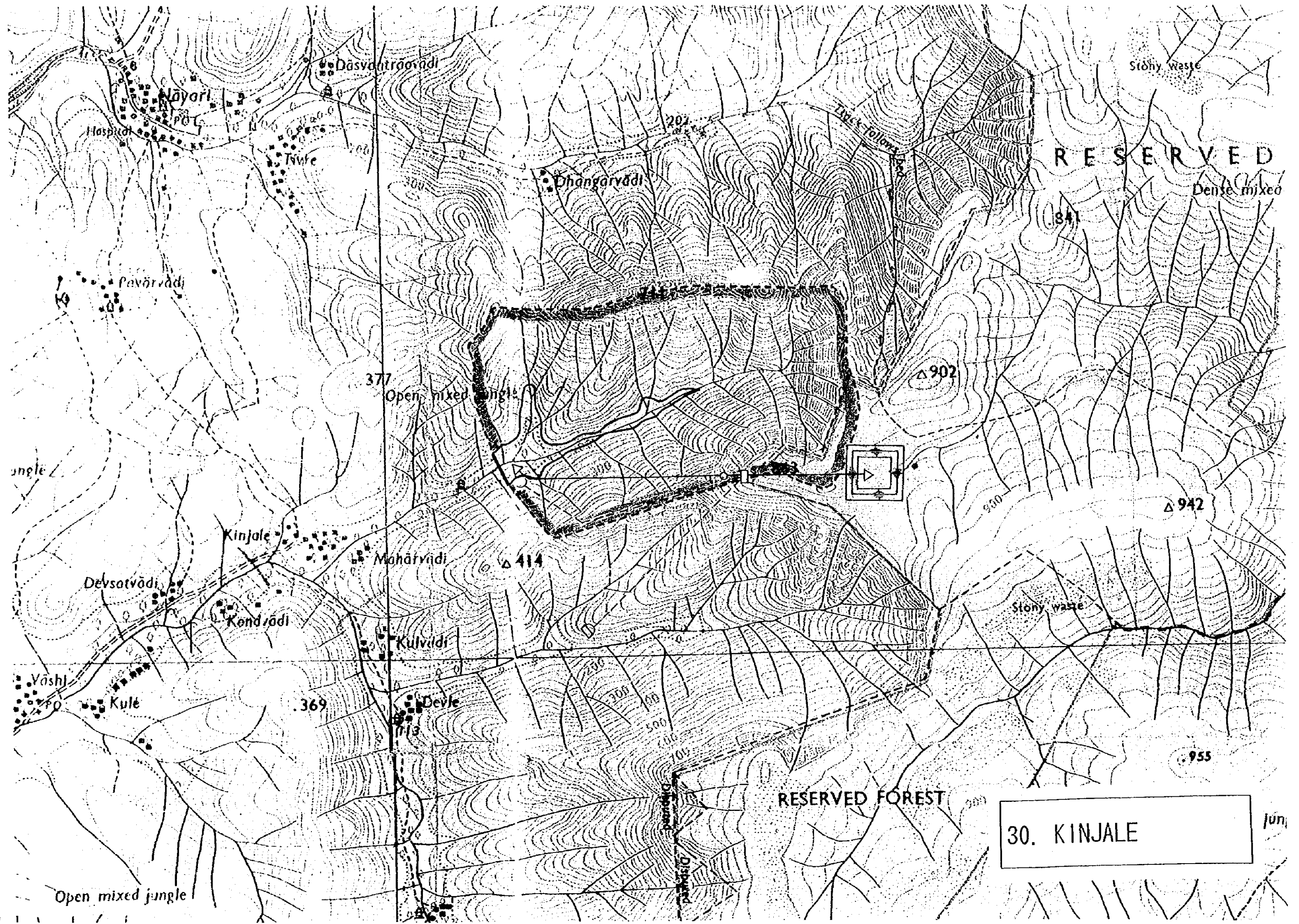


30. General Description of the KINJALE Project Site

1. Environment conditions	(1) Upper reservoir	The submerged area involves 12 ha of reserved forest	
	(2) Lower reservoir	The submerged area involves 18 ha of non-reserved forest and 10 ha of agricultural land	
2. River basin conditions	(1) Upper reservoir	(1) River basin	The artificially excavated and embanked pondage is to be layouted on a plateau of 800m elevation in the most upstream part of the river Gadgadi Nadi, a tributary emptying into the river Sonvi Nadi on the right bank.
		(2) Submerged dwellings	As it is an artificial pondage, there is no submerged dwelling.
		(3) River bed condition	Non
		(4) Circum-reservoir terrain	The plateau forms a tableland of 800m elevation.
	(2) Lower reservoir	(1) River basin	The basin is located in the most upstream part of the river Gadgadi Nadi. The watershed peak is a steep mountainous area with a ridgeline of steep cliffs. Only the river has a gentle gradient near the reservoir.
		(2) Submerged dwellings	Because of the steep topography, there is no dwelling.
		(3) River bed condition	The main river branches off into numerous mountain streams. Only the main stream, however, has signs of gouged-out/washed-out rock. Near the reservoir, the terrain has a relatively gentle gradient. It is not likely that the river bed is covered with thick deposits.
		(4) Circum-reservoir terrain	The terrain around the reservoir is flanked by steep slopes. It is assumed the fresh rock is exposed.
	(3) Changes in river basin	As the artificial pondage is adopted, this river basin is considered not to need any changes to the natural relief. The reconnaissance study, however, was proved that the upper pondage is belonged to the basin of the opposite direction.	
	3. Location and condition of structures	(1) Upper reservoir dam	(1) Dam site status
(2) Dam			The effective water storage capacity of this artificial pondage is $3 \times 10^6 \text{ m}^3$ . The storage capacity of the lower reservoir is the limit. It would be economically out of the question to raise the dam height on the upper pondage.
(2) Lower reservoir dam		(1) Dam site status	The upper reaches have a rapid river flow. Near the reservoir, the river dips at a relatively gentle gradient. The river basin has a small catchment area, so that the amount of rock washed down is actually small. On both banks of the dam site, the topography shows steep gradients and narrow ridges.
		(2) Dam	The location is somewhat favorable for the siting of the dam, but even when the dam height is raised, the water storage efficiency remains poor. Nor would it be economically advantageous to try to increase output by raising the dam height any further.
(3) Waterway route		(1) Geographical profile	The waterway system layout has been determined by taking into consideration the natural relief. This steep mountainous relief changes to a flat plain near the plateau on the upper pondage, and past this point there are few undulations.
		(2) Layout	The waterway system has a total extended length of 3.5km. In view of the topographical constraints, the powerhouse has to be located on the intake side. No headrace tunnel needs constructing.
(4) Intake and outlet		Since the upper pondage is artificial type, the intake needs a morning glory type structure. The outlet is to be constructed using an ordinary horizontal type of structure.	
(5) Surge tank		Since there is no headrace, there is also no need to provide a surge tank on the intake side. The tailrace has a significant length of 2.6km. This requires the construction of a surge tank.	
(6) Powerhouse		The powerhouse is located on the intake side. The structures belonging to the powerhouse therefore take an uneconomic layout.	
4. Access road and tunnel		(1) Upper and lower reservoirs	For access to the upper pondage an access road of approximately 10km length needs to be led from the existing road at Peth. For access to the lower reservoir, a new stretch of road of approximately 2km length needs to be constructed, forking off from the existing road at Kinjale.
	(2) Access tunnel to powerhouse	A 1.8km long access tunnel is to be provided from the left bank of the lower reservoir to connect to the powerhouse.	
	(3) Cable tunnel	The switchyard is to be layouted near the entrance of near the access road to the powerhouse. From this point, an approximately 1.8km long cable tunnel is required on the plan to connect to the underground transformer room.	
5. Power transmission lines	To construct the 400kV one circuit, double conductors, 45km from Kinjale PPS to planning of New Koyna S/S.		
6. General evaluation	This project uses an artificial pondage type structure for the upper pondage. It has a high water head, however, the lower reservoir has a limited water storage capacity. Consequently, the power output is not large. Thus, the project has a somewhat inferior economic efficiency, though there are also few areas of submerged forest.		





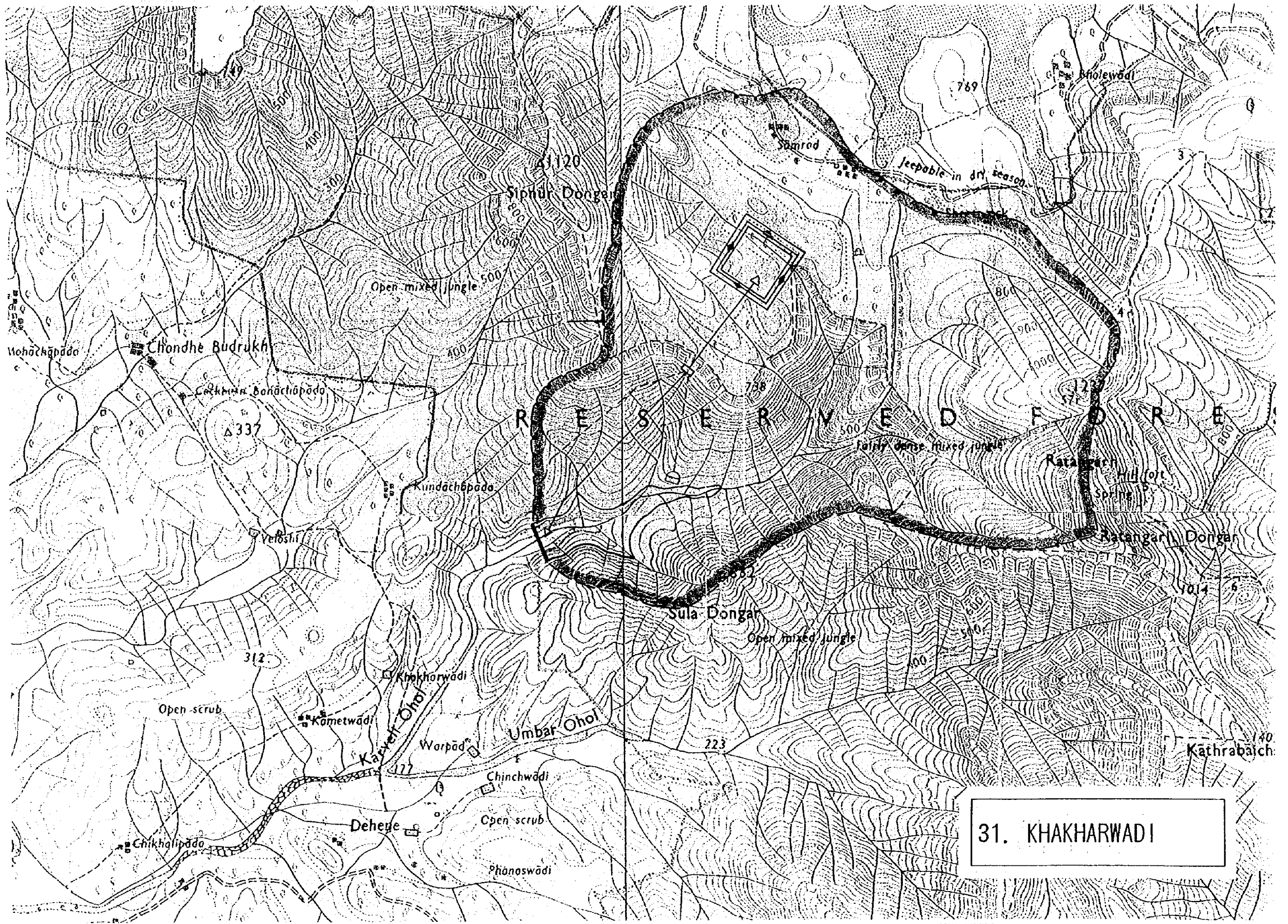
30. KINJALE

Jun

31: General Description of the KHAKHARWADI Project Site

1. Environment conditions	(1) Upper reservoir		The submerged area involves 24 ha of non-reserved forest
	(2) Lower reservoir		the submerged area involves 30 ha of reserved forest and 5 ha of agricultural land
2. River basin conditions	(1) Upper reservoir	(1) River basin	The artificially excavated and embanked type pondage is to be layouted on a plateau of 750m elevation in the most upstream part of the river Karveli Ohol, a tributary emptying into the river Shal on the right bank.
		(2) Submerged dwellings	As it is an artificially created pondage, there is no submerged dwelling.
		(3) River bed condition	Non
		(4) Circum-reservoir terrain	Near the reservoir, terrain forms a plateau. On the south side, there are steep cliffs towards the lower reservoir.
	(2) Lower reservoir	(1) River basin	The basin is located upstream of the river Karveli Ohol, and the most upstream part forms a plateau of 700 ~ 800m elevation. Past the steep cliffs the steep slopes dip toward the river. Both bank of the river have a relatively gentle topography.
		(2) Submerged dwellings	It is not a flat plain. Consequently, there is no dwelling here.
		(3) River bed condition	The river in its most upstream part flows through a gently inclined highland plateau. At around 700m elevation, it passes through steep cliffs and becomes a rapid torrent. At the 2km upstream of the dam site there is a confluence of two streams merging, with the river forming a gentle gradient to empty into the reservoir.
		(4) Circum-reservoir terrain	The terrain around the reservoir, including both banks, has comparatively gentle gradient, with being assumed to be the deposits of rock debris. Only around the dam site does the relief become steep on both banks.
	(3) Changes in river basin		The artificial type pondage is in the most upstream part of the Karveli Ohol river. The lower reservoir is to be located at a distance of 2.6km from the upper pondage. This river basin does therefore not need any changes to the natural water flow.
	3. Location and condition of structures	(1) Upper reservoir dam	(1) Dam site status
(2) Dam			The effective water storage capacity is $5.7 \times 10^6 \text{ m}^3$
(2) Lower reservoir dam		(1) Dam site status	Both banks have steeply dipped slopes. They are narrow, and the dam is to be sited in a position in which the river bed has a relatively steep gradient. There are few deposits on the river bed.
		(2) Dam	On both banks of the dam site, a steep gradient of slope is formed. It is a favorable location for siting the dam, but even when the dam height is raised, the water storage efficiency remains poor and no further increase in storage capacity can be expected. The limit for the dam height at present is considered to be 80m.
(3) Waterway route		(1) Geographical profile	The waterway system has been selected by taking into consideration the natural relief. From the intake the terrain forms a flat plateau. After this there are slopes dipping down smoothly without undulations towards the lower reservoir.
		(2) Layout	For topographical reasons, a penstock line runs immediately from the intake, passing through the powerhouse to terminate in the tailrace. As a result, there is the weakness that the tailrace from the powerhouse is rather long.
(4) Intake and outlet		Since the pondage is planned as an artificial type, the intake uses a morning glory type structure while the outlet is planned as a normal horizontal type structure.	
(5) Surge tank		Since there is no headrace, there is also no need to provide a surge tank on the intake side. As the tailrace tunnel is rather long as 1.6km, this requires the construction of a surge tank.	
(6) Powerhouse		The powerhouse is located on the intake side at a position of 1.6km from the outlet. The access tunnel to the powerhouse is therefore rather long.	
4. Access road and tunnel		(1) Upper and lower reservoirs	
	(2) Access tunnel to powerhouse		A 2.3km long tunnel is to be built from the right bank of the lower reservoir dam.
	(3) Cable tunnel		The switchyard is to be layouted near the dam site on the lower reservoir. From this point, an approximately 0.8km long inclined tunnel is to connect to the transformer room.
5. Power transmission lines			To construct the 400kV one circuit, double conductors, 50km from Khakharwadi PPS to Padghe S/S along planning of the 500kV HVDC transmission line between Chandrapur S/S and Padghe S/S.
6. General evaluation			This project uses a pondage type structure for the upper pondage. The reservoir storage capacity is by no means inferior, but it has a somewhat low head in the order of 400m. This site is therefore not an economically favorable site, though there are few areas of submerged forest.





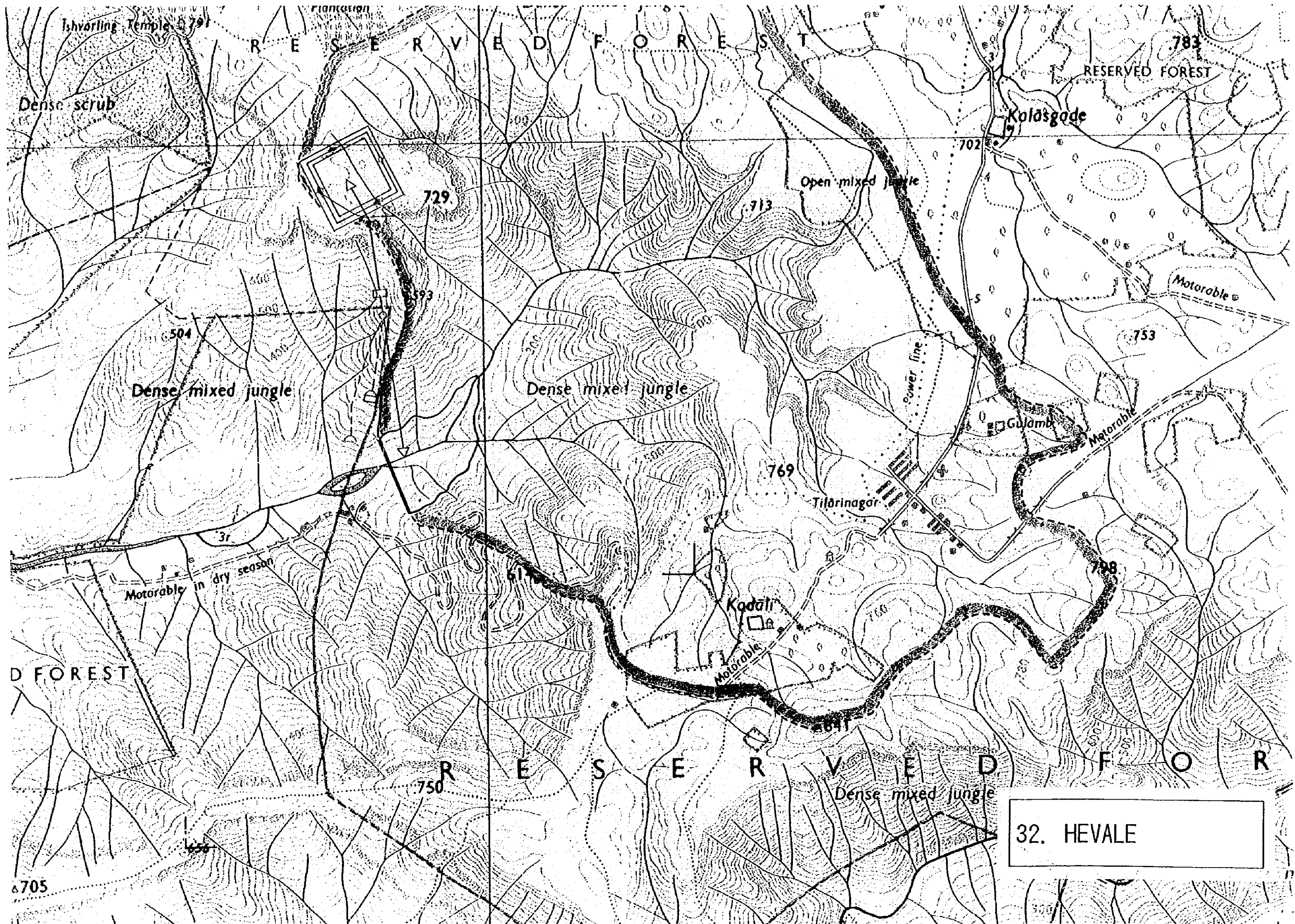
31. KHAKHARWADI

32: General Description of the HEVALE Project Site

1. Environment conditions	(1) Upper reservoir	The submerged area involves 32 ha of reserved forest		
	(2) Lower reservoir	The submerged area involves 50 ha of reserved forest		
2. River basin conditions	(1) Upper reservoir	(1) River basin	The excavated and embanked pondage is to be built in a hilly area on a plateau in the order of 700m elevation in the most upstream part of the river Kharal Nadi, a tributary emptying into the river Tilari Nadi on the right bank.	
		(2) Submerged dwellings	As the plan is the artificially created pondage, the site is selected with no submerged dwelling.	
		(3) River bed condition	Non	
		(4) Circum-reservoir terrain	The relief is a hilly plateau in the order of 700m elevation with the grass area. Except for the north side, the relief consists of the rock exposed steep cliffs.	
	(2) Lower reservoir	(1) River basin	The river basin is located upstream of the Kharal Nadi and the most upstream part of this river forms a hilly plateau of over 700m elevation. Past this plateau, the relief is one of steep mountains with overhanging cliffs in some parts. From the skirting areas of the reservoir, the terrain marks a drop in elevation toward the river with its gentle gradient of flow.	
		(2) Submerged dwellings	The reservoir is planned at a location in which the steep mountainous terrain becomes somewhat more gentle. There is no dwelling.	
		(3) River bed condition	At a position approximately 0.5km upstream of the dam site, the river branches off into two streams extending to the plateau which forms the watershed peak of the basin. These streams divide into innumerable mountain streams and the signs of erosion are in evidence. Near the dam site there are rock debris layers which are not thought to be of any considerable depth.	
		(4) Circum-reservoir terrain	In the terrain around the reservoir on the left bank in the upper reaches, the topography is relatively gentle and there are somewhat deposits of rock debris. Apart from this, the surrounds of the reservoir present a gentle topography. The rock bed is exposed with the thin deposits.	
		(3) Changes in river basin	The upper pondage is located on a plateau in the most upstream part of the same river Kharal Nadi. The lower reservoir is located further downstream so that this river basin does not need any changes to the natural water flow.	
	3. Location and condition of structures	(1) Upper reservoir dam	(1) Dam site status	Non
(2) Dam			The effective storage capacity of this artificial pondage is estimated as $7.6 \times 10^6 \text{ m}^3$ .	
(2) Lower reservoir dam		(1) Dam site status	On both banks of the dam site, the relief has a steep gradient and the rock bed is not deep. Near the river bed the gradient eases to a gentle slope, with evidence that there are somewhat deposits at the both banks of the river.	
		(2) Dam	On both banks of the dam site, the relief has a steep gradient, and the dam basement has a relatively wide. Great increasing of the storage capacity by raising the dam height is not an economically tenable proposition, as the water storage efficiency is poor because of the steep relief on both banks around the reservoir.	
(3) Waterway route		(1) Geographical profile	The waterway system layout has been selected by taking into consideration the natural relief. The perpendicular relief shows few undulation past the plateau on which the upper reservoir is located. The slope falls at a gentle gradient towards the lower reservoir.	
		(2) Layout	Because of the short stretch of the plateau on which the pondage is provide, the waterway system is layouted so that a penstock line runs from the intake and connects to the tailrace, with the powerhouse located on the intake side. The waterway system has a small total extended length of only 2.5 km.	
		(4) Intake and outlet	Since the upper pondage is an artificial type, the intake requires a morning glory type structure. The outlet is to be constructed using an ordinary horizontal type of structure.	
		(5) Surge tank	The upper pondage needs no headrace and, consequently, there is no need to provide a surge tank on the intake side. The tailrace is 1.45km long, however, so that it requires the construction of a surge tank.	
		(6) Powerhouse	The powerhouse is located at a position 1.45km from the outlet, situated deep underground a little close to the intake.	
4. Access road and tunnel		(1) Upper and lower reservoirs		For access to the upper reservoir an access road of approximately 1.5km length needs to be led from the existing road which passes through the outskirts of the village Dhangarvadi. For access to the lower reservoir, however, there is already a nearby road in existence.
	(2) Access tunnel to powerhouse		A 1.5km access tunnel is planned to be provided from the right bank of the lower reservoir.	
	(3) Cable tunnel		The switchyard has to be built downstream of the lower reservoir in the same manner as the access tunnel to the powerhouse. A 1.5km long cable tunnel is required on the layout to connect to the underground transformer room.	
5. Power transmission lines	To construct the 400kV one circuit, double conductors, 100km from Hevale PPS to Kolhapur II S/S along the 220kV double circuits transmission line between Ponda S/S and Kolhapur II S/S.			
6. General evaluation	This project is consisted of the artificial pondage type and has a water storage capacity in the order of 1,000mw with a short extended length of the waterway. Since the powerhouse is located close to the intake, the construction costs for the structures belonging to the powerhouse are somewhat high, making it the project of somewhat poor economic efficiency. There are however a little big areas of submerged forest on the lower reservoir.			





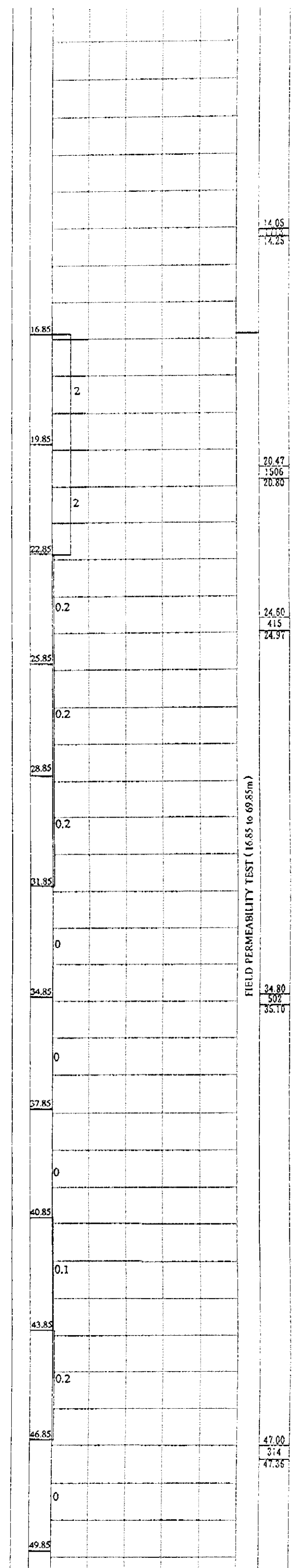
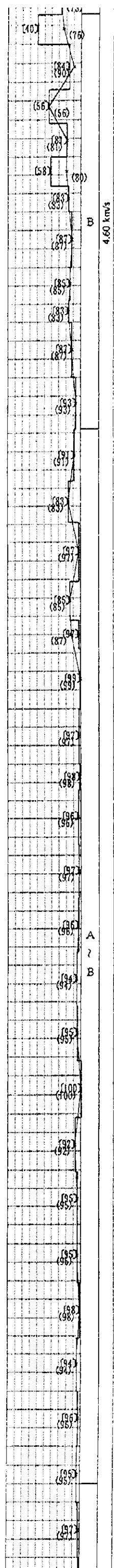
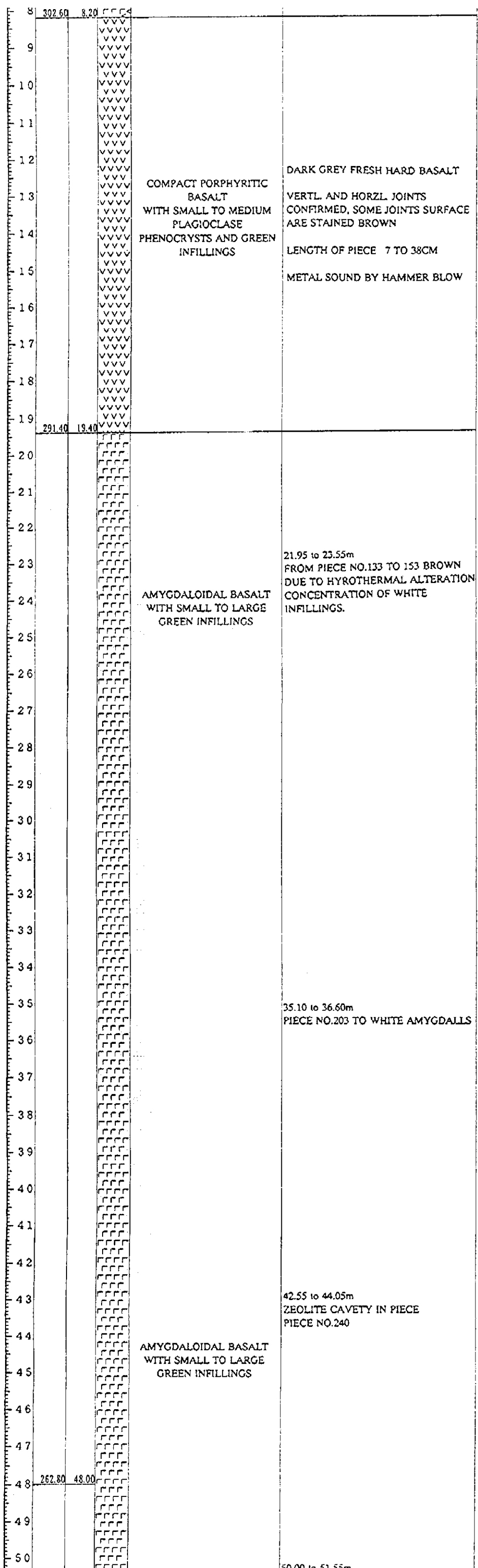


32. HEVALE

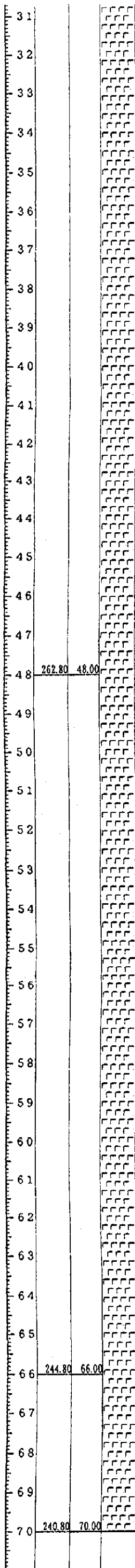
## **Appendix 2 Drilling Log**







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AMYGDALOIDAL BASALT  
WITH SMALL TO LARGE  
GREEN INFILLINGS

35.10 to 36.60m  
PIECE NO.203 TO WHITE AMYGDALLS

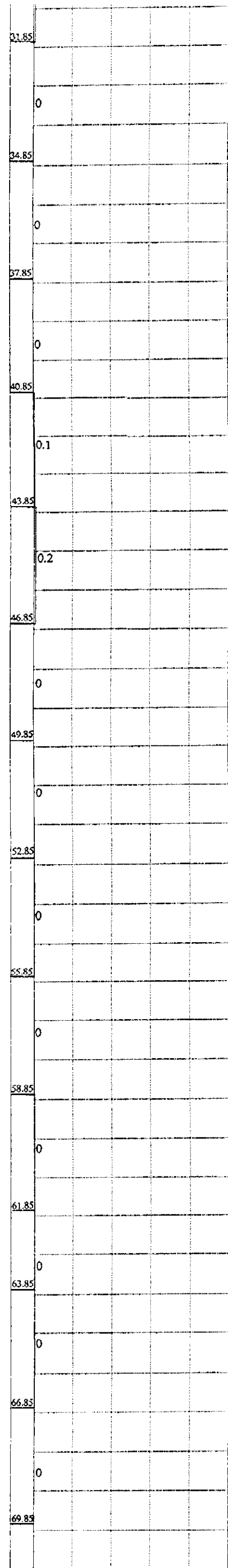
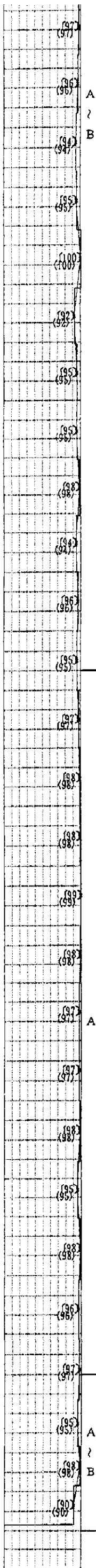
42.55 to 44.05m  
ZEOLITE CAVITY IN PIECE  
PIECE NO.240

50.00 to 51.55m  
CONCENTRATION OF WHITE  
AMYGDALLS IN PIECE NO.279 TO 283

53.00 to 54.50m  
ZEOLITE VEIN IN PIECE NO.287,288,289

59.00 to 60.55m  
CONCENTRATION OF WHITE  
AMYGDALLS IN PIECE NO.307 TO 310

60.55 to 62.05m  
LARGE PIPE AMYGDALLS IN PIECE  
NO.310

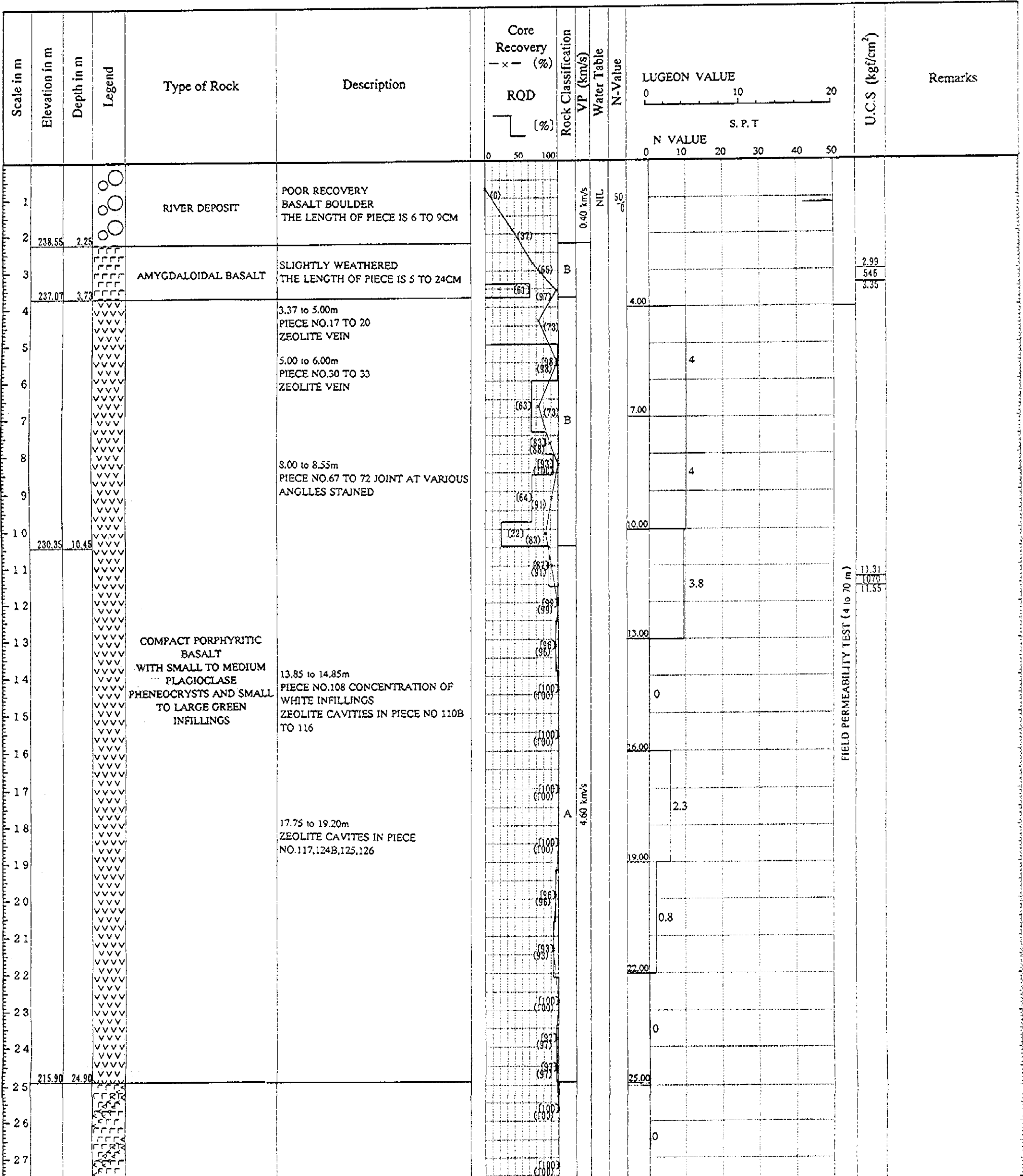


FIELD PERMEABILITY TEST

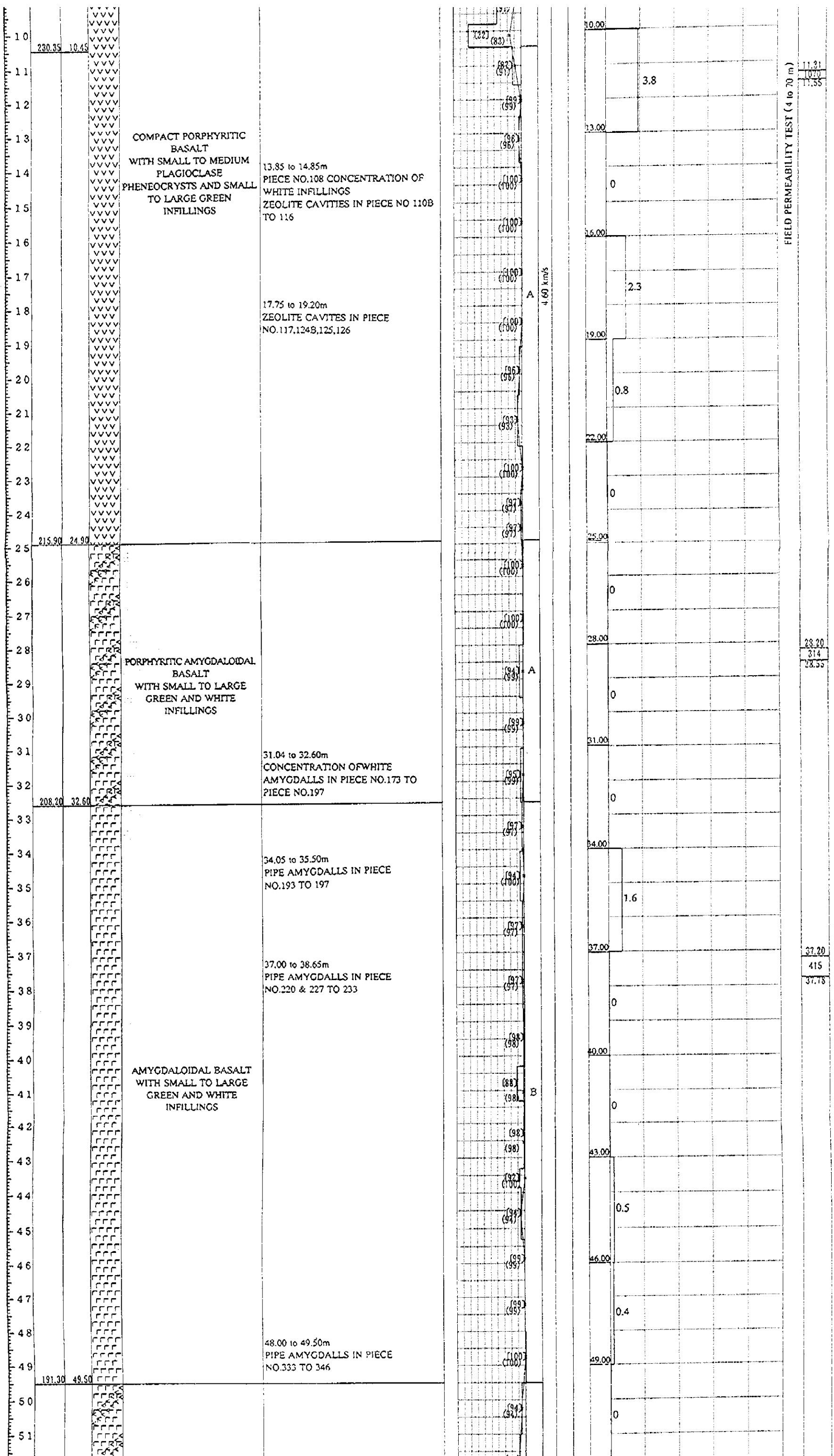
34.80
502
35.10
47.00
314
47.36
59.00
262
59.36

Fig. APP.2-2 **DRILLING LOG**

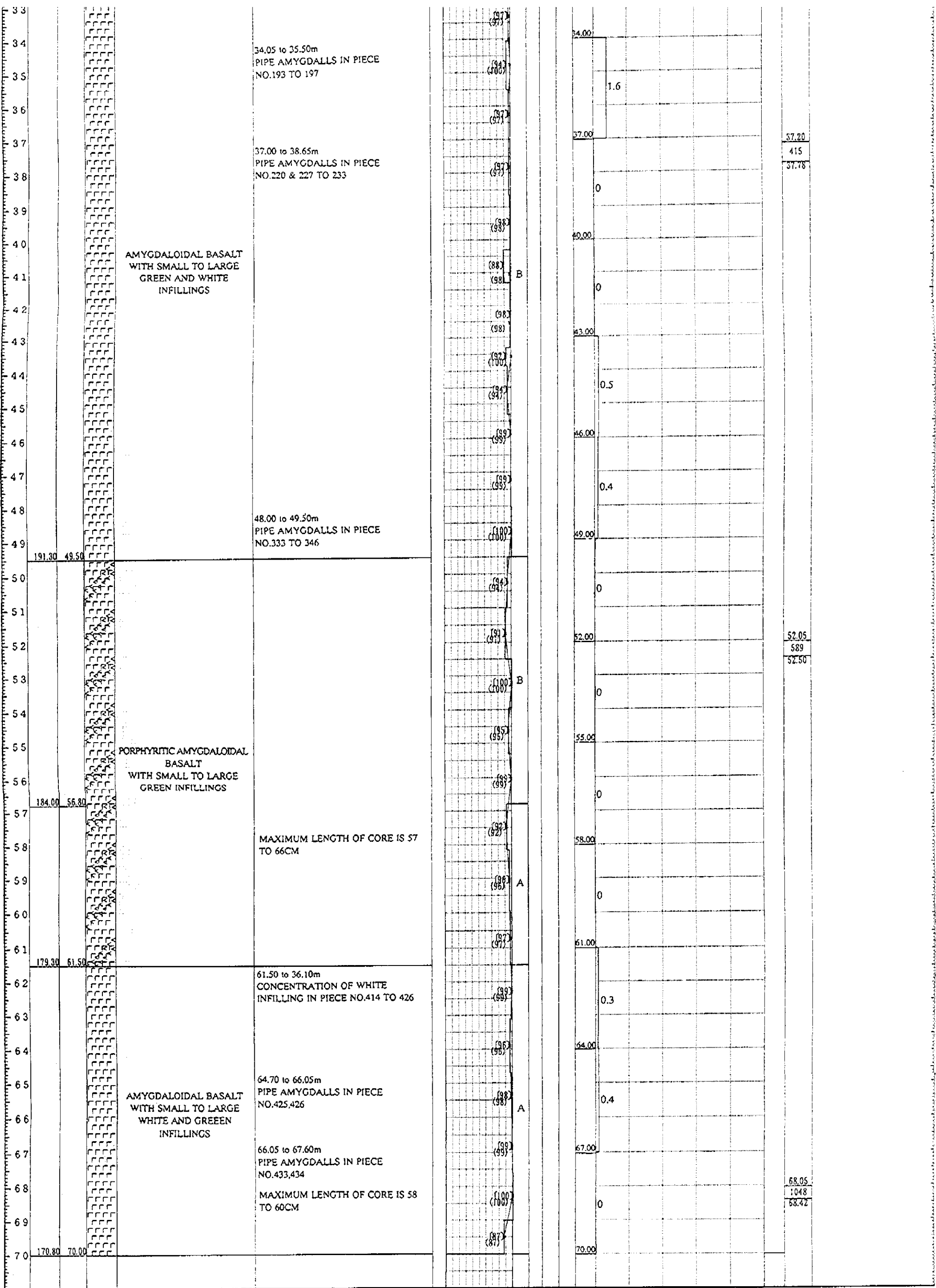
<b>PROJECT</b>	GEOLOGICAL SURVEY FOR MASTER PLAN STUDY ON POMPED STORAGE HYDROELECTRIC POWER DEVELOPMENT IN MAHARASTRA STATE, INDIA							
<b>CLIENT</b>	JAPAN INTERNATIONAL COOPERATION AGENCY			<b>DATE</b>	12/DEC./1995 ~ 5/FEB./1996			
<b>CONSULTANT</b>	CONSULTING ENGINEERING SERVICES ( I ) PVT. LTD			<b>DRILLER</b>	DBM GEOTECHNICS AND CONSTRUCTIONS PVT.LTD			
<b>B.H.No.</b>	LJ-2	<b>Elevation</b>	R.L. 240.8 m	<b>Total Depth</b>	70.00 m	<b>Location</b>	JAROND PROJECT SITE	
<b>Equipment and Method</b>	CALYX Rotary Coring Water Flush and with Diamond Bits			<b>Diameter of Hole : NX (mm)</b>		<b>Sheet No.</b>	OF	



FIELD PERMEABILITY TEST (4 to 70 m)





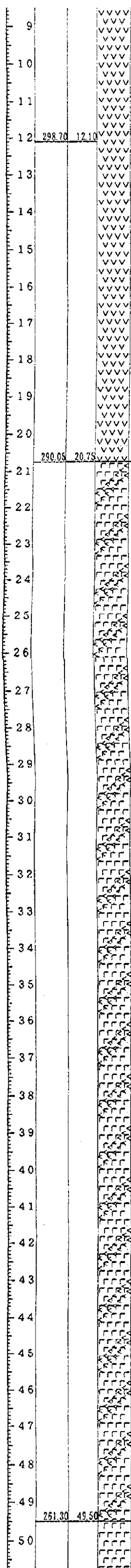


57.20  
415  
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52.05  
589  
52.50

68.05  
1048  
68.42





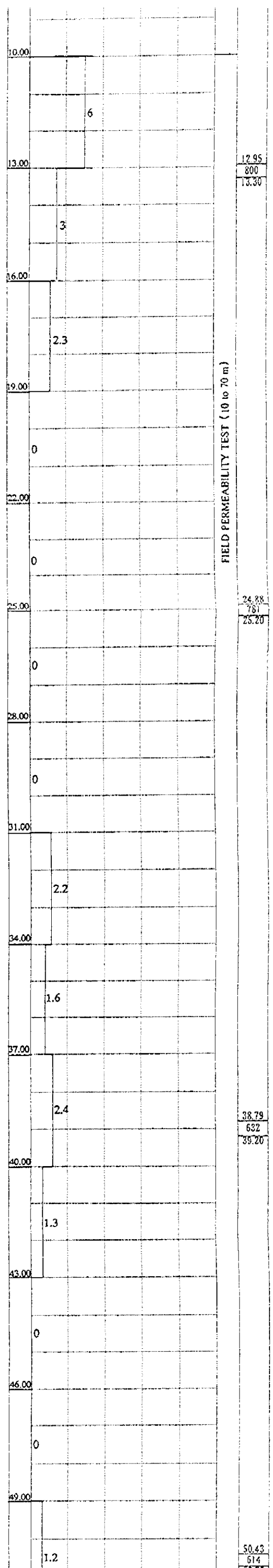
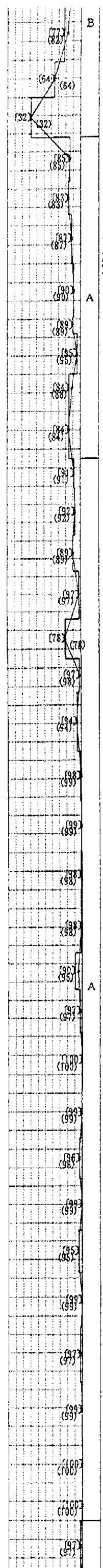
COMPACT PORPHYRITIC  
BASALT  
WITH SMALL TO MEDIUM  
PLAGIOCLASE  
PHENOCRYSTS AND  
GREEN  
INFILLINGS

PORPHYRITIC AMYGDALOIDAL  
BASALT  
WITH SMALL TO MEDIUM  
PLAGIOCLASE  
PHENOCRYSTS AND SMALL  
TO LARGE GREEN  
INFILLINGS

11.00 to 12.10m  
FROM PIECE NO.68 TO 86  
CONCENTRATION OF WHITE  
INFILLINGS  
PIPE AMYGDALLS IN PIECE NO.78,81.  
ETC

40.30 to 41.70m  
CONCENTRATION OF LARGE WHITE  
INFILLINGS FROM PIECE NO.136 TO  
188

47.30 to 48.70m  
CONCENTRATION OF SMALL TO  
LARGE WHITE INFILLINGS IN PIECE  
NO.203 TO 207



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PORPHYRITIC AMYGDALOIDAL  
BASALT  
WITH SMALL TO MEDIUM  
PLAGIOCLASE  
PHENOCRYSTS AND SMALL  
TO LARGE GREEN  
INFILLINGS

40.30 to 41.70m  
CONCENTRATION OF LARGE WHITE  
INFILLINGS FROM PIECE NO.136 TO  
188

47.30 to 48.70m  
CONCENTRATION OF SMALL TO  
LARGE WHITE INFILLINGS IN PIECE  
NO.203 TO 207

AMYGDALOIDAL BASALT  
WITH SMALL TO LARGE  
GREEN INFILLINGS

57.45 to 58.65m  
PIECE NO.225 TO 228 BROWN DUE TO  
HYDROTHERMAL ALTERATION  
CONCENTRATION OF WHITE  
AMYGDALLS  
PIPE AMYGDALLS IN PIECE NO.226 TO  
250

MAXIMUM LENGTH OF PIECE IS 60  
TO 81CM

MAXIMUM LENGTH OF PIECE IS 58  
TO 87CM

261.30 49.50

240.80 70.00

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34.00	2.2	
37.00	1.6	
40.00	2.4	38.79 632 39.20
43.00	1.3	
46.00	0	
49.00	0	
52.00	1.2	50.43 614 50.76
55.00	0	
58.00	0	
61.00	0	
64.00	0	
67.00	0.3	
70.00	0	68.26 484 68.65

Fig. APP.2-4 **DRILLING LOG**

<b>PROJECT</b>	GEOLOGICAL SURVEY FOR MASTER PLAN STUDY ON POMPED STORAGE HYDROELECTRIC POWER DEVELOPMENT IN MAHARASTRA STATE ,INDIA							
<b>CLIENT</b>	JAPAN INTERNATIONAL COOPERATION AGENCY			<b>DATE</b>	27/DEC./1995 ~ 13/JAN./1996			
<b>CONSULTANT</b>	CONSULTING ENGINEERING SERVICES ( I ) PVT. LTD			<b>DRILLER</b>	DBM GEOTECHNICS AND CONSTRUCTIONS PVT.LTD			
<b>B.H.No.</b>	UM-1	<b>Elevation</b>	R.L. 871.092 m	<b>Total Depth</b>	60.00 m	<b>Location</b>	MARLESHWAR PROJECT SITE	
<b>Equipment and Method</b>	CALYX Rotary Coring Water Flush and with Diamond Bits			<b>Diameter of Hole : NX (mm)</b>		<b>Sheet No.</b>	OF	

Scale in m	Elevation in m	Depth in m	Legend	Type of Rock	Description	Core Recovery - x - (%)	RQD (%)	Rock Classification VP (km/s)	Water Table N-Value	LUGEON VALUE		U.C.S (kg/cm <sup>2</sup> )	Remarks
										0	10		
1					0 to 1.5m DRY HARD GROUND SURFACE LIKE ROCK MASS	(0)	(69)		NIL				
2						(0)	(34)						
3					1.5 to 28.00m RED LATERITE SOIL AND PARTIALLY WEATHERED BOULDER	(0)							
4						(0)							
5						(0)							
6					GENERALLY POOR RECOVERY WITH WATER FLUSH DRILLING	(0)							
7						(0)							
8						(0)							
9						(0)							
10						(0)							
11						(0)							
12						(0)							
13						(0)							
14				LATERITE		(0)							
15						(0)							
16						(0)							
17						(0)							
18						(0)							
19						(0)							
20						(0)							
21					PARTIALLY WEATHERED ROCK PIECES	(10)							
22						(58)							
23						(50)							
24						(20)							
25						(20)							
26						(0)							
27						(20)							

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LATERITE

PORPHYRITIC BASALT (WEATHERED)

PORPHYRITIC BASALT WITH SMALL TO LARGE GREEN INFILLINGS

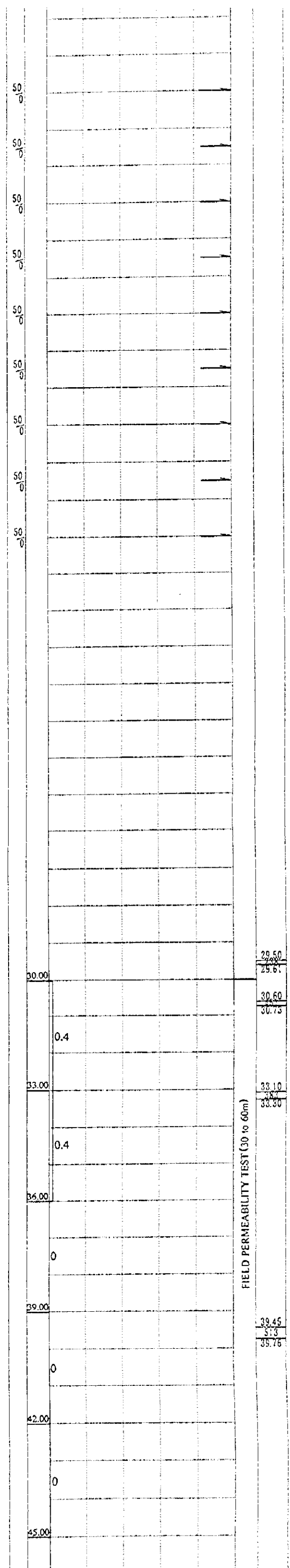
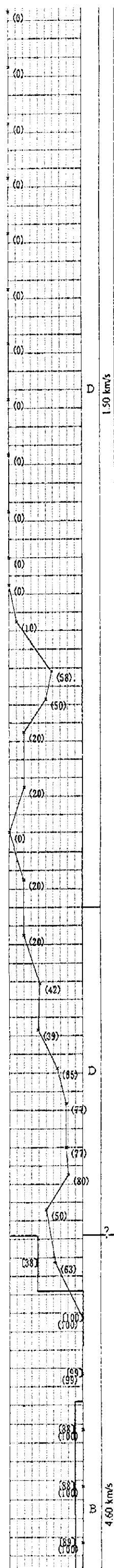
GENERALLY POOR RECOVERY WITH WATER FLUSH DRILLING

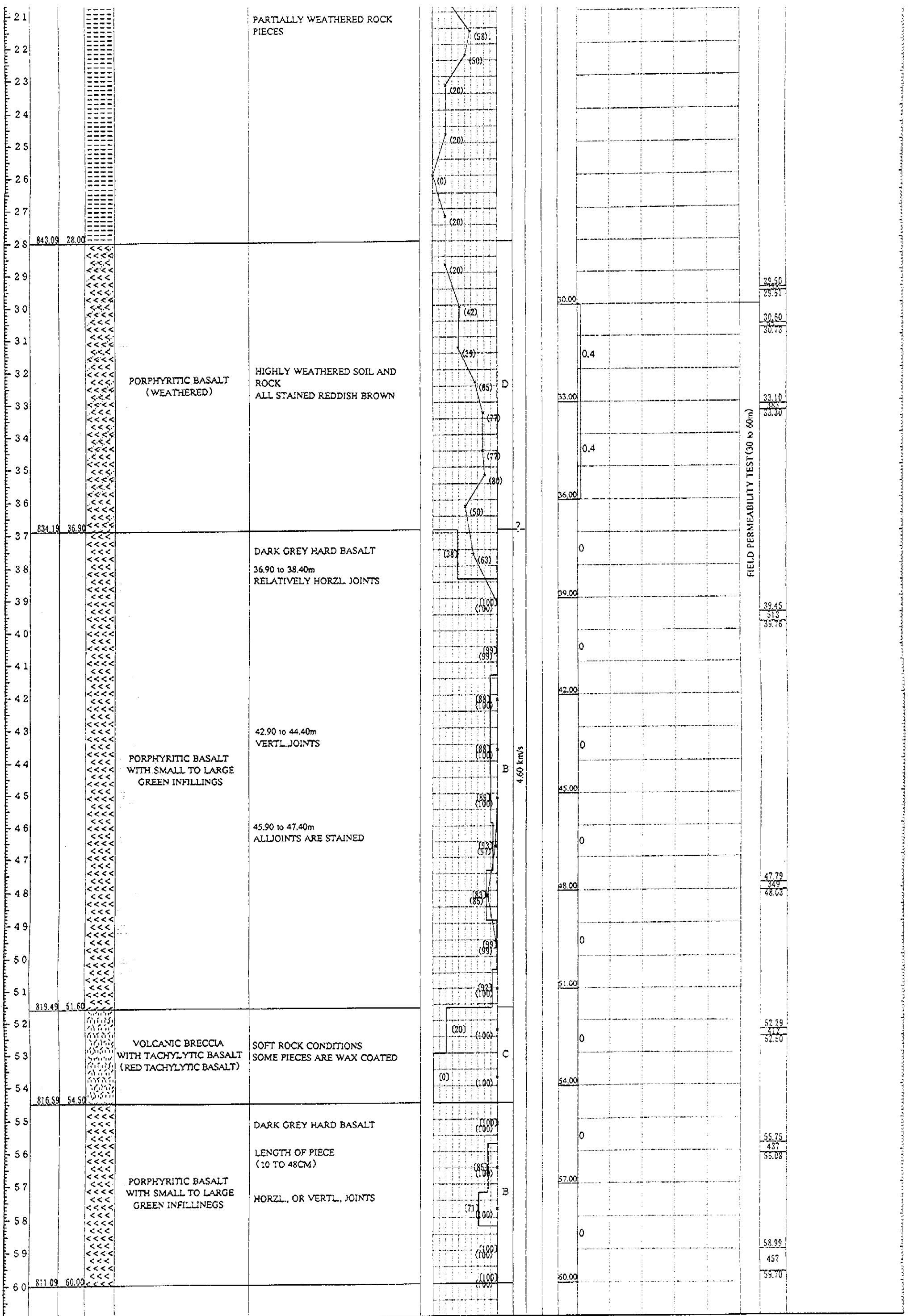
PARTIALLY WEATHERED ROCK PIECES

HIGHLY WEATHERED SOIL AND ROCK ALL STAINED REDDISH BROWN

DARK GREY HARD BASALT  
36.90 to 38.40m  
RELATIVELY HORZL JOINTS

42.90 to 44.40m  
VERTL JOINTS





FIELD PERMEABILITY TEST (30 to 60m)

4.60 km/s

843.09 28.00

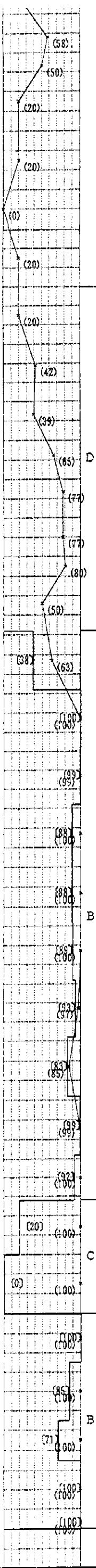
834.19 36.90

819.49 51.60

816.59 54.50

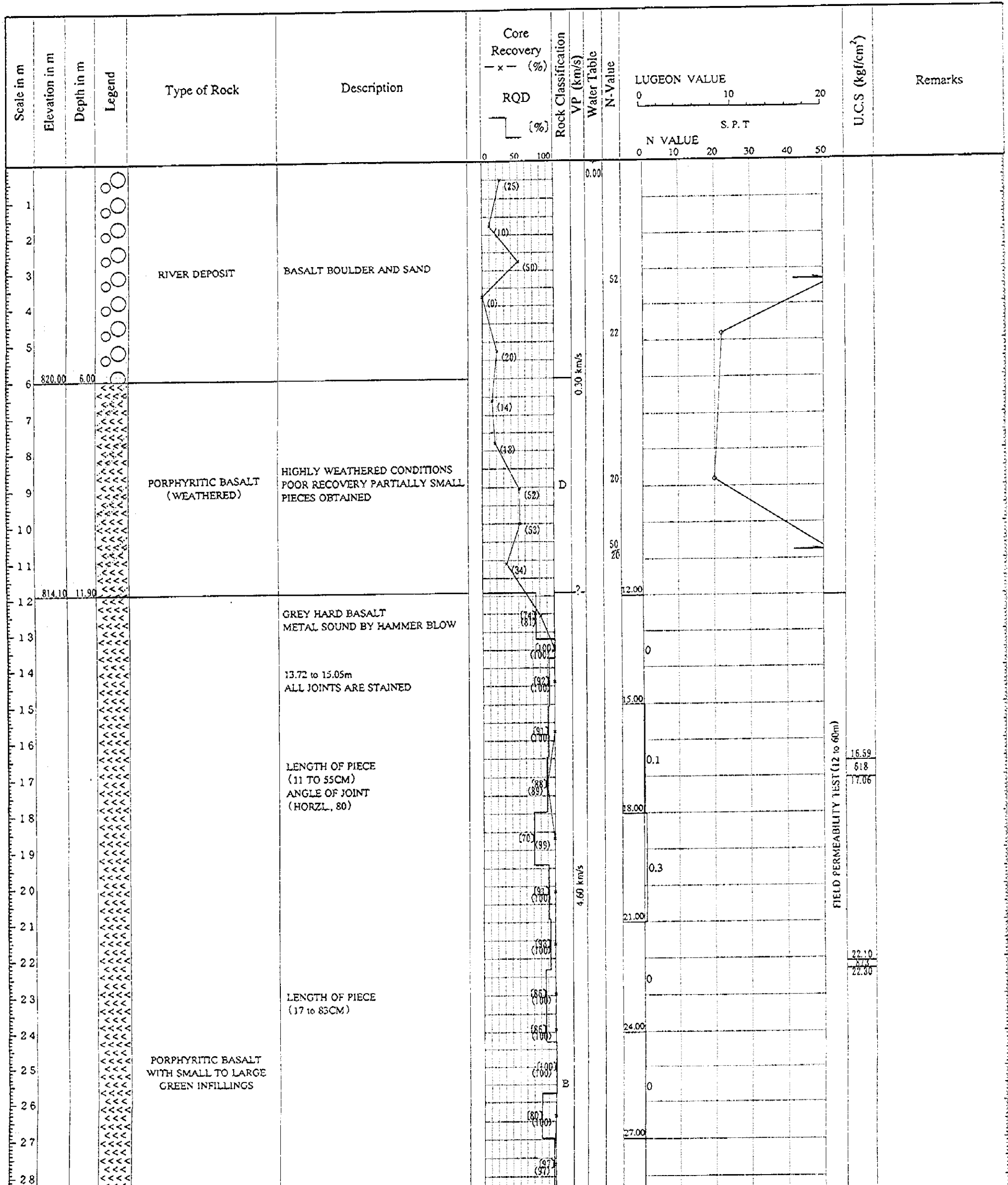
811.09 60.00

28.50  
29.51  
30.50  
30.73  
33.10  
33.30  
39.45  
39.76  
42.00  
45.00  
47.79  
48.03  
51.00  
52.29  
52.50  
55.75  
437  
55.08  
58.99  
457  
59.70

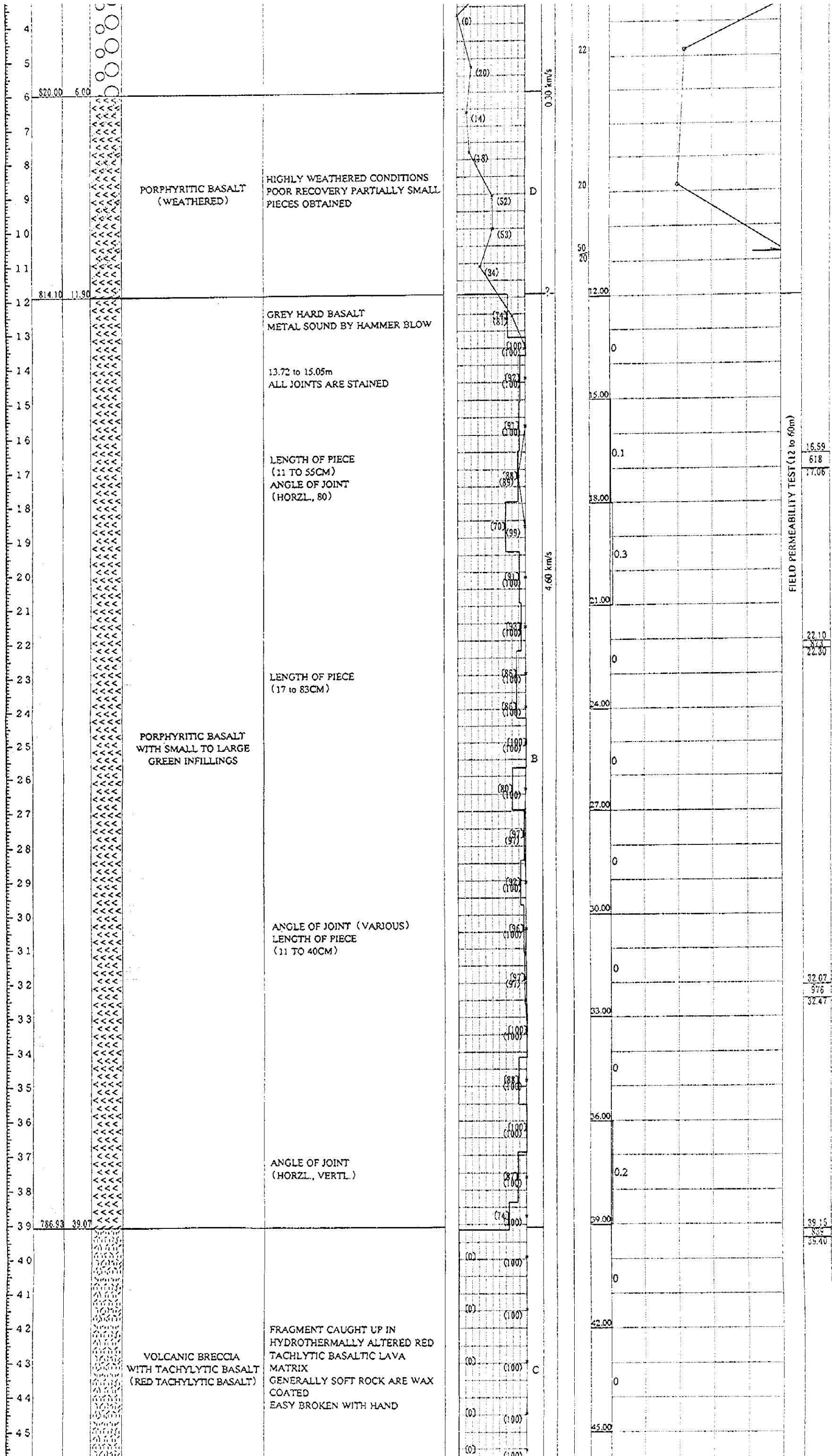


## Fig. APP.2-5 DRILLING LOG

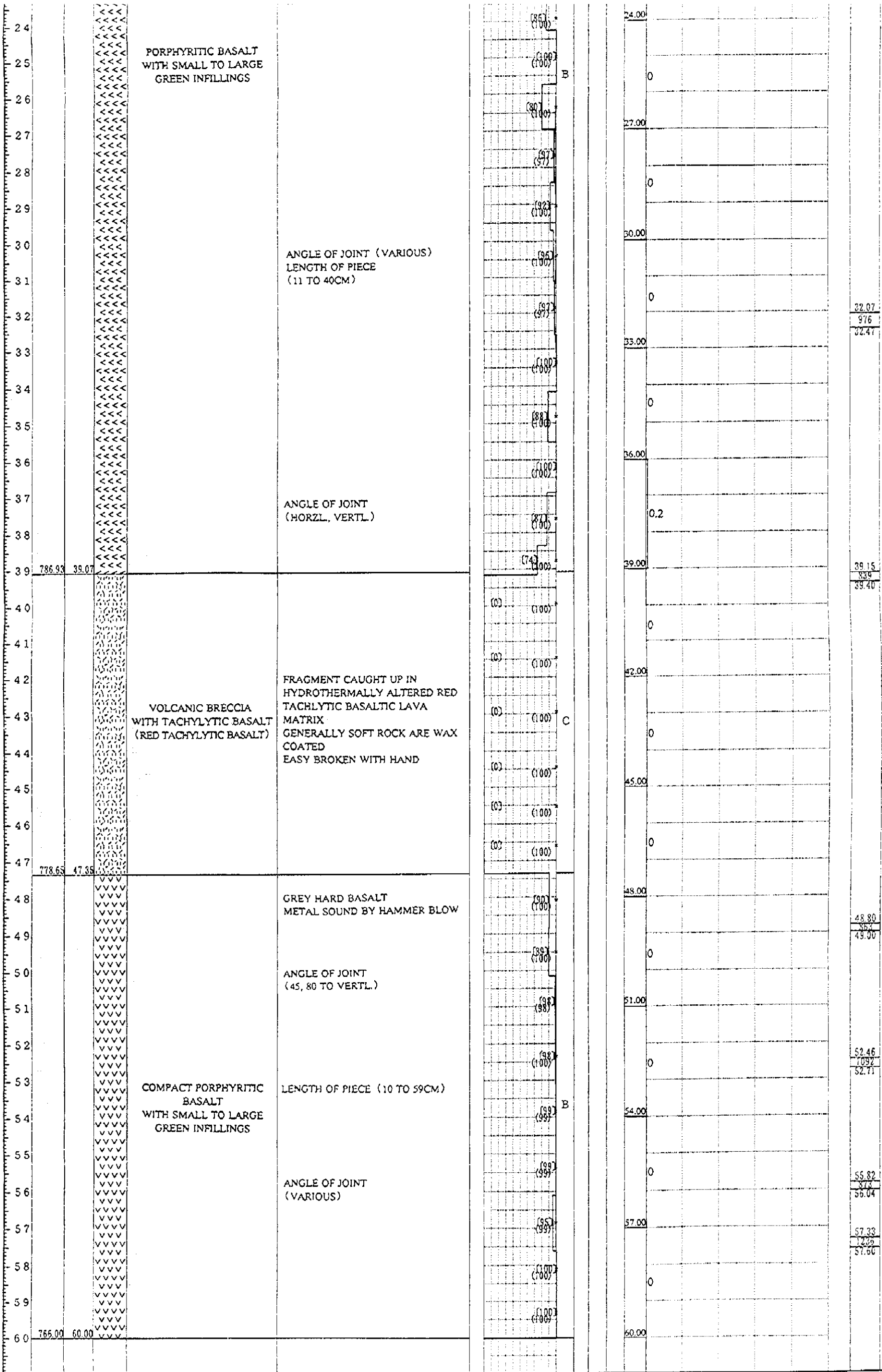
<b>PROJECT</b>		GEOLOGICAL SURVEY FOR MASTER PLAN STUDY ON POMPED STORAGE HYDROELECTRIC POWER DEVELOPMENT IN MAHARASTRA STATE, INDIA						
<b>CLIENT</b>		JAPAN INTERNATIONAL COOPERATION AGENCY		<b>DATE</b>		14/DEC/1995 ~ 4/JAN./1996		
<b>CONSULTANT</b>		CONSULTING ENGINEERING SERVICES ( I ) PVT. LTD		<b>DRILLER</b>		DBM GEOTECHNICS AND CONSTRUCTIONS PVT.LTD		
<b>B.H.No.</b>	UM-2	<b>Elevation</b>	R.L. 826	m	<b>Total Depth</b>	60.00	m	
					<b>Location</b>	MARLESHWAR PROJECT SITE		
<b>Equipment and Method</b>		CALYX Rotary Coring Water Flush and with Diamond Bits			<b>Diameter of Hole : NX (mm)</b>		<b>Sheet No.</b>	OF







This log was prepared from field notes and photographs of the rock face. The rock face was exposed by the removal of the overlying soil and vegetation. The rock face was exposed by the removal of the overlying soil and vegetation. The rock face was exposed by the removal of the overlying soil and vegetation.



32.07  
976  
32.41

39.15  
839  
39.40

48.80  
357  
49.00

52.46  
1092  
52.71

55.82  
372  
56.04

57.33  
1225  
57.50



4  
5  
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45



LATERITE

RED LATERITE SOIL AND PARTIALLY WEATHERED BOULDER

GENERALLY POOR RECOVERY WITH WATER FLUSH DRILLING

CORE RECOVERY GRADUALLY INCREASE 23M TO BOTTOM HIGHLY WEATHERED SMALL PIECES

30.65 to 32.15m CLOSELY JOINT SPACING SLIGHTLY WEATHERED

GENERALLY GREY HARD BASALT

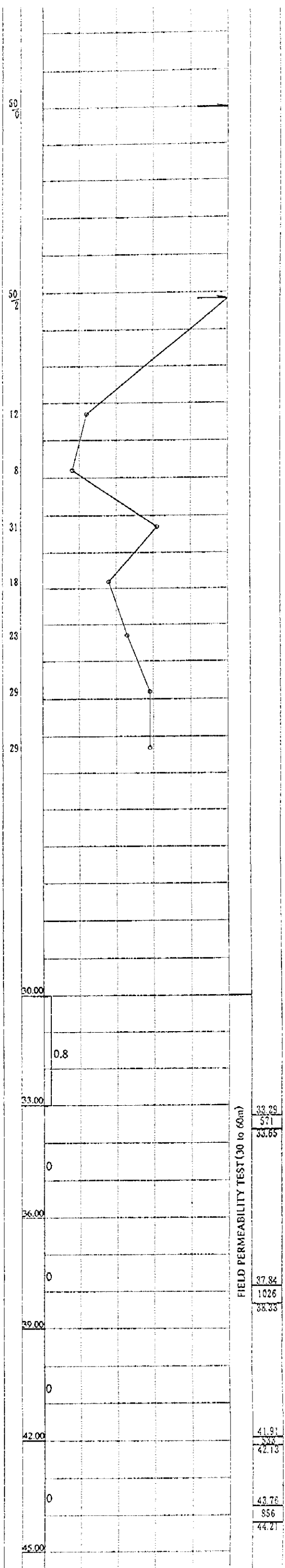
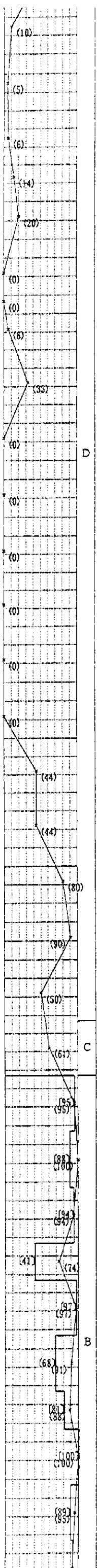
METAL SOUND BY HAMMER BLOW

ANGLE OF JOINT (30,45,HORZL.)

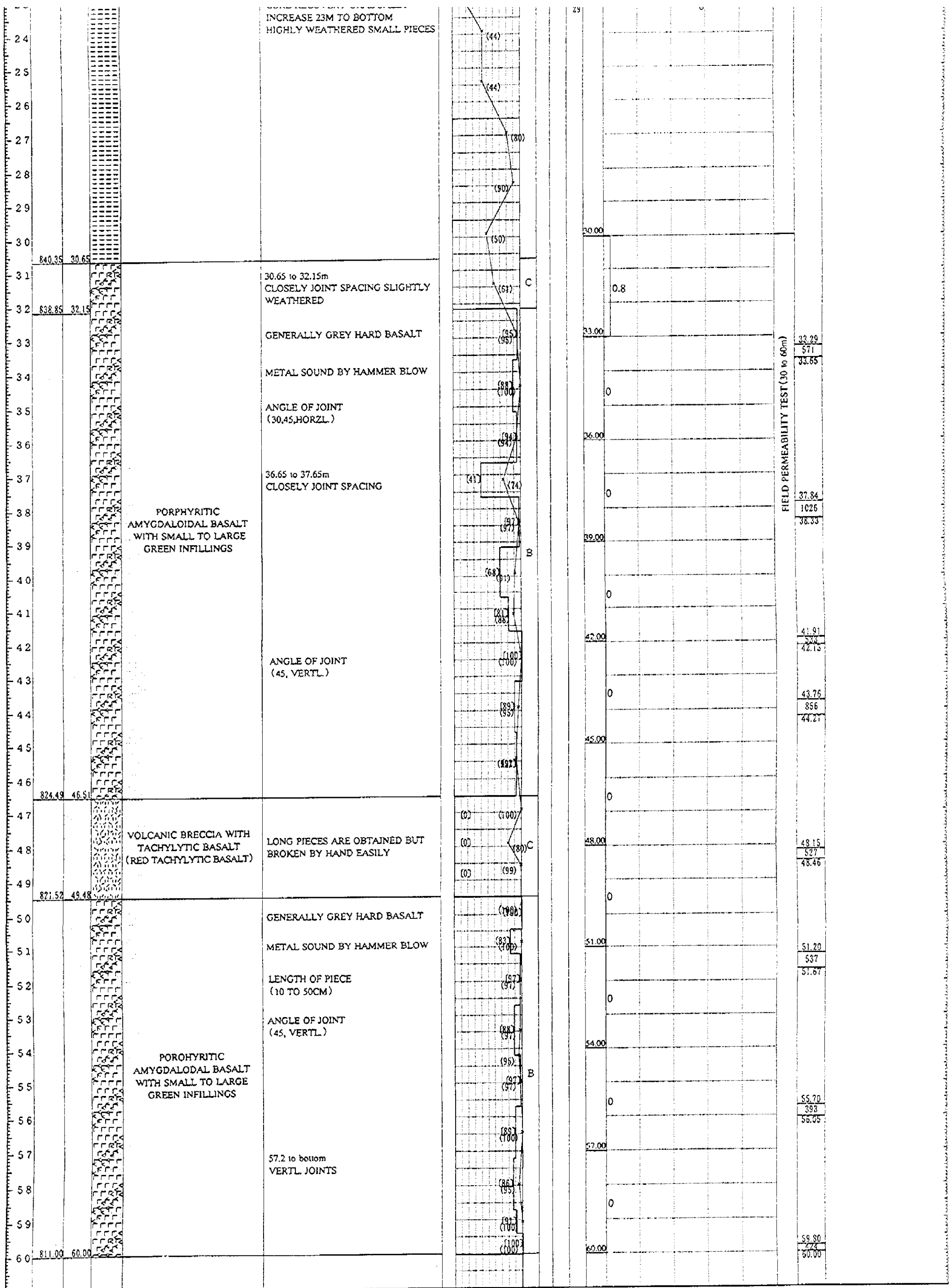
36.65 to 37.65m CLOSELY JOINT SPACING

PORPHYRITIC AMYGDALOIDAL BASALT WITH SMALL TO LARGE GREEN INFILLINGS

ANGLE OF JOINT (45, VERTL.)



30.00	0.8	33.25
31.00	0	571
32.00	0	33.65
33.00	0	
34.00	0	
35.00	0	
36.00	0	
37.00	0	37.84
38.00	0	1026
39.00	0	38.33
40.00	0	
41.00	0	
42.00	0	41.91
43.00	0	42.13
44.00	0	43.78
45.00	0	856
		44.21



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Scale in m	Elevation in m	Depth in m	Legend	Type of Rock	Description	Core Recovery - x - (%)	RQD (%)	Rock Classification VP (km/s)	Water Table N-Value	LUGEON VALUE 0 10 20	S. P. T N VALUE 0 10 20 30 40 50	U.C.S (kgf/cm <sup>2</sup> )	Remarks
1					RED LATERITE SOIL	(0)			NIL				
2					PARTIALLY HIGHLY WEATHERED SMALL PIECES	(0)		0.40 km/s					
3													
4					GENERALLY POOR CORE RECOVERY WITH WATER FLUSH DRILLING	(0)							
5						(27)						4.57	
6						(0)						4.68	
7						(0)							
8						(0)							
9						(0)							
10						(0)							
11				LATERITE		(0)		1.50 km/s					
12						(0)							
13						(0)							
14						(0)							
15						(0)							
16						(0)							
17					SMALL WETHERED PIECES	(39)							
18						(38)							
19						(38)							
20						(53)							
21						(21)							
22						(22)							
23	826.69	22.50			THE ROCK IS IN BROKEN AND WEATHERED CONDITION	(22)							
24					POOR CORE RECOVERY								
25				PORPHYRITIC BASALT (WEATHERED)	SMALL PIECES (2 TO 7CM) 24.65 to 26.15m PIECE NO.86,89,90,98,99,102,103 NO.OF SMALL PCS	(44)		D C					
26						(42)							
27						(77)							
28	821.24	27.95				(87)							
29				PORPHYRITIC BASALT WITH SMALL TO LARGE INFILLINGS	DARK GREY HARD BASALT LENGTH OF PIECE (10 TO 28CM)	(90) (80)		B					
30	819.19	30.00				(85)						29.75	
												388	
												30.00	

This report was prepared by the Geotechnical Engineering Department, Indian Institute of Technology, Kharagpur, West Bengal, India. The data and conclusions are based on the field observations and laboratory tests conducted during the execution of the project. The design and construction of the project should be based on the recommendations made in this report.





Scale in m	Elevation in m	Depth in m	Legend	Type of Rock	Description	Recovery - x - (%)	RQD [ % ]	Rock Classificat VP (km/s)	Water Table N-Value	LUGEON VALUE 0 10 20	S.P.T N VALUE 0 10 20 30 40 50	U.C.S (kgf/cm <sup>2</sup> )	Remarks
1						(12)		0.35 km/s	NIL				
2						(0)							
3						(13)							
4						(15)							
5						(26)							
6						(16)							
7						(0)							
8						(20)							
9				LATERITE	RED LATERITE SOIL PARTIALLY HIGHLY WEATHERED SMALL PIECES	(0)							
10						(20)		1.20 km/s	32				
11					POOR CORE RECOVERY	(34)							
12						(54)							
13						(0)							
14						(0)							
15						(0)							
16						(0)							
17						(0)							
18						(50)							
19						(29)							
20	854.72	19.50		PORPHYRITIC BASALT (WEATHERED)	THE ROCK IS BROKEN AND WEATHERED CONDITION POOR CORE RECOVERY	(55)							
21						(30)							
22	852.22	22.00		PORPHYRITIC BASALT WITH SMALL TO LARGE INFILLINGS	DARK GREY HARD BASALT CORE RECOVERY IS POOR BECAUSE OF PARTIAL CLOSELY JOINT SPACING	(51)							
23						(39)							
24						(39)		4.00 km/s					
25						(71)							
26						(74)							
27						(70)							
28						(70)							
29						(73)							
30	844.22	30.00				(73)							

5.90  
3.36  
6.15

26.99  
4.73  
27.27  
28.27  
28.44