# SUPPORTING REPORT

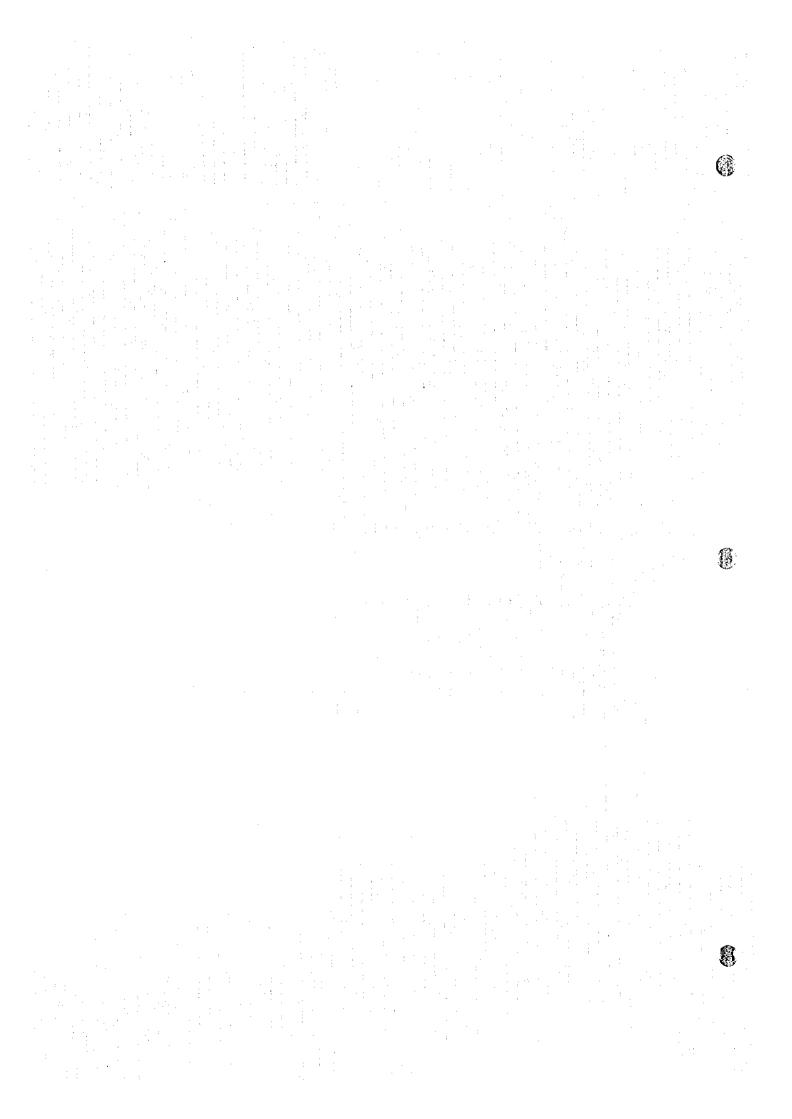
# PART-P

### **ECONOMIC EVALUATION**

# THE STUDY ON FLOOD CONTROL FOR AMBON AND PASAHARI AREA IN THE REPUBLIC OF INDONESIA SUPPORTING REPORT PART-P

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### CHAPTER 1 ECONOMIC EVALUATION

### 1.1 Yearly Average of Damage Alleviation

The following table shows the yearly averages of damage alleviation of Samal River and Kobi River at a 20 year return period, estimated by the Study Team:

Table-P.1.1 Yearly Averages of Damage Alleviation at a 20 Year-Return Period

Name of River	Yearly Average of Damage Alleviation
Samal River	Rp 1,657 million
Kobi River	Rp 768 million

Source: Study Team

### 1.2 Economic Analysis

### 1.2.1 Assumptions for Economic Analysis

Economic analysis was conducted under the following assumptions:

Price level
Design scale
End of December 1996
20 years return period

Project life 50 years

- Maintenance costs : 0.5% of the total construction costs per year

- Shadow pricing : Standard Conversion Rate 85%
- Growth rate of property value : Until 2010 : 6.0% per annum
From 2011 : 1.6% per annum

### (1) Price Level

The price level for the estimation of costs and benefits was set at the end of December 1996. The exchange rate for the Master Plan was fixed at Rp 2,500 to US\$ 1.00 for calculation purposes.

### (2) Design Scale

The design scale was set at 20-year return period, taking into account that damage alleviation is not expected to increase significantly when the design scale is raised to more than 20-year return period.

### (3) Project Life

The economic life of the project was set at 50 years; the residual value of the facilities is considered to be zero after 50 years when they will need to be replaced.

### (4) Maintenance Costs

The maintenance work is assumed to require 0.5% of total construction costs every year. The maintenance activities will be necessary from the year following completion of construction until the last year of the project life.

### (5) Shadow Pricing

Taxes and duties must be deducted from financial costs in order to obtain economic costs. 0.85 was used for the standard conversion rate.

### (6) Growth Rate of Property Value

The value of the houses in the Pasahari Area is expected to increase significantly in accordance with the increase in rice production resulting from the completion of construction of irrigation facilities. The residents' income in the area is expected to be doubled by 2010 and most of their houses, which are currently in poor condition, will be upgraded. The Study assumed a 6.0% increase per year in property value until 2010, while from 2011, a 1.5% increase per year is assumed to be achievable due to an increase in agriculture productivity and residents' opportunities to work in cities during agricultural off-seasons.

### 1.2.2 Economic Analysis and Sensitivity Analysis on Samal River

Table-P.1.2 shows the results of economic analysis on the construction of the flood control facilities in Samal River, on the assumption that the facilities are constructed in three stages. Since a 16.0% IRR will be achieved in this Project component, the construction of the flood control facilities in Samal river is judged to be economically feasible.

Table-P.1.2 Economic Cost, NPV, B/C and IRR of Samal River

Stage	Economic Cost	NPV at 10%	B/C at 10%	IRR
1 <sup>st</sup> Stage 2 <sup>nd</sup> Stage	Rp 5,688 million Rp 5,688 million	Rp 7,885 million	1.88	16.0%
3 <sup>rd</sup> Stage Total	Rp 5,688 million Ro 17,065 million			

Table-P.1.3 shows the results of sensitivity analysis under the assumption of follow:

1) Case-1: the growth rate in property value in the Study Area is 3 % per year until 2010.

2) Case-2: the construction cost increases by 10%.

The Project component is economically feasible in either case since their IRR are above 12%.

Table-P.1.3 Sensitivity Analysis - Samal River

NPV	at 10%	Internal Rate of Return (IRR)					
Case-1 3 % Increase in Property Value	Case-2 10% Increase in Cost	Case-1 3 % Increase in Property Value	Case-2 10% Increase in Cost				
Rp 2,449 million	Rp 6,985 million	12.1%	15.0%				

### 1.2.3 Economic Analysis and Sensitivity Analysis on Kobi River

Table-P.1.4 shows the results of economic analysis on the construction of the flood control facilities in Kobi River, under the same assumption as that of Samal River. Economic feasibility on the construction of Kobi River flood control facilities is marginal: its IRR is 8.2%.

Table-P.1.4 Economic Cost, NPV, B/C and IRR of Kobi River

Stage	Economic Cost	NPV at 10%	B/C at 10%	IRR
1 <sup>st</sup> Stage	Rp 6,287 million Rp 6,287 million	- Rp 2,122 million	0.79	8.2%
3 <sup>rd</sup> Stage Total	Rp 6,287 million Rp 18,862 million			

Table-P.1.5 shows the results of sensitivity analysis on Kobi River under the same assumption as those for Samal River.

Table-P.1.5 Sensitivity Analysis - Kobi River

NPV :	at 10%	Internal Rate o	f Return (IRR)
Case-1 3 % Increase in Property Value	Case-2 10% Increase in Cost	Case-1 3 % Increase in Property Value	Case-2 10% Increase in Cost
- Rp 4,641 million	- Rp 3,116 million	5.5%	7.5%

Under the assumption that property value increase by 3% per year, construction of flood control facilities in Kobi River is not economically justifiable. However, it should be noted that flood control facilities in Kobi River can be used as roads which have additional impact on the local economy, although its benefits are difficult to quantify due to lack of traffic data.

The future development plan of the Pasahari Area is currently not yet defined by the Government. The feasibility of the construction of Kobi River's flood control facilities cannot be judged at this moment since it is contingent on future development prospects of the area.



### Appendix P Economic Analysis

Economic Analysis: Samal River

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Samal		/					· .							_					
Growth			7-2010)		1.5		Grew!	٠.			0) 1033			Grow'	th.		7-2310		
c - c			(-2050)	1.015					120		0 1015	١.		_	•		-2050		
Fin Cost			20,077				Fin Co			20,073		*		Fin Q			22,085		
Eco-Cost			17.065				Eco-C			17065				Eoo (			18772		
Benefit/y		_	1.557			1.	Benefi	Uγ		1.85				Berel	Fa/y		1.657		
Year		Cost		Benefit	Net		Year		Cos	t	Penelik	t Net		Year		Cost		Benefit	: Nat
	0								0			0				0			
	2		1,138	. 0					<u> </u>	1.138		0 (1,138)			: :		1,251	q	
	3			-					2	1,138		0 (1,138)			3		1.251	0	
	4	. 1	1 138	0					3	1,138				100	3	•	1,251	Ç	
	5		1.138	0					4	1,138					4		1.251	0	
	8	:	1.138	933	(1.138) (233)	100	14 11		5	1,138							1.251	. 0	
	3								6	1.166		(445)			•		1.283	933	(350)
	· ģ	- 1	1,168	989	(177)				?	1.168					3		1.283	939	(294)
			1.156 1.158	1.043	(\$18)				8	1,166		(402)			٠ ٤		1,283	1.043	(234)
	10			1,111 1,118					9	1.155		(379)			9		1.283	1,111	(171)
	11		1.166	2.495	12 1303			: 10		1,168		(355)			10		1.283	1,178	(105)
	12	1.4	1.195	2 535	1,340			11.5		1,195		476			11		1,314	2.498	1,184
	13		1.135	2.573	1,340			11		1,195		501			12		1.314	2.535	1.221
		- 1					;	13		1,195		527			13	4 4	1.314	2.573	1 253
	. 14	1	1.195 1.195	2.612	1,417			. 14		1,195		553			14		1.314	2,612	1,298
	15	4 4		2.651	1.456			- 15		1,195		579	1 1		15		1,314	2,651	1,337
			55	4.036	3.951		1	16		85		2615			18		94	4,036	3 942
	17	100	85 85	4.098	4.011			1.		65		2,655			17		94	4.098	4,003
	19	100		4,158	4.073			18		65		2,596			18		94	4,156	4,064
	20		. 85 85	4.220	4.135			18		85	2,823	2.736			19		94	4,220	4,126
	21		85	4,348	4,198		:	20		85	2 8 5 6	2.790			20		94	4.284	4,190
					4.282			21		85	2,909	2 823			. 21		94	4.348	4,254
	5.5	1	85	4,413	4,328			22		65	2,952	2.887			22		84	4,413	4.319
	23		85	4.479	4,394			23		85	2,997	2811			23		24	4,473	4.385
	24		85	4.545	4,461			24		- 65	3.042	2 9 5 8			24		P4	4.548	4.452
	25		85	4,515	4,529			25		85	3,087	3,002			25		94	4.615	4.521
	28 27		85	4.884	4,598			28		85	3,134	3.049			26		34	4.684	4,590
			65	4,754	4,669			27		85	3,181	3,095			27		94	4,754	4,660
	28		85	4.825	4 740			28		85	3,225	3,143			28		94	4.825	4 731
	29		65	4.899	4,812			29		85	3,277	3,191		400	29		94	4.898	4.804
	30		85	4.971 :	4.888		100	30		- 85	3.326	3.240	:		30		94	4.971	4,877
	31		85	5.043	4.960			31		.85	3.376	3,290			31	1	84	5.046	4,952
1 to 1	35		85	5,121	5.036			32		85	3.426	3,341	1		32	:	94	5,121	5 028
	33 34		85	5.198	5.113			33		85	3,478	3.392		1.1	33		84	5,198	5,104
			55	5 2 76	5.191			34		85	3.530	3,445			34		94	5.276	5.182
	35		8.5	5,355	5,270			35		85	3,593	3,435		100	35		94	5,355	5.262
A 4 4 4 4	35		85	5,436	5.350			36		85	3,637	3,551			36		94	5,438	5,342
3	37		85	5517	5.432			37		. 85	3501	3,600			37		£4	5,517	5.423
1	38 . 39		85 85	5.600 5.684	5.515 5.539			38		85	3,747	3,661			38	:	94	5,600	5,506
								39		85	3,603	3,717			33		24	5.884	5.59Q
	40 41		85 85	5.769 5.858	5.684 5.770			40 41		85	3,860	3,774			40		64	5,769	5.675
	42		85	5.944			. ,			85	3,918	3,832			41		94	5,856	5,762
100	42		85	B 033	5,858 5,947			42		85	3,976	3,881			42		94	5.944	5 650
								4,3		85	4,036	3.951			43		94	8.033	5,939
1.0	44 45		65	6,123 - 5,215	6.038			44		85	4,097	4,011			44		94	6.123	6,029
					6,130					85	4.158	4.073			45		84	8215	5.128
	44	1	. 85	6,308	6.223			45		85	4.220	4 135			48		84	6.308	6.214
	47		65 - 65	6,403	6,318			47	· /	85 85	4.264	4,198			47		94	6.403	6.309
•				6.499	6.414			43			4.348	4.253			49		94	6.499	6.405
	43	:	85	6.597	6.511			49		85	4,413 4,473	4,328			49		94	8.597	6 503
	50		65	6 695	0.010			50		55	4,479	4.394			50		94	6.695	6 602
IDD.	•						00					1							
IF.R					18 04		RR					1214		RR					15 CN
NPV (101)					7.885		VPV (16					2.449		VPV (I					6,965
PV-Cost (1					9.000		V-C₀ st					9.000			# (101)				9,901
PV-Banafit	1101				16.835		V-8en		• '			11,443			sefat (10%	•			16,865
8/C (10%)					1 68	8	3/Ç (10	¥7				127	E	3/C (1)	75.				17(

### Economic Analysis: Kobi River

							:				
Δ	4100 2 0010		4.7	Kobi	(4603 5516)			Kobi			
Growth	(1997-2010)				(1997-2010)				[1997-2010		
Fin Cost	(2011-2050) 22 190	1015		Fin Cost	(2011-2050) 22.190	1015			(2011-2650	1015	
Eco-Cost	18.882			Eco-Cost	18,862		-	Fin Cost Eco-Cost	24,409 20,748		
Benefat/y	768			Benefit/v	758			Benefit/y	768		
Year	Cost	Benefit	Net		Cost	Saref4	Net		Cost	Beneft.	Net
(	,		0	0			0	0			. 0
- 1	1257	0	(1,257-	i	1,257	Ó	(1,257)	. 1	1,383	. 0	(1,393)
. 2	1257	0	(1257)	. 2	1,257	0	(1.257)	2	1,383	0	(1,383)
3	1257	. 0	(1,257)	. 3	1 257	0	(1,257)	3	1,383	. 0	(1,383)
4	1 257	0	(1257)	4	1.257	0	(1.257)	4	1.383	0	(1.383)
5 5	1 257	: 0	(1257) 1856)	5	1.257	0	(1,257)	5	1.383	0	(1.383)
7	1 289 1 286	433	830	6	1289	334	(955) (945)	6 :	1,415	433	(685)
8	1 289	483	(833)		1 289 1 289	244 354	(935)	7 8	1,418 1,418	458	(959
ě	1 289	515	(274)	9	1,789	- 365	(924)	9	1,418	486 515	(932) (903)
15	1,289	546	(743)	10	1 283	376	(913)	10	1,418	548	(872)
13	1,320	1.158	(163)	n	1.320	774	(546)	ŧi	1,452	1,158	(285)
12	1.320	1.175	(145)	12	1,320	786	(534)	12	1,452	1,175	(277)
13	1,320	1,193	(128)	13	1,320	798	(522)	13	1,452	1,133	(260)
14	1,320	1210	(110)	14	1,320	810	(510)	14	1.453		(242)
15	1,320	1,229	(92)	15	1,320	822	(498)	15	1,452	1229	(224)
16	84	1871	1,776	19	94 .	1.251	1,157	16	104	1 871	1,787
17	94	1.899	1 804	17	94	1.270	1,178	17	104	1,899	1,795
18	94	1.927	1,833	19	94	1,299	1.195	18	104	1.927	1823
18	94	1,958	1,862	. 19	94	1,309	1.214	19	104	1,956	1,852
20	94	1985	1.691	20	94	1,328	1 234	20	104	1,985	1,882
: 21	\$4	2 015	1 92 1	21	94	1.343	1.254	21	. 104	2,015	1911
2₹ 23	94 94	2,045 2,076	1,951	?2	94	1368	1.274	. 22	104	2.045	1.942
24	94 94	2.070	2013	23 24	94 94	1,410	1 295 (315	23	104	2.076	1,972
25	84	2.139	2 044	25	94	1,431	1,337	24 25	104	2.103	2.003
26	. 24	2.171	2 077	25	. 84	1,452	1,358	25 26	104	2.139 2.171	2.035
27	94	2,203	2,109	27	94	1,474	1,350	27	104	2,203	2.067 2.100
28	94	2 233	2.142	25	54	1,496	1,402	28	104	2,236	2.100
29	24	2 2 7 0	2 176	29	94	1.519	1.424	29	104	2.270	2165
30	24	2 304	2210	30	94	1,541	1,447	30	104	2.304	2 200
31	B4	2.339	2 244	31	£4	7.565	1,470	31	104	2,339	2 235
32	94	2 3 7 4	2 2 7 9	32	. 64	1,539	1,494	32	104	2,374	2270
33	94	2.409	2 315	33	94	1,612	1.518	33	104	2.408	2,308
34	94	2 445	2 351	34	9.4	1,636	1.542	34	134	2,445	2342
35 38	94 94	2.432 2.519	2 3 9 3	35	94	1 661	1,56\$	35	104	2.482	2,378
37	84 84	2 55 7	2.425 2.463	36 37	94 94	1 686	1,591	36	104	2,519	2,416
39	24	2 595	2,501	38	94	1.711 1.736	1.616 1.642	31	104	2 5 5 7	2,453
39	94	2634	2.540	39	94	1,763	1.668	38 39	104	2.596	2.492
40	94	2 5 7 4	2 580	40	94	1,789	1,695	40	104	2.634 2.874	2,531 2,570
41	94	2.714	2.820	41	94	1,818	1,721	11	;04	2314	2,510
42	5.4	2 755	2 660	42	94	1.843	1 743	42	104	2,755	2,651
43	84	2 798	2.702	43	84	1.071	1.776	43	104	2.798	2.692
44	84	2.638	2.744	44	84	1.099	1,804	44	104	2.838	2 734
45	94	2881	2.795	. 45	<b>§</b> 4	1927	1.833	45	104	2,681	2,777
46	9.4	2.924	2 630	43	94	1658	1.802	46	104	2,924	2 820
47	94	2969	2.873	47	9.4	1935	1.891	47	104	2,968	2864
43	94	3012	2818 .	48	94	20.5	1.921	49	104	3.012	2,908
49 50	24	3 057	2 953	49	94	2045	1.951	49	104	3,657	2,954
30	64	3 103	3 009	50	94	2 076	1.982	50	154	3,103	3,000
IRR			821	ira			5.5%	1RR			7.54
NEV (104)			(2.122)	NPV (ICK)			(4,641)	1KK NPV (104)			7.54
FV-Cod (10V			9 945	FV-Cost (164)			9 945	PV-Cost (101)			(3,116) 10,943
PV-Benef2 (10%	5		7.826	PV-Benefit (104)			5307	PV-Senefit (10%			7,628
B/C (10¥)			0.73	B/C (15%)			0.53	B/C (164)			0.72
											0 / 6

# SUPPORTING REPORT

# PART-Q

TOPOGRAPHIC SURVEY





# THE STUDY ON FLOOD CONTROL FOR AMBON AND PASAHARI AREA IN THE REPUBLIC OF INDONESIA SUPPORTING REPORT PART-Q

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### CHAPTER 1 GENERAL

### 1.1. Scope of Work

1. Monumentation 24 points for bench marks

2. Distance marks 498 points for two rivers and two tributaries

3. Leveling 118 km

4. Profiling 4 profiles for two rivers and two tributaries

5. Cross sectioning 63 sections for two rivers and two tributaries

1 sections for bridges

2 sections for staff gauges

6. Tidal observation 1 station

### 1.2. Existing Data and Information

Existing maps and data concerning the topographic survey found in the offices concerned were as follows:

Topographic maps

1:250,000 in Ambon and Pasahari areas prepared by joint operation US/UK in 1972

1:100,000 in Ambon and Pasahari areas prepared by US Army in 1944 and revised by

**BAKOSURTANAL** in 1977

Profile and Cross section maps

Profile and Cross section data in Kobi river in Pasahari area prepared by PU in 1996

Aerial Photographs

1:25,000 aerial photographs covering Pasahari area BAKOSURTANAL 1988

**GPS** Station

N1.5007 Wahai at Wahai Camat Office

Coordinates data and location description prepared by BAKOSURTANAL in 1995, The data and descriptions are hereto attached as Appendix Q.1.

### 1.3. Technical Specifications

Technical specifications for the survey were prepared by JICA engineer based on the JICA Standards for Survey and Mapping of Overseas Development Project. The original text in English was translated into Indonesian, so that it readily be understood by Indonesian surveyors.

The technical specifications in English are hereto attached as Appendix Q.2.

### 1.4. Members and Equipment

Members worked for the survey are as follows:

Ir Gatot Nugroho

Team leader

Rivanto

Co-Team leader for Pasahari area

Swadi

Surveyor for Pasahari area

Suyono

-ditto-

Tatang

-ditto-

Muchtar

-ditto-

Yahamanto Tiansyah

-ditto-

Johan

Operator for Automatic Plotter

Equipment used for the survey are as follows:

Theodolite T2

3 units

Theodolite TO

2 units

Total station

Lunis

**EDM** 

1 unit

Level

6 units

Computer (laptop)

1 unit

Plotter HP Design Jet type 1 unit

Other equipment such as staves and base plates, measuring tapes etc.

### 1.5. Datum Elevation for the Survey

Datum elevation for the survey should be mean sea level (MSL) as 0m. To know the MSL at Pasahari, a tidal observation station was installed and the tidal observation was carried out for 35 days at the station. The details of the tidal observation are discussed later.

# CHAPTER 2 FIELD OPERATIONS FOR THE CONCEPTUAL PLAN IN PASAHARI AREA

### 2.1. Field Reconnaissance

The JICA supervisor and the Indonesian counterpart together with surveyors from the Contractor carried out the field reconnaissance in Pasahari area. During the field reconnaissance, survey methods of profiling and cross sectioning were discussed and decided. Location of the tidal observation station was selected and established at the Kobi Sadar sea port.

Boats with and without motors should have been used for the transportation in rivers for the survey, rather than cars due to the very limited access roads.

### 2.2. Monumentation

### 2.2.1. Kobi and Tinupa

Twelve (12) bench marks (SR16-19, SR21-28) were monumented approximately 2km intervals along the Kobi and Tinupa rivers. Seven (7) monuments (BM1-6 and BM11) established by the regional office of Public Works were also used as the bench mark monuments in the downstream area of Kibi river. The location of the bench marks are shown in Figure-Q.2.1

Descriptions of the bench marks containing information such as location, access, date of establishment, elevation, were prepared for convenience to future users.

#### 2.2.2. Samal and Musi

Twelve (12) bench marks (SR1-3, SR5-13) were monumented approximately 2km intervals along the Samal and Musi rivers.

Existing bench mark monument BMS02 by the regional office of Public Works was used as SR4, so that SR-4 was not monumented. The location of the bench marks are shown in

Figure-Q.2.1

Descriptions of the bench marks containing information such as location, access, date of establishment, elevation, were prepared for convenience to future users.

### 2.3. Tidal Observation

The station was located at the joint of pier in Kobi Sadar Sea Port, approximately 7km from the Samal river and 13km from Kobi river to the west.

The observation started on 29 November, continued until 8 January 1997 with interruption on 29, 30 and 31 December by the stormy weather. A temporary bench mark was also located near the staff gauge of the station.

The levels of sea water surfaces were measured with a staff gauge fixed at the pier of the Sea Port at exactly every one hour, 24 hours a day for 35 day. The observation data were plotted to decide the highest and lowest tide of each day. The plotting are as shown in Figure-Q.2.2. The mean sea level (MSL) was calculated as follows:

Highest mean sea level

= Σ (highest sea level of each day) / Number of days

= 61.520/32 = 1.923m

Lowest mean sea level

= Σ (lowest sea level of each day) / Number of days

= 32.500/32 = 0.703m

Mean sea level

= (Highest mean sea level + Lowest mean sea level) / 2

= (1.923 + 0.703)/2 = 1.312m

There were two ebb and flow a day. The greater ebb and flow of each day were used to calculate the mean sea level. Graphics showing the daily tidal range are attached hereto as Figure-Q.2.2. The vertical distance between the mean sea level(MSL), staff gauge and the temporary bench mark(SR29) are as shown in Figure-Q.2.3. The elevation of SR29 from MSL at Kobi Sadar Sea Port was decided as H = 1.405m.

The highest, lowest and mean sea levels of each day were as shown in Table-Q.2.1

Table-Q.2.1 Daily Highest, Lowest and Mean sea level at Kobi Sadar Port

		Linhart	Time	Lowest	Time	Mean(m)
dat		Highest 1,900	19.00	0.660	12.00	1.280
Nov	30	1,900	19.00	0.720	12:00	1.310
Dec			19.00	0.850	13:00	1,300
ļ <u>.</u>	2	1.750	22.00	0.950	13:00	1,335
	3	1.720	21.00	1,010	13:00	1.425
		1.840 1.740	22:00	1.150	16:00	1.445
	. 5	1.790	23.00	1.010	5:00	1.400
	6	1.790	0.00	0.980	7:00	1.315
<del></del>	7	1.760	0.00	0.830	6:00	1.295
<u> </u>	<u>8</u>	1.810	14:00	0.640	8:00	1.225
<u> </u>		2,010	15:00	0.580	8:00	1.295
	$-\frac{10}{11}$	1,990	15:00	0.440	9:00	1.215
	11	2.060	16:00	0.230	10.00	1.145
	12	2.060	18:00	0.360	11:00	1.210
	13	2.140	18:00	0.380	11:00	1.260
	15	2.130	19.00	0.370	12.00	1.250
	16	1.980	19:00	0.630	13:00	1.305
	$\frac{10}{17}$	2.060	20,00	0.790	13:00	1.425
ļ	18	1.990	21:00	0.910	14:00	1.465
<b> </b>	19	1.920	21:00	0.940	3:00	1.430
	<del></del>	1.980	22:00	0.940	6:00	1.460
	$\frac{20}{21}$	1.930	0:00	0.840	6:00	1.385
	22	1,750	14:00	0.580	8;00	1.165
	23	1,870	15:00	0.570	8:00	1.220
}	24	1,950	16:00	0.600	10.00	1.275
<b> </b>	25	2,050	16.00	0,530	10:00	1.290
	26	2.090	17:00	0.420	10:00	1,225
}	27	2.020	17:00	0.480	10:00	1,250
<b> </b>	$\frac{27}{28}$	1.950	18:00	0.580	10:00	1.265
	29					- 1
1	30	1		•		•
<b> </b>	31		-		•	
Jan.		1.890	19,00	0.800	13.00	1.345
7(1(1.	2		20.00	0.800	3.00	1,370
1-	3	_1	21:00	0.900	3:00	1.400

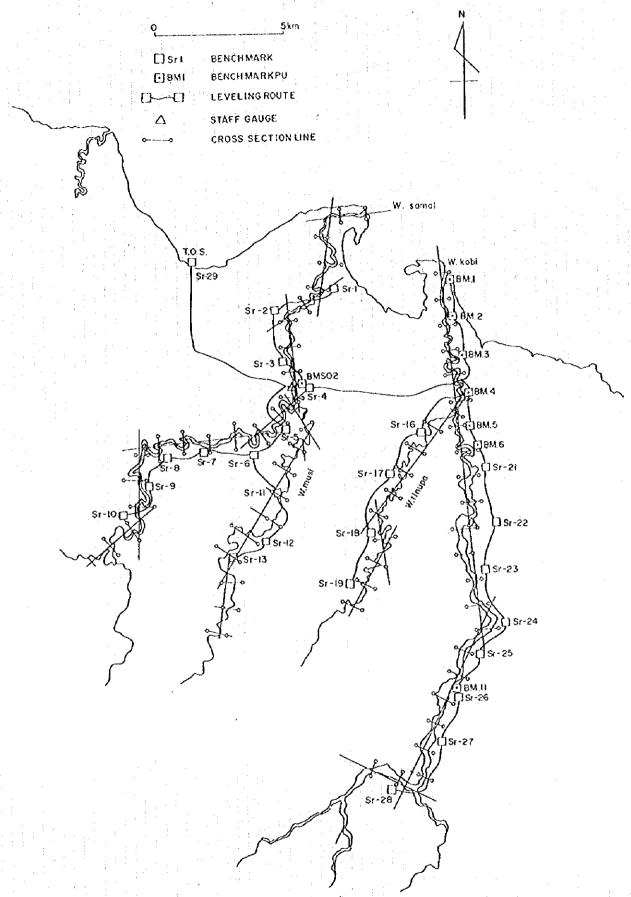


Figure-Q.2.1 Location of Bench Marks, Cross Sections, Tidal Station and Leveling Routes

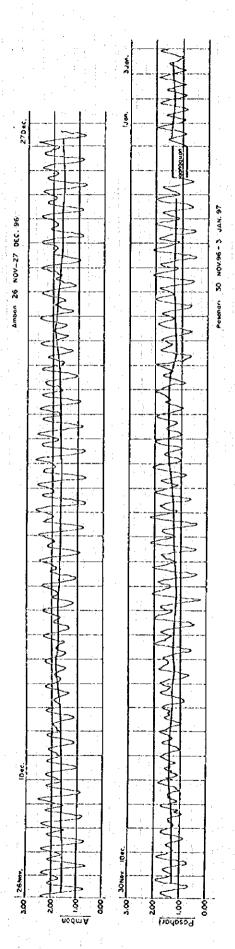


Figure-Q.2.2 Graph of Daily Tidal Ranges at Ambon and Kobi Sadar

6

LOXASI: X081 SADAR / PASAHARI.

1



1.00

MSL=1.313

2.00

### 2.4. Setting Out Distance Marks

Setting out of the distance marks for the rivers in Pasahari area was carried out not along the partial water flow of the rivers but along the overall directions of the rivers due to their meandering across the fields. The direction lines were staked out from the azimuth by a theodolite, and the distances were measured along the direction lines. Distance marks were set out perpendicular to the direction lines across the rivers. The direction lines and distance marks at every 1km are shown in Figure-Q.2.1.

### 2,4.1. Kobi and Tinupa

Distance marks at every 250m from the estuaries and confluence on the left and right banks of Kobi river and its tributary Tinupa river were set out and shown with concrete piles. The distance marks set out for the rivers were as follows:

Kobi River

87 points on the each bank, a total of 174 points

Tinupa Rivers

38 points on the each bank, a total of 76 points

### 2.4.2. Samal and Musi

Distance marks at every 250m from the estuaries and confluence on the left and right banks of Samal river and its tributary Musi river were set out and shown with concrete piles. The distance marks set out for the rivers were as follows:

Samal River

82 points on the each bank, a total of 164 points

Tinupa Rivers

42 points on the each bank, a total of 84 points

### 2.5. Leveling

The leveling to establish thirty three (33) bench marks, SR1-3, SR5-13, BM1-BM6, SR16-19, SR21-29 and BM11 was started from the temporary bench marks (SR29) at the tidal observation station, connected every bench marks and ended on the SR29 again. The total leveling distance was 119km and the location of bench marks and leveling routes are shown in Figure-Q.2.1. The measurement of double standing and double run were adopted, so that the accuracy of observation and measurement were confirmed. The misclosures of the each leveling route were as follows:

Table-Q.2.2 Accuracy of Leveling

Route	From	To	Distance(km)	Misclosure(mm)	mm √S
ī	SR29 - BMS02	BM4	19.9	27	6
2	BMS02	SR1	7.1	5	2
3	BMS02	SRIO	18.3	1	l
4	BMS02	SR13	14.0	8	2
5	BM4	BMI	9.1	35	12
6	BM4	SR19	15.0	41	11
7	BM4	SR28	35.4	27	6

The elevation of the bench marks obtained by the leveling are as follows:

Table-Q.2.3		Elevation of Bench Marks		
Code	Elevation(m)	Code	Elevation(m)	
SRI	2.291	SR2	5.286	
SR3	7.237	BMS02	7.248	
SR5	9.768	SR6	14.950	
SR7	21.871	SR8	27.34	
SR9	32.900	SR10	39.512	
SRII	18.646	SR12	26.351	
SR13	34.477	BM1	0.825	
BM2	1.575	BM3	2,790	
BM4	4.600	BM5	6.432	
BM6	8.252	BM8	13,329	
BM9	20.439	BM10	27.469	
BM11	39,177	SR16	8.039	
SR17	12.112	SR18	19.747	
SR19	24.874	SR21	9.296	
SR22	14.486	SR23	21.651	
SR24	29.529	SR25	38.310	
SR26	46.930	SR27	78.296	
SR28	71.349	BM11	39.177	

The elevations are from Mean Sea Level at Kobi Sadar Sea Port as Om.

### 2.6. Profiling

### 2.6.1. Kobi and Tinupa

To plot the rivers' longitudinal profiles, distance marks at every 250m intervals which was previously set out on the right and left banks of the rivers were measured its elevation by the leveling from the bench marks. Structures relevant to rivers such as bridges, small tributaries or drains etc. were also measured their elevation and distances. Elevation of the river beds at every 500m intervals were obtained from the measurements of the cross sectioning.

### 2.6.2. Samal and Musi

Profiling for the Samal and Musi rivers were similar to those of the Kobi and Tinupa rivers. Elevation of the river beds at every 500m intervals were also obtained from the measurements of the cross sectioning.

### 2.7. Cross Sectioning

### 2.7.1. Kobi and Tinupa

The cross sectioning of Kobi river and its tributary, Tinupa river at every 500m intervals from the estuary and confluence were carried out as follows:

River	Length (m)		No. of sections	
Kobi	22,000	44	+ one bridge + t	wo staff gauges
Tinupa	9,000	19		
Total	31,000	63	+ 3	

The distance marks setting out and cross sectioning at the upper most section(22+000) of Kobi river could not be carried out due to the local territorial problem.

One bridge across Kobi river and former staff gauge site at Seti Bakti village and new staff gauge site at Kobi Intake site were also measured their cross section.

#### 2.7.2. Samal and Musi

The cross sectioning of Samal river and its tributary, Musi river at every 500m intervals from the estuary and confluence were carried out as follows:

River	Length (m)	No. of sections	
Samal	20,000	41 + one bridge + two staff gauge	s
Musi	10,000	21	
Total	30,000	62 + 3	14.4

One bridge across Samal river and new staff gauge site at the bridge and former staff gauge site at the Samal Intake site were also measured their cross section.

### 2.8. Drawing Cross Section and Profile

### 2.8.1. Plotting and Drawing Cross Section

Data obtained by the field survey were processed with computers and stored in disk. The plotting of cross section was carried out with a computer program "The survey editor ver. 1.1., C. 1989, Tripangarso" directly by a computerized automatic plotter. Plotted manuscripts were edited and checked, and sometimes inspected in the fields. After the completion work, the cross sections were drawn on the polyester base #300. The drawing sheet was basically A1(50cmx70cm) size. Occasionally, however, larger sizes were used depending on the terrain features especially in the Pasahari area. The plotting and drawing scales were as follows:

Two rivers and two tributary in Pasahari	horizontal 1:1,000	vertical 1:1,000
Bridges across rivers in Pasahari	horizontal 1:1,000	vertical 1:1,000
Staff gauge sites in rivers in Pasahari	horizontal 1:1,000	vertical 1:1,000

### 2.8.2. Plotting and Drawing Profile

Data obtained by the field survey were processed with computers and stored in disk. The plotting of profile was carried out with a computer program "The survey editor ver. 1.1., C. 1989, Tripangarso" directly by a computerized automatic plotter. Plotted manuscripts were edited and checked, and completed. Elevation of the river beds were obtained from the cross section data. After the completion work, the profiles were drawn on the polyester base #300. The drawing sheet was basically A1(50cmx70cm) size. Occasionally, however, larger sizes were used depending on the terrain features especially in the Pasahari. The plotting and drawing scales were as follows:

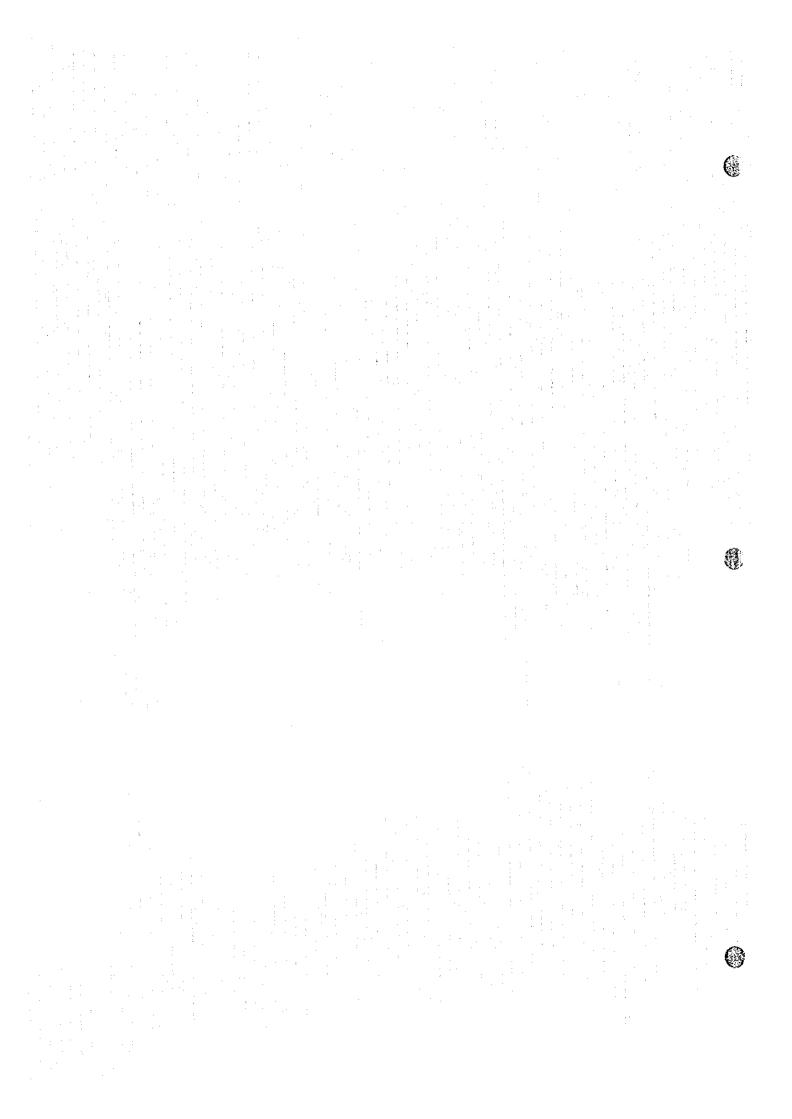
Five rivers and one tributary in Ambon horizontal 1:2,500 vertical 1:100

Two rivers and two tributary in Pasahari horizontal 1:10,000 vertical 1:1,000

### CHAPTER 3 PROGRESS

Progress of the topographic survey for the Conceptual Plan in Pasahari areas is shown in Table-Q.3.1.

November 28	December	January
1		
		South Market Market Street Control of the Market Street St
2829		
28	31	4 20
29		8
28	Control and Colorador (School by Section 1)	20
28	31	
		4 20
28	a maraninas e estada ionistras aestas ionis de estado e estado e estado e estado e e e e e e e e e e e e e e e	20
28	RCMA ADERVA ANALO MARIA MARIONA NA AMERICANA	20
British and a		20
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## **APPENDICES**



### Appendix Q.1. Existing GPS station; N15007



### BAUAN KOORDINASI SURVEY DAN PEMETAAN NASIONAL (BAKOSURTANAL)

Al. Baya Jakarin Bogor KM-16 - Ciblining, Telepon (BLI)M151654, Telex 48305 UAKOST IA . 1 Por. (024)8151064 - 8753067, FO HOX 46/CBI Cibling

### PUSAT PEMETAAN BIDANG SURVEY GEODESI SISTEM INFORMASI GEODESI

### JARING KONTROL HORISONTAL NASIONAL

Datum : WGS - 84

a: 6378137.000 m 1/t : 298.257223563 m

No.Stasian: N1.5007

Nama : Walial

Koordinat Geografi

Koordinat UTM

Lintang

: S 2° 47' 44.2453"

555167.944 Timur

Bujur

: E 129° 29' 46.8995"

Utara

9620285,468

Tinggi Ellipsoid :

64.0170 m

Zone

: 52 St : 0.9996400

Konv. Grid : ( 27.156"

- Koordinat Karteslan

X : -4851926.3750 m Y : 4916015.8680 m

Z : -302007,0020 m



### BADAN KOORDINASI SURVEY DAN PEMETAAN NASIONAL

### (BAKOSURTANAL)

### LAPORAN DESKRIPSI STASIUN G P S

STASION

N1.5007

BLRITHOMORPHAR

D3. DESA/KAMPUNG 65. KABUPATEN/KOTAMADYA N1.5007

: Alr Buaya : Maluku Tengah

04. RECAMATAN

, Air Buaya t Buru Ulara Barat

06 PROPINSI

02. NANTA

, Maluku

07, PENCAMATAN OLEIT

08. RECEIVER

09. WAKTU

. Geodesi Bakosurlanal Ashtoch LM XII 3 1 08:00 - 21:00 UTC

i 21 Juli 1995

11. KETERANGAN PILAR

10. TANGGAL / JULIAN DAY

Standar pilar GDS

ROORDINAT PENDERATAN

12. LINEANG

83511"

13. BVJUR

1 E 126 26' 5"

14. TINGGI (distas ellisaid)

i 71.9362 m

15. DRAFAN LORASI STATIUN

: Pilar terletak di halaman depan Kentor Camat Buru Ulara Barat di Air Buaya

16 KENAMPAKAN YANG MENUNJUL : Kantor Carbat Buru Ulara Baral (Ali Buaya)

17. JALAN KE LOKASI

ı Dari pelabuhan Slamet Riyadi di Ambon nalk kapat motor 12 jam ke Namlea atau dari pelabuhan Ferry Galala di Ambon nelk Ferry 10 jam ke Namies. Lalu dari pasar Lama di Namiea naik bis ke Ali Buaya 5 jam (96 km).

18. TRANSPORTASI/AROMODASI

: Dengan kapal motor ferry dan bis. Akomodasi diperoleh dari Namisa.

19. DIRUMT OURH

Heru O, & Abun P.

10. TANGGAL : 07/21/95

21. DIPERIKSA OLEH

1 C. Subarya M.Surv.Sc.

then: Hota

10/11/96

DADAN KOOKOMASI SURYEY DAN PEMETAAN NASIONAL : N 15007 No. Stasion (UAKOSURTANAL) 2713 Lembar Pela DESKRIPSI STASIUN OPS SKETSA 27. Sketsa Umum Lokasi. P. SERAM Huishnigo 28. Sketsa Detail Lakasi 7 A M DESA : WAHAI KECAHATAH SERAH UTARA BUKIT / HUTAN ikii Bake-GD

1

### Appendix Q.2. Technical specifications

### D. Technical Specifications

for

The Study on Flood Control for Ambon and Pasahari Areas in the Republic of Indonesia

### 1. Survey Areas

The survey areas covered by the Contract shall consist of the Ambon Central Area including five rivers and the Pasahari Area including two rivers in Seram Island. The location of each area is shown in Figure 1.

### 2. Scope of Work

### (1) Work Items

The Work consists of the following items. The work quantities of each item are stipulated in the Bill of Quantities.

- Mobilization
- Monumentation
- Distance Marks Setting
- Leveling
- Profiling
- Cross Sectioning
- Plotting
- Drawing
- Reporting

### (2) General Specifications

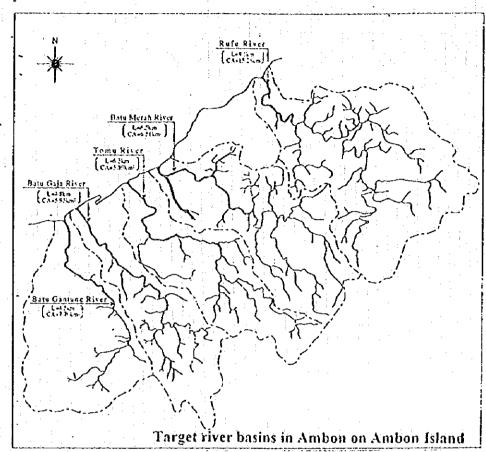
### (2-1) JICA Standard

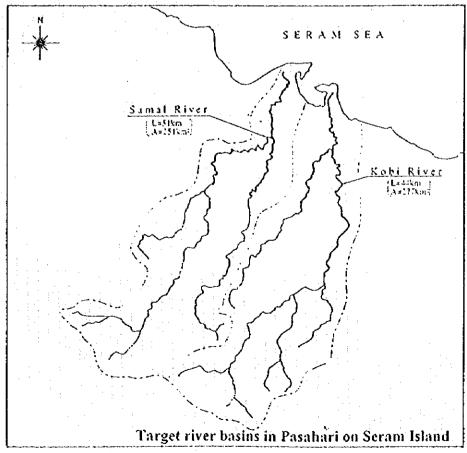
For the accuracy of the survey, "Specifications of Geodetic and Photogrammetric Surveying for Overseas (JICA) shall be applied with the instructions by the Engineer unless otherwise specified.

### (2-2) Datum Elevation

The datum elevation shall be mean sea level(MSL) at Ambon sea port and Pasahari in Seram Island as 0m. This datum elevation shall connect the existing bench marks.

Figure 1 Survey Area for Profiling and Cross Sectioning





### (2-3) Standard of Drawing

Style of drawing sheets, marginal information, legend and symbols will basically follow those of the drawings previously prepared for the projects of the Ministry of Public Work. Special styles may be prepared for the Study, if any.

#### 3. Methods of the Work

### (1) Mobilization

Mobilization shall include the followings:

- a) Preparation of materials, equipment and laborers including supervising personnel of the Contractor
- b) Transportation of materials, equipment and taborers including supervising personnel
  - of the Contractor
  - c) Accommodation for laborers and supervising personnel of the Contractor
  - d) Transportation of the Engineer of the JICA Study Team for the supervision of the Work

### (2) Monumentation

Forty (40) monuments of bench marks shall be marked with concrete piles. The size and features of the concrete piles are shown in Figure 2. The location of the monumentation shall be approximately 2 km intervals along the left banks of rivers selected on the existing maps and confirmed in the fields. The bench mark monuments can be used as distance marks.

### (3) Distance Marks Setting

### (3-1) Location

劉;

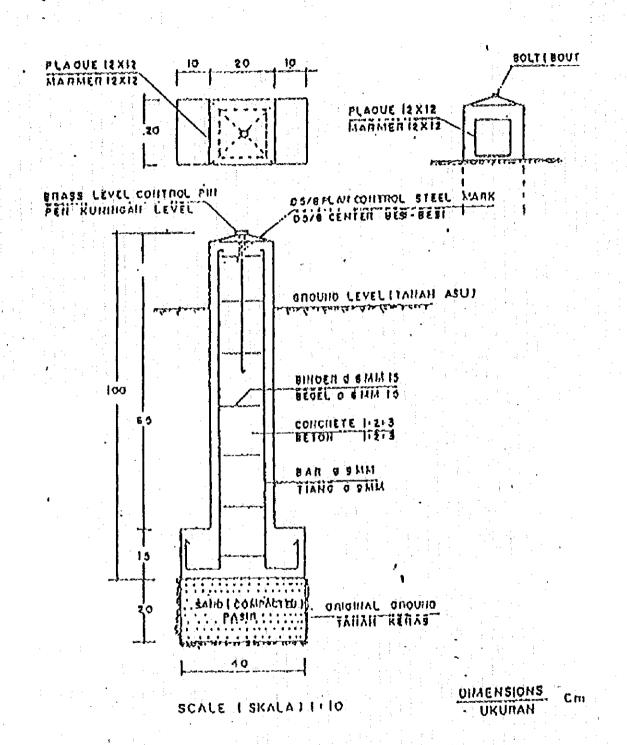
12

Distance marks with every 100m intervals along the rivers in Ambon area, and with 250m intervals along the rivers in Pasahari area shall be set out from their estuary or confluence on the right and left banks.

### (3-2) Setting

Distance and angle measurement to set out the distance marks shall be carried out on the left banks of the rivers. The marks on the right banks will be staked out from the established marks on the left banks perpendicular to direction of water flow. These points shall be marked with concrete piles east in plastic pipes 10cm diameter and 80cm long.

Figure 2 Specification of Bench Mark



### (3-3) Equipment

Equipment to be used for the setting out shall be electro-optical distance meter (EDM) and/or measuring tapes and theodolite. Capacity of the equipment shall be as follows:

- EDM 1km measuring capacity

10mm ± 5ppm x D D: measuring distance

-Theodolite 20 direct reading

### (3-4) Accuracy

19

The accuracy of the setting out shall be within 1:2,000 of the measuring distance.

### (4) Leveling

### (4-1) Routes and Reference Points

Direct leveling shall be carried out to determine the elevation of the profiling and cross sectioning. Datum elevation for the leveling shall be derived from the existing MSL bench marks in and around the Study Areas. The routes of the leveling shall be loops along the right and left banks of the rivers. In case, there are no existing MSL bench marks in and around the Study areas, tidal observation for over one(1) month shall be carried out to provide the datum elevation for the leveling. The specifications of the tidal observation will be shown by the Engineer.

### (4-2) Measurement

The direct leveling shall be started at and ended on the existing MSL bench marks in and around the Study areas. Double run or double stand methods shall be employed to check the accuracy of the observation. Every leveling route shall form a loop to check the accuracy of the leveling. Lines of sights shall not exceed 60m and length of back and fore sights shall be equalized.

### (4-3) Equipment

Equipment to be used for the direct leveling shall be:

- Automatic levels : 40 second / 2mm second order level
- Metric staves : 3 or 5 m wooden or metal staff with base plates.

Before starting the observation, the equipment shall be tested and adjusted.

### (4-4) Accuracy

Accuracy of direct leveling shall be within 30mm  $\sqrt{S}$  misclosure in double run or in loop. Where, S is the length of leveling in kilometer.

### (5) Profiling

#### (5-1) Profiling routes

Profiling routes shall be left and right banks of the rivers. Elevation of every distance mark previously set out and marked, its ground height and existing river structures shall be measured and recorded.

### (5-2) Equipment

Equipment to be used for the profiling shall be equivalent to those used for the leveling.

### (5-3) Accuracy

Accuracy of the profiling shall be equivalent to those stipulated for the leveling.

### (6) Cross Sectioning

### (6-1) Location

The cross sectioning shall be carried out every 50m intervals along the rivers in the Ambon area, and 500m intervals in the Pasahari area including the sounding of the rivers. The cross sectioning of the existing bridges shall also be carried out in the Ambon area. The areas and lines of the cross section will be shown by the Engineer to the Contractor. Survey length and number of section are shown in Table-1.

### (6-2) Cross Sectioning and Sounding

Cross section lines shall be staked out from the distance marks on the left and right banks. Cross section lines in the proposed dam sites will be shown by the Engineer. Ground height every 2m intervals in the Ambon area and 10m in the Pasahari area, and points where slopes abruptly change on the cross section lines shall be measured and recorded.

Elevation of water surface of the rivers at the measuring time shall be measured and recorded. Sounding shall be carried out every 2m intervals on the cross section lines. The cross sectioning shall be carried out 50m from the shore lines both the left and right banks. However, in city areas, cross section lines may be stopped at houses' walls, regardless of the stipulation above.

#### (6-3) Equipment

Equipment to be used for the cross sectioning shall be equivalent to those of the leveling. Equipment to be used for the sounding will be sounding rods and measuring tapes.

### (6-4) Accuracy

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Accuracy of the cross sectioning on the ground shall be within 5cm and that of sounding in the rivers be within 20cm in elevation, and 1: 300 in distance.

### (7) Plotting

### (7-1) Plotting Profiles

Elevation and distance data acquired in the fields shall be plotted at scales of 1:5,000 or 1:10,000 for the length and 1:100 or 1:200 for the elevation depending on the features of the rivers. Scales of the plotting will be decided by the Engineer before the commencement of the plotting.

The profiles shall be plotted from estuary or confluence at the left, towards the upper stream to the right on the sheets. Profiling data and information such as station No., distance, accumulated distance, planned river bed height, planned high water, planned bank heights etc. shall be shown on the sheets. The sample sheet of the profiling will be shown by the Engineer before the commencement of the plotting. The plotting points will be digitized and stored in floppy disks to be submitted to the Engineer.

### (7-2) Plotting Cross Section

Elevation and distance data acquired in the fields shall be plotted at scales of 1:100 or 1:200 depending on features of the rivers. The scales of the cross section shall basically be equivalent to the vertical scale of the profile. The scales of the cross section and sample sheet will be shown by the Engineer before commencement of the plotting. The plotted points will be digitized and stored in floppy disks to be submitted to the Engineer.

### (7-3) Materials

Materials to be used for the profile and cross section plotting sheets shall be polyester base with thickness of #300.

### (8) Drawing

### (8-1) Drawing Profiles

Plotted profiles manuscripts shall be traced with black ink on the polyester bases to prepare original profiles. Marginal information such as scales, titles sheet number etc. shall also be drawn on the sheets. The drawing may be carried out directly from the digitized data with a computerized automatic plotter.

### (8-2) Drawing Cross Sections

Plotted cross sections manuscripts shall be traced with black ink on the polyester bases to prepare original cross sections. Marginal information such as scales, titles, sheet number etc. shall also be drawn on the sheets. The drawing may be carried out directly from the digitized data with a computerized automatic plotter.

### (8-3) Materials

Materials to be used for the profile and cross section plotting sheets shall be the polyester base with thickness of #300.

### (9) Report

The Contractor shall submit to the Engineer the following final survey results:

1) Data and results of the Distance Marks setting out	1 set
2) Data and results of the Leveling	1 set
3) Data and results of the Profiling including the floppy dis	sks Iset
4) Data and results of the Cross Sectioning including the	
floppy disks	1 set
5) Original Drawing of the Profiling	1 set
6) Original Drawing of the Cross Sectioning	1 set
7) Blue prints of the Drawing o the Profiling	5 sets
8) Blue prints of the Drawing of the Cross Sectioning	5 sets
9) Survey Report	5 sets

Table-1. Cross Section Survey

Area	River	Length(m)	No. of Section
Ambon City	Rufu	1,600	32
	Batu Merah	2,600	52
	Main river	1,600	32
	Tributary	1,000	20
	Tomu	2,900	58
	Batu Gaja	2,900	58
	Batu Gantung	2,000	40
	Total	12,000	240
Pasahari	Samai	19,000	38
(Seram Island)	Musi(tri.)	9,000	18
	Kobi	22,000	55
	Chinupa (tri.)	7,000	14
	Total	57,000	114
Ambon Bridges			15
Dam sites	Rubu		2
	Batu Merah		3
	Tomu		3
	Batu Gaja		3
·	Batu Gantung		2
	Total		13

Cross Sectioning: 50m intervals in Ambon

500m intervals in Pasahari

### Specifications on the Tidal Observation

#### 1. Location

The tidal observation station shall be established one in Ambon sea port and one in Pasahari. The stations shall be located at points near the survey areas, free from disturbance by winds, waves, current and water from rivers.

### 2. Duration

Duration of the observation shall be 35days from the middle of November, 1996.

### 3. Equipment

Equipment to be used for the tidal observation shall be staffs and 2nd order levels.

#### 4. Methods

A staff shall be fixed firmly and perpendicularly in the sea water. A bench mark shall be monumented at a place within 50 to 100m from the station. The vertical distance between the 0m 00 point of the fixed staff and the monumented bench mark shall be measured and recorded. (refer to Fig. I.)

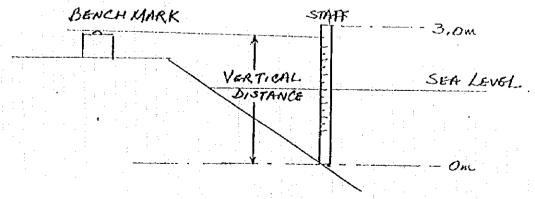
Observation and reading of the elevation of water surface with the fixed staff shall be carried out every one(1) hour, 24hours a day for consecutive 35days.

### 5. Data processing

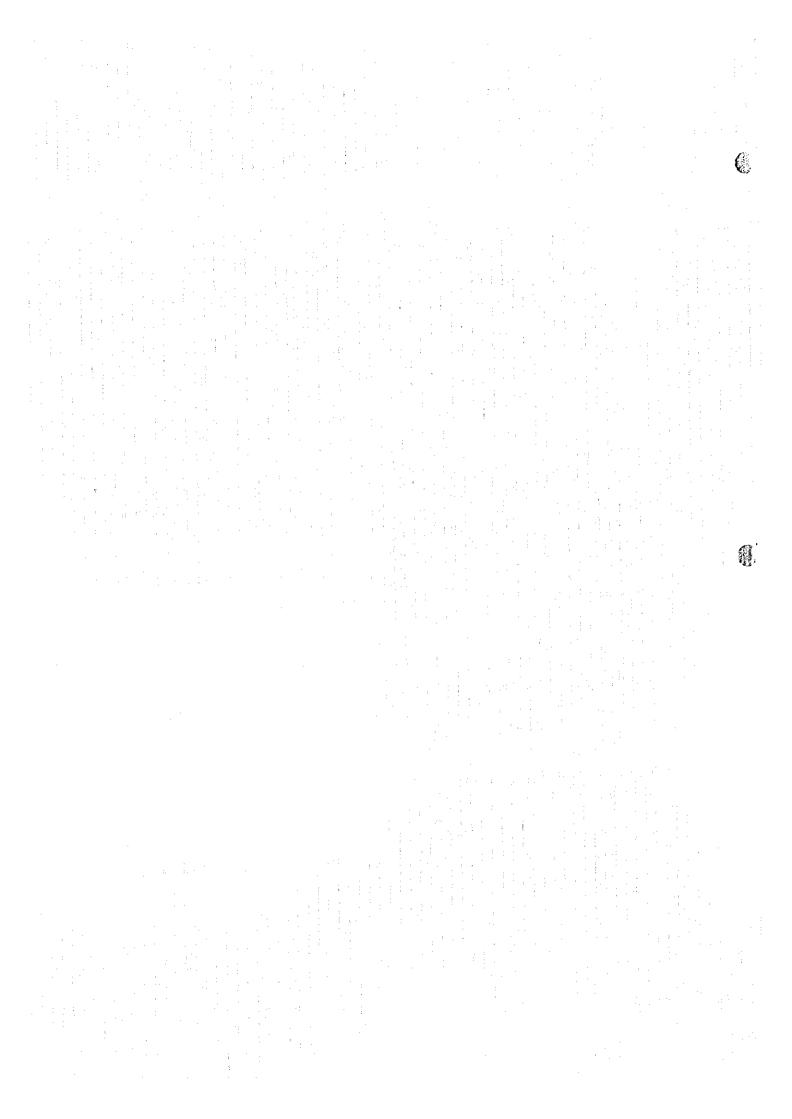
Acquired data shall be processed to obtain MSL as follows:

- 1. Mean highest sea level (MHL) = Σ highest sea level of each day / number of days
- 2. Mean lowest sea level(MLL) = Σ lowest sea level of each day / number of days
- 3. Mean sea level (MSL) = (MHL + MLL)/2.

Fig. 1. Vertical distance between bench mark and tidal observation station



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