JAPAN INTERNATIONAL COOPERATION AGENCY (JICA)

MINISTRY OF PUBLIC WORKS
THE REPUBLIC OF CHILE

THE REHABILITATION AND CONSERVATION PROGRAM ON THE BRIDGES IN THE REPUBLIC OF CHILE (PHASE 2)

FINAL REPORT

STANDARD BRIDGE DESIGN DRAWINGS (VOLUME 8/8)



JULY 1998

PACIFIC CONSULTANTS INTERNATIONAL

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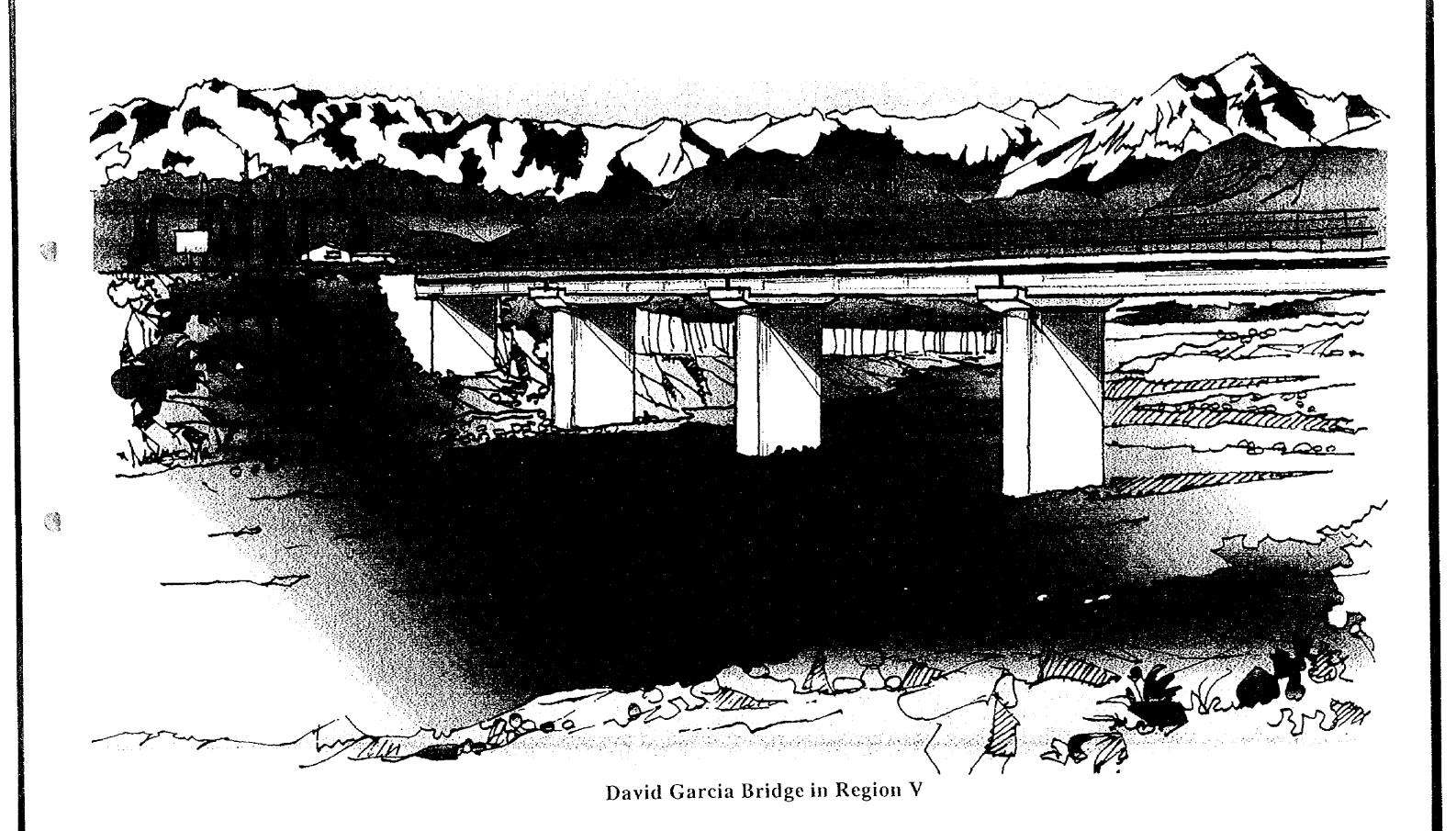
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DETAIL

STEEL SUPERSTRUCTURE

I, General

1. Outline

The "Drawings" may be used in the case that the budgeting for an implementation plan be required by the Ministry of Public Works in Chile, or as a kind of the data in preliminary design for engineers. So it must be recognized that they are not considered as a detailed design.

2. Specifications

The design is based on the following specifications.

- 1) "Standard Specifications for Highway Bridges" adopted 1992 and published by the American Association of State Highway and Transportation Officials 444 North Capitol Street, N. W., Suite 249 Washington, D.C. 20001.
- 2) "Specifications for Highway Bridges" adopted 1994 and published by Japan Road Association.

3. Contents

This set of the part for the steel superstructure constitutes of the following chapters.

- I. General
- II. Design Conditions
- III. Table of Design Variables
- IV. Drawings
- V. Calculation report (Input and Generalization table)
- VI. Material List
- VII. Span-steel weight Diagram

4. Composition of the Drawings

Each set of drawings for a steel superstructure consists of;

For 1 Lane

- 1. Concrete deck slab and general cross-section
- 2. Main girder

3. Lateral bracing and cross frames 7 000 5 000 1 000

For 2 Lanes

5. Instruction

- 1) The standard bridges dealt here are straight and right-angled only, hence some modifications and consideration should be added to the standard design, when applied to skewed or curved bridges.
- 2) All dimensions on the drawings are in "mm" unless otherwise stated.
- 3) The standard bridges are intended to be used for rural bridges.
- 4) The number of lanes are available for one or two, and the width for these lane numbers are shown at below-left
- 5) Cross-fall on the road-way is 1.5 % and on the side-walk is 1.0 %.
- 6) Curb height and width are 250 mm and 200 mm respectively.
- 7) Railings are 1100 mm high.
- 8) The minimum thickness of pavement is 50 mm at both sides of the road-way, and it is thickest at the center according to the cross-fall.
- 9) All the drawings of the standard bridges are made by use of CADD System program separately worked out for the project.
- 10) Combinations of span lengths and number of lanes are shown below.

	Steel													
	1	Lane	2	Lane										
Span Length (m)	Roll-H	Built-I	Roll-H	Built-I										
14	•		•											
16	•		•	na atractica.										
18	•		•											
20	•		•											
22	•		•	T -										
24	•	—	•											
26		•		•										
28		•		•										
30		•		•										
32		•		•										
34	-	•		•										
36	_	•		•										

11) Structures of which applicable span length are not in the drawings can be designed using the CADD system program.

II. Design Condition

- 1. Design Method: Allowable Stress
- 2. Loading
- 1) Dead Loads

Plane Concrete

 $W_c = 2.30 \text{ t/m}^3$

Reinforced Concrete : $\gamma_C = 2.50 \text{ t/m}^3$

Steel

 $\therefore \gamma = 7.85 \text{ t/m}^3$

Pavement

 $\gamma = 2.30 \text{ t/m}^3$

Soil

 $\gamma_{\rm S} = 1.80 \, {\rm t/m^3}$

2) Horizontal Force of Railing 3) Sidewalk Live Load

 $Lc \le 7.6 \text{ m}$

 \rightarrow W_P = 0.415 t/m²

Lc; Span Length

: $W_B = 0.050 \text{ t/m}$, $W_L = 0.020 \text{ t/m}$, h = 1.100 m

 $7.6 \text{ m} < \text{Lc} \le 30.5 \text{m}$

 \rightarrow W_P= 0.293 t/m²

 $30.5 \, \text{m} < \text{Lc}$

 $Wp = \left(147 + \frac{4464}{1c}\right) \times \left(\frac{16.76 - (Sw - 0.25)}{15.24}\right) \times \frac{1}{1000}$

% In case of $W_p > 0.293 \rightarrow W_p = 0.293 \text{ t/m}^2$

Sw; Sidewalk width

- 4) Live Load: HS20-44(100%)
- $W_{\rm V} = 0.244 \, {\rm t/m^2}$ 5) Wind 6) Earthquake: A = 0.15, Category B
- 3. Materials

Slab Concrete : H-30, $f_c' = 250 \text{ kg/cm}^2$, $E_c = 2.50 \times 10^5 \text{ kg/cm}^2$

Reinforcing Bar: A63-42H, $f_v = 4200 \text{ kg/cm}^2$, $f_w = 1690 \text{ kg/cm}^2$, $E_s = 2.10 \times 10^6 \text{ kg/cm}^2$

Concrete Cover: 3.0 cm (Lateral Beam 2.5cm)

Anchor Bar

: A44-28H, $f_v = 2800 \text{ kg/cm}^2$, $f_s = 1400 \text{ kg/cm}^2$

Steel

: A52-34ES, $f_v = 3400 \text{ kg/cm}^2$, $f_s = 1870 \text{ kg/cm}^2$

Bolt

: ASTM A490, $f_{sa} = 1400 \text{ kg/cm}^2$, $\phi = 22 \text{mm}$

4. Design Concept

1) Beam Type

Two types of steel beams are employed, rolled H and built-up plate girder. Rolled H beam—is used for shorter span, which is of a simple structure and easily constructed. Built-up plate girder is for rather longer than the other. Both types of beams are used as steel girders for composite girders with concrete slab.

2) Edge distances from end of beam to center of bearing(ED) vary according to the span length as follows.

Span :Lc(m)	Lc<20	20 <lc<30< th=""><th>30≦Lc</th></lc<30<>	30≦Lc
ED (mm)	250	300	350

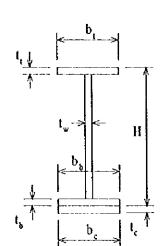
- 3) Distance from center to center of beams is limited to less than 3m, considering the girder deflection by the live and impact load not to be excessive, and then selected appropriate girder depth in order to maintain an economic design.
- 4) End cross frame is designed by reinforced concrete according to conventional Chilean designing method, and through the structure anti-seismic bars are connected with sub-structure.
- 5) Steel elements are connected at site by use of high strength friction grip bolts, notwithstanding the fact that welding has been used for a long time in Chile for site connection, because if this technique is properly performed, the it is easier but firmer.

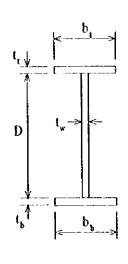
III. Table of Design Variables

- 1. Lv; Girder Length (m)
- 2. Lc; Span Length (m)
- 3. II/D; Girder Height (m), Girder Depth (m)
- 4. t.; Web Thickness (mm)
- 5. Ls; Length of Section (m)
- 6. b_i; Width of Top Flange (mm)
- 7. t,; Thickness of Top Flange (mm)
- 8. b_b; Width of Bottom Flange (mm)
- 9. t_h; Thickness of Bottom Flange (mm)
- 10. b.; Width of Cover Plate for Bottom Flange (mm)
- 11. t.; Thickness of Cover Plate for Bottom Flange (mm)
- 12. R.Bar; Diameter and Pitch of Main Reinforcement Bar for Concrete Deck Slab (mm) [Upper row]
 Diameter and Pitch of Distribution Reinforcement Bar for Deck Slab (mm) [Lower row]
- 13. Main Gird.; Number of Main Girders
- 14. Main Gird.; Spacing of Main Girders (m)
- 15. H. Stiff.; Location of Horizontal Stiffeners from Top Flange (mm)

 Section for Horizontal Stiffener, With × Thickness (mm)
- 16. Splice ; Location of Field Splice from Bearing (m) $n_w \times m_w$; Number of Rows of Bolts for Web Plate (Longitudinal × Transverse)
- 17. $n_U \times m_U$; Number of Rows of Bolts (Longitudinal \times Transverse) for Top Flange [Upper row] $n_L \times m_L$; Number of Rows of Bolts (Longitudinal \times Transverse) for Top Flange [Lower row]
- 18. x(stud); Range of Stud Densely Attached (mm)
- 19. Pe; Pitch of Stud Densely Attached (mm)
- 20. Pc; Pitch of Stud Coarsely Attached (mm)
- 21. S. Bracing; Steel Shape Depth × Width × Thickness (mm)
 (Channel is used for rolled H, angle for plate girder)
- 22. A.S.B.; Diameter and Number of Anti-seismic Bars (mm)
- 23. Area; Deck Surface Area of Bridge (m²)
- 24. Vc ; Concrete Volume (m³)
- 25. S. Weight; Steel Weight Employed for Steel Superstructure (kg)
- 26. W_R; Weight of Reinforcement Bar (kg)
- 27. W/A; Unit Weight of Steel used for Steel Structure (kg/m²) [Upper row]

 W_R/Vc; Weight of Reinforcement Bar for Unit concrete volume (kg/m³) [Lower row]
- 28. Rd(t); Dead Load Reaction Force per Each Girder at One Bearing, (ton) [Upper row] RL(t); Live Load Reaction Force per Each Girder at One Bearing, (ton) [Lower row]





(1-Lane)

						·						· · · · · · · · · · · · · · · · · · ·			1	~	$n_U \times m_U$	L	Ţ	T							W/A	Rd(t)
INDEX	L _V (m)	L _C (m)	H/D	t _w	Ls	b _t	t,	b _b	t _b		t _c				1	n _W ×m _W	$n_L \times m_L$					A.S.B.	Area		S. Weight	W _R	W _R /Vc	RL(t)
	j.	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28
1-SRH-L14_n3	14.500	14.000	0.800	14	14.800	300	16	300	16			φ 16@150	3	2,400				4.000	150	300	C300×100×10	φ22n2	87.00		7904.71		90.86	13.521
												φ 12@125												30.91		6181.61	199.99	18,956
1-SRH-L16_n3	16.500	16.000	0.800	14	10.600	300	16	300	16	250	10	φ 16@150	3	2,400				4.700	150	300	C300×100×10	φ22n2	99.00		9487.37		95.83	15,421
					3.100	300	16	300	16			φ12@125									<u></u>			34.72		6938.31	199.84	19.396
1-SRH-L18 n3	18.500	18.000	0.800	14	12.000	300	16	300	16	250	13	φ 16@150	3	2.400				5.100	150	300	C300×100×10	φ22n2	111.00		10740.35		96.76	17.233
			-		3.400	300	16	300	16			φ 12@125												38.58		7695.02	199.46	19.707
1-SRH-L20 n3	20.600	20.000	0.900	16	13.200	350	18	350	18	300	10	φ 16@150	3	2.400				7.800	200	400	C300×100×10	φ22n2	123.60		15116.53		122.30	19.670
					3.800	i			Γ			φ 12@125	П											42.94		8524.36	198.52	19.929
1-SRH-L22 n3	22,600	22,000	0.900	16	14.600	350	18	350	18	300	14	φ 16@150	3	2.400				8.600	200	400	C300×100×10	φ22n2	135.60		16872.61		124.43	21.585
					4.100							φ 12@125	T											46.81	•	9324.24	199.19	20.089
1-SRH-1.24 n3	24.600	24.000	1.000	18	15.800	350	20	350	20	300	12	φ 16@150	3	2.400				8.600	200	400	C300×100×10	φ22n2	147.60		20862.36		141.34	23.930
					4.500				1			φ 12@125								1				50.89		10094.09	198.35	20.206
1-SBI-L26 n2	26 600	26,000		10	17.200	360	12	440	36			φ 16@125	1 2	3.000		5.850	2×2	8.000	200	400	L-80×80×8	φ 25n4	159.60		14865.92		93.14	37.404
1 0001 1000			1.300	†	4.800	<u> </u>			1	1		φ 16@175	T^-				6×2							55.87		12841.33	229.84	23,715
1-SBI-L28 n2	28 600	28,000	1	10	18.400	360	14	460	30			φ 16@12 ⁵	1	2 3.000	-	6.300	2×2	5.800	200	3 400	L-80×80×8	φ 25n4	171.60		15955.00		92.98	40.183
10111120112	20.000	20.00	1.400	†	5.200				1	1	<u>† </u>	φ 16@17:	1			2×14	6×2							59.98		13772.99	229.63	23.784
1-SBI-L30 n2	30.700	30,000		10	19.800	360	15	500	29			φ 16@12:	┰	2 3.000	300	6.750	3×2	6.400	200	3 400	L-80×80×8	φ 25n4	184.20		18799.32		102.06	43.379
T GIN TOO III		20.000	1.500	 	5.500					T		φ 16@17:			120×16		7×2	1						64.31		14751.41	229.38	23.831
1-SBI-L32_n2	32 700	32 000	 	+-	17.400	† 		1	†	1		φ 16@12:	+-	2 3.000	320	8.575	3×2	7.000	0 20	0 400	L-80×80×8	φ 25n4	196,20		20816.80		106.10	46.570
1.001.102 110	52.700	32.000	1.600	1	3.800	1	1			T	1	7 23 3 2 2				2×17			1									
-	-		1.000		3.900			1		1	1	φ 16@17.	-				7×2		1					68.45		15676.01	229.01	23.862
1-SBI-L34 n2	24.700	34 000	1	30	18.400	1					 	φ 16@12	+-	2 3 000	340	9.163		7.000	0 20	0 400	180×80×8	φ 28n4	208.20		23203.12		111.45	49.742
1-301-1234_112	, 54.700	37.000	1.700		4.100	1	†	1		1	T	¥ 10@12			120×16		1	1	 	-1							<u> </u>	
		1	1.700	+	4.100		1	1		1	†-	φ 16@17	<u>, </u>		120/10	2/10	7×2	1	\dagger			†		72.56		16633.84	229.24	23.880
1 CDI 124 -0	26 700	36 00		1,	19.400	_	1	1	1	 	+	φ 16@12	†	2 3 000	360	9.750	+	4 20	0 20	0 40	L-80×80×8	<i>ბ</i> 28n₄	220.20		25507.09		115.84	
1-SBI-L36_n2	21 30.700	30.00		1		1		—	-t			Ψ 10@12	1	2 3.000	120×16	1	7^2	7.20	120	3 40	1 2000000	T Lon	220,20					
	 	 	1.800	+	4.300		1				+-	416@17	-	 	120011	2810	9×2		+	-	<u> </u>	†	 	76.71		17559.16	228.90	23.888
	}	<u> </u>			4.400	360	I	300	/[11		<u> </u>	φ 16@17	٥	1	1	<u> </u>	37.2	<u> </u>	Щ.	-	1		<u> </u>	1 ,0.71	J	1,000.10	1 220.70	20,000

(2-Lane)

	·1			₁	T		-	 7		1						Splice	n _U × nt _U										W/A	Rd(t)
INDEX	L _V (m)	L _C (m)	H/D	t,	Ls	b _t	tı	b_b	t _b	b _c	tc	R. Bar	Mai	n Gird.	H. Stiff.	n _w ×m _w	$n_L \times m_L$	x(stud)			S. Bracing	A.S.B.	Area		S. Weight	$-\frac{W_R}{}$	W _R /Ve	RL(t)
	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28
2-SRH-L14-n4	14.500	14.000	0.800	14	14.800	300	16	300	16			φ 16@150	4	2.400			m	5.500	140	300	C300×100×10	φ22n2	131.60		10664.04		81.03	15.243
												ø 12@125						<u> </u>						47.36		9001.63	190.07	24.033
2-SRH-L16-n4	16.500	16.000	0.800	14	10.600	300	16	300	16	250	10	φ 16@150	4	2.400				6.500	140	300	C300×100×10	φ22n2	150.40		12774.26		84.94	17.364
					3.100	300	16	300	16			ø 12@125				·								53.21		10098.13	189.78	24.592
2-SRH-L18-n4	18.500	18.000	0.800	14	12.000	300	16	300	16	250	18	φ 16@150	4	2.400				7,350	130	300	C300×100×10	φ22n2	169.20		14915.87		88.16	19.489
					3.400							φ 12@125										-		59.12		11194.62	189.35	24.985
2-SRH-L20-n4	20.600	20.000	0.900	16						300	10	φ16@150	4	2.400				7.450	150	300	C300×100×10	φ 22n2	188.00		20343.79		108.21	22.082
2 (/111) 200 // /	20.000	20.000	3,75		3.800							¢ 12@125												65.78		12398.03	188.48	25.267
2-SRH-L22-n4	22 600	22 000	0.900	16						300	19	φ 16@150		2.400				8.450	150	300	C300×100×10	φ22n2	206.80		23372.89		113.02	24.353
2-31(1-1.22-114	22.000	22.000	0.200	10	4.100			:		-		φ 12@125	†											71.70		13556.74	189.08	25.471
2-SRH-L24-n4	24 600	24 000	1.000	18						300	17		1	2.400				8.250	150	300	C300×100×10	φ22n2	225.60		28750.83		127.44	26.945
2-3811-1.24-114	24.000	24,000	1.000	10	4.500					300		φ 12@125	1	2.700			<u> </u>	10.000				,		77.95	 	14672.94		
2 CDL L262	26,600	26,000		10	17.200					 	 	φ 16@125	+	3.200		5,850	3×2	8 500	160	300	L-80×80×8	ტ25n4	244 40		25109.78		102.74	
2-SBI-L26-n3	20.000	20.000	1.300	10	4.800				†			φ 16@175	1-	3.200	ļ	2×12	7×2	0.500	1200	1300	,, concorne	7 23.0	2	87.10	† · · · · · · · · · · · · · · · · · · ·	19245.02		
0.004.400.3	20,600	20.000	 	10								φ 16@125	 	3.200		6.300		800	170	300	L-80×80×8	d 25n4	263.20	 	27884.87		105.95	42.003
2-SBI-L28-n3	28.600	28.000	1	 	18.400			†	36	-			1	3.200	<u> </u>	2×13	7×2	0.000	170	300	120000000	Ψ 25114	203.20	93.55	1	20639.53	 	
			1.400	 	5.200			\vdash	1	┼	 	\$ 16@175	+	2 000	 		 	7.650	100	200	L-80×80×8	4.25-4	282.00		31044.12	20037.33	110.09	45.316
2-SBI-L30-n3	30.700	30.000	1	†	19.800	†		-	 		-	φ 16@125	1	3.200		6.750 2×13	4×2 7×2	7.050	100	300	L-OUAOUAO	Ψ23114	202.00	100.32		22098.84	 	
			1.500	+	5.500	1	10		1	 	 	¢ 16@175	1	2 200	220			7.450	100	200	I 20,420,42	4.25-4	200.00	 	34936.01		 	48.490
2-SBI-1.32-n3	32,700	32,000	1	1	17.400	T					 	φ 16@125		313.200	320	8.575	4×2	7.430	180	300	L-80×80×8	φ 23114	300.60		34930.01		110.14	46.430
	 	<u> </u>	1.600	 	3.800			1			├		+	 	120×16	2×17	0.0	-	-	-		ļ		106.01	 	23478.19	219.81	34.511
	<u> </u>	1		╁	3.900			 				φ 16@17:	+-	-	 		8×2						0.10.50	106.81	20272 60		 	
2-SBI-L34-n3	34.700	34.000)	10	18.400		i		T	T	 	φ 16@125		3 3.200	340		4×2	7.250) 190 	300	L-80×80×8	φ 25n4	319.60		38272.60		119.75	51.710
	<u> </u>	 	1.700	 	4.100				T		-	<u> </u>	+	-	120×16	2×17	<u> </u>		-		<u> </u>	 					- بد ماند	
	<u> </u>	<u> </u>	1	_	4.100	360	. 10	360	13	 	┼	\$ 16@17:	5	 	 	<u> </u>	8×2	-	 	┼		 		113.27	+	24872.60	 	34.538
2-SBI-1.36-n3	36.700	36.000	<u> </u>	10	19.400	400	21	600	33	<u> </u>	igg	φ 16@128	5 :	3 3.200	360	9.750	4×2	6.000	200	300	L-80×80×8	φ25n4	338.40		41836.87		123.63	55.005
	<u> </u>	ļ. <u>-</u>	1.800	1	4,300	360	11	520	26		<u> </u>	1	_	<u> </u>	120×16	2×18	<u> </u>		4_	 	ļ	<u> </u>		 	<u> </u>	<u></u>	 	
	1				4.400	360	10	360	14			φ 16@17.	5		1		6×3			<u> </u>	<u> </u>		<u> </u>	119.70		26251.84	219.31	34.549

IV. Drawings

- 1. 1-SRH-L14-n3 1 (slab and cross-section)
- 2. 1-SRH-L14-n3_2 (Main girder)
- 3. 1-SRH-L14-n3_3 (Lateral bracing and cross frames)
- 4. 1-SRH-L16-n3_1 (slab and cross-section)
- 5. 1-SRH-L16-n3_2 (Main girder)
- 6. 1-SRH-L16-n3_3 (Lateral bracing and cross frames)
- 7. 1-SRH-L18-n3_1 (slab and cross-section)
- 8. 1-SRH-L18-n3_2 (Main girder)
- 9. 1-SRH-L18-n3 3 (Lateral bracing and cross frames)
- 10. 1-SRH-L20-n3_1 (slab and cross-section)
- 11. 1-SRH-L20-n3 2 (Main girder)
- 12. 1-SRH-L20-n3 3 (Lateral bracing and cross frames)
- 13. 1-SRH-L22-n3 1 (slab and cross-section)
- 14. 1-SRH-L22-n3_2 (Main girder)
- 15. 1-SRH-L22-n3_3 (Lateral bracing and cross frames)
- 16. 1-SRH-L24-n3_1 (slab and cross-section)
- 17. 1-SRH-L24-n3 2 (Main girder)
- 18. 1-SRH-L24-n3 3 (Lateral bracing and cross frames)
- 19. 1-SBI-L26-n2 1 (slab and cross-section)
- 20. 1-SBI-L26-n2_2 (Main girder)
- 21. 1-SBI-L26-n2_3 (Lateral bracing and cross frames)
- 22. 1-SBI-L28-n2 1 (slab and cross-section)
- 23. 1-SBI-L28-n2 2 (Main girder)
- 24. 1-SBI-L28-n2 3 (Lateral bracing and cross frames)
- 25. 1-SBI-L30-n2_1 (slab and cross-section)
- 26. 1-SBI-L30-n2_2 (Main girder)
- 27. 1-SBI-L30-n2_3 (Lateral bracing and cross frames)
- 28. 1-SBI-L32-n2_1 (slab and cross-section)
- 29. 1-SBI-L32-n2 2 (Main girder)
- 30. 1-SBI-L32-n2_3 (Lateral bracing and cross frames)
- 31. 1-SBI-L34-n2_1 (slab and cross-section)
- 32. 1-SBI-L34-n2_2 (Main girder)
- 33. 1-SBI-L34-n2_3 (Lateral bracing and cross frames)
- 34. 1-SBI-L36-n2_1 (slab and cross-section)
- 35. 1-SBI-L36-n2_2 (Main girder)
- 36. 1-SBI-L36-n2_3 (Lateral bracing and cross frames)

- 37. 2-SRH-I.14-n4_1 (slab and cross-section)
- 38. 2-SRH-L14-n4_2 (Main girder)
- 39. 2-SRH-L14-n4 3 (Lateral bracing and cross frames)
- 40. 2-SRH-L16-n4_1 (slab and cross-section)
- 41, 2-SRH-L16-n4_2 (Main girder)
- 42, 2-SRH-L16-n4 3 (Lateral bracing and cross frames)
- 43. 2-SRII-L18-n4_1 (slab and cross-section)
- 44. 2-SRH-L18-n4_2 (Main girder)
- 45. 2-SRH-L18-n4_3 (Lateral bracing and cross frames)
- 46. 2-SRH-L20-n4_1 (slab and cross-section)
- 47. 2-SRH-L20-n4_2 (Main girder)
- 48. 2-SRH-L20-n4_3 (Lateral bracing and cross frames)
- 49. 2-SRH-L22-n4_1 (slab and cross-section)
- 50. 2-SRH-L22-n4_2 (Main girder)
- 51. 2-SRH-L22-n4 3 (Lateral bracing and cross frames)
- 52. 2-SRH-L24-n4_1 (slab and cross-section)
- 53. 2-SRH-L24-n4_2 (Main girder)
- 54. 2-SRH-L24-n4_3 (Lateral bracing and cross frames)
- 55. 2-SBI-L26-n3_1 (slab and cross-section)
- 56. 2-SBI-L26-n3 2 (Main girder)
- 57. 2-SBI-L26-n3 3 (Lateral bracing and cross frames)
- 58. 2-SBI-L28-n3 1 (slab and cross-section)
- 59. 2-SBI-L28-n3_2 (Main girder)
- 60. 2-SBI-L28-n3 3 (Lateral bracing and cross frames)
- 61. 2-SBI-L30-n3_1 (slab and cross-section)
- 62. 2-SBI-L30-n3_2 (Main girder)
- 63. 2-SBI-L30-n3_3 (Lateral bracing and cross frames)
- 64. 2-SBI-L32-n3_1 (slab and cross-section)
- 65. 2-SBI-L32-n3_2 (Main girder)
- 66. 2-SBI-L32-n3_3 (Lateral bracing and cross frames)
- 67. 2-SBI-L34-n3_1 (slab and cross-section)
- 68. 2-SBI-L34-n3_2 (Main girder)
- 69. 2-SBI-L34-n3_3 (Lateral bracing and cross frames)
- 70. 2-SBI-L36-n3_1 (slab and cross-section)
- 71. 2-SBI-L36-n3_2 (Main girder)
- 72. 2-SBI-L36-n3_3 (Lateral bracing and cross frames)

