# APPENDIX G AEROPHOTOGRAMMETRIC AND TOPOGRAPHIC SURVEY

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# AEROPHOTOGRAMMETRIC AND TOPOGRAPHIC SURVEY

# Table of Contents

CHAPTER	I	AEROPHOTOGRAMMETRIC SURVEY	G-J
1	1.1	Outline	G-1
	1.2	Contents and Quantities of Survey Works	G-1
	1.3	Method of Survey	G-1
•	1.4	Work Periods	G-3
V.	1.5	Results of the Survey	G-3
CHAPTER	11	TOPOGRAPHIC SURVEY	G-4
	2.1	Outline	G-4
	2.2	Contents and Quantities of Survey Works	G-4
	2.3	Method of Survey	G-4
	:	2.3.1 Datum Height and Coordinates	G-4
		2.3.2 Survey for Kilometer Post Installation	G-:
		2.3.3 Longitudinal Profile Survey	G-5
		2.3.4 Cross-sectional Survey	G-6
	2.4	Work Period	G-6
	2.5	Results of the Survey	G-6
ATTACHMI	2N/P•	CDS SUBVEY NETWORK	

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### CHAPTER I AEROPHOTOGRAMMETRIC SURVEY

### 1.1 Outline

The Survey Works covered the topographic mapping works for the six (6) proposed sabo dam sites (Cura, Labugaon, Solsona No. 1, Solsona No. 2, Madongan, and Papa) and Laoag river basin area by means of photogrammetric mapping.

## 1.2 Contents and Quantities of Survey Works

- (1) Preparation of 1/20,000 and 1/8,000 scale aerial photographs
- (2) 1/10,000 scale photogrammetric mapping
  - (a) Field verification (15 km<sup>2</sup>)
  - (b) Machine plotting (15 km²)
  - (c) Drawing (15 km<sup>2</sup>)
- (3) 1/1,000 scale photogrammetric mapping
- (4) Setting up of photo control points (24 points: 6 sites, 4 points each)
- (5) Photo pricking (24 points in total)
- (6) GPS survey (24 new points in total)
- (7) Leveling (15 km)
- (8) Field verification (1.29 km<sup>2</sup> in total)
- (9) Machine plotting (1.29 km<sup>2</sup> in total)
- (10) Editing and drawing (1.29 km<sup>2</sup> in total)

### 1.3 Method of Survey

(1) Photo Control Point Survey (GPS Survey)

To ensure the accuracy of horizontal positions in photogrammetric mapping, GPS surveying was carried out with the existing GPS stations established by JICA in 1996 as given to obtain coordinates of 24 newly established points. Observations and computations to determine the coordinates were made in the following manner:

- (a) Five or more satellites were observed for signals with 3 receivers in simultaneous use.
- (b) The average height of satellites as observed was more than 15 degrees above the horizon.
- (c) Observation was made for a duration of more than one hour at a time at each station.
- (d) The results of initial computations were based on WGS-84 ellipsoid and, therefore, WGS-84 based coordinates were converted to Philippine standard Clark 1886 ellipsoid based coordinates by applying conversion parameters. After the conversion, they were made into plane coordinates of PTM Zone 3.

The GPS computation results were found to be within the specified limits of accuracy as shown below. The GPS survey network is presented in the ATTACHMENT.

- Horizontal Positions

±2PPM × distance between two points observed

simultaneously.

- Height

±5PPM × distance between two points observed

simultaneously.

### (2) Field Verification

Field verification was conducted to identify and reaffirm in the field such items to be represented in photogrammetric mapping, including geographical names and administrative boundaries, based on the map symbols and their application rules. Findings were incorporated in the two-time enlarged aerial photographs and other related materials to help subsequent plotting and compilation. Prior to the field survey, aerial photographs and other related materials were studied for areas and features that were not clearly identifiable and needed to be clarified, which then were marked on the aerial photographs and other related materials with color dermatograph pencils. The findings of the field verification were edited on the two-time enlarged aerial photographs using color drafting pens to help the compilation work that followed.

### (3) Plotting

Plotting was done by placing a pair of consecutive aerial photographs (diapositive films) side by side in the stereo plotter and recreating the photographed features in stereo (by setting orientation elements of the aerial photographs involving positions, rotation, tilts, etc.). By observing the stereo images a topographic map was created and initially delineated to make plotting manuscript sheets.

Contour line intervals were delineated as follows:

	Item	Contour Line Interval	Intermediate Contour l Interval	ine
• • •	Scale 1/10,000			
	Alluvium	2 m	1 m	
	Mountains and Hills	5 m	<b>-</b> 1	
(b)	Scale 1/1,000			
	Alluvium	1 m	0.5 m	
	Mountains and Hills	2.5 m		

### (4) Compilation

The plotting manuscript sheets were edited according to the map symbols and their application rules and made into 1/10,000 and 1/1,000 scale compilation manuscript sheets with penciled representations.

### (5) Drafting

Based on the compilation manuscript sheets, original topographic maps were drawn on the polyester base (#500) least subject to expansion/contraction in black ink by the standard drafting method (fair drawing) according to the map symbols and their application rules as agreed to by the Department of Public Works and Highways of the Philippine Government.

The same map design as that of the existing 1/10,000 scale topographic maps was applied.

### 1.4 Work Periods

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The field survey was started on 21 January 1997 and completed on 6 March 1997, as originally planned.

- GPS Surveying : 21 January 1997 to 15 February 1997
- Leveling : 21 January 1997 to 15 February 1997
- Field Verification : 21 January 1997 to 15 February 1997
- Stereo Plotting : 21 January 1997 to 06 March 1997
- Drafting : 21 January 1997 to 06 March 1997

1.5 Results of the Survey

All the results are compiled in the following maps.

- (1) 1/10,000 scale topographic maps (3 sheets)
- (2) 1/1,000 scale topographic maps at 6 proposed dam sites (Cura, Labugaon, SolsonaNo. 1, Solsona No. 2, Madongan and Papa)

# CHAPTER II TOPOGRAPHIC SURVEY

### 2.1 Outline

Longitudinal profile and cross-sectional surveys were conducted for the drainage of Laoag City, the 6 proposed dams (Papa, Madongan, Solsona No. 1, Solsona No. 2, Cura, Labugaon), Cura River and Labugaon River.

## 2.2 Contents and Quantities of Survey Works

Contents and quantities of the survey works are summarized as follows:

- (1) Leveling Survey (15 km for Daorao Creek)
- (2) Longitudinal Profile and Cross-sectional Survey

River & Proposed Dam Sites	Longitudinal Survey Length (km)	No. of Cross- sections	Average Cross-section Width (m)	Total Cross-section Survey Length (km)
Daorao Creek &	13.43	61	50	3.05
Tributary			100	
Irrigation Canal	1.48	10	30	0.30
Labugaon & Cura River	12.70	23	400	9.20
Papa Dam	- i -	2	200	0.40
Madongan Dam	•	2	250	0.50
Solsona No. 1 Dam	1.50	7	100	0.70
Cura Dam	•	2	250	0.50
Labugaon Dam	•	1	200	0.20
Total	27.63	108		14.85

## 2.3 Method of Survey

### 2.3.1 Datum Height and Coordinates

The leveling survey was conducted based on the control points and bench marks established by JICA in 1996 as given below:

Point	Coor	dinates	Height	Remarks
Name	В	N		
ILN-1	457,278.457	2,034,691.882		existing GPS point
ILN-3	480,250.294	2,000,321.738	117.114	existing GPS point
FCP-1	460,051.435	2,002,070.770	30.809	established by JICA
FCP-2	454,693.509	2,006,942.481	12.255	ditto
FCP-3	450,198.027	2,011,698.032	2.989	ditto
FCP-4	453,575.167	2,016,709.293	3.206	ditto
FCP-5	462,711.610	2,016,421.710	25.504	địtto
FCP-6	462,154.555	2,008,388.811	10.449	ditto
FCP-7	469,802.942	2,014,350.172	64.197	ditto
FCP-8	469,875.894	2,009,308.932	45.896	ditto
FCP-9	473,176.291	2,010,091.260	45.119	ditto
FCP-10	479,041.395	2,005,007.217	74.381	ditto
FCP-12	475,009.671	1,996,919.224	49.380	ditto
FCP-13	467,849.803	2,005,679.219	22.431	ditto
FCP-14	468,090.910	1,998,722.829	31.207	ditto
FCP-15	465,312.131	1,992,785.510	53.127	ditto
FCP-16	470,378.350	1,989,111.157	87.993	ditto
FCP-17	468,395.324	1,983,408.292	163.955	ditto
FCP-18	464,307.048	1,981,544.604	127.702	ditto
FCP-19	463,637.191	1,983,265.863	103.906	ditto
FCP-20	463,739.865	1,986,499.597	61.288	ditto

The above surveys were performed with the following accuracy.

- (1) Control point survey: closing error of coordinates: < 1/5,000
- (2) Benchmark leveling survey: error < 10 mm S, where, S: one-way distance (km)

# 2.3.2 Survey for Kilometer Post Installation

The kilo-posts were set up through the following procedures.

(1) The cross section lines were instructed by the JICA Study Team on the field.

(a) Daorao Creek (-0.1 km - 13.43 km)	: .	61 sites
(b) Irrigation Canal		10
(c) Cura River	: :	23
d) Proposed Dams (6)	:	14

(2) Closing Error: < 1/3,000

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# 2.3.3 Longitudinal Profile Survey

The longitudinal profile survey measured the elevation of the kilo-posts, tributary confluence, drainage culverts, intake and other major river structures.

The longitudinal profiles are drawn in the following scales:

	Horizontal Scale	Vertical Scale
Solsona Dam	1:5,000	1:600
Labugaon River	1:40,000	1:400
Cura River	1:40,000	1:600
Daorao Creek	1:40,000	1:200
Daorao Tributary	1:5,000	1:100

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The survey was performed with the following accuracy:

Error < 20 mm S, where, S: one-way distance (km)

### 2.3.4 Cross-sectional Survey

The cross-sectional survey was carried out for the 66 river sections at kilo-post. The survey width in cross-sectional direction is as follows:

(1) Drainage Canal / Daorao Creek : Up to 10 m inland of the channel bank

2) Alluvial Fan Area : Up to the proposed dike

(3) Dam Area : Up to 30 m higher than the riverbed

The cross-sections are drawn in the following scales:

	Horizontal Scale	Vertical Scale
Proposed Dam &	1:4,000	1:200
Labugaon, Cura River		
Daorao Creek	1:200	1:200
Daorao Tributary	1:100	1:100
Daorao mouth	1:400	1:100
Irrigation	1:100	1:100

The survey was performed with the following accuracy.

Distance: Error < 1/300

Elevation: Error < 5 cm + 15 cm S/100, where, S: survey length (m)

### 2.4 Work Period

All survey works were completed within one and a half (1.5) months from January 21,1997 to March 6, 1997.

### 2.5 Results of the Survey

All the results are compiled in the following maps, drawings, tables, notes and calculations.

		Foliant and the state of the st
River and	Longitudinal	Cross-Section
Proposed Dam	A-1, A-3 size	A-1, A-3 size
Daorao Creek	1	61
Irrigation Canal	1	10
Cura River	1	23
Papa Dam	-	2
Madongan Dam	-	2
Solsona No. 2 Dam	1	7
Cura Dam	-	2
Labugaon Dam	•	11
Total	4	108

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# ATTACHMENT G

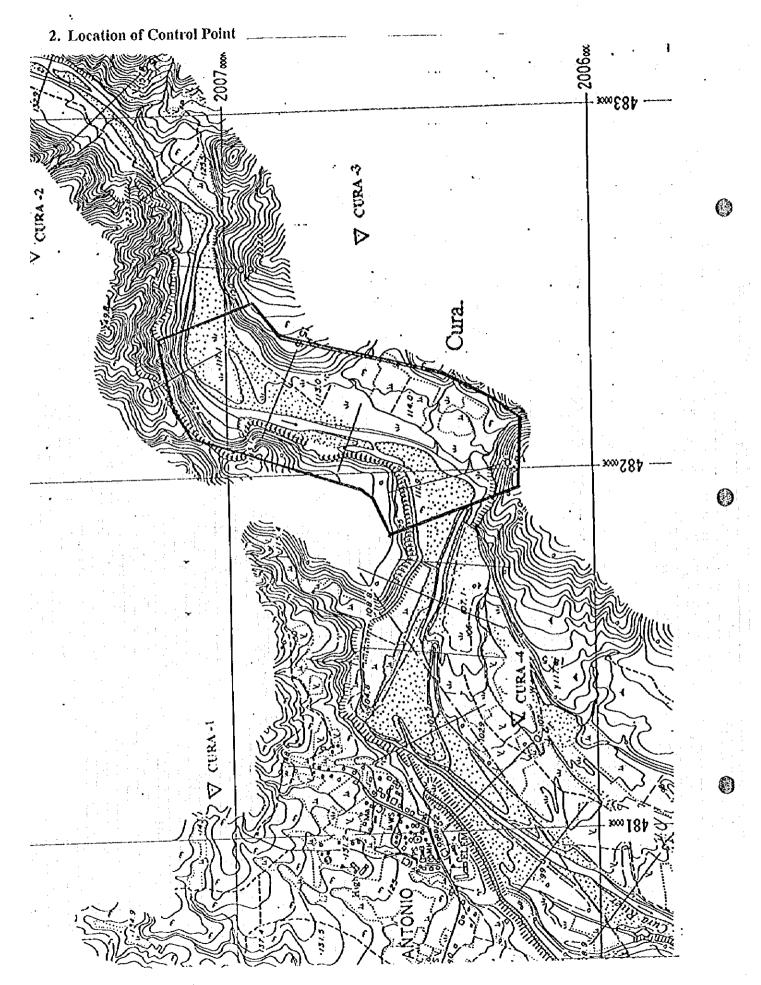
# GPS SURVEY NETWORK

- 1. Name and Coordinates of GPS Photo Control Point
- 2. Location of Control Point
- 3. Description of Control Point (Survey Mark)
- 4. Coordinates Adjustment Data of Control Point

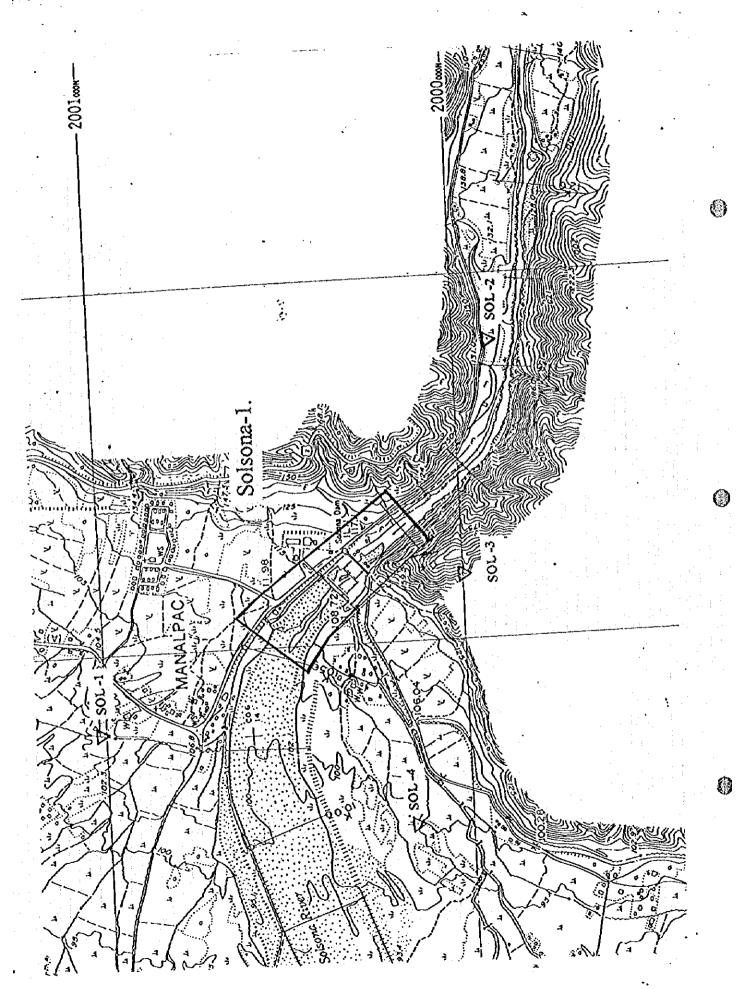
# 1. Name and Coordinates of GPS Photo Control Point

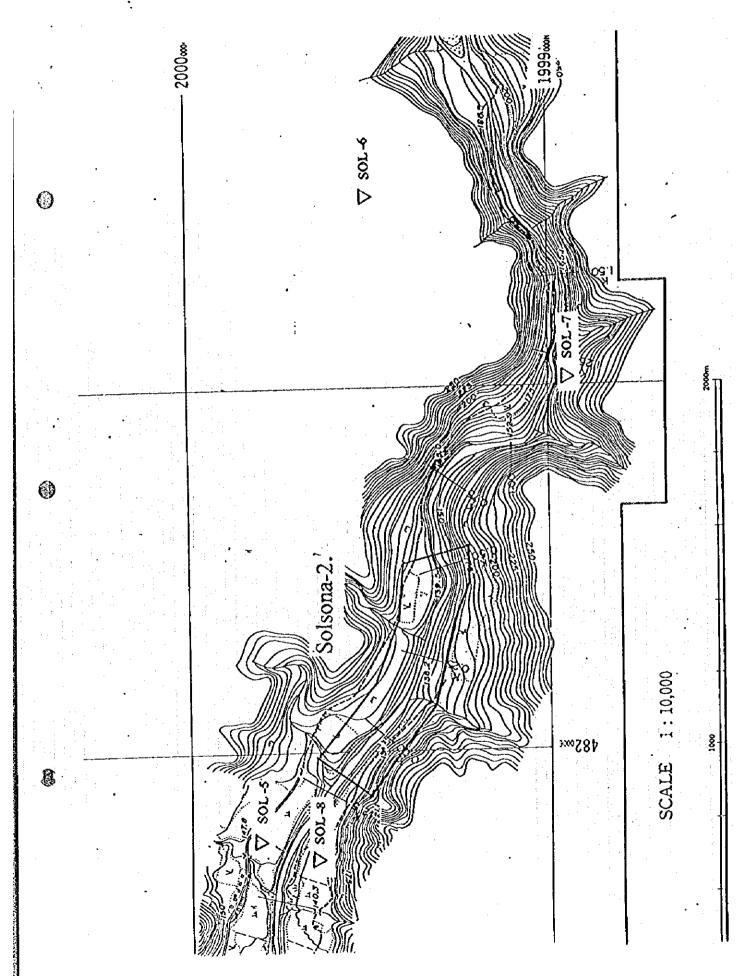
GPS Photo control point

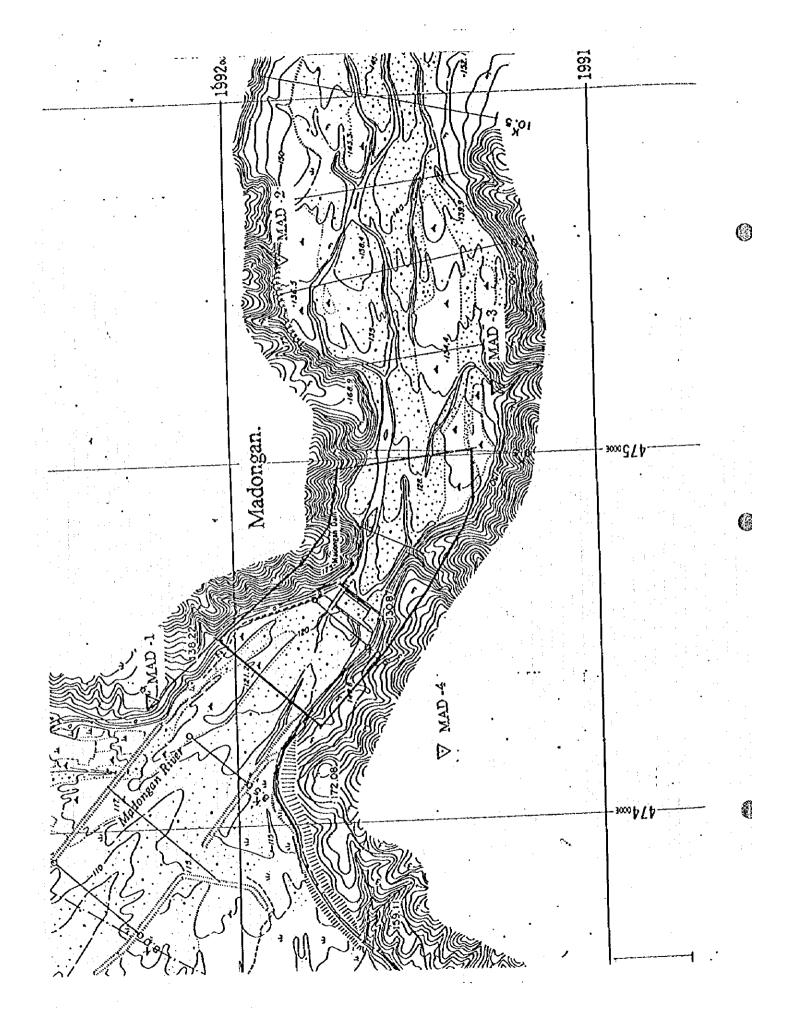
NAME	N	Ε	Н
CURA-1	2007008,659	481137.286	163,059
CURA-2	2007590,109	482627.845	272.545
CURA-3	2006676.996	482724.063	243,207
CURA-4	2006232.717	481316.441	103,184
<del></del>			
LAB-1	2003396.602	482293,886	110,334
LAB-2	2003323.802	483291.466	270.737
LAB-3	2002778.460	483300.845	123.941
LAB-4	2002959.086	481880.621	105.531
SOL-1	2001022.382	479779,891	103.627
\$0L-2	1999870.238	480818,663	124.524
SOL-3	2000008.564	480233.257	210.379
SOL-4	2000168.308	479757.757	104.556
SOL-5	1999814.068	481775.860	145.895
SOL-6	1999515.754	483528,287	458.751
SOL-7	1998986.476	483019.975	159.977
SOL-8	1999662.643	481695.141	141.378
MAD-1	1992233.009	474365.023	147.109
MAD-2	1991838.304	475557,271	138.804
MAD-3	1991263,447	475178.215	136.257
MAD-4	1991439.970	474182.229	260.039
PAP-1	1987302.760	469804.660	109.907
PAP-2	1986935, 197	471380.948	211.500
PAP-3	1986505.496	471469.911	141.112
PAP-4	1986350,932	469800.095	114.439
	:		

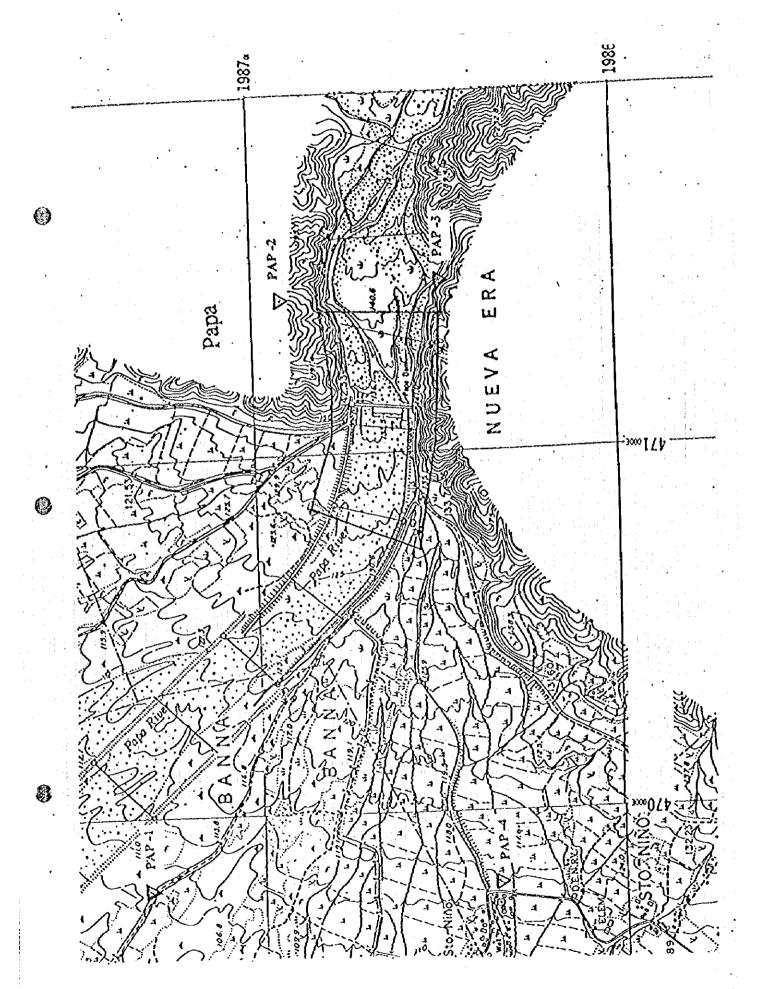












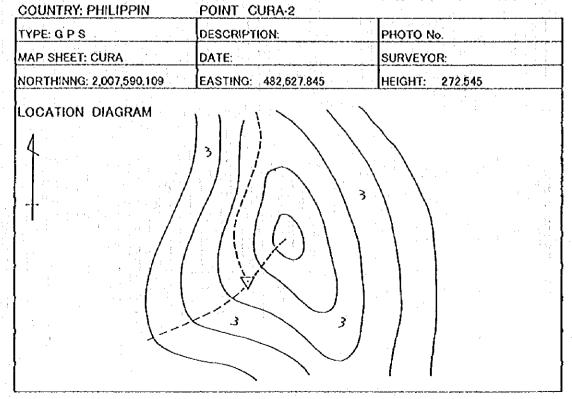
# 3. Description of Control Point (Survey Mark)

# SURVEY MARK DESCRIPTION

### PROPOSED DAM SITE

COUNTRY: PHILIPPIN	POINT CURA-1	
TYPE: Q P S	DESCRIPTION:	PHOTO No.
MAP SHEET: CURA	DATE:	SURVEYOR:
NORTHINNG: 2,007,008.659	EASTING: 481,137.286	HEIGHT: 163,059
LOCATION DIAGRAM		

# SURVEY MARK DESCRIPTION



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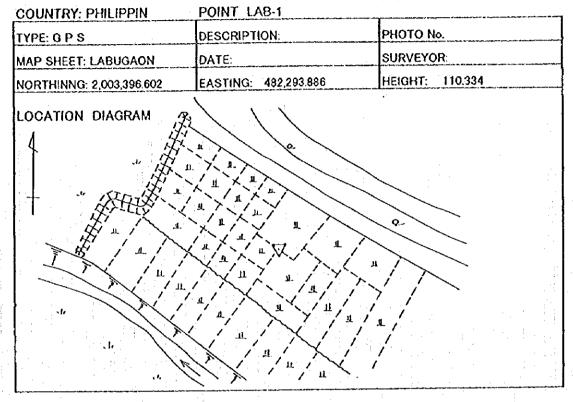
# PROPOSED DAM SITE

COUNTRY: PHILIPPIN	POINT CURA-3	
TYPE: G P S	DESCRIPTION:	PHOTO No.
MAP SHEET: CURA	DATE:	SURVEYOR:
NORTHINNG: 2,006,676.996	EASTING: 482,724.063	HEIGHT: 243.207
LOCATION DIAGRAM	3	

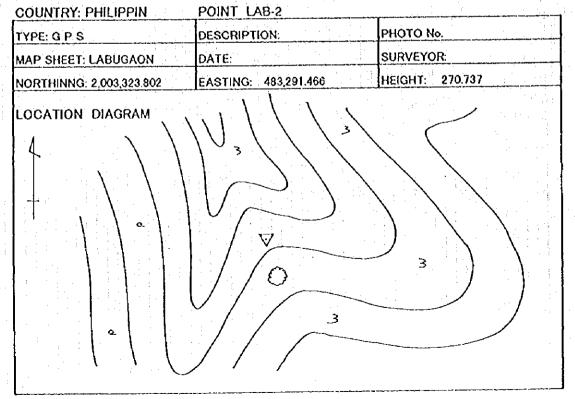
# SURVEY MARK DESCRIPTION

COUNTRY: PHILIPPIN	POINT CURA-4	<del></del>
TYPE: G P S	DESCRIPTION:	PHOTO No.
MAP SHEET: CURA	DATE:	SURVEYOR:
NORTHINNG: 2,006,232.717	EASTING: 481,316.441	HEIGHT: 103.184
LOCATION DIAGRAM		

### PROPOSED DAM SITE



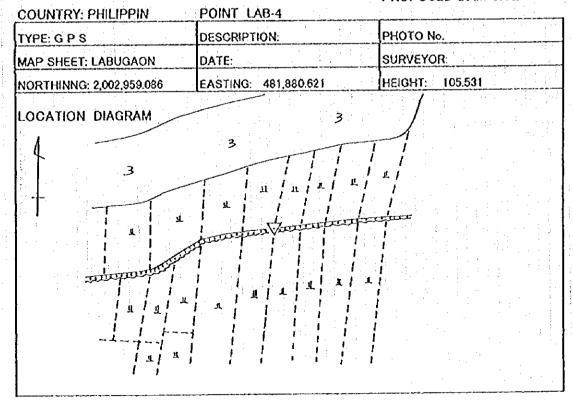
# SURVEY MARK DESCRIPTION



# PROPOSED DAM SITE

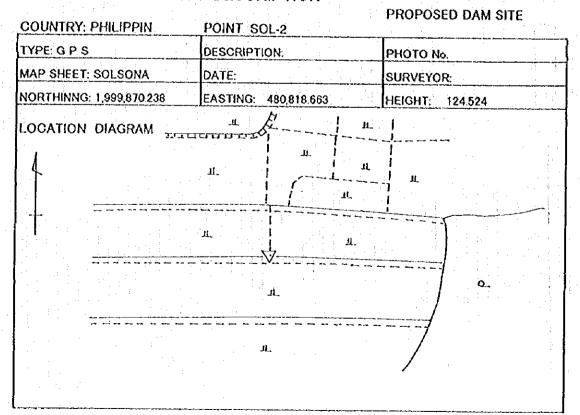
COUNTRY: PHILIPPIN	POINT LAB-3	
TYPE: G P S	DESCRIPTION:	PHOTO No.
MAP SHEET: LABUGAON	DATE:	SURVEYOR:
NORTHINNG: 2,002,778.460	EASTING: 483,300.845	HEIGHT: 123,941
LOCATION DIAGRAM		a

# SURVEY MARK DESCRIPTION



COUNTRY: PHILIPPIN	POINT SOL-1	PROPOSED DAM SITE
TYPE: G P S	DESCRIPTION:	PHOTO No.
MAP SHEET: SOLSONA	DATE:	SURVEYOR:
NORTHINNG: 2,001,022.382	EASTING: 479,779.891	HEIGHT: 103.627
LOCATION DIAGRAM	и и и о о о п и и о о о о о о о о о о о	

# SURVEY MARK DESCRIPTION



# PROPOSED DAM SITE

COUNTRY: PHILIPPIN	POINT SO	L-3	
TYPE: G P S	DESCRIPTIO	N: PH	OTO No.
MAP SHEET: SOLSONA	DATE:	SU:	RVEYOR:
NORTHINNG: 2,000,008.	564 EASTING: 4	180,233.257 HE	IGHT: 210.379
LOCATION DIAGRAM			

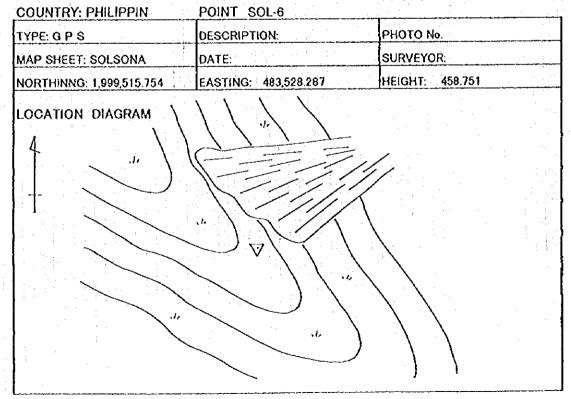
# SURVEY MARK DESCRIPTION

The state of the s		PROPOSED DAM SITE
COUNTRY: PHILIPPIN	POINT SOL-4	
TYPE: G P.S	DESCRIPTION:	PHOTO №.
MAP SHEET: SOLSONA	DATE:	SURVEYOR:
NORTHINNG: 2,000,168.308	EASTING: 479,757.757	HEIGHT: 104.556
LOCATION DIAGRAM		

### PROPOSED DAM SITE

COUNTRY: PHILIPPIN	POINT SOL-5	
TYPE: G P S	DESCRIPTION:	PHOTO No.
MAP SHEET: SOLSONA	DATE:	SURVEYOR:
NORTHINNG: 1,999,814.068	EASTING: 481,775.860	HEIGHT: 145.895
LOCATION DIAGRAM	0.	a .
П /		

# SURVEY MARK DESCRIPTION



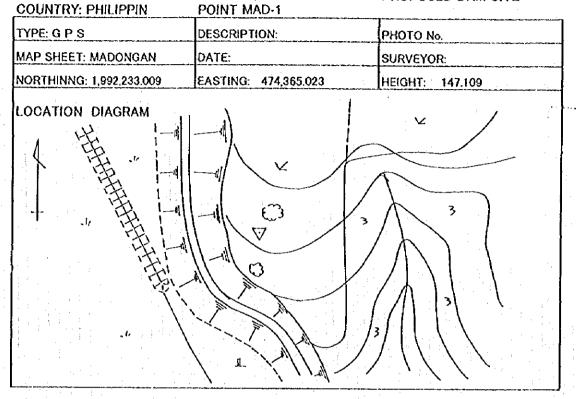
# PROPOSED DAM SITE

COUNTRY: PHILIPPIN	POINT SOL-7		·····		
TYPE: G P S	DESCRIPTION:		РНОТО N	O.	
MAP SHEET: SOLSONA	DATE:		SURVEYO	PR:	
NORTHINNG: 1,998,986.476	EASTING: 483,	019.975	HEIGHT:	159.977	
LOCATION DIAGRAM		•			
4				0.	
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		o			_
			<u> : : : : : : : : : : : : : : : : : :</u>	0.	
			1 7 y		

# SURVEY MARK DESCRIPTION

TYPE: G P S	DESCRIPTION:	PHOTO No.
MAP SHEET: SOLSONA	DATE:	SURVEYOR:
NORTHINNG: 1,999,662.643	EASTING: 481,695.141	HEIGHT: 141.378
LOCATION DIAGRAM	TIL.	
	11.	
<b>1L</b>	т.	4,
	Л.	a a

### PROPOSED DAM SITE



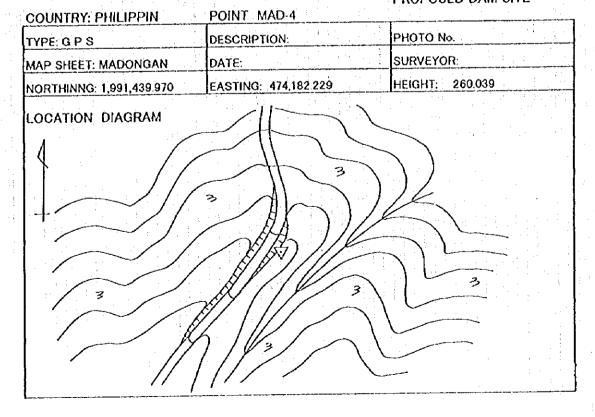
# SURVEY MARK DESCRIPTION

COUNTRY: PHILIPPIN	POINT MAD-2	
TYPE: G P S	DESCRIPTION.	PHOTO No.
MAP SHEET: MADONGAN	DATE:	SURVEYOR
NORTHINNG: 1,991,838.304	EASTING: 475,557.271	HEIGHT: 138.804
LOCATION DIAGRAM		1/1/1/10
	~ //// / I	(0.)
3		
+ 3		3
	3	
3		<b>3</b> /
	.,	3
	•	
L .		

### PROPOSED DAM SITE

COUNTRY: PHILIPPIN	POINT MAD-3	
TYPE: G P S	DESCRIPTION:	PHOTO No.
MAP SHEET: MADONGAN	DATE:	SURVEYOR:
NORTHINNG: 1,991,263.447	EASTING: 475,178.215	HEIGHT: 136.215
LOCATION DIAGRAM		
3	3	3

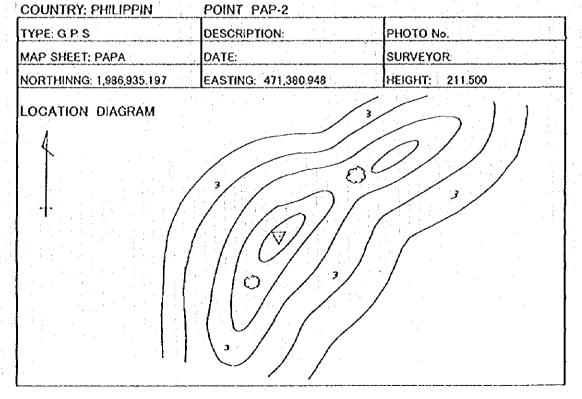
# SURVEY MARK DESCRIPTION



### PROPOSED DAM SITE

COUNTRY: PHILIPPIN	POINT PAP-1	
TYPE: G P S	DESCRIPTION:	PHOTO No.
MAP SHEET: PAPA	DATE:	SURVEYOR:
NORTHINNG: 1,987,302.760	EASTING: 469,804.660	HEIGHT: 109.907
LOCATION DIAGRAM		

# SURVEY MARK DESCRIPTION



### PROPOSED DAM SITE

PROPOSED DAM SITE

COUNTRY: PHILIPPIN	POINT PAP-3	
TYPE: G P S	DESCRIPTION:	PHOTO No.
MAP SHEET: PAPA	DATE:	SURVEYOR:
NORTHINNG: 1,986,505.496	EASTING: 471,469.911	HEIGHT: 141.112
LOCATION DIAGRAM	3 3	

# SURVEY MARK DESCRIPTION

# 

# 4. Coordinates Adjustment Data of Control Point

000RDS, LOG

FINAL RESULTS

COORDINATE ADJUSTMENT SUMMARY
NETWORK = cura
TIME = Won Dec 9 17:04:01 1996

Datum = CLARKE

Coordinate System = User-Defined Transverse Mercator

Zone = 51

Network Adjustment Constraints:

- . 2 fixed coordinates in y
- 2 fixed coordinates in X.
- 2 fixed coordinates in H

POINT	NAME	OLD COORDS	ADJUST	NEW COORDS	1, 00
1.0	URI				
, ,	Υ= ·	2007008.6587	+0.0000	2007008.6587	0.035806m
	X=	481137, 2855	+0,0000	481137, 2855	0.041415m
1 .	ELL HT=	163, 0594	10.0000	163,0594	<b>0.060840</b> m
	THO HT=	0,0000	+0,0000	0,0000	NOT KNOWN
2 0	UR2				.•
	Υ=	2007590, 1091	+0.0000	2007590, 1091	0.530685m
:	X=	482627. 8450	+0.0000	482627.8450	1.843351m
· . · ·	ELL HT=	272.5450	+0.0000	272, 5450	1. 1 <b>312</b> 38m
	THO HT=	0.0000	+0.0000	0.0000	NOT KNOWN
3.0	บคร				
	/οι.\υ Υ=	2006676, 9955	+0.0000	2006676.9955	0.044318m
	X=	482724, 0633	+0,0000	482724.0633	O. 048277m
4 L	ELL HI=	243. 2073	+0.0000	243, 2073	0.067597m
	ETH OHTS	0.0000	+0.0000	0,0000	NOT KNOWN
4 (	CUR4				
•	Y≂	2006232, 7167	+0.0000	2006232, 7167	0.031944m
	χ=	481316, 4410	+0.0000	481316, 4410	0. 036402m
	FIL HT=	103, 1836	.40,0000	103, 1836	0, 055321m .
01	RTHO HT=	0.0000	+0.0000	0.0000	NOT KNOWN
5 1	FC10				
	Υ=	2005007, 2170	+0.0000	2005007, 2170	FIXED
	X=	479041, 3950	+0,0000	479041, 3950	FIXED
	ELL HT=	74, 3810	+0,0000	74, 3810	FIXEO
3 1 1	RTHO HT=	0.0000	+0.0000	0,0000	NOT KNOWN
6 1	FC11				
· .	Y=	2000321. 7380	+0.0000	2000321.7380	FIXED
	χ=	480250. 2940	40.0000	480250. 2940	FIXED
1.7	ELL HT=	117, 1140	10,0000	117, 1140	FIXED
0:	RTHO HT=	0.0000	+0.0000	0.0000	NOT KNOWN

### coords. Log

()

0

COORDINATE ADJUSTMENT SUMMARY NETWORK = Labugaon TIME = Tue Dec 10 21:35:24 1996

Datum = CLARKE Coordinate System = User-Defined Transverse Mercator Zone = UTM51

Network Adjustment Constraints:

- 2 fixed coordinates in y
- 2 fixed coordinates in x
- 2 fixed coordinates in H

	NAME	OLD COORDS	ADJUST	NEW COORDS	1.00
POINT	NAME		1,0000		1
1 F	C10			0000007 0170	FIXED
	Y≂	2005007, 2170	+0.0000	2005007.2170	FIXED
	Х=	479041, 3950	+0, 0000	479041.3950	FIXED
111	ELL HT=	74, 3810	+0.0000	74. 3810	NOT KNOWN
OR	THO HT=	0.0000	+0,0000	0,0000	MUI ANUMI
9.1	LN3				
٤ ١	L113 Y=	2000321, 7380	+0,0000	2000321.7380	FIXED
*	X≍	480250. 2940	+0,0000	480250, 2940	FIXED
	ELL HT=	117, 1140	+0.0000	117, 1140	FIXED
	THO RT=	0,0000	+0.0000	0,0000	NOT KNOWN
3 l	AB1	0000000 0000	+0.0000	2003396, 6023	0.012648m
	Y=	2003396. 6023	+0.0000	482293.8858	0.014858m
	X=	482293.8858	+0.0000	110.3340	0. 022205m
	ELL HT=	110, 3340 0, 0000	+0,0000	0.0000	NOT KNOWN
. 08	тно нт=	0.000	10.0000		1 1 1
	100				1
4, 1	_A82 Y=	2003323, 8016	+0,0000	2003323, 8016	0.017521m
	X=		+0,0000	483291, 4660	0,019141m
	ELL HT=	270, 7373	+0.0000	270. 7373	O. 031399m
01	CLC III- RTHO HT=	0,0000	+0.0000	0,0000	NOT KNOWN
U	«ino mi⇒	0,000			
5 1	LAB3				
	Y=	2002778, 4596	+0.0000	2002778, 4596	0.017834m
	χ=	483300, 8454	+0.0000	433300. 8454	0.018756m
· · · · · · · · · · · · · · · · · · ·	ELL HT=	123, 9411	+0.0000	123. 9411	0.032352m
	=TH OHTS	0.0000	+0.0000	0.0000	NOT KNOWN
	LAB4				
: 0	L∧D4 Y≍	2002959, 0856	+0.0000	2002959.0856	0,011331m
	χ=	481880. 6214	+0.0000	481880. 6214	O. 012868m
	ELL HT=	105. 5308	+0.0000	105, 5308	0.021193m
^	RTHO HT=	0,0000	+0,0000	0.0000	NOT KNOWN
U	KINU NI-	J. 5500	<b></b>	∵ .	•

### COORDS, LÓG

COORDINATE ADJUSTMENT SUMMARY NETWORK = solsonal TIME = Fri Dec 13 18:05:20 1996

Datum = CLARKE Coordinate System = User-Defined Transverse Mercator Zone = selsenat

# Network Adjustment Constraints:

- 2 fixed coordinates in y
- 2 fixed coordinates in x
- 1 fixed coordinates in H

		-		A Para Caraca Ca	
POINT	NAME	OLD COORDS	ADJUST	NEW COORDS	1,00
, 1 F	111.		10.000	2000342, 0530	FIXED
	Υ=:	2000342, 0530	+0.0000	480151, 2910	FIXED
	χ=	480151, 2910	+0.0000	108, 2907	0. 148113m
	ELL HT=	108, 2907	+0.0000	0.0000	NOT KNOWN
08	eth ohts	0.0000	+0.0000	0.0000	(101 1010)
	•				
2.1	CII		0.000	2000321.7380	C3X13
	Υ=	2000321, 7380	+0,0000	480250, 2940	FIXED
	χ=	480250. 2940	+0,0000	117. 1140	FIXED
	ELL HT=	117, 1140	+0.0000	0.0000	NOT KNOWN
0	RTHO HT=	0.0000	+0.0000	0.000	101 1010111
	00) 4			1 - 1 1 - 1	* :
. 3	SOL1	2001022. 3822	+0,0000	2001022, 3822	0. 280178m
	. Υ= X=	479779, 8909	10,0000	479779.8909	0.333665m
		103, 6270	10,0000	103. 6270	0. 194126m
: 0	ELL HT= RTHO HT=	0,0000	+0.0000	0.0000	NOT KNOWN
. 0	Kino ni-	0.000	47870777		
	SOL 2				
4	301.2 Y≃	1999870, 2383	+0.0000	1999870, 2383	0.324710m
1 1 1	X=	480818, 6630	+0.0000	480818, 6630	0, 309458m
	ELL HT=	124, 5239	10,0000	124. 5239	0.090086m
	RIHO HI=	0,0000	+0.0000	0.0000	NOT KNOWN
	KINO III-				1 de 1
5	SOL4				A 100701
	γ=	2000168, 3076	+0.0000	2000168.3076	0, 188701m 0, 153104m
	χ=	479757, 7574	+0,0000	479757.7574	0, 153104m 0, 176100m
	ELL HT=	104, 5556	+0,0000	104, 5556	NOT KNOWN
(	RTHO HT=	0.0000	10,0000	0.0000	MOL VAOUR
6	SOL1			0001000 2510	<b>0.30333</b> 5m
	γ=	2001022, 3619	+0,0000	2001022, 3619 479779, 9548	0.352357m
. 1	X=	479779. 9548	+0,0000	103, 8784	0.097317m
*	ELL HT=	103. 8784	+0.0000	0,0000	NOT KNOWN
	ORTHO HT=	0,0000	+0.0000	0.0000	NOT ROOM
-	401.0				
7	SOL3	2000008, 5635	+0,0000	2000008, 5635	O. 118195m
	Y=	480233, 2570	40,0000	480233, 2570	0.151849m
	X=	210.3786	+0,0000	210, 3786	0, 137701m
	ELL HT=	0.0000	+0.0000	0.0000	NOT KNOWN
,	=TH OHTRO	0.000			

## coords, Log

COORDINATE ADJUSTMENT SUMMARY NETWORK = SOLSONA2 TIME = Fri Dec 13 17:54:18 1996

Datum = CLARKE Coordinate System = User-Defined Transverse Mercetor Zone = SOLSONA2

Network Adjustment Constraints:

1 fixed coordinates in y

I fixed coordinates in x

1 fixed coordinates in H

POINT NAME	OLD COORDS	ADJUST	NEW COORDS	1.00
1 FC11		•		
Y≃	2000321, 7390	+0.0000	2000321.7390	FIXED
Х=	480250, 2940	+0,0000	480250. 2940	FIXED
ELL HT=	117, 1140	+0.0000	117, 1140	FIXED
ORTHO HT=	0.0000	+0.0000	0,0000	NOT KNOWN
2 SOL5				
2 30L3 Y=	1999814, 0679	+0,0000	1999814.0679	0.005190m
X=	481775, 8599	+0.0000	481775. 8599	0. 006172m
ELL KI=	145. 8947	+0,0000	145. 8947	0. 016286m
ORTHO HT=	0.0000	+0.0000	0.0000	NOT KNOWN
3 SOL7				•
Y=	1998986, 4755	+0,0000	1998986. 4755	0.018340m
X=	483019, 9749	40,0000	483019.9749	0, 012063ct
ELL HT=	159.9769	10,0000	159. 9769	0. 027460m
ORTHO HI=	0,0000	+0.0000	0,0000	NOT KNOWN
4 SOL6 Y=	1999515, 7543	+0,0000	1999515. 7543	0, 006436m
X≂	483528, 2873	+0.0000	483528, 2873	O. 007350m
ELL HT=	458, 7510	+0,0000	458, 7510	0, 017798m
orino HI=	0.0000	+0.0000	0.0000	NOT KNOWN
OKING III-		. • [].		
5 S0L8				
Y≖	1999662.6431	+0.0000	1999662. 6431	0. 006168m
<b>X</b> =	481695, 1407	+0.0000	481695, 1407	0.007150m
ELL HT=	141. 3778	+0.0000	[41, 3778	0, 015104m
ORTHO HT=	0,0000	+0.0000	0.0000	NOT KNOWN
	and the second s		5	

### COORDS, LOG

COORDINATE ADJUSTMENT SUMMARY NETWORK = Madongan TIME = Fri Dec 20 10:28:19 1998

Datum = CLARKE Coordinate System = User-Defined Transverse Mercator Zone = Madongan

Network Adjustment Constraints:

1 fixed coordinates in y
1 fixed coordinates in x
Inner constraints in H

POINT NAME	OLD COORDS	ADJUST	NEW COORDS	1.00
1 LF13				
Y=	1991661.0790	+0.0000	1991661.0790	FIXED
χ=	474617, 0390	+0.0000	474617, 0390	FIXED
ELL HT=	177, 7531	+0,0000	177, 7531	<b>0</b> , 008560m
ORTHO HT=	0,0000	+0.0000	0,0000	NOT KNOWN
2 MADI				
Z MADI Y=	1992233, 0089	+0,0000	1992233, 0089	0, 000954m
X=	474365. 0226	+0.0000	474365, 0226	0.001175m
ELL RT=	193, 8793	+0,0000	193, 8793	0.008557m
ORTHO RT=	0.0000	+0,0000	0.0000	NOT KNOWN
Oltilo III			• •	
3 MAD2		文章 医生态电影		
Y=	1991838, 3037	+0,0000	1991838, 3037	0, 001134m
X=	475557, 2708	+0.0000	475557, 2706	0.001329m
ELL HT=	185, 9280	+0, 0000	185. 9280	O, 008306m
ORTHO HT=	0,0000	+0, 0000	0.0000	NOT KNOWN
	1947			
4 MAD3				0.000004
Y=	1991263.4466	10.0000	1991263. 4466	0.002884m
χ=	475178, 2145	+0,0000	475178, 2145	0.003508m
ELL HT=	183, 2757	40,0000	183, 2757	0. 011536m
ORTHO.HT=	0,0000	+0.0000	0.0000	NOT KNOWN
5 WAD4				
Y=	1991439, 9703	+0.0000	1991439, 9703	0. 011774m
X=	474182, 2285	+0.0000	474182. 2285	0.062340m
בנר או=	307, 0104	+0,0000	307. 0104	0.031068m
ORTHO HT=	0.0000	+0,0000	0.0000	NOT KNOWN

## COORDS, LOG

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COORDINATE ADJUSTMENT SUMMARY NETWORK = papa TIME = Fri Dec 20 10:34:04 1996

Datum = CLARKE Coordinate System = User-Defined Transverse Mercator Zone = papa

Network Adjustment Constraints: 1 fixed coordinates in y 1 fixed coordinates in x Inner constraints in H

POINT	NAME	OLD COORDS	ADJUST	NEW COORDS	1.00
1	LFCP-12	4 .		4040000 0500	FIXED
•	Y=	1986636, 9500	+0.0000	1986636, 9500	FIXED
	X=	471130, 0760	+0.0000	471130.0760	0.000179m
	ELL HT=	178. 1641	+0.0000	178. 1641	NOT KNOWN
	RTHO HT=	0.0000	+0,0000	0.0000	NOT KNOW
` `					
9	PAPI				Q. 000147m
	'~' Y=	1987128, 1616	+174. 5984	1987302, 7600	0.000147m
100	χ=	469945. 5284	-140. 8683	469804, 6602	0,000175m 0,000306m
100	ELL HT=	145, 1351	-0.0002	145, 1350	NOT KNOWN
	ORTHO HT=	0,0000	+0.0000	0,0000	MOI VUORU
	261110 111				
	PAP2		11.		0.000121m
	γ	1986760, 6300	+174, 5673	1986935, 1973	0.000121m 0.000094m
	X=	471521.8419	-140.8937	471380, 9482	0,000034m
	בנו אז≃	246.8986	+0.0001	246. 8987	
	ORTHO HT=	0.0000	+0.0000	0,0000	NOT KNOWN
	UKINO III-				
	PAP3				0. 000135m
•	Y≃	1986330, 9524	+174,5437	1986505, 4960	0.000104m
	X=	471610.8092	-140.8983	471469, 9109	0.000104m
	ELL HT=	176, 6698	+0,0003	176, 6702	NOT KNOWN
	ORTHO HT=	0,0000	ю, 0000	0.0000	KUI KNOMIT
. :	OKTIO III-	777			
	PAP4				0.000157m
	, γ Y=	1986176. 3844	+174.5476	1986350, 9320	0.000137m
	χ=	469940, 9699	-140.8752	469800, 0947	0,000333m
	ELL HT=	149, 8487	-0.0002	149.8485	NOT KNOWN
	ORTHO HT=	0.0000	10,0000	0.0000	MOT VHORIA
	ONTINO THE				•

# APPENDIX H RIVER MONITORING

#### APPENDIX H

### RIVER MONITORING

### Table of Contents

CHAPTER	I	GENERAL	H-1
CHAPTER	П	INSPECTION OF RIVER CONDITION	II-2
	2.1	Location	H-2
	2.2	Measures	H-2
CHAPTER	III	RIVER MORPHOLOGICAL SURVEY	H-3
	3.1	Location	H-3
	3.2	Measures	H-3
CHAPTER	ΪV	HYDROLOGICAL OBSERVATION	11-4
	4.1	Maintenance Works of Rainfall Gauge	H-4
	4.2	Maintenance Works of Stream Gauge	H-4
CHAPTER	v	ADMINISTRATIVE ORGANIZATION	Ĥ-6
	5.1	Required Staff and Period	H-6
	5.2	Organizational Arrangement	H-6

### List of Tables

Table H.2.1	Inspection Sheet of River Condition	H-7	
Table H.3.1	Fluctuation Record of Riverbed	H-8	
Table H.4.1	Inspection Sheet of Rainfall Gauge	H-9	
Table H.4.2	Daily Rainfall	H-10	
Table H.4.3	30 Minutes Rainfall	H-11	
Table H.4.4	Inspection Sheet of Water Level and Flow Velocity Gauges	H-13	
Table H.4.5	Daily Mean Water Level and Flow Velocity	H-16	
Table H.4.6	30 Minutes Mean Water Level and Flow Velocity	H-18	
Table H.5.1	Cost of River Monitoring	H-22	::.
	List of Figures		
	List of Figures		
Fig. H.5.1	Organizational Chart of District Engineering Office 1	1 1	
	Region 1, DPWH	H-23	

### CHAPTER I GENERAL

River monitoring before/during/after the construction of the proposed project is needed to verify the effect of the project and to find the necessity of maintenance works of sabo/flood control structures. Monitoring works are as follows:

(1) Inspection of River Condition

- To monitor the safety and functions of sabo/flood control structures such as sabo dams, embankment, revetments, groins and other related structures.
- (2) River Morphological Survey
  - To monitor the fluctuation of riverbed inspecting the safety of bank protection structures such as revenuent, toe protection and spur dike and verifying the flow capacity of each river.
- (3) Hydrological Observation
  - To observe rainfall, water level and flow velocity during flood time, in daily, monthly and yearly.

Location, measures and organizational arrangement for the above monitoring works are explained hereinafter.

### CHAPTER II INSPECTION OF RIVER CONDITION

### 2.1 Location

All the stretches to be improved in the proposed project for the Cura/Labugaon, Solsona, Madongan, Papa, Bongo and Laoag rivers should be inspected.

#### 2.2 Measures

This inspection should be made once a year in dry season and also after flood according to the decision of DPWH District Engineering Office. The contents of inspection are description of failure, cause of failure and applicable countermeasure. Inspection sheet is tabulated in Table II.2.1. It is desirable to attach an illustration of damage.

Failure of embankment is generally classified into weathering, surface erosion, saturation failure, depression and scouring. Damage of concrete structure and wet masonry structure is classified into crack, pothole, depression and free fall.

### CHAPTER III RIVER MORPHOLOGICAL SURVEY

#### 3.1 Location

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The Cura/Labugaon, Solsona, Madongan and Papa rivers in alluvial fan areas should be monitored because a riverbed aggradation or degradation may occur in alluvial fan areas. The survey in the Cura/Labugaon river will be conducted after the construction, because the present distributed river courses are planned to be integrated by the proposed project.

The survey at Cauplasan Bridge in Bongo River and at Gilbert Bridge in Laoag River should also be carried out to confirm the minor fluctuation of riverbed.

#### 3.2 Measures

Installation of maintenance marker post or kilometer post should be firstly carried out on the bank of river together with the elevation survey of the top of post. Cross-sectional survey will be made at the location of kilometer post at one (1) kilometer interval. This survey should be made once a year at the same locations in dry season. A totaling of 45 sections will be surveyed.

Based on the above survey, riverbed fluctuation volume including water level, mean riverbed elevation and water surface width are recorded as shown in Table H.3.1. This table also shows the design highwater level as a datum water level and the volume of riverbed excavation for construction material.

#### CHAPTER IV HYDROLOGICAL OBSERVATION

Hydrological observation such as rainfall, water level and flow velocity will be carried out using automatic recording gauge to observe during heavy rainfall and flood time. Location, configuration of equipment and details of available automatic hydrological observation stations are described in Supporting Report Volume III-1 (Master Plan Study). The rainfall stations are located at Piddig, Solsona and Nueva Era. The stream gauge (water level and flow velocity) stations are placed at Gilbert Bridge, Cauplasan Bridge and Solsona Dam.

Required maintenance works are described hereinafter while hydrological observation is carried out automatically.

### 4.1 Maintenance Works of Rainfall Gauge

Recording chart and recording pen need to be changed every month, and dry battery needs to be changed every three (3) months. Whenever recording chart is changed, zero point of the recorder should be adjusted and time line of the chart is adjusted to the actual time. Also date and time when the new chart is inserted should be recorded on the new chart for convenience in reading recorded rainfall data.

The cleaning of equipment and in/around stations should be carried out every month. Also, the accuracy check should be made before rainy season. The sheet for these inspection is presented in Table H.4.1.

The daily rainfall data will be recorded in whole year as shown in Table H.4.2. A rainfall data berfore/during/after flood caused by typhoon will be recorded every 30 minutes as tabulated in Table H.4.3. The 30 minutes rainfall data will be used to analyze the relation between the rainfall intensity and the occurrence or the discharge of flood.

### 4.2 Maintenance Works of Stream Gauge (Water Level and Flow Velocity)

Recording charts, IC memory cards and recording pens installed in the stations should be changed every month together with the cleaning work in/around stations. Battery and other related equipment should be inspected monthly and changed due to the conditions.

Whenever recording chart is changed, zero point of the recorder should be adjusted and time line of the chart is adjusted to the actual time. Also date and time when the new chart is inserted should be recorded on the new chart for convenience in reading recorded data. When an IC memory card is inserted into the IC card logger, formatting and creation of a data file are required to be made.

To create a data file for water level, necessary items to be input are file name, data length (number/times of data to be stored), base time to store data and time interval/cycle to store in/write to IC card. To create a data file for flow velocity data, necessary items to be input are file name, data length and time interval/cycle to store/write to IC card.

The inspection sheets for the stations are prepared for water level and flow velocity as shown in Table 11.4.4.

Water level and flow velocity data which are recorded in the IC memory card are retrieved using the software "M9711.EXE" provided on the computer. This program reads the data recorded in the IC memory card and makes text file.

The velocity data is directly recorded in the text file, but the water level data is recorded as the change of voltage in the IC memory card and therefore conversion of the voltage into the water level will be required using conversion factor 2.0.

A daily and monthly mean water level/flow velocity is recorded using the data sheet shown in Table H.4.5. Table H.4.6 tabulates the record format of water level and flow velocity before/during/after flood caused by typhoon every 30 minutes.

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### CHAPTER V ADMINISTRATIVE ORGANIZATION

#### 5.1 Required Staff and Period

To undertake the above activities, the following number of staff and period will be required:

Monitoring Items	Required Staff	Required Period
Inspection of River Condition	one civil engineer one or two assistants	one week in dry season annually *
River Morphological Survey	one civil engineer one survey expert	three weeks in dry season annually
Hydrological Observation	three assistants one civil engineer one assistant	two days monthly

()

#### 5.2 Organizational Arrangement

The monitoring works explained above will be carried out by the DPWH District Engineering Office-1 because it is technically and financially capable of undertaking of the work. The district office has presently 61 staff consisting 17 civil engineers, 1 architects, 18 technical staff and 25 administrative staff as shown in Fig. H.5.1.

Table H.5.1 shows the cost of river monitoring including inspection of river condition, river morphological survey, hydrological observation and flood forecasting and warning. The annual budget in 1996 for flood control in the district office is 14.5 million pesos.

In case the required maintenance activities would exceed the technical and financial capability of the District Engineering Office-1, the office will request the necessary technical and financial assistance from the central office of DPWH through Regional Office-1.

<sup>\*</sup> Inspection after flood will be undertaken according to the decision of the chief of the District Engineering Office.

## **TABLES**

Table H.2.1 Inspection Sheet of River Condition

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	1) Name of River 2) Date of Inspec 3) Date of Disast 4) Outline of Ins  Location (Section No.)	(2) Date of Inspection: (3) Date of Disaster: (4) Outline of Inspection Results:  Location (Section No.)  Description of Failure	Cause of Failure	Applicable Countermeasure	Remarks
·					

Approved by

Checked by

Prepared by

H - 7

Table H.3.1 Fluctuation Record of Riverbed

(1) Name of River:
(2) Date of Survey:
(3) Date of Previous Survey:

Station Width of (Kilometer Post) Water Surface	Width of Water Surface	Mean Riverbed Elevation (1)	Width of Water Surface in Previous Survey	Mean Riverbed Elevation in Previous Survey (2)	Depth of Fluctuation (3)=[(1)-(2)]	Distance between Station (4)	Volume of Fluctuation (5)=(3)x(4)	Volume of Riverbed Design Excavation for Highwater Construction Level Material	Design Highwater Level
									<u>-</u>
			The second secon						
Prepared by			Checked by			Approved by			

### Table H.4.1 Inspection Sheet of Rainfall Gauge

0

(1) Name of Station				
(2) Date of Inspection	on:			
(3) Recorder (Mont)	hly Check)			
Equipment	Portion	Maintenance	Problems & Measures	Results
Recorder	Exterior &	Cleaning		
	Interior	ént hanta-		
	Terminals	Tightening		<u> </u>
		:		
(4) Tipping Bucket	(Monthly Check	<b>3</b>		
Equipment	Portion	Maintenance	Problems & Measures	Results
	Exterior	Cleaning		
Tipping Bucket				
	Setting (Level)	<u>)                                    </u>		
	Terminals	Tightening		
(5) Accuracy Check	(before Rainy	Season)		
		i		
Number of Pulse <sup>1)</sup>	R	eading of Recorder2)	Result	
0			mm	
10			mm	
20			mm	
	s given by movinable error is with	ng the tipping bucket in }0.5mm.	by hand.	
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Prepared by		Checked by	- <b></b>	•
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Table H.4.2 Daily Rainfall

Station Year

	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec
1											. <u></u>	
2												
3												
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R.Day									1			<u> </u>			L	<u> </u>
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### Table H.4.4 (1/3) Inspection Sheet of Water Level and Flow Velocity Gauges

(1) Name of Station:	
(2) Date of Inspection:	<del></del> -

### (3) Equipment for Water Level Gauge

Equipment	Portion	Maintenance	Problems & Measures	Results
Rack		Cleaning		
Transformer	Exterior	Cleaning		
	Terminals	Tightening		
	Exterior	Cleaning		
Power Supply	<u> </u>			<u> </u>
	Terminals	Tightening		
	Indicator	·		· · · · · · · · · · · · · · · · · · ·
Battery	Exterior	Cleaning		
	Terminals	Tightening		
Convertor	Exterior	Cleaning		
	Terminals	Tightening		
•	Exterior	Cleaning		
Portable				
Recorder				<u></u>
	Terminals	Tightening		
	Recording			
	Chart Feeding			
Transmitter-	Exterior	Cleaning		
receiver				
	Terminals	Tightening		<u> </u>
	Setting			
Temperature	Exterior	Cleaning		
Sensor				
	Terminals	Tightening	<u> </u>	
	Setting			<u></u>

### Table II.4.4 (2/3) Inspection Sheet of Water Level and Flow Velocity Gauges

### (4) Equipment for Flow Velocity

Equipment	Portion	Maintenance	Problems & Measures	Results
	Exterior	Cleaning	<b> </b>	
Convertor				
	Interior	Cleaning		
	Terminals	Tightening		
Transformer	Exterior	Cleaning		
	Terminals	: · · · · ·		
Power Supply	Exterior	Cleaning	. '	. :
	Interior	Cleaning	:	
	Indicator			
Battery	Exterior	Cleaning		
	Terminals	Tightening		
Portable Recorder	Exterior	Cleaning		
	Terminals	Tightening		
	Recording			
	Chart Feeding			

### (5) Radio Wave Current Sensor

Equipment	Portion	Maintenance	Problems & Measures	Results
	Exterior	Cleaning		
Sensor				
	Terminals	Tightening		
	Angles	Adjustment		

## Table H.4.4 (3/3) Inspection Sheet of Water Level and Flow Velocity Gauges

### (6) Setting of Equipment

Item	Setting	Problem & Measures	Results
Number of Channels			
Range of Velocity	~ m/s		
Sampling Time	sec		
Times for Average			
Angles of Depression	Deg.		:
& Deviation	Deg.		
Output (Printing)	min.		
Interval	1 4	± 1	:
Supplementary	min.		
Output Interval			
Writing Interval to	min.		
IC Card			
Specified Velocity	m/s		

### (7) Power Supply Unit

Item	Criteria	Measured Voltage
	AC 90~110 V in AC Output Indicator	AC V
	DC 10.0~16.5 V in DC Output Indicator	DC V
	Confirmation of power supply when power off	

Prepared by	Checked by	Approved by

# Table H.4.5 (1/2) Daily Mean Water Level and Flow Velocity (Water Level)

										Station.		
										Year		
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# Table H.4.5 (2/2) Daily Mean Water Level and Flow Velocity (Flow Velocity)

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Table H.4.6 (4/4) 30 Minutes Mean Water Level and Flow Velocity (Flow Velocity)

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Table II.5.1 Cost of River Monitoring

### I. Monitoring/Inspection of River Condition

	Unit	Quantity	Unit Cost	Amount (peso)
(1) Initial Cost				0
(2) Annual Cost				28,880
Remuneration				
River/Civil Engineer	month	0.5	15,000	7,500
Assitant Engineer	month	0.5	10,000	5,000
Typist	month	0.5	7,500	3,750
Transportation/Car Service	month	0.5	20,000	10,000
Contingency(10% of above)				2,630

### II. River Morphological Survey/Observation

	Unit	Quantity	Unit Cost	Amount (peso)
(1) Initial Cost (every 10 years)				90,000
Kilometer post	post	45	2,000	90,000
(2) Annual Cost				68,920
Remuneration				
River/Civil Engineer	month	0.7	15,000	10,500
Survey Expert	month	0.7	12,000	8,400
Technical Assistant	month	1.4	10,000	14,000
Typist	month	0.7	7,500	5,250
Survey Equipment	month	0.7	15,000	10,500
Transportation/Car Service	month	0.7	20,000	14,000
Contingency (10% of above)				6,270

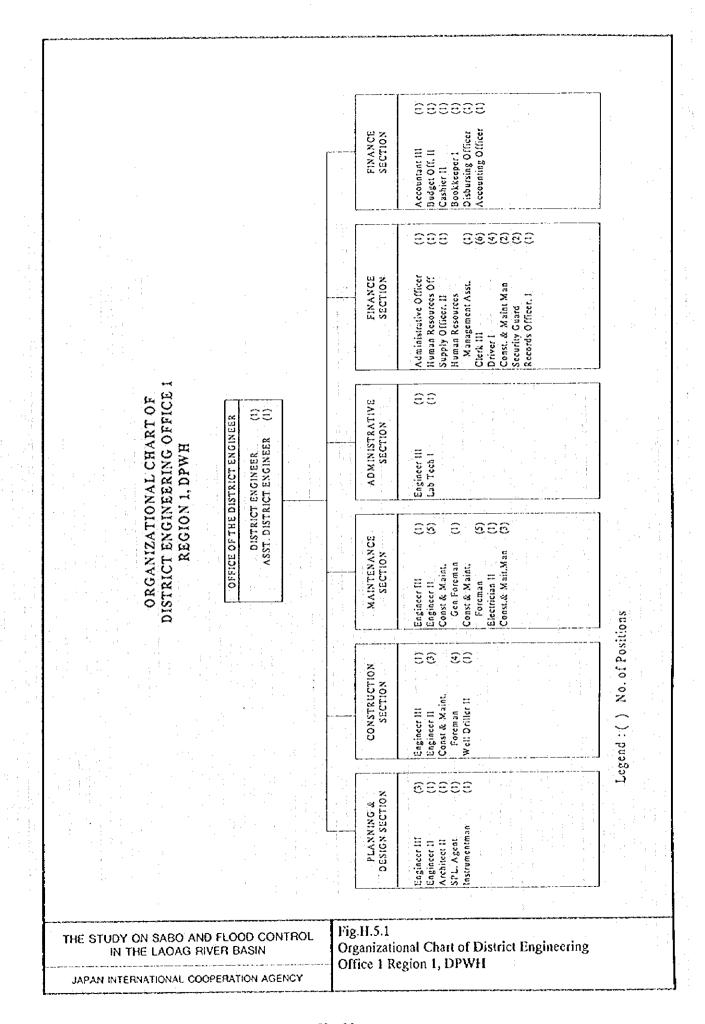
### III. Hydrological Observation

	Unit	M/M	Unit Cost	Amount (peso)
(1) Initial Cost (every 10 years) (2) Annual Cost				0 46,750
Remuneration River/Civil Engineer	month	1	.0 15,000	15,000
Typist Transportation/Car Service	month month	_	.0 7,500 .0 20,000	7,500 20,000
Contingency(12% of above)				4,250

### IV. Flood Forecasting and Warning

		Unit	Quant	ity	Unit Cost	Amount (peso)
(1)	Initial Cost (every 10 years)					368,000
11	Staff Gauge	site	:	6	8,000	48,000
	Handy Telephone	site		10	5,000	50,000
- 1	Gauge Keeper's House	site	:	9	30,000	270,000
(2)	Annual Cost					40,700
- :	Remuneration					
	Gauge Keeper	site		9	3,000	27,000
	Operation of Telephone	site		10	1,000	10,000
	Contingency(10% of above)				i	3,700

# **FIGURES**



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