

Japan International Cooperation Agency (JICA)

Department of domigation and Drainage (DID)
Malaysia

# THE STUDY ON S

MODERNIZATION OF IRRIGATION
WATTER MANAGEMIENT SYSTEM

IN THIE GRANARY ARIEAS OF

PENINSULARMAILAYSIA

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ANNEXES

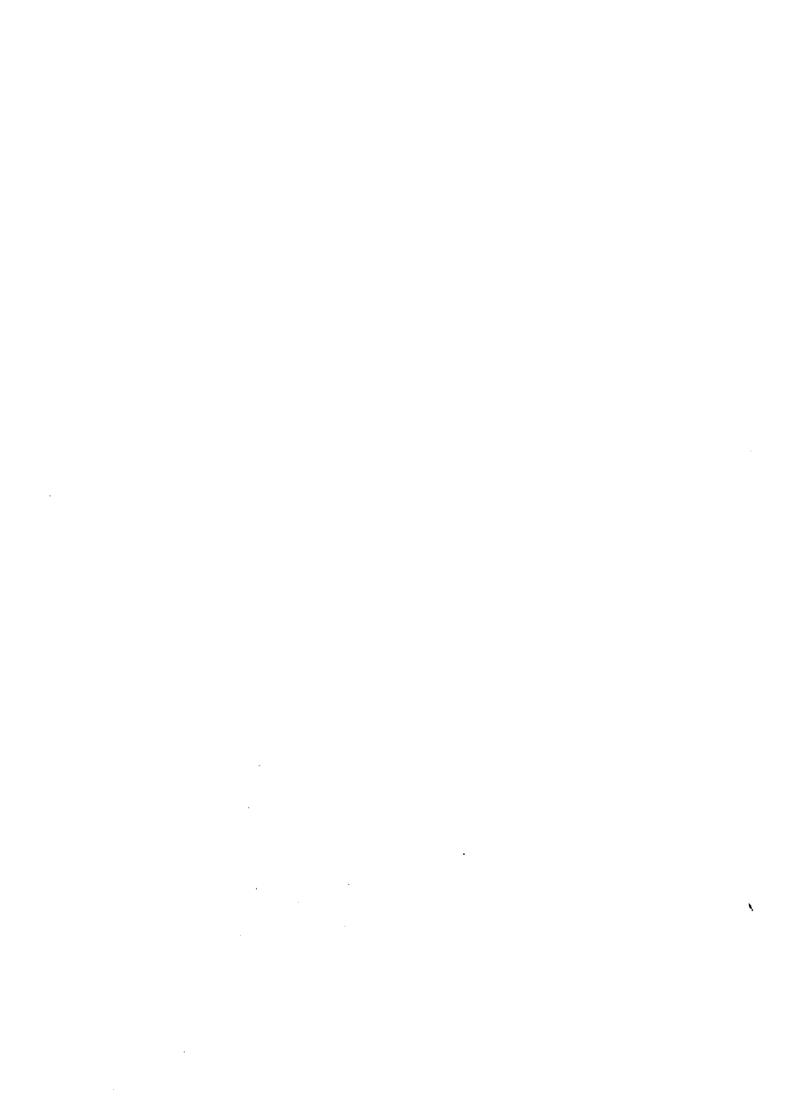
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July 1998

Nippon Koei Co., Ltd.

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Japan International Cooperation Agency (JICA)

Department of Irrigation and Drainage (DID) Malaysia

# THE STUDY ON MODERNIZATION OF IRRIGATION WATER MANAGEMENT SYSTEM IN THE GRANARY AREAS OF PENINSULAR MALAYSIA

Volume-II

**ANNEXES** 

July 1998

Nippon Koei Co., Ltd.

# LIST OF REPORTS

# Volume - I MAIN REPORT

# Volume - II ANNEXES

ANNEX - I WATER BALANCE STUDY

ANNEX - II RRIGATION AND DRAINAGE

ANNEX - III WATER MANAGEMENT AND OPERATION AND

MAINTENANCE

ANNEX - IV AGRICULTURE AND AGRO-ECONOMY

ANNEX - V FARMERS' ORGANIZATION

ANNEX - VI ENVIRONMENT

ANNEX - VII COST ESTIMATE

ANNEX - VIII PROJECT EVALUATION

ANNEX - IX PILOT PROJECT

# **CURRENCY EQUIVALENT**

US\$ 1.0 = RM 4.4 = Yen 129.5 (as of January, 1998)

# ANNEX-I WATER BALANCE STUDY

# ANNEX - I

# WATER BALANCE STUDY

# **Contents**

	GENE	RAL	
	METH	ODOLOGY	
	2.1	Available Water for Irrigation	
	2.2	Irrigation Water Requirement	
		LT OF WATER BALANCE STUDY IN THE	
	FIVE(	5) GRANARY AREAS	
	3.1	Pulau Pinang Scheme	
	3.2	Kerian Scheme	
	3.3	Sungai Manik Scheme	I
	3.4	Seberang Perak Scheme	I
	3.5	Kemasin / Semerak Scheme	I
	3.6	Ketara(Besut) Scheme	]
٠.	RESU	LT OF WATER BALANCE STUDY AND PROPOSED PLAN	1
	ADDI	TIONAL STUDY FOR THE FUTURE WATER SHORTAGE	1

# List of Tables

Table I-1	Inventry of Principal Meteorological Station Used for the Study	I-T-1
Table I-2	Areal Rainfall of Pulau Pinang Scheme	I-T-1
Table I-3	Areal Rainfall of Kerian Scheme	I-T-1
Table I-4	Areal Rainfall of Sungai Manik Scheme	I-T-2
Table 1-5	Areal Rainfall of Seberang Perak Scheme	I-T-2
Table I-6	Areal Rainfall of Kemasin / Semerak Scheme	I-T-3
Table I-7	Areal Rainfall of Besut Scheme	I-T-3
Table I-8	River Discharge of Kurau River	I-T-4
Table I-9	River Discharge of Kerian River	I-T-4
Table I-10	River Discharge of Batang Padang River	I-T-5
Table I-11	River Discharge of Kemasin River	I-T-5
Table I-12	River Discharge of Besut River	I-T-6
Table I-13	River Discharge of Angga River	I-T-6
Table I-14	Projected Domestic and Industrial	1000
m 11. 1.16	Water Demand for Relevant Rivers Estimation of Domestic and Industrial Water Demand	1-T-7
Table I-15	of Perak River (1/2 - 2/2)	I-T-8
Table I-16	Inventry of Secondary Granary Areas	
	in Relevant River Basin	
Table I-17	Tributary Catchment Area Factor of Relevant Rivers	I-T-11
Table I-18	River Maintenance Flow to be Used for Water Balance Study	I_T_11
Table I-19	Inflow of Bukit Merah Reservoir (Kurau River) in Present Condition	
Table 1-20	Available Water of Kerian River in Present Condition	
Table I-21	Available Water of Batang Padang River in Present Condition	
Table I-22	Available Water of Kemasin River in Present Condition	
Table I-23	Available Water of Besut River in Present Condition	
	(1/2 - 2/2)	. I-T-16
Table I-24	Available Water of Angga River in Present Condition	. I-T-18
Table I-25	Inflow of Bukit Merah Reservoir (Kurau River)	
	in Future Condition (1/2 - 2/2)	. 1-1-19
Table I-26	Available Water of Kerian River in Futrue Condition	1.00.01
Table I-27	(1/2 - 2/2)Available Water of Batang Padang River	
radio i 27	in Future Condition (1/2 - 2/2)	. I-T-23
Table I-28	Available Water of Kemasin River in Future Condition	
	(1/2 - 2/2)	. I-T-25
Table I-29	Available Water of Besut River in Future Condition	
	(1/3 - 3/3)	. I-T-27
Table I-30	Available Water of Angga River in Future Condition	
200000000000000000000000000000000000000	(1/2 - 2/2)	I-T-30
Table I-31	Sample of Paddy Field Water Balance Study to	
,	Ectimate Effective Painfall	I-T-32

Table I-32	Relevant Data Used for Estimating Water Requirement
	(Case 1, Case 2)
Table I-33	Relevant Data Used for Estimating Water Requirement
	(Case 3)
Table I-34	Planting Area (Case 1, Case 2)
Table I-35	Planting Area (Case 3)
Table I-36	Breakdown of Irrigation Water Requirement in Case 1/
	Irrigation Dury (Kerian Scheme) (1/3 - 3/3) 1-T-37
Table 1-37	Breakdown of Irrigation Water Requirement in Case 1/
	Irrigation Dury (Sungai Manik Scheme) I-T-40
Table I-38	Breakdown of Irrigation Water Requirement in Case 17
	Irrigation Dury (Seberang Perak Scheme) 1-T-40
Table 1-39	Breakdown of Irrigation Water Requirement in Case 1/
	Irrigation Dury (Kemasin River System) I-T-41
Table I-40	Breakdown of Irrigation Water Requirement in Case 1/
	Irrigation Dury (Besut) I-T-42
Table I-41	Breakdown of Irrigation Water Requirement in Case 1/
	Irrigation Dury (Besut Scheme / Angga Barrage
	Sub-Scheme) I-T-42
Table I-42	Breakdown of Irrigation Water Requirement in Case 3/
	Irrigation Dury (Kerian Scheme) (1/3 - 3/3) I-T-43
Table I-43	Breakdown of Irrigation Water Requirement in Case 31
	Irrigation Dury (Besut River System) I-T-46
Table I-44	Breakdown of Irrigation Water Requirement in Case 3/
	Irrigation Dury (Angga River System) 1-T-46
Table I-45	Water Balance of Bukit Merah Reservoir
	in Present Condition (Case 1)
Table I-46	Water Balance of Bukit Merah Reservoir
	in Present Condition (Case 2)
Table I-47	Water Balance of Bukit Merah Reservoir
	in Present Condition (Case 3)
Table I-48	Water Balance of Bukit Merah Reservoir
	in Future Condition (Case 1)
Table I-49	Water Balance of Bukit Merah Reservoir
	in Future Condition (Case 2) I-T-49
Table I-50	Water Balance of Bukit Merah Reservoir
	in Future Condition (Case 3)1-T-49
Table 1-51	Water Balance of Kerian River in Present Condition
	(1/3 - 3/3)1-T-50
Table 1-52	Water Balance of Batang Padang River
	in Present Condition (1/2 - 2/2)
Table I-53	Water Balance of Kemasin River in Present Condition
:	(1/2 - 2/2)
Table I-54	Water Balance of Besut River in Present Condition
1110.0101	(1/2 - 2/2)
Table I-55	Water Balance of Angga River in Present Condition

	(1/2 - 2/2)
Table I-56	Water Balance of Kerian River in Future Condition (2010)
1400 1-30	(1/3 - 3/3)
Table I-57	Water Balance of Batang Padang River
1 aug 1-57	in Future Condition (2010) (1/2 - 2/2) 1-T-64
Tal. 1 50	Water Balance of Kemasin River
Table I-58	in Future Condition (2010) (1/2 - 2/2) I-T-66
m 14. 1.60	Water Balance of Besut River in Future Condition (2010)
Table I-59	
C 11 I (A	(1/2 - 2/2)
Table I-60	Water Balance of Angga River in Future Condition (2010)
	(1/2 - 2/2)
	List of Figures
Fig. I-1	Flow of Water Balance Calculation I-F-1
Fig. I-2	Monthly 24-Hour Mean Temperature
	at Principal Meteorological Station I-F-2
Fig. <b>1-3</b>	Monthly 24-Hour Mean Relative Humidity
	at Principal Meteorological Station I-F-3
Fig. I-4	Monthly Mean Daily Sunshine Hours
	at Principal Meteorological Station I-F-4
Fig. I-5	Monthly Mean Daily Surface Wind Speed
	at Principal Meteorological Station I-F-5
Fig. I-6	Potential Evapotranspiration Estimated by
	Modefied Penman Method I-F-6
Fig. I-7	Inventry of Rainfall Stations Used for Estimating
	Irrigation Water Requirement (1/2 - 2/2) I-F-7
Fig. I-8	Inventry of River Discharge Stations Used for Estimating
	Available Water for Irrigation
Fig. I-9	4 x 4 Tank Model Used for the Study I-F-10
Fig. I-10	Calibration Result of Tank Model (Besut River)
T	(Monthly Mean Discharge)
Fig. I-11	Location Map of Rainfall and River Discharge Station
	(Pulau Pinang)
Fig. I-12	Location Map of Rainfall and River Discharge Station
D: 1.10	(Kerian) I-F-13
Fig. I-13	Location Map of Rainfall and River Discharge Station
	(Sungai Manik) I-F-14
Fig. I-14	Location Map of Rainfall and River Discharge Station
D' 117	(Seberang Perak)
Fig. I-15	Location Map of Rainfall and River Discharge Station
P: 116	(Kernasin / Semerak)
Fig. 1-16	Location Map of Rainfall and River Discharge Station
T21 - T 1/7	(Ketara (Besut))
Fig. I-17	10days Effective Rainfall I-F-18

.

Fig. I-18	Irrigation Supply Pattern Used for Water Balance Study
C	(10 Days Basis) (Wet Direct Seeding Method
	<growth 130="" days="" period="">) I-F-19</growth>
Fig. 1-19	Irrigation Supply Pattern Used for Water Balance Study
	(10 Days Basis) (Wet Direct Seeding Method
	<growth 120="" days="" period="">) I-F-20</growth>
Fig. I-20	Irrigation Supply Pattern Used for Water Balance Study
· ·	(10 Days Basis) (Dry Direct Seeding Method
	<growth 130="" days="" period="">) I-F-21</growth>
Fig. I-21	Irrigation Supply Pattern Used for Water Balance Study
J	(10 Days Basis) (Dry Direct Seeding Method
	<growth 120="" days="" period="">) I-F-22</growth>
Fig. I-22	Irrigation Supply Pattern Used for Water Balance Study
1.8, 1 ==	(10 Days Basis) (Transplanting Method) I-F-23
Fig. I-23	Present Cropping Schedule Used for Water Balance Study
1 181 1 = 1	(10 Days Basis) (Kerian-Compartment A-F) I-F-24
Fig. 1-24	Present Cropping Schedule Used for Water Balance Study
1 16. 1 2 1	(10 Days Basis) (Kerian-Compartment G,H) I-F-25
Fig. I-25	Present Cropping Schedule Used for Water Balance Study
118.120	(10 Days Basis) (Sungai Manik, Seberang Perak) I-F-26
Fig. 1-26	Present Cropping Schedule Used for Water Balance Study
115.120	(10 Days Basis) (Kemasin / Semerak, Besut) 1-F-27
Fig. 1-27	Proposed Cropping Schedule Used for Water Balance Study
118.12.	(10 Days Basis) (Kerian) I-F-28
Fig. 1-28	Proposed Cropping Schedule Used for Water Balance Study
116.120	(10 Days Basis) (Besut)
Fig. I-29	Water Distribution of Pulau Pinang Scheme I-F-30
Fig. I-30	Water Balance Model of Kerian Scheme
Fig. 1-31	Water Balance Model (Inflow of Bukit Merah Reservoir)
• • <b>o</b> ·	of Kurau River I-F-32
Fig. I-32	Water Balance Model of Kerian River I-F-33
Fig. I-33	Water Balance Model of Batang Padang River I-F-34
Fig. I-34	Water Balance Model of Perak River I-F-35
Fig. I-35	Water Supply from Kemubu Pump Station to KADA and
	IADP Kemasin / Semerak in Drought Period I-F-36
Fig. I-36	Water Balance Model of Kemasin River 1-F-37
Fig. 1-37	Water Balance Model of Besut River I-F-38
Fig. I-38	Water Balance Model of Angga River I-F-39
Fig. I-39	Elevation - Storage - Surface Area Curve of
	Bukit Merah Reservoir 1-F-40
Fig. I-40	Relationship between Monthly Rainfall, River Discharge
1.6.1	(Kurau River) and Water Requirement(Kerian Darat)
	(1/5 Probability Drought Year)
Fig. I-41	Relationship between Monthly Rainfall, River Discharge
0 //	(Kerian River) and Water Requirement(Kerian Laut)
	(1/5 Probability Drought Year)

Fig. 1-42	Relationship between Monthly Rainfall, River Discharge
U	and Water Requirement (Batang Padang River System)
	(1/5 Probability Drought Year) I-F-42
Fig. I-43	Relationship between Monthly Rainfall, River Discharge
7.6	and Water Requirement (Kemasin River System)
	(1/5 Probability Drought Year) I-F-42
Fig. 1-44	Relationship between Monthly Rainfall, River Discharge
116.11.	and Water Requirement (Besut River System)
	(1/5 Probability Drought Year) I-F-43
Fig. 1-45	Command Area Distribution in Kerian Scheme
r (g. 1-40	(in Present Condition: Case 1)
Fig. I-46	Command Area Distribution in Kerian Scheme
1 ig. 1-40	(in Present Condition: Case 2)
Fig. I-47	Command Area Distribution in Kerian Scheme
14g. 1247	(in Present Condition: Case 3)
Fig. I-48	Command Area Distribution in Kerian Scheme
14g, 1540	(in Future Condition: Case 1) 1-F-47
Fig. I-49	Command Area Distribution in Kerian Scheme
14g. 149	(in Future Condition: Case 2)
Fig. I-50	Command Area Distribution in Kerian Scheme
Fig. 1-30	(in Future Condition : Case 3) 1-F-49
Fig. I-51	Command Area Distribution in Kerian Scheme
11g. 1-51	(1/5 Probability Drought Year)
Fig. 1-52	10-Days Basis Transition of Bukit Merah Reservoir
14g. 1-52	Water Level (1/5 Probability Drought Year) I-F-51
Fig. I-53	10-Days Basis Water Balance of Kerian River
14g. 155	(1/5 Probability Drought Year)
Fig. I-54	10-Days Basis Water Balance of Batang Padang River
1 1g. 1-04	(1/5 Probability Drought Year)
Fig. 1-55	10-Days Basis Water Balance of Kemasin River
11g. 1-05	(1/5 Probability Drought Year)
Fig. I-56	10-Days Basis Water Balance of Besut River
1 ig. 1-30	(1/5 Probability Drought Year)
Fig. 1-57	Sample "Drought - Required Reservoir Storage Curve"
rig. 1-57	of Bukit Merah Reservoir (in Future Condition : Case 3) I-F-54
Fig. I-58	Comparison of Water Requirement (Besut Scheme)
116.130	(Wet Seeding 100% vs Wet 80% - Dry 20%)

# I. WATER BALANCE STUDY

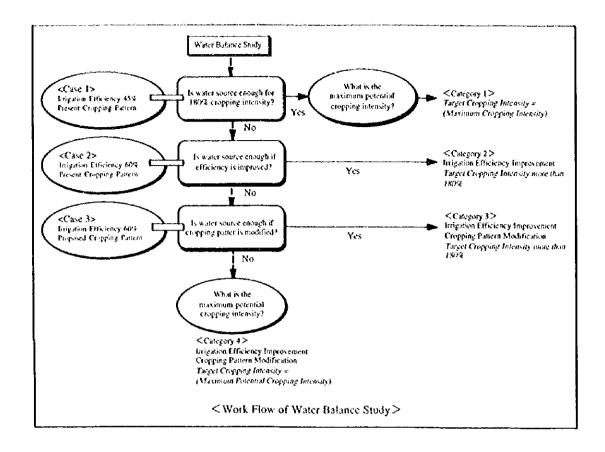
#### 1 GENERAL

Water balance study was made in order to confirm whether five(5) granary areas have enough irrigation water for achieving cropping intensity as high as 180%. By conducting the water balance study, present condition and effect of irrigation water management system modernization can be clearly recognized, which contributes the formulation of water management system planning.

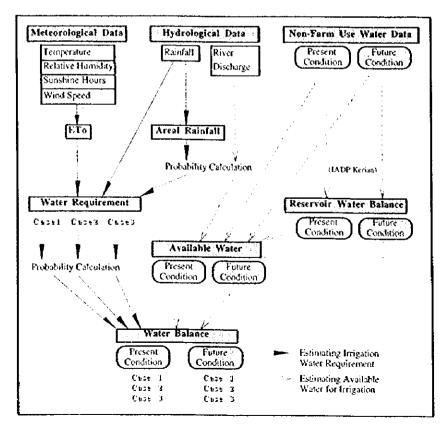
As the first step of the study, available water for irrigation in probable drought years were estimated based on river discharge and relevant data. Secondly, irrigation water requirement in probable drought years were calculated on the basis of various climatic data etc. Then, the available water for irrigation and irrigation water requirement were compared to judge if the available water meets the requirement. In the study, several conditions shown below are assumed.

- (1) Available water for irrigation
  - Present Condition
  - Future Condition (year 2010)
- (2) Irrigation water requirement
  - Case 1 (overall irrigation efficiency: 45%, present cropping pattern)
  - Case 2 (overall irrigation efficiency: 60%, present cropping pattern)
  - Case 3 (overall irrigation efficiency: 60%, proposed cropping pattern)

The water balance study was conducted along the work flow shown below. Firstly, using the assumed present overall irrigation efficiency of 45% and present cropping pattern, water balance was conducted <Case 1>. Secondly, if water sources do not meet the requirement, with the assumption that overall irrigation efficiency is improved as much as 60% thanks to the irrigation system modernization, the calculation is repeated <Case 2>. Finally, when water sources still remained insufficient, proposed cropping pattern was applied <Case 3>. In the study, target drought probability is set to be less than 1/5, in other words, drought will be occur less than once a five years.



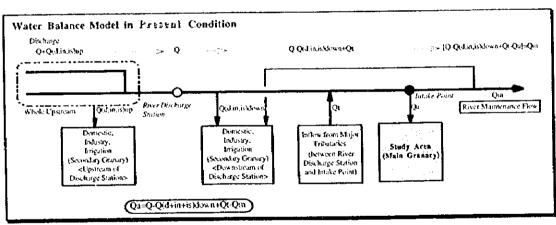
The flow of water balance calculation is shown below. Tables No. and figures No. used for the calculation are shown in Fig. I-1.



#### 2 METHODOLOGY

# 2.1 Available Water for Irrigation

In the study areas, except Jarak river in Pulau Pinang and Angga river in Ketara (Besut), river discharge data is gauged by DID. In the study, available water for irrigation was estimated based on the data in the latest 10 years river discharge data. However, as for Besut river, reliable data period falls on between 1961 and 1964. Hence, 4 x 4 tank model shown in Fig. 1-9 and 1-10 was used for estimation of discharge in the past ten years. For Angga river, river discharge record is not available, so that it is assumed that the drought probability of Angga sub-scheme in Ketara(Besut) is same as that of Besut sub-scheme. Available water of study areas were determined considering present and future water demand of domestic water, industrial water and river maintenance flow etc. Present and provision data of domestic and industrial water demand are corrected at relevant agencies. For the river which does not have detail water distribution plan of domestic and industrial water, the estimation based on the capacity of intake facilities are made. Concerning river maintenance flow, only data of Muda river is available. Accordingly, the river maintenance flow of the other rivers are estimated based on specific river maintenance flow of Muda river. The general models used for determining available water for irrigation are shown below. The models for respective rivers are shown in Fig. I-30 to I-39.



<Available Water for Irrigation in Present Condition (Qa)>

Qa = Q - Q(d+in+is)down + Qt - Qm

where, Qa: Available irrigation water for the study area in present water demand

Q : River discharge at the river discharge station

Q(d) : Domestic water in present condition
Q(in) : Industrial water in present condition

Q(is) : Irrigation water for secondary granary areas in present condition

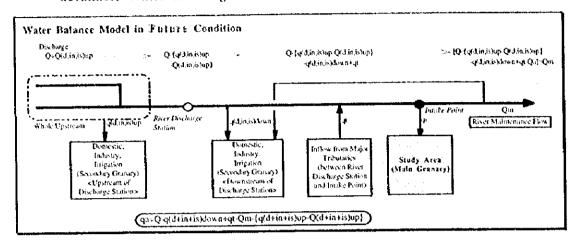
Q()down: Water demand for downstream of river discharge station in present

condition

Qt : Inflow from major tributary or excess water from another schemes

#### : River maintenance flow Om

# <Available Water for Irrigation in Future Condition (Year 2010) (qa)>



 $qa = Q - q(d+in+is)down + qt - Qm - \{q(d+in+is)up - Q(d+in+is)up\}$ 

where, qa : Available irrigation water for the study area in future water demand

: River discharge at the river discharge station Q

: Domestic water in future condition q(d)

: Industrial water in future condition q(in)

: Irrigation water for secondary granary areas in future condition q(is)

: Water demand for upstream of river discharge station in present condition Q( )up : Water demand for upstream of river discharge station in future condition

q( )up

q()down: Water demand for downstream of river discharge station in future

condition

: Inflow from major tributary or excess water from another schemes Qt

: River maintenance flow Qm

Q(d), Q(in) and Q(is) consist of two parts, one is the water taken from upstream of river discharge station, and the other is the water taken from downstream. In present condition, river discharge at the station does not include upstream water demand already taken from the river. Consequently, only downstream water demand is considered in present condition. On the other hand, in future condition model, river discharge at the station might be decrease because of water demand increment in upstream. Hence, upstream water demand increment is considered in the future condition model.

#### Domestic and Industrial Water (1)

The data of domestic and industrial water are obtained from relevant departments or agencies as shown below. The amount of water to be released at the intake point for domestic and industrial water use in the downstream is estimated considering inflows from tributaries located between the both intake points for irrigation and domestic/industrial water. Estimated domestic and industrial water demands at intake points of the study areas are shown below and their breakdowns are shown in Table I-14 and I-15.

		D/L	Water De	emand (n	1 <sup>3</sup> /s)		
Scheme	River	Present		Future (2010)		Data Source	
		U/S	D/S	U/S	D/S		
Pulau Pinang	Muda	-	7.96	-	16.11	Penang Water Authority	
C						Comprehensive Management Plan of Muda River Basin (JICA)	
	Kulim	-	0.54		0.88	Penang Water Authority	
Kerian	Kurau		0.88		1.40	Perak Water Board	
	(Bukit Merah)						
	Kerian		0.32	<u>-</u>	0.73		
Sungai Manik	Big. Padang	-	<u> </u>	-			
Seberang Perak	Perak	6.41	3.76	8.83	6.72	Perak Water Board	
Kemasin/Semerak	Kemasin	-					
Ketara (Besut)	Besut	-	0.25	i -	0.77	7JBA <sup>n</sup> Terengganu	
	Angga	<u> </u>	<u> </u>	-	-		

Note:

D/I : Domestic and Industrial

: Jabatan Bekalan Air (Waterworks Department)

U/S: Upstream of River Discharge Station
D/S: Downstream of River Discharge Station

# (2) Irrigation Water for Secondary Granary Areas

There are 74 secondary granary areas in Malaysia and its demarcation is decided based on Feasibility Study on Rationalization and Crop Diversification in Non-Granary Irrigated Areas in Malaysia (JICA, 1990). Secondary granary areas are also considered in the water balance model because it is projected that cropping intensity of secondary granary areas will increase 120 % to 170% based on NAP. In the water balance model, cropping patterns of secondary granary areas are assumed as same as those of granary areas. Inventory of secondary granary areas in relevant river basin is shown in Table I-16.

# (3) Inflow from Major Tributaries

Some of the river discharge stations are located far from intake point of the study area. There might be some significant inflow from tributaries between river discharge station and intake point. In the water balance model, inflows from tributaries are estimated from specific discharges of the main rivers by using tributaries catchment area factors. In the water balance model, tributaries catchment area factor is defined as shown below.

TCF = (Catchment Area of Tributaries / Catchment Area of Main Flow at discharge station) where, TCF: Tributaries Catchment Area Factor

Table I-17 shows tributaries catchment area factors of relevant rivers to be used for the study. The discharges of tributaries can be estimated by use of the following equation:

$$Qt = TCF \times Q$$

where, Qt: Discharge of Tributaries

O: Discharge of Main Flow

# (4) River Maintenance Flow

The river maintenance flow is the indicator of the allowable limit of water withdrawal from the river. Water withdrawal should not be increased, if it is expected to impair the river maintenance flow frequently. In Muda river, it is agreed between Pinang State, Kedah State and Penang Water Authority that 5 m³/s of water should be released from Muda Barrage as river maintenance flow. On the other hand, JICA study team could not get any information about river maintenance flow for the rest of relevant river.

For the water balance study, river maintenance flows shown below are considered based on specific river maintenance flow of Muda river (0.00119m³/s/km²). In the water balance model, downstream water demand is already considered so that river maintenance flow is used only for maintaining river environment and so on, not for downstream water demand. Procedure to estimate river maintenance flow to be used in water balance study is shown in Table I-18, and summarized below.

Scheme	River	River Maintenance Flow for Water Balance Study at Intake Point (m <sup>3</sup> /s)				
Pulau Pinang	Muda	5.00				
···-	Kulim	0.15				
Kerian	Kerian	1.60				
Sungai Manik	Batang Padang	0.54				
Seberang Perak	Perak	17.55				
Kemasin/Semerak	Kemasin	0.16				
Ketara (Besut)	Besut	0.69				
- 35 my	Angga	0.09				

# 2.2 Irrigation Water Requirement

Diversion irrigation water requirement was calculated from every 10 days on the basis of the formula shown below. Concerning cropping method, wet direct seeding, dry direct seeding and transplanting which are being practiced in the study areas were used for the water balance study. For planting period, 120 days for the schemes located on the west coast and 130 days for the schemes located on the east coast ware applied.

# (1) Presaturation, 2nd standing water supply period

DWR = (S + H + ETo + PL - ER) / E

DWR : Diversion Water Requirement

S : Soil Saturation DepthH : Standing Water Depth

ETo: Potential Evapotranspiration

PL: Percolation

ER : Effective Rainfall

E : Overall Irrigation Efficiency

For soil saturation depth, DID standard value 150mm was applied. For standing water depth, 100mm derived from DID standard was used for presaturation period and 0mm was used for 2nd standing water supply period. Potential Evapotranspiration (ETo) was estimated by using modified Penman method. Required climatic data were collected from principal meteorological stations closest to the study areas as shown below. Percolation (PL) data were obtained from operation and maintenance manuals collected from DID local offices and "Assessment of Selected Performance Indicators for Paddy Irrigation in Kerian Scheme" published by DID in 1995. The maximum and minimum limit of effective rainfall were set as 165mm and 5mm respectively. Maximum limit 165mm consists of 100mm to make standing water and 65mm to keep standing water depth. 65mm derives from paddy plot water balance shown below. The minimum limit 5mm was decided considering the rainfall to be kept in stems of rice plants. In presaturation and 2nd standing water supply period, 10-days areal rainfall in 1/5 probability drought year were used for estimating effective rainfall. Overall irrigation efficiency were assumed as 45% for present irrigation system and 60% for modernized irrigation system.

Meteorological Stations for Estimation of Potential Evapotranspiration

Scheme	P.Pinang Kerian	Sg.Manik Sb.Perak	Kemasin/ Semerak	Besut
Meteorological	Bayan Lepas	Sitiawan	Kota Bahru	Kota Bahru
Station Calculation Period	1977-1996	1977-1996	1977-1996	1977-1996

(Paddy Plot Water Balance Study)

Effective rainfall is given by conducting water balance calculation at paddy field level. The water balance calculation is performed based on the following formula.

$$WL(i) = WL(i-1) + R - ETo \times Kc - PL$$

where, WL(i): Water Level at time(i)

R : Rainfall more than 5 mm (rainfall less than 5 mm is assumed as

ineffective rainfall because it'll be kept around stem of rice plant)

ETo: Potential Evapotranspiration

Ke : Crop CoefficientPL : Percolation Loss

Based on Assessment of Selected Performance Indicators for Paddy Irrigation in Kerian Scheme, storage capacity of the model and irrigation & drainage interval are assumed to be 75 mm and 10 days respectively. The results of the calculation are shown in Table I-1 and Fig I-5. The other factors used for estimating irrigation water requirement are shown in Table I-2.

# (2) Normal supply period

 $DWR = (ETo \times Kc + PL - ER) / E$ 

DWR : Diversion Water Requirement

ETo : Potential Evapotranspiration

Kc : Crop Coefficient

PL: Percolation

FR : Effective Rainfall

E : Overall Irrigation Efficiency

The maximum and minimum effective rainfall for normal supply period were set as 65mm and 5mm, respectively. The nearest rainfall station data (more than 20years) was used for estimating diversion water requirement. Probability calculation was done in order to estimate water requirement in each probability drought year.

# 3. RESULT OF THE WATER BALANCE STUDY IN THE FIVE(5) GRANARY AREAS

# 3.1 Pulau Pinang Scheme

Pulau Pinang Scheme has three(3) main water sources, namely, Muda river (for Sungai Muda Sub-Scheme, Pinang Tunggal Sub-Scheme and Pokok Tampang Block in Sungai Jarak Sub-Scheme), Kulim river (for Sungai Kulim Sub-Scheme) and Jarak river (for Padang Menora Block in Sungai Jarak Sub-Scheme). Kulim and Jarak Sub-Scheme receive supplementary irrigation water from Muda river system as shown in Fig. I-29, so that water balance study of Muda river is quite important for Pulau Pinang Scheme. The water balance study of Muda river basin including the biggest granary MADA has been done in "Comprehensive Management Plan of Muda River Basin (JICA,1995)". According to the result, granary areas in Muda river basin can achieve 200% of cropping intensity if proposed water resources structures such as Beris Dam, Jeniang Transfer, Naok Dam and Reman Dam are constructed.

## 3.2 Kerian Scheme

For Kerian Scheme, water balance model considering Bukit Merah reservoir storage function was developed. In the model it is assumed that Kerian Darat Sub-Scheme which completely depends on the reservoir water source is given priority for the water distribution. Kerian Laut Sub-Scheme is given excess water from Bukit Merah reservoir and supplemented by Bogak Pump Station. The water balance model used for the study is described in Fig. I-30 and equation/operation rule used for the reservoir water balance study are summarized below.

Storage - Area - Elevation Curve of Bukit Merah Reservoir used for the study is shown in Fig 1-39.

The water balance study of Bukit Merah Reservoir

Se = Sb + In - RD - RL - Ev - Sp

where, Se : Reservoir storage volume at the end of the period

Sb : Reservoir storage volume at the beginning of the period

In : Inflow to the reservoir from Sungai Kurau during the period

RD: Release from reservoir to Kerian Darat including domestic, industrial

and irrigation water requirement

RL: Release from reservoir to Kerian Laut including domestic, industrial and

irrigation water requirement

Ev : Evaporation loss from reservoir

Sp : Spillout discharge if any

The rule of operating reservoir used in the study

- (1) The water level should be the maximum level on December 1st.
- (2) The release will first meet the domestic, industrial and irrigation water requirement in Kerian Darat.
- (3) The balance will be supplied to Kerian Laut.
- (4) If this balance is inadequate to meet the demands in Kerian Laut, pumping augmentation shall be initiated to meet the deficit to the extent possible.
- (5) In the Reservoir Operation Study, it is mentioned that the reservoir level should be kept low level in October, November, December because of anticipation of flood. But there are no certain regulation to operate the reservoir level, so that the this release is not consider in this study.

In Kerian Scheme, present cropping pattern is quite different from the original one. According to the water balance study, the scheme can achieve 200% of cropping intensity in <Case 1> (present cropping pattern). However, from agriculture and water management point of view, it is recommended to use proposed cropping pattern <Case 3> which is similar to the original pattern to achieve 5.5t/ha yield. <Case 3> has high water demand peak compare with <Case 2>, but <Case 3> can also achieve 200% of cropping intensity. In Kerian Scheme, it is quite important to operate Bukit Merah reservoir with proper management method. For proper reservoir operation, it is important to decide command area of Bukit Merah reservoir and Bogak Pump Station. Command areas of Bukit Merah reservoir and Bogak Pump Station are shown in Fig. I-45 to I-51. 10-days basis reservoir water level transition in 1/5 probability drought year is shown in Fig. I-52. 10-days basis water balance of Kerian river is shown in Fig. I-53.

It is quite important to keep desirable reservoir water level to achieve optimum reservoir operation. "Drought - Required Reservoir Storage Curve Method" is recommended for agriculture purpose reservoir because of its effectiveness and simplicity. Reservoir operator have to adjust two kind of requirement shown below:

- (a) To release water as much as possible for the farmers,
- (b) To restore water as much as possible for present or future drought.

Drought - Required Reservoir Storage Curve Method is developed to adjust the opposite object shown above. If reservoir storage is larger than "Drought - Required Reservoir Storage", reservoir storage will not become zero even in design drought year (1/5 probability drought year). Drought - Required Reservoir Storage should be converted to Drought - Required Reservoir Storage Curve by using H-Q curve of the reservoir considering simplicity of the reservoir operation. Drought - Required Reservoir Storage Curve consists of two curves, namely "Standard Storage Curve" and "Release Limitation Curve".

At first, it is necessary to determine "Standard Storage Curve" by using formula as follows:

$$K(i) = P(i) - Y(i)$$
  
 $V(i) = V(i+1) - K(i)$   
in case  $V(i) < 0$ ,  $V(i) = 0$ 

where, K(i): Water balance

P(i) : Available inflow in design drought year

Y(i): Irrigation water requirement in design drought year

V(i): Required reservoir storage

i : Calculation time (i = 1, 2, 3, ..., n)

Reservoir water level should be kept to "Standard Storage Water Level". Nevertheless, the water level often falls down below "Standard Storage Water Level" because of fluctuation of weather condition. In case the water level is below "Standard Storage Water Level", the reservoir should be operate according to the "Release Limitation Curve". "Release Limitation Curve" is determine by using formula shown below.

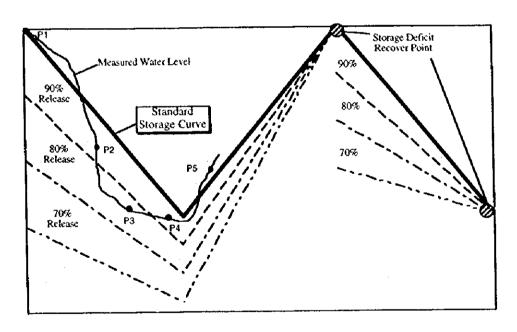
$$K(i) = P(i) - Y(i) \times Z$$
  
 $V(i) = V(i+1) - K(i)$   
in case  $V(i) < 0$ ,  $V(i) = 0$ 

where, Z: Release Rate

"Storage Deficit Recover Point" is the critical point of this method. The water level must be higher than "Standard Storage Curve" at this point to supply 100% of water demand after "Storage Deficit Recover Point".

The example of "Drought - Required Reservoir Storage Curve" is shown below. At first, water level was maximum level (P1) so that release was set as 100% of water demand. Nevertheless, because of unexepectable drought (more than 1/5 probability), water level of reservoir went down from P1 to P2. P2 is located at 90% release zone, hence release from reservoir was set as 90% of water demand. Unfortunately, drought is very severe in the sample year, water level went down from P2 to P3 even with 90% release. P3 is located at 80% release zone, hence release from reservoir was set as 80% of water demand. Thanks to the 20% water release decrease, water level went up from P3 to P4, so that water release was set as 90%. At last, water level went up to P5 which is located higher than "Standard Storage Curve" so that reservoir can supply 100% of water demand. By using this method, reservoir operator will be able to keep "Standard Storage Curve".

Fig. I-57 shows sample "Drought - Required Reservoir Storage Curve" of Bukit Merah Reservoir in Case 3 (Future Non-Farm Water Demand). In Fig. I-57, "Storage Deficit Recover Point" was determined based on the growth stage of rice plant. For rice plant, it is very important to be given water in heading period so that water supply in this period must be 100% of water demand to attain high yield. Consequently, in Fig. I-57, "Storage Deficit Recover Point" were set considering beginning of heading period of main season (September) and off season (March).



# 3.3 Sungai Manik Scheme

Sungai Manik Scheme has sufficient water to achieve 180% cropping intensity even in present condition <Case 1>, and the scheme can conduct 200% cropping intensity from water resource point of view. 10-days water balance result is shown in Fig. I-54.

# 3.4 Seberang Perak Scheme

For Seberang Perak scheme, detail water balance study could not be achieved because of non-availability of proper Perak river discharge data. However, it can be said that the scheme has abundant water resource according to the rough estimated river discharge data given by DID Ipoh. The scheme can accomplish not only 180% but also 200% cropping intensity even in low irrigation efficiency condition <Case 1>.

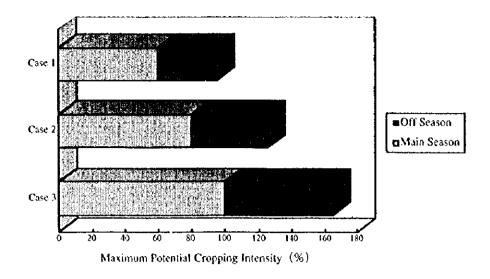
#### 3.5 Kemasin / Semerak Scheme

In Kemasin / Semerak Scheme, only Kemasin Hilir sub-scheme and Jelawat Rusa sub-scheme has operated so that the water balance study was conducted only for the two(2) sub-schemes. In the study, two(2) sub-schemes were regarded as one(1) system since they receive water from the same river (Kemasin river). Kemasin scheme has planed to be provided 5 m³/sec water from Kemubu Pump Station located at Kelantan river which has constructed for KADA. The water balance study was conducted with supply water of 5 m³/sec. The scheme can attain 200% cropping intensity, if Kemubu Pump Station supply water to Kemasin scheme properly. 10-days water balance result is shown in Fig. I-55.

## 3.6 Ketara(Besut) Scheme

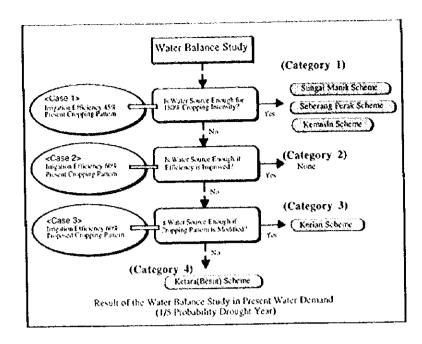
Ketara(Besut) scheme has 2 water sources, namely Besut river and Angga river. In this area, north east wind peculiar in the east coast area has great influence and as a result, annual and monthly variation of water sources is significant and effective use of river discharge is very difficult. In Besut scheme, the original cropping pattern is considered as recommended cropping pattern <Case 3> and adjustment of present cropping pattern <Case 1> to the original cropping pattern <Case 3> makes great benefit. In addition, introducing dry direct seeding in off season in 20% of total area can decrease peak water demand as shown in Fig. I-58. In the water balance study, the original cropping schedule with 20% dry direct seeding is recommended as <Case 3>.

However, even in <Case 3>, the scheme cannot attain 180% cropping intensity in 1/5 probability drought year, hence the scheme is recognized as (Category 4) in work flow of water balance study. The maximum potential cropping intensity of the scheme in 1/5 probability drought year is about 165% as shown below. 10-days water balance result is shown in Fig. I-56.



#### 4. RESULT OF WATER BALANCE STUDY AND PROPOSED PLAN

Based on the water balance study, the study areas can be classified as shown below. Sungai Manik, Seberang Perak and Kemasin Schemes have enough water to achieve not only 180% cropping intensity but also 200%. Consequently, these schemes were classified as (Category 1) and the target cropping intensity should be set as 200%. Kerian scheme was classified as (Category 3), hence irrigation efficiency improvement and cropping pattern modification are required to get more than 180% cropping intensity and high yield. Ketara(Besut) scheme was classified as (Category 4), so that irrigation efficiency improvement plan, cropping pattern modification plan and drainage water recycling plan should be taken. In Ketara(Besut) scheme, capacity of recycling pump is about 10% of peak diversion irrigation water requirement. If the scheme can recycle about 10% of irrigation water, maximum potential cropping intensity in 1/5 probability drought year is about 175%. Pulau Pinang scheme will be able to achieve 180-200% cropping intensity with the proposed dam projects. In the water balance study, it is assumed that there is no water distribution problem. In case of water distribution problem, there will be water deficit in local areas.



# 5. ADDITIONAL STUDY FOR THE FUTURE WATER SHORTAGE

In order to help policy formulation contingency plans in the event of water shortages in the future, additional study to grasp maximum cropping intensity for the future water shortage is made. In the additional study, more stringent drought year condition than one in five years (1/5) such as one in ten years (1/10) and one in twenty years (1/20) are assumed. The result of the additional study is summarized below.

Maximum Cropping Intensity (%) of Case 3 in Future Water Demand Condition

Drought Probability	1/5	1/10	1/20
Scheme			
Pulau Pinang 1)	200	200	-
Kerian	200	140	75
Sg. Manik	200	190	170
Perak <sup>2)</sup>	200	200	200
Kemasin <sup>3)</sup>	200	200	200
Ketara(Besut)	165	145	130
Ketara(Besut) 4)	175	155	140

<sup>1):</sup> Comprehensive Management Plan of Muda River Basin (JICA, 1995)

Estimated from minimum discharge (113m³/s) guaranteed by Tenaga Nasional Berhad at Iskandar Bridge (upstream of Telok Sena Intake)

<sup>3):</sup> with supplementary water from Kemubu pump station (KADA)

<sup>4):</sup> with 10% recycling water

**TABLES** 

Table 1-1 Inventry of Principal Meteorological Station Used for the Study

	Latitu	de(N)	Longit	ude(E)	Height above	Data Collected Period	Height of Anenwereter
	Deg.	Min.			M S L (m)		Head above Ground (m)
Pinang International Airport (Bayan Lepas)	- 5	18	100	16	28	1977-1996	12.5
Sitiowan	4	13	100	42	7.0	1977-1996	£6.B
Kota Shara Airport	6	10	102	17	4.6	1977-1996	14.0
Kuala Terengganu Airport	5	23	103	06	5.2	1985-1996	14.0

Table 1-2 Areal Rainfall of Pulau Pinang Scheme

Rainfall Station		
Kamian Station	5303053, 5304046, 5403042, 5404041, 5503031, 5503034, 5504035, 5505033	
L	3505053, 555 60 10, 555	/mm\

Month/10days	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec	Annua
Year			i			i						160.0	Total
1980	9.6	90.3	137.9	157.8	196.6	124.8	149.5	379.6	656.6	391.3	282.9	150.8	
1981	38.4	121.7	123.1	2368	2368	144.4	89.4	155.1	398.7	192.8	182.7	19.6	
1982	2.7	181	142.8	215.8	238.7	50.2	139.2	113.8	323.2	319.1	289.8	97.1	1950.
1983	20.1	46.1	39.1	123.0	378.7	163.0	124.9	90.4	427.4	225.7	96.5	98.9	
1984	196.0	135.0	107.9	453.4	146.4	119.0	231.9	117.8	178.9	171.2	154.4	139.8	
1985	54.2	133.0	178.6	187.5	220.4	46.5)	95 6	150.2	342.1	383.6	231.2	86.9	
1986	30.5	6.6	80.3	94.1	239.7	107.4	1492	203.1	400.5	466.6	(73.8)	76.8	
1987	5.2		65.6	132.1	219.0	93.4	124.8	251.2	336.7	214.5	353.7	133.7	1934
1988	45.8	91.0	t40.4	206.9	131 2	146.3	115.7	388.5	329.4	183.3	177.5	27.1	1983
1989	80.8		206.1	270.8	138.7	212.5	179.3	231.4	392.3	385.5	115.8	73.4	2332
1990	63.8		87.8	181.8	266.6	119.2	155.3	1112	348.7	274 6	324.5	78.9	2120
1991	64.8	1	1 1	205.9	380.3	268.6	281.6	241.7	272.0	200.8	131.8	86.4	2317
1991	29.6	t .	81.7	152.4	266.3	62.6	201.1	386.3	185.3	408.3	154.0	91.4	2131
Average	49.3	•		201.4	235 3	127.4	156.7	2169	353.2	293.7	205.3	89.9	2120

#### Probable Areal Rainfall

Estimated by Pearson Method

(mm)

Month/10days	Jan	Feb	Mar	Anc	May	Jun	Ju	Aug	Sep	Oct	Nov	Dec	Annual
Return Period		- **			•								Total
20	5.8	2.8	58.6	98.1	130.5	51.i	86.5	84.0	199.0	154.8	97.8	40.4	1009.3
iŏ	7.5	2.9	64.0		146.2	59.5	97.3	100.6	220.0	175.7	113.0	43.0	A CAMPAGE STATE
	11.6	**3.1	73.5	134.1	158.6	72.7	1126	125.7	2509	205.4	135.1	43.3	1341.0
	13.5		79.5	145.3	180.4	80.4	120-5	139.5	268.0	221.0	146.9	526	
1	17.3		88.0	161.3	196.9	91.5	131.5	159.6	291.8	2428	163.5	58.6	
5	26.0		101.6	185.7	2226	110.0	148.8	1929	329.0	277.2	190.3	68.4	1860.4

Table I-3 Areal Rainfall of Kerian Scheme

m * * *1 m*		
Rainfail Station		
Italinas Station		
	4905032, 5003028, 5004027, 5005002, 5006021, 5104011, 5104012, 5104052	
1	1703032,300340,210	

(mm)

Month/10days	Jan	Feb	Mar	Apr	May	Sun	Jui	Aug	Sep	Oct	Nov	Dec	Annual Tota?
Year 1979	101.0	147.4	74.6	351.9	189.1	150.6	197.3	122.9	230.4	366.5	255.6	32.1	2219.7
1980	57.0	171.9	142.8	232.9	255.1	131.7	102.5	317.0	317.0	347.6	3343	239.2	2649.
1981	93.7	117.5		375.4	242.2	57.8	143.4	86.7	260.6	147.6	152.3	82 9	
1982	54.2		174.6	235.0	250.7	42.9	106.4	110.0	238 2	421.5	379.8	158.7	
1985	87.5	1510	246.9	163.9	95.9	20.9	92.6	104.3	182.8	311.7	368.4	124.5	1
1986	45.9	48.8	237.6	226.2	283.7	61.6	105.9	197.3	314.6	285.8	163.8		
1987	55.7	5 2	158.4	162.6	192.8	163.9	123.7	242.4	321.9	297.0	289.2	289.6	
1988	172.1	175.1	358.8	296.7	154.5	229.2	108.9	259.9	245.6	104.6	282.7	77.5	
1990	128.1	100.9	145.2	213.6	2066	98 3	97.7		277.1	287.1	338.5	178.9	•
1993	139.3	93.3	323.8	148.5	141.5	121.3	224.6		164.8	277.7	357.1	204.0	
Average	93.4	1063	194.9	240.7	2012	107.8	130.1	165.9	255.4	284.7	292.2	154.8	2227

## Probable Areal Rainfall

Estimated by Pearson Method

(mm)

	23(11.12.10	,											
Month/10days Return Period		Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec	Annuat Total
20	40.4	30.4	77.8	136 2	125.9	32.4	75.5	70.0	181.5	164.3	187.9	59.1	11813
10	47.5					37.7	84.1	82.6	190.9	171.8			
Albert Stone	58.0	<ul> <li>■ 100 € 1 (20 €)</li> </ul>		175.6	145 1	47.4	96.2	101.3	205.6	186.5	212.6	76 3	
4	63.7	39.4	124.0	187.5	153.4	53.9	102.4	111.5	214.2	198.3	223.8		
i	71.9	!	[4] 4	204.0	161.7	63.7	111.0	126.1	225.8	214.5	l .	£	
	663	1	120.0	1 229 B	1821	80.5	1243	149.9	. 243.3	239.6	262.5	117.2	1940.2

Table I-4 Areal Rainfall of Sungai Manik Scheme

Rainfall Station 4010098, 4010138, 411137

(enm)

Month/10days	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec	Annual
Year	l			į	_ 1	. 1					1		Total
1979	131.5	208.4	94.9	169.6	132.2	81.3	163.0	94.0	379.0	353.4	316.7	214.9	2338.9
1980	175.6	257.2	349.3	219.1	196.7	143.1	125.3	189.6	222.3	221.8	331.5	198.5	2629.9
1982	43.6	110.6	182.8	446.4	253.4	64.3	164.0	137.9	103.5	462.3	581.0	326.1	2875.7
1987	116.0	115.4	180.7	304.2	104.0	97.7	183.3	267.2	204.0	299.0	128.2	273.2	2273.0
1988	105.4	143.2	300.1	355.0	146.6	175.9	178.6	367.1	313.0	39.7	377.4	169.3	2671.3
1989	372.8	126.3	183.1	260.4	289.7	112.6	41.8	39.8	460.8	155.8	379.6	109.1	2531.9
1990	83.8	120.7	119.9	82.4	267.5	92.2	111.5	40.7	203.6	261.9	161.3	199.5	1745.1
1991	123.4	160.6	200.9	306.8	276.0	134.7	56.1	94.8	81.1	167.8	280.3	221.2	2103.5
1992	69.5	41.9,	99.3	95.1	166.7	38.9	146.7	108.9	94.5	147.2	438.7	214.3	1661.7
1993	117.6	64.6	210.3	241.2	345.5	134.4	193.2	50.0	234,7	212.7	359.4	333.5	2497.1
Average	133.9	134.9	192.1	248.0	217.8	107.5	136.4	139.0	229.6	232.2	335.4	226.0	2332.8

## Probable Areal Rainfall

Estimated by Pearson Method

(mm)

Month/10days	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec	Anoual
Return Period													Total
20	45.3	58.2	88.2	105.6	112.2	56.1	67.6	30.7	79.3	92.9	172.1	136.6	1044.6
10	55.4	65.0	102.3	117.0	125.4	60.2	71.7	40.2	94.9	99.8	185.3	. 147.0	1164.2
5	71.1	76.9	123.2	137.9	145.5	68,1	79.8	56.3	119.9	113,6	210.4	163.6	1366.1
4	79.8	84.4	134.5	151.7	156.9	73,4	86.1	65.9				173.5	1492.8
3	92.7	95.2	150.5	171.5	173.0	80.8	95.1	80.7	156.3	141.5	250.5	187.1	1674.8
2	114.5	112.9	176.5	203.9	198.5	92.5	109.3	107.3	192.8	168.4	287.7	208.2	1972.4

Table I-5 Areal Rainfall of Seberang Perak Scheme

Rainfall Station 4008102, 4009096, 4010098

(mm)

Month/10days	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec	Annual
Year		1		1			1	- 1		1	- 1	1	Total
1979	109.5	217.0	128.2	233.8	136.9	156.7	227.8	110.8	35.7	349.2	203.7	141.3	2050.6
1980	136.5	159.5	193.4	172.8	237.0	138.3	226.8	161.9	258.3	219.8	273.1	215.7	2393.0
1982	102.9	86.9	191.4	268.1	160.2	95.5	143.6	133.4	200.9	275.7	312.0	315.0	2285.6
1984	359.3	530.2	239.7	192.5	168.4	131.1	139.9	116.9	113.9	272.5	179.1	162.8	2605.4
1988	129.7	153.4	325.8	229.4	163.9	128.4	124.4	229.1	255.7	62.3	317.2	111.4	2230.5
1989	272.2	52.4	164.1	220.3	154.8	35.6 <sup>1</sup>	27.1	101.0	330.9	158.5	352.1	181.2	2050.2
1990	184.8	85.8	124.4	178.1	152.2	35.4	120.0	63.2	210.5	262.0	125.8	239.6	1782.9
1991	95.9	91.2	258.0	229.6	123.4	152.8	44.7	48.1	232.0	172.1	351.3	256.4	2055.6
Average	173.9	172.2	203.1	215.6	162.1	109.2	131.8	120.5	204.7	221.5	264.3	202.9	2181.8

## Probable Areal Rainfall

Estimated by Pearson Method

(mm)

Month/10days	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec	Annual
Return Period													Total
20	70.8	41.5	111.5	168.0	116.8	44.4	42.1	50.9	88.1	112.5	154.6	113.6	1114.7
10	84.0	53.4	125.6	176.5	125.0	48.3	47.0	58.9	91.6	117.9	164.2	126.2	1218.6
5	103.8	73.2	145.3	187.8	135.8	- 56.2	57.2	71.3	100.2	. 129.3	182.1	144.7	1387.2
4	114.6	84.7	156.1	193.6	141.2	62.0	64.6	78.5	108.9	139.0	193.9	154.7	1491.8
3	130.2	102.4	170.6	201.2	148.5	70.3	75.6	88.8	121.3	152.6	210.2	168.6	1640.4
2	155.7	133.6	193.3	212.6	159.4	84.1	94.5	105.8	141.2	174.0	235.6	190.5	1880.2

Table I-6 Areat Rainfall of Kemasin / Semerak Scheme

Rainfall Station 5823076, 5824080, 5923001, 5923081, 6022001, 6023001, 6023072, 6024074

(mm)

Month/10days	Jan	Fcb	Mar	Aρε	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec	Annual
Year	• • • • • • • • • • • • • • • • • • • •				•	•		_					Total
1982	24.8	23.2	56.3	31.0	105.0	113.7	276.3	186.1	161.6	150.3	240.6	602.1	1971.0
1983	67.8	13.9	94.9	11.7	63.4	143.5	292.8	203.4	240.0	182.2	323.8	1199.2	2836.6
1984	139.7	110.1	194.9	78.3	149.7	181.3	101.6	181.1	197.4	191.6	338.8	706.9	2571.3
1985	101.9	132.7	458.2	98.4	207.4	37.3	170.0	228.0	292.5	340.3	283.4	646.4	29 <del>9</del> 6.5
1988	70.5	112.7	60.7	104.2	180.4	115.1	208.5	225.8	321.1	239.0	1256.6	543.3	3437.8
1989	86,7	35.6	34.3	87.4	118.0	103.1	163.3	245.8	373.3	355.2	339.8	187.2	2129.6
1990	100.1	21.9	30.9	261.9	133.1	104.1	180.6	196.5	286.7	236.6	169.3	246.9	1968.5
1991	298.9	25.8	29.1	62.8	132.5	104.0	72.9	301.5	292.0	219.2	866.4	421.3	2826.5
1992	71.9	76.0	26.3	25.8	82.4	204.6	216.7	69.6	110.8	118.7	672.8	426.9	2102.3
1993	189.4	38.0	236.2	149.9	62.4	127.5	248.5	187.6	302.7	515.7	538.7	457.7	3054.3
Average	115.2	59.0	122.2	91.1	123.4	123.4	193.1	202.5	257.8	254.9	503.0	543.8	2589.4

#### Probable Areal Rainfall

Estimated by Pearson Methed

(mm)

Month/10days	Jan	Feb	Mar	Apr	May	Jun	Joi	Aug	Sep	Oct	Nov	Dec	Annual
Return Period	, Jane				•							<u>-</u>	Total
20	33.6	12.0	14.5	17.6	61.1	68.6	103.0	135.9	156.1	114.6	152.2	213.3	1082.3
10	41.3	15.9	20.7	22.1	69.1	71.8		137.8			189.6	and a second of the second	and the second second
10 <b>3</b> 3 3 5 5	53.7	22.6	BUSINESS WALL	30.3	81.1	78.2	123.3	143.1	179.0	161.9	249.1	308.0	1462.6
4	61.3	26.7		35.9		1						341.5	1607.0
1	72.5	33.0	• • • • •		ł	90.6	145.4	156.9	204.3	198.5	333.2	390.0	1817.9
2	92.0	44.6		1	l	101.8	165.6	168.9	226.8	233.5	419.8	470.3	2171.3

Table I-7 Areal Rainfall of Besut Scheme

Rainfall Station 5625004, 5625011, 5725005, 5725006

(mm)

Month/10days	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec	Annual
Year		- 1	ĺ				}			1			Total
1978	123.5	71.0.	45.0.	35.5	160.8	111.8	192.5	202.6	344.5	309.0¦	369.1	359 I	2324.3
1980	71.9	97.9	47.0	97.3	129.1	127.3	148.4	276.7	220.7	346.3	404.4	770.8	2737.8
1981	40.4	85.0	54.1	72.8	196.0	60.2	147.2	90.8	240.9	238.5	428.5	620.8	2275.
1982	24.6	13.6	87.0	107.0		171.6	253.1	250.3	163.4	242.7	223.9	651.1	2307.
1983	198.2	18.8	96.4	0.0		164.0	348.5	249.3	222.0	195.6	293.8	1412.0	3305.
1984	371.4	385.5		111.2			198.9	205.6	294.4	158.7	278.8	1040.7	3536.
			697.5	171.2			114.6	307.2	350.9	410.6	218.9	832.8	3800.
1985	127.1					230.0	64.5	55.1	412.3	252.6	1189.7	335.1	
1986 Average	141.1	4.5 112.8	85.8 167.9	7.6 75.3	14 55 51	130.7	183.5		281.2	269.3	425.9	752.8	<u> </u>

## Probable Areal Rainfall

Estimated by Pearson Method

(mm)

Month/10days Return Period	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec	Annual Total
20	26.9	5.1	22.5	0.7	90.8	56.0	77.4	89.6	167.1	157.4	149.9	335.0	1158.4
10	34.8			0.7	101.9	62.4	88.5	96.0	184.0	174.9	181.6	La contrata de la contrata del contrata de la contrata del contrata de la contrata del contrata de la contrata de la contrata de la contrata del contrata de la contrata del contrata del contrata del contrata del contrata de la contrata del c	A . 14 (2004) 77/2013
5 7 1	48.6	14.1	47.2	0.9	117,7	73.8	106.3	108.8	208.2	199.6	230.5	448.4	1603.9
4	57.5	18.9	57.2	1.3	125.8	81.1	117.0	118.9	221.2	212.4	257.5	494.1	
3	71.4		•	2.0	137.2	91.6	132.5	133.4	239.1	230.1	297.2	559.9	
2	97.0	i	ı	3.7	155.0	108.9	157.7	156.8	266.9	257.7	363.6	667.9	2385.5

Table I.8 River Discharge of Kurau River

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Kurau Kiver	Kenan	Kenan Darat, Kenan Laur
HIVE	Scheme	Sub-Scheme

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# Table I-9 River Discharge of Kerian River

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# 10-Days Basis Probable River Discharge Data

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Table I-10 River Discharge of Batang Padang River

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10-Days Basis Probable River Discharge Data

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Table I-11 River Discharge of Kemasin River

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Table I-12 River Discharge of Besut River

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	Caren	(Sel1 (closed)	. Try km	
	Location	Action No.	Constitution and	
	Besti River	Kotony Reports		Besut Barrage
	1000	T. Special	ALE INC.	Sub-Schutz

	(m/k)	# 5 6 8 3 \$ C 1 2 C 2
		No. 2014 1982 2014 1982 2014 1982 2014 1982 2014 1982 2014 2014 2014 2014 2014 2014 2014 201
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: 9411 (closed) ca - 747 km²		ន្ទិនិនិធីជន្នាជន្នា
Station No. 1941 (cla Cachingia Area 1747 km <sup>2</sup>	į	W
Ketaral Benuti Benut Barrage		Mid-
Scheme (Sub-Scheme		200 X 200 200 200 200 200 200 200 200 20
	ed River Discharge by Tank Model	747 Mar. 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
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Table I-13 River Discharge of Angga River

	(m,1/s)	Average	
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Statum No. Calchinent Area : 78 km²	X Goust Bestage	į.	**************************************
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	0.00	S KIVET DISCIM	2.00 (2.00 ) 10.
	0	New Medical Services	1902 1903 1904 1904 1904 1904 1904 1904 1904 1904

Table I-14 Projected Domestic and Industrial Water Demand for Relevant Rivers

Hinde					Daylor, ou	est land Va	mand for	Ž	Name of Major Intake	intake Location	Note	Data Source
Hard   Harden   Har	Scheme		Kiver	- 04	elevant R	iver Syst				from River		
Mudd			<u></u>		2000	2005				Sischarge Station		Penane Water
13   Water Authority   1,62 3,02 4,02 5,01   Sungai Petani eff.   Downstream   13   Bukit Tok Allang   0,54 0,54 0,54 0,54   0,54 0,54	Pulau Pinang	Muda	[1]Managed by Penang	6.34	7.29	<del>5</del> .04	11.10	<u> </u>	har Tiang	Downstream	,	Authority
(3)Total (1+2)   7.96   10.32   15.06   16.11			(2)Managed by Others	1.62	3,02		5.01	Su	ngai Petani etc.	Downstream		Muda River Basin Study (JICA)
Kulim         [1]Busti Text         O.54         0.54         0.54         0.54         0.54         0.54         0.54         0.54         0.54         0.54         0.54         0.54         0.55         0.57         Mak Sulong         Downstream         Construction by 1997           [2]Mak Sulong         (3)Peral         (1)P(2)+(3)         0.24         0.27         0.27         0.29         0.00         0.57         0.59         0.00			(C) [/ ] [(C)	40 %	10,32	3.06	16.11	:				- M
1   Downstream   D			(5)10tal (1+2)	₽\$ Q	0.54	3,	0.54	β	ikit Tok Allang	Downstream		renang water
(3)Perai with Suburg   (1)Perai with Cachment   0.24   0.35   0.41   0.57   0.33   0.41   0.57   0.33   0.41   0.57   0.33   0.42   0.35   0.33   0.43   0.35   0.43   0.35   0.43   0.35   0.43   0.35   0.43   0.35   0.44   0.35   0.34   0.34   0.35   0.34   0.34   0.35   0.34   0.35   0.34   0		Valle	CONTRACTOR CONTRACTOR	0.0	8	0.57	0.57	Σ	ak Sulong	Downstream		Authority
(4)Total ([1]+[2]+[3])   0.82   1.65   2.70   2.7			(3)Perai		=======================================	1.59	1.59	ଟ୍ଲ	rai (Proposed)	- 1	Construction by 1997 Operation from 1999	
(5)Pera with Catchment			(4)Total ([1]+[2]+[3])	0.82	1.65		2.70	<u> </u>				
Area Factor ([3]x0,215)*   0.54   0.78   0.88   0			[5] Perai with Catchment		0.24	•						
(6)Demand at Kulim  (b)Demand at Kulim  (c)Demand at Kulim  (c)Demand at Kulim  (d)Downstream of Headworks (1]+[5])  (e)Downstream of Bukit Merah  (e)Downstream of Bukit Bunga Downstream  (e)Downstream of Bukit Bunga Downstream  (e)Downstream of Bukit Bunga Bukit Bunga Downstream Implementation around 2000  (e)Downstream of Bukit Bunga Extension Downstream Implementation around 2000  (e)Downstream Implementation  (e)Downst			Area Factor ([3]x0.215)*				;	~ <del>-</del>	į	:		
Northern			(6)Demand at Kulim	7.	0.78	0.88	98.0 98.0					
Kerian         Kerian         0,32         0,41         0,57         0,73         1,05         1,05         1,05         1,05         1,05         1,05         1,05         1,05         1,05         1,05         1,00	Kenan	Kurau	Downstream of Bukit Merah	0.88	00.1	0.1		.50 		Reservoir Release	Projected Water Demand of Kerian Distret in 2020 is	Lembaga Air Perak
Total   Balang Padang   1.20   141   1.87   2.13   2.55       Balang Padang   1.35   1.46   1.66   1.89   2.08       Berak   [1] Upstream of   6.41   6.97   7.90   8.83   10.70       Discharge Station   3.76   4.44   5.58   6.72   9.00       Campung Paloh   Campung Pa		Kerian		0.32	0,41	0.57	1	<u> </u>	lan Bharu (ew Pump Station)	Downstream	2.55m/s (220,000 m/day)	
Satistic Padang   1.35   146   1.66   1.89   2.08   Editar Idns Shah II   Upstream of Perak   Editar Idns Kin   Editar Idns Kemasin   10.16   11.41   13.48   15.55   19.70   Editar Idng Excension   Downstream   Implementation   around 2000		Total		1.20	141	1.87		55				
Perak		Doctor Dodge		35	1.46	3		80.				
Discharge Station   3.76	Seberang Perak	Perak	.1	17.9	6.97	7.90			ilian Idris Shah II	Upstream	Projected water Demand of Perak	Perak
Discharge Station   10.16   11.41   13.48   15.55   19.70   Existing Bukit Bunga   Downstream   Implementation     Bukit Bunga   Downstream   Implementation   Bukit Bunga   Excension   Downstream   Implementation     Solid   Sol			Discharge Station [2]Downstream of	3.76	4	5.58			sluk Kepung	Downstream	River Basin in 2020 in 19.70 m/s	
6.25 6.50 0.63 6.77 Existing Bukit Bunga Downstream Implementation Bukit Bunga Extension Downstream Implementation around 2000			Discharge Station [3]Total ([1]+[2])	10.16	11 41	13.48	1 1	-	ampong raion		(1.702.010 m3/day)	
6.25 0.50 0.63 0.77 Existing Burit Burga Extension Downstream Implementation around 2000 N.11	Kemasan/Semera	ik Kemasin						-	Some District	Downstraam		Jabatan Bekalan
	Besut	Besut		0.25	0.50	0.63	0.77	ii œ	xisting bukit bunga ukit Bunga Extension	Downstream	Implementation around 2000	Air Terengganu
		A 5.100		,				<u>/</u> .	11	٠		

Note: In Kulim River System, Mak Sulong Intake is not considered in the model because the priority is given to Kulim Headworiks.

Bold

: Data will be Used for Water Balance Study
: Given Data by Relevant Departments / Agencies
: Catchment Area Factor for Perai Pump Station (0.215=1.29/600)
Assumed Catchment Area at Sungai Kulim Headworks (1.29 km² = at Ara Kuda River Discharge Station)
Assumed Catchment Area at Pump Station (600 km² = at 13km Upstream of Estuary)
Source: National Water Resource Study (1982)

Table 1-15 Estimation of Domestic and Industrial Water Demand of Perak River

Unit (m³/s)

Water Demand to be Used for the Study <total capacity=""> ([1]+[3])</total>	6.41	3.76	10.16	10.70	9.00	19.70
Capacity of Water Other Intakes to be (Estimated) [3] the <total< th=""><th>4.21 (=10.16x[2]-[1])</th><th>2.47 (=10.16x[2]-[1])</th><th>6.67</th><th>6.29 (=19.70x(2)-(1])</th><th>5.29 (=19.70x[2]-[1])</th><th>11.58</th></total<>	4.21 (=10.16x[2]-[1])	2.47 (=10.16x[2]-[1])	6.67	6.29 (=19.70x(2)-(1])	5.29 (=19.70x[2]-[1])	11.58
ູ ຂ	63.0% (=10.	37.0%	100.0%	54.3% (=19	45.7%	100.0%
Capacity of Upstream / Confirmed Downstream Intakes [1] Ratio [2]		1.29	3.49	4.4	3.71	8.12
	Upstream Confirmed Intakes	Downstream Confirmed Intakes	Total	Upstream Confirmed Intakes	Downstream Confirmed Intakes	Total
Capacity	0.01 41.0 50.2 0.7	0.00 148.00 44.00	3.49	0.01 0.29 3.48 0.03 0.15 0.45	0.58 1.74 0.12 7.27	3.71
Location from Capacity River Discharge Station	Upstream Upstream Upstream	Downstream Downstream	Yotal	Upstream Upstream Upstream Upstream Upstream	sub-Total Downstream Downstream Downstream	sub-Total
Confirmed Major Intakes	1997 Air Ganda Kota Lama Kiri Sultan Idris Shab II	Kampung Paloh Teluk Kepung Kampung Gajah		2020 Air Ganda Kota Lama kiri Sultan Idris Shah II Upstream Banding Crik V Bandar Baru Seri Iskandar Upstream	Kampung Paloh Teluk Kepung Kampung Gajah Hilir Perak	

: Proposed Intake : Data will be Used for Water Balance Study : Data Given by Lembaga Air Perak Italic Bold

Data Source : Lembaga Air Perak (Total Water Demand)

Data Source: The Study on the Establishment of the River Basin (Comfirmed Information System in Malaysia (JICA, 1997)
Major Intakes) - Interim Report -

Table I-16 Inventry of Secondary Granary Areas in Relevant River Basins

(1) IADP Pulau Pinang

	<ul> <li>Moda River (Downsti</li> </ul>	ream of La	idang Vi	ictoria)					
Code	Name	Type	Water		eca (ha)	CE (1987.	1989)	Caregory	Intake Location
Number		"	Source	Main	Off	Main	Off	l	from Discharge Station
PP 001	Pinang Tunggal	Pump	SD	710	710.	>50%	>50%	3	Downstream
PP 003	Tesek Gelugor	Pump	SD	221	221	>50%	>50%	3	Downstream
KH 008	Sidam Kapan	Pump	SD	500	500	>50%	>50%	7	Downstream
KH 015	Terat Batu	Pump	SD	28	28	>50%	>50%	7	Downstream
KH 035	Sidam Kiri	Punio	SD	219	219	>50%	>50%	7	Downstream
KH 036	Pekula	Pump	SD	1557	1083	>50%	>50%	6	Downstream
KH 039	Pinang Tunggal	Pump	SD	279	198	>50%	>50%	2	Downstream
KH OD	Pantal Prai / Serukam	Pumo	SD	259	259	>50%	>50%	7	Downstream

	Kulim, Jarak, Peral Ku	rer							
Code	Name	Type	Water	Imigable A	era (ba)	CI (1987-	989)	Category	; <u> </u>
Number			Source	Main	Off	Main	Off	L	from Discharge Station
PP 004	Jarak Tengah	Pump	SD	105	105		<50%		Upstream
PP 006	Sg. Kulim	Gravity	SD	561	561	>50%	>50%		Downstream
KH 009	Sg. Scluang	Gravity	-	134	- 1	Converted to o			Upstream
KH 013	Otak Kerbau	Gravity	SD	193	193	idle_	Idle	!	Upstream

## (2) JADP Kerian / Sungai Manik

#### Kerlan Scheme

Kurau River Intake Location from Discharge Station Upstream Water Imigable Aera (ha) Ct (1987-1989) Category 939)
Off
>50%
<50%
<50%
>50%
>50% Type Name Code Number PK 001 PK 002 PK 003 PK 004 PK 005 PK 008 Main
>50%
<50%
<50%
>50%
>50%
>50%
>50% Main 562 127 278 93 Source SD 562 Gravity Batu Kurau Gravity Gravity Upstream Air Kuning Jelai dan Tambahan 127 278 SD Upstream SD SD Gravity Gravity Upstream 93 Pantai Besar Jemerang Setar Bukit Bertam Upstream 191 Uostream SD 287 Pump 287

	Kerian River		<del></del> .		- 3731	C1 (1007	1000	Catazoni	Intake Location
Code	Name	Type	Water	Imigable A		ČI (1987- Main	Off	Category	from Discharge Station
Number			Source		Off	<50%	<50%	6	Upstream
PK 012	Sg. Segar	Gravity	SD	144	144		<50%		Upstream
PK 013	Sg. Chop	Gravity	SD	26	26	%0%			
PK 014	Sg. Simpol Kiri	Gravity	SD	37	37	>50%	>50%		Uostream
PK 015	Sg. Rambutan	Gravity	SD	76	76	>50%	>50%		Upstream
PK 015	Sg. Damak	Gravity	SD	22	22	>50%	>50%		Upstream
PK 017	Sg. Berdarah	Pump	SD	101	101	>50%	>50%		Upstream
PK 018	Sg. Nor	Gravity	SD	127	127	>50 %	>50%		Upstream
PK 019	Sg. Garok	Gravity	SO	74	74!	>50%	>50%		Upstream
PK 020	Batu 3, Kg. Medan	Pump	SD	57	57	<50%	<50%		Upstream
	Gua Petai	Pump	SD	33	33	>50%	>50%	2	Upstream
			SD	95	95	>50%	>50%	2	Upstream
PK 022	Bukit Torak/Lubuk Sengga	Constant	SD	66	66	>50%	>50%		Upstream
PK 023	Tapah Hulu	Gravity	SD	65	65	>50%	>50%	1	Upstream
PK 025	Belukar Hantu	Gravity			800	>50%	>50%	_	Downstream
KH 001	Bandar Baharu	Pump	SD.	800		>50%	>50%		Upstream
KH 002	Serdang Bt. 16	Gravity	SD	. 29	26		>50%	_	Upstream
KH 003	Kilang Bt. Kg. Ulu Relau	Gravity	SD	40	40	>50%			
KH 004	Serdang Batu 18	Gravity	SD	40	40	ldle	Idle		Upstream
KH 005	Kg Sg Tengas	Gravity	SD	. 40	40	>50%	>50%		Upstream
KH 006	Kg. Sg. Taka	Gravity	SD	97	. 97	>50%	>50%		Upstream
KH 007	Kg. Berjaya	Pump	SD	150	150	Idle	die		Upstream
KH 010	Ulu Mahang	Gravity	SD	61	61	>50%	>50%		Upstream
KH 011	Rendang Sena	Gravity	SD	1 23	23	>50%	>50%	b 7	Upstream
KH 019	Kg. Lobak	Pump	SD	28	20	>50%	>50%	6 3	Upstream

Sungai Manik Scheme

i	Batang Padang River							TX	m valent da sal	1
Code	Name	Type	Water	trrigable.	Aera (ba)	CI (198)		Category	Intake Location	
Number			Source	Main	Off	Main	Off	1i	from Discharge Station	
Nil				•		-		<u> </u>		

# Table I-16 Inventry of Secondary Granary Areas in Relevant River Basins

(3) IADP Seberang Perak

Code	Perak River (Downstrea Name	Type	Water	Imigable A	era (ha)	CI (1987-		Category	Intake Location
Number		-76 -	Source	Main	on	Main	Ott		from Discharge Statio
PK 026	Bdg Jeliang	Gravity	SD	47	37	>50%	>50%		Upstream
PK 028		Gravity	LS	250	115	Idle	ldie		Upstream
PK 029		Gravity	LS	42	42	>50%	>50%		Uostream
PK 031	Bdg. Ketiou	Gravity	SD	49	20	<50%	<50%	1	Upstream
PK 032		Gravity	SD	180	150	>50%	>50%	6	Upstream
PK 033		Gravity	SD	100	100	>50%	>50%	6	Upstream
PK 037	Kroh Hulu	Gravity	SD	15	15	>50%	>50%	3	Upstream
PK 038	Bendang Telang	Gravity	SD	70	70,	>50%	>50%		Upstream
PK 039	Bendan Ulu Kenas	Gravity	SO	27	27	>50%	>50%	2	Upstream
PK 040	Kota Lama Kiri	Gravity	SD	112	142	>50%	>50%	6	Upstream
	Salong	Pump	SD	180	180	>50%	>50%	6	Upstream
	Chepias	Gravity	SD	90	90.	>50%	>50%	2	Upstream
	Bendan Kuala Dal	Gravity	SD	20	20	dle	Idle	2	Upstream
	Parit Bukit Cupak & Merus		SD	220	220	<50%	<50%	2	Upstream
PK 059		Pump	SD	746	746	>50%	>50%	2	Upstream
PK 024	Bukit Tunggal	ì	SD	2309	2309	<50%	<50%		Upstream
PK 055		, .	SD	754	751	<50%	<50%	! 7	Upstream
PK 056	Bots/Lambor	Pump Pump	SD	137	137	<50%	<50%		Upstream
PK 057	Senin	,	SD	168	168	<50%	<50%		Upstream
PK 058 PK 059	Lambor Kiri Parit Bukit & Merun	Pump Pump	50	220	220	>50%	>50%		Upstream

(4) IADP Kemasin/Semerak

	Kemasin River									
-	Code Name	Type	Water		Aera (ba)	CI (198		Category		ı
1	Number	ļ	Source	Main	Off	Main	Off		from Discharge Station	ĺ
	No.			-	-	-	-		·	1

## (5)1ADP Ketara (Besut)

### (i) Besut Barrage Sub-Scheme

	Besut River							151	
Code Number	Name	Type	Water Source		era (ha) Off	CI (1987- Main	1989) Off	Category	Intake Location from Discharge Station
TR 001	Telabak	Gravity	LS	120	60	<50%	<50%	2	Upstream  Downstream
TR 004	Pelagut	Pump	SD	650	650	<50%	<58%	6	(Water Source is not Besut River but Pelagat River - Tributary of Besut River)

#### (ii) Angga Barrage Sub-Scheme

		Angga River							<u>·</u>	
1	Code	Name	Type	Water	Inigable			1-1989)	Category	Intake Location
1	Number			Source	Maio	Off	Main	Off		from Discharge Station
	Nil					-	-	-	-	

Category 1: Schemes to be converted to high value crop cultivation under irrigated condition,

Category 1: Schemes to be converted to high value crop cultivation under irrigated condition,
Category 2: Schemes to be converted to tree crop cultivation,
Category 3: Schemes to introduce two-cropping system planting paddy during main season and short-term
annual crops during the off-season
Category 4: Shemes to be converted to animal feeding crop cultivation or cattle raising fields,
Category 5: Schemes to be converted to freshwater fish culture ponds,
Category 6: Schemes to be maintained as secondary granary areas,
Category 7: Schemes to be maintained as paddy cultivation areas within a definite period of time for social
welfare purposes and thereafter to be further evaluated, and
Category 8: Schemes to be converted to housing/industrial and other uses.

: Sufficient for Double Cropping i.s Ci Limited for Single Cropping

Souce : Feasibility Study on Rationalization and Crop Diversification in Non-Granary Irrigated Areas in Malaysia (JICA 1990)

: Cropping Intensity Bold : Secondary Granary Areas Used for the Water Balance Model

Bold : Secondary Granary Areas Out of the Water Balance Model

Table I-17 Tributaries Catchment Area Factor of Relevant Rivers

Scheme	River System		Catchment Area at Intake Point (km²) [2]	Catchment Area of Tributaries Tributaries (3) Catchment Area (=[2]-[1]) Factor (=[3]/[1])	Tributanes Catchment Area Factor (=[3]/[1])	Data Source		Note
Kerian	Kurau	337	492	<u> </u>	0.46	0.46 Kurau River Flood Mingation (1987) Bukit Metah Reservoir Inflow 1.14 Kurau River Flood Mitigation (1987)	(7861) (7861)	Bukit Merah Reservoir Inflow
Kemasin/Semerak Kemasin	Kenan Kemasin	.89		69	*1.0	0.7 Kemasin-Semerak Integrated Rural Development Project	1	* Catchment Area Factor is (1980) based on Return Flow Factor of KADA
Besut	Besut	787	579	-208		1.Angga River is excluded     2. River Discharge Station is located downstream of the Intake Point	(1988)	1.Angga River is excluded 2. River Discharge Station is located downstream of the Intake Point

Table I-18 River Maintenance Flow to be Used for Water Balance Study

Scheme	River System	Approximate Catchment Area at Intake Point (km²) [1]	Note	Specific River Maintenance Flow of Muda River (m\s/km?) [2]	River Maintenance Flow at Intake Point (m/s) (=[1]x[2])	Data Source of Catchment Arca at Intake Point	
Pulau Pinang	Muda	4,200	4.200 Effective Catchment Area at Muda Barrage	0.00119	9.00	5.00 National Water Resource Study (1982)	(1982)
	Kulim	129	at Ara Kuda	0.00119	0.15	0.15 Hydrological Data Book (DID) (1991)	(186 (188 (188 (188 (188 (188 (188 (188
Kerian	Kerian	1,346		0.00119	1.60	1.60 Kurau River Flood Mitigation	(1987)
Month	Batang Padang	455		0.00119	0.54	0.54 Hydrological Data Book (DID) (1991)	(1991)
Suite in the suite	ام		6	011000	\$ 21	The Establishment of the River	(1997)
Seberang Perak Perak	Perak	14,/43	at Bagan Daton	0.0011		Malaysia - Interim Report -	
Kemasin/Semerak Kemasir	Kemasin	134	34 Melor river is excluded	0.00119	0.16	0.16 Kemasin-Semerak Integrated Rural Development Project	(1980)
Becut	Besut	579		0.00119	69:0	0.69 Besut Flood Mitigation Project	
	Angga	78		0.00119		0.09 F/S of Paya Peda Dam	(1995)

Table I-19 Inflow of Bukit Merah Reservoir (Kurau River Storm River Storm River Storm Stor

<b>6</b>	(Present River Discharge at River Discharge Station)	2> Q(d+in)down	(Present Domestic and Industrial Use water taken from Downstream of River Discharge Station)	e Station)		(m <sup>7</sup> / <sub>5</sub> )
Monty/Iokaya Jan Regum Penaci 3 3 3 4 10 5 8 3 4 10 5 8 3 4 10 5 8 10 10 10 10 10 10 10 10 10 10 10 10 10	Feb. 34 1 2 3 1 2 3 7 7 8 1 2 8 1 1 2 3 1 2 3 1 2 8 1 2 1 3 1 2 3 1 2 1 3 1 2 3 1 3 1	May Jun 1 2 1 1 1 2 4 1 1 1 2 4 1 1 2 4	Jul 3 3 1 1 3 3 1 1 3 3 1 3 3 3 3 3 3 3 3	21 3.4 3.6 6.3 11.0 13.7 3.6 3.1 3.5 3.1 3.5 3.1 3.5 3.1 3.5 3.1 3.5 3.1 3.5 3.1 3.5 3.1 3.5 3.1 3.5 3.1 3.5 3.1 3.5 3.1 3.5 3.5 3.1 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5	Nov DV	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2
As Q(is)down  MoonVludays Jan  Return Period  20  00  00  00  00  00  00  00  00  0	(Present Irrigation Use Water for Schrieban of Arcas (aken from Downstream of Arcas (aken from Downstream of Arcas (aken from Downstream of Arcas (aken from 100 0.00 0.00 0.00 0.00 0.00 0.00 0.00	Condary Granary River Discharge Station)  May Jun 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	2 1 1 2 1 40 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6	2 1 2 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	Nov Dvc  2 3 1 2 1 1  3 0.00 0.00 0.00 0.00 0.00  5 0.00 0.00 0	(m/s)  1 2 3 Annual  0 00 00 00 00 00 00 00 00 00 00 00 00
Compared   Compared	Station Major Tributary between Station and Intake Point of the Stuties and Intake Point of the Stuties Fision Flow from Kertian Rivert=1.5 traines Catchment Area Factor (=0, 14	S> Qm (River Mainte   Maint	Flow)    Jul	2 3 1 2 3 1 2 2 1 2 2 2 2 3 3 3 3 3 3 3	5.4 6.2 No. 2 3 10.5 (4.5 kg)   5.4 6.2 No. 8.7 21 6.1 (4.5 kg)   5.5 6.3 No. 8.7 9.2 7.8 No. 7.8 (5.5 kg)   5.5 6.3 No. 10.7 10.5 No. 10.	(m/s)  N Annual  (1 4.5 4.9 4.6  (N.5 5.2 5.2) 5.0  7.7 5.1 5.2 5.3  N.6 7.5 6.5 6.8  N.6 7.5 6.5 6.8
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Table I-20 Available Water of Kerian River in Present Condition

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Table I-21 Available Water of Batang Padang River in Present Condition

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Table I-23 Available Water of Besut River in Present Condition
Scheme Besut Barrage Condition: Present Water Demand

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for Case 1 Qci+Qci(a-0.20xQ)	Oct : Excess Water from Angga River System in Case 1 (0.7) (m//s)  Oct : Excess Water from Angga River System in Case 1 (0.7) Sound : Beau River Flood Mitigation Project  Oct : Tributaries Catchment Area Factor (=-0.26) Sound : Beau River Flood Mitigation Project  Oct : Now Dec
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Table I-23 Available Water of Besut River in Present Condition

For Case 3
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Table I-24 Available Water of Angga River in Present Condition	
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Scheme Bosut Angga Barrage Condition: Present Water Demand  <2> Q(d+in)down (Present Domestic and Industrial Use Water  (aken from Downstream of River Discharge Station)	1
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Table I-25 Inflow of Bukit Merah Reservoir (Kurau River ) in Future Condition

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Foune Inflow from Major Tributary between River Discharge Station and Intake Point of the Study Area)	n Kerian River(=1,501%) nt Area Factor (±0,46) — Sour	Apr		6 30 42 3x 59 5.	त प्राप्त कर्न कर्न कर्न	4 3.5 50 5.5 6.9 %	7 37 53 61 73 60	3 4.0 5.8 7.0 8.2 7.	X PO 42 .44 37 3
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Table 1-25 Inflow of Bukit Merah Reservoir (Kurau River) in Future Condition

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ure Water Demand	Funce Domestic and Industrial Use Water taken from Upstream of River Discharge Station)	Sep	2 0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.5
or Condition : Ful	(Future De taken fro	Aus	11 0.4 0.8 0.8 0.0 0.9 0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.5
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(Present Domestic and Industrial Use Water taken from Upstream of River Discharge Station)	Aug Sep Oct Nov			02 02 02 04 04 04 04 04 04 04 04 04 04 04 04 04	0.2 6.1 0.2 0.3 0.4 0.3	0.15 0.1 0.2 0.3 0.4 0.4 0.3 0.3 0.3 0.3 0.3 0.3 0.3 0.3 0.3 0.3	
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Table I-26 Available Water of Kerian River in Future Condition

Demand	arge Station) (m'/s)	Jul
River : Keran River Condition : Future Water Demand	(Future Domestic and Industrial Use Water Iaken fromDownstream of River Discharge Station)	3 1 2 3 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2
Scheme Kerian Sub-Scheme Kerian Laut	<2> q(d+in)down \$\frac{1}{4}\$ (m/s)	70 40 175 145 15 100 175 175 175 175 175 175 175 175 175 175
	Cl> Q (Present River Discharge at River Discharge Station)	Month (July   Jun   1.50
	₩	Month Julian Jan Weigen Jan Weigen Period 77 20 77 10 8.6. 10 8.6. 11.2. 4 11.2. 11.

1)b <\$>	<3> q(is)down (Fut	(Future Irrigation Use Water for Secondary Granary Granary Granary Granary Scheme (Cropping Intensity 1705)  Areas taken from Downstream of River Discharge Station) = Water Requirement of Kerna Lou (13,725/kg) 13,725 x KKO x 1707/kg	( <del>4</del> / <del>8</del> )
MANULY FORM 1 1 2 20 6.9 0 5.0 0.9 0 6.8 0	2 3 Feb. 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0	Month/Godys   and   Feb	0.45 0.45 0.45 0.45
\$	(Future Inflow I Discharge Stat	<4> qt (Future Inflow from Major Tributary between River <5> Qm (River Maintenance Flow)  Discharge Station and Intake Point of the Study Area)    Proceedings of the Study Area   Proceedings   Procedure	
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19.9 15.1 25.8 22 21.5 18.6 27.7 22 26.6 25.6 38.6 38. 26.7 27.0 34.1 34. 29.8 31.4 37.9 4.

3.4<u>%</u> 5.55

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Table I-26 Available Water of Kerian River in Future Condition

	River : Nerian River Condition : Future Water Demand	Tuture Domestic and Industrial Use Watter taken from Upstream of River Discharge Station)  (m <sup>3</sup> k)	1   Not   2	(Present Domestic and Industrial Use Wanter taken from Upstream of River Discharge Station)  (m*/s)  Anus Sep Ox Nov Dx
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		dn(q))b <9>	Mondy/10(apy) Jan  Return Perfod  10  00  01  4  4  03  03  04  05  04  05  05  06  05  06  06  07  06  07  07  07  07  07  07	<b>⊗</b>

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Table I-27 Available Water of Batang Padang River in Future Condition (Future Domestic and Industrial Use Water taken fromDownstream of River Discharge Station) River : Batang Padang River Condition: Future Water Demand Kerian / Sg. Manik Sungai Manik <2> q(d+in)down (s/m)(s) Scheme Sub-Scheme (Present River Discharge at River Discharge Station) **\$** Month/10days Jan Return Period

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Table I-27 Available Water of Batang Padang River in Future Condition
Scheme Kornan / Sg. Manik River : Batang Padang River
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<10> ga (Available Water at Intake Point [qa=Q-Q(d+in+is)down+qt-Qm-{q(d+in+is)-Q(d+in+is)}] of the Study Area)	May   May
	Henry Renat

Table I-28 Available Water of Kemasin River in Future Condition

	Scheme : Kemasin/Semerak River : Kemasin River   Sub-Scheme : Jelawat Rusa, Kemasin Hili/Condition : Future Water Demand	: Kemasin River n : Future Wake Demand
0 <1>	(Present River Discharge at River Discharge Station)  (A+in)down (A-in)down (A-in)down (A-in)down	(Future Domestic and Industrial Use Water taken from Downstream of River Discharge Station)
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\$	(Future Irrigation Use Water for Secondary Granary Areas taken from Downstream of River Discharge Station)	
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\$\\\	4> qt (Future Inflow from Major Tributary between River	Q12 : Supplementary Water from Source: Sources
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Table 1-28 Available Water of Kemasin River in Future Condition Kemasin/Semenak River : Kemasin River : Kemasin River : Kejawat Rusa, Kemasin Hill, Condition : Future Water Demand

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Scheme : Kemasin Sub-Scheme : Jelawat	<7> q(d+in)up n arge Station) † 0.00 (m/s)	Apr May May May May May May May May May May	arge Station)	Apr
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Table I-29 Available Water of Besut River in Future Condition

Xi.y         Jun         Jul         Avg         Exp         Cut         Avg         Exp         Cut         Cut <th>                                     </th> <th></th> <th></th> <th></th> <th></th> <th></th> <th>ı</th> <th></th> <th></th> <th></th> <th></th> <th>ļ</th> <th></th> <th>Ì</th> <th></th> <th></th> <th></th> <th></th> <th>í</th> <th></th> <th>ľ</th>							ı					ļ		Ì					í		ľ
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<4-1>qt/Cl (Future inflow from Major Tributary between River <5> Qm (River Maintenance Flow)   Discharge Station and Intake Point of the Study Area	QU.: Excess Water from Angga River System in Case 1 0.2] (m/s) O.C.: Tributaries Catchmant Area Factor (=-0.26) Source: Beaut River Flood Multipation Protect O.C.: Tributaries Catchmant Area Factor (=-0.26) Source: Beaut River Flood Multipation Protect	$\begin{array}{cccccccccccccccccccccccccccccccccccc$
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	(m/k)  Annual  O(x) -1.2  O(x) -1.1  O(x) -0.2  1.3 -0.2  2.7 -0.0
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n River tudy Area)	6) Source Beat River Flood Mingatum Project  May July  -0.3 -0.2 -1.8 -1.8 -1.1 -1.8 -1.2 -1.0 -1.8 -1.9 -1.0 -0.8 -
Future Inflow from Major Tributary between River Discharge Station and Intake Point of the Study Area	ngga River System in It Area Factor (=-0.26)  Apr
<4-2> qUC2 (Future Inflow from Major Tributary between River Discharge Station and Intake Point of the Study As	For Case 2   Qt1 : Excess Water from Angga River System in Case 2   Qt2 : Tributaness Catchment Area Factor (=0.26)   South Month/Iddays Jan   Per   Mar   Air
<+2> qVC2	Mar 2   Q11   Q1
	For Case 2 Out-Out-out-out-out-out-out-out-out-out-out-o

Table I.29 Available Water of Besut River in Future Condition

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	<4-3> qt/C3 (Future Discha	for Case 3 Oct-Oct-626x0) Oct : Excess \	Month ithays Jan Feb	Return Period 1 2 3 1 1 2 20 20 20 20 20 20 20 20 20 20 20 20 2	10 - 11. 51. 60. 00. 01.	4 0.0 0.6 d.X 40.5 3 0.3 0.3 0.3 0.3 40.2	

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for Case 1	<10-1	10-1> qa/C1	ភ្ជ	<b>કે</b> જે	Available Water at of the Study Area)	Water ody Ar	r at IN: rea)	ake Po	ייש (	Own H	<b>₽</b> ).	in+is)x	towo!	qt-Qn	-b)-t	÷in +is	(Available Water at Intake Point [qu=Q-Q(d+in+is)down+qt-Qn-(q(d+in+is)-Q(d+in+is))]) of the Stody Area)	in+is)}	<b>-</b>														(s/, m)	- 5
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Table I-29 Available Water of Besut River in Future Condition

cesut River : Besut River cesut Barrage Condition : Future Water Demand	+is)-Q(d+in+is)}}
Scheme : Besut Sub-Scheme : Besut Barrage	(Available Water at Intake Point [qa=Q-Q(d+in+is)down+qt-Qm-[q(d+in+is)-Q(d+in+is))] of the Study Area)
	<10-2> qa/C2

(m/k)	7 35 104 7 35 302 7 520 214 7 520 214 9 751 252 2 101 2 253
ઝરત	1 212 421 290 1 322 481 356 2 473 620, 463 0 561 729 597 0 698 897 759 9 643 1198 1062 1
ACA!	2 3 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2
3	2 3 1 2 156 164 103 103 179 191: 131 127 256/27/23/34/2/36/2/3 259 26.1 213 19.3 30.3 30.2 259 229 30.3 370 341 290
дX	2 3 1 2 1 1 2 1 1 2 1 1 2 1 1 2 1 1 2 1 1 2 1 1 2 1 1 2 1 1 2 1 1 2 1
Aug	3.5 3.7 3.7 3.7 3.7 3.7 3.7 3.7 3.7 3.7 3.7
	7 1 2 3 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
	May   2   3   4   2   3   4   3   4   4   4   4   4   4   4
	Apr. 2 3 3 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4
	Mar 33 25 2 1 34 25 2 1 44 35 40 30 82 83 91 35 113 83 91 35 113 83 91 35 113
	1
for Caw 2	Model (104.9% Jan 1 2.0% July 1 32.4 2.0 32.4 10 35.7 2.0 32.4 2.0 35.7 3 3.4 4 4.0 5.3 3 53.1 2 3 53.

	(m)/k)	25.0 NS 10.3 N
	Surv.	1 1 2 3 1 2 3 4 4 2 4 2 4 2 4 2 4 2 4 2 4 2 4 2 4
	200	20 147 140 140 170 120 120 120 120 120 120 120 120 120 12
d+in+is}}]		2 7 1 2 3 1
Point [qa=Q-Q(d+in+is)down+qt-Qm-{q(d+in+is)-Q(d+in+is)}]		2 1 2 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3
int (qa <del>xQ-Q(d+in+is</del> )		2-18233425
(Available Water at Intake Po of the Study Area)		Month/Lodges   Jan
<10-3> qa/C3		100   100
	for Case 3	Month/totarys Jan Return Period 20 33 10 83 10 85 10 8

41> Q (Pres Ray   A   A   A   A   A   A   A   A   A	Table I-30 Available Water of Angga River : Angga River   Scheme   Besut   Scheme   Besut   Angga Bafrage   Condition : Future Water Demand   Sub-Scheme   Scheme   Scheme   Scheme   Condition : Future Water Demand   Condition : Future Water Demand   Cap (d+in)down   (Future Domestic and Industrial Use Water Discharge Station)	(1975年の第三元の第三元の第三元の第三元の第三元の第三元の第三元の第三元の第三元の第三元
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<3>q(in)down (Future Imigation Use Water for Secondary Granary Areas taken from Downstream of River Discharge Station)	1	4> qt (Pature Inflow from Major Tributary between River	Feb   Mar   Mar   Apr   1969   Jun   Jul   Aug   Sep   Get
	Month Jolish Reum Period 10		Month/10Juys Jan Return Period 20 10 10

Table I.30 Available Water of Angga River in Future Condition

	(3/,W)	Now   2   Annual	•
River : Angga River Condition : Future Water Demand	(Future Domestre and Industrial Use Water taken from Upstream of River Discharge Station)	Novembriolable   Land   Note	(Present Domestic and Industrial Use Water taken from Upstream of River Discharge Station)
Scheme : Besut Sub-Scheme : Angga Barrage	qu(nt+b)p <7>  qu(nt+b)p <7>  y  (s/m)(m)	2	dn(ui+b)Q <6>
	<6> q(is)up (Future Impation Use Water for Secondary Granary Areas taken from Upstream of River Discharge Station)	2 1 Not 1 2 1 1 2 1 Not 1 1 1 2 1 Not 1 1 1 1 2 1 Not 1 1 1 1 2 1 Not 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	<8> Q(is)up (Present Irrigation Use Water for Secondary Granary Areas taken from Upstream of River Discharge Station)
	(i)b <\$>	Month (OLLys Lun   Month (OLLys Lun   Month (OLLys Lun   OLly Collys Lun   OLlys Lun   Oll	j} \$\$

(s/m) (m/s)

<10> qa (Available Warer at Intake Point [qu=Q-Q(d+in+is)down+qt-Qm-(q(d+in+is)-Q(d+in+is))] of the Study Area)

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9	10.0 7.5 4.7 2.2 1.6 1.1 1.3 1.0 1.7	\$1 TT 9	10 17 17	1.7	20 13 24	× .	21 1.6 1.7	× -		100		51. 33. 6	6.9 336 17.2	134 145	5.1
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12.9 4.7	47 K.I 45 3	2. 2.1 2.1	2.5 2.K: 2.7		7	9.			07	1X	3	•	7 X/61 A/0	6 55 X	7.
~ 2	10.1	8.6 A.9.9		3.0	J.	6.1.				401	3	=	11 244 11	F1 _ Z	5

Table I-31 Sample of Paddy Field Water Balance Study to Estimate Effective Rainfall

Pulau Pinang Scheme, Sungai Muda Sub-Scheme, Block M1

f. Rainfal	t Ef	Spillout	Drain	later E	V L	omsumptive Use	Rainfall	lo. of Day	ige	St	Month	Year
7.0	0	0.0	0.0	50.0		67.3	7.0	10	1		<u>_</u>	1993
28.5	0	0.0	0.0	50.0	} ]	67.3	28.5	10	2		1	1993
23.0	0	0.0	0.0	50.0	) 1	74.0	23.0	11	3		i	1993
0,0	0	0.0	0.0	50.0	)	64.9	0.0	10	1		2	1993
0.0	0	0.0	0.0	50.0	)	64.9	0.0	10	2		2	1993
14.0	0	0.0	0.0	50.0	•	52.0	14.0	8	3		2	1993
0.0	0	0.0	0.0	50.0	6	61.6	0.0	10	1		3	1993
61.6	0	0.0	40.9	190.9	6	61.6		10	2			1993
0.0	0	0.0	0.0	150.0	7	67.	0.0	11	3			1993
62.3	0.	0.0	13.2	63.2	8	62.		10	1			1993
32.	0.	0.0	0.0	150.0	8	62.		10	2			1993
6.			0.0	150.0	8	62.		10	3			1993
39.			0.0	150.0	2	65.		10	1	5		1993
31.	0.	0.0	0.0	150.0	2	65.		10	2	5		1993
71.			75.0	225.0	7	71.		11	3	5		1993
50.			0.0	150.0	1	65.	50.0	10	1	6		1993
53.			0.0	150.0	. 1	65.	53.0	10	2	6		1993
20.			0.0	150.0	. 1	65.	20.0	10	3	6		1993
65.			45.7	195.7		65.	) 111.0	10	1	7		1993
59.	0.0		9.0	150.0		65.	59.0	10	2	7		1993
71.			75.0	225.0		71	194.0	11	3	7		1993
65	0.0		9.5	159.5		65	75.0	10	}	8		1993
37	0.6		0.0	150.0	.5	65	37.0	10	2	8		1993
72	0.0		0.4	150.5		72	72.5	11	3	8		1993
0	0.0		0.6	150.0	.7	65	0.0	10	i	9		1993
0	0.0		0.0	150.0		65	0.0	3.6	2	9		199
0	0,0	=	0.0	150.0		65	0.0	10	3	9		199
0	0.0		0.0	150.0	6.6		0.0	10	1	10	3 1	199
0	0.0		0.0	150.0	5.6		0.00	10	2	10	3 1	199
0	0.0		0.	150.0	2.1			1	3	10	3 1	199
66	0.0			221.3	5.7		0 138.		1	13	3 1	199
36	0.0	-		150.0	5.7		0 36.		2	11	3 1	199
66	0.0			159.3	5.7		0 76.	1	3	11	3 1	199
63	0.0			150.0	1.3		0 63.		1	12	13	199
25	0.0			150.0	1.3		0 25.	1	2	12		199
11	0.0	.0 0	0.	150.0	8.5	_ 78	1 11.	1	3	12		199

Morthern Terrorgann Incgrased Dwylophnern Pro-2 Kemenningement MAN SECTION OF THE SE \$ 5 ° ° Seherang Perak S. 2. 2. Sunger Manik Selection 5 - 65 - 5 - 65 رن! بندر 7 1000 M Table F.32 Relevant Data Used for Estimating Water Requirement (Case 1, Case 2) ភិនិ Compartners C-H Pricing Aurori S-165 S-65 0.00.Nam 150.0 150.0 11.0 14.0 14.0 14.0 14.0 14.0 Assessment of Selected Performance Indicaton for Paddy Unigation Schemes Kenan Compartment Dup Punng Aupert Scients ž Š ₹\$ Penant Auron 8 Ÿ 'n 178 (2000) (2000 3 Pinney August 5-168 Pulau Pinanz OSEM Manuel \$ S Flating Methods
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Table I-33 Relevant Data Used for Estimating Water Requirement (Case 3)

Scheme					Kerian						Besut	Ħ	
<u></u>	Ŭ	Compartment		ŭ	Compartment		Ü	Сотрантеп					-
		Ų.	-		Ų F			G.H					
Meteorological Station	ξ	Pinang Airport		Pig	Pinang Airport	1	Pil	Pinang Airport		1	Kota Bharu Airport	u Airport	:
Presaturation Period Effective		5-165			5-165			\$-165			\$-165	55	
Normal Supply Period		5-65			\$-65			2-65			5 - 65	65	
Derophion (mm)		0.5			». 1.			1.8		!	3.0		
(Data Source)	<b>A</b>	Assessment of Selected Performance Indicators for Paddy Irrigation Schemes	of Selected	Performan	ice Indicato	ns for Pad	dy Irrigatio	n Schemes		North	Northern Terrengganu Integrated Development Project	gano Integ ent Project	rated
				Presatur	Presaturation Requirement (mm	guireme	nt (mm)						
1st Supply (10days)							1		1			350	
Season	Main	ci	Off	Mann		ö	Man		5	Main			
Planting Method	Wet	Ď	Ω	₩et	Š	Š.	ĕ ≪ĕ	<u>ት</u>	<u>λ</u>	ಶ ≽ .	<u>Ş</u> .	ĕ ;	<u>}</u>
Month	Aug	Sep	Mar	Sep	క	Apr	ğ	No.	May	Sep	i S	Nar.	ΑĎ
1. Soil Santration (10days)	150.0	150.0	150.0	150.0	150.0	150.0	150.0	150.0	0.00	0.00	0.00	200	200
2 Standing Water (10days)	100.0	100.0	0.00	0.001	0.00	o. 8	99.	0.001	2.00	3 3	2.6	2 6	2.5
3. ETo (10days)	45.0	41.0	51.0	0.14	40.0	48.0	40.0	42.0	7 5	45.0	42.0	0.20	4 6 5 6
4. Percolation (10days)	2.0	5.0	2.0	18.0	18.0	18.0	0.8	18.0	18.0	30.0		0,0	200
5. Areal Rainfall (Monthly)	101.3	205.6	112.1	205.6	186.5	175.6	186.5	212.6	145.1	208.2	9.66	47.2	6.0
<1/>	٠ د	68.5	37.4	68.5	62.2	58.5	62.2	40.6	48.4	69.4	66.5	15.7	0.1
7 Effective Printel (10dovs)	33.8	68.5	37.4	68.5	62.2	58.5	62.2	50.9	48.4	<del>*</del> .69	66.5	15.7	0.1
8. Requirement (10days)	263.2	227.5	268.6	240.5	245.8	257.5	245.8	239.1	263.6	255.6	255.5	316.3	333.9
(=1+2+3+4-7)													
2nd Supply (10days)						1			100	7		350	
Season	Main		Off	Ma	.8	Ö	Main		5,	Main			
Planting Method	Wet	Š	Ď	Wet	Ų.	Š	, Vet	ς Ω	٠ <u>٠</u>	×et •	Š	<u>ن</u> بۆ	<u>5</u> ,
Month	Sep			ŏ		. [	Ś			ŏ.	F-7	Apr	
1. Soil Saturation (10days)	0.0			0.0			0.0			0.0		0.5	
2. Standing Water (10days)	100.0	ı	•	<u>0.0</u>			0.0	•	•	233	•	33,	
3. ETo (10days)	41.0	ì		900		•	42.0	•		42.0		ج ج م	
4. Percolation (10days)	5.0	,	4	18.0	٠	•	18.0			0.0		0.0	
5. Areal Rainfall (Monthly)	205.6			186.5			212.6			9.661		670	
6. Areal Rainfall (10days)	68.5	٠	•	62.2			70.9			66.5		۳. c	
7, Effective Rainfall (10days)	68.5		*	62.2	•	•	70.9		,	96.5	•	C.5	
8. Requirement (10days)	77.5			8.56	•		89.1		,	103.5	•	1857	
(=1+2+3+4-7)													

Table I-34 Planting Area (Case 1, Case 2)

Cropping Intensity   Crowth   Scheme	Sub-Scheme	Block	Area		Area (ha)	(ha)		Plan	Planting Method (% of area)	kd (% of :	rea)	
Cropping Intensity   Crowth Growth				(ha)		2	m	4	Wet	Wet	Dry	I rans- planting
Cropping Intensity   Growth Growth   Cropping Intensity   Days		•						(130	(120		i	
Kerian Laut         A         2,403         144         96         2163         -         0         6           C         3,960         238         158         3564         -         0         6           C         3,960         238         158         3564         -         0         6           Kerian Darat         E         2,342         984         656         703         -         0         42           G         2,143         133         755         809         -         0         42           G         2,143         138         656         703         -         0         42           G         2,143         1286         857         0         -         0         42           G         2,143         1286         857         0         -         0         60           Manik Sungai Manik         3,602         3,602         0         -         0         100           RBC         H         2,250         1390         -         -         0         100           RBC         Besut Barrage         1         1,343         4,343         -         -					Ö	Suiddo.	Intensit	>.	Growth	Growth		
Kerian Laut         A         2.403         144         96         2163         0         6           C         3.960         238         158         3564         0         6         6           Kerian Darat         E         2.344         984         656         703         0         42           G         2.143         1286         857         0         0         42           G         2.143         1286         857         0         0         42           G         2.143         1286         857         0         0         42           Manik Sungai Manik         3.602         1590         1060         0         0         42           Labu Kubong         2.716         0         2.716         0         0         0         60           Manik Sungai Manik         3.602         3.602         0         -         0         100         42           RBC         4.350         4.365         4.365         -         0         100         0           RBC         4.343         4.343         4.343         -         0         100         0           Besut Barrage         <				-		) (			Days)	Days)	ļ	
B	Kerian	Kerian Laut	A	2.403	14.	96	2163	,	0	9	4	8
C 3,960 238 158 3564 - 0 6 D 3,362 1412 941 1009 - 0 42 E 2,344 984 656 703 - 0 42 F 2,697 1133 755 809 - 0 42 G 2,143 1286 857 0 - 0 60 H 2,650 1590 1060 0 - 0 60 C 2,716 0 2,716 - 0 100 C 2,716 1,343 100 0 Kemasin Kilir 261 261 100 0 Kemasin Kilir 261 261 100 0 C 3,602 1,343 0 - 100 0 C 4,343 1,344 100 0 C 7,145 1,475 1,475 0 - 100 0 C 7,148 730 417 - 100 0	The last		 	4.001	240	160	3601	•	0	9	ব	Φ.
D         3,362         1412         941         1009         -         0         42           F         2,344         984         656         703         -         0         42           F         2,697         1133         755         809         -         0         42           G         2,143         1286         857         0         -         0         60           H         2,650         1590         1060         0         -         0         60           H         2,650         1590         1060         0         -         0         100           A         3,602         3,602         0         -         0         100         0           A         3,602         3,602         0         -         0         100         0         100           A         3,602         3,602         0         -         0         100         0         100           A         3,343         4,345         -         -         0         100         0           Kemasin Hilir         2,61         261         2,716         -         -         0         100			U	3,960	238	158	3564		0	9	4	06
E       2,344       984       656       703       -       0       42         F       2,697       1133       755       809       -       0       42         G       2,143       1286       857       0       -       0       60         H       2,650       1590       1060       0       -       0       60         A       3,602       3,602       0       -       0       100       60         A       3,602       3,602       0       -       0       100       100         A       3,602       3,602       0       -       0       100       0         A       3,602       3,602       0       -       0       100       0       100         A       3,602       3,602       0       -       -       0       100       0       100         A       3,343       4,343       -       -       -       0       100       0         B       1,376       1,376       -       -       -       100       0         B       1,475       1,475       0       -       -       0			. Ω	3,362	1412	941	1009	•	0	42	28	
F 2,697 1133 755 809 - 0 42 G 2,143 1286 857 0 - 0 60 H 2,650 1590 1060 0 - 0 60 C 2,716 0 2,716 - 0 100 C 4,365 4,365 0 100  Semain Hilir 261 261 100 0 Kemasin Hilir 261 261 100 0 T 1,235 1,235 0 - 100 0 T 1,235 1,235 0 - 100 0 T 1,235 1,235 0 - 100 0 T 1,235 1,235 0 - 100 0 T 1,235 1,235 0 - 100 0 T 1,235 1,235 0 - 100 0 T 1,235 1,235 0 - 100 0 T 1,235 1,235 0 - 100 0 T 1,235 1,235 0 - 100 0 T 1,235 1,235 0 - 100 0		Kerian Darat		2 344	984		703	,	0	42	28	
G 2,143, 1286, 857, 0 - 0 60 H 2,650, 1590, 1060, 0 - 0 60 3,602, 3,602, 0 - 0 100 2,716, 0 2,716 - 0 100 4,365, 4,365 0 100 Jelawat Rusa 1,384, 1,384 100 0 Kemasin Hilir 261, 261 100 0 1 1,235, 1,235, 0 - 100 0 3 1,306, 0 1,306 - 100 0 4,475, 1,475, 0 - 100 0		ANCHUM ACME	į (L.	2.697	1133		809	•	0	42	28	30
H 2,650 1590 1060 0 - 0 60 3,602 3,602 0 - 0 100 2,716 0 2,716 - 0 100 4,365 4,365 0 100 Jelawat Rusa 1,384 1,384 100 0 Kemasin Hilir 261 261 100 0  Kemasin Hilir 261 261 100 0 3 1,306 0 1,306 - 100 0 4 1,475 1,475 0 - 100 0 6 2 1,148 730 417 - 100 0			٠. ك	2.143	1286		0		0	8	40	
3,602 3,602 0 0 100 2,716 0 2,716 0 100 4,365 4,365 0 100 4,343 4,343 0 100  Jelawat Rusa 1,384 1,384 100 0  Kemasin Hilir 261 261 100 0  1 1,235 1,235 0 - 100 0 3 1,306 0 1,306 - 100 0 4 1,475 1,475 0 - 100 0 c 2 1,148 730 417 - 100 0		_	ж	2,650		-	0	ı	0	09	40	
2,716 0 2,716 0 100 4,365 4,365 0 100 4,343 4,343 0 100  Jelawat Rusa 1,384 1,384 100 0  Kemasin Hilir 261 261 100 0  1 1,235 1,235 0 - 100 0  3 1,306 0 1,306 - 100 0  4 1,475 1,475 0 - 100 0  e 2 1,148 730 417 100 0	Sungai Mani	ik Sungai Manik		3,602	3,6		,	١.	0	100	0	
LBC       4,365 4,365 0         RBC       4,343 4,343 0         KEmasin       Jelawat Rusa       1,384 1,384 100         Kemasin Hilir       261 261 100         Besut Barrage       1 1,235 1,235 0 - 100         Angga Barrage       1,475 1,475 0 - 100         Angga Barrage       2 1,148 730 417 - 100	manna manna	Labu Kubong		2,716	0	2,716		•	0		0	
RBC         4.343 4.343	Seberang	LBC		4,365	4,365	,		,	0		0	
ak Kemasin Jelawat Rusa 1.384 1.384	Perak	RBC		4.343	4,343			٠,	0		0	
ak         Kemasin Hilir         261         261         -         -           Besut Barrage         1         1,235         1,235         0         -           3         1,306         0         1,306         -         -           4         1,475         1,475         0         -         -           Angga Barrage         2         1,148         730         417         -         -	Kemasin/	Kemasin	Jelawat Rusa	1.384		1	1	•	100	0	0 !	- 1
Besut Barrage 1 1,235 1,235 0	Semerak		Kemasin Hilir	261	261	,	•	•	100	0	0	
3 1,306 0 1,306 4 1,475 1,475 0 Angga Barrage 2 1,148 730 417	Besut	Besut Barrage	ľ	1,235	1,235	0	t	1	100	0	0	_
4 1,475 1,475 0 2 2 1,148 730 417			<sub>C</sub>	1,306	0	_		1	100	0	O	_
2 1,148 730 417 -			4	1,475	1,475	0	,	,	100	0	ا	:
		Апдда Ваттаде	2	1.148	•		i.	٠,	100	0		

Table I-35 Planting Area (Case 3)

Scheme	Sub-Scheme	Block	Area		Area (ha)	(ha)			Planting !	Planting Method (% of area	of area)	F
Scilcing			(ha)	-	7	т	4	Wet	Wet	Dry	Ωry	planting
								(130	(120	(130	(120	
				č	Suiddo	Cropping Intensity	_	Growth Davs)	Growth Days)	Growth Days)	Growth Days)	
1	7	<	2 403	2403	0	0		0	100	0	0	0
Kenan	Kerian Laut	ζ μ	4 001	4001	0	0	1	0	8	0	0	0
(Main Season)	<b>(</b> )	ą C	3.960	3960	0	0	1	0	8	0	o ;	0
		) C	3,362	0	3362	0	1	0	4	0	8 :	0
	Vones Dans	) tr	2 344	0	2344	0	1	0	8	0	2 :	5
	Nellan Danat	Ţ	2.697	0	2697	0	•	0	80	0	50	<b>5</b> 0
		, יַ	2.143		0	2143		0	70	0	30	5
	-	ב	2,650	0	0	2650	,	0	100	0	0	0
	**** / //	V.	2 403	1	0	0	١.	0	0	0	100	<u> </u>
Kerian	Nerian Laur	ζ α	4 001	80	0	0		0	0	0	001	0
(Off Season)	-	a C	3 960		0	0		0	0	0	8	0
	-	, ב	3363		100	0	,	0	0	0	8	<b>O</b>
	W. C.		2 3 4 4	o I	100	0	,	0	0	0	20	0
	Kenan Darat	n t	2,697	) C	100	0	•	0	0	0	91	0
		ن ر	2 143	· C	0	100		0	0	0	8	0
		) II	2,650	0	0	100	•	0	0	0		
Decort	Recut Barrage	:	1.235	1,235	0			100	0	0	0 (	<u>٠</u>
Desuit Constant		۲۰ در	1,306	0	1.306			8	0	0	· C	<b>&gt;</b> (
(Main Season)	. (1	7 (	1 475	1.475	0	ı	•	8	0	0		
	Assert Borroge	, ,	1 148	730	417		•	188	0		0	
	Aligha Daliago	1	1 224	1 235	C	,		80	0	20	0	
Besut	Besut Barrage	<b>→</b> (1	306	3	306	,	,	08	0	20	O	0
(Main Season)	(F	υ <i>z</i>	1 475	1.475	0	,		8	0	8	O	O
	Angea Barrage	<u> </u>	1.148	730	417	· .		08	0	20		0
	mega parago	1										

Table I-36 Breakdown of Irrigation Water Requirement in Case 1 / Irrigation Duty (Kerian Scheme)

	: Kenan
<1> Compartment A	
Water Requirement	Mar Apr 'May Jun
22 12 12 13 13 14 15 15 15 15 15 15 15 15 15 15 15 15 15	01 03 13 27 29 1 2 3 01 03 13 27 29 37 40 49 01 02 12 25 27 33 36 49 01 02 12 25 27 37 36 40 01 02 12 25 28 27 31
Irrigation Duty	01 02: 1.1 2.1 2.0; 22 23 24 12 04 13 12 1J 03' 07 04 02 0.1 0.1 0.5 0.2 0.1 0.1 0.0 0.0 0.0 0.1 0.2 0.3 0.3 0.4 0.5 0.6 0.0 0.1 0.1 0.5 0.6 0.1 0.2 0.3 0.3 0.4 0.5 0.6
	Rainfal
Vater Requirement	May Jul Aug Sep Oct New Dec
Month/Idays Jun   Feb   Roum Period   2   1   2   1   1   2   2   2   2   2	NATE AND TO THE STATE OF THE ST
2 23 20 13	0.1 0.4 1.9 3.8 3.7 4.1 4.3 5.0 5.0 5.0 5.0 6.1 0.1 0.2 0.2 0.6 1.0 1.4 1.2 1.3 1.8 5.7 5.3 5.3 5.3 5.0 1.0 0.1 0.3 1.8 3.6 3.4 3.7 3.9 3.7 2.1 1.2 1.7 1.4 1.0 0.6 0.7 0.1 0.1 0.2 0.2 0.6 1.0 1.4 1.2 0.5 0.7 0.7 0.7 0.7 0.7 0.7 0.7 0.7 0.7 0.7
Diversion Point 0.4 0.3 0.2	0.8 0.9 0.7 0.5 0.5 0.5 0.5 0.4 0.7 0.2 0.2 0.1 0.7 0.2
<3> Compartment C Water Reminiment	ozk Area : 3960 ha
Mount/Glays Jan 2 / Feb Return Period 47 3.0 2.1 1 2 0.0 10 10 10 10 10 10 10 10 10 10 10 10 10	
Transaco Point 0.4 0.3 0.2.  Diversion Point 0.4 0.3 0.2.	01 00 00 03 03 05 05 07 08 09 07 05 05 05 05 04 03 02 01 01
. 14	
Water Rejunement	Mar Apr May Jun Jul 3 Aug 35 2 3 3 3 3 3 3 3 5 50 3 50 5 50 5 50
2. 2. 2. 2. 2. 2. 2. 2. 2. 2. 2. 2. 2. 2	11   08   21   25   37   45   58   58   58   58   58   58   58
n Duty n Point 0.5	03 03 0.5 07 0.8

Table I-36 Breakdown of Irrigation Water Requirement in Case 1 / Irrigation Duty (Kerian Scheme)

Table 1-30 Dreamuwii of Artigation
Į.
Area 1972 ha
Nat Apr 11 2 3 1 1 2 3
1 2 3 1 2 3 1 2 3 1 2 3 1 100 2037 223 221 170 130 84 54 29 13 20 25 38 81 81 81 11 11 11 11 11 11 11 11 11 11
8 12.7 7.7 7.7 11 30. 70 14.4 16.1 17.3 22.5 12.5 12.5 12.5 12.5 12.5 12.5 12.5
<u> </u>
11.4 9.0 5.8 29 1.0 2.8 7.1 1.5 1.4 1.5 1.5 1.5 1.5 1.5 1.5 1.5 1.5 1.5 1.5
70 44° 23 10 24 AS 121 44 137 137 137 137 137 137
1 05 03 02 01 00 01 03 05 00 08 08 08 07 05 05 05 05 0
Aus Sep Oct New DX
Nor Apr 1 May July 2 3 1 2 3 1 2 3 1 3 1 3 1 3 1 3 1 3 1 3
7 2 3 18 18 18 18 18 18 18 18 18 18 18 18 18
20
18   18   18   18   18   18   18   18
24 38 28 24 22 17 21 21 21 19 13 09 05 04 02 05 19 11 14 17 12 08 09 07 08 14 11 11
13 11 07 04 05 11 12 10 17 1
0.5 0.4 0.7 0.1 0.1 0.1 0.1 0.1 0.1 0.1 0.1 0.1 0.1
Mass July Aug Sep Oct Sing Die
2 3 1 2 3 1 5 3 1 5 3 1 5 3 1 5 3 1 5 3 1 5 3 1 5 1 8 10 22 27 2.6 2.0 24 28 3.6 4.3 \$1 3.5
5 27 17: 0.9 06 17: 18 29 33 40 44 38 41 40 43 41 37 27 17 13 06 07 12 13 16 21 25 23 17: 55 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5
24 12 14 15 15 15 15 15 15 15 15 15 15 15 15 15
21 17 11 0.6 0.6 14 15 25 29 33 33 28 27 24 26 24 16 0.9 07 0.3 0.6 11 11 14 18 21 17 12 12 13 13 20 23 14 15 13 10 0.6 0.6 10 10 10 10 13 17 19 14 10 0.9 0.8 14 15 10 10 15 13 10 0.5 0.5 14 15 10 10 10 10 13 17 19 14 10 0.9 0.8 14 15 10 10 10 13 17 19 14 10 0.9 0.8 14 15 10 10 10 13 17 19 14 10 0.9 0.8 14 15 10 10 10 13 17 19 14 10 0.9 0.8 14 15 10 10 10 13 17 19 14 10 10 10 10 10 10 10 10 10 10 10 10 10
0x 05 05 13 14 33 36 23 26 32 20 15 15 15
Ì
) 103
Sept. Mar. Age May Jun Jul Awg Sept. Co. 3 Awg
31 1 2 3 1 2 3 1 2 3 1 2 3 1 4 2 3 3 3 5 6 6 6 6 6 6 7 15 15 18 20 24 22 22 32 32 32 32
44 40 33 24 13 00 06 12 13 20 20 20 20 30 30 30 28 28 20 15 07 03 07 15 15 15 19 25 20 17 25 26 27 24 30 30 31 32 32 32 32 32 32 32 32 32 32 32 32 32
22 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
28 30 25 21 14 08 08 08 10 11 12 24 25 23 21 25 24 21 16 16 11 10 63 01 07 13 13 13 16 18 19 15 15 12 12 12 12 12 12 12 12 12 12 12 12 12
13. 14. 0.9. 0.3. 0.3. 1.0. 1.0. 1.0. 1.0. 1.0. 1.0
2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2
OX OX OX OX OX OX OX OX OX

Table I-36 Breakdown of Irrigation Water Requirement in Case 1 / Irrigation Duty (Kerian Scheme)

Table 1-30 Dicardonii	1 111 154 1154 1154 1154 1154 1154 1154	
I Canada	Area 1140 ha Condition : Case 1	
Subscribe Kerjan Darat, Kerjan Laut		nxity 180%)
	Station 4905023 Area : 2650 ha	
	May Jun Jul Aug	Oct Now Dec Annual
Jan Feb Star		1 10 22 24 30 22 23 34 52 40
1	34 40 40 40 54 54 53 48 45 45 45 31 33 13	25 C4 C5 C7 C7 C7 C7 C7 C7 C7 C7 C7 C7 C7 C7 C7
47 50 42 35 24 13 06 08 14 15		27 20 22 26 21 25 33
THE STATE OF THE S	29. 39. 35. 33. 29. 24. 23. 1.6. 1.4. 0.5.	20
31 32 27 23 15 0X 04 07 13 13	29 3.1 2.8	15 17 19 22 16 11 17 18
24 21 18 11 06 03 07 12	29 23 24 30 25 25 25 13 15 25	(Mcha)
		80 80 90 50 50 50 50 50 50 50 50 50 50 50 50 50
Impation Duty   08 08 07 05 04 03 91 02 03 93 93	03 97 68 98 97 98 98 05 96 96 95 95 94	
<10> Total for Kerian Darat Sub-Scheme	Area: 9834 ha	(m/s)
Water Requirement		
Feb	May Jun Jun 2 3 2 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3	4 1 2 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
3) 1 2 3 1 6	ľ	10.5 10.5 10.0 10.0 10.0 10.0 10.0 10.0
	13.4 12.5 13.4 16.6 16.0 14.9 12.3	100 114 XA
22 12 104 A2 63 44 46 39 A2 KO 90	110 110 110 100 100 100 100 110 110 110	7.1 6.8 6.2 7.3 6.3
103 99 37 58 43 44 50 00 77 65 65 65 65 65 65 65 65 65 65 65 65 65	10,3 10,2 8,8 9,0 11,2 10,2 9,6 7,5	5.4 4.K
53, 41 30 35 31 55 70	74' V.1 78 77 SA 38 SA 20 20	
Privative Day		01 04 04 04 04 07 07 07 06 07 08 07 08
Diversion Point 0.7 0.6 0.4 0.3 0.3 0.2 0.3 0.5 0.6		
<11> Total for All Kerian Scheme	Area : 23400 ha	(x/m)
		Oct Nov Dev Annual
MONDVIOLENS Jan Feh Mar Apr		1 2 3 1 2 3 1 2 3/Am
19. 0.22 4.21 2.4 2.20 26.2	392; 37,1 31.4 35.8; 41.6 41.3	16.1 15.7 21.0 24.2 26.9 34.0 36.2 27.3
11.2 6.6 K.4 11.9 21.0 24.5 29.3	31.8 26.0 29.9 34.7 34.0 28.1 35.0 35.0 35.0 5.1	#12/06/14/9   144   1511   164   164   204   265   263   220   164   175   164
21.7 18.9 18.5 8.7 5.3 7.2 10.7 19.4 22.0 23.8 18.6 20.8 18.8 20.8 23.8 23.8 23.8 23.8 23.8 23.8 23.8 23	28.71 26.2 18.7 21.4 26.8 23.8 20.1 15.8 11.0 26.8 21.5 16.0 18.5 21.4 20.4 17.4 13.3 9.0	124 11.1 13.5 14.4 14.9 20.7 22.0 17.6 11.6 93 10.5 10.7 15.7 16.6 13.8
64 40 59 96 176 193	227' 121 120 142' 162 (5.1 131' 94 62 45 20 11	N()
		20 SO CO CO AO AO AO AO AO CO

Table I-37 Breakdown of Irrigation Water Requirement in Case 1 / Irrigation Duty (Sungai Manik Scheme)

Condition : Case	our sec. Demant Cronoing Pattern, Cropping Intentity 180%)	(Billies Ver I Textife Control of	
: 631X Da			
Area		on Kubong	
Suncai Manik		: Sungai Manik, La	
 Schamo	OCHOLINE.	Sub-Scheme	

J. All Suggest Manik Scheme	Rainfall Station 4010;38	Area : 6318 ha
		No. Our Co.
Water Schutzung in Feb. Peh. Mar	Apr May	
Propried Louis Parked	~	23 53 53 65 53 53 55 55 55 55 55 55 55 55 55 55 55
20 2.7 5.3 9.8 11.6 12.0 15,8 16.2 14.8 9.	11.2 10.4 12.8	27 CT 28 21 42 81 95 (0.7 12.2 14.8 15.2 10.1 9.8 88 9.0 65 65 55 55 55 55 55 55 55 55 55 55 55
(0 21 51 96 110 114 149 149 132 86 101 938 101 938 132 86 101 938 132 86 101 938 132 86 101 9000000000000000000000000000000000	7.4 6.9 2.4	18.   18
20 00 00 90 90 00 00 00 00 00 00 00 00 00	67 63 7.5	5.8 42 29 15 40 76 83 48 111 131 131 131 131 131 131 131 131 13
4 10 129 123 100 64	3.9 5.8 19.8	3.7 2.6 13 3.9 4.6 3.4 4.4 4.0 13 8.9 5.9 5.7 4.4 4.5 2.7 2.7 2.0 1.8 1.6 0.9
2 0.7 46 8.7 9.1 9.8 12.0 11.1 8.7 5.5 6.1 5.5	4.5 4.3 4.9	

Table I-38 Breakdown of Irrigation Water Requirement in Case 1 / Irrigation Duty (Seberang Perak Scheme)

 Condition: Case 1	Copposed Internation District Copposed Internative 180%)	(Eff 45%), reesent cropping l'utterni, cropping
s 70% ha		
Ama		
. Cabacana Perak	Contain Summer	(40 (41
1	30000	

m/s)	13.4 11.9 9.7 7.9 7.7 7.7	<b>6</b>
S/m)	14.6 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.	9.0 4.0
	2. 2. 2. 2. 2. 2. 2. 2. 2. 2. 2. 2. 2. 2	90 90 90 90 11
2	1 2 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	٠ C
	4 4 4 5 8 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6	ŝ
	11.5 HZ 11.5 HZ 11.5 HZ 12.0 10.5 8.5 V.7 8.7 8.1 7.5 8.7	Ö
1	3.00 16.0 12.4 14.8 11.5 13.4 11.5 12.0 8.7 10.8 7.5	
	14.5 12.3 11.1 10.0	13 09 11
	255 194 143 255 194 143 256 184 134 256 183 123 250 168 113 251 161 113 211 149 100	<u>:</u>
	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	
-	3.79 0.00 11.1 25.0 0.00 11.1 25.4 0.00 10.0 25.4 0.00 10.7 22.3 0.00 10.4 21.9	0.6
	Aug. 159 1 1 2 3 1 2 3 1 2 3 1 2 3 1 2 3 1 2 3 1 3 2 3 1 3 2 3 3 1 3 3 3 3	0.0
	(	<u>.</u>
	A 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	0.3
	183 115 183 115 183 27 183 27 112 2.1 112 2.1 112 2.1 112 2.1	7 0
	-199 <b>2</b> 3 = 21	×
g	3. 18.0 18.0 18.0 18.0 19.0 19.1 19.0 10.9 10.9	Ö.X
Area: x70x ha		č
Area	100 14.1 15.5 14.1 15.5 11.0 11.5 10.0 10.1 8.4 7.9	, X
	C XX XX XX XX XX XX XX XX XX XX XX XX XX	0.0
4009096	May	10 97: 09 09 0x 0x 0x 0x 0x 0x 0x 0.5 0.5 0.3 0.1 0.0 06 14 1.
Station 4	3388	12.0
Rainfall Sta	2 136 5 130 5 136 7 136 7 136	1 9
Raur	25 17.0 5.5 1.5 1.5 1.5 1.5 1.5 1.5 1.5 1.5 1.5	-
	lar Apr   Ap	0.5
	A 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.	0.0
	000 000 000 000 000	0.0
ي	6 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	10
Schem	6 4 2 6 4 4 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	4
erak !	1 2 3 1 2 3 3 4 1 2 3 4 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	É
rang F	15.8 12.8 12.8 7.3 7.3 7.3	è
l Sebe	Mater Requirement Month/10days Jan Return Persod 10 10 23 3 3	Duty
<1> All Seberang Perak Scheme	Water Requirement Month/10days Jan Return Perrod 20 10 10 3 4 4 2	Trigation Duty  Procession France 69 06 04 01 00 00 00 05 01 12
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Table I-39 Breakdown of Irrigation Water Requirement in Case 1 / Irrigation Duty (Kemasin River System)

Scheme : Kemasin/Semerak Area : 1645 ha Condition : Case 1 Sub-Scheme : Kemasin (Jerawar Rusa, Kemasin Hilir) (Eff.45%, Present Cropping Pattern, Cropping Intensity 180%)
Cobyst         Lan         Feb         Mar         Apr         Apr         Any         Jun         Jun         Jun         Any         Any<
15 16 20 18 10 06 01 00 00 14 31 30 22 13 16 14 10 09 09 08 09 06 03 02 01 00 15 15 10 07 08 08 08 08 08 08 08 08 08 08 08 08 08
Bainfall Station 6024074 Area : 261 ha
Month/lidders   Her   Her   Mair   Apr   2   May   3
1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
nDuty   Penni nx nx nx nx ny nx no na na