B7 Existing Water Distribution Network Survey

B7.1 Intoroduction

B7.1.1 General

The present water supply for Kisumu Town is operated and maintained by Kisumu Municipal Council. The first system was established in 1920s when Kajule Water Treatment was constructed. Kajulu Treatment Works is situated about 11km north of the Town at the side of the Kibosu river. The Works comprises of a river intake, formed by a concrete weir across the river, sedimentation tanks and 5 rapid gravity pressure filters. Water is fed to the service reservoir at Kibuye under gravity via a 150mm steel / AC main. Rehabilitation works have been carried out since 1920. The second system was established in 1956 when Lake Water Treatment Works was constructed. Lake Water Treatment Works is situated on the edge of Lake Victoria south west of the Town. The Works comprises on intake on the bank of the lake, water is pumped via low lift pumping station to the Treatment Works. Treated water is pumped for storage and distribution.

B7.1.2 Purpose of the Survey

Purpose of the survey is to determine the status of the existing Kisumu Municipality Water Supply System with regard to Water Production, Leakage and Pressure distribution with the System, and to carry out condition survey of existing Distribution Network and Meter Condition Survey.

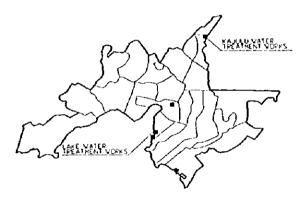
B7.2 Water Production Survey

B7.2.1 Methodology

(1) Location of Treatment Works

The water production survey was carried out at two sites of the water treatment works; the one named Lake Water Treatment Works is located on the edge of Lake Victoria south west of the Town, the other named Kajulu Water Treatment Works is located about 11 km north of the Town at the side of Kibosu River. Fig B-2 shows the locations.

Fig B-2 Location Map

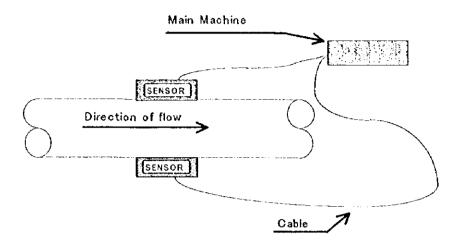


(2) Measuring Method

Water production measurements were carried to determine the bulk supply into the system by using a device named "Krohne Ultrasonic Flow Meter". This meter is "clamp on" one, giving an hourly flow which is automatically printed out and can accurate to $\pm 5\%$ if the internal pipe diameter and material is known accurately and if there is no turbulence in the pipeline.

This meter measures a flow of pipe water like as follows;

Fig B-3 Ultrasonic Flow Meter



Measuring items are as follows;

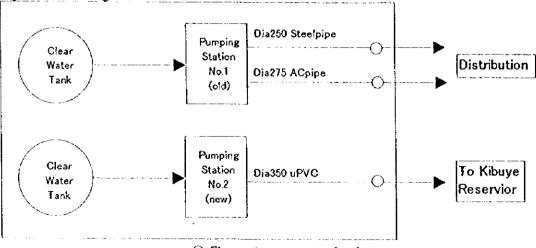
- Flow of rate (m^3/h)
- Duration of measuring : For more than seven days
- Frequency of sampling: 1 hour interval

B7.2.2 Results of Survey

(1) Lake Water Treatment Works

The water is distributed to the system by pumping from two pumping stations which are located within the Treatment Works. Measured points are as follows;

Fig B-4 Measuring Points

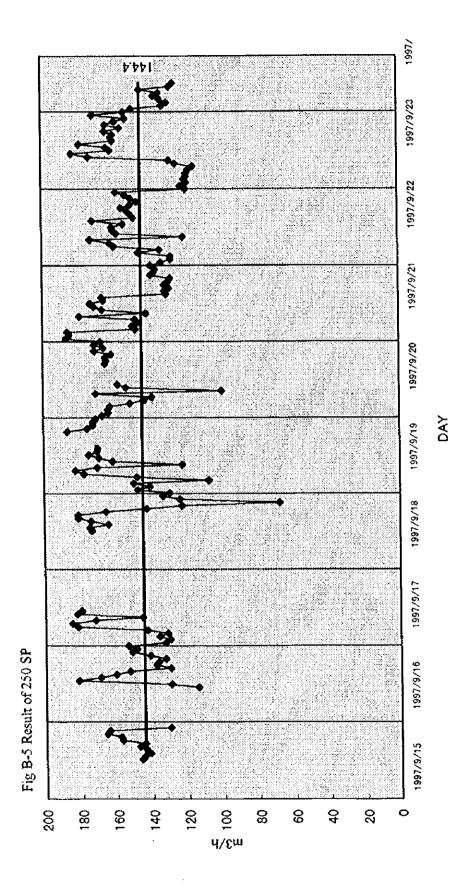


○;Flow rate meassured point

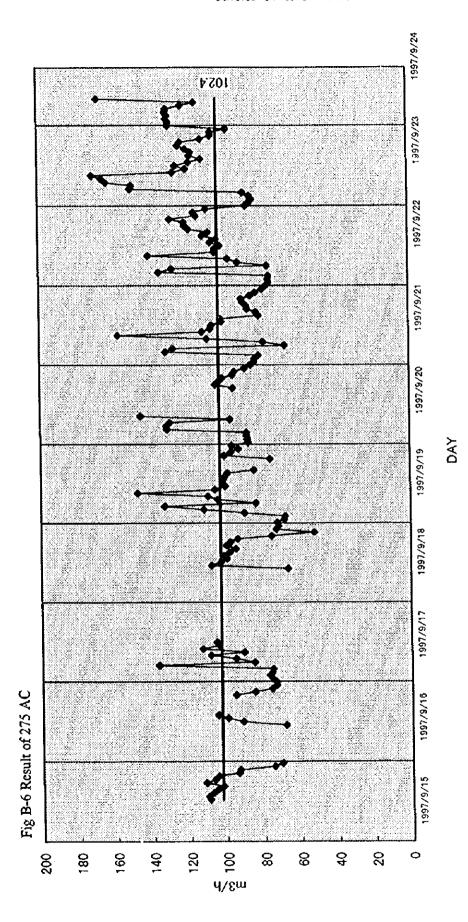
Pumping Station No.1(OLD) has 6 Nos. duty pumps and no stand by pump. Three of them feed water to a 275 mm dia. AC rising main. The others feed water to a 250 mm dia. steel rising main. However, at the time of carrying out the survey one pump on each rising main was out of order for the last one year.

Pumping station No.2(NEW) has 8 Nos. duty pumps and no stand by pump, which feed water to a 350 mm dia. uPVC raising main. However, at the time of carrying out the survey 6 pumps were only operational.

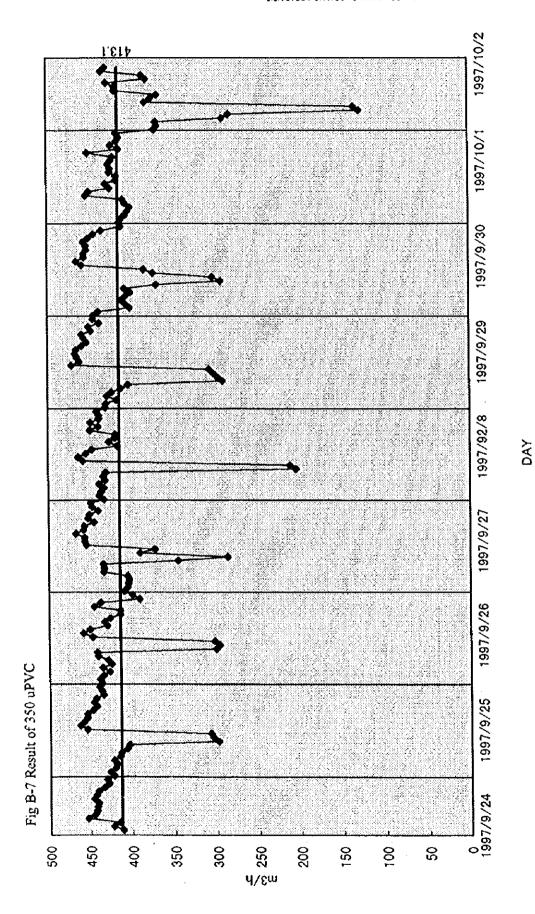
The survey results in each pipe are shown in Fig B-5, Fig B-6 and Fig B-7.



B - 30



B - 31



B - 32

As shown in figures above, duration of missing line means that the rate of flow could not be measured because of lack of power supply. Average flow rate was calculated by way of summation of the flow rate's value divided by total number of the flow rate. Average flow rate is shown in Table B-3.

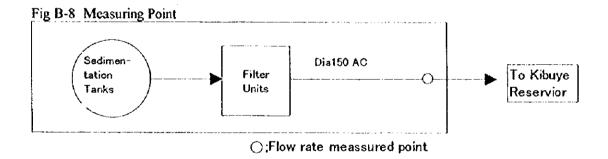
Measured pipe	Average flow rate (m3/h)	Period of measurement
250 SP	144.4	9/15 - 9/24
275 AC	102.4	9/15 - 9/24
350 uPVC	413.1	9/24 - 10/2
TOTAL	659.9	

Table	B-3	Average	flow	rate
10010		11101050	11017	1010

Measured average production per day in Lake WTW is 15,800 m³/day.

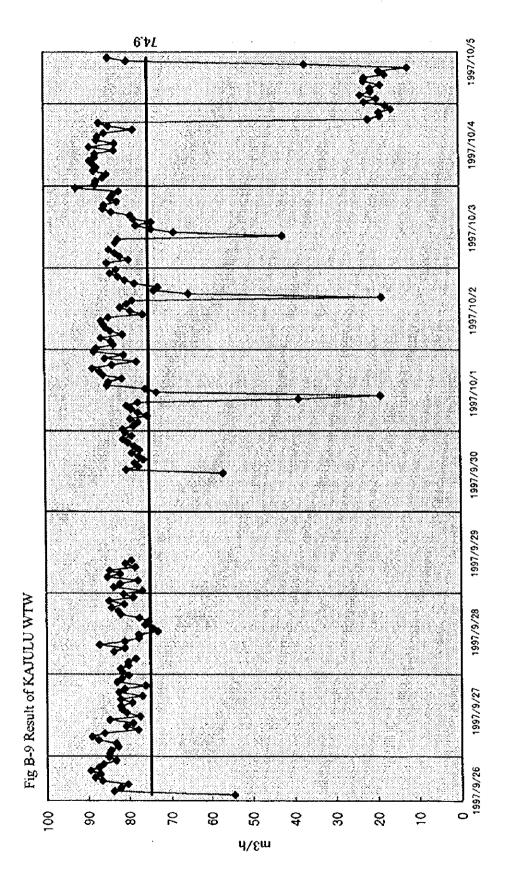
(2) Kajulu Water Treatment Works

Water from Kajulu WTW is fed to the service reservoir at Kibuye under gravity via 150 mm AC main, approximate length 10 km. Measured points are as follows;



At the time of survey, two pressure filter units were out of order but were repaired during the course of the flow measurement survey.

The survey results are as follows;



DAY

Calculation of average flow rate is the same as that of Lake WTW.

Average flow rate is 74.9 m3/h, in other words;

Average production per day in Kajulu WTW is 1,800 m³/day.

(3) Summary

Water production of each water treatment works is as follows;

Table B-4 Measured Production of existing WTW

Water Treatment Works	Water Production (m ³ /day)
Lake WTW	15,800
Kajulu WTW	1,800
Total	17,600

B7.3 LEAKAGE SURVEY

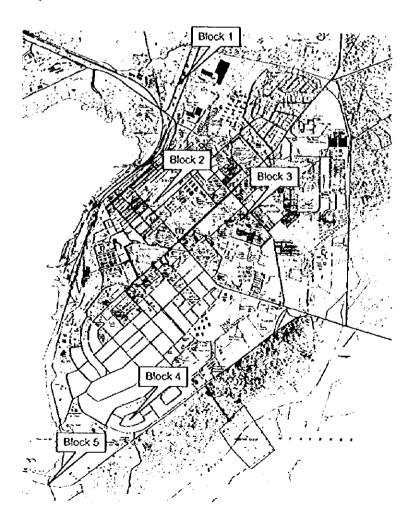
B7.3.1 Location of the survey

The locations of leakage survey were a representative of different parts of the distribution network. The areas were selected as followed;

Block1 Industrial area Subuni Road
Block2 Commercial area Odera Street
Block3 Residential area Ondick Shopping Center
Block4 Residential area Milimani Estate
Block5 Residential area Ndunga Estate

Fig B-10 shows locations of the blocks.

Fig B-10 Location of Block



B7.3.2 Methodology

In order to assess the magnitude of water losses within the system, Minimum Flow Method and Meter Reading Method were used.

(1) Minimum Flow Method

Minimum Flow method is that minimum flow rate in a 24 hour measurement is assumed to be water loss flow rate because no consumer probably used water at the time when the minimum flow happened. Minimum Flow Method was conducted as follows;

- The block was isolated; only one pipe supplied water to the block and no water exited the block by any other pipe to close valves.
- A device measured flow rate and pressure was set on the inflow pipe.
- Flow rate and pressure were measured.

How to estimate ratio of leakage is as follows;

Identify minimum flow rate Q₀; Q₀ is supposed to be maximum leakage flow

•

Identify pressure P₀ when minimum flow rate Q₀ happened

 Calculate leakage flow Q_L at any point of pressure measurement Pt using by relations between Q_L and Pt;

• $Q_L = A \cdot Pt^{1/2}$

where

A: Constant

QL: Leakage Flow Rate at any time

Pt: Pressure at any time

 $A = Q_0 / P_0^{1/2}$

Calculate consumption flow Qc as follows;

 $Qc = Q - Q_L$ where Qc; Consumption Flow Rate at any time Q; Inflow Rate to the Block at any time

Calculate Leakage Ratio between QL and Q

Leakage Ratio = $(\int Q_L dt) / (\int Q dt)$ where;

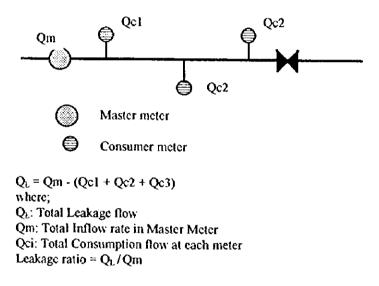
 $\int Q_L dt$: total leakage flow in a day

 $\int Q dt$: total inflow to the block in a day

(2) Meter Reading Method

Meter Reading method is that the difference of figures between a master meter reading figures and total consumer's meter reading figures should be water losses. The measurement was carried out for 24 hours in some blocks.

Fig B-11 Schematic Meter Reading Method System



B7.3.3 Results of the survey

The measurement by Meter Reading Method was supplementary carried out. To mention below, the results of some blocks by Minimum Flow Method were extremely high values, so re-survey was carried out by Meter Reading Method whether these results were good or not. Table B-5 shows cases of the survey;

	Minimum Flow Method	Meter Reading Method
Block 1	DONE	DONE
Block 2	DONE	
Block 3	DONE	DONE
Block 4	DONE	
Block 5	DONE	DONE

Table B-5 Executed cases of the Survey

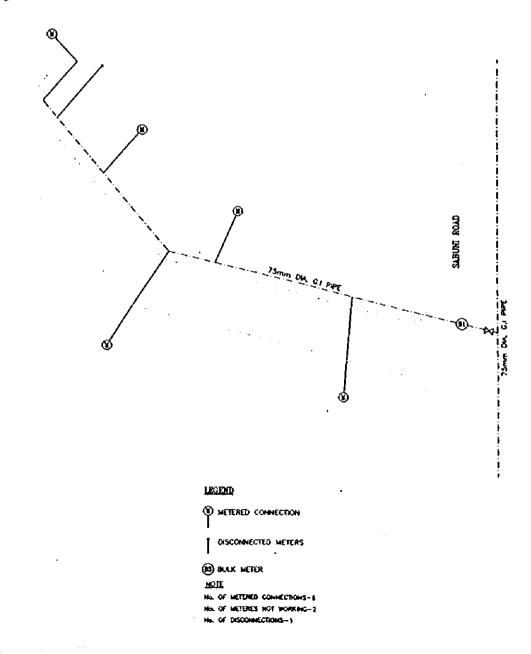
(1) Block 1

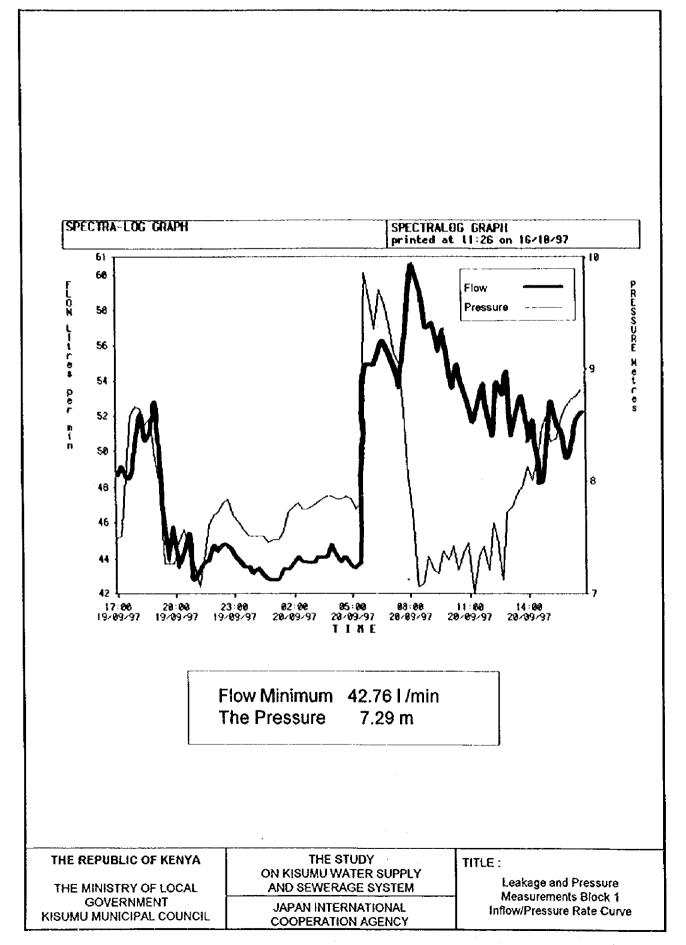
a) Minimum Flow Method

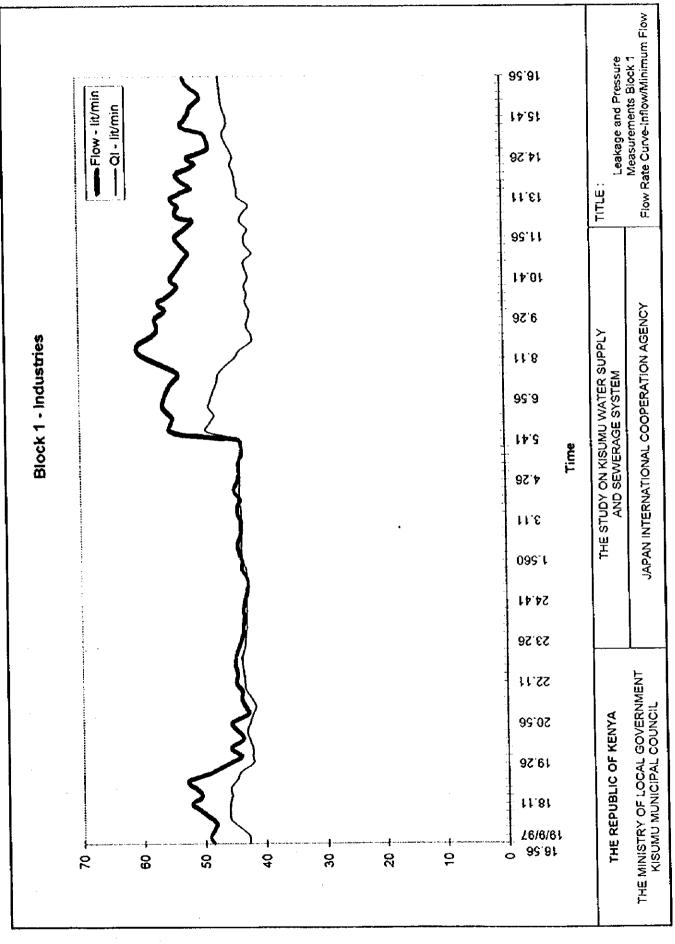
Location, Measured flow rate and pressure, Calculated Q₁ and Q are shown Fig B-12, Fig B-13, Fig B-14.

Leakage Ratio is 89%.

Fig B-12 Block1 Location







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JICA STUDY ON KISUMU WATER SUPPLY AND SEWERAGE SYSTEM

b) Meter Reading Method

Location is shown in Fig B-15. Table B-6 shows results of meter reading;

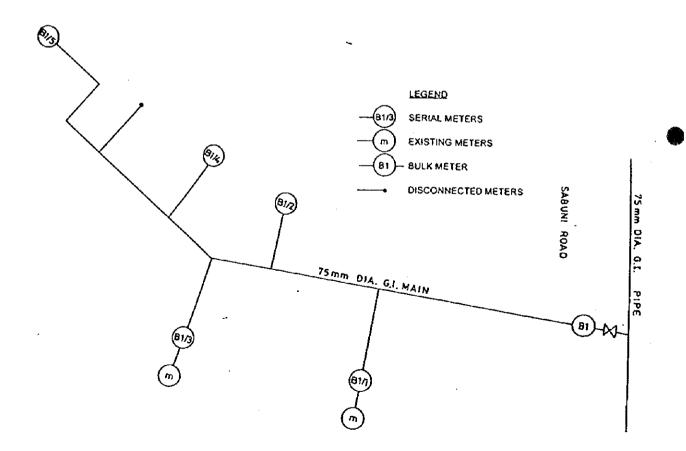
Meter Location	Start Meter Reading	End Meter Reading	Total Consumption m ³	Start Date	Measured period min	Consumptio n m ³ /day
BI	1505,390	1546.220	40.830	97/10/7	1485	39.59
Total				·		39.59
B1/1	0,289	0,485	0.196	97/10/7	1482	0.19
B1/2	0.008	0.054	0.048	97/10/7	1481	0.05
B1/3	3,831	9.964	6.133	97/10/7	1480	5.97
81/4	1723,846	1724,751	0.905	97/10/7	1485	0.88
B1/5	2.012	2.089	0.077	97/10/7	1487	0.08
Total						7.16

 Table B-6
 Results of Meter Reading in Block 1

 $Q_L = 39.59 - 7.16 = 32.43$

Leakage ratio is 82%

Fig B-15 Block 1 Location



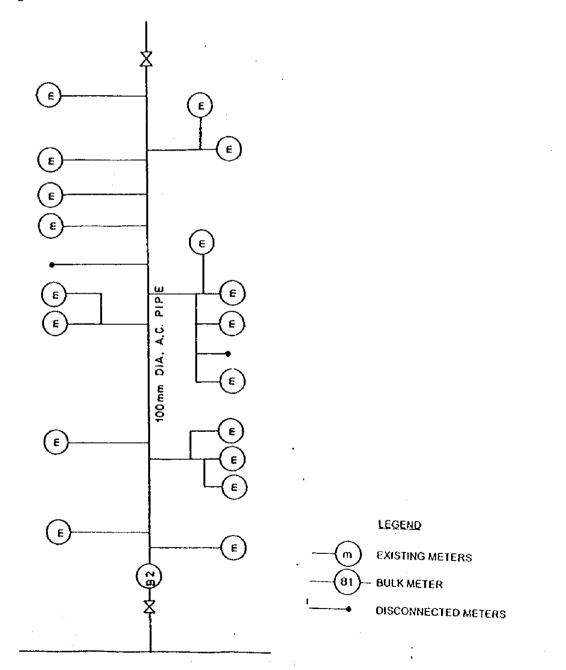
(2) Block 2

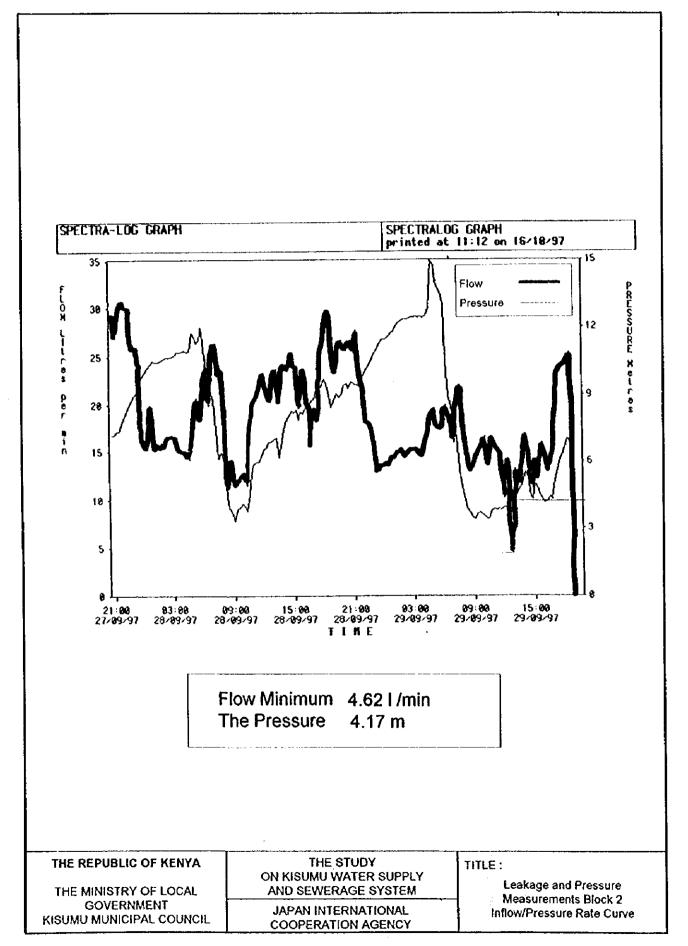
a) Minimum Flow Method

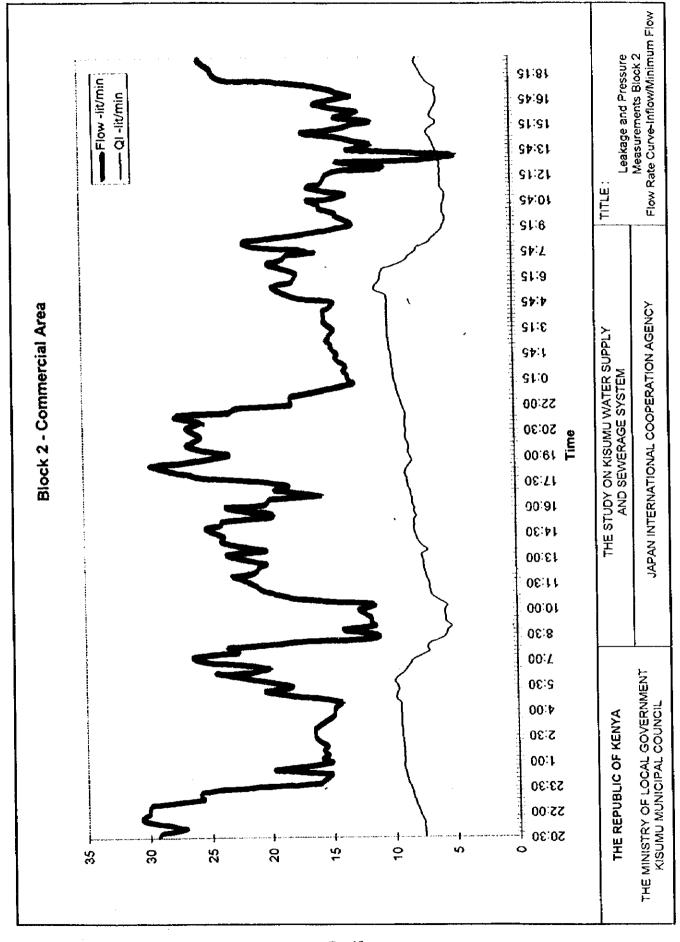
Location, Measured flow rate and pressure, Calculated Q_L and Q are shown Fig B-16, Fig B-17, Fig B-18.

Leakage Ratio is 51%.

Fig B-16 Block 2 Location







D

JICA STUDY ON KISUMU WATER SUPPLY AND SEWERAGE SYSTEM

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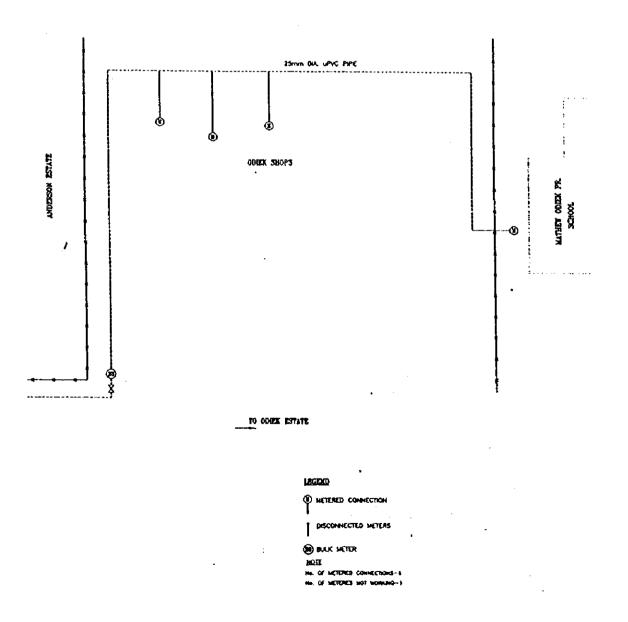
(3) Block 3

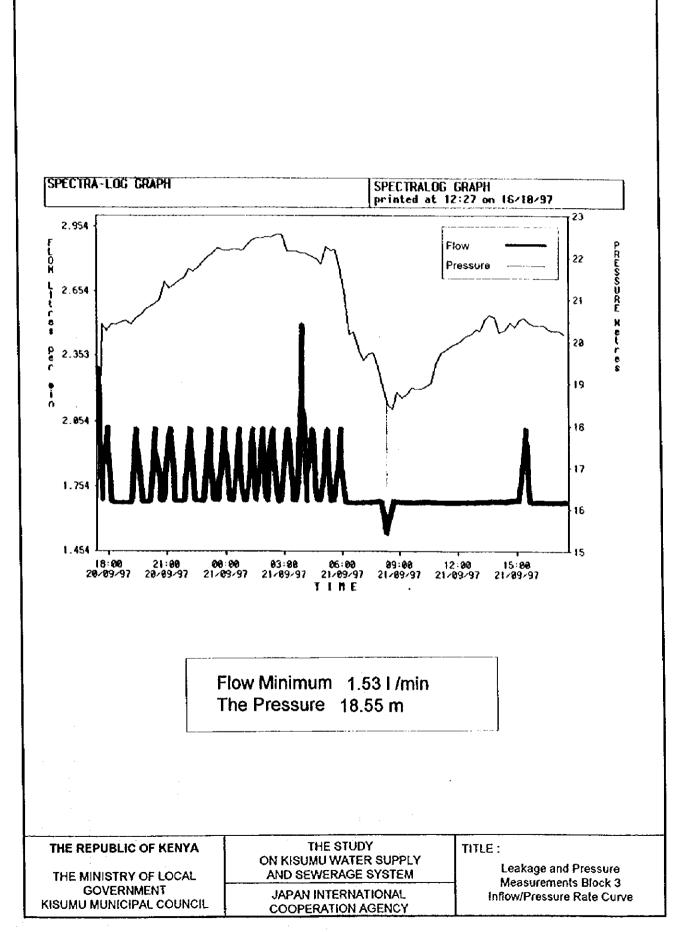
a) Minimum Flow Method

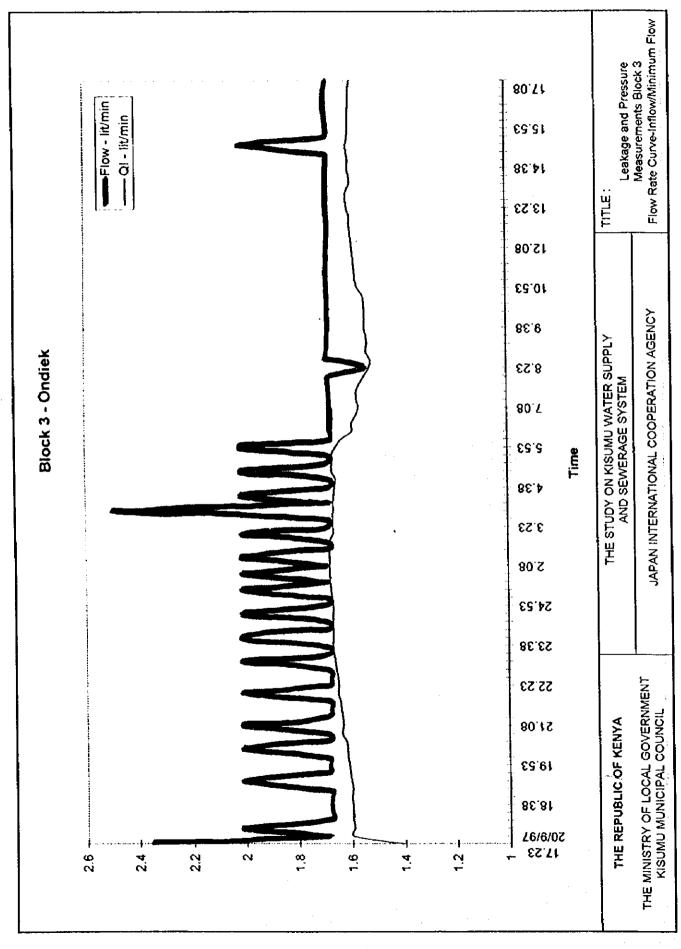
Location, Measured flow rate and pressure, Calculated Q_L and Q are shown Fig B-19, Fig B-20, Fig B-21.

Leakage Ratio is 92%.









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b) Meter Reading Method

Location is shown in Fig B-22. Table B-7 shows results of meter reading;

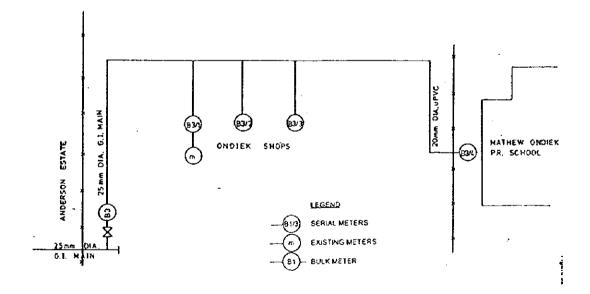
Meter	Start Meter	End Meter	Total	Start	Measured	Consumptio
Location	Reading	Reading	Consumption	Date	period	n
			n'		min	m³/day
B3	30.012	32.953	2.941	97/10/7	1510	2.80
Total						2.80
B3/1	1.889	2.396	0,507	97/10/7	1508	0.48
B3/2	3,151	4.379	1.228	97/10/7	1510	1.17
B3/3	0.058	0.087	0.029	97/10/7	1500	0.03
B3/4	2.055	2.122	0.067	97/10/7	1500	0.06
Total						1.74

Table B-7 Results of Meter Reading in Block 1

 $Q_L = 2.80 - 1.74 = 1.06$

Leakage ratio is 38%

Fig B-22 Block 3 Location



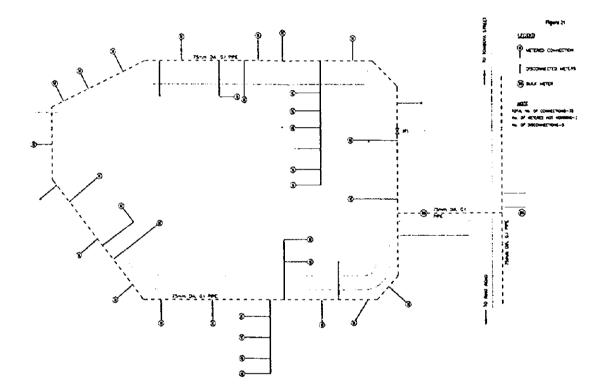
(4) Block 4

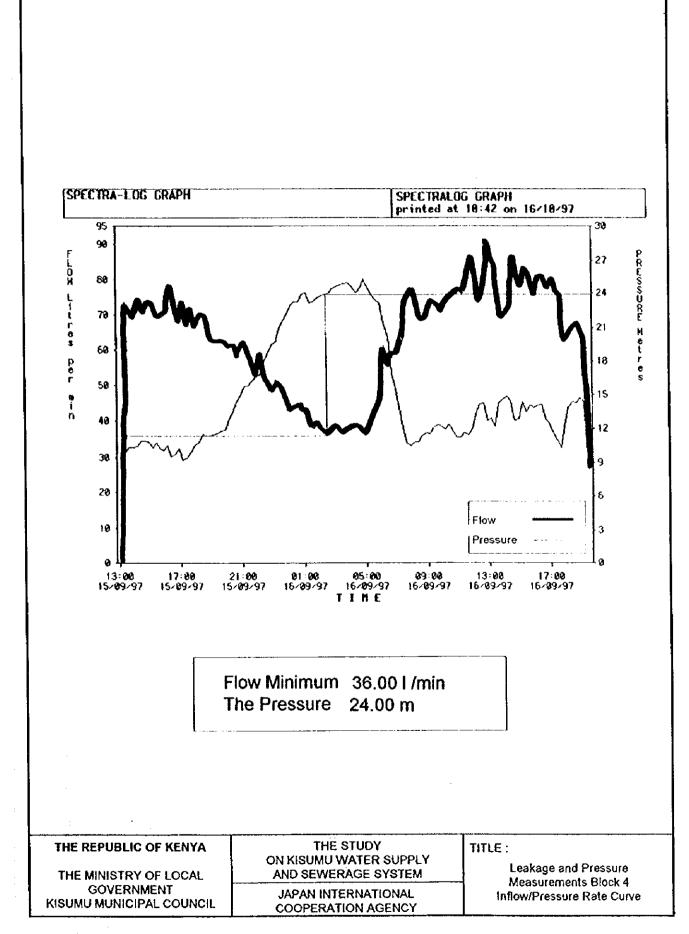
a) Minimum Flow Method

Location, Measured flow rate and pressure, Calculated Q_L and Q are shown Fig B-23, Fig B-24, Fig B-25.

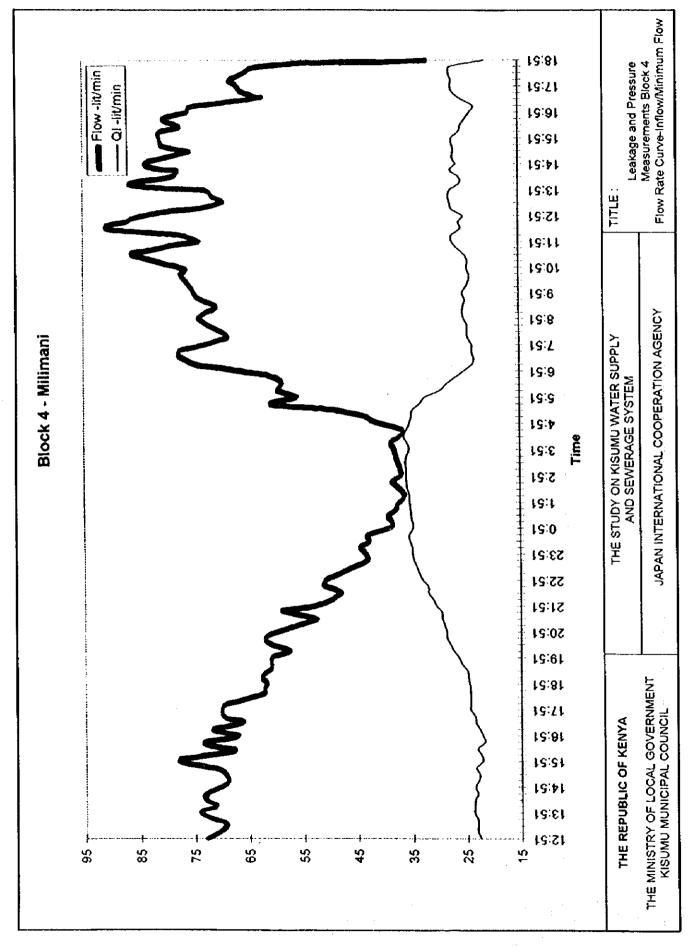
Leakage Ratio is 44%.

Fig B-23 Block 4 Location





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JICA STUDY ON KISUMU WATER SUPPLY AND SEWERAGE SYSTEM

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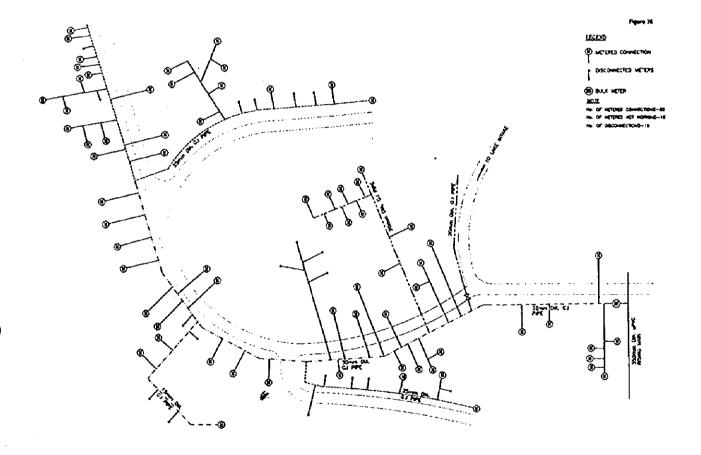
(5) Block 5

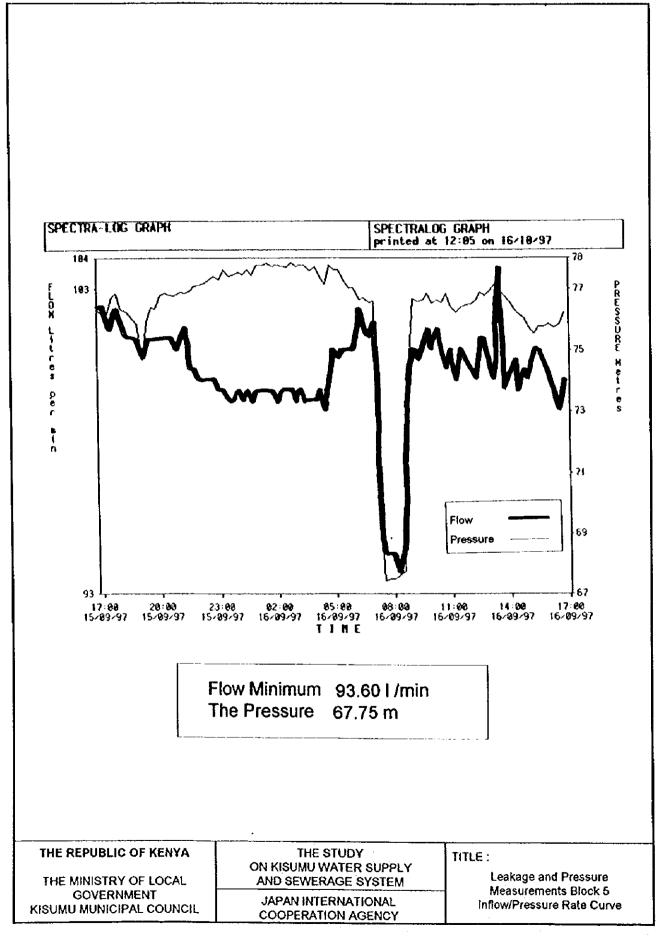
a) Minimum Flow Method

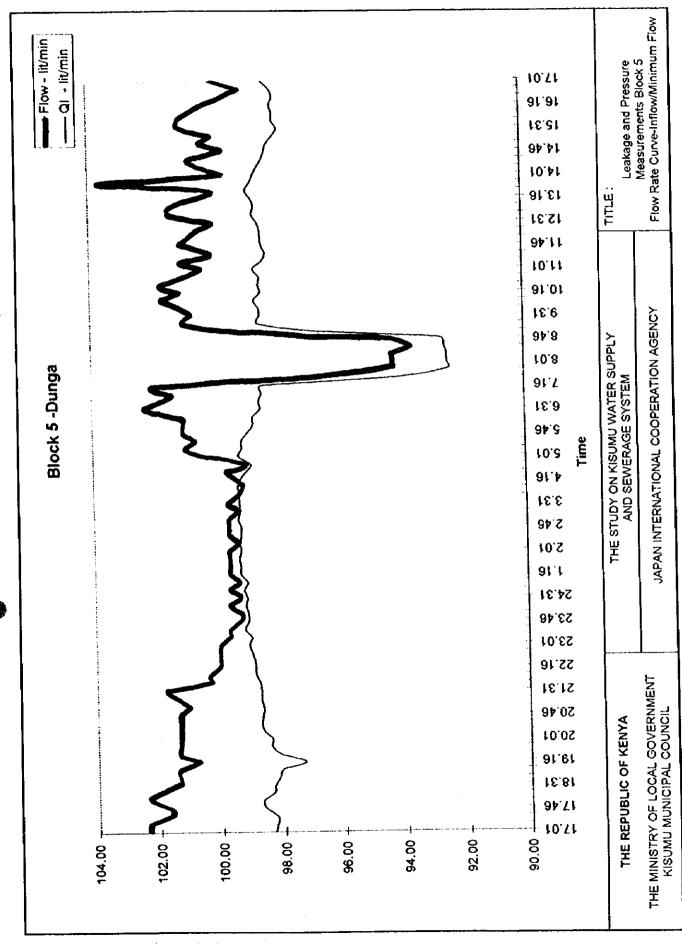
Location, Measured flow rate and pressure, Calculated Q_L and Q are shown Fig B-26, Fig B-27, Fig B-28.

Leakage Ratio is 98%.

Fig B-26 Block 5 Location







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b) Meter Reading Method

Location is shown in Fig B-29. Table B-8 shows results of meter reading;

TADIC D-0	· · · · · · · · · · · · · · · · · · ·	The second m	· · · · · · · · · · · · · · · · · · ·			
Meter	Start Meter	End Meter	Total	Start	Measured	Consumptio
Location	Reading	Reading	Consumption	Date	period	n
			m'		mia	m³/day
B5	5463.280	5606.545	143.265	97/10/5	1465	140.82
Total					:	140,82
B5A	3091.385	3159.237	67.852	07/10/5	1475	66.24
Total				,		66.24
B5/1	18.422	26.561	8.139	97/10/5	1460	8.03
B5/2	11.781	22.986	11.205	97/10/5	1460	11.05
B5/3	4.314	7.090	2.776	97/10/5	1460	2.74
B5/4	1.754	4.630	2.876	97/10/5	1465	2.83
B5/5	0.853	1.466	0.613	97/10/5	1460	0.60
B5/6	2574.868	2576.092	1.224	97/10/5	1460	1.21
Total				•		26.46

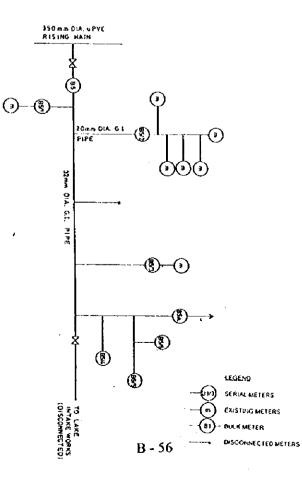
 Table B-8
 Results of Meter Reading in Block 1

Q = 140.24 - 66.24 = 74.00

 $Q_{i} = 140.82 - 66.24 - 26.46 = 48.12$

Leakage ratio is 65%

Fig B-29 Block 5 Location



B7. 3.4 Summary

Survey results are shown in Table B-9.

Table B-9 Summary

Block	No. of	Leakage Ratio (%)		Remarks
	Connections	Minimum Flow Method	Meter Reading Method	
81	5	89	82	Industrial
B2	17	51	-	Commercial
B3	4	92	*38	Residential
B4	27	*44	-	Residential
B5	66	98	65	Residential

Note: Average leakage ratio is considered to be between the figures with *mark.

B7.4 CONDITION SURVEY OF THE EXISTING DISTRIBUTION PIPE

B7.4.1 General

A condition survey has been carried out on the existing distribution pipe network. The purpose of this survey was to record all components of the system, generally as follows;

- Pipe location
- Pipe diameter (Nominal) 80mm diameter and above
- Length of pipe
- Material of pipe
- Year of construction
- Location of bulk meters, Valves, hydrants, air valves and other appurtenances
- Updating of 1:2500 scale existing drawings

B7.4.2 Methodology

In order to identify mains on the ground, node points were adopted which were transferred to 1:2500 scale maps.

Casual labourer were engaged to manually excavate the junction points in the distribution net work and notes made by a engineer in the field about the status of appurtenances, size of pipe etc.

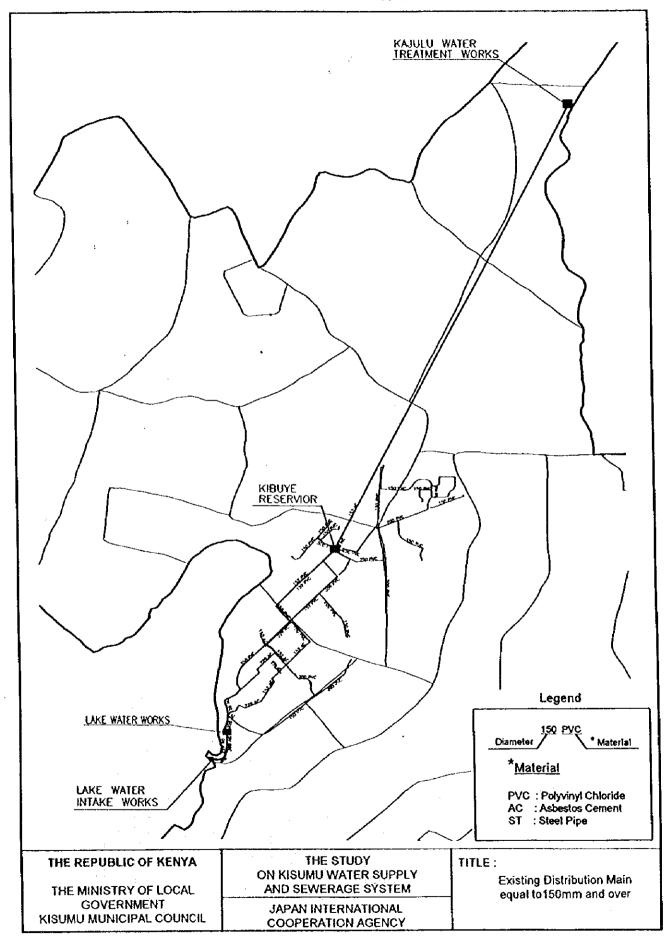
The data obtained in the field has then been transferred to the 1:2500 drawings.

During the entire period of investigation, liaison was maintained with the staff of the KMC such as General Manager, Water Works Superintendents, Operator, Meter Reader, Line Patrollers.

B7.4.3 Results of the Survey

Results are shown in Fig B-30 and Table B-10.

JICA STUDY ON KISUMU WATER SUPPLY AND SEWERAGE SYSTEM



Diameter	Material				Leng	Length, m			
		1950-1955	1956-1960	1961-1965	1966-1970	1971-1975	1976-1980	1981 Onwards	Total
350 mm	GMS				450				450
350 mm	DVGu	130				1,050	3,890		2'020
300 mm	AC	1,330							1,330
300 mm	uPVC						:		580
250 mm	DVQU						2,280		2,280
225 mm -	AC	4,575							4,575
200 mm -	uPVC					2,223	240		2,463
150 mm	AC	15,070							15,070
150 mm	GMS				4,090			5,720	9,810
150 mm	uPVC			1,760		1,360	2,250		5,370
125 mm	GMS				1,580				1,580
100 mm -	AC	2,155	1,310		360				3,825
100 mm	GMS	220			S.470				5,690
100 mm	uPVC			2.090			285	1.000	3,375
80 mm	AC	3.610	525		630				4,765
80 mm	GMS	7,170	2,900		25,079			800	36,254
80 mm	nPVC			8,500			2/0		9,070
TOTAL		34,260	4.736	12,360	37,659	4,633	9,515	7,620	111,567
		212	2		141 0	107	202		

Material	350 mm	300 mm	250 mm	225 mm	200 mm	150 mm	125 mm	100 mm	80 mm	TOTAL	*
uPVC Pipe	S.070	580	2,280		2.463	5,370		3,375	9,070	28,208	25.3
IC Pipe		1.330		4,575		15.070		3.825	4,765	29,565	26.5
GMS Pipe	450					9,810	1,580	5,690	36,254	53,784	48.2
TOTAL	5,520	1,910	2,280	4,575	2.463	30,250	1,580	12.890	50.089	111,557	100.0

As shown in Fig B-30, existing pipe is concentrated in the center of old town (Kibuye and millimani). On the other hand, Fig B-31 shows relation between pipe construction year and length of pipe. The ratio of pipe length laid by the year of 1970 reached to 80%.

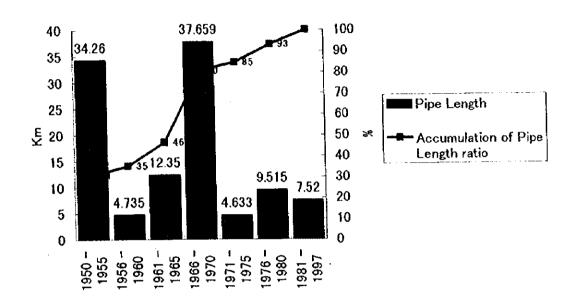


Fig B-31 Pipe Length and Construction Year

B7.5 Service Meter Survey

Sampling survey of service meter condition was carried out. Results are shown in Table B-11 and Table B-12.

Table 5-1-1 Service Meter Condition Survey

		Surveyed on 19	97/9
S	Survey Items	Number of	Component
		Meters	%
No. of household v	visited	3	09
Meter condition	Working	1	92 62.1
	Wrong or no working	1	17 37.9
	Total	3	09 100.0

Table 5-1-2 Service Meter Accuracy Survey

Location	Consump	tion (m3)	Difference					
	Test meter	Existing meter	m3	%	%			
	A	B	A - 8	8/A x100	Simpl average			
ndustrial area								
Sabuni Road								
B1/3- Alro meat factory	6.109	5.169	0.940	84.61				
Industrial area		1						
Sabuni Road	0.289	0.270	0.019	93.43	1			
8 1/1- Grada Agencies								
Ndunga estate			1					
B 5/3 - Building under								
construction	2.776	2.460	0.316	88.62				
Ondieki Estate								
B 3/1 - Butchery	0.506	0.392	0.114	77.47	 			
Total	9.680	8.291	1.389	85.65	86.0			

.

Note: Location of each number (eg. B1/3) is referred to O3.2 Results of Survey

APPENDIX-C

EXISTING WASTEWATER MANAGEMENT FACILITIES

APPENDIX C

.

EXISTING WASTEWATER MANAGEMENT FACILITIES TABLE OF CONTENTS

Cl	Introdu	ction
C2	Wastev	vater Collection Facility
	C2.1	Trunk Sewers
	C2.2	Pump Stations
C3	Sewag	e Treatment Works
	C3.1	Conventional Sewage Treatment Works
	C3.2	Nyalenda Sewage Treatment Works
C4	On-site	z/Community Wastewater Treatment Facilities
C5	Indust	rial Wastewater Pre-treatment FacilitiesC-4

C Existing Wastewater Management Facilities

C1 Introduction

The data and information on the existing wastewater management system are summarized to evaluate the system for future use and to formulate a future physical plan. The existing wastewater management system in Kisumu Municipality is a combination of sewerage system, composed of sewers and pump stations and sewage treatment works, and on-site wastewater treatment system. Important findings on pre-treatment facilities for industrial wastewater are also explained.

C2 Wastewater Collection Facilities

C2.1 Trunk Sewers

All sewers within the existing sewer net works are concrete pipes with ogee joints except for the rising mains which are either uPVC or asbestos cement. The first sewers were laid in 1958, thus the oldest sewers are around forty years old, but collapses are rare and sewers appear to be in sound condition. However, blockages are very frequent in certain sections to cause sewage overflowing the streets.

The capacity of existing trunk sewers in Central and Eastern WTD was calculated based on the information of pipe diameter, invert level and ground surface level collected from the previous F/S report and of pipe length read directly from the drawings in the F/S. For calculation, the Manning equation is used for gravity sewers with "n" value of 0.015. The results are shown in Table C-1 and C-2 for Central and Eastern WTD, respectively. The capacity of existing sewers will be examined to be able to cover the future design flow rate, the detail will be given in Appendix H.

In addition to the information of existing sewers collected from the previous F/S report, the following data and information on the existing sewers were collected by the longitudinal surveys conducted by the JICA Study Team. The collected data and information is pipe location, pipe diameter, pipe length, ground level, invert level and manhole cover level. The sewer pipes are 225 to 450 mm in diameter and 10.33 km in total length:

- Sewers from the municipal staughter house to the Kibos trunk sewers (diameter 225 mm, tength 3.82 km)
- Kibos trunk sewer

(diameter 225 mm, length 2.77 km) (diameter 300 mm, length 1.87 km) (diameter 400 mm, length 0.69 km)

 Sewers along the Ring road (from Kisumu Christian Center to Nairobi roads) (diameter 225 mm, length 1.18 km)

Based on the information collected, the capacity of above sewers is calculated as shown in Table C-3. The results indicate that these sewers do not have enough capacity required for the flow of the year of 2015 as summarized in Table C-4. In addition, some part of the sewers, which installed at shallower depth under the ground or installed above the ground level, do not have a function as trunk sewers, means that branch sewers cannot be connected with the sewers without any pump system.

Therefore, these sewers are judged not to treat as trunk sewers for future use, and the figures of existing trunk sewers mentioned in Section 4.3.2 (2) Eastern WTD excludes those of sewers. Some part of the sewers will be used as branch sewers in future.

C2.2 Pump Stations

Existing three pump stations, Sunset Hotel P.S., Kendu Lane P.S., and Mumias Road P.S., are not operated due to the broken pumps without any repairs. This resulted in sewage overflows at manholes upstream of the stations or direct charge to Lake Victoria. These pump stations are necessary to be rehabilitated urgently to improve the living environmental conditions and dispose the health hazards smoothly.

C3 Sewage Treatment Works

C3.1 Conventional Sewage Treatment Works

Conventional Treatment Works with a treatment capacity of 6,800 m³/d was initially completed in 1957 and once rehabilitated in 1987. It provides primary and secondary treatment to an inflow comprising of domestic, commercial and industrial wastewater. At present the mechanical/ electrical equipment is in need of rehabilitation/replacement, and the Conventional Treatment Works has experienced frequent stoppages due to failure of mechanical and electrical facilities. The Works is not operated properly during the night time due to the absence of operating staff.

The sewage treatment works has been subjected to severe over-loading in terms of sewage

volume and loads, the quality of effluent is more than 100 mg/l in terms of BOD. The inlet, primary sedimentation tanks and humus tanks will need to be expanded to cope with the flows proposed in 2015. The bio-filters are the limiting component at present, with three not functioning and the remaining three filters operating at approximately twice the design loading, thereby achieving a very much reduced BOD removal rate. In order to preserve the quality of effluent below 50 mg/l BOD it is necessary to rehabilitate the existing sewage treatment works and to introduce adequate non-structure measures to control the industrial wastewater, which is the major pollution load contributor.

In addition to treating wastewater the existing works contain sludge digestion and drying facilities. The facilities convert the liquid sludge removed from the wastewater to a dry cake form which is then sold as an agricultural fertilizer. A schematic layout is shown in Fig. C-1.

C3.2 Nyalenda Sewage Treatment Works

This STW treats about 2,000 m³/d of sewage, which is only about 20 % of the design wastewater treatment capacity and provides preliminary, secondary and tertiary treatment to an inflow predominantly domestic in origin but including effluent from the New Nyanza Hospital. The works consists of an inlet works with screening and grit removal, 3 No. facultative ponds in parallel and 6 No. maturation ponds arranged as three (3) parallel pairs. The treated final effluent is discharged to an adjacent watercourse, from whence it percolates to the lake via the Nyalenda papyrus swamp. A schematic fayout is shown in Fig. C-1.

C4 On-site/Community Wastewater Treatment Facilities

Following on-site wastewater treatment systems such as septic tanks, pit latrines and bucket latrines are commonly used not only in rural area but also in urban and peri- urban areas.

Septic tanks are used in the low density residential areas in Milimani area and at certain institutions. Septic tanks operate satisfactorily in the area. Emptying sludge service for septic tanks are provided by the municipality.

Pit latrines are commonly used in the peri-urban and rural area in the municipality. Pit latrines of various designs are used as follows:

- Pit with a slab at ground level and a superstructure of timber poles and iron sheets without a roof or ventilation to the pit;
- Pit raised above ground level and a superstructure of timber/iron sheets with a roof.

The raised stab prevents surface water from entering the pit and provides extra pit volume; and

• Ventilated improved pit with a raised slab and sound superstructure. Pits were seen with a manhole for emptying by a vacuum tanker or manually.

In areas prone to flooding, a common problem is collapse of pit latrines. This is probably due to their poor construction in unstable soils and the problem can be overcome with lining of the pit and raising the floor slab to prevent surface waters entering the pit.

Another common problem is that of high ground water level that reduces the available pit volume and increases the frequency of emptying the pits or digging new pits. Contamination of the groundwater is a serious consequence especially in areas where shallow wells and boreholes are used for local water supply. Emptying of pits in many areas is not possible by vacuum tanker due to the limited or no access to the sites.

Bucket latrines are used in a very limited area. A daily collection service are still operated by the Municipality, and the waste is disposed to Nyalenda STW. But daily collection service becomes difficult to operate without any proper vehicle or manpower.

In rural area where population density is low, the on-site facilities function well. However, in high population density area, especially in informal settlements, combination of poor state of pit fatrines, poor drainage and lack of proper solid waste disposal leads to a deteriorate the sanitary environment and significant increase in water borne diseases such as mataria and diarrhea. In those areas, wastewater from washing, cooking, bathing is usually discharged into the nearest drainage channels. In case that drainage channels are not well defined, this feaves stagnant pools of wastewater. The close proximity of pit latrines can cause the contamination of shallow wells which constitutes the main water supply.

C5 Industrial Wastewater Pre-treatment Facilities

A questionnaire survey on existing industries was carried out with assistance of WSD, in which industrial wastewater pre-treatment facilities were one of the questions.

The only pre-treatment of industrial effluent appears to be at Kicomi Textile Factory. The treatment of the final effluent is by settlement tank, this does not however remove toxic substances, such as sodium sulphide, copper and chromium which are used in the drying process, high temperatures and high pH values were also observed in the effluent. These substances cause poor bio-filter performance in the Conventional Treatment Works.

Pre-treatment of the textile factory waste would allow the Conventional Treatment Works to produce a satisfactory BOD. The most effective ways of dealing biologically with textile wastes are by activated sludge and oxidation ditches (aerated lagoons).

Sewer	M.H.	M.H.]			<u> </u>	t	Invert	Ground Surface	Remarks
Line	No.	No.	Dia.	Slope	Velocity	Capacity	Length	Elevation	Elevation	110100189
No.				0/00	m/S	m³/S	m	m	m	
		{	mm			0.027	92	1,180.300	1,182.624	
-1	K18		175	17.9	1.11	0.027			1,181.100	
		<u>K17</u>	7"				92	1,178.651 1,178.651	1,181.100	
	<u>K17</u>		175	-8.8			94		1,179.576	• • • • • • • • • • • • • • • • • • •
		K16					186			
	K16		175	41.6	1.69	0.041	78		1,179.576	
	· ·	K15					· · · · · · · · · · · · · · · · · · ·	1,176.234		<u> </u>
	K15		175	10.2	0.84	0.020			1,178.509	
	 	K14					379		1.176.985	
	K14		175	11.3	0.88	0.021		1,175.061	4	
		K13				_	482	1	1,175.766	I
4-2	К13	<u> </u>	225	8.3	0.89	0.035				C
		K12	9°		ļ		50			
A-3	K12		225	2.7	0.51	0.020	75	1,173.482		
		K11	9″			ļ	75			
	K11		225	4.1	0.63	0.025	8		1,175.156	
		K10					160	1,172.927	1,174.699	
	K10		225	4.4	0.65	0.020	3 78	3 1,172.927	1,174,699	
	•••••••	К9	Ĩ	Γ			238	1,172.58	1,174.394	
A-4	К9	1	225	3.6	0.5	0.02	3 10	3 1,172.58	6 1,174.394	
		К8	9″	T	1		10	3 1,172.21	1,174.09)
<u> </u>	К8	+	225	3.	0.6	0.02	4 9	1 1.172.21	7 1,174.09)
•••••		К7			1		19	4 1,171.87	5 1,173.48	<u>)</u>
	K7	1	225	3.6	3 0.6	0 0.02	4 7	5 1,171.87	6 1,173.48	<u>b</u>
····		K6					26	9 1,171.59	2 1,172.56	<u> </u>
	K6		225	5 3.	5 0.5	8 0.02	3 8	0 1,171.59	2 1,172.56	6
		К5					34	9 1,171.31	2 1,172.26	1
	K5	-	225	5 4.	9 0.6	9 0.02			2 1,172.26	
		K4		<u></u>	×		····	9 1,170.96		
		- 124	22	5 2.	8 0.5	2 0.02		7 1,170.96		1
	K4	КЗ		<u> </u>	<u> </u>		52	6 1,170.67	2 1,171.95	6
			22	5 4	0 0.6	2 0.02			2 1,171.95	
	К3			<u>. 4</u>	<u>vi 0.0</u>	<u>- 0.02</u>			1 1,171.19	
		K2			3 0.8	4 0.03			1 1,171.19	- r
	K2		22	57.	<u></u>		63	···· I ······	5 1,171.04	
	-	K1A	+	<u></u>		0.0		35 1,170.17		
	KIA		22	5 8	4 0.9				2 1,170.73	
		<u>K1</u>								
	KI		22	5 11	.6 1.0	x5 0.04		*****	2 1,170.73	
		T36	·				+	54 1,168.78	32 1,169.67	¥
1	1					1				

Table C-1 Inventory of Existing Trunk Sewer in the Central WTD

Sewer Line	M.H. No.	M.H. No.	Dia.	Slope	Velocity	Capacity	Length	Invert Elevation	Ground Surface	Remarks
No.			mm	0/00	m/S	m³/S	m	m	Elevation m	
				0/00	10/3	1170				
<u>4'-1</u>	<u>T41A</u>		375						1,171.804	
		T41							1,171.956	
•••••	T41								1,171.956	,
	 	T40			<u></u>			1,170,431		
	T40								1,171.956	
		T39						N/A	N/A	
	T39							N/A	N/A	· · · · · · · · · · · · · · · · · · ·
		T38							1,170,584	
	T38							1,169.544	1,170.584	
	ļ	T37						1,169.062	1,169.822	
	T37						.	1,169.062	1,169.822	
	<u> </u>	T36						1,168.782	1,169.670	
To Sew	er A-5									
								ļ		
A-5	T36		375	3.7	0.84	0.092	42	1,168.782	1,169.670	
		T34	15*				42	1,168.626	1,169.365	
	T34		375	16.0	1.74	0.192	61	1,168.626	1,169.365	
•••••••••••		T33			1		103	1,167.648	1,168.603	
A-6	T33		375	11.8	1.49	0.165	80	1	1.1.1.1	
		T32	15″				80	1,166.700	1,168.146	1
- <u></u> -	T32		375	6.8	1.13	0.125	88		1,168.146	
		T31					168		1,167.384	
	T31	1	375	4.4	0.91	0.101	+		1,167.384	
		τ30			1		243	· · · · · · · · · · · · · · · · · · ·		
A-7	T 30	1.00	375	11.4	1.47	0.162	1		1,167.689	
· · · ·		T29	15″				100		1,165.860	
L	T29	1.5	375	37.3	2.66	0 2 93		1,164.624		
·····		T28	1 373		1		1	1,160.891		
	T28	120	375	14.4	1.6	5 0.182		1,160.891		
	140	1.62	3/0	14.4	1.0	V.104	1			T
	400	A63		05		0.04	308		6 1,160.678	
	A63	700	375	25.4	2.1	0.24	1	F	6 1,160.678	
		T26	1				350		1,159.764	
	T26		375	33.6	6 2.5	2 0.27		8 1,158.269		
	_	T25	<u> </u>	<u> </u>			400		9 1,158.088	
	T25		375	32.0	2.4	6 0.27	····		1,158.088	
L		T24		<u> </u>	<u> </u>	<u> </u>	450	0 <u>1,154.97</u> 4	4 1,156.106	<u>մ</u>

Table C-1 Inventory of Existing Trunk Sewer in the Central WTD

Sewer	M.H.	M.H.						Invert	Ground	.
Line	No.	No.	Dia.	Slope	Velocity	Capacity	Length	Elevation	Surface	Remarks
No.						m³/S			Elevation	
			<u> </u>	0/00	m/S		m	m	m	
<u> </u>	T24		525	45.5	3.67	0,795			1,156.106	
		T23	21″				37	1,153.289	1,154.887	
	T23		525	42.3	3.54	0.767	45		1,154.887	
		T21A				L	82	1,151.387	1,153.516	
B-2	M8		225	6.0	0.76	0.030	62	1,177.213	1,178.222	
		M7	9″				62	1,176.843	1,178.239	
	M7		225	6.0	0.76	0.030	62	1,176.843	1,178.239	
		M6					124	1,176.472	1,178.264	
	M6		225	6.2	0.77	0.031	60			
		M5					184	1		
	M5		225	6.2	0.77	0.031	60	<u> </u>		
		M4					244		I	
	M4		225	6.2	0.77	0.031	60			
		M3		V.L	1		304	1		
	142	1410	225	6.2	0.77	0.031	60		1,177.192	1
••••••••••	M3	M2	220	<u>U.</u> 2	0.77	0.001	364		1,176.591	
·		MZ	0.05	10.0	1.01	0.050	t	1		
•••••	M2		225	18.0	1.31	0.052	T			
		M1				0.100	428		1,175.064	
	<u>M1</u>		225	69.0	2.57	0.102	1		1,175.064	T
		A18A					474	1		
	A18A		225	16.0	1.24	0.049	1		1	
	·	A18		ļ		╂	496	1	1	
	A18		225	24.4	1.53	0.061	1	1	1,172.870	1
		A17	_		<u> </u>	_	591			1
	A17		225	35.7	1.85	0.074			1,169.822	
		A16			<u> </u>	ļ	651		1,168.298	
B-3	A16	[300	16.1	1.50	0.106			1,168.298	1
	1	A15	12″		<u> </u>	 			1,167.079	1
	A15		300	5.6	0.89	0.063	85	1,164,856	1,167.079	
		A14	<u> </u>		L	<u> </u>	147	1,164.378	1,167.232	·
8-4	A14		300	37.2	2.29	0.162	2 73	1,164.378	1,167.232	
		A13	12″		1	L	73	1,161.659	1,164.336	s
	A13		300	22.7	1.79	0.126	6	1,161.659	1,164.336	
	Ī	A12	Ι			1	13	1,160.299	1,163.117	/
	A12	T	300	22.4	1.7	7 0.12	5 60			1
		A11	1	Γ	1	ſ	19:		.	
B-5	A11		300	9.9	1.1	8 0.08		1	2 1,162.355	1
<u>×</u>		A10	12″	<u> </u>					1,160.678	
	A10		300	17.9	1.5	9 0.11			1,160.678	
		100	1	· · · · · · · · · · · · · · · · · · ·		·			1,157.47	
L		A9	<u>i</u>	1	J	<u></u>	1 131	1,130.13	1 1,101.470	<u>'i</u>

Table C-1 Inventory of Existing Trunk Sewer in the Central WTD

Sewer	MH.	M.H.						Invert	Ground	· _ ·
Line	No.	No.	Dia.	Slope	Velocity	Capacity	Length	Elevation	Surface	Remarks
No.						3/0			Elevation	
		•	mm	0/00	m/S	m³/S	m	m	m	
3-6-1	A9		300	10.6	1.22	0.086	*********	1,156.157	1,157.478	******
		A8	12″				45	1,155.682	<u>1,157.173</u>	
	A8		300	11.6	1.28	0.090	104	1,155.682	1,157.173	
		A7					149	1,154.478	1,155.649	
	A7		300	11.4	1.27	0.089	102	1,154.478	1,155.649	
		A6					251	1,153.316	1,154.887	
B-6-2	A6		300	4.6	0.80	0.057	60	1,153,316	1,154 887	
		A5					60	1,153.042	1,156.959	
	A5		300	6.0	0.92	0.065	60	1,153.042	1,156.959	
••••••	I	A4A					120	1,152.682	1,155.954	
	A4A		300	4.9	0.83	0.059	1		1	
		A4	······				152		1,155.192	
·	A4		300	5.8	0.90	0.064			1,155.192	
•••••	1	АЗА					217			
	A3A		300	11.6	1.28	0.090	1			
		A3					248		**********************	
B-8	A3	/10	375	1.5	0.53	0.059	1		1,153.016	
		A2	15″		0.55	0.005	52		1,154.430	
	A2	<u>n2</u>		0.0	0.72	0.080	t			· · · · · · · · · · · · · · · · · · ·
	<u></u>	A 1	375	2.8	0.73	0.060			1,154,430	T
		AI	0.75			0.000	112		1	
	A1		375	3.5	0.81	0.090	1		1,152.274	T
		T21A	{	·	<u> </u>		156	1,151.387	1,153.516	<u> </u>
To Sev	wer A-9	•			.					
		 			 				[
							.			
A-9	T21A		525	3.6	1.03	0.224	1			······································
	-↓	T21	21″	 	ļ		32		1,152.296	
	T21		525	2.4	0.84	0,183	32	1,151.271	1,152.296	
		T20		ļ	<u> </u>		64	1,151.195	1,153.058	
	T20		525	2.3	0.83	0.179	50	1,151.195	1,153.058	
		T19		<u> </u>			114	1,151.079	1,153.820	
	T19		525	2.1	0.79	0.171	41	1,151.079	1,153.820	
		T18	<u> </u>				155	1,150.991	1,152.754	
	T18		525	2.8	0.91	0.197	85	1,150.991		
		T17			<u> </u>		1	1,150.756		_
	T17		525	3.1	0.96	6 0.208				
		T16	-	ſ	1		275		1,152.906	.
	T16	1	525	2.3	0.83	3 0.179	1		1,152.906	
	1	T15	1	T	1		342		1,153.668	T
	T15	1	525	2.3	0.83	3 0.179		1,150.497		1
		T14A		<u> </u>			1	1,150.437	T	T
	· · · · ·	1147	L	L	<u> </u>	·		,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	1,104.144	· 2

Table C-1 Inventory of Existing Trunk Sewer in the Central WTD

Sewer Line No.	M.H. No.	M.H. No.	Dia.	Slope	Velocity	Capacity	Length	Invert Elevation	Ground Surface Elevation	Remarks
			mm	0/00	m/S	m³/S	m	m	<u>m</u>	
	T14A		525	1.8	0.73	0.158	10	1,150.427	1,152.144	
		T14					382	1,150,409	1,152.144	
	T14		525	2.3	0.83	0.179	92	1,150.409	1,152.144	
		T13					474	1,150,195	1,152.144	_ <u></u>
	T13		525	4.4	1.14	0.247	25	1,150.195	1,152,144	
		T12					499	1,150.085	1,152,144	
A-10	T12		525	3.0	0.94	0.204	103	1,150.085	1,152.144	
		T11	21"			<u>]</u>	103	1,149.772	1,152.144	
	T11		525	2.0	0.77	0.167	91	1,149.772	1,152.144	
		T10					194	1,149.592	1,152,144	
	T10		525	2.4	0.84	0.183	104	1,149.592	1,152.144	
		T9			<u> </u>		298	1,149.345	1,151.382	
	T9		525	33.0	3.13	0.677	102	1,149.345	1,151.382	
		T8					400	1,145.983	1,150.620	
A-11	T8		600	2.9	1.0	0.287	7 81	1,145.983	1,150.620	
		17	24″				81	1,145.751	1,151.382	
	T7		600	-29.0)		96	3 1,145.751	1,151.382	
	Ι	T6	I				17	1,148.531	1,151.534	l
	T6		600	2.4	1 0.9	2 0.26	1 11	2 1,148.53	1,151.534	
		T5			<u> </u>		289	1,148.26	3 1,150.62(
					1			<u> </u>		

Table C-1 Inventory of Existing Trunk Sower in the Central WTD

Ì

Sewer	M.H.	M.H.						Invert	Ground	
Line	No.	No.	Dia.	Stope	Velocity	Capacity	Length	Elevation	Surface	Remarks
No.									Elevation	
			mm	0/00	m/S	m³/S	m	m	m	
C-1	KA6		225					1,174.378	1,176.833	
		KA5	9″					1,172.597	1,174.507	:
	KA5							1,172.597	1,174.507	н. А.
		KA4						1,165.743	1,167.630	
	KA4							1,165.743		
		KA3						1,157.906		
	КАЗ							1,157.906		
		KA2						1,153.264		
	KA2							1,153.264		
	<u></u>	KA1	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,			•••••••• • •••••		1,150.764		
	KA1							1,150.764	1,152.144	
		T5		• • • • • • • • • • • • • • • • • • • •				***********		
T. O		· · · · · ·		· · · · · · · ·				1,148.263	1,150.620	
To Sew	er A-12	2 		•••••••••••••••••••••••••••••••••••••••						
								ļ		
			·····							
	ļ	 			ļ		ļ		147 - 14	
A-12	Т5		600	2.5	0.94	0.266	102	1,148.263	1,150.620	
		T4	24″				102	1,148.004	1,150.315	
	T4	·	600	2.2	0.88	0.250	120	1,148.004	1,150.315	
		T3					222	1,147.745	1,150.011	
	тз		600	3.0	1.03	0.291	44	1,147.745	1,150.011	
		T2					266	1,147.614	1,150.620	
	T2		600	2.6	0.96	0.271	80	1,147.614	1,150.620	
		TIA	Ţ			1	1	T	1,149.197	
			1			1				
		1					1			
D-1	197	<u> </u>	300	3.3	0.68	0.048	50	1 156 415	1,158.094	
<u> </u>		196	12″	<u> </u>	<u></u>	<u></u>	1	•••••••••••••••••••••••••••••••••••••••	1,158.480	1 • • • • • • • • • • • • • • • • • • •
[196	1.30	300	3.7	0.72	0.051			1,158.480	*
	1.30	99	1	·····	V.//		1		1,158.538	
	00	35	200	20		0.050	95	1	1	1
	99	100	300	3.6	0.7	0.050	1	T	1,158.538	
<u> </u>	100	198	+				141		1,159,212	
	98	-	300	3.3	0.68	0.048	1		1,159.212	
		83	 	1	<u> </u>	<u> </u>	201		1,159,296	
[83		300	3.3	0.68	0.048			1,159,296	
		82	}	{		<u> </u>	242		1,159.536	1
ļ	82		300	3.3	0.68	3 0.048	49	1,155.579	1,159.536	
	I	81	<u> </u>	 			291	1,155.417	1,159.576	il
	81	<u> </u>	300	3.9	0.74	4 0.052	49	1,155.417	1,159.576	
		80A		<u> </u>			340	1,155.225	1,159.700	
[80A	1	300	1.9	0.5	2 0.03	24	1,155.225	1,159.700	
1		80	1			I	1		1,159.060	

Table C~1 Inventory of Existing Trunk Sewer in the Central WTD

Sewer Line	M.H. No.	M.H. No.	Dia.	Slope	Velocity	Capacity	Length	Invert Elevation	Ground Surface Elevation	Remarks
No.			mm	0/00	m/S	m ³ /S	m	m	m	
)2	80		300	3.3	0.68	0.048	50	1,155.180	1,159.060	
) <u>2</u>	00	X15	12″	<u> </u>	<u> </u>		50	1,155.013	1,157.768	
	X15	<u> </u>	300	3.5	0.70	0.050	50		1,157.768	
		X14					100		1,157.690	
	X14	<u></u>	300	3.1	0.66	0.047	42		1,157.690	
		X13					142	1,154.708	1,157.649	
	X13		300	3.6	0.71	0.050	30	1,154.708	1,157.649	
		X12					172	1,154.600	1,157,120	
	X12	1	300	3.3	0.68	0.048	60	1,154.600	1,157.120	
		X11_					232	1,154.400	1,157.390	
	X11		300	3.3	0.68	0.048	40	1,154,400	1,157.390	
		X10	L		<u> </u>		272	1,154.267	1,157.706	
	X10		300	3.3	0.68	0.048	50	1,154.267	1,157.706	
**********		X9	<u> </u>				322	1,154.100	1,157.816	
	X9		300	3.2	0.67	0.047	65	1,154.100	1,157.816	
		X8				<u> </u>	387	1,153.893	1,157.863	
	X8		300	3.3	0.68	0.048	58	1,153,893	1,157.863	
••••••		X7					445	1,153.700	1	E
	X7		300	3.4	0.69	0.049	45	1,153.700	1,156.618	
		X6				<u> </u>	490	1,153.545	1,156.165	
	X6		300	3.2	0.6	0.047	47	1,153.545	1,156.165	
		X5					537	1,153.396	1,156.375	ļ
	X5		300	3.7	0.7	0.051	54	1,153.39	1,156.375	
		X4		<u> </u>			591	1		' l
	X4		300	3.	3 0.6	3 0.048	42	2 1,153.19	1,154.997	
		X3		<u> </u>			633			
	X3		300	40.	3 2.3	3 0.168	4		1	
		X2		<u> </u>			678	1		1
	X2		300) 36.	4 2.2	6 0.160		1,151.24		
		X1		ļ					8 1,150.839	
	<u>X1</u>		300	<u> </u>	8 2.5	9 0.18			6 1,150,839	
		XIA					75		0 1,150.029	
	<u>X1A</u>		30	<u>) 15</u>	5 1.4	8 0.10			0 1,150.02	
		TIA		_			80	3 1,147.40	2 1,149.19	4
To Se	wer A-	13								
					<u> </u>					+
				. .						
L				<u> </u>			1			

Table G-1 Inventory of Existing Trunk Sewer in the Central WTD

Sewer Line No.	M.H. No.	M.H. No.	Dia.	Slope		Capacity	Length	Invert Elevation	Ground Surface Elevation	Remarks
			mm	0/00	m/S	m³/S	m	m	່ m	
E-1	KK6		600				- 86	N/A	N/A	
		KK5	24"				86	<u>N/A</u>	N/A	
	KK5						92	N/A	N/A	
		KK4				[178	N/A	N/A	
	KK4						88	N/A	N/A	
		ККЗ					266	N/A	N/A	
	ккз							N/A	N/A	
		KK2					*********	N/A	N/A	
	KK2							N/A	N/A	
,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,		ККІ				• • • • • • • • • • • • • • • • • • • •		N/A	N/A	
	KK1							N/A	N/A	
		T۱						1,147,409		
To Sew	er A-14					1	t		1,	
	<u> </u>				•			•••••••••••••••		
					- <u></u> .	<u> </u>	<u> </u>			<u> </u>
	. • 14									
A13	TIA		600	-1.0		<u> </u>		1 1 47 402	1 149 197	
<u></u>	<u></u>	T1	24~	1.9						
A-14	TI		600	30.4	3.28	0.928	1	1,147.409		
<u>A-14</u>			24"	<u> </u>	3.20	0.920		1,147,409		
	S4	<u>S4</u>				0.005			1,149.858	
••••	34	0.0	600	32.9	3.41	0.965	*****		1,149,858	
<u> </u>	00	S3	000		1.00				1,143.000	
	<u>S3</u>		600	4.3	1.23	0.349			1,143.000	
		S2				<u> </u>			1,141.476	
••••••	S2	<u> </u>	600	.				******	1,141.476	• • • • • • • • • • • • • • • • • • • •
		SI	 			 	424	N/A	N/A	
	 	 				<u> </u>				
F-1	SA12	1							N/A	
		<u>SA11</u>				ļ			1,146.300	
F-2	SA11	 	.						1,146.300	I
		SA10	┣───	 	ļ	 	180	1,144.543	1,146.150	
	SA10	 	.				54	1,144.543	1,146.150	
	ļ	SA9		L	 	Ļ	234	1 1 44 162	1,146.062	
•••••••••••••••	SA9	.	.				77	1,144.162	1,146.062	1
		SA8			L	_	311	1,143.571	1,146,180	
	SA8			[60	1,143.571	1,146.180	
		SA7						T	1,143,880	
	SA7					1	1		1,143.880	
		SA6	I	[1	I	1,143.090	
	SA6		I		[1		1,143,090	
	l	SA5	I	1	[· · · · · · · · · · · · · · · · · · ·	1	1	I	1,142.679	

Table C-1 Inventory of Existing Trunk Sewer in the Central WTD

Sewer Line No.	M.H. No.	M.H. No.	Dia.	Slope	Velocity	Capacity	Length	Invert Elevation	Ground Surface Elevation	Remarks
			mm	0/00	m/S	m³/S	m	m	m	
	SA5						84	1,141.274	1,142.679	
		SA4					585	1,140.156	1,141.910	
	SA4						38	1,140.156	1,141.910	
		SA3					623	1,139.767	1,141.535	
	SA3						32	1,139.767	1,141.535	
454.11.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1		SA2					655	N/A	1,141.231	
	SA2			I			28	N/A	1,141.231	
		SA1					683	1,138.832	1,141.261	
	SAI						28	1,138.832	1,141.261	
		SAIA]	711	1,138.349	1,140.929	ļ
	SAIA						74	1,138.349	1,140.929	
		SAIB	_	I			785	<u>1,137.100</u>	1,140.960	
							-		-	

Table C-1 Inventory of Existing Trunk Sewer in the Central WTD

Sewer		M.H.						Invert	Ground	
Line	No.	No.	Dia.	Slope	Velocity	Capacity	Length	Elevation	Surface	Remarks
No.				0/00	m/S	m ³ /S		· · · · · · · · · · · · · · · · · · ·	Elevation	
	010						<u>m</u> :	m	m 1 1 00 000	
<u>A~1</u>	B10		175	15.6	1.03	0.025	102	1,168.413	1,169.822	
		B9	7″	- 10.0		0.000	102	1.166.825	1,167.603	
	B9		175	12.9	0.94	0.023		1.166.825		
		B8	4.75	47.4			180	1,165.816		
	B 8		175	17.1	1.08	0.026		1,165.816	1,167.384	
		87	476				270	1,164.274		
	87		175	10.4	0.84	0.020	35	1,164.274	1,168.165	
		B6					305	1,163.911	1,165.860	
A-2	B 6	[175	14.7	1.00	0.024	50	1,163.911	1,165.860	
		B5	ז"				50	1,163.177	1,164.793	
	B5	.	175	12.0	0.91	0.022		1,163.177	1,164,793	
		64				ļ	118			
A-3	B4		225	3.5	0.58	0.023		1,162.360		
		B3	9″				92	1,162.037	1,162.507	
	83		225	2.7	0.51	0.020	107	1,162.037	1,162.507	
		B2					199	1,161.750	1,162.507	
	82	.	225	3.4	0.57	0.023		1,161.750		
		<u> B1</u>				<u> </u>	303	1,161.400	1,163.117	
	B1		225	5.5	0.73	0.029	20	1,161.400	1,163.117	Í
		N6				<u> </u>	323	1,161.290	1,162.812	
A4	N6		225	11.3	1.04	0.041	77	1,161,290	1,162.812	
		N5	9″					1,160.421	1,161.288	
	N5		225	25.4	1.56	0.062	90	1,160.421	1,161.288	
		N4					167	1,158.135	1,162.964	
	N4	ł	225	28.2	1.64	0.065	75	1.158.135	1,162.964	
	[N3					242	1,156.017	1,157.173	
	N3		225	33.8	1.80	0.072	68		1,157.173	
	L	N2					310	1,153.716	1,155.954	
	N2		225	10.0	0.98	0.039	108			
		N1	{				418		1,153.668	
A-5	N1	1	N/A	6.4			14		1,153.668	
		W24					14			
A-8	W24	T	375	27.7	2.29	0.253	63			
		W23	15″				63	1	T	1 ····································
	W23	T		20.4	1.96	0.217				
	1	W22		I	1	1			1 150.534	I
	W22	1	375	22.4	2.06	0.227				
		W21	1				230		1,148.746	
	W21	Î.	375	20.7	1.98	0.219				
		W20	1]			306			
• • • • • •	W20	1	375	24.8	2.17	0.239				1
	1	W19	1	1	1		370		1	1
	W19	<u> </u>	375	5.6	1.03	0.114			1	
.	1	W18	1	1X:¥	1	1	381	· · · · · · · · · · · · · · · · · · ·	1,145.671	

Table C-2 Inventory of Existing Trunk Sewer in the Eastern WTD

Sewer	M.H.	MH						Invert	Ground	
Line	No.	No.	Dia,	Slope	Velocity	Capacity	Length	Elevation	Surface	Remarks
No.		ļĮ						·····	Elevation	
			លាញ	0/00	m/S	m³/\$	m	m	m	
<u>A-7</u>	W18		450	11.0	1.63	0.259	75	1,143.998	1,145.671	
		W17			<u> </u>		- 15	1,143,173		
	W17		450	11.7	1.68	0.267	72		1,144.161	
		W16					147	1,142.333		
	W16		450	11.0	1.63	0.259	76	1,142.333	1,143,083	
		W15A					223	1.141.496	1,142,245	
	W15A		450	6.9	1.29	0.205		1,141.496	1,142,245	
	·	W15				0.004	235	1,141,413		
	W15		450	14.2	1.85	0.294		1,141,413	1,142,163	
	[W14			0.70	0.070	325	1,140,138		
A-8	<u>W14</u>		675	1.4	0.76	0.273		1,140,138		
	 	W13	27″	<u> </u>		0.070	86	1,140.019		
	W13		675	1.4	0.76	0.273		1,140,019		
ļ	}	W12				0.050	173			
	W12		675	1.2	0.71	0.252		1,139.901	1,140.901	
ļ	ļ	W11					259			
	<u>W11</u>		675	0.3	0.35	0.126				
ļ	<u> </u>	W10A				0.704	327	1,139.782	1.140.782	
A-9	WIOA	}	675	11.8	2.21	0.791		1,139.782		
		W10	27″	<u> </u>	0.70		10	1,139.664		
	W10		675	1.4	0.76	0.273		1,139.664		
L		W9			0.70	0.000	99	1,139.535		
	W9		675	1.5	0.79	0.282				
	1	W8		ļ	0.70	0.070	188			
	W8		675	t.4	0.76	0.273			1	
1		W7	0.35		0.01	0.000	280		1	
	W7		675	1.7	0.84	0.300				
	+	W6			1 0.7	0.252	370 2 94			
	W6		675	1.2	2 0.71	0.202	464			
	1.115	W5			0.74	0.07				
	W5	·	675	1.4	0.76	0.273	558	· · · · · · · · · · · · · · · · · · ·		
1	-	W4	671	1.4	0.70	0.27				
A-10	W4	1412	675 27″	· · · · · · · · · · · · · · · · · · ·	' <u> v./</u>		100			
	-	<u>W3</u>	21		0.7	6 0.27		the second s	1,139.744	
	W3	14/0	675	<u>.</u> 1.4	1 0.7	v.z/	200	1 1 3 8 60 9	1,139.603	1
 		W2	675	5 1.4	4 0.70	5 0.27;			1,139.603	
	W2	144.5	+ <u>0/3</u>	<u> </u>	' <u> ''</u>	<u> </u>	302			1
 		WI	+	+	+	+	+		1,100.10	<u> </u>
						•				
A1-1	052		225	5 -16.	,	1	20	1,164.612	1,165.86	5
<u>A'-1</u>	852	N7	9″	<u> </u>	:		20		1,163.83	
	NT.		9 22	5 48.	1 2.1	5 0.08				
	N7	N6	643		· · · · ·	·	90			
	+	-	<u> </u>	1	+	-	<u> </u>			-
			1		1					
L			1		. I					

Table C-2 Inventory of Existing Trunk Sewer in the Eastern WTD

Sewer	M.H.	MH	ļ					Invert	Ground	
Line	No.	No.	Dia.	Slope	Velocity	Capacity	Length	Elevation	Surface	Remarks
No.									Elevation	
			mm	0/00	m/S	m³/\$	m	<u>m 🗠 s</u>	m	
1-1	B14		225	4.8	0.66	0.026	116	1,165.240		
		B13	9″				116	1,164,710		
	B13		225	3.6	0.59	0.023	119		1,167.384	
		B12					235		1,166.012	
	B12		225	8.7	0.91	0.036			1,166.012	
	 	B11					330		1,164.946	
	811		225	25.5	1.56	0.062			1,164.946	
		N6					415	1,161.290	1,162.812	
										······
2-1	CA10		225						1,163.117	
	L	CA9	9″				ļ	1,160.967		
	CA9							1,160.967	1,163.726	
		CA8			ļ	}	 		1,160.678	
	CA8								1,160.678	
		CA7				ļ	L	the second s	1,161.440	
	CA7		.						1,161.440	
		CA6					<u> </u>	1,158.306	1,158.545	
	CA6	1						1,158.306		
		CA5	L					1,158.157		
	CA5							1,158.157	1,159.002	
		CA4								
	CA4							0.000	0.000	
		CA3			l					· ·
	CA3				<u> </u>			0.000	0.000	
		CA2					<u> </u>	1,157.599	1,156.716	
-	CA2		1					1,157.599	1,156.716	
		CA1						1,157.462	1,157,478	
	CA1		Į			1		1,157.462	1,157.478	
		C15			-			1,157.032	1,157.478	
	T	1								I
	[I								
C-2	C18		300)				1,158.163	1,158.545	5
		C15	12″					1,157.032	1,157.478	3
		Γ								
C-3	C15		375	5 4.	0.9	4 0.104	4 32	2 1,157.032	1,157.478	3
		C14	15″	1	<u> </u>		3	1,156.883	1,156.868	3
	C14		375	5 4.	1 0.8	8 0.09	7 74	1,156.883	1,156.868	3
		C13					10	6 1,156.578	1,156.86	3
	C13		37	5 3.	1 0.7	7 0.08	5 10	6 1,156.578	1,156.86	9
		C12	L			1	21	2 1,156.24	1,156.71	<u>5</u>
	C12		37	5 3.	2 0.7	8 0.08				
	1	011	Î	1	1	1	32		3 1,156.71	
	C11	1	37	5 3.	8 0.8	5 0.09		1		1
		C10	1		1		42			
	1	1	1	1	1		-1	1	T	
		- 		1	-	1	-41		1	Î

Table C-2 Inventory of Existing Trunk Sewer in the Eastern WTD

Sewer	M.H.	M.H.						Invert	Ground	
Line	No.	No.	Dia.	Slope	Velocity	Capacity	Length	Elevation	Surface	Remarks
No.									Elevation	
			៣៣	0/00	m/S	m³/S	m	m	m	
;-4	C20		N/A					1,158.297	1,159.154	
		C19						1,157.785	1,159.154	
	C19							1,157.785	1,159.154	
		C18						1,157.672	1,159,154	
	C18							1,157.672	1,159.154	
		C17						1,156.212	1,157.630	
)-5	C22		N/A					1,157.324	1,157.783	
		C21						1,156.608	1,157.783	
	C21					1		1,156.608	1,157.783	
		C17						1,158.212		
					<u> </u>	<u> </u>				
						1				
C-6	017	╂	N/A		<u> </u>	<u> </u>		1,156.212	1,157.630	
0-0	<u>C17</u>	010		 			-	1,155.523		
		C10	· · · ·			<u> </u>	ł	1,100.020	1,107.02.0	
	ļ									
	1						400		4457.000	<u> </u>
C-7	C10		375	3.9	0.86	0.095			1,157.326	
		<u>C9</u>	15″	ļ			100	1		
	C9		375	3.5	0.8	0.090				
		C8	ļ		<u> </u>	<u> </u>	211			
C~8	C8		375	3.4	0.80	0.089	114			
		C7	15″	<u> </u>			114			
	07		375	3.7	0.84	0.092	2 105	1,154.356	1,155.497	1
		C6					219	1,153.966	1,155.192	
	C6		375	5.3	3 1.00	0 11	1 52	1,153.966	1,155.192	}
		C5					271	1,153.688	1,155.192	1
	C5		375				27	1,153.688	1,155.192	1
· · - · · · · · · · · · · ·	1	C4A			1		298	N/A	N/A	l .
	C4A		375	<u></u>		1		N/A	N/A	
		C4	-		1		331		1,154.430	Į
	C4	1.	375	3.	0.8	4 0.09				
		СЗ		¥.				1,153.289	· · · · · · · · · · · · · · · · · · ·	
	102	<u>₩</u>	375	2	07	4 0.08		1,153.289		
	C3	100	1 3/0	5 2.	·	4 0.08	AA9	1,153,197	1 153 669	
	102	<u>C2</u>	375	j <u>3</u> .	1 07	7 0.08				
	C2		3/3	<u>, 3</u>	1 0.7	1 0.08		1,153.021		
	+	<u>01A</u>	+	+	1					
		-	375	5 3.	4 0.8	0.08				
		<u>C1</u>		. <u> </u>	+	1	574			
	C1		378	56.	1 1.0	7 0.11				
	4	<u>N1</u>	+	I			599	1,152.631	1,153.66	<u>*</u>
	.									
	1		<u> </u>	<u> </u>		_ _	<u>_</u>	<u> </u>	<u> </u>	<u> </u>
D-1	G1		22	5 5	1 0.7	0 0.02			5] 1,181.46	
		H45	9″		<u> </u>		5			5
	H45		22	5 2.	0 0.4	4 0.01	7 10	3 1,179.28	1,180.53	5
I		H44	Ī	1	1	1	16	5 1.179.074	1 180.64	3

Table C-2 inventory of Existing Trunk Sewer in the Eastern WTD

Sewer	M.H.	M.H.						Invert	Ground	
Line	No.	No.	Dia.	Slope	Velocity	Capacity	Length	Elevation	Surface	Remarks
No.						m³/S	· · ·		Elevation	
			mm	0/00	m/S		m	m	m	
D-2	H44		225	8.6	0.91	0.036	102	1,179.078		
		H43					267	1,178,196		
	H43		225	16.2	1.25	0.050	80	1,178.196		
		H42					347	1,176.901		
	H42		225	10.5	1.00	0.040	110	1,176.901	1,178,166	
		<u>H41 .</u>		<u> </u>			457	1,175.741	1,176.982	
	H41		225	12.6	1.10	0.044	110	1,175,741	1,176.982	
		H40					567	1,174.352	1,175.644	
	H40		225				66	1,174.352	1,175,644	
		H39					633	N/A	Ň/A	
	H39		225				52	N/A	N/A	
		H38					685	1,171.968	1,173.313	
	H38		225				59			
		H37					744	N/A	N/A	
	H37	1	225					N/A	N/A	
		H22					852		1,172.500	· · · · · · · · · · · · · · · · · · ·
D-3	H22	1	225	4.2	0.63	0.025				
		H21	9″		0.00	0.020	111	1,170,702		·····
	H21	112)	225	14.6	1.18	0.047	1	1,170,702		
	1 12 1	H20	220	14.0	1.10	0,047		1		· · · · · · · · · · · · · · · · · · ·
	H20	1120	0.05	+ 4 0	1 10	0.047	221	1,169.092		
	HZU		225	14.8	1.19	0.047		1,169.092		
<u> </u>		H19					332	1,167.448		
	H19		225	18.4	1.33	0.053		1,167.448		·····
		H18				· · · · · · · · · · · · · · · · · · ·	444			
	H18		225	41.3	1.99	0.079				
	.	H17					554	1,160.842		
	H17		225	51.8	2.23	0.089	110	1,160.842	1,163,117	
		H16	ļ			ļ	664			7
.	H16		225	25.4	1.56	0.062	110	1,155.142	1,157,553	
	[H15			.		774	1,152.353	1,154.233	·
	H15		225	25.9	1.57	0.063	110	1,152.353	1,154,233	
<u> </u>	L	H14					884	1,149.505	1,151.625	
ļ	H14	_	225	13.1	1.12	0.045	128	1,149.505	1,151.625	- 1
		H13	_				1012	1,147.822	1,149.471	
0-4	H13		300	8.9	1.12	0.079	60	1,147.822	1,149.471	
		H12	12″				60	E Contraction of the second se	1	
	H12	[300	6.3	0.94	0.067	T	1		
[[нп	{	[I	l	160			
	H11	Ţ	300	5.0	0.84	0.059			1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	
		HIO	1			1	270			
	H10		300	5.7	0.90	0.063	1			
		Н9		<u> </u>	<i></i>		380			
 	H9	1	300	4.6	0.80	0.057	3			
	1113	Н8		1.5	U.OU	0.03/				
	100	10	200	1	1.07	0.000	486			
l	H8		300	11.4	1.27	0.089				
L	1	<u> H7</u>	L	L	1	<u>i </u>	588	1,143.820	1,147.015	1

Table C-2 Inventory of Existing Trunk Sewer in the Eastern WTD

Sewer Line No.	M.H. No.	M.H. No.	Dia.	Slope	Velocity	Capacity	Length	Invert Elevation	Ground Surface Elevation	Remarks
			mm	0/00	m/S	m³/S	m	m	m	
D-5	H7		300	11.3	1.26	0.089	103	1,143.820	1,147.015	
		H6	12″				103	1,142.651	1,145.326	
	H6		300	12.5	1.33	0.094	102	1,142.651	1,145.326	
		H5					205	1,141.375	1,144.067	
D-6	H5		375	1.8	0.58	0.064	104	1,141.375	1,144.067	
		H4	15″				104	1,141.191	1,144.115	
	H4		375	11.4	1.47	0.162	104	1,141,191	1,144.115	
		НЗ					208	1,140.004	1,144.146	
	H3		375	-64.9		Ţ	105	1,140.004	1,144.146	
		H2					313	1,146.816	1,142.808	
	H2	Ţ	375	60.1	3.37	0.373	103	1,146.816	1,142.808	
		н					416	1,140.629	1,141.702	
· · · · · ·	HI		375	4.5	0.92	0.102	110	1,140.629	1,141.702	
		W14			I		526	1,140.138	1,141.138	
		1		[
					I		1			

Table C-2 Inventory of Existing Trunk Sewer in the Eastern WTD

										C C - AL	
M.H.	M.H.	<u>р</u> ,		M 1	A	1 + 1-	Invert	Ground Surface	Manhole Cover	Earth Cover	Remarks
No.	No.	Dia.	Slope	Velocity	Capacity	Lengu	Elevation		Elevation	COVER	I GINGINS
	ŀ	mm	0/00	m/S	m ³ /S	m	m	m	m	m	
120A		225	24.8	1.54	0.061	60			1,184.082		Slaughter
	110A	225	24.0	1.04	0.001				1,182.367		House
	1104	225	24.7	1.54	0.061				1,182.367	1 357	Sewer
<u>110A</u>	1104	223	24.7	1.34	0.001		1,178.952	· · · · · · · · · · · · · · · · · · ·		1.001	001101
1104	110A	0.05	047	1.54	0.061		1,178.952			1 322	[
110A		225	24.7	1.54	0.001		1,177.467			1.022	
	110A	0.05	017	1.57	0.061		1,177.462			1.347	
110A		225	24.7	1.54	0.001		· · · · · · · · · · · · · · · · · · ·		1.178.282	*	
	110A	0.05	170	1.00	0.051		1,175.982			1.	
110A		225	17.0	1.28	0.051		[0.007	
	110A	0.05	17.0	1.00	0.001				1,176.961	1 163	
110A		225	17.0	1.28	0.051				1.176.961		
	110A			4.00	0.054				1,175.990		
110A		225	17.0	1.28	0.051			······································	1,175,990		•••••
	110A		170	1.00	0.054				1.174.919		
<u>110A</u>		225	17.0	1.28	0.051				1,174,919		
	110A								1,173.848		
110A		225	17.0	1.28	0.051			· · · · · · · · · · · · · · · · · · ·	1,173,849		
	<u>110A</u>								1 172 927		· · · · · · · · · · · · · · · · · · ·
110A		225	9.9	0.97	0.039				1,172,927		
<u> </u>	110A		<u> </u>						1,172,221		
110A		225	9.9	0.97	0.039				1,172,221		
	100A								1,171.725		
100A		225	19.9	1.38	0.055				1,171.725		
	100A	L	.		L				1.170.429		<u>_</u>
100A		225	9.9	0.97	0.039				1,170.429		2
	100A		<u> </u>						2 1,169.873		
100A		225	i <u>9</u> .9	0.97	0.039				2 1,169.873		5
	100A		 		_				2 1,168.13		
1007		225	5 9.9	0.97	0.039	**************			2 1,168.13		2
	100A	÷	_	ļ					2 1,168.68		
100/		225	5 9.9	0.97	0.039				2 1,168.68		8
	<u>99A</u>			\vdash	I	the second s			7 1,168.18		
99/		22:	5 9.9	0.97	0.039		-1		7 1,168.18		9
	<u>98A</u>		<u> </u>	_	_	+			7 1,167.53		-
98/		22	5 9.9	0.97	0.03			an an antistant a subset of and an ad-	7 1,167.53		5
	97A								2 1,167.14		
97/			5 198.	4.3	6 0.17:			and a second s	2 1,167.14		6
	96A				<u> </u>		2 1,154.04				
96/		22	5 ####	#NUM	#NUM				2 1,166.08		2
	95A		<u> </u>	<u> </u>					2 1,165.92	_	
95/		22	5 -54.	7 #NUM	#NUM		9 1,163.62				2
	944	<u>\</u>	<u> </u>	<u> </u>	_	_		_	2 1,165.52	_	
94/	A	22	5 67.	8 2.5	5 0.10		a i e chine e ha na an an an an an an an an		2 1,165.52		71
	93/	-	1						2 1,165.88		_
93/	A	22	5 6.	5 <u>0.8</u>	0 · 0.03		1 1,162.98	ter beren biter ett inniger			4]
	92/	_	<u> </u>		1		5 1 162 58				
92	A	22	5 6.	6 0.8	0.03		0 1,162.58				9
	91/				Ì		5 1,162.19				
91	Ą	22	56.	6 0.8	0.03	2 6	0 1,162.19	2 1,164.18	1 1,164.89	2 1.73	6
L	90/				1	1,47	<u>5 1,161.79</u>	9 1,164.04	8 1,164.54	9	

Table C-3-(1) Inventory of Existing Sewer in the Eastern WTD based on Longitudinal Survey (Slaughter House Sewer)

							Invert	Ground	Manhole	Earth	
M.H.	M.H. No.	Dia.	Sional	Velocity	Canacity	Length	Elevation	Surface	Cover	Cover	Remarks
No.	NO.	Ula.	Siope	VEIDUILY	Oapaony	Longar	Lioradon	Elevation		~~~~	
	ł	mm	0/00	m/S	m³/\$	m		m	m	m	
		_	-10.1	#NUM!	#NUM!		1,161.799		the second s		
<u>90A</u>		225	-10.1	#INOM:	#NUM:		1,162.406	*************************			
	89A			4 40	0.050						
<u>89A</u>		225	23.2	1.49	0.059		1,162.406				
	88A						1,161.013				
88A		225	6.5	0.79	0.031		1,161.013				
	87A						1,160.738				
87A		225	6.6	0.80	0.032		1,160.738				
	86A		l	;		1	1,160.607			· · · · · · · · · · · · · · · · · · ·	
86A		225	6.6	0.80	0.032		1,160.607				
	85A			L			1,160,214				
85A		225	6.6	0.80	0.032		1,160.214				
	84A		[1.777	1,159.821	1,162.313	1,162.951		
84A		225	6.6	0.80	0.032	61	1,159.821	1,162.313	1,162.951	2.239	<u>[</u>
in The Cher	83A		1			1,838	1,159.421	1,161.808	1,162.221		
83A		225	6.6	0.80	0.032	60	1,159.421	1,161.808	1,162.221	2.134	
	82A				ļ	1,898	1,159.028	1,161.513	1,162.128		
82A		225	6.6	0.80	0.032	41	1,159.028	1,161.513	1,162.128	2 2 3 2	
	81A		1				1,158.759	1.161.353	1,161.689		
81A		225	7.5	0.85	0.034				1,161.689		
017	80A		· · · · · ·				1,158,438				
80A		225	5 7.1	0.82	0.03		1 158 438	-			
1 80A	79A		<u>'</u>	0.02	1 0.00		1,158.056				
104			5 7.8	0.86	0.034				1,161.05		1
794		225	<u>/ </u>	0.00	0.00		1,157.643				
	78A		5 7.8	3 0.80	0.03				3 1,160.72		1
78/		22	/ /.c	0.00	1 0.03		1,157.261				
	77A		+	3 0.86	3 0.03	- 1	1.157.261				2
11/		22	5 7.8		0.03		1,156.793			-	
	764	_			#NUM	_	1,156.793				,
76/		22	5 -1.3	3 #NUM!	#NUM:						· · · · · · · · · · · · · · · · · · ·
ļ	754		<u></u>			2,242		5 1,159.5			2
75/		22	5 6.	5 0.79	0.03		A				
	74/		<u> </u>	Ļ		2,285		7 <u>1.159.1</u>			1
74/	<u> </u>	22	56.	4 0.7	B 0.03						4
	73/		- 		_	2,32		1,159.2			
73	A	22	5 6.	7 0.8	0 0.03						/
	72/		· _ ·		_	2,38					
72		22	5 13.	0 1.1	2 0.04		• • • • • • • • • • • • • • • • • • •				/
	71/		÷		_	2,44					
71	A	22	5 0.	0.0	0 0.00						
	70/	A				2,50					
70	A	22	56.	5 0.7	9 0.03						7
	69/					2,56					_ _
69		22	5 2	3 0.4	7 001	96	0 1.154.7	3 1,158.6	5 1,158.8	4 3.66	7
	68		l			2,62	7 1,154.5	9 1,157.6	6 1,159.0)4	
68		22	5 4	7 0.6	7 0.02			9 1,157.6	6 1,159.0	4 2.81	7
Ì	67					2,68				2	
67		22	5 4	.6 0.6	6 0.02		5 1,154.3				17
	66					2,72					
66			5 4	7 0.6	0.02		1 1,154.1				7
00	65				·····	2,77					1
L	1 00	<u>^</u>	t				-1 -1100.0				

Table C-3-(1) Inventory of Existing Sewer in the Eastern WTD based on Longitudinal Survey (Slaughter House Sewer)

C-22

M.H. No.	M.H. No.	Dia.	Slope	Velocity	Capacity	Length	Invert Elevation	Ground Surface Elevation	Manhole Cover Elevation	Earth Cover	Remarks
		mm	0/00	m/S	m³/S	m	m	m	m	m	
65A		225		0.66	0.026	: 50	1,153.91	1.159.03	1,159.51	4.867	
	64A					2,823	1,153.68	1,158.71	1,159.28		
64A		225	2.3	0.47	0.019	-44	1,153.68		1,159.28	4.777	
	63A					2,867	1,153.58		1,159.48		
63A		225	2.2	0.46	0.018		1,153.58	1.158.32	1,159.48	4.487	
	62A					2,917	1,153.47	1,158.33	1,159.17		
62A		225	2.2	0.46	0.018	51	1,153.47	1,158.33	1,159.17	4.607	
	61A					2,968	1,153.36	1,157.81	1,158.76		
61A		225	2.3	0.47	0.019	56	1,153.36	1,157.81	1,158.76	4.197	
	60A					3,024	1,153.23	1,158.57	1,159.00		
60A		225	2.2	0.46	0.018	50	1,153.23	1,158.57	1,159,00	5.087	
	59A					3,074	1,153.12	1,158.16	1,157.84		
59A		225	2.3	0.47	0.019	52	1,153.12	1,158.16	1,157.84	4.787	
	58A					3,126	1,153.00	1,157.75			
58A		225	2.2	0.46	0.018	50	1,153.00			4.497	
	57A					3,176					
57A		225	2.4	0.48	0.019	P			A CHARLES AND A CHARLE AND A CHARLE AND A CHARLE AND A CHARLE AND A CHAR		
	56A					3,226					
56A		225	2.2	0.46	0.018						
	55A		L			3,286					<u></u>
55A		225	2.3	0.47	0.019			1,157 23		and the second second	• • • • • • • • • • • • • • • • • • • •
	_54A		ļ			3,346					
54A		225	2.2	0.46	0.018			T	T		.
	53A		ļ		ļ	3,396					
53A		225	2.2	0.46	0.018						
	52A		.		}	3,456					_
52A		225	4.0	0.62	0.025						<u> </u>
<u> </u>	<u>51A</u>		 	<u> </u>	ļ	3,516					<u> </u>
<u>51A</u>		225	4.0	0.62	0.025		T				
İ	50A		_			3,576					<u> </u>
<u>50A</u>		225	3.8	0.60	0.024						
	<u>49A</u>		·			3,639					
49A		225	3.8	0.60	0.024	T				3.687	
	<u>48A</u>		<u> </u>			3,699					
48A		225	-5.8	#NUM!	#NUM!	62					
h	<u>47A</u>		+	.		3,761					+
<u>47</u> A		225	13.8	1.15	0.046		a land the set of the	and the second sec	Turrent to be the second		
	<u>46A</u>	· · · ·	<u> </u>	_		3,821	1				<u> </u>
474		225	5 17.8	1.31	0.052				· · · · · · · · · · · · · · · · · · ·		-
{	<u>46A</u>		<u> </u>	<u> </u>	1	3,821	1,150.60	0 1,153.85	1,154.90	4	
		con	nects	to the Kil	os Trunk	(Sewer			.		
	1		<u> </u>	1	<u> </u>	1	<u> </u>		1	1	1

Table C-3-(1) Inventory of Existing Sewer in the Eastern WTD based on Longitudinal Survey (Slaughter House Sewer)

M U	6411				· · · · · ·		Invert	Constraint 1	Markala	C. d.	·
M.H. No.	M.H. No.	Dia.	Siene	Valasitu	Conneitu	Longth	Elevation	Ground	Manhole	Earth	Remarks
NO.	110.	Dia.	Siobe	VOICELY	Capacity	Laugui	Clavation	Surface Elevation	Cover Elevation	Cover	nemarks
		mm	0/00	m/S	m ³ /S	m '	m	m	m m	m	
1B		225	2.7	0.51	0.020	60	1.157.76	1,160.23	1,161.41	2.22	Kibos
	2B				V.ULV	60	1,157.60		1,161.15	******************	Trunk
2B	20	225	3.3	0.56	0.022	60	1,157.60		1,161.15		Sewer
20	3B	220	0.0	0.00	0.022	120	1,157.40		1,160,40		Sewer
3B	<u>эр</u>	225	2.8	0.52	0.021	57	1,157.40		1,160,40	2.56	
<u>sp</u>	4B	225	2.0	0.32	0.021	177	1,157.40			2.30	
4B	40	225	2.6	0.50	0.020	53	1.157.24	1,160.33	1,160.77	2.04	
4D	5B	223	<u>2.0</u>	0.00	0.020	230		- and a second		2.84	
6 D	эв	205	0.1	0.51	0.020				1,160.45		
5B	<u>~</u>	225	2.7	0.51	0.020	63			1,160.45		· • • • • • • • • • • • • • • • • • • •
<u></u>	6B	005	0.7	0.51	0.000	293	5	-	1,160.18		
6 B		225	2.7	0.51	0.020	63	* ••••••••••••••••••••••••••••••••••••		1,160.18		
	7B			0.54	0.000	356					
7 <u>8</u>		225	2.7	0.51	0.020	62			1,160.26	3.05	
	8 <u>B</u>					418					
8B		225	2.7	0.51	0.020					2.61	
ļ	9B					478			1,159.80		
98		225	2.7	0.51	0.020	26	· · · · · · · · · · · · · · · · · · ·		1,159.80		
	10B					504			1,159.98		
10B		225	2.7	0.51	0.020				1,159.98	I	
L	118		· · ·			564	1,156,20				
11B		225	2.7	0.51	0.020	60	1,156.20	1,159.63	1,158.45	3.18	
Ŀ	128					624	1,156.04	1,159.55	1.160.24		
128		225	2.8	0.52	0.021	60	1,156.04	1,159.55	1,160.24	3.26	
	138				L	684	1,155.87	1,159.35	1,160.37		
13B		225	2.7	0.51	0.020	60	1,155.87	1,159.35	1,160.37	3.23	
	14B					744	1,155.71	1,159.39	1,160.22		
14B		225	2.7	0.51	0.020	60	1,155.71	1,159.39	1,160.22	3.43	
F	15B		I			804	1,155.55	1,159.22	1,159.39	I	
15B		225	12.3	1.09	0.043	60	1,155.55	1,159.22		1	
	16B					864	1,154.81				
16B		225	1.2	0.34	0.013						
	178	· · · · · · · · · · · · · · · · · · ·				924					
17B	<u> </u>	225	0.8	0.28	0.011		1				
	18B					984					
18B	1	225	-5.2	#NUM!	#NUM!	60					
	19B		<u> </u>			1,044					
19B	1	225	8.0	0.88	0.035	<u> </u>		1		1	
1.00	208		1	0.00	0.000	1,104		.T			
20B		225	1.2	0.34	0.013	T					,
1.00	21B	220	1.4	0,04	0.013	1,156	······································				
21B	1 <u>~10</u>	225	1.2	0.34	0.013						
1210	22B	<u> </u>	1.2	0,34	0.013						
220	1220	007	1 1 1	0.00	0.010	1,216			Les de la construcción de la con		,
22B	200	225	1.2	0.34	0.013						
0.00	<u>23B</u>			0.07		1,276					
23B	-	225	1.3	0.35	0.014						-
	<u>248</u>	+	·			1,336					
24B	_	225	1.2	0.34	0.013	T					2
	258	1	<u> </u>	<u> </u>		1,396					<u> </u>
25B		225	i ļ 1.1	0.32	0.013						4
L	26B	I	<u>i</u>		<u> </u>	1,449	1,154.11	1,156.87	1,157.61		<u> </u>

Table C-3-(2) Inventory of Existing Sewer in the Eastern WTD based on Longitudinal Survey (Kibos Trunk Sewer, Upstream)

M.H.	M.H.						Invert	Ground	Manhole	Earth	
No.	No.	Dia.	Slope	Velocity	Capacity	Length	Elevation		Cover	Cover	Remarks
					- 1 (0				Elevation	· · · · · · · · · · · · ·	
		mm	0/00	m/\$	m³/S	m	- m	<u> </u>	m	m	
26B		225	1.2	0.34	0.013	60	1,154.11	1,156.87	1,157.61	2.51	.,,
	<u>27B</u>					1,509	1,154.04		1,157.71		
278		225	1.3	0.35	0.014	60	••••••••••••••••••••••••••••••••••••••		1.157.71	2.39	
	28B	· ·				1,569	1,153.96		1,157.72		
28B		225	1.2	0.34	0.013	60	1,153.96		1,157.72	2.20	
	29B					1,629			1,157.34		
298		225	1.2	0.34	0.013	60		· Personal and an entre state of the second	1,157.34	2.23	
	<u>308</u>					1.689			1,157.80		
30B		225	1.1	0.32	0.013		1,153.82		1 157.80	2.00	
	31B					1,750			1,157.81		
31B		225	1.2	0.34	0.013	60			1,157.81	3.71	
	<u>328</u>					1,810			1,157.67		
328		225	1.3	0.35	0.014		······		1,157.67	·····	
	33B					1,870			1,157.60		
338		225	1.2	0.34	0.013		· · · · · · · · · · · · · · · · · · ·		1,157.60	····	ļ
	34B	0.05				1,912			1 157.35		
<u>34B</u>		225	1.2	0.34	0.013	60			1,157.35		
air o	35B	005			0.040	1,972					· · · ·
358		225	1.2	0.34	0.013			· · · · · · · · · · · · · · · · · · ·			}
	<u>36B</u>	0.05	1		0.010	2,032					
<u>36B</u>	010	225	1.2	0.34	0.013	·					
070	37B			0.04	0.010	2,092			1,157.01	3.28	
37B		225	1.2	0.34	0.013				1,157.01		
200	<u>38B</u>	005				2,152					<u> </u>
388	39B	225	-32.0	#NUM!	#NUM!	60		** ***********			
200	330		42.0	1 202	0.001	2,212					·
<u>398</u>	408	225	43.0	2.03	0.081					· · · · · · · · · · · · · · · · · · ·	
40B	1400	225	8.9	0.92	0.037	2,272					<u> </u>
TUD	418	1 223	0.3	0.92	0.037	2,335		******************************			
41B	1410	225	25.2	1.55	0.062						
	42B		1 23.4	1.50	0.002	2,398				• • • • • • • • • • • • • • • • • • • •	
42B	1420	225	5 -5.7	#NUM!	#NUM!	2,390					
1720	46A	223	<u>,, /</u>	*1103/1	HINOM:	2,46				·	
}		1		L	<u> </u>		1	1 1,104.40	1,100.14	·	┨━━━━━━
1	Join	t with i T	the Slau	ughter Ho	use Trun' I	k Sewer				-	
L		L	1	<u></u>	<u> </u>	<u> </u>	<u> </u>	<u>i</u>	<u> </u>	<u> </u>	<u> </u>

 Table C-3-(2) Inventory of Existing Sewer in the Eastern WTD

 based on Longitudinal Survey (Kibos Trunk Sewer, Upstream)

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A.H.	M.H.	Dia.	Slone	Velocity	Canacity	Length	Invert Elevation	Ground Surface	Manhole Cover	Earth Cover	Remarks
No.	No.	via.	Siohe	ABIOOILÀ	Capacity	Longui	LIVERUUT		Elevation		
	ľ	mm	0/00	m/S	m³/S	m	m	m	m	m	
45A		225	102.1	3.13	0.124	61	1,156.60	1,153.85	1,154.90	-3.00	Joint
	44A					61	1,150.37	1,153.50	1,154.58		with the
44A		225	3.8	0.60	0.024	60	1,150.37	1,153,50	1,154.58	2.88	Slaughter
	43A					121	1,150.14	1,153.29			House
43A		225	4,0	0.62	0.025	60	1,150.14	1,153.29		2.90	Sewer
	42A					181	1,149.90				
42A		225	3.3	0.56	0.022	60	1,149.90				
	<u>41A</u>				0.001	241	1,149.70				
41A		225	3.8	0.60	0.024		1,149.70 1,149.47				
	40A					301	1,149.47	1,192.00	1,100.17		
· · · · · ·											
40A		300	3.9	0.74	0.052	44	1,149.47	1,152.06	1,153.17	2.26	<u> </u>
404	39A	300			0.002	44		1,151.49			
39A	4	300	3.9	0.74	0.052						
0.011	38A					101	1,149.08				
38A	-	300	3.8	0.73	0.052				1,152.50	2.24	
	37A					161	1,148.85	1,151.13	1,152.19		
37A		300	3.8	0.73	0.052	60	1,148.85	1,151,13			
•••••••	36A					221					
36A		300	4.(0.75	0.053						
	35A	ļ		1	<u> </u>	281		1,151.0			
35A		300	3.8	3 0.73	3 0.052						
	<u>34A</u>	_				34		1,151.0			
34A		300) 3.8	3 0.7:	3 0.052						•
	<u>33A</u>				0.05	410) 1,150.9) 1,150.9			<u></u>
33A		300) 4.0	0.7	5 0.053	3 60		*****			2
20.4	32A	300	3.0	8 0.7	3 0.05;						8
32/	31A		<u>,</u>	<u>v. r.</u>	si <u>v.</u> v.	53		0 1,150.6			
31/		300	3	8 0.7	3 0.05						5
	30A		<u> </u>			59		7 1,150.7			
30/		30	0 3	5 0.7	0 0.05						1
	29/	1				66		4 1,150.3	0 1,151.1		
29/		30	0 3.	8 0.7	3 0.05		5 1,146.9	4 1,150.3	0 1,151.1	4 3.0	3
	28/					71	6 1,146.7	3 1,1 <u>50.2</u>			
28/	A	30	0 3.	8 0.7	3 0.05	26	0 1,146.7				8
	27/					17					<u>_</u>
27		30	0 3	9 0.7	4 0.05						6
	26/	_			_	83					.
26		30	0 4	2 0.7	7 0.05		2 1,146.2				
<u> </u>	25/		<u>_</u>			84					6
25		30	<u>v 4</u>	0 0.7	5 0.05		0 1,146.2				
	24/			1	2 0.05	87	5 <u>1,146.1</u> 0] 1,146.1				4
24		30	<u>N 3</u>	.7 0.7	2 0.05	90					
	23/		<u>n</u>	.8 0.7	3 0.05		0 1,146.0				4
23		30	<u>~3</u>	.8 0.7	0.00	96					
20	22	30		.0 0.7	5 0.05		0 1,145.7				4
22	21		~~~~~ ⁷	······································		1,02					
I		<u> </u>	-								

Table C-3-(3) Inventory of Existing Sewer in the Eastern WTD based on Longitudinal Survey (Kibos Trunk Sewer, downstream)

M.H. No.	M.H. No.	Dia.	Slope	Velocity	Capacity	Length	Invert Elevation	Ground Surface Elevation	Manhole Cover Elevation	Earth Cover	Remarks
		mm	0/00	m/S	m³/S	m	m	m	m	m	
21A		300	3.7	0.72		40			1,148.63	2.27	· · · · · · · · · · · · · · · · · · ·
	20A			·····		1,065	1,145.38		1,148.06		
20A	201	300	3.8	0.73	0.052	60			1,148.06	1.77	
207	19A	300	3.0	0.75	0.002						• • • • • • • • • • • • • • • • • • • •
104	TAM	300		0.75	0.050	1,125					
<u>19A</u>		300	4.0	0.75	0.053	35		1,146.80	1,147,36	1.32	
	18A					1,160		1,146.52	1,146.76		:
<u>18A</u>		300	3.4	0.69	0.049	35		1,146.52	1,146.76	1.18	
	17A					1,195				•	
17A		300	14.7	1.44	0.102	38			1,148.76	1.21	
	16A			• •		1,233			1,146.58		· . ·
16A		300	-2.3	#NUM!	#NUM!	65	1,144.33	1,146.31	1,146.58	1.65	
	15A					1,298	1,144.48	1,146.28	1,146.71	3	
15A		300	4.6	0.80	0.057	65	1,144.48	1,146.28	1,146.71	1.47	
	14A					1,363	1,144.18		1,147.06		
14A		300	3.8	0.73	0.052	65	1,144.18				
	13A				***********	1,428			1,147.73		
13A		300	3.8	0.73	0.052	65	1,143.93			2.84	
	12A					1,493					
12A		300	14.0	1.40	0.099	65	1,143.68			1.03	
	11A			1.79	0.033	1,558	**********************			1.03	
11A		300	141	1.41	0.100					0.00	
	101	300	14.1	1.41	0.100	64					••••••••••••••••
104	10A					1,622		1,143.68			
10A		300	2.1	0.54	0.038	63	1,141.87		**********************	**********	
	<u>9</u> A					1,685		1,143.17			
<u>9</u> A		300	2.2	0.58	0.039		********************				
	<u>88</u>					1,750					
88		300	2.1	0.54	0.038	121	1,141.60		1,142.35	0.17	
	<u>7</u> A		[L		1,871	1,141.34	1,141.20	1,142.30		
7A		450	2.1	0.71	0.113	121	1,141.34	1,141.20	1,142.30	-0.63	
	6A			Ι		121	1,141.08	1,140.36			
6A		450	2.1	0.71	0.113				1,141.72		:
	5A			[242	1,140.83	1,140.05	1,141.93		[].
5A		450	2.1	0.71	0.113		the second s	1,140.05		1	
	4A		[·····	<u> </u>	1	363		1,139.57			
4A		450	2.1	0.71	0.113						
	3A		••••••••••••••••••••••••••••••••••••••	<u></u>	V.113	484	*****************				
ЗA		450	21	071	0.110						
JA.		400	2.1	071	0.113				***********************************		
	<u>2</u> A			t		605		1,140.13			
2A		450	2.3	0.75	0.119			1,140.13			Nairobi
	1A	ł.	I	<u>l</u>		689	1,139.87	1,140.23	1,140.88		Road

 Table C-3-(3) Inventory of Existing Sewer in the Eastern WTD

 based on Longitudinal Survey (Kibos Trunk Sewer, downstream)

λĦ.	M.H.	T					Invert	Ground	Manhole	Earth	
No.	No.	Dia.	Slope	Velocity	Capacity	Length	Elevation	Surface	Cover	Cover	Remarks
	ŀ		0/00	m/S	m³/S	m	m	Elevation m	Elevation m	m	
120		mm 150	5.3	0.54	0.010		1,152.573	1,153,413	and the second secon	the second s	Kisumu
<u>129</u>	N28	150	0.0	0.34	0.010	61	1,152.249	1,153.669			Chistian
N28	1120	150	5.3	0.54	0.010	· · · · · · · · · · · · · · · · · · ·		1,153.669	1,154.249		Center
120	N27	100	0.0		0.010	121		1,153.943		••••••••••••••••••••••••••••••••••••••	
N27		150	5.3	0.54	0.010			1,153.943			
	N26						1,151.636	1,154.206	1,154.736		
N26		150	5.2	0.54	0.010	37	1,151.636	1,154,206	1,154.736	2.39	
	N25					214		1,154.331			
N25		150	5.3	0.54	0.010			1,154.331			
	N24							1,154,541			
N24		150	5.3	0.54	0.010			1,154.541			
	N23							1,154.771			
N23		150	5.3	0.54	0.010			1.154.771			
	N22					359	1,150.676	<u>1,155.536</u>	1,100.470	' 	
1100		0.05	10.0	0.98	0.020	32	1 150 676	1,155.536	1 155 476	4.61	
N22		225	10.0	0.98	0.039			1,155.161			
N21	<u>N21</u>	225	10.1	0.98	0.039				· · · · · · · · · · · · · · · · · · ·		<u> </u>
NZI	N20		10.1	0.50	0.000	87		1,152.030			
N20		225	10.1	0.98	0.039			1,152.030			
nzu	N19		10.1	0.00	0.000	155		1.151.780			
N19		225	10.1	0.98	0.039			1,151,780			
	N18							1,151.864			
N18		225	10.1	0.98	0.039			1,151.864			
	N17					251	1,148.152				ļ
N17		225	10.0	98.0	0.039) 44	1,148.152				
	N16					295	5 1,147.710		1,152.360		
NI		225	83 5	2.83	0.112				1,152.36		
	N15			ļ	<u> </u>		1,144,454) 1,152,260		
NI		225	-70.4	I #NUM!	#NUM!				1,152 26		2
	<u>N14</u>				 		3 1,147.199		5 1,151,54		<u> </u>
N14	!	225	5 <u>39.</u> 9	1.95	0.07		0 1.147.199		5 1,151.54		along
	NI			<u></u>				6 1.150.67			the Ring
<u>N1</u>		22	5 -27.9	HNUM!	#NUM!			6 1,150.67 6 1,150.00			Road
	N12			1 0.00	- 0.00			5 1,150.00			2
NI	2 N1	22	5 42.9	€ 2.03	8 0.08			5 1,149.64			
NI	_	22	5 -28.	5 #NUM!	#NUM			6 1,149.64			3
14.1	NIC		J - 20		WINOW:			3 1,149.10			
NI		22	5 6.	5 0.79	0.03			3 1,149.10			5
	N	*****	Ť Š		1		a sea a sea de ser y ser es da se da se da se	3 1,148.70			1
N	- <u>t</u>	22	5 6.	5 0.79	0.03			3 1,148.70			1
	N		1		1			3 1,148.65		*****	
N		22	5 6.	6 0.80	0.03			3 1,148.65			2
	N				L			6 1.147.37			
N	-	22	5 6.	5 0.79	0.03			6 1,147.37			1
	N					82	2 1,144.25	9 1,147.25	5 1,147.40	9	
N		22	5 6	6 0.8	0.03			9 1,147.25			4
· · · · · · ·	N		I	1		84	5 1,144.10	8 1,147.21	5 1,147.35	8	

Table C-3-(4) Inventory of Existing Sewer in the Eastern WTD based on Longitudinal Survey

M.H. No.	M.H. No.	Dia.	Slope	Velocity		Length	Invert Elevation	Ground Surface Elevation	Manhole Cover Elevation	Earth Cover	Remarks
		mm	0/00	m/S	m³/S	m ·	m	- 1. m - 1.	ent m anna	m	
N5		225	6.6	0.80	0.032	178	1,144.108	1,147.215	1,147.358	2.85	
	N4					1,023	1,142,942	1,143.200	1.144.565		
N4		225	6.5	0.79	0.031	55	1,142.942	1,143.200	1,144.565	0.01	
	N3					1,078	1,142.582	1,143.325	1,143,882		
N3		225	6.6	0.80	0.032	40	1,142.582	1,143.325	1,143,882	0.49	
	N2					1,118	1,142.320	1,142.820	1,143.480		
N2		225	6.6	0.80	0.032	58	1,142.320	1,142.820	1,143.480	0.25	Nairobi
	Nĭ	I	I			1,176	1,141.940	1,143.012	1,143,270		Road

Table C-3-(4) Inventory of Existing Sewer in the Eastern WTD based on Longitudinal Survey

Table C-4	Estimated Capacity of Existing Sewers in Easte	rn WTD
	- Domination Chefferry of Dalithie Conceasing Dance	

	Estimated	Required
Sewers	Capacity	Pipe Capacity
	(m ³ /s)	(m ³ /s)
Staughter House Sewers	$0.018 \sim 0.061$	$0.038 \sim 0.456$
	(mainly 0.032)	
Kibos Trunk Sewers		
Upper Stream Section	$0.013 \sim 0.020$	$0.196 \sim 0.456$
Down Stream Section	$0.024 \sim 0.113$	$0.570 \sim 0.628$
Ring Road Sewers	$0.031 \sim 0.039$	$0.096 \sim 0.110$
N (D (ID) O (

Note: Required Pipe Capacity is the design flow rate + allowance(100%)

JICA STUDY ON KISUMU WATER SUPPLY AND SEWERAGE SYSTEM

