*	70.00	0.09	6.18	6.1	6.05	6.05	6.05	5.93	5.94	5.81	5.99	6.1	6.2	6.28	6.4	6.54	6.38	6.21	6.3	6.35	6.18	6.15	5.97	6.00
No.2	School Well	6.00	-0.37	4.9	4.87	4.78	4.87	4.78	5.05	5.45	5.46	5.48	5.5	5.7	6	5.22	5.14	5.06	5.12		5.21	4.97	5.27	5.50	5.45
No.3	Park Well	31.00	0.65	10.67	10.67			10.7	10.67	10.7	10.7	10.72	10.7	10.7	10.71	9.21	broken	-	-	-	-	-	-	-	-
No.4	Power Station*	30.00	0.55	7.09	7.04		7	6.98	6.93	6.97	7.02	7.05	7.1	7.12	7.15	7.2	7.16	7.16	7.14	7.13	7.05	6.90	6.83	6.86	6.90
No.5	Kharzat Zuun Well*	(50?)	0.00				9.1	8.96	9.05	8.67	8.94	8.96	9.7	9.9	10.1	10.39	10.83	9.62	9.92	10.15	10.57	10.36	10.07	9.59	9.40
No.6	Water Supply*	6.60	0.35	2.7	2.75		2.73	2.69	2.85	3.12	3.21	3.28	3.35	3.35	3.34	3.2	2.95	2.86	2.87	2.98	2.98	2.80	3.03	3.20	3.22
No.7	Brick Well	51.00	0.47	12.25	12.26		12.23	12.24	12.17	12.78	12.23	12.3	12.36	12.4	12.48	11.70△	11.70△	-	-	-	-	-	-	-	-
(No.8)	Gashuny Well	3.35	0.40	2.4	2.68	1.78		2.95	3.08	3.34	3.4	3.4	3.5	3.4	3.4	3.26	3.2	3.15	2.92		2.91	2.95	3.21	3.56	3.78
No.9	Ulaan Sharon	2.20	0.00	1.68	1.57	0.52	1.32	1.3	1.45	1.3	1.37	1.00**	frozen	frozen	frozen	frozen	1.80**	1.90**	0.83		1.40	0.75	1.32	1.44	1.38
(No.10)	Esin Amni	1.80	0.00	0.63	0.81	0.73		0.6	0.83	0.85	0.86	0.73**	frozen	0.73**	frozen	frozen	frozen	0.7	0.7	0.31	0.9	0.72	0.00	0.75	0.85
-	Sukhin Khooloi Ekh	4.20	0.75		3.12			2.94	3.05	3.23	3.3	3.4	3.30**	3.30**	0.65**	frozen	3.26	3.19	3.36		3.30	2.80	2.96	3.24	3.40
-	Onsogyn Well	1.70	0.00	0.78	0.68	0.59		0.5	0.72	1.3	1.4	1.00**	0.65**	0.65**	frozen	0.65**	1.00**	1.40**		1.05	1.30	0.60	0.91	1.05	
-	Mandajin Amni		0.00		2.96			2.93	3.08	3.15	3.16	3.1	frozen	frozen	frozen	3.2	3.2	3.4	3.25	3.35	3.15	3.07	2.83	2.71	
-	Boint Well	4.20	0.80		3.39			3.15	3.23	3.54	3.86	4.2	frozen	frozen	frozen	frozen	4.2	4.2	4.15		4.03	3.40	3.50	3.72	3.89
-	Sukhin Khooloi Well	2.10	0.30	0.89	0.99	1.19	0.72	0.82	1.26	1.4	1.5	1.00**	frozen	frozen	frozen	frozen	1.50**	1.50**	1.3		2.04	0.85	1.42	1.75	1.90
-	Tsagaan Dersny We	4.50	0.70			4.32		4.39	no water	no water	no water	no water	no water	no water	no water	no water	no water	no water	no water						
-	Tsagaan Dersny 2																		1.55		1.40	1.07	1.80	1.95	Dry
-	Mountain Hand Dug	1.16	-1.00			0.75		0.74	0.8	0.84	0.85	frozen	frozen	frozen	frozen	frozen	frozen	no water	-	-	-	-	-	-	-

** Partially froze △stucked

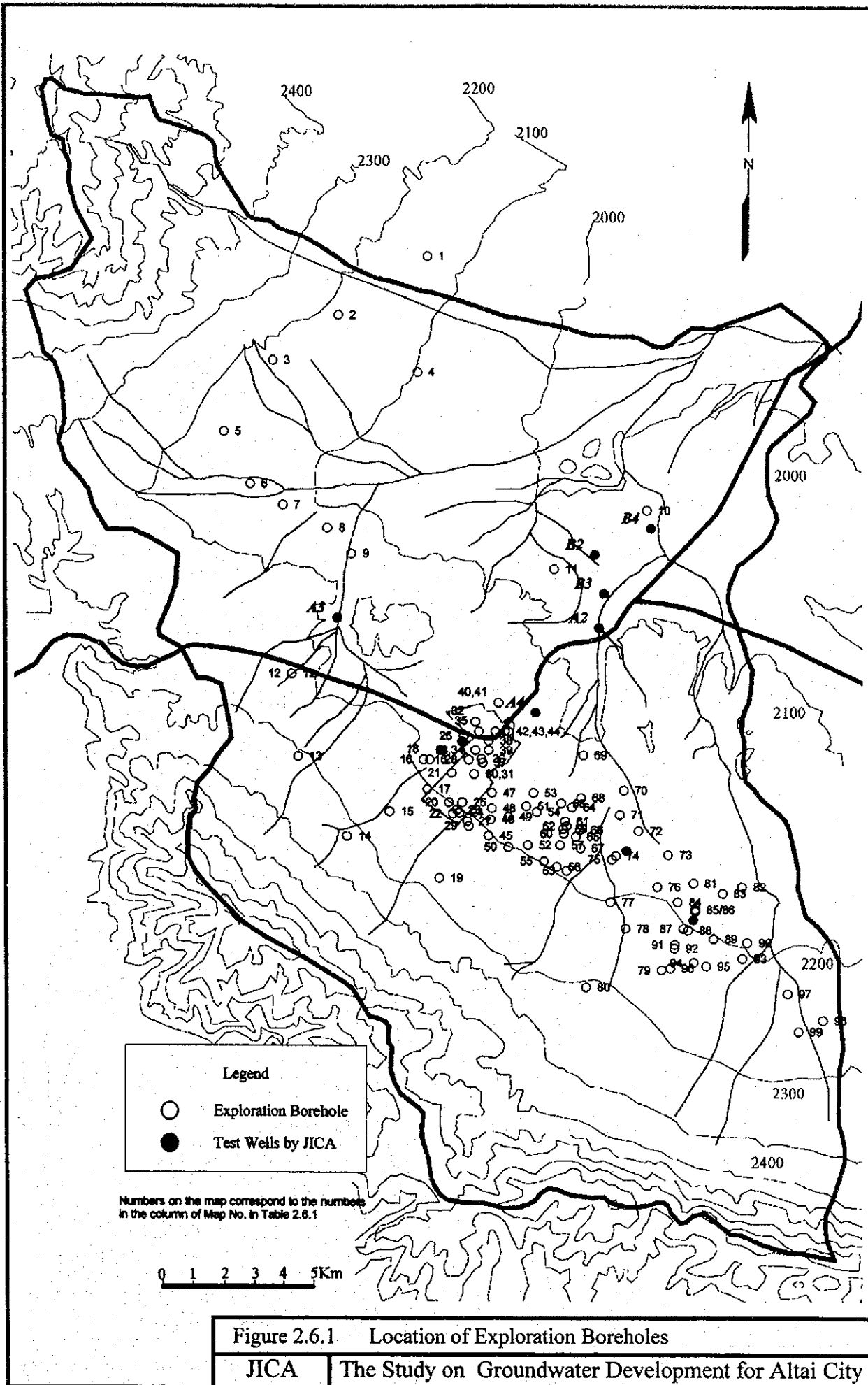


Figure 2.6.1 Location of Exploration Boreholes

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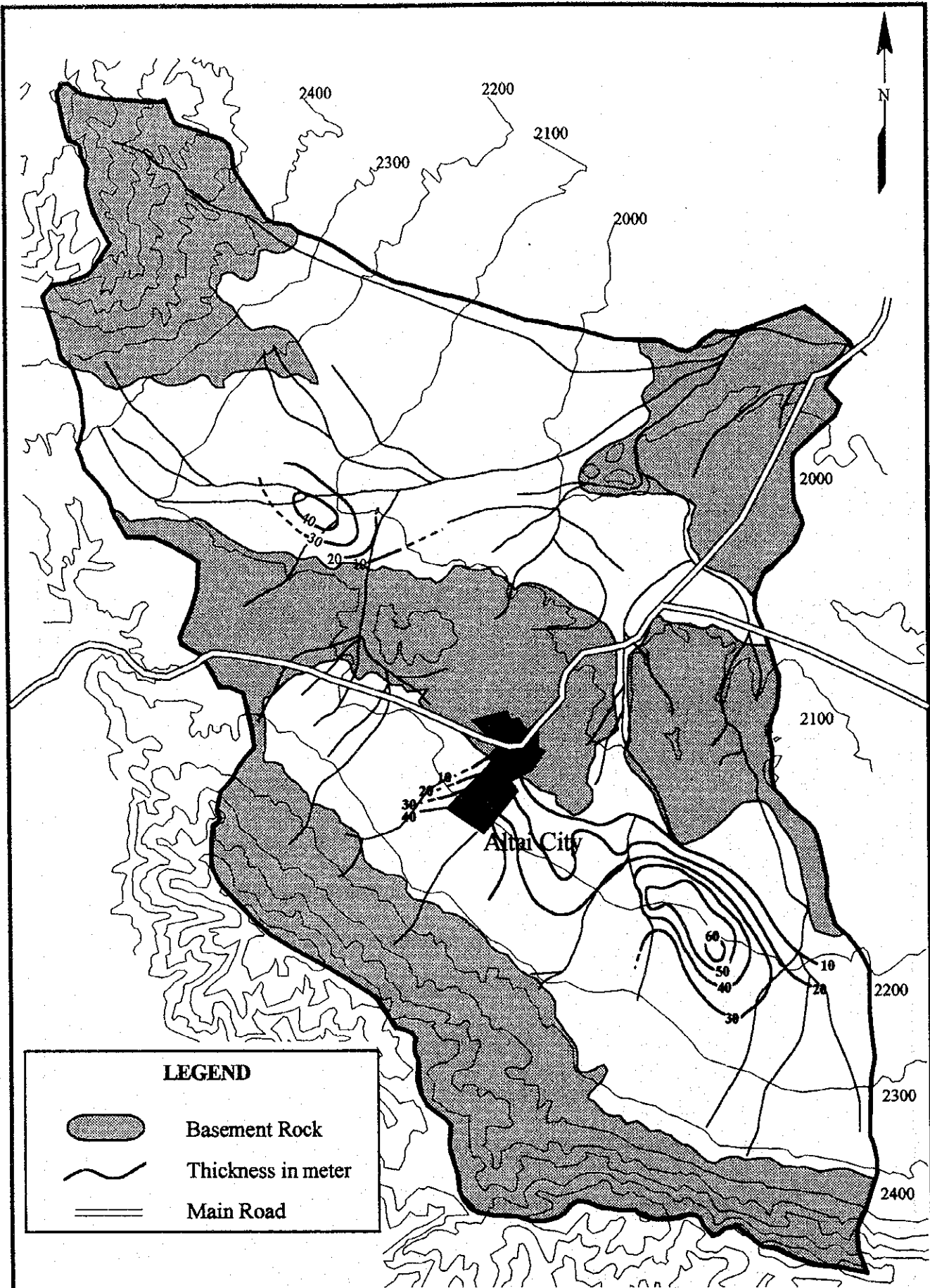


Figure 2.6.2 Isopach Map of Quaternary Deposits

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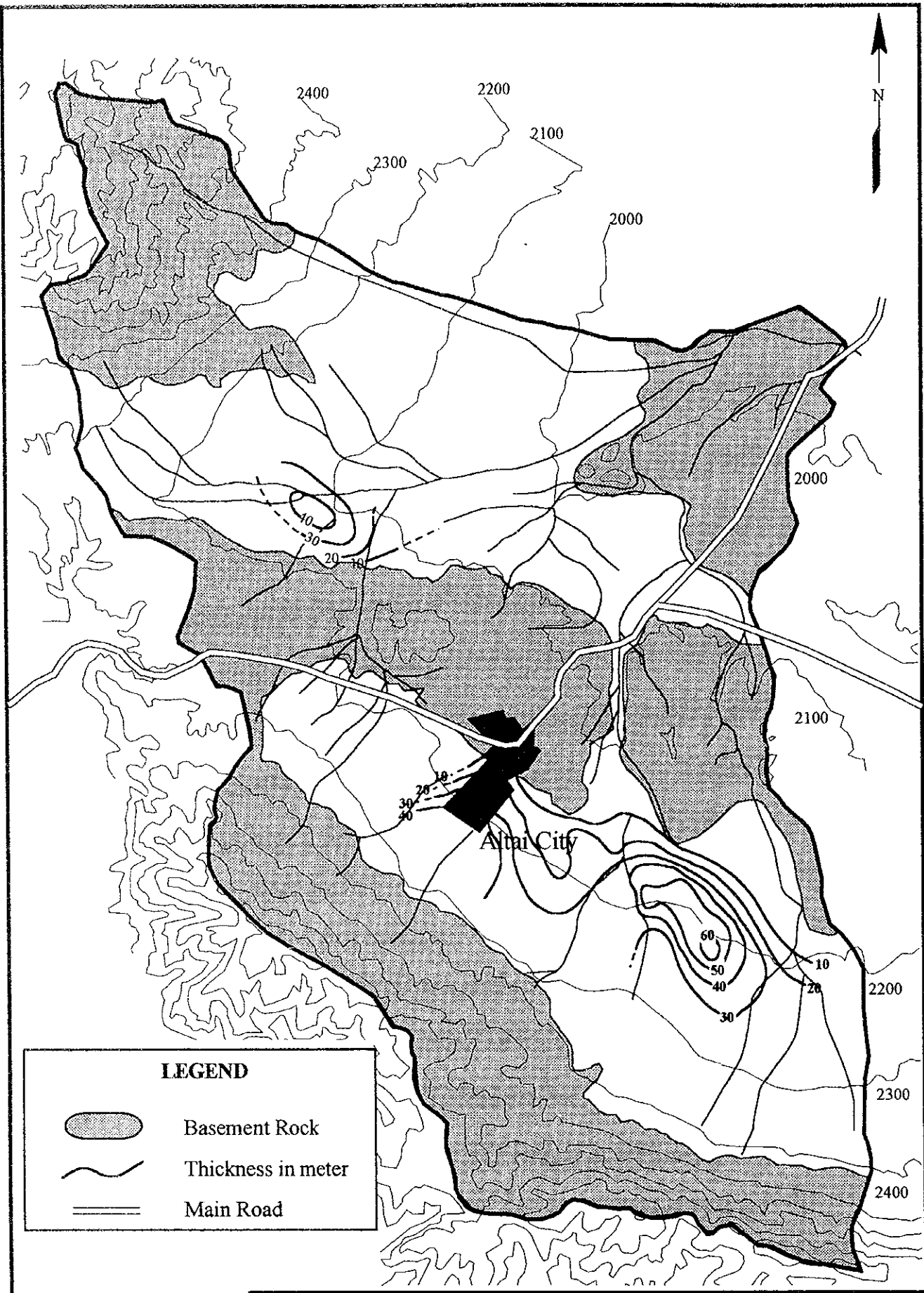


Figure 2.6.2 Isopach Map of Quaternary Deposits

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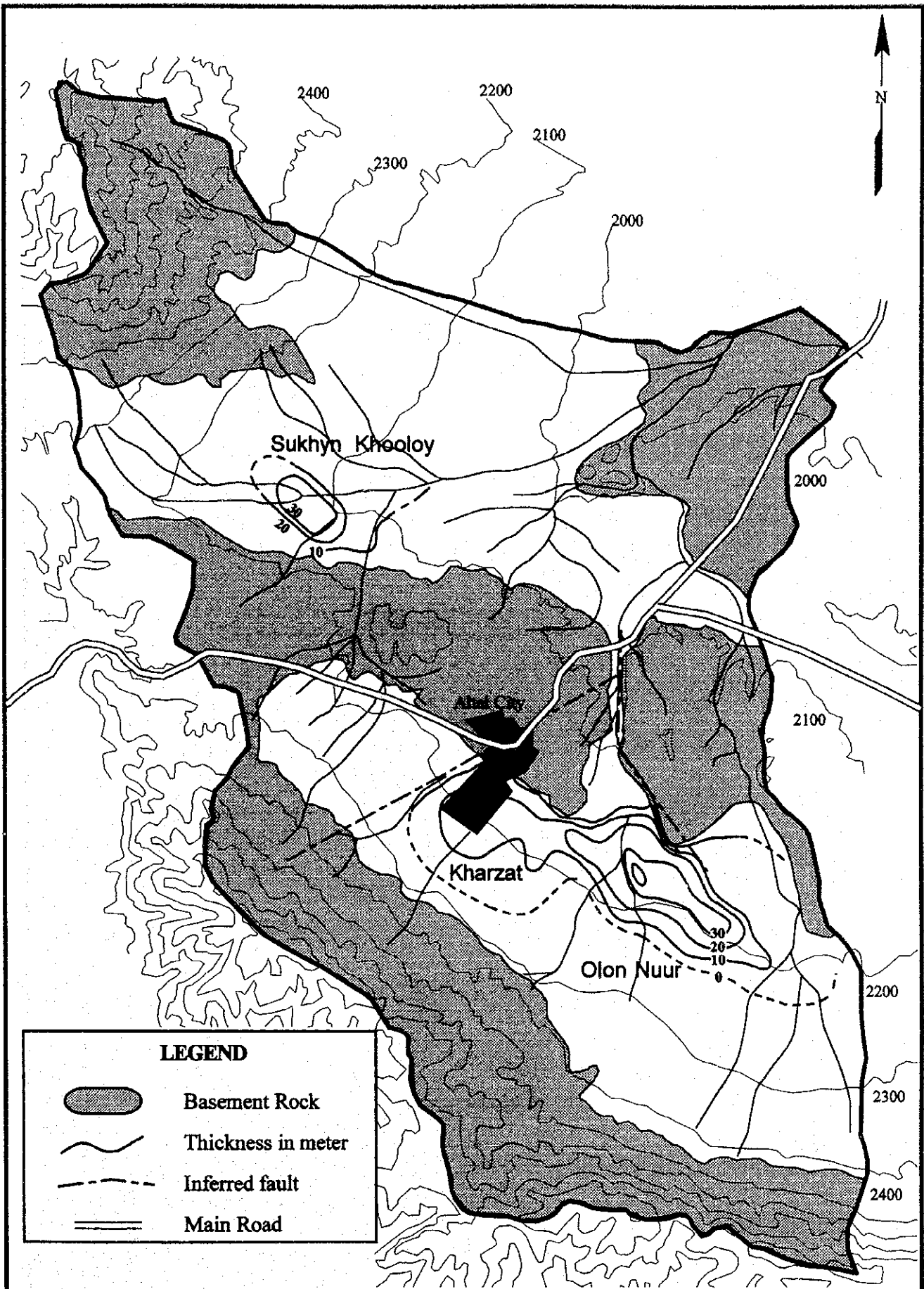


Figure 2.6.3 Isopach Map of the Main Aquifer
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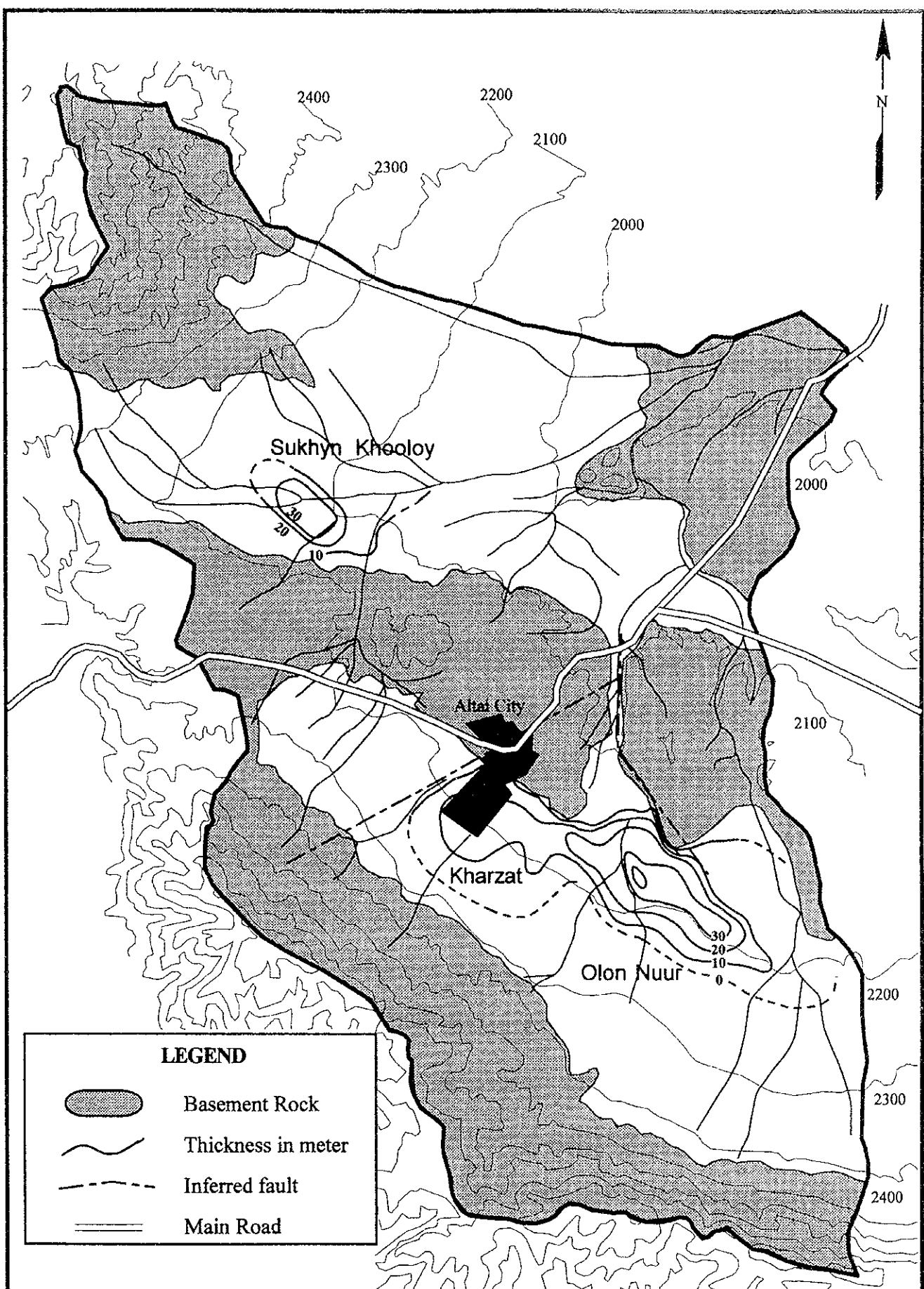
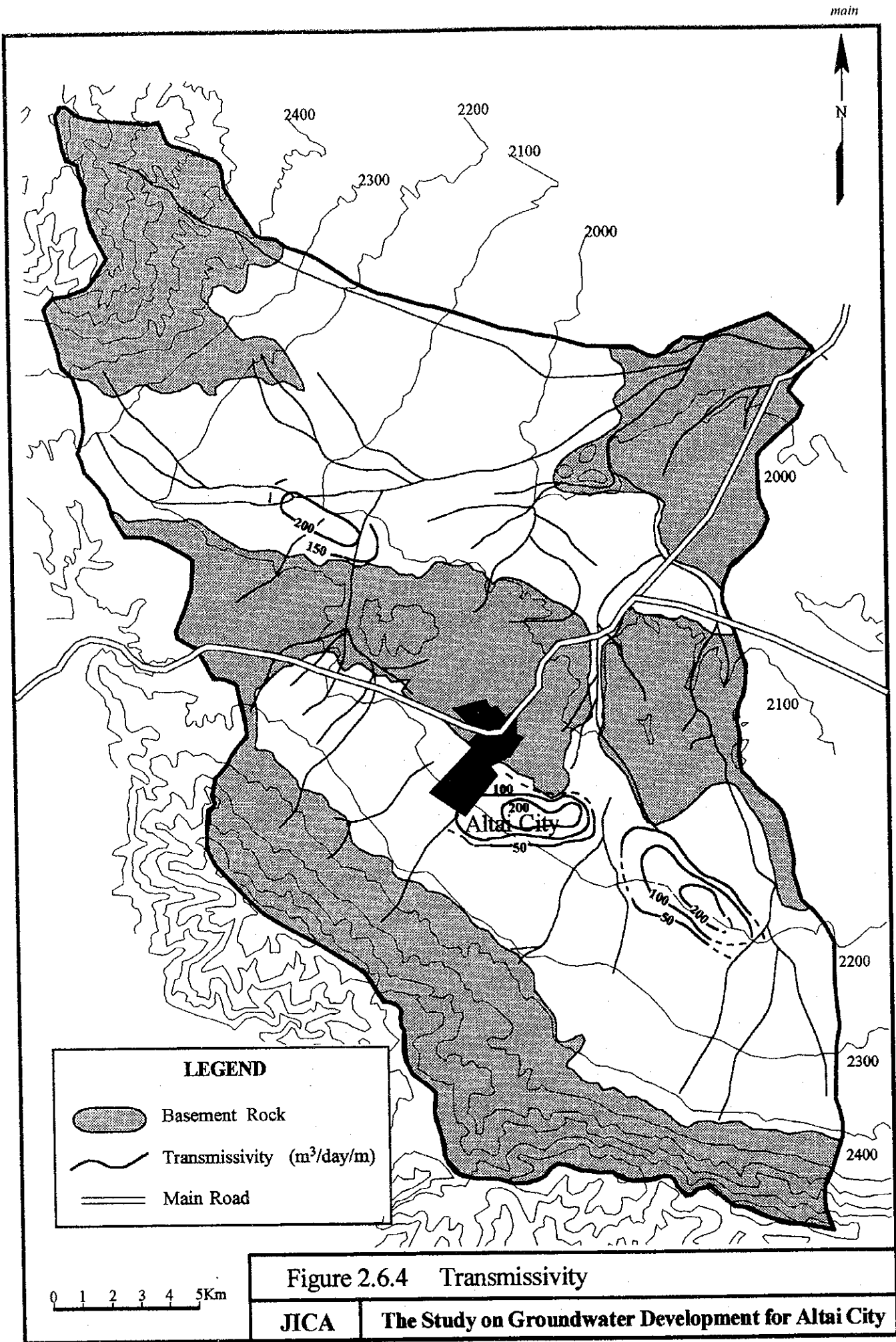


Figure 2.6.3 Isopach Map of the Main Aquifer

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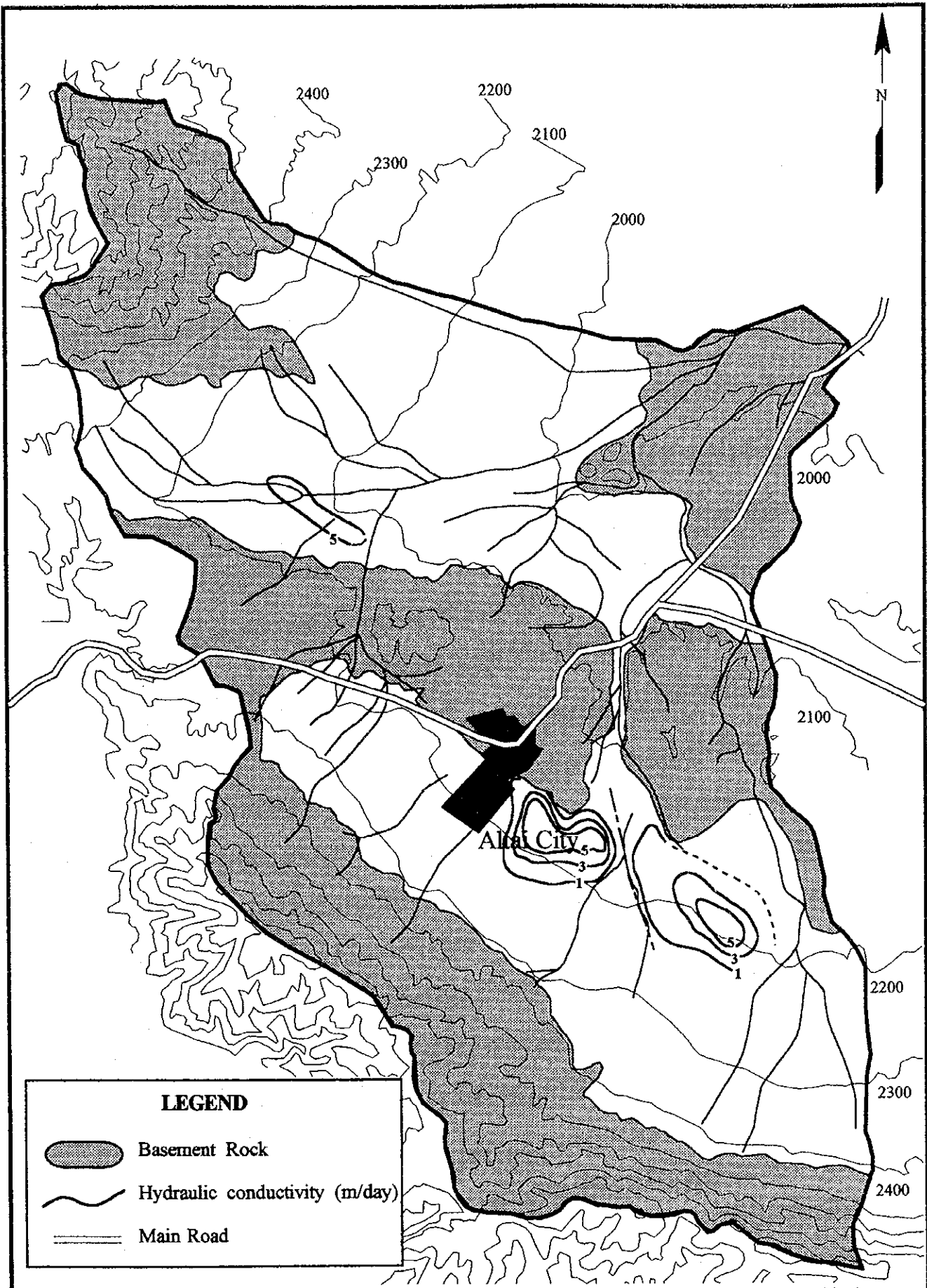


Figure 2.6.5 Hydraulic Conductivity

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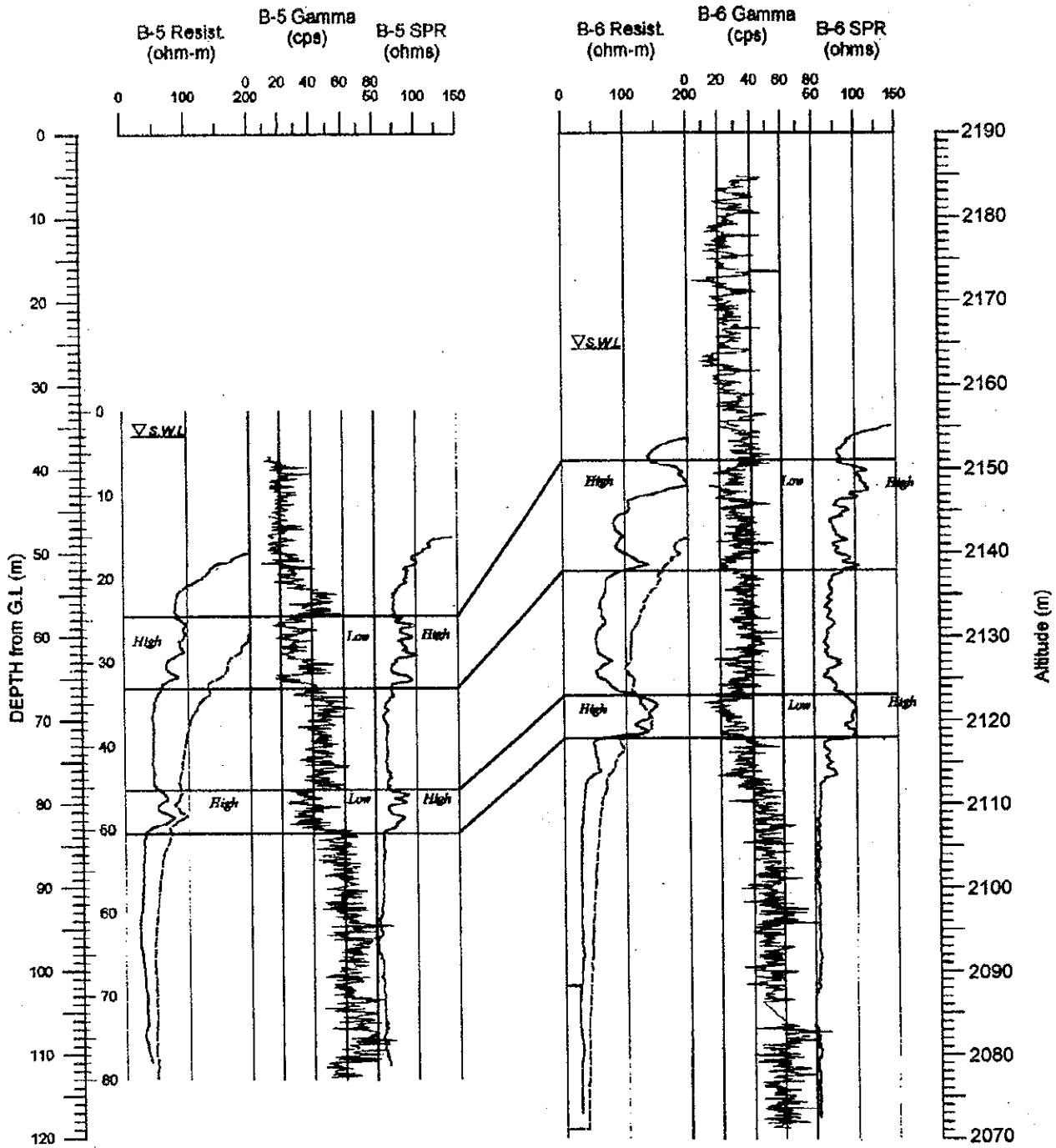
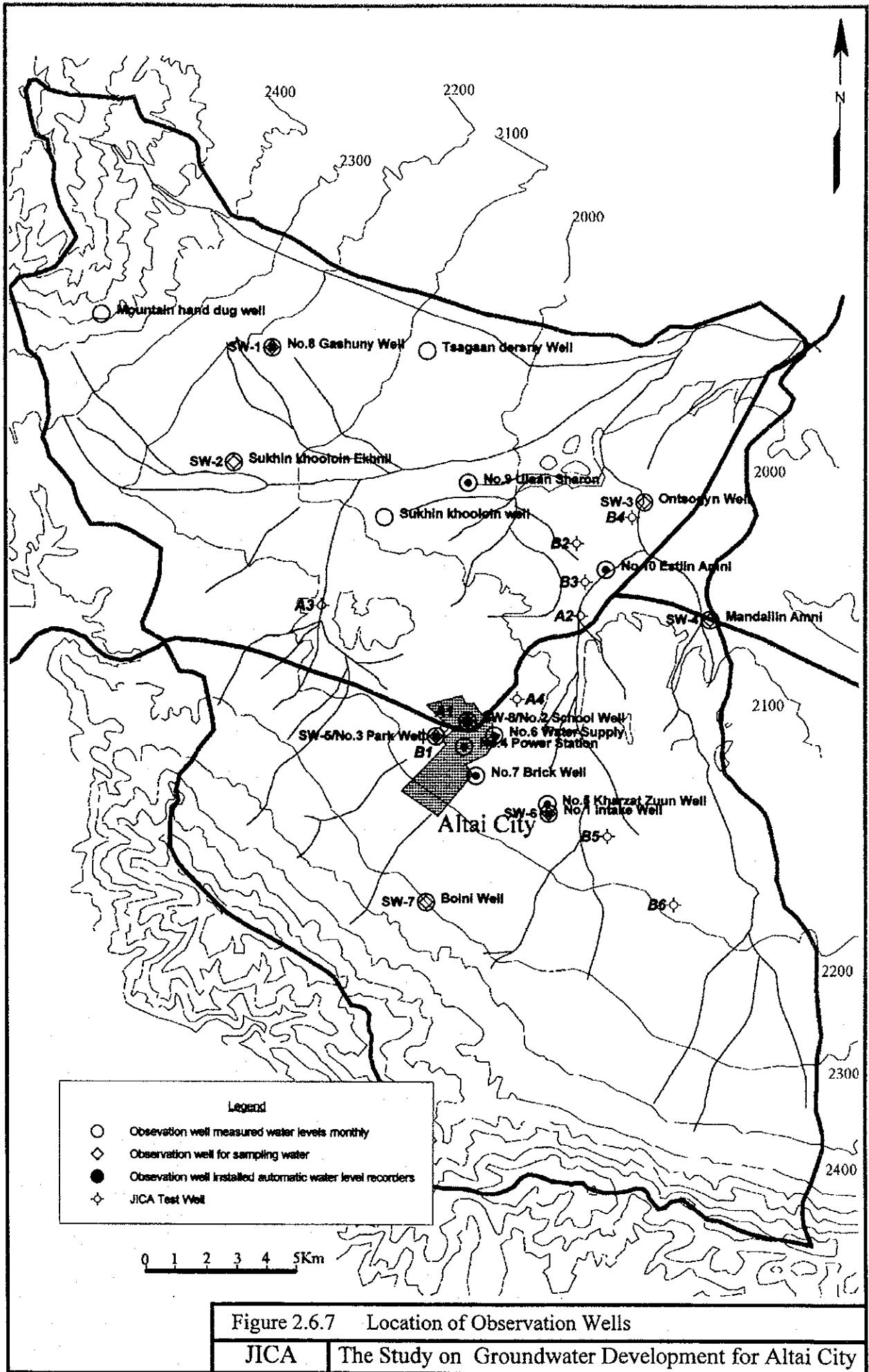
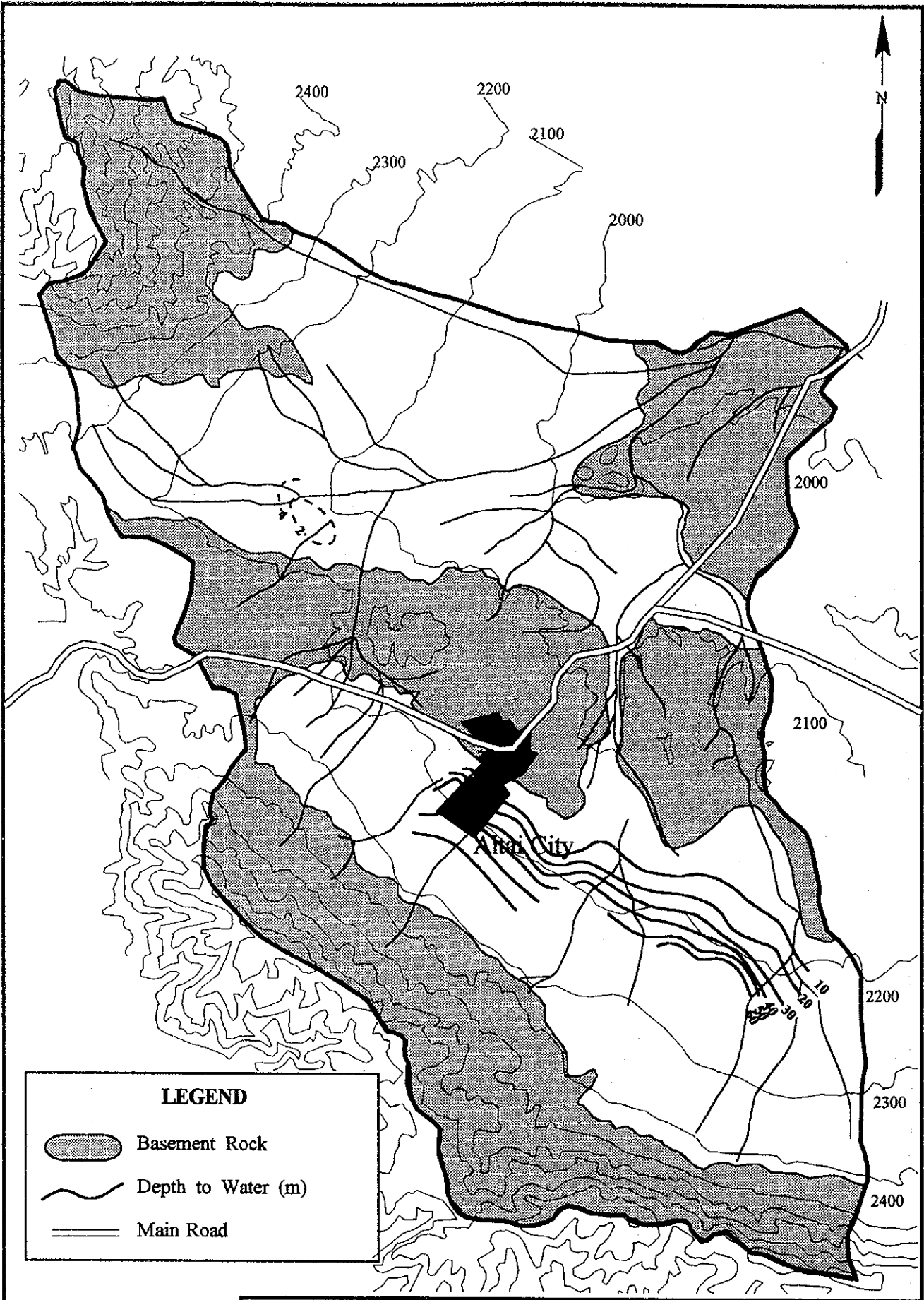


Figure 2.6.6 Geophysical Log of B-5 and B-6

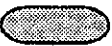

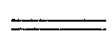
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-  Basement Rock
-  Depth to Water (m)
-  Main Road

0 1 2 3 4 5Km

Figure 2.6.8 Depth to Water

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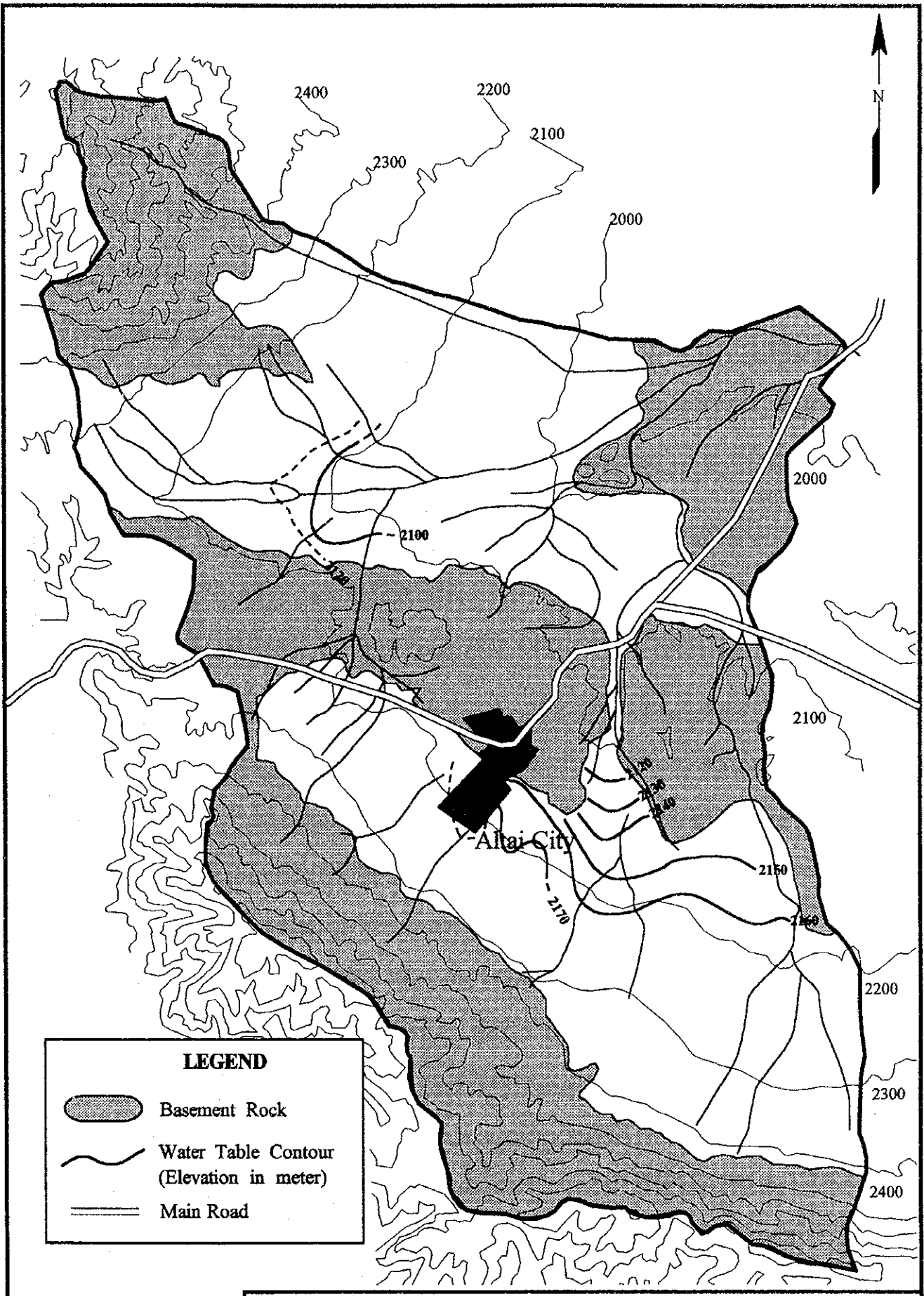


Figure 2.6.9 Water Table Contour

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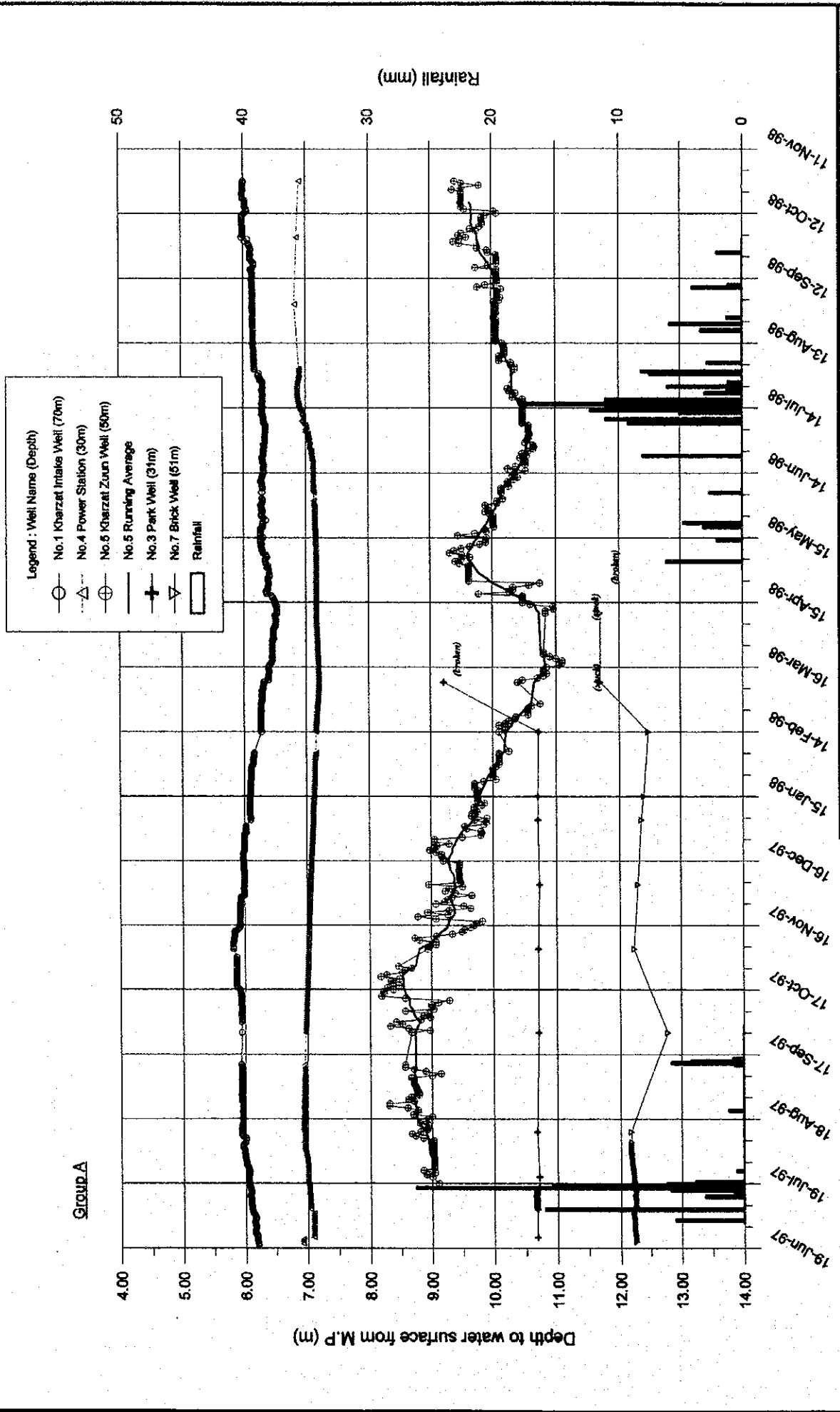


Figure 2.6.10 (1) The Result of the Water Level Measurement of Group A
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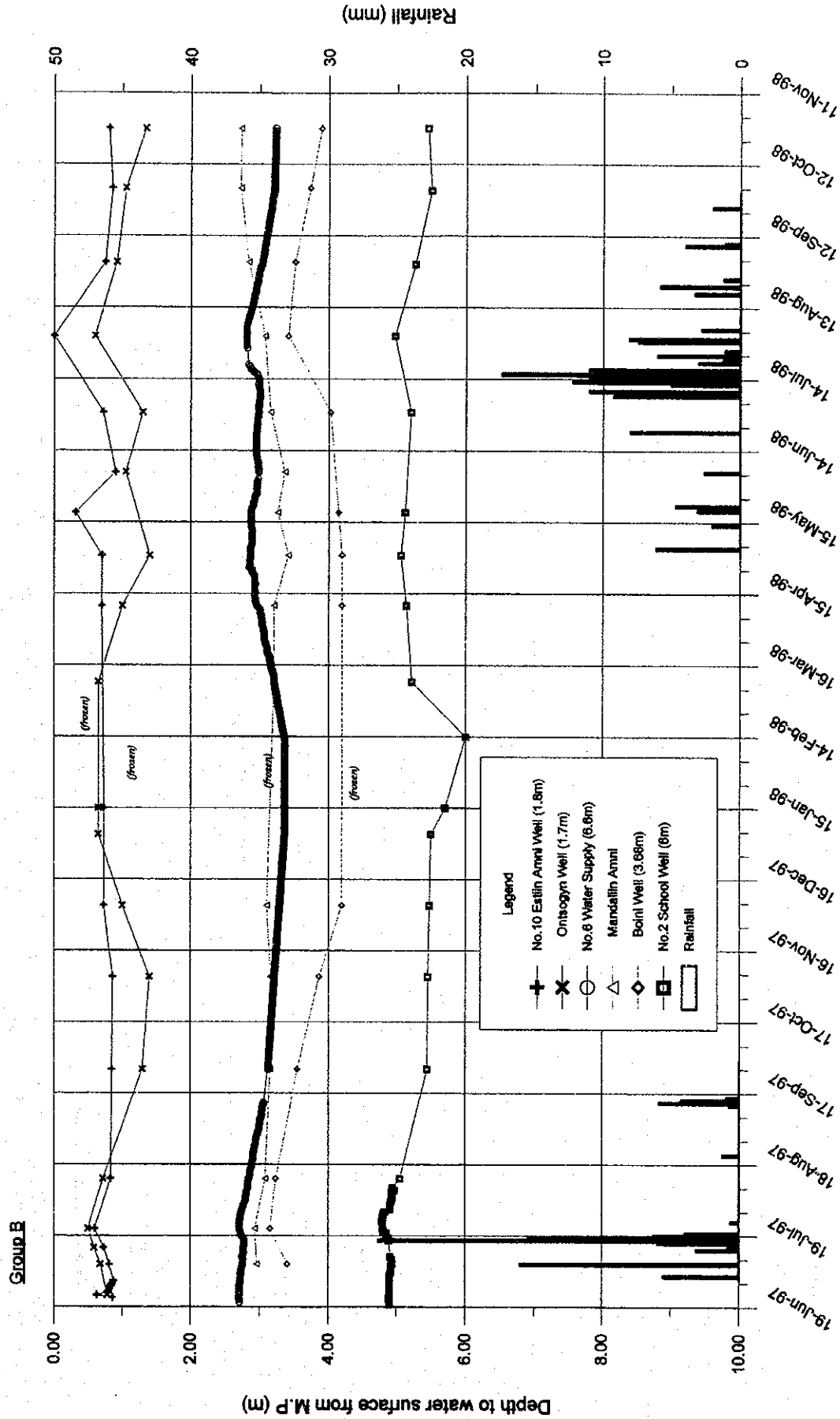


Figure 2.6.10 (2) The Result of the Water Level Measurement of Group B

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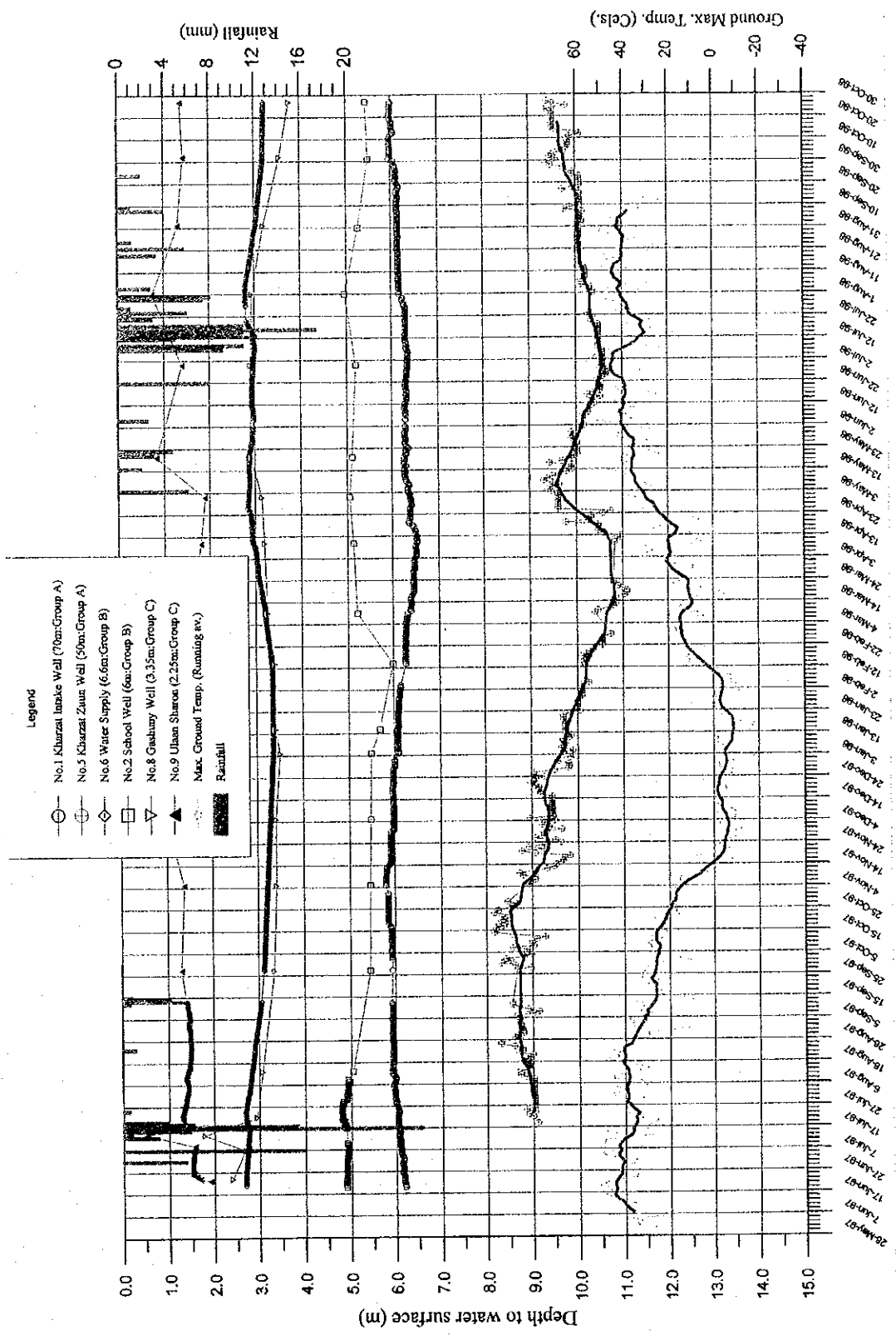


Figure 2.6.10 (4) Water Level Fluctuation in the Study Area

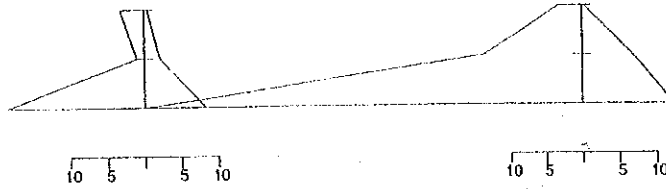
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Water Quality of Test Wells

Well Name : Depth

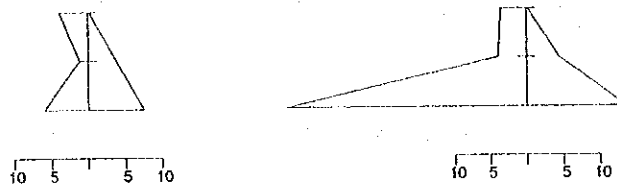
A1 Test Well : 200m 28

A2 Test Well : 193m



A3 Test Well : 150m 46

A4 Test Well : 160m



B1 Test Well : 54m 29

B2 Test Well : 73m



B3 Test Well : 131m 31

B4 Test Well : 41m



B5 Test Well : 80m 27

B6 Test Well : 120m

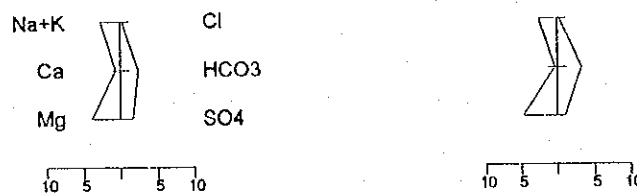


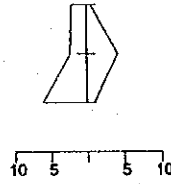
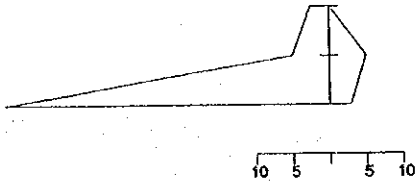
Figure 2.6.11(1) Stiff Diagrams of Test Wells

Water Quality of Selected Wells

Well Code, Name : Depth

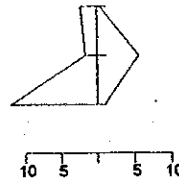
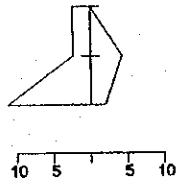
SW1 Gashyuny Well : 3.3m 1

SW2 Sukhiin Khooloi Ekhni:4.2



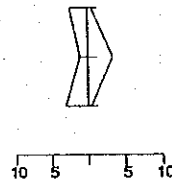
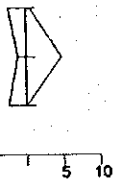
SW3 Ontsogin Well : 1.7m 4

SW8 School Well : 6m 8



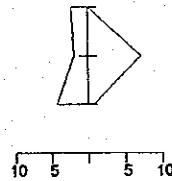
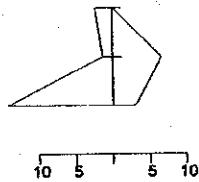
SW5 Park Well : 31m 6

SW6 Kharzat Intake Well : 70m



SW7 Boini Well : 3.6m 7

SW8 School Well : 6m 8



LEGEND

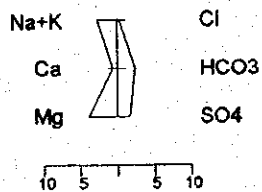


Figure 2.6.11(2) Stiff Diagrams of Selected Wells

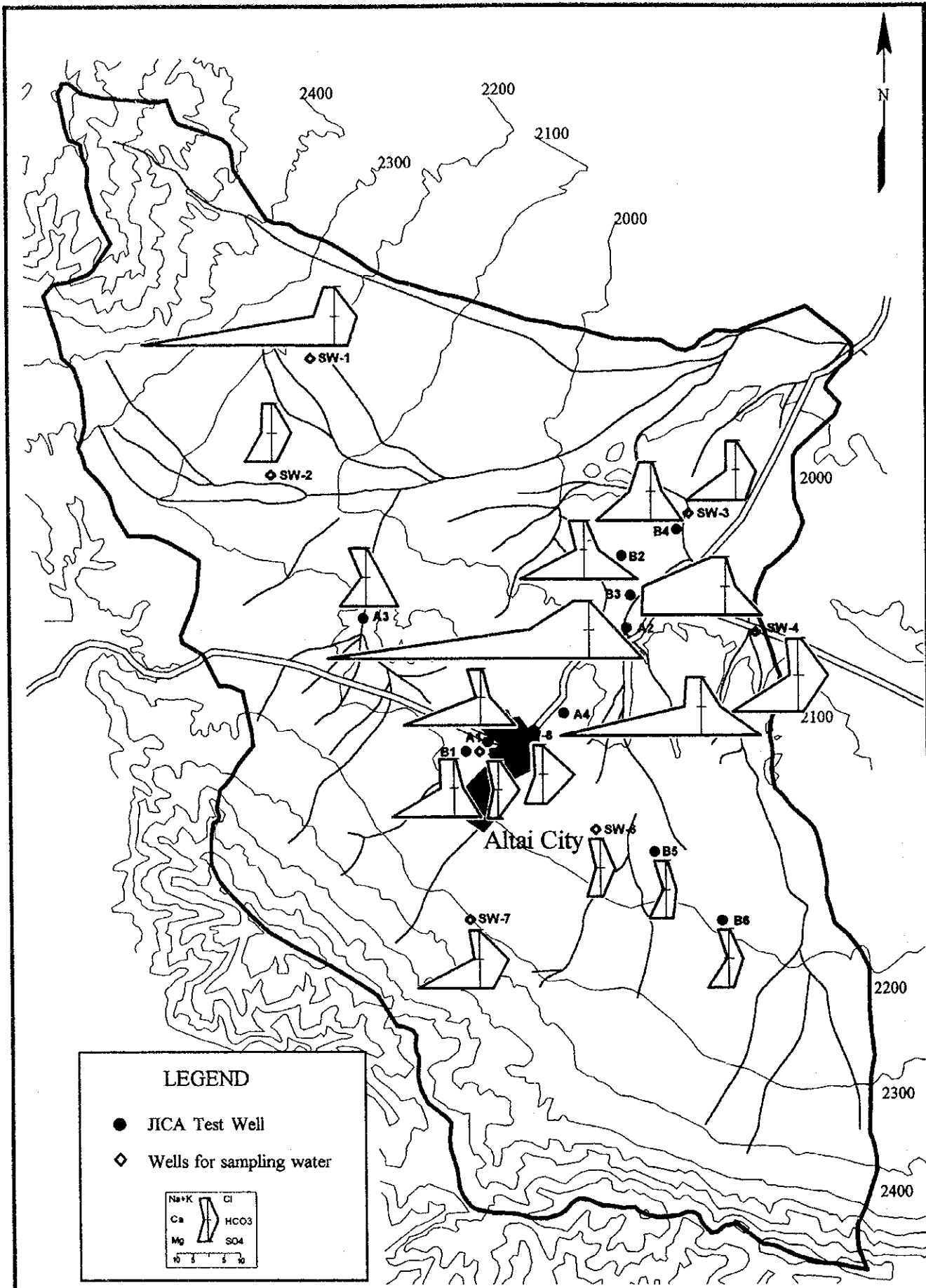


Figure 2.6.12 Water Quality in the Study Area

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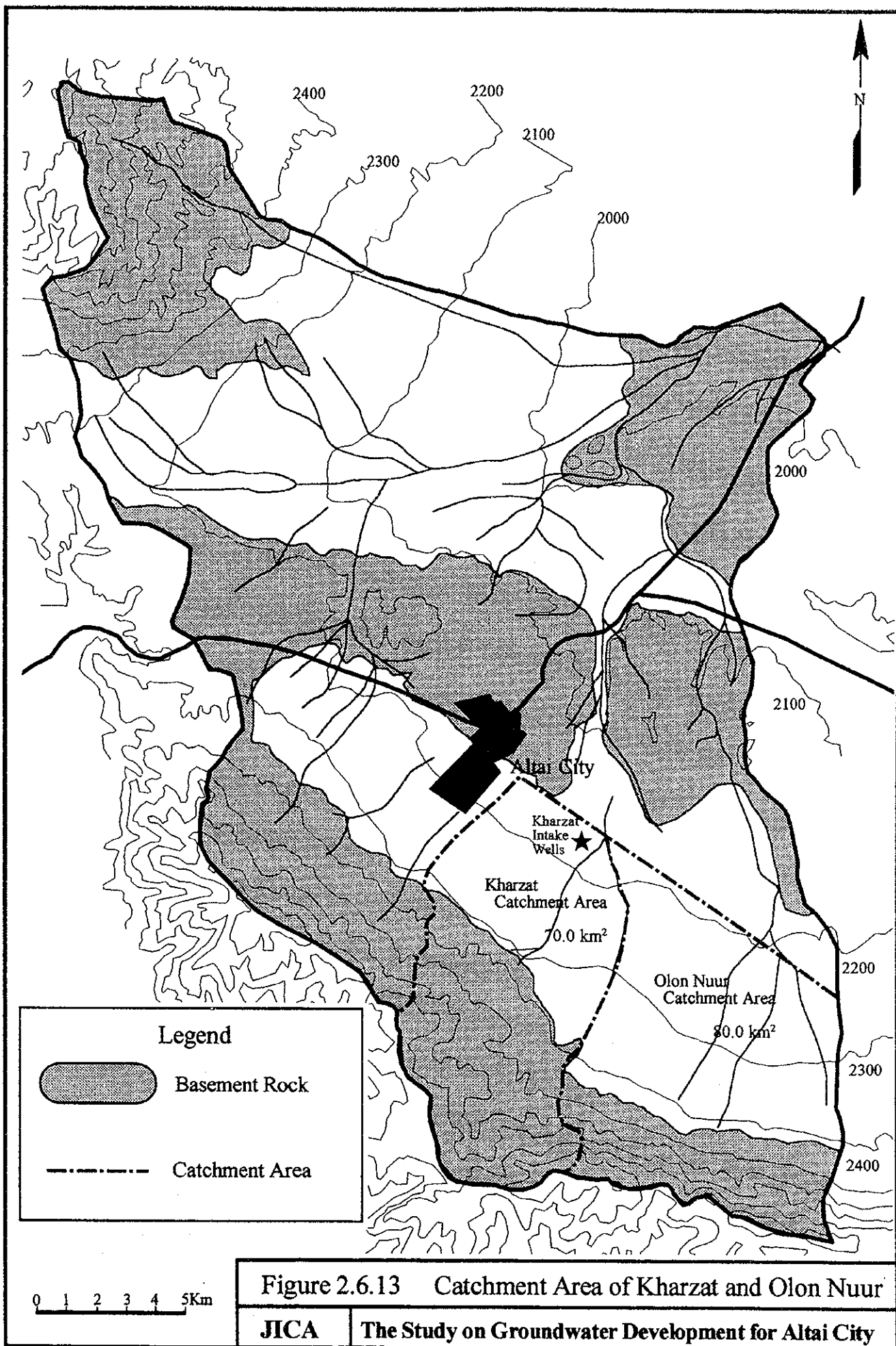


Figure 2.6.13 Catchment Area of Kharzat and Olon Nuur

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