

社会開発調査部報告書

SULTANATE OF OMAN MINISTRY OF COMMUNICATIONS DIRECTORATE GENERAL OF ROADS



CONSTRUCTION OF FLYOVER AT NASEEM GARDEN ROUNDABOUT BATINAH HIGHWAY

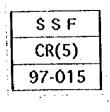
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DRAWINGS



PACIFIC CONSULTANTS INTERNATIONAL FUKUYAMA CONSULTANTS INTERNATIONAL

MARCH, 1997



DRAWING SCHEDULE

(FO1-R/A2 A'NASEEM GARDEN)

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SHEET NO.	TITLE	Sheet no.	TITLE	SHEET NO.	TITLE
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-				W-42	Re-bar Arrangement (18)
				W-43	Re-bar Arrangement (19)
				W-44	Re-bar Arrangement (20)
				Т	TEMPORARY WORKS
				T-1	Construction Sequence
				T-2	Detour Layout (1/2)
				T-3	Detour Layout (2/2)
			JAPAN INTERNATIONAL COOPERATION AGENCY	CLIENT : MINISTR	Y OF COMMUNICATIONS, DIRECTORATE GENERAL OF ROADS
			(JICA)		ROAD DEVELOPMENT PROJECT ON BATINAH HIGHWAY
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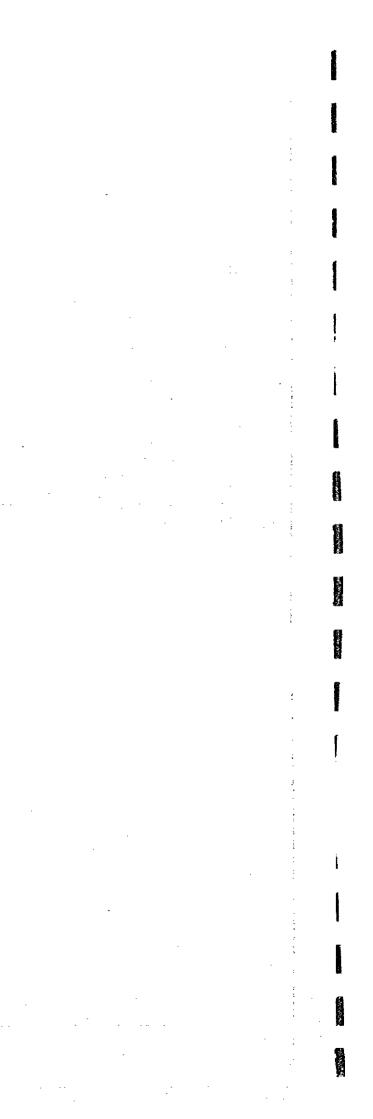
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LOADING SPECIFICATIONS

The loading specifications used for the design of structures are as follows:

- HIGHWAY DESIGN MANUAL, February 1994, Sultanate of Oman - STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES.
- 1990, Ameriacn Association of State Highway and Transportation Officials

- SPECIFICATIONSFOR HIGHWAYBRIDGES,

February 1994, Japan Road Association

According to the above specifications, basic design condition are as follows:

1. CLASSIFICATION OF LIVE LOAD

- Special truck type A (Oman)
- Special truck type B (Oman)

- HS20-44 increased 100% (AASHTO)

- TL-25 (Japan)

2. SEISMIC LOAD

0.1g of acceleration coefficient for seismic loads is applied in accordance with the Highway Design Manual in the Sultanate of Oman.

3. DESIGN METHOD

Allowable stress design is applied for this detailed design study in accordance with Specifications for Highway Bridges by Japan Road Association. Allowable stress design is similar to service load design by AASHTO.

4. STRUCTURAL ANALYSIS

The load distribution is calculated by using of Guyon - Masonnet's method based on orthotropic plate theory.

MATERIALS FOR STRUCTURES

1. CONCRETE

Design strength of concrete is specified as follows: Specified

Class	compressiv	ve Chara	cterictic	strength at	t 28 days	
of	strength					Application
concre	te (28days)	Cylii	nders	Cu	ibes	
	(kgf/cm²)	(N/mm²)	(kgf/cm	²)(N/mm²)	(kgl/cm ²)	•
16	160	16	163	20	204	Blinding(leveling),
						Stone masonry
24	240	24	245	30	306	Substructure, Retaining wall,
						Box culvert
32	320	32	326	40	408	Floor slab, Cross beam,
			÷			Felloe guard & parapet (precast),
	-					Cast-in-place concrete pile
40^	400	40	408	50	510	Prestressed concrete girder

presenteerin General Specification for Roads in the Sultanate of Oman, however, it is necessary for prestressed concrete girder.

GENERAL NOTES

2. REINFORCING STEEL

Reinforcing bars are deformed bars according to AASHTOM31/M31M. Grades and tensile requirements are specified as follows:

	·····	
Grade	Tensile strength,	Yeiled strength,
	min (kgf/cm²)	min (kgt/cm ²)

Grade40	4921	2812
Grade60	6327	4218

Bar designaton numbers used in this design are correspond to ones by AASHTO as follows:

AASHTONo.	3	4	5	6	7	8	9	10
This design	D9	D13	D16	D19	D22	D25	D28	D32
3 PRESTR	RESS	ING	TENT	ัดพ				

3. PRESTRESSING TENDON

Prestressing strand comply with the requirements of AASHTO M203, M204 and M275 or BS5896 and BS4486. Prestressing strands for this design are based on Japanese specifications prescribed as follows:

Туре	Area (mm²)	Designation	Ultimate strength (kgf/mm ²)	Yeiled strength (kgt/mm ²)
12T15.2	1664.40	SWPR7B	190	160
1T15.2	138.70	SWPR7B	190	160

ALLOWABLE STRESSES

1. CONCRETE

The allowable stresses in concrete for each class and type are as follows: (1) Prestressed concrete structures (kgf/cm²)

	Class32	Class40
Allowable compressive stress		
- Temporary stress before losses due to creep and shrinkage	140	180
- Stress at service load after losses have occured	110	140
Allowable tensile stress		
- Temporary stress before losses due to creep and shrinkage	-12	-15
- Stress at service load after losses have occured at dead load	0	0
- Stress at service load after losses have occured at service load	-12	-15
Allowable shearing stress		
- Stress at service load after losses have occured at service load		5,5
- Stress at service load after losses have occured at ultimate load	1	53
Allowable diagonal stress		
- Stress at service load after losses have occured at service load		-10

(2) Reinforced con

Allowable compre

- Flexural commp - Axial compressi

Allowable shear st

- only by concrete

- with diaagonal re

- Puncning shear:

Allowable bond st

- with round bar

- with deformed ba

(3) Cast-in-plcae concrete pile Cast-in-concrete piles are constructed by concrete class32, but its allowable stresses are for concrete class24.

(4) Reinforcing Bar

General use Under water

OTHER DESIGN CONDITIONS

OTHERS

- Elevations, staitions and coordinetes are shown in meters. - Other dimensions are shown in millimeters

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NOTES:	JAPAN INTERNATIONAL COOPERATION AGENCY	CLIENT	: MINISTRY OF COMMUNICAT	IONS, DIRECTORATE GENERAL OF ROADS
	(AOR)	PROJECT	T: D/D ON ROAD DEVELOPMENT	PROJECT ON BATINAIL HIGHWAY
	JICA STUDY TEAM	TITLE	GENERAL NOTES	
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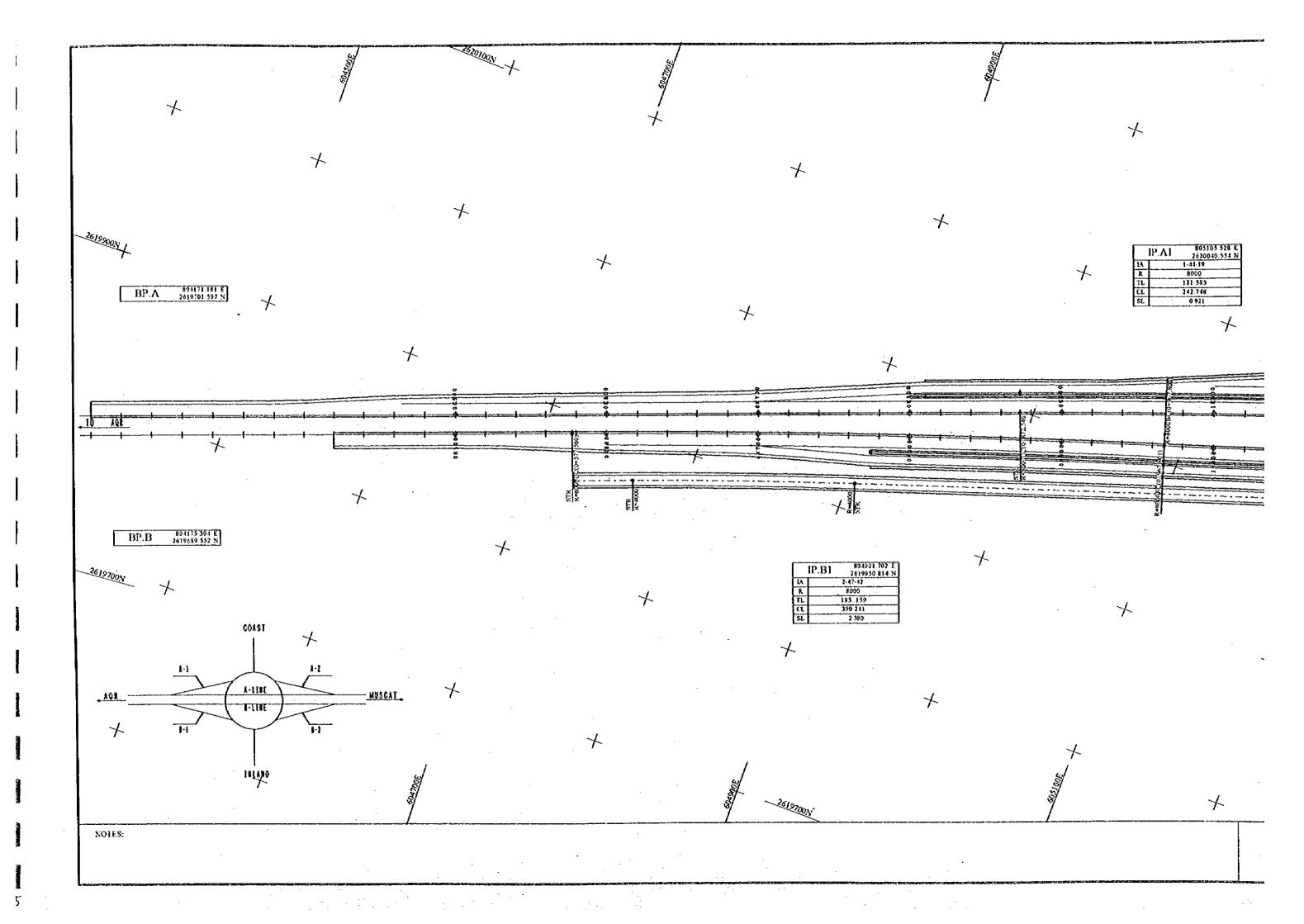
ncrete structure	es (kgť/cm²)			
	Class20	Class24	Class28	Class32
essive stress			-	
pressive stress	65	80	90	100
ive stress	50	65	75	85
stress				
e	3.5	3.9	4.2	4.5
reinforcement	15	17	18	19
stress	8.0	9.0	9.5	10.0
tress				
	7.0	8.0	8.5	9.0
ar	14	16	17	18

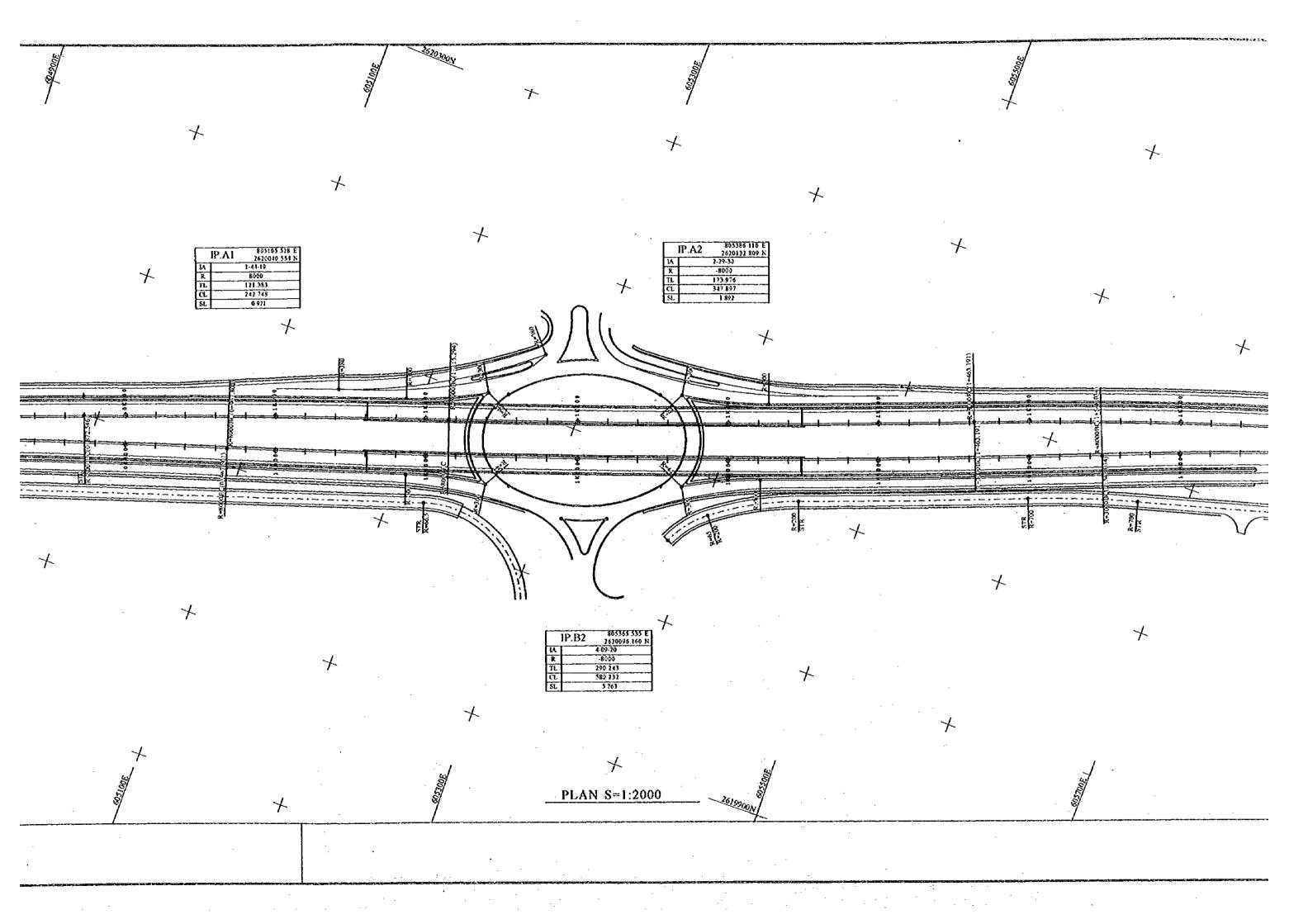
Allowable stresses(kgf/cm³) for each grade of reiforcing bar are as follows: Grade40 Grade60 1400 1800 1400 1600

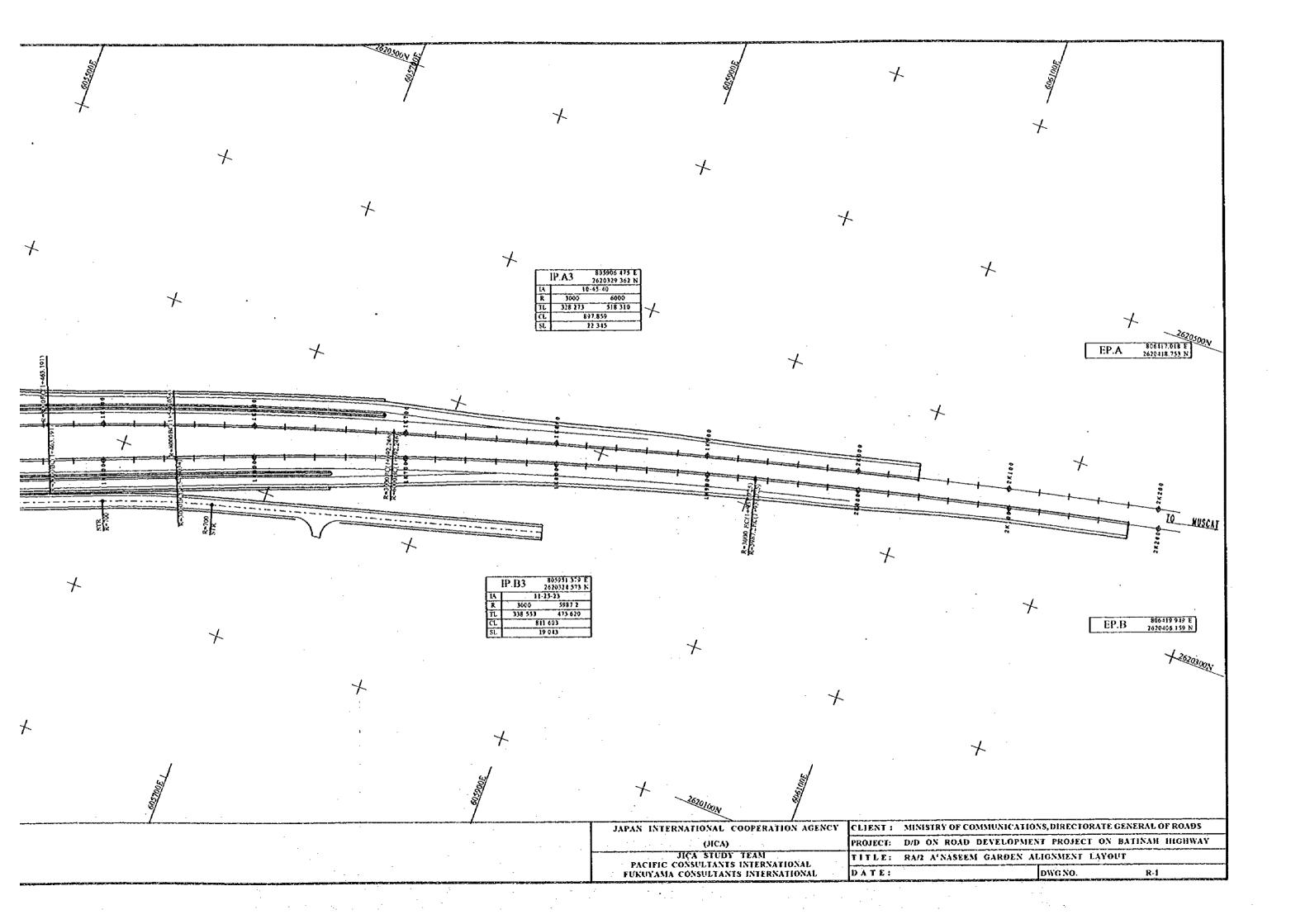
- Lap splicing is applied for all reinforcing bars - Minimum N-value of bearing layer is 30.

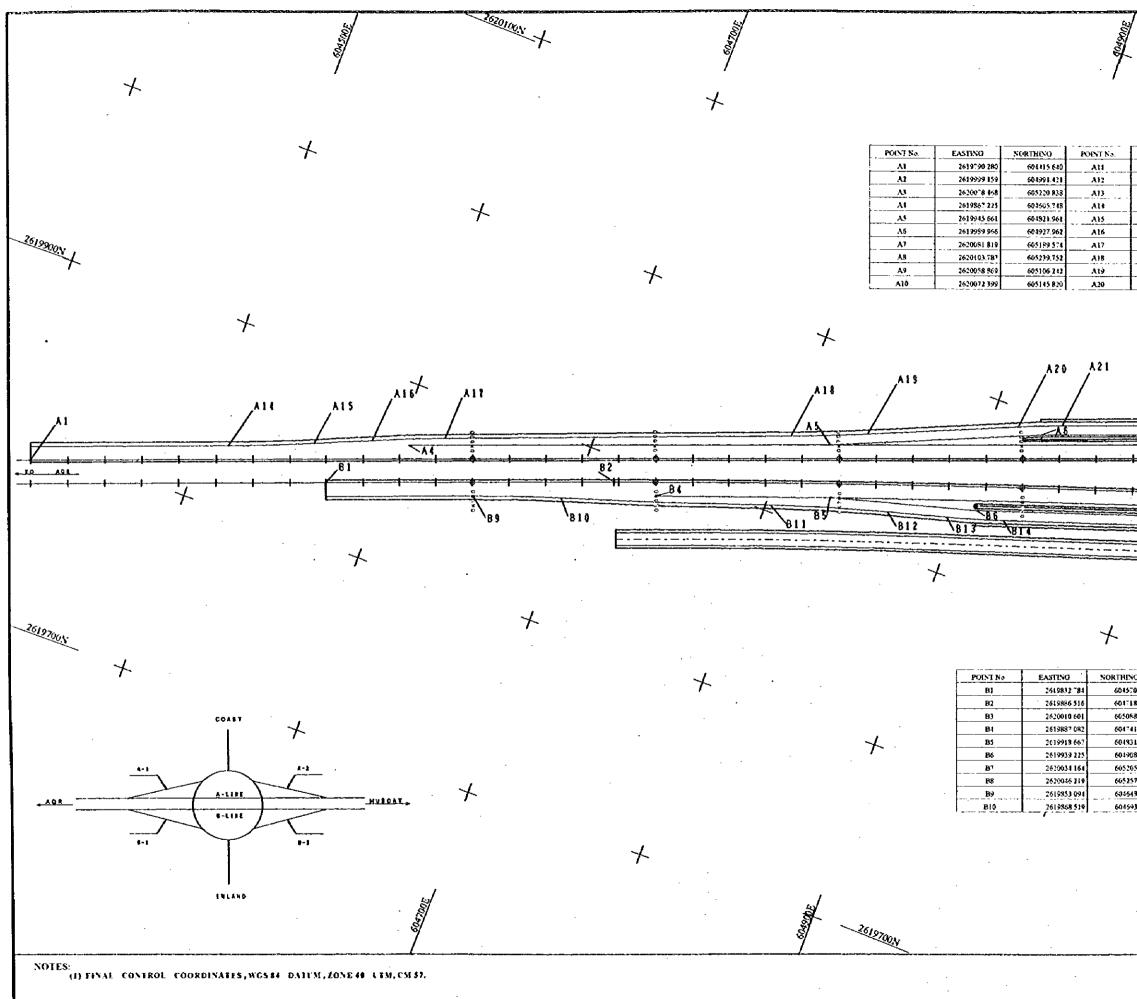
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604570 127	BH	2619903 723	604802 537
601°18 242	B12	2619922 073	604363 688
665058139	813	2619930 403	604895 651
604741 585	B14	2619539 259	601924 67
604831 385	815	2620023 850	603150 031
604908 *83	B15	2620039 524	605240 861
605205 665			
665257 892	•		
605543 107			
604593 872			

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POINT No	EASTING	NORTHINO	POINT No.	EASTING	NORTHING	POINT No
A27	2620194 284	605548 863	Å37	2620193 555	605495 545	<u>A</u> 17
A28	2620266 95*	665766 021	A38	2620158.893	605313 606	A43
A29	2620354 950	606102411	A39	2620157 509	605326 223	A43
A30	2620319 097	605925 750	A 10	2620150 422	605372.647	A50
AN	2620145339	605350 102	AIL	2620179744	665151986	A51
A32	2620159 505	605416723	A12	2620163 905	605313 211	A\$1
A33	2620277_389	605155 385	AØ	2626172 420	605487.932	A53
A34	2630294 201	605833.947	Alt	2520201 743	605510-189	
A35	2620151 352	605325 965	AIS	2620206 648	665526 047	
A36	2620172 245	605437.347	A45	2620222 673	665573.595	

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_	6045 0 127	811	2619903 723	604802 537
-	604718 242	B12	2619922 013	604853 688
	665088 183	B13	2619930 403	601995 05
	664143 885	814	2619939 259	604924 67
	604531 385	215	2420023.850	635190 032
	601908 785	816	2620039 524	66529386
	605205 665			
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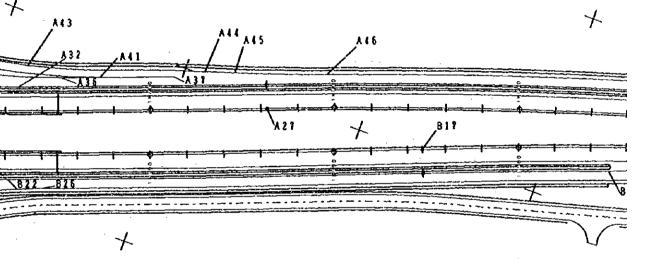
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2620317.760	606001 361	B28	2620272 981	665882 833
2620372 694	605243 463	829	2620318 021	605019 628
2620265 864	605812 982	830	2620327 388	606081537
2620088 165	605380 509	831	2620331 188	566110 893
2620110-064	605428 142	832	2620348 848	606167 197
2620224,744	605735 872			
2620325 415	60606 \$ 577		1	
2620065 561	605361 929			
2620111 603	605447 232			

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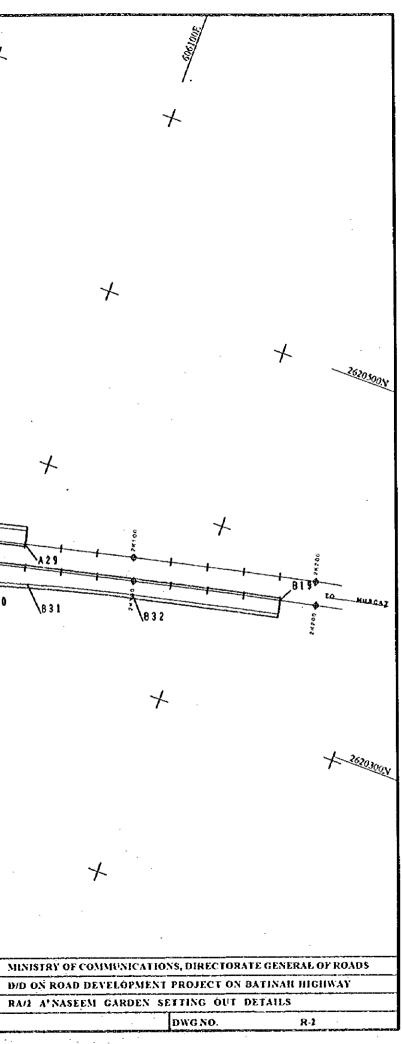
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	2620372 694	606243.463	B29	2620318 021	606048 628
	2620265 864	605812.992	B30	2620327 388	606081 537
-	2020088 165	665330 569	B 31	2620334.188	606110 830
	2620110-064	605428 142	832	2620343 848	606167 197
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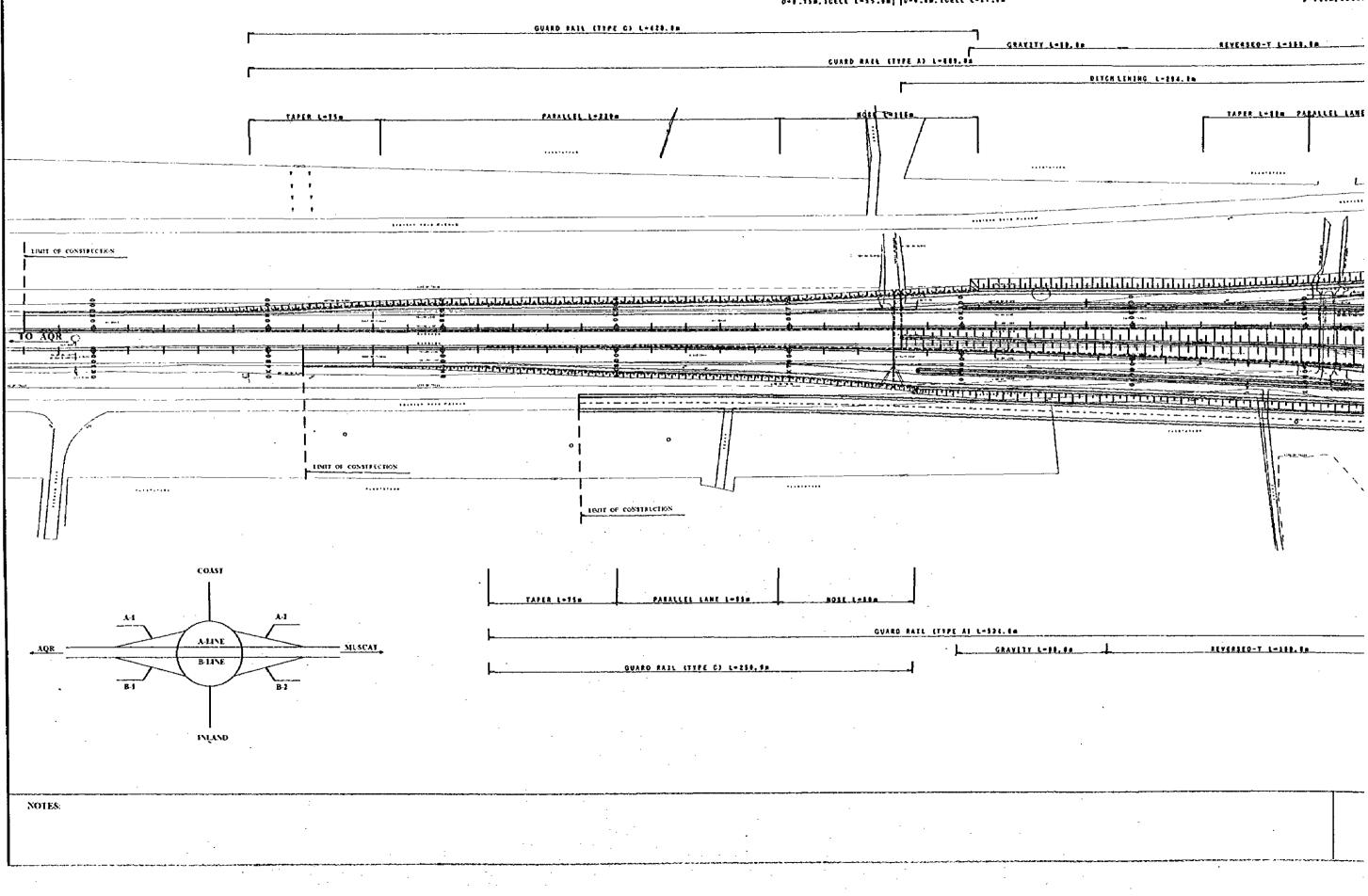
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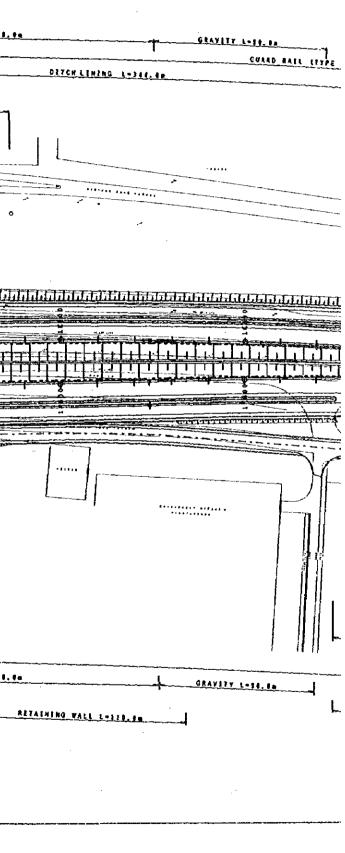


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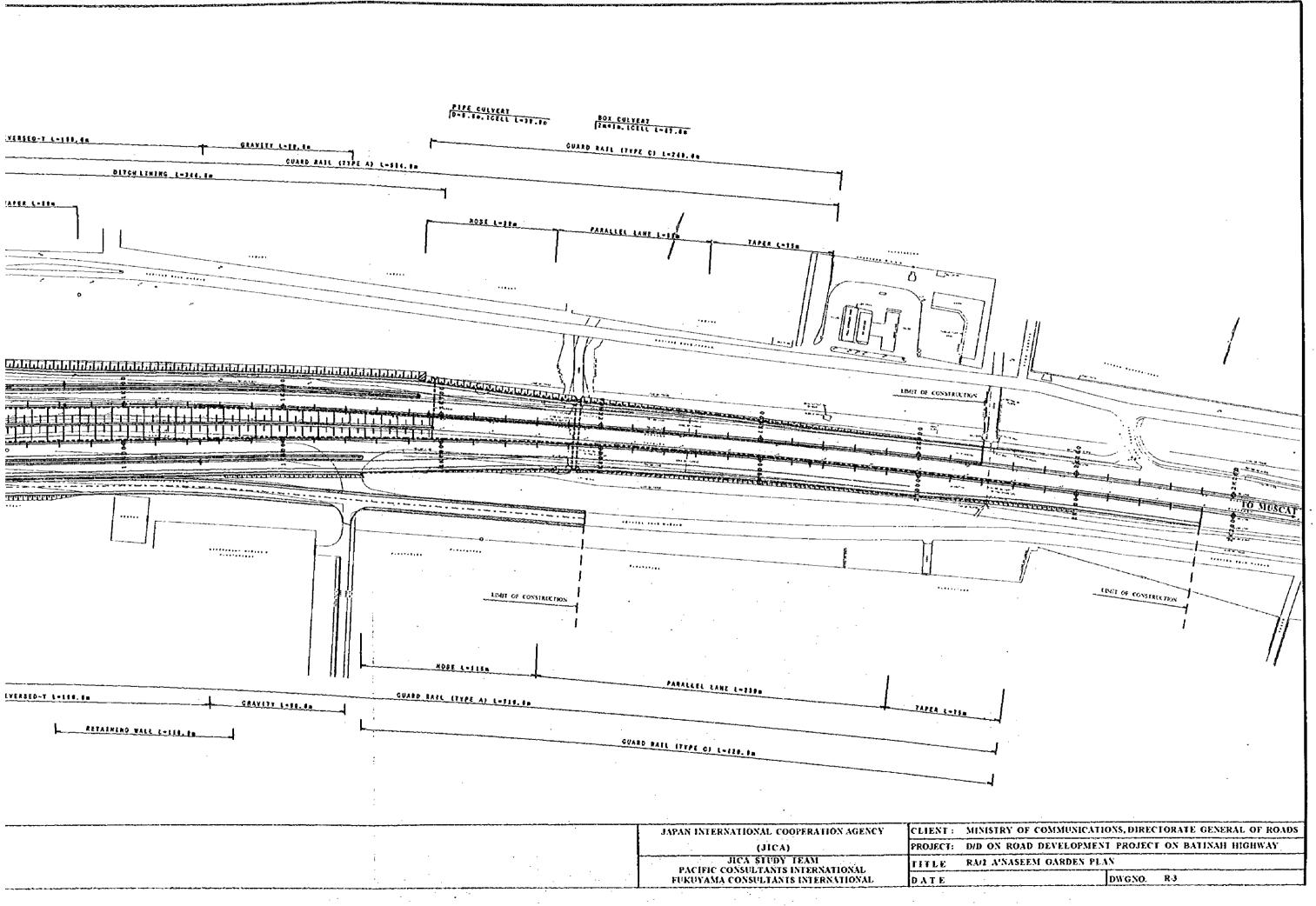


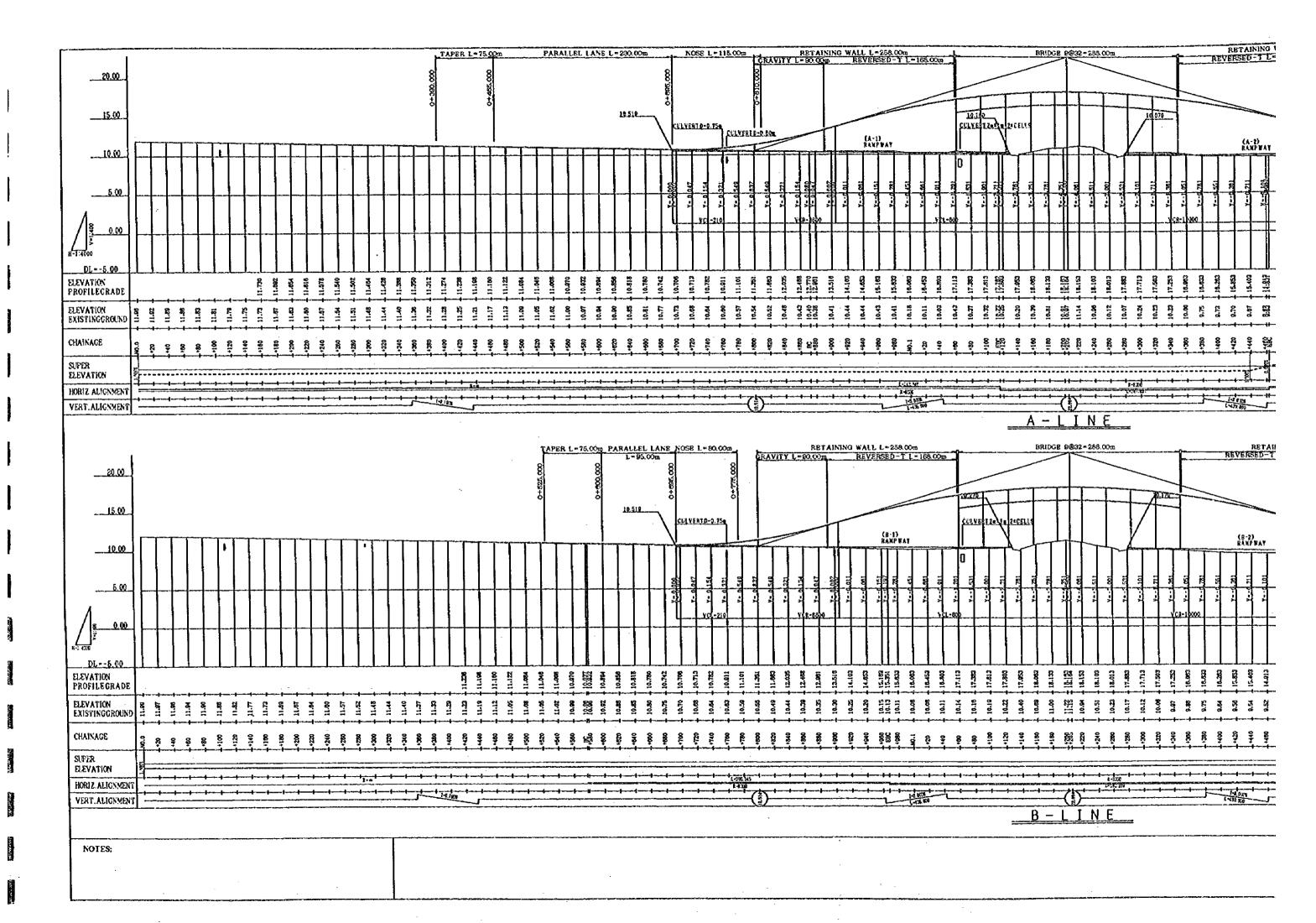
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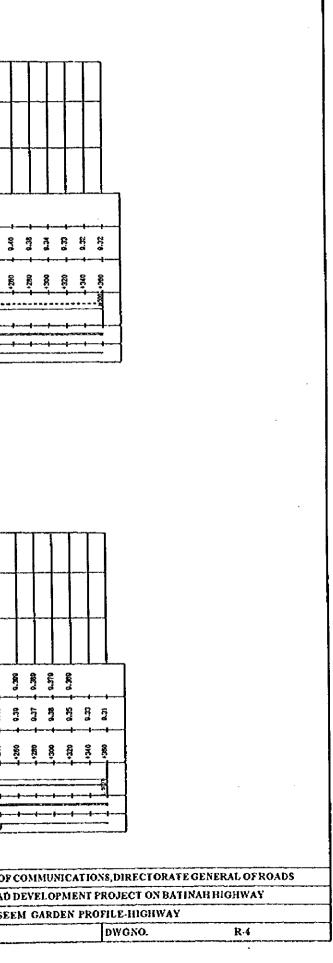


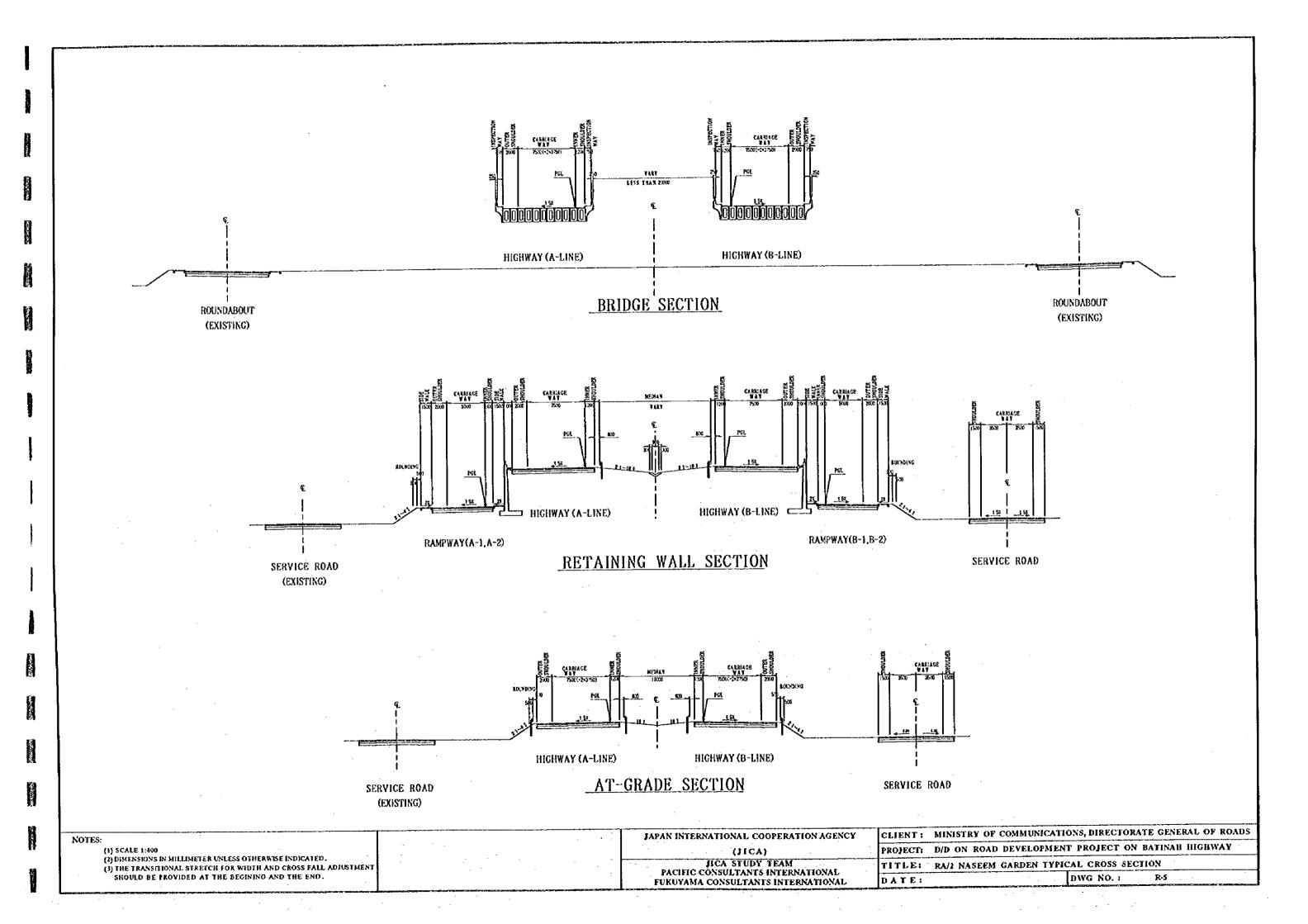
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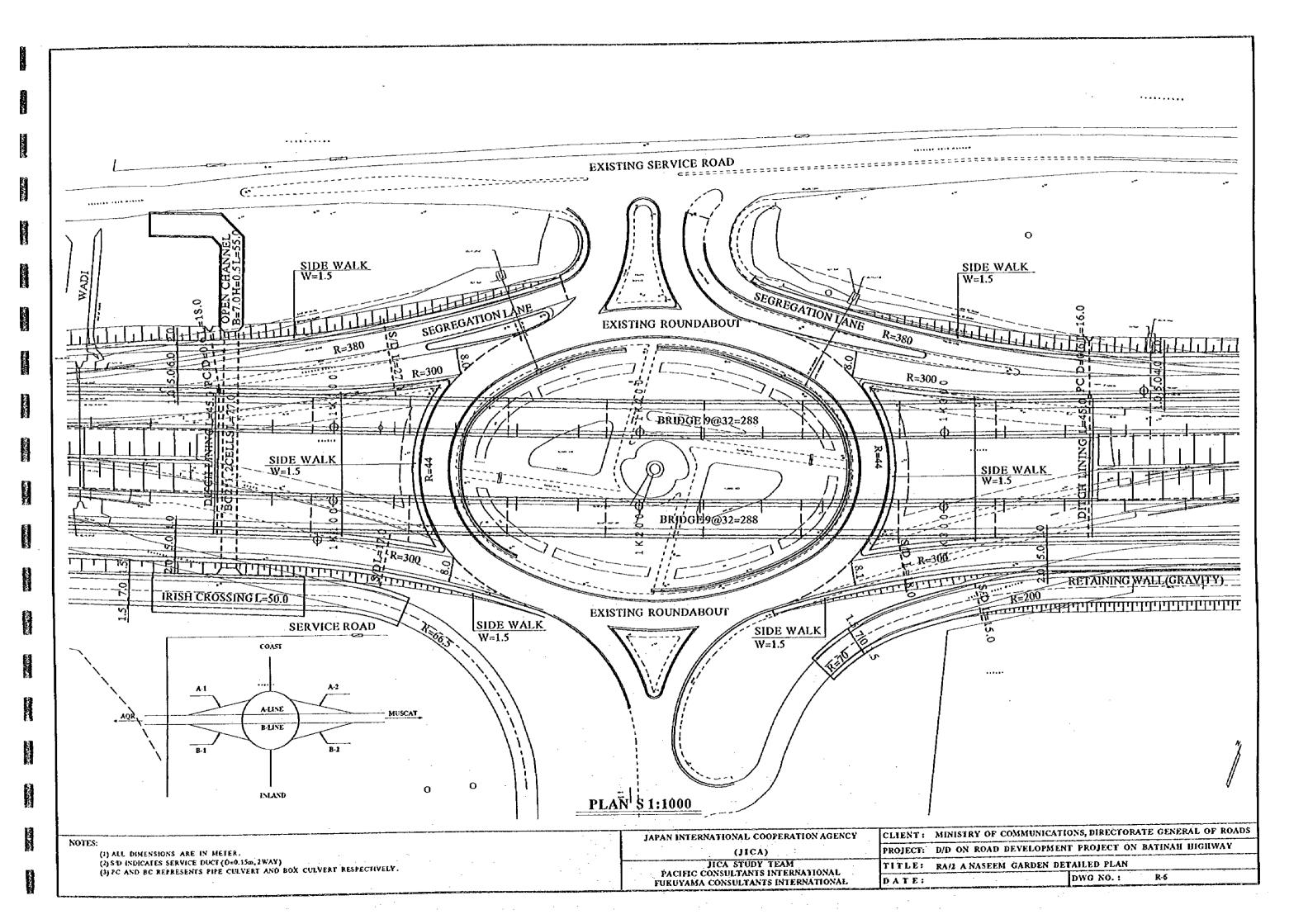


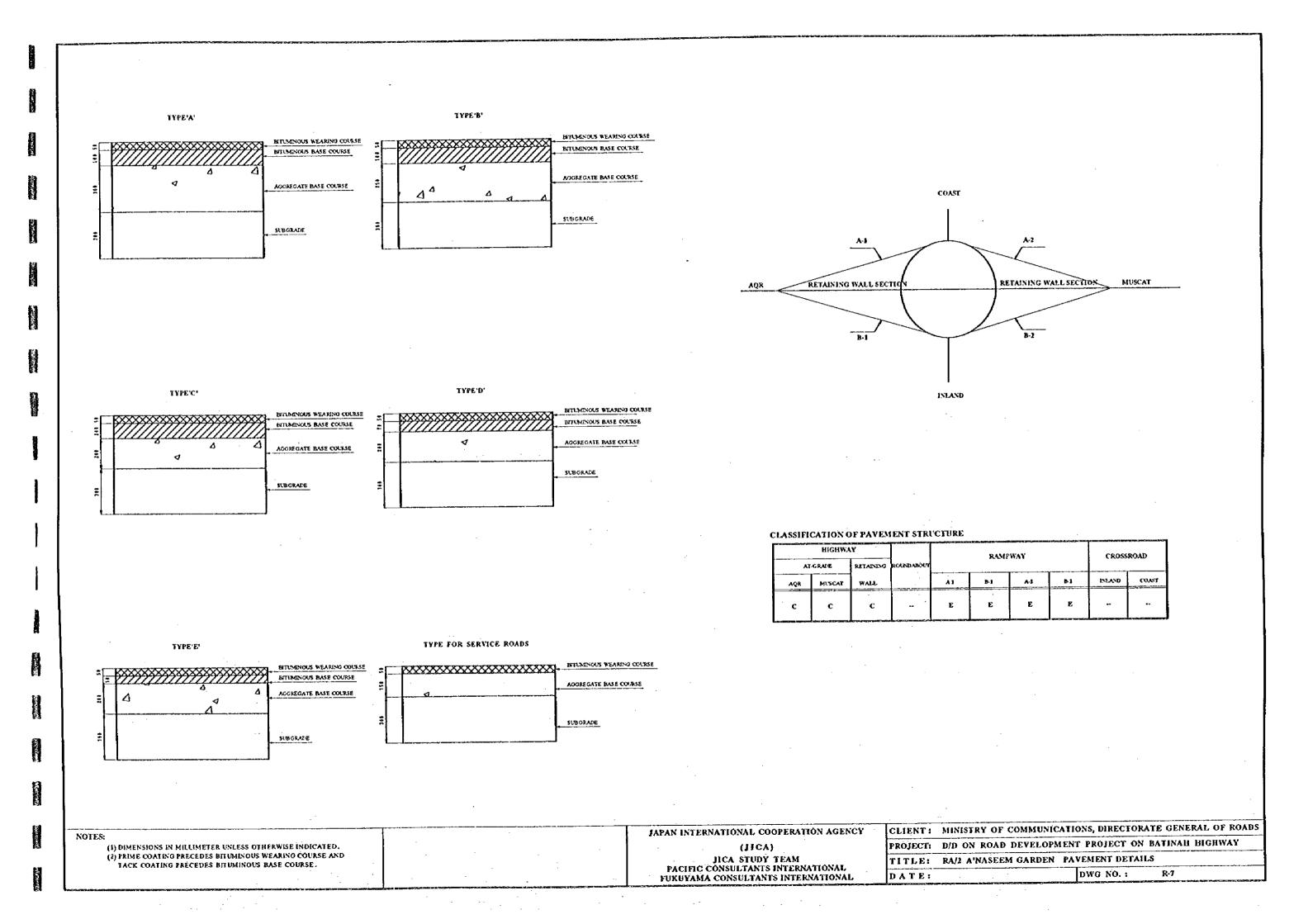


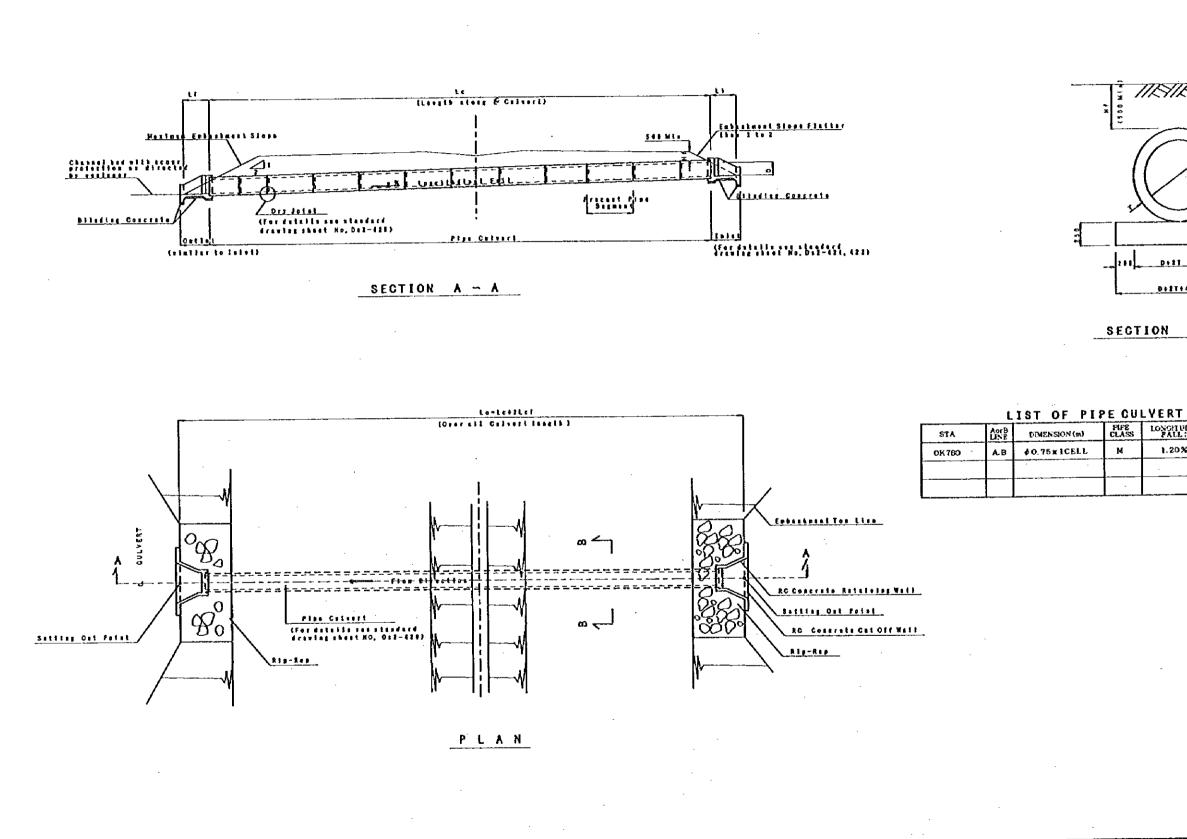
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	<u>A-LINE</u>		.			
258.00m	BRIDGE 9532=258.00m	RETAINING WALL L=268.00m REVERSED=T_L=365.00mGRAVITY_L=80.00m	NOSE L=115.00m	PARALLEL LANE L-230.00	m TAPER L=75.00m	
			80.000 86.000		88 000 200	
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:-3) .MP WAT		(9-2) RIVPBAT				┎╍╍┰╍┈┎╵╍┰╼╍┰╶╍┰╴╍┰╺╍╌┠╴╍┑
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				JAPAN INTERNATIONAL	COOPERATION AGENCY	CLIENT : MINISTRY OF C
				(J10	A)	PROJECT: D/D ON ROAD I
				JICA STU PACIFIC CONSULTAN FUKUYAMA CONSULTA	TS INTERNATIONAL	TITLE RA/2 A'NASEE DATE
	<u> </u>		<u></u>			
		· · · ·	· · · .		·	











	NOTES: (1) DIMENSIONS IN MILUMETER UNLESS OTHERWISE INDICATED. (2) THE LONGITUDUXAL FALL OF OPEN CHANNEL IS 35% AND IT'S WIDTH DEPENDS ON THE WIDTH OF THE ADJACENT BOX CULVERT. (3) THE DEPTH OF DITCH UNING BECOMES 400mm AT CATCH PIT. (4) THE UPVC OF 100mm IN DIAMETER IS INSTALLED AT AN INTERVAL OF ABOUT 20m. (5) THE DROPPED SIDEWALK IS INSTALLED ALONG RAMPWAYS, AUNE HIGHWAY AND THE BIGINING SECTION OF BLINE AT AN INTERVAL OF 50m.	(JICA) JICA STUDY TEAM PACIFIC CONSULTANTS INTERNATIONAL	PROJECT	MINISTRY OF COMMUNICATI D/D ON ROAD DEVELOPMEN RA/2 A'NASEEM GARDEN D
ł	(5) THE DROPPED SIDEWALK & INSTALLED ADDITION FOR THAT THE POINT AND THE			· · · · ·

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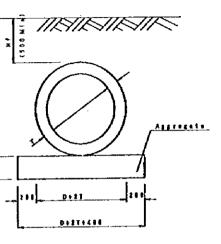
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Management

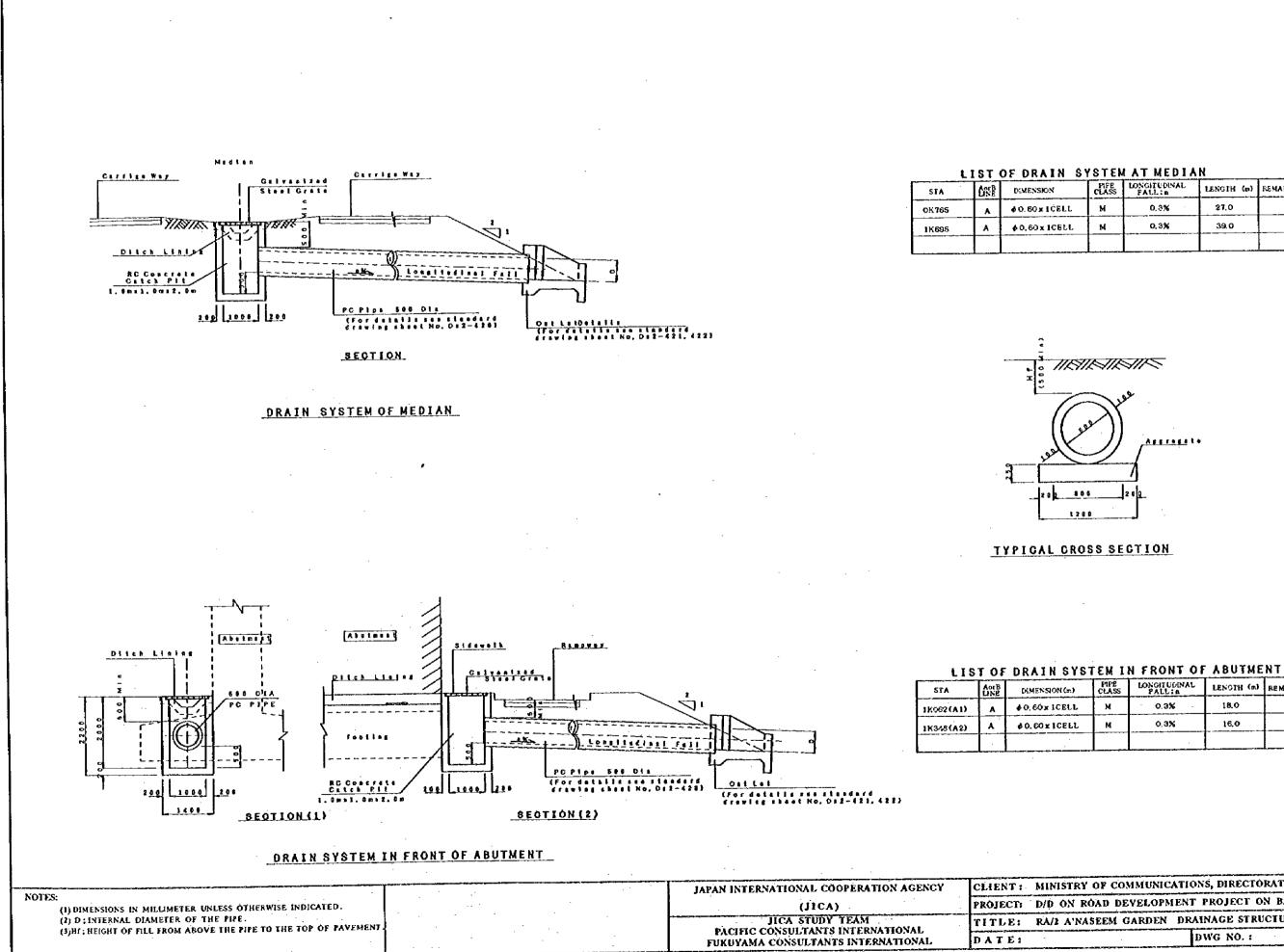
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SECTION B В

PIFE CLASS	LONGITUDINAL FALL: a	LENGIH (m)	REMARK
м	1.20%	65.0	New Construction

P COMMUNICA	TIONS, DIRECTOR	ATE GENERAL OF ROADS
D DEVELOPM	ENT PROJECT ON	I BATINAH HIGHWAY
EM GARDEN	DRAINAGE STRU	CTURE (1/3)
	DWG NO. :	R-8
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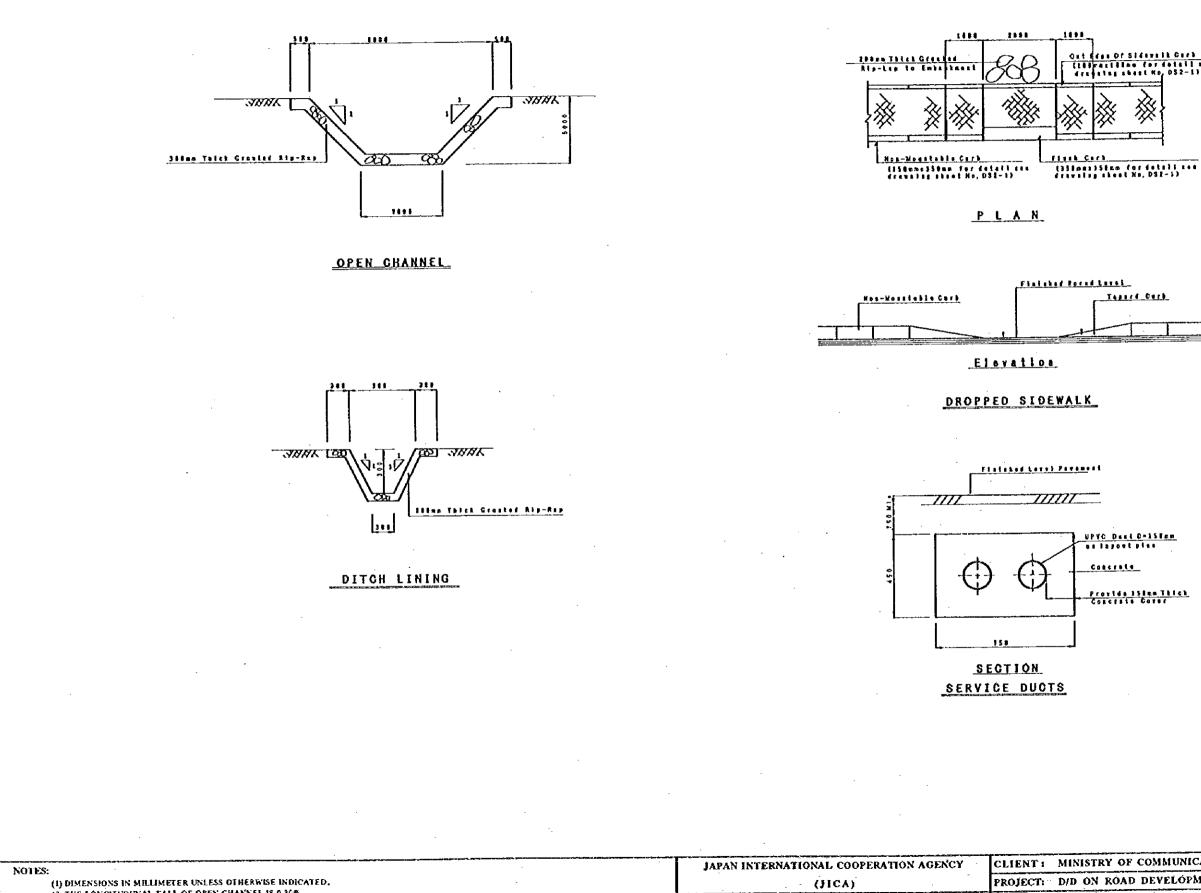
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PIFE CLASS	LONGITUDINAL FALL:n	LENGIH (m)	REMARK
м	0.3%	27.0	
м	0.3%	39.0	

PIPE CLASS	LONGIT UDINAL PALL : a	LENGTH (m)	REMARK
м	0.3%	18,0	
м	0.3%	16.0	

OR COMMUNICAT	TIONS, DIRECTORATE GENERAL OF ROADS
	NT PROJECT ON BATINAH HIGHWAY
in the second	DRAINAGE STRUCTURE (2/3)
	DWG NO. : R-9



(2) THE LONGITUDUNAL FALL OF OPEN CHANNEL IS 0.30%. (3) THE DROPPED SIDEWALK IS INSTALLED ALONG RAMPWAYS AT AN INTERVAL OF 500.

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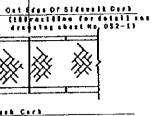
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JICA STUDY TEAM PACIFIC CONSULTANTS INTERNATIONAL TITLE: RA/LA'NASE DATE: FUKUYAMA CONSULTANTS INTERNATIONAL

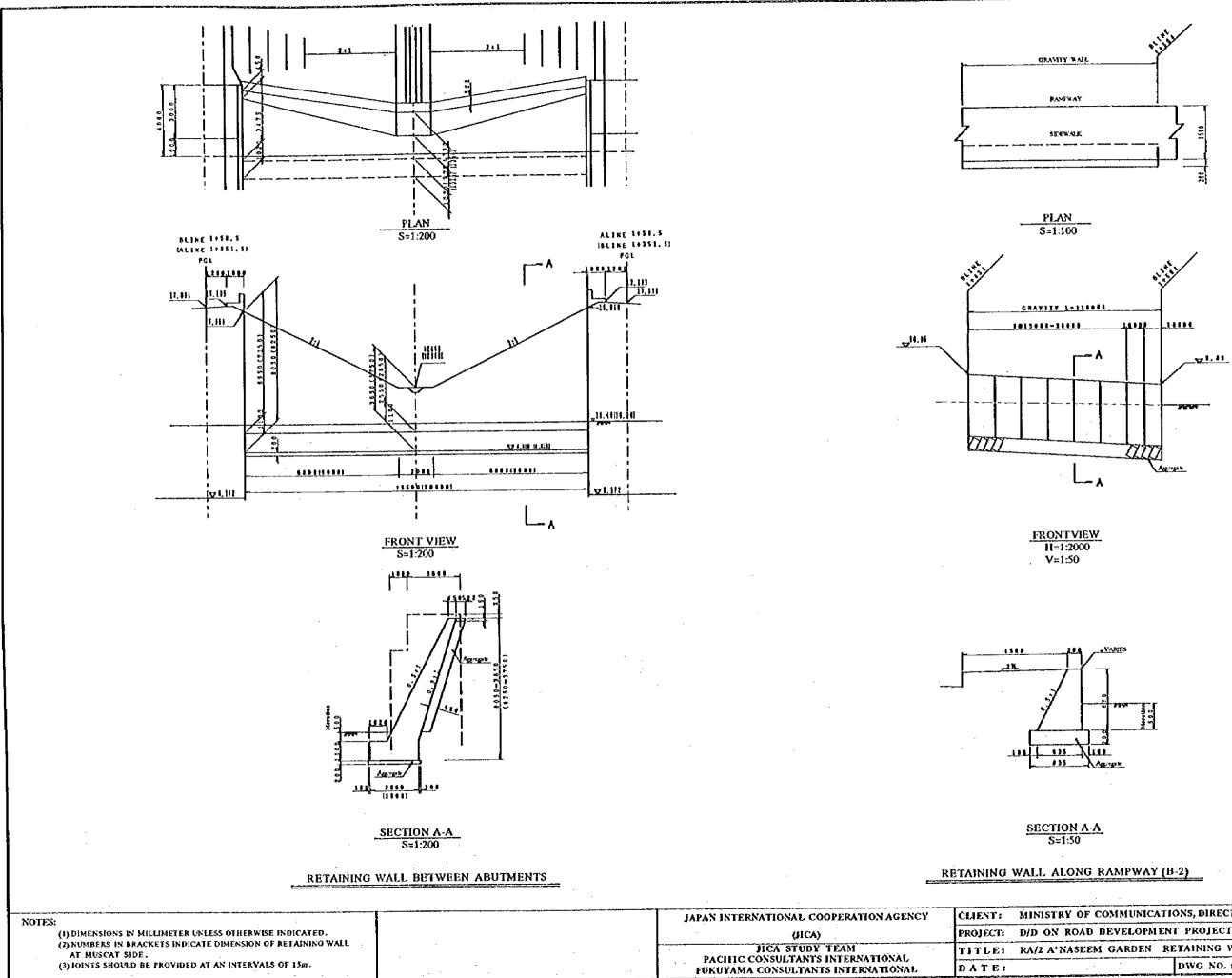


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Provids 151mm Thick Concesses Cover

ATIONS, DIRECTOR	ATE GENERAL	OF ROADS
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RAINAGE STRUCTU	RE(J/J)AND SEF	VICE DUCTS
DWG NO. :	R-10	
	IENT PROJECT ON RAINAGE STRUCTUI	ATIONS, DIRECTORATE GENERAL IENT PROJECT ON BATINALI III RAINAGE STRUCTURE(3/3)AND SEF DWG NO. : R-10



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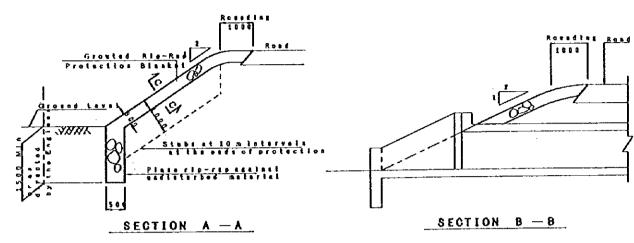
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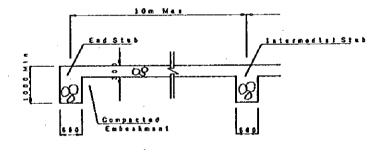
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F COMMUNIC/	TIONS, DIRECTOR	ATE GENERAL OF RO	DADS
D DEVELOPM	ENT PROJECT ON	BATINAN HIGHWAY	(
EM GARDEN	RETAINING WALL	L	
	DWG NO. :	R-11	

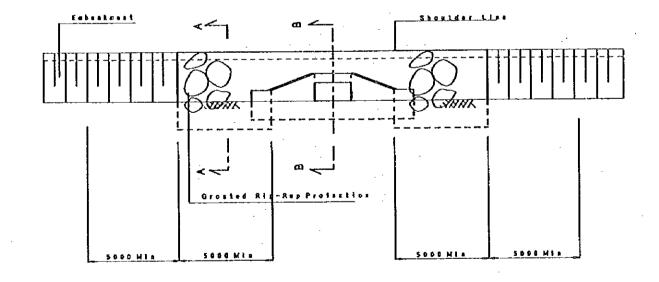
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PLAN





SECTION C - C

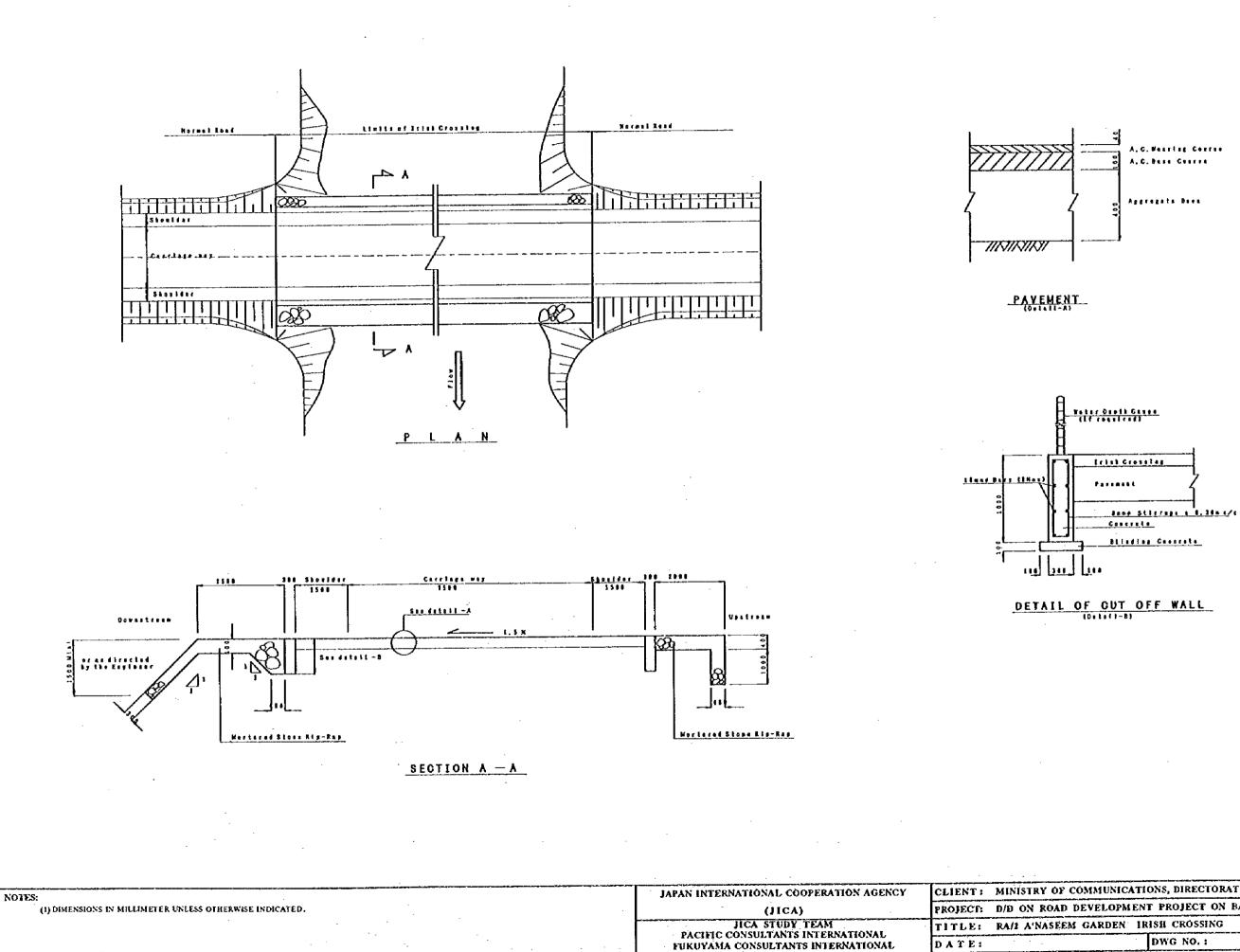


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FRONT VIEW

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NOTES: (1) DIMENSIONS IN MILLIMETER UNLESS OTHERWISE INDICATED.				·	(JICA) JICA STUDY TEAM	PROJECT: D/D ON ROAD DEVELOPMENT PROJECT ON BATINALI HIGHWA TITLE: RA/2 A'NASEEM GARDEN SLOPE PROTECTION			
			· .						
	. * .	-	. ·		PACIFIC CONSULTANTS INTERNATIONAL FUKUYAMA CONSULTANTS INTERNATIONAL	DATES		DWG NO. :	R-12
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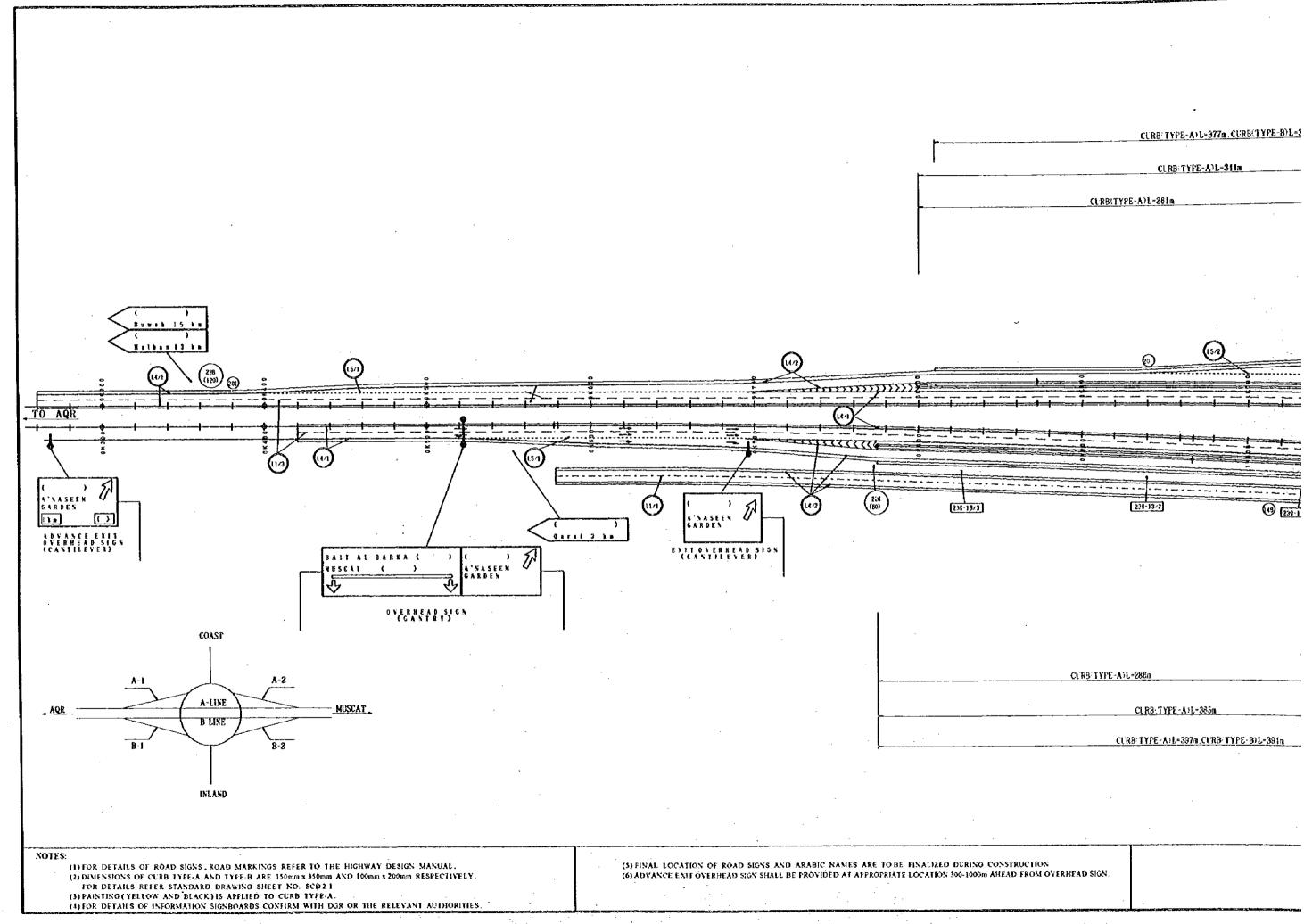
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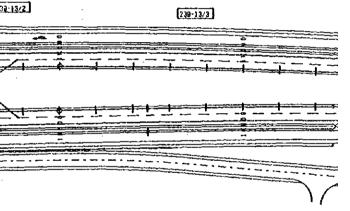
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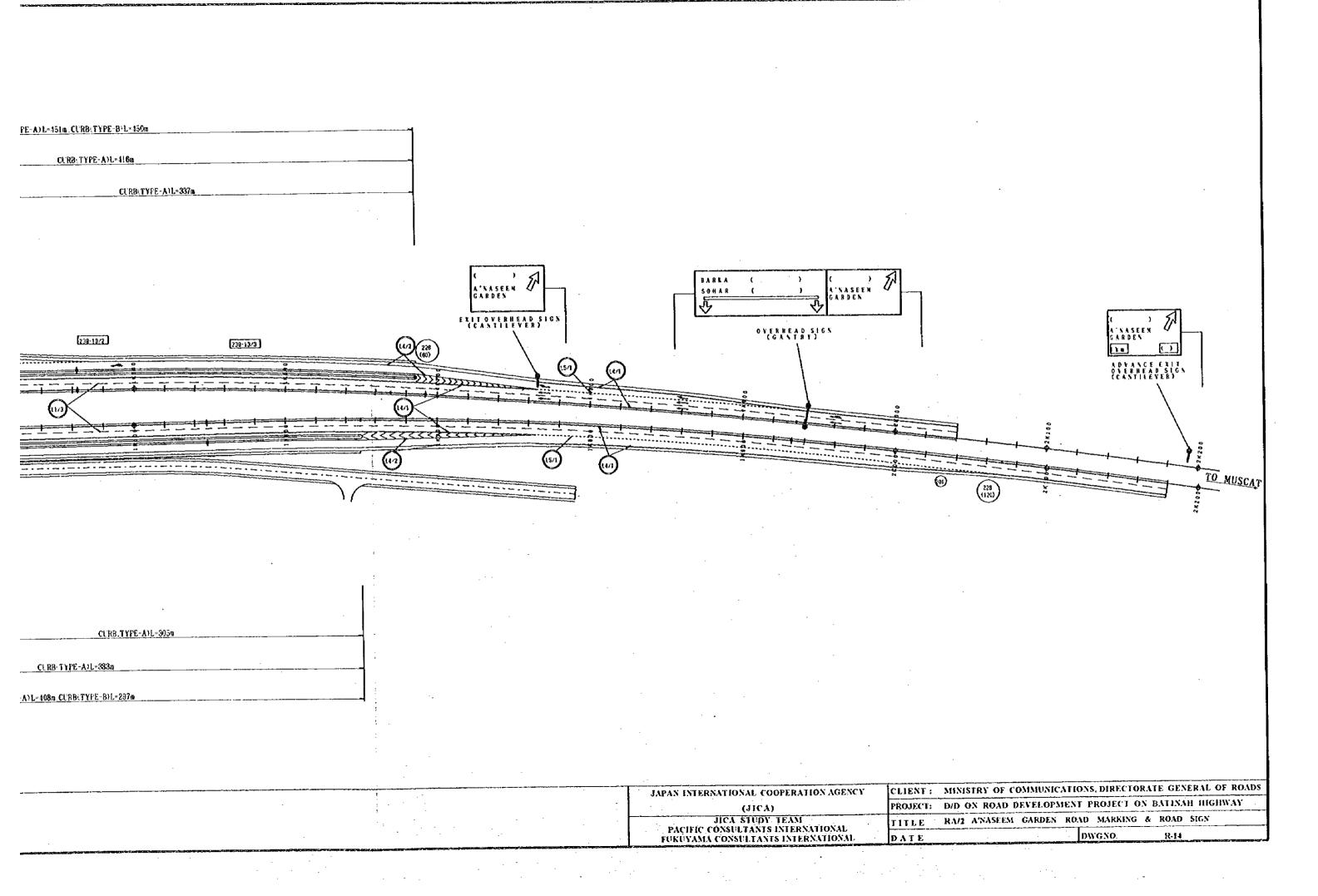
COMMUNICA	TIONS, DIRECTORATE GENERAL OF ROADS
DEVELOPMI	ENT PROJECT ON BATINAH HIGHWAY
M GARDEN	IRISH CROSSING
	DWG NO. : R-13

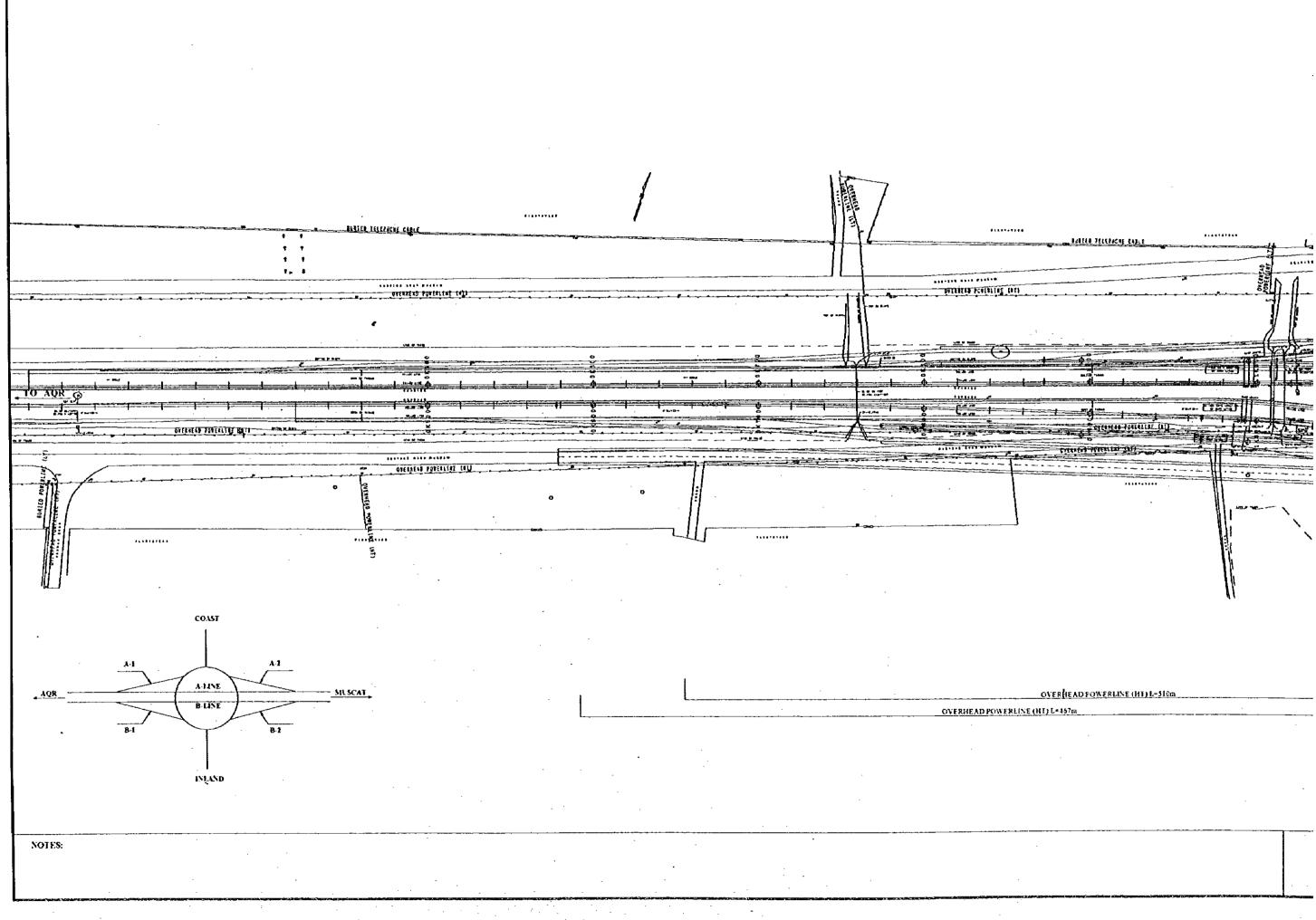


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	CURB: TYPE-A:L-880	<u>URB: TYFE-A)L=117a</u>	
CLRB: TYFE-A\L=377 <u>n_CL'RB: TYFE-B)L=37</u> 5n		CURB: TYP	E-A)L=151m.CLRB.TYPE-B-L=150m
<u>CLR3 TYPE-A)L-314a</u>			CURB:TYFE-A)L=416m
CURB:TYPE-A)L=261m	CURB(TYPE-B)L-80m	CURB.TYPE-B1L-76a	CURB(TYPE-A)L=337n
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ÇU'RB' TYPE-A`L=286m	CLRB(TYFE-8)L-801	CURB TYFE-BיL=78መ	CURB&TYPE-AIL=303m
CLRB-TYPE-A)L=365m		· · · · · · · · · · · · · · · · · · ·	CURB(TYPE-A)L-383m
CURB TYPE-A:L=397n_CURB:TYPE-B1L=394n		CURB(TYPE-A)	L-4089.CLRB(TYFE-B)L-297n
JURING CONSTRUCTION. 10-1000m AHEAD FROM OVERHEAD SIGN			
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OVER HEAD FOWERLINE (HT) L=510	)m	
LINE (HI) L*467m	·	

