PRE-STRESSEDE CONCRETE SUPERSTRUCTURE

I. General

1. Outline

The "Drawings" may be used in the case that the budgeting for an implementation plan be required by the Ministry of Public Works in Chile, or as a kind of the data in preliminary design for engineers. So it must be recognized that they are not considered as a detailed design.

2. Specifications

The design is based on the following specifications.

- 1) "Standard Specifications for Highway Bridges" adopted 1992 and published by the American Association of State Highway and Transportation Officials 444 North Capitol Street, N. W., Suite 249 Washington, D.C. 20001.
- 2) "Specifications for Highway Bridges" adopted 1994 and published by Japan Road Association.

3. Contents

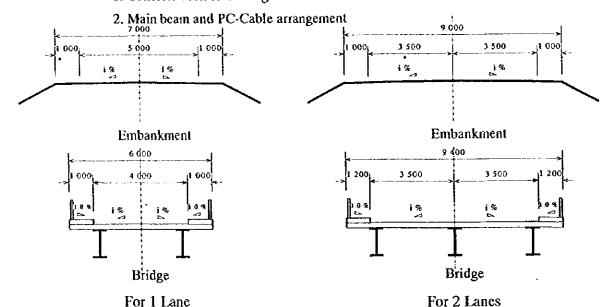
This set of the part for the steel superstructure constitutes of the following chapters.

- I. General
- II. Design Conditions
- III. Table of Design Variables
- IV. Drawings
- V. Calculation report (Input and Generalization table)
- VI. Material List

4. Composition of the Drawings

Each set of drawings for a steel superstructure consists of;

1. Concrete deck slab and general cross-section



5. Instruction

- 1) The standard bridges dealt here are straight and right-angled only, hence some modifications and consideration should be added to the standard design, when applied to skewed or curved bridges.
- 2) All dimensions on the drawings are in "mm" unless otherwise stated.
- 3) The standard bridges are intended to be used for rural bridges.
- 4) The number of lanes are available for one or two, and the width for these lane numbers are shown at below-left
- 5) Cross-fall on the road-way is 1.5 % and on the side-walk is 1.0 %.
- 6) Curb height and width are 250 mm and 200 mm respectively.
- 7) Railings are 1100 mm high.
- 8) The minimum thickness of pavement is 50 mm at both sides of the road-way, and it is thickest at the center according to the cross-fall.
- 9) All the drawings of the standard bridges are made by use of CADD System program separately worked out for the project.
- 10) Combinations of span lengths and number of lanes are shown below.

	PC													
	1	Lane	2	Lane										
Span Length (m)	PRE	POST	PRE	POST										
14	•	<u> </u>	•											
16	•		•											
18	•		•											
20	•		•											
22	•	_	•	T										
24	•	•	•	•										
26		•		•										
28		•		•										
30		•		•										
32		•		•										
34		•		•										
36		•		•										

- 11) Structures of which applicable span length are not in the drawings can be designed using the CADD system program.
- 12) Anchorage for PC tendon and drainage device are not included in the drawings, because anchoring system shall be determined by client and contractor

II. Design Condition

1. Design Method: Allowable Stress

2. Loading

1) Dead Loads

Plane Concrete $W_c = 2.30 \text{ t/m}^3$

Reinforced Concrete: $\gamma_C = 2.50 \text{ t/m}^3$

Steel

 $\gamma = 7.85 \text{ t/m}^3$

Pavement

 $y = 2.30 \text{ t/m}^3$

Soil

 $: \gamma_S = 1.80 \text{ t/m}^3$

2) Horizontal Force of Railing: $W_B = 0.050 \text{ t/m}$, $W_L = 0.020 \text{ t/m}$, h = 1.100 m

3) Sidewalk Live Load

 $Lc \le 7.6 \text{ m}$

 \rightarrow W_p = 0.415 t/m²

Le; Span Length

 $7.6 \text{ m} < \text{Lc} \le 30.5 \text{m}$

 $\rightarrow W_p = 0.293 \text{ t/m}^2$

30.5 m < Lc

$$Wp = \left(147 + \frac{4464}{Lc}\right) \times \left(\frac{16.76 - (Sw - 0.25)}{15.24}\right) \times \frac{1}{1000}$$

% In case of $W_p > 0.293 \rightarrow W_p = 0.293 \text{ t/m}^2$

Sw; Sidewalk width

4) Live Load: HS20-44(100%)

5) Wind : $W_V = 0.244 \text{ t/m}^2$

6) Earthquake: A = 0.15, Category B

3. Materials

Slab Concrete

: H-30, $f_c' = 250 \text{ kg/cm}^2$, $E_c = 2.50 \times 10^5 \text{ kg/cm}^2$

Beam Concrete : H-40, $f_c' = 350 \text{ kg/cm}^2$, $E_c = 3.01 \times 10^5 \text{ kg/cm}^2$

Reinforcing Bar : A63-42H, $f_v = 4200 \text{ kg/cm}^2$, $f_{sa} = 1690 \text{ kg/cm}^2$, $E_s = 2.10 \times 10^6 \text{ kg/cm}^2$

Concrete Cover : 3.0 cm (Lateral Beam 2.5cm)

Anchor Bar

: A44-28H, $f_y = 2800 \text{ kg/cm}^2$, $f_s = 1400 \text{ kg/cm}^2$

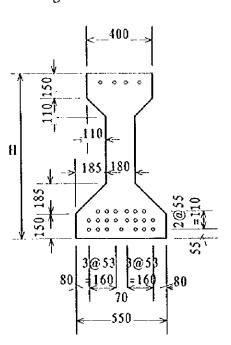
PC Cable

: 1-12.7 (Pre-tensioned), 7-12.7 (Post-tensioned)

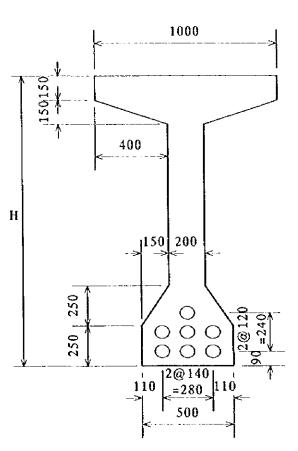
 $f_{pd} = 18980 \text{ kg/cm}^2$, $f_{py} = 16100 \text{ kg/cm}^2$, $E_s = 1.97 \times 10^6 \text{ kg/cm}^2$

4. Beam Type

1) Beam Configuration and dimensions







Post-tensioned PC beam

2) The distance from end of girder to center of bearing shoe

Standardized that distance are as follows.

Span :L _c (m)	$L_{c} < 20$	$20 \le L_{\rm C} < 30$	30 ≤ L _c
Distance (mm)	300	350	400

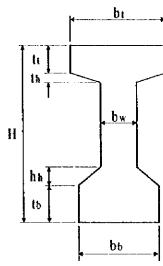
3) Concept for beam derivation

Pre-tension PC girder: It is considered that would be available to suit for variable width of bridge then decided 1.5m distance from center to center on economical competition which are 1.0m, 1.5m and 1.8m.

Post-tensioned PC girder: They were decided by economical reason with considering beam height.

III. Table of Design Variables

- 1. Le; Span Length (m)
- 2. Ly; Girder Length (m), Lower is the distance from end of girder to center of bearing shoe (m)
- 3. E_L; Thickness of the deck slab (mm)
- 1. R.Bar; Diameter and Pitch of Main Reinforcement Bar for Concrete Deck Slab (mm) [Upper row]
 Diameter and Pitch of Distribution Reinforcement Bar for Deck Slab (mm) [Lower row]
- 4. Main Gird.; Number of Main Beam
- 5. Main Gird.; Spacing of Main Beam (m)
- 6. H; Beam Height (m)
- 7. b.; Width of Top Flange (mm)
- 8. t_c; Thickness of Top Flange (mm)
- 9. th; Hunch height of Top Flange (mm)
- 10. b, ; Web Thickness (mm)
- 11. h, ; Hunch height of Bottom Flange (mm)
- 12. t_b; Thickness of Bottom Flange (mm)
- 13. b_b; Width of Bottom Flange (mm)
- 14. N_T; Number of Cross beam
- 15. ST; Distance of Cross beam (m)
- 16. Nd; Number of PC-cable (Pre-tensioned) or Duct (Post-tensioned)
- 17. XB; Distance of Bond Control point for Pre-tensioned Beam (m)
- 18. NB; Number of Bond Control for Pre-tensioned Beam
- 19. N1; Number of PC-cable at Top Flange
- 20. N5,N6,N7; Number of PC-cable at Bottom Flange
- 21. CS; Diameter of PC-cable or Duct (mm)
- 22. RB3; Diameter of reinforcement Bar for Cross Beam side (mm)
- 23. RB4; Diameter of reinforcement Bar for Cross Beam bottom (mm)
- 24. RB6; Diameter(mm) and Number of reinforcement Bar for Main Beam side
- 25. A.S.B.; Diameter(mm) and Number of Anti-seismic Bars
- 26. Vc ; Concrete Volume(m³) of Cast-in-place Member (Deck-slab and Cross-flame)
- 27. Wc; Weight of Reinforcement Bar at Deck-slab and Cross-flame (kg)
- 28. W_c/Vc; Unit Weight of R.Bar for Unit concrete volume (kg/m³)
- 29. V_B; Concrete Volume(m³) of Main Beam (Pre-stressed Member)
- 30. W_R; Weight of Reinforcement Bar (kg) [Upper row]
 - W_P; Weight of PC-cable (kg) [Lower row]
- 31. W_R/Vc; Weight of Reinforcement Bar for Unit concrete volume (kg/m³)
- 32. Rd(t); Dead Load Reaction Force per Each Girder at One Bearing, (ton) [Upper row]
 - RL(t); Live Load Reaction Force per Each Girder at One Bearing, (ton) [Lower row]



1-Lane																																		
INDEX	L _C (m)	L _V (m)	EL	R. Bar	Mai	in Gird	H(m)	bt	tt	th	bw	hh	tb	bb	N ₃	ST	Nd	XB	NB	NIN	5NO	N7	cs	RB3	RB4	RBe	A.S.B.	Ve	Wc	Wc/Ve	V _B	W_R	W_R/V_B	Rd(t) RL(t)_
	1	2	3	4	5		7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	<u> </u>	22	23	24	25	26	27	28	29	30	31	32	33
1-PRE-L14_n4	14.000	14.600	170	φ 12@125	4	1.500	0.700	400	150	110	180	185	150	550	1	7.000	17	2.100	9	2 (8	7	70	φ 12	φ 22	2 φ 12ı	12 φ 22π2	29.1	4444	152.9	15.2	1935	127.1	13.150
		0.300	1	φ 12@175															7			<u> </u>				<u> </u>						769	50.5	15.021
1-PRE-L16_n4	16.000	16.600	170	φ 12@125	4	1.500	0.800	400	150	110	180	185	150	550	1	8.000	19	2.800	11	2 2	8 2	7	70	ф 12	φ2	2 φ 12ι	n2 φ 22n2	33.0	4995	151.6	18.5	2202	119.0	15.337
		0.300		φ 12@175															8		\perp											978	52.8	15.370
1-PRE-L18_n4	18.000	18.600	170	φ 12@125	4	1.500	0.900	400	150	110	180	185	150	550	1	9.000	21	3.500	13	2 4	1 8	7	70	φ12	φ2	2 φ 12	13 φ 22n	36.8	5563	151.0	22.1	2631	119.2	17.588
		0.300		φ 12@175															9		_		ļ									1211	54,8	15.616
1-PRE-L20_n4	20.000	20.700	170	φ 12@125	4	1.500	1.000	400	150	110	180	185	150	550	1	10.000	23	4.100	13	2 (5 8	7	70	φ 12	φ2	2 φ 12	n3 φ 22m	40.9	6146	150.2	26.1	2949	113.1	20.047
		0.350		φ 12@175					<u> </u>			_			Ш				10		_	_		<u> </u>	ļ	<u> </u>			<u> </u>	<u></u>		1476	56,6	15.792
1-PRE-L22_n4	22.000	22.700	170	φ 12@125	4	1.500	1.100	400	150	110	180	185	150	550	1	11.000	23	4.600	13	2	5 8	7	70	$\phi 12$	$\phi 2$	2 φ 12	n4 φ 22n	2 44.8	6714	149.8	30.2	3436	113.7	22.503
		0.350	<u> </u>	φ 12@175	_	<u> </u>	ļ	<u> </u>	ļ			<u> </u>	<u> </u>	ļ	Ш				10	\sqcup	4	╀	<u> </u>	<u> </u>	<u> </u>			ļ				1619	53.6	15.919
1-PRE-L24_n4	24.000	24.700	170	φ 12@125	4	1.500	1.200	400	150	110	180	185	150	550	1	12,000	25	5.100	13	2	8 8	7	70	φ1	2 φ2	2 φ 12	n4 φ22n	2 52.7	9833	186.4	32.3	3136	97.0	
		0.350		φ 12@175			ļ	ļ	_	_	<u> </u>	_		<u> </u>					11		4	_	<u> </u>		-	 				<u> </u>		152	4.7	16.011
1-PST-L24_n2	24.000	24.700	200	φ 16@150	2	3.000	1.600	1000	150	150	200	250	250	500	1	12.000	4			\sqcup		╄	140	φ1:	2 φ2	2 φ 12	n5 φ 25n	5 48.7	7264	149.2	34.7	3760	108.5	
	<u> </u>	0.350	<u> </u>	φ 12@125			<u> </u>	ļ	-	igspace	_	<u> </u>	<u> </u>	 	╀-		Ļ.	 	-		+	 -	-	_	-	-		-			<u> </u>	1914	55.2	<u> </u>
1-PST-L26_n2	26.000	26.700	200	φ 16@150) 2	3.000	1.700	1000	150	150	200	250	250	500	2	8.667	5	 	ļ	- -	+	+	140	ϕ 1	2 φ2	2 φ 12	n 5 φ2 5π	5 57.8	10704	185.1	36.1	3425	95.0	
		0.350		φ 12@125	1	<u> </u>	 -	 	┼-		-	ļ	igapha	╀	╀		├	 	┡	\vdash	+	+-	-		+-	+-						1444	40.0	
1-PST-L28_n2	28.000				1	2 3.000	1.800	1000) 150	150	200	250	250	500	2	9.333	5	<u> </u>	 	\vdash	+	+	140	$\phi 1$	2] φ2	$2 \phi 12$	n6 φ 25n	5 62.0	11459	184.7	39.9	3827	95.8	
ļ	-	0.350		φ 12@125	7				+-	 - -	-	┼-	-	┼-	╁		╀	┼	<u> </u>		+	+	-	 	+-				10015	1015	15.6	1552	1	23.784
1-PST-L30_n2	2 30.000	— ——			\top	2 3.000	2.000	1000	0 150) 150	200	250) 250) 500	2	10.000	5	<u> </u>	┼	${f H}$	+	+	140) φ1	2 φ2	⁽² φ 12	n7, φ28n	5 66.7	12315	184.5	45.6	4300 1667	94.3 36.6	• • • • • • • • • • • • • • • • • • • •
		0.400		φ 12@12	+	<u> </u>	-	-	-	-					+			1	╁	\vdash	•	+	146	 	1.0	2 4 12	7 4 20	5 70.0	12004	1946	49.9	4588	92.0	
1-PST-L32_n2	2 32.000				1	2 3.000	2.100	100	U 150	0 150) 200) 250	J 259	J[50(1 2	10.660	6	 	╁╌	H	+	+	140	лφΙ	<u> 2</u> φ 2	Ζ[Φ 12	n7 φ28r	5 70.9	13094	184.6	49.9	2131	42.7	
		0.400	+	φ 12@12	╅	-			 		300	 			1	11.00	+	1-	+	H	+	+	14	1 4 1	1 4	2 4 10	n8 φ28r	5 75.2	13851	184.3	54.3	5038	92.7	
1-PST-L34_n2	2 34.000	***			1	2 3.000	3 2.20	0[100	U 151	U 150	J 20	J 25!	J 25	u SU	J 2	11,53.	, 0	-	╁	+	\dashv	+	141	μφι	2 Ψ 2	-2 φ 12	ποι φ 20Ι	13.2	13031	104.7	37.3	2262	41.6	
1 DOT 106 -	2 26 00	0.400	1	φ 12@12	╅	2 2 00	0 2 40	0.100	N 15	0 15	0.20	0.25	0 25	0 504	+	12.00	1,	1	+	H	+	\dagger	141	ገ ሐ 1	2 42)2 & 13	n9 φ32r	5 79.6	14684	184.4	60.6	5547	91.5	<u> </u>
1-PST-L36_n/	2 30.00				_	2 3.00	U 2.40	91100	U 13	U 13	0120	0/23	U 43	0 30	+	12.00	+	 	1-		+	+		1 4 1	1 4	7	71.7 ¥ 32.1	77.0	1.004	1	1 20.0	2792	1	
		0.400	'	φ 12@12)	Ц			Т.				Щ.	1		.l		1	ــــــــــــــــــــــــــــــــــــــ	ئــــــــــــــــــــــــــــــــــــــ			<u> </u>	ــــــــــــــــــــــــــــــــــــــ	<u> </u>			<u> </u>	<u> </u>		<u> </u>		1 .0.0	1 23,000

PRE: Pre-tensioned
PST: Post-tensioned

2-Lane																																			
INDEX	L _C (m)	L _V (m)	EL	R. Bar	Mai	n Gird	H(m)	bt	ŧŧ	th	bw	hh	tb	bb l	N ₁	st	Nd	ХB	NB	N1	N5	N6 N	17 C	cs	RB3	RB4	RB6	A.S.B.	Vc	Wc	Wc/Vc	VB	W_R W_P	$\frac{W_R/V_B}{W_P/V_B}$	Rd(t) RL(t)
	i	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20		21	2	22	23	24	25	26	27	28	29	30	31	32	33
2-PRE-L14_n6	14.000	14.600	170	φ 12@125	6	1.500	0.700	400	150	110	180	185	150	550	1	7.000	17	2.100	9	2	0	8	7	70	φ 18	φ25	φ 12n2	φ 22n2	44.9	6752	150.4	22.9	3215	140.7	13.278
		0.300		φ 12@175															7														1154	50.5	15.021
2-PRE-L16_n6	16.000	16.600	170	φ 12@125	6	1.500	0.800	400	150	110	180	185	150	550	1	8,000	19	3.100	11	2	2	8	7	70	φ 16	φ25	φ 12n2	φ 22n2	50.9	7530	148.0	27.8	3532	127.2	15.337
		0.300		φ 12@175															8														1467	52.8	15.370
2-PRE-L18_n6	18.000	18.600	170	φ 12@125	6	1.500	0.900	400	150	110	180	185	150	550	1	9.000	21	4.100	13	2	4	8	7	70	ϕ 16	φ25	φ 12n3	φ22n2	56.9	8397	147.5	33.1	4222	127.4	17.588
		0.300		φ 12@175				<u> </u>							_				9	<u></u>			_	_									1816	54.8	15.616
2-PRE-L20_n6	20,000	20.700	170	φ 12@125	6	1.500	1.000	400	150	110	180	185	150	550	1	10.000	23	5.100	13	2	6	8	7	70	φ16	φ22	φ 12n3	φ 22n2	63.2	9230	146.0	39.1	4614	118.0	20.047
		0.350		φ 12@175				<u> </u>			ļ				_				10			_	\bot					ļ					2214	56.6	15.792
2-PRE-L22_n6	22.000	22.700	170	ф 12@125	6	1.500	1.100	400	150	110	180	185	150	550	1 1	11.000	25	5.600	13	2	8	8	7	70	φ 16	φ22	φ 12n4	φ22n2	69.2	10097	145.9	45.3	5391	118.9	22.737
		0.350		φ 12@175				ļ							4				11				4										2639	58.2	15.919
2-PRE-L24_n6	24,000	24.700	170	φ 12@125	6	1.500	1.200	400	150	110	180	185	150	550	1 1	12.000	27	6.300	15	2	8	8	7	70	φ16	φ22	φ 12n	φ22n2	75.2	10912	145.1	52.0	5877	113.0	25.305
		0.350	-	φ 12@175				<u> </u>	-	 	ļ	<u> </u>			_	· · ·	_		12	<u> </u>		_	4				<u> </u>					:	3101	59.6	16.011
2-PST-L24_n4	24.000	24.700	170	φ 16@150	4	2.250	1.600	1000	150	150	200	250	250	500	1	12.000	4		 -			_	1	40	φ12	φ22	φ 12n:	φ25n3	75.6	14907	197.2	64.6	6298	97.5	41.964
	<u> </u>	0.350		φ 12@125	-		ļ	-	ļ	<u> </u>		-	-				<u> </u>		 			_	+					 					2136	33.0	24.017
2-PST-L26_n4	26.000	26.700	170	φ 16@150	4	2.250	1.700	1000	0 150	150	200	250	250	500	2	8.667	4		-	-			1	140	φ12	φ22	φ 12n:	φ25n3	83.6	16276	194.7	72.1	6898	95.7	46.654
		0.350	ļ	φ 12@125	-		 	 	 		-	-			_		╀		┼	_	Н	_	+			<u> </u>					<u> </u>		2310	32.0	24.116
2-PST-L28_n4	28.000		1	<u> </u>	1	2.250	1.800	1000	0 150	150	200	250	250	500	2	9.333	4	<u> </u>	╁┈	<u> </u>		\dashv	1	140	φ12	φ22	φ 12m	φ 25n3	90.0	17442	193.9	81.3	7741		51.311
	-	0.350		φ 12@125	<u> </u>	<u> </u>		 	 	-	-	-	_	-			┡		+	<u> </u>		\dashv					ļ	<u> </u>			ļ		2484		24.186
2-PST-L30_n4	30.000	1	1	1	1	2.250	2.000	3 1000	0 150	150	200	250	250	500	2	10.000	5		┼	-			- 1	140	φ12	ϕ 22	φ 12n	7 φ 28n3	96.6	18748	194.0	91.2	8659	95.0	56.289
	1	0.400	 	φ 12@125		<u> </u>	<u> </u>	<u> </u>	-	-	-	<u> </u>		-			\vdash		-	╀╌		┽	-										3335	36.6	
2-PST-L32_n4	32.000		 	 	+-	2.250	2.100	0 1000	0 150	150	200	250	250	500	2	10.666	1_5		+	-		4	- 1	140	φ 12	φ22	φ 12n	7 φ 28n4	102.7	19926	194.0	99.8	9231	92.5	60.733
	┼	0.400	+	φ 12@125	1	<u> </u>	 	-	+-	╁	 	╀	 				+-	<u> </u>	1	\vdash		+	+		,		<u> </u>	1					3552	35.6	
2-PST-L34_n4	1 34.000	 	1 -	1	\top	2.250	2.200	01100	0 150	150	200	250	250	500	2	11.333	1 5	1	+	+	\vdash	_		140	ф 12	φ22	φ 12n	β φ 28 π5	108.8	21080	193.8	108.7	10135	93.3	
	1	0.400	_	φ 12@125	╅				<u> </u>		-		1		$\left\lfloor \frac{1}{2} \right\rfloor$	10.000			-	┼┈		\dashv	+		,	 					1000		3770	34.7	1
2-PST-L36_n4	4 36.00			 	+-	2.250	2.300	U 100	0 150) 150) 200) 25(J 25{	500	2	12.000	1	<u> </u>	╂	\vdash	╂╌┨	\dashv	- ³	140	φ12	$\phi 22$	φ 12n	β φ 32n3	114.9	22305	194.2	117.9	10736	91.1	69.923
PRF · Pre-ten	1	0.400	<u> </u>	φ 12@12:	5		1	1		<u> </u>	<u>.</u>	1	1	<u></u>				<u> </u>	<u> </u>	<u> </u>						<u> </u>	<u> </u>	1	<u> </u>		<u> </u>	<u> </u>	4785	40.6	24.292

PRE: Pre-tensioned
PST: Post-tensioned

IV. Drawings

- 1. 1-PRE-L14-n4 1 (Deck slab and general cross-section)
- 2. 1-PRE-L14-n4_1 (Main beam and PC-Cable arrangement)
- 3. 1-PRE-L16-n4_1 (Deck slab and general cross-section)
- 4. 1-PRE-L16-n4_1 (Main beam and PC-Cable arrangement)
- 5. 1-PRE-L18-n4_1 (Deck slab and general cross-section)
- 6. 1-PRE-L18-n4_1 (Main beam and PC-Cable arrangement)
- 7. 1-PRE-L20-n4_1 (Deck slab and general cross-section)
- 8. 1-PRE-L20-n4_1 (Main beam and PC-Cable arrangement)
- 9. 1-PRE-L22-n4_1 (Deck slab and general cross-section)
- 10. 1-PRE-L22-n4_1 (Main beam and PC-Cable arrangement)
- 11. 1-PRE-L24-n4_1 (Deck slab and general cross-section)
- 12. 1-PRE-L24-n4_1 (Main beam and PC-Cable arrangement)
- 13. 1-PST-L24-n2_1 (Deck slab and general cross-section)
- 14. 1-PST-L24-n2_1 (Main beam and PC-Cable arrangement)
- 15. 1-PST-L26-n2_1 (Deck slab and general cross-section)
- 16. 1-PST-L26-n2_1 (Main beam and PC-Cable arrangement)
- 17. 1-PST-L28-n2_1 (Deck slab and general cross-section)
- 18. 1-PST-L28-n2_1 (Main beam and PC-Cable arrangement)
- 19. 1-PST-L30-n2_1 (Deck slab and general cross-section)
- 20. 1-PST-L30-n2_1 (Main beam and PC-Cable arrangement)
- 21. 1-PST-L32-n2_1 (Deck slab and general cross-section)
- 22. 1-PST-L32-n2 1 (Main beam and PC-Cable arrangement)
- 23. 1-PST-I.34-n2 1 (Deck slab and general cross-section)
- 24. 1-PST-L34-n2 1 (Main beam and PC-Cable arrangement)
- 25. 1-PST-L36-n2_1 (Deck slab and general cross-section)
- 26. 1-PST-L36-n2_1 (Main beam and PC-Cable arrangement)

- 27. 2-PRE-L14-n6_1 (Deck slab and general cross-section)
- 28. 2-PRE-L14-n6_1 (Main beam and PC-Cable arrangement)
- 29. 2-PRE-L16-n6_1 (Deck slab and general cross-section)
- 30. 2-PRE-L16-n6 1 (Main beam and PC-Cable arrangement)
- 31. 2-PRE-L18-n6_1 (Deck slab and general cross-section)
- 32. 2-PRE-L18-n6_1 (Main beam and PC-Cable arrangement)
- 33. 2-PRE-L20-n6_1 (Deck slab and general cross-section)
- 34. 2-PRE-L20-n6_1 (Main beam and PC-Cable arrangement)
- 35. 2-PRE-L22-n6_1 (Deck slab and general cross-section)
- 36. 2-PRE-L22-n6_1 (Main beam and PC-Cable arrangement)
 37. 2-PRE-L24-n6_1 (Deck slab and general cross-section)
- 38. 2-PRE-L24-n6_1 (Main beam and PC-Cable arrangement)
- 39. 2-PST-L24-n4 1 (Deck slab and general cross-section)
- 40. 2-PST-L24-n4 1 (Main beam and PC-Cable arrangement)
- 41. 2-PST-L26-n4_1 (Deck slab and general cross-section)
- 42. 2-PST-L26-n4_1 (Main beam and PC-Cable arrangement)
- 43. 1-PST-L28-n4_1 (Deck slab and general cross-section)
- 44. 1-PST-L28-n4 1 (Main beam and PC-Cable arrangement)
- 45. 1-PST-L30-n4_1 (Deck slab and general cross-section)
- 46. 1-PST-L30-n4 1 (Main beam and PC-Cable arrangement)
- 47. 1-PST-L32-n4_1 (Deck slab and general cross-section)
- 48. 1-PST-L32-n4 1 (Main beam and PC-Cable arrangement)
- 49. 1-PST-L34-n4_1 (Deck slab and general cross-section)
- 50. 1-PST-L34-n4_1 (Main beam and PC-Cable arrangement)
- 51. 1-PST-L36-n4_1 (Deck slab and general cross-section)
- 52. 1-PST-I.36-n4 1 (Main beam and PC-Cable arrangement)

