FIGURES

: Water Kiosks Constructed : Boundary of Supply Area : Existing Intake : Existing Treatment Plant : Existing Storage Tank

: Boundary of Study Area

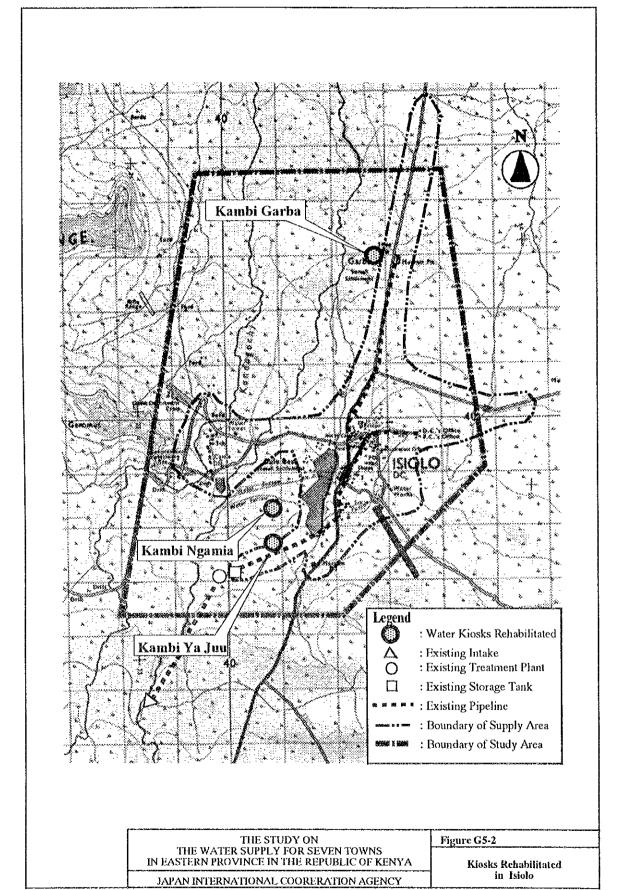
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THE STUDY ON
THE WATER SUPPLY FOR SEVEN TOWNS
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Figure G5-1

Kiosks Constructed in Meru

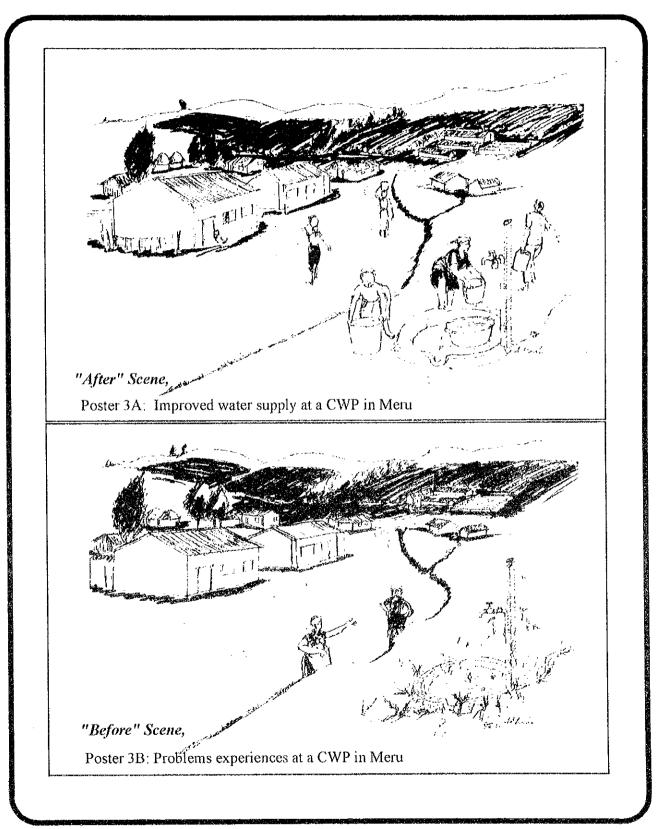








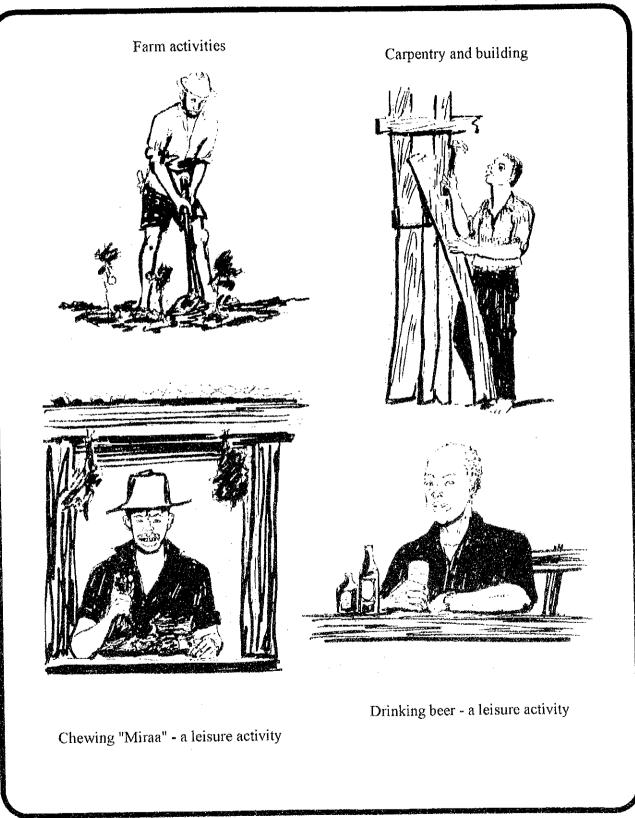
Posters 3A & 3B: Story with a Gap, Meru



Poster 3C Role Played by Gender in Water and Sanitation Activities (Meru)



Poster 3F Roles and Activities That Men are Engaged in Meru



Poster 3G Roles Performed by Women in Meru





Child care - bathing children



Cooking for the family

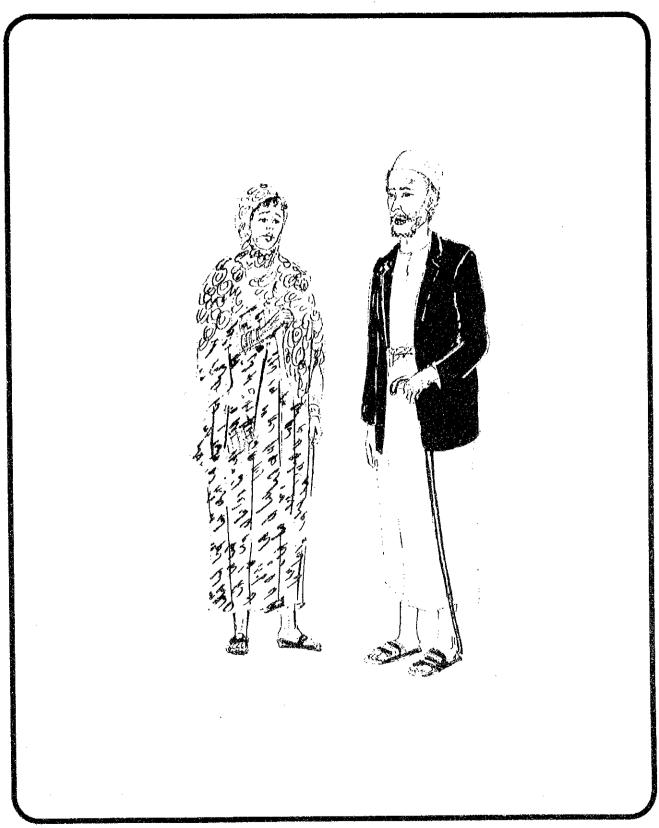


Digging - farm activities

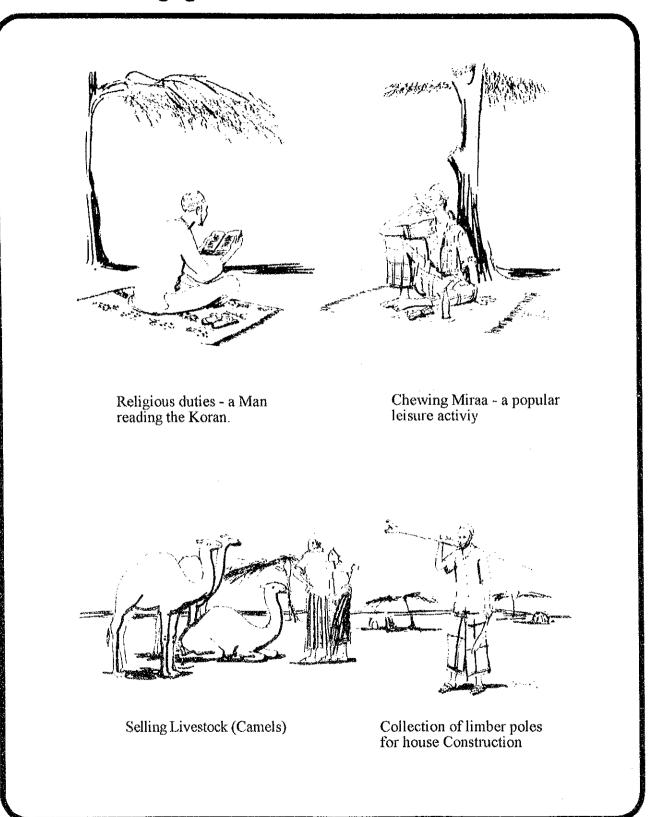


Collecting firewood

Poster 3K Role Played by Gender in Water and Sanitation Activities

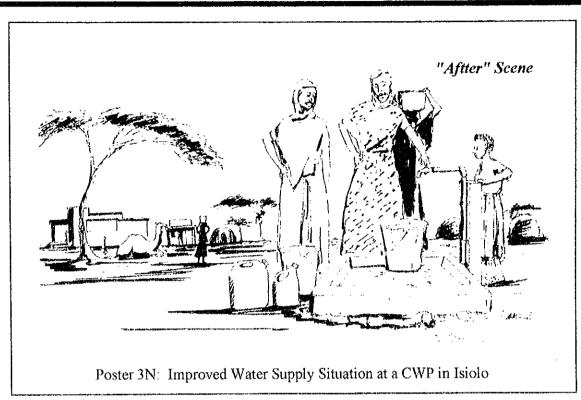


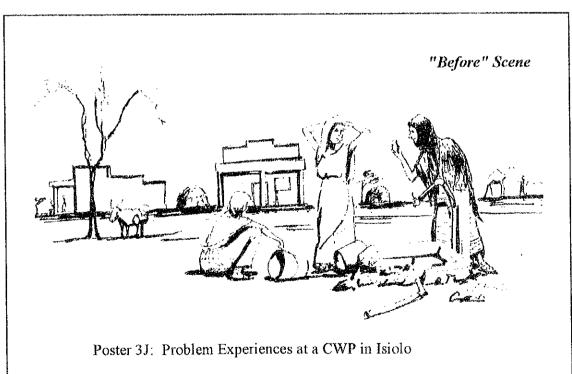
Poster 3M Roles and Activities that Men are Engaged in Isiolo



Poster 3N & 3J Story with a Gap Isiolo

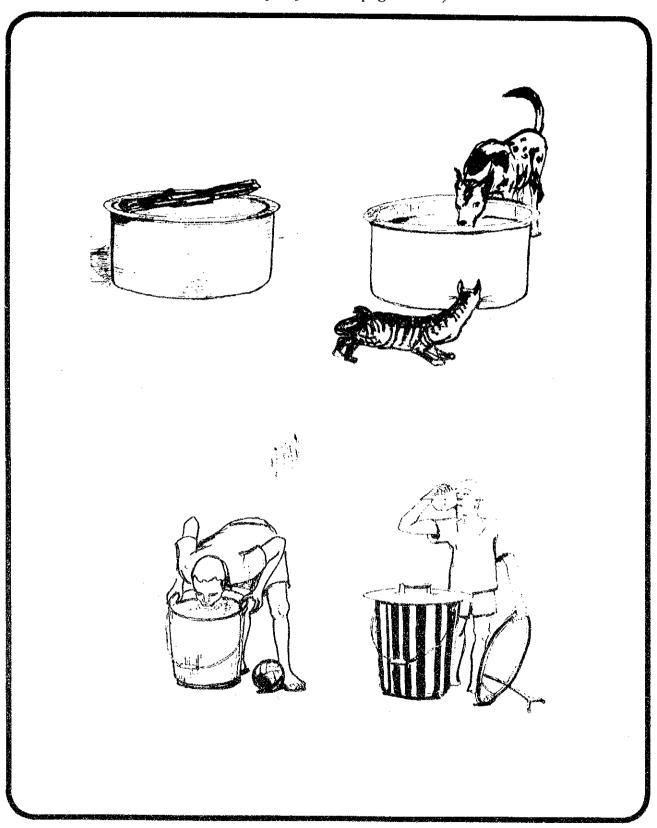
Adopted from TCP pages 118-119





Poster 4B Water Storage and Usage

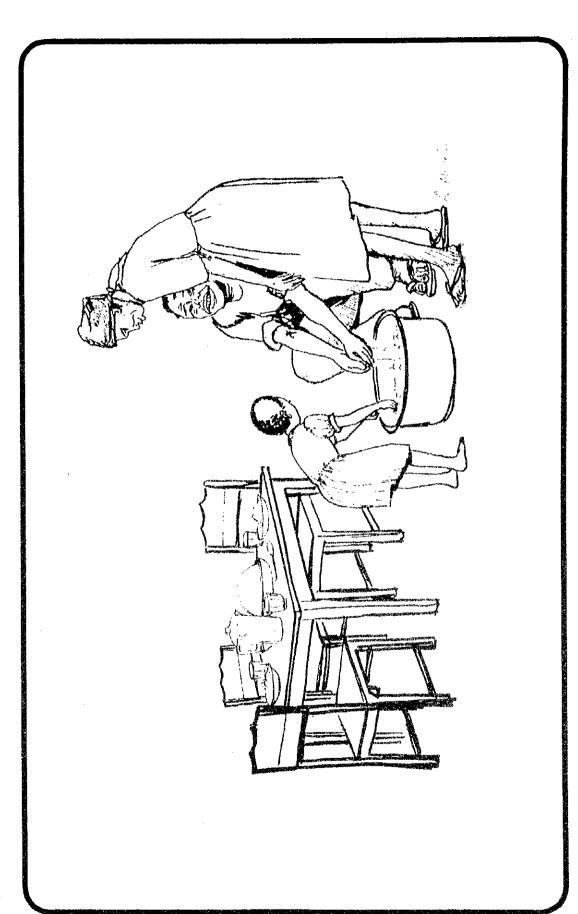
(Pocket Chart - Adopted from TCP pages 96-97)



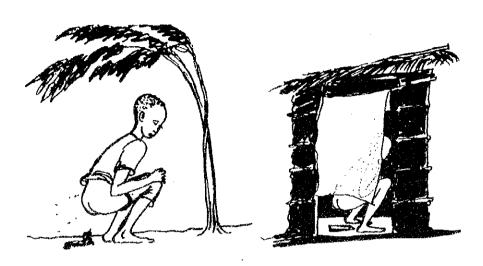
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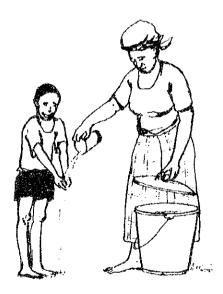


Defecating in the bush.

Partly constructed pit latrine.



A boy scratches his cleans himself with the open palm.



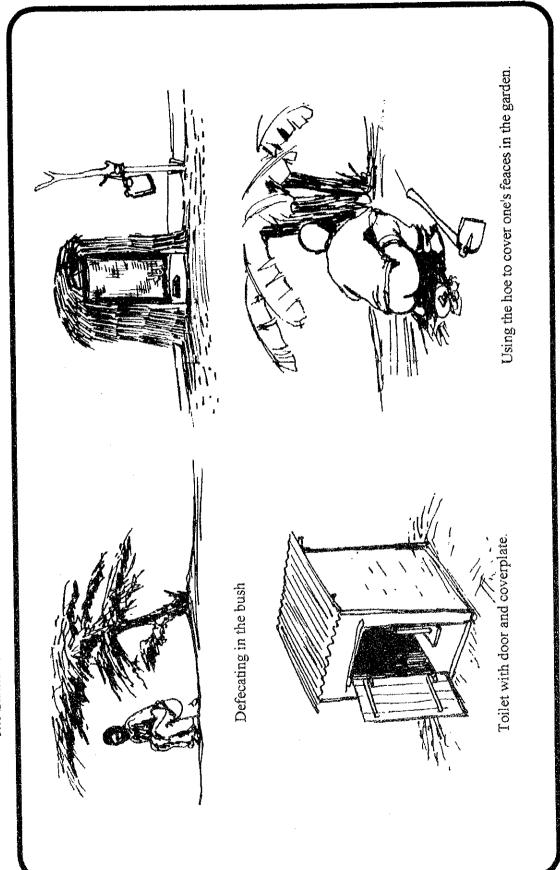
Washing hands after using the latrine.

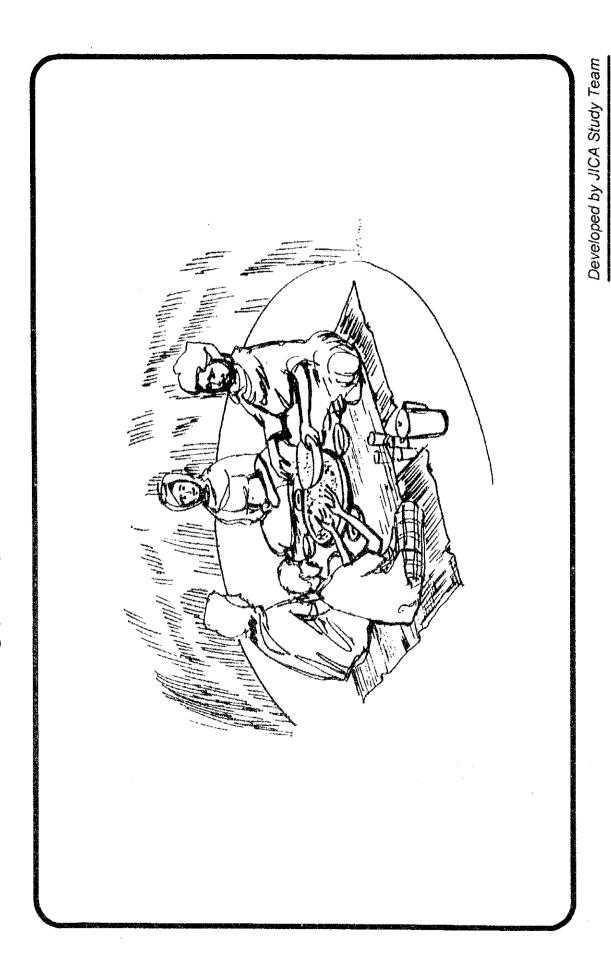
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Poster 4E1: Ways of Excreta Disposal

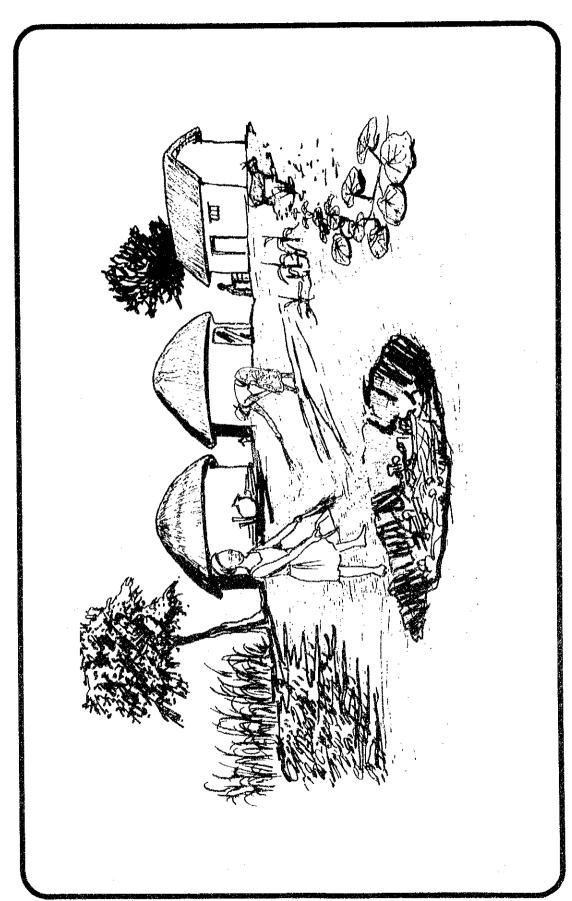
-The Sanitation Ladder



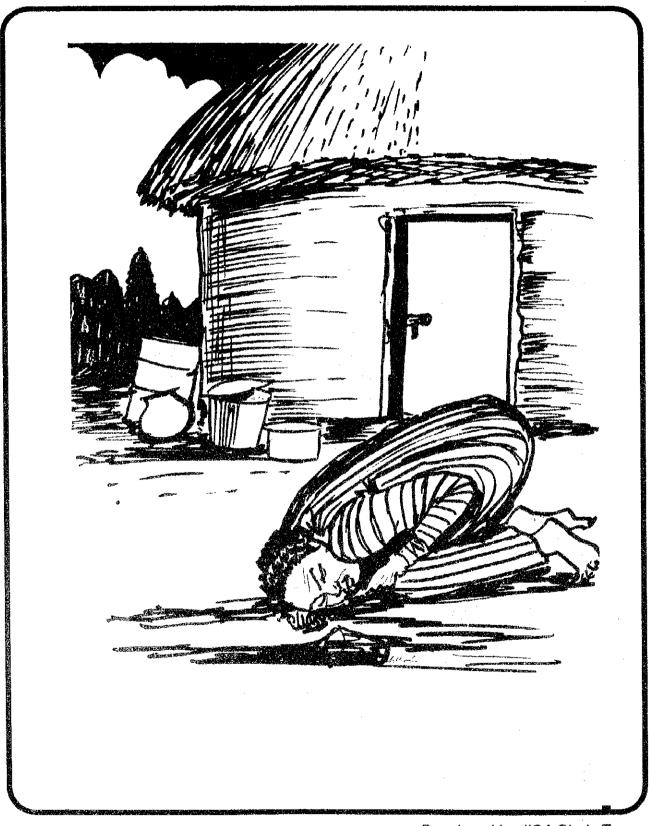








Poster 4K: Consumption of Contaminated "Unsafe Water" - The aftermath



Poster 5: Mosquito Breeding Grounds and its Attack on humans - Adopted from Environmethal Health Engineering in the Tropics, page 201

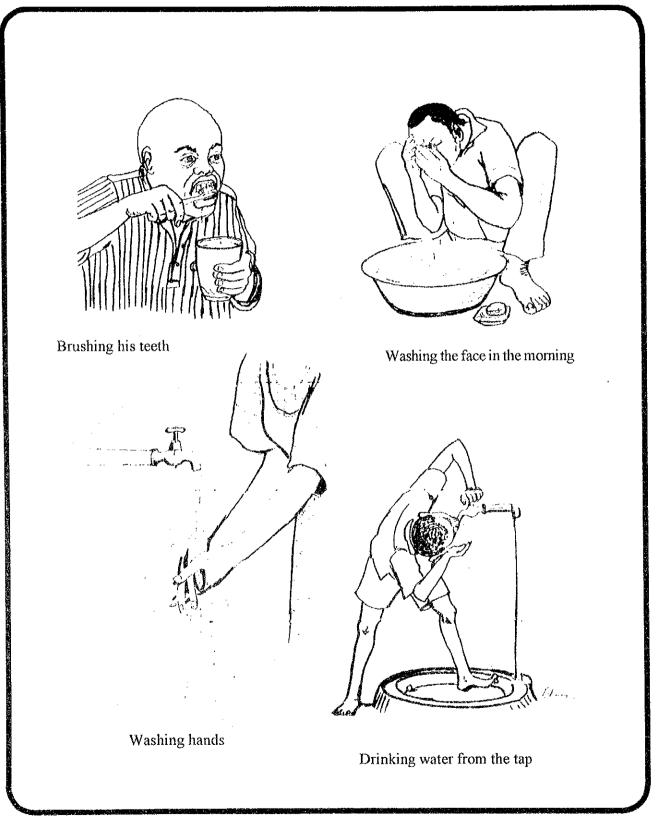
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Poster 4H Personal Hygiene Practices

Adopted for TCP pages 102-103



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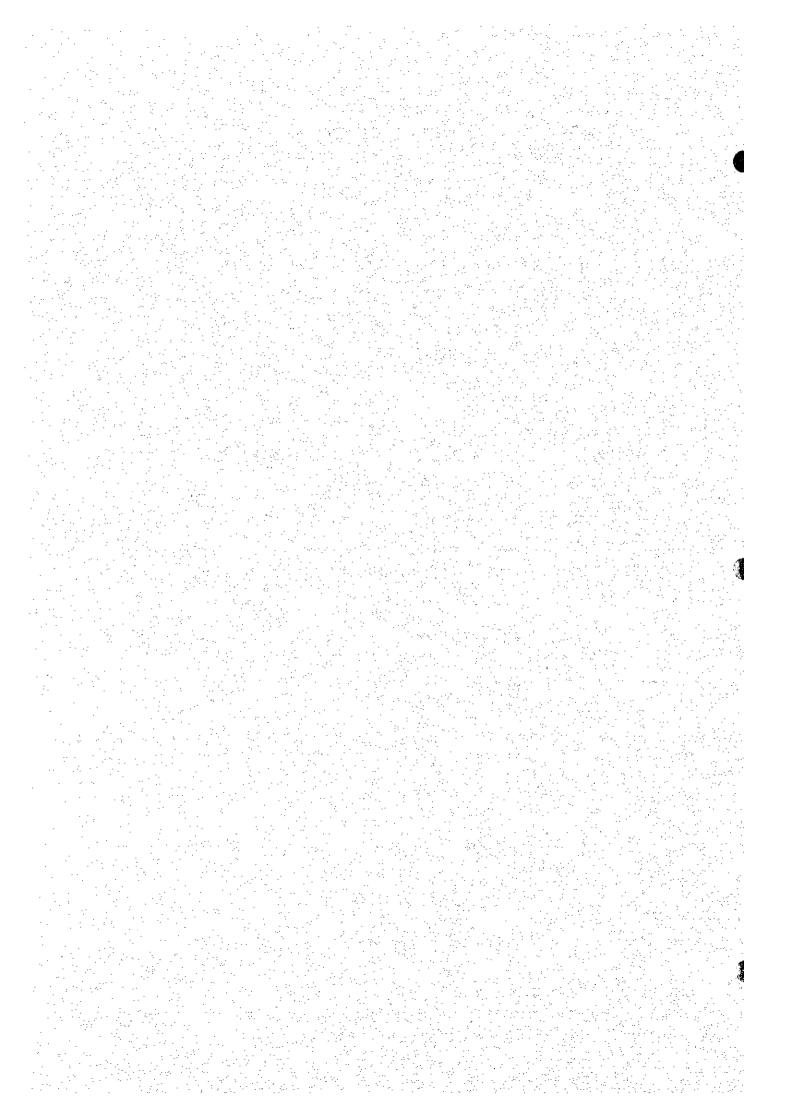




THE STUDY ON WATER SUPPLY FOR SEVEN TOWNS IN EASTER PROVINCE IN THE REPUBLIC OF KENYA

APPENDIX H

WASTEWATER DISPOSAL FACILITIES



APPENDIX H WASTEWATER DISPOSAL FACILITIES

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1. GENERAL BACKGROUND

1.1 Sanitation and health

The seven schemes included in this study are mostly rural in character, and the existing widespread use of pit-latrines in these relatively low population density areas is the most appropriate form of sanitation for the foresceable future. The Study of social survey indicated that 88% of the population used pit latrines, many of which however were poorly constructed and maintained.

Isiolo however, where roughly 25% of the population use the bush, was an exception, due to the traditional nomadic way of life of many of its inhabitants.

Nevertheless, roughly 80% of the 700 households interviewed revealed a reasonable knowledge of how water related diseases are conveyed.

Malaria was the most frequent disease reported as affecting households. Typhoid was high in both Meru and Isiolo, suggesting that areas of higher urbanisation may be more prone to such diseases.

Table H-1 provides health statistics obtained from the social survey, and also indicates the current and projected urban levels in the different scheme areas.

Tigania Isiolo Chuka Chogoria Mana Nkubu Meru Scheme 3,223 0 1,314 35,053 3,578 12,482 1,845 1989 Urban population 0% 100% 5% 52% 67% 6% 28% 1989 % urban 13,344 0 3,653 34,785 14,936 9,969 2010 Urban population 97,743 0% 100% 23% 8%39% 64% 80% 2010 % urban Number of households reporting diseases in last year: (100 household sample in each scheme) 96 51 81 62 85 78 Malaria 76 3 2 5 25 5 13 4 Typhoid 4 2 0 0 5 3 0Cholera 8 5 6 0 13 3 1 Dysentery 50 66 58 66 44 46 40 Others

Table H-1 Urban Population and Disease Incidence

1.2 Sewerage

Only the centres of Meru and Isiolo have waterborne sewerage systems. In both cases they were intended to serve approximately 30% of the urban population, and discharge to waste



stabilization pond systems. The systems were surveyed during the first phase of this study by the Jomo Kenyatta University of Agriculture and Technology. The following is a summary of their main findings.

1.2.1 Meru Sewerage

The sewerage system, constructed in 1974 to serve a population of 20,000, serves only the central part of the town, with a total length of sewers of 7.7 km, discharging an average total flow of 270 m3/day to a treatment plant located just to the east of the market area. The sewerage system, and treatment plant is managed by Meru Municipal Council.

A dairy plant located about 1 km to the south west, pre-treats its effluent of about 60 m3/day before pumping it to the municipal treatment plant.

No design details exist for the Municipal treatment plant. It consists of inlet works, a single waste stabilization pond and an irrigation/soakage system. However, only the pond itself is operational.

The inlet screens to the pond have been removed, and the grit chamber is silted up and is not functioning. The pond itself is oval in shape, with a baffle wall located along its central axis and an estimated volume of 8,000 m3. It was last desludged in 1991.

The outlet passes through what was an irrigation/soakage system, but which has not been used for many years. The outlet from this area was blocked, with effluent overflowing into an open ditch running down the side of Moi Road

The site was unfenced, with access to children and livestock being unrestricted. The area to the north of the pond was, in addition, being used as a solid waste dump.

Average untreated, and treated effluent quality is given in *Table H-2*:

Table H-2 Influent and effluent water volumes and quality - Meru

SHEEL STORY I WAS AN AREA STORY OF THE STORY	Influent			Effluent		
	Max	Min	Average	Max	Min	Average
Flow (m3/d)	1057	19	270	191	49	127
BOD (mg/l)	725	375	502	250	96	160
Suspended Solids (mg/l)	685	445	621	309	30	197

The sewerage system is managed by the Municipal Council of Meru. The sewerage section comes under the responsibility of the Town Engineer, and is headed by a foreman. An





operator inspects daily operations overseen on site by charge hands. There is nobody assigned permanently to the treatment plant, and nobody carries out routine operation and maintenance at the ponds however.

There are no records kept of sewage flows, and there is no local capability for water quality analysis.

1.2.2 Isiolo Sewerage

The sewerage system, constructed in 1984, serves 30% of the population living in the central part of the town, with a total length of sewers of 13.4 km, discharging an average total flow of 630 m3/day to a treatment plant located to the north of the town. The sewerage system, and treatment plant is managed by Isiolo Municipal Council.

The treatment plant consists of inlet works, three anaerobic ponds, one facultative and two maturation ponds. The solid waste dump for Isiolo is located on the same plot and wind blown paper covers the site, including the ponds.

One of the anaerobic ponds has become silted up and is due for emptying.

Average untreated, and treated effluent quality is given in *Table H-3*:

Effluent Influent Average Min Average Max Min Max 330 447 Flow (m3/d) 1125 353 630 563 251 755 426 605 200 118 BOD (mg/l) 930 383 653 343 47 170 Suspended Solids (mg/l)

Table H-3 Influent and effluent water volumes and quality - Isiolo

The raw sewage thus appears to be very high strength, and represents about 12.5% of the volume of water supplied to the town.

It should be noted however, that the above analyses were conducted on unfiltered samples. It has been estimated, (Mara, 1992) that between 70 to 90 % of BOD in unfiltered samples of effluent from maturation ponds is due to the algae which will be present. Taking this into account therefore, the above results, while still not acceptable, are not as bad as they initially look. The poor performance may be due to the reduced retention time available in the ponds due to the accumulation of sediment over time.



The waste stabilization ponds are managed under the Works Department of Isiolo Municipal Council, which has a total establishment of 15. Eight attendants are responsible for maintaining the waste stabilization pond and solid waste dump site. However the personnel are not trained, and their understanding of the requirements of their job would appear to be limited.

There are no records kept of sewage flows, and there is no local capability for water quality analysis.

2. FUTURE PROGNOSIS

The provision of increased quantities of water under this project will, at the same time, increase the quantities of wastewater to be disposed of, and will therefore increase the risk of an adverse impact on the environmental health conditions in the areas.

Most consumers will however be living in rural areas, where any impact can be mitigated by simple, well proven, cost effective methods, such as pit latrines for households without individual connections or, septic tanks and soakaways, for houses with individual connections. The main requirement to mitigate any adverse impact is therefore to ensure that households are well aware of the risks to health of poorly constructed and maintained facilities, and to ensure that households have adequate facilities before a water connection is made.

The main problem foreseen is in the urban areas which, as can be seen from Table "Urban Population and Disease Incidence", are projected to be increasing at a higher rate than the rural areas, and that by the year 2010, all the urban centres are predicted to become quite sizable towns.

The provision of sewerage facilities to these towns is beyond the scope of the present study, but it is clear that it will be necessary to investigate appropriate alternatives, study affordability issues and, to assess the acceptable housing density for different on-site facilities in different geological and soil conditions.

Nevertheless it is seen that it will be difficult to avoid providing sewerage systems to serve the central commercial and institutional areas of these towns, and, in the case of Isiolo and Meru, to expanding the existing system coverage. All these systems will require wastewater treatment facilities.







3. CONSTRUCTION OF PILOT WASTEWATER TREATMENT PLANT IN MERU

In order to assess the performance of extensions to the existing waste stabilization ponds at Meru, a pilot plant was designed and constructed. Fencing of the entire site was also included for safety and security purposes.

With minor improvements to the inlet and outlet arrangements, the plant was designed to conform with the 1992 longer term proposals of Wanjohi Consulting Engineers prepared on behalf of the Ministry of Local Government. The pilot plant can therefore form part of the long term proposals for wastewater treatment.

After analysis of tenders for the two construction and supervision components of the project, the construction contract was let to Njiru Builders and the contract for construction supervision was let to Mangat I.B. Patel and Partners.

The construction program required commencement by the beginning of September and completion by the middle of October. Considering the extent of the works, this was a fairly ambitious schedule, but was required in order to commence the sampling program as soon as possible, and in order to complete the works prior to the onset of the rainy season which usually occurs in mid October. The works were substantially complete by mid October, and the wastewater was diverted into the new pond on October 24th.

A program of sampling and analysis is on-going. Following the satisfactory completion of this program, appropriate design parameters will be established for the future extensions of the plant.

4. MONITORING OF PILOT WASTEWATER TREATMENT PLANT IN MERU

The sewerage system for Meru Town was constructed in 1974 with no extensions to the system since then despite a large increase in wastewater quantities due to the rapid growth rate of the Town. The wastewater treatment plant is indicated of *Figure H-5*. It consists of one waste stabilization pond of 7,550 m3 capacity (Wanjohi, 1992), which is insufficient to provide an acceptable effluent quality.

In response to the Scope of Work for the Study, a pilot pond was constructed of 2,000 m³ capacity during the first field work period of the Study Team, as indicated on *Figure H*-5. This was monitored from December 1996 to February, 1997 by the Jomo Kenyatta University of Agriculture and Technology in order to define design criteria for extension of



the pond system.

The pilot pond was designed so that it could be incorporated into the extended system by forming part of the second pond in accordance with preliminary designs prepared by Wanjohi for the Meru Town Council in 1992. Accordingly the pond was 1.75m deep, 39m long and 22.5 m wide at the bottom, with side slopes of 1:2.5.

Wastewater was first diverted into the new pond on October 24th. However, due to blockages in the system, flow rates were very low, and by the commencement of monitoring on 5th December, inflow to the old pond was at 295 m3/day and overflow from the new pond was still at a very low level of only 12 m3/d.

Flow measurements were taken at hourly intervals at the inlet and outlet of the pond system on nine separate days. During the days on which monitoring was conducted, inflows varied widely from 295 m3/day to 1,700 m3/day, averaging at about 1,000 m3/d. The outlet from the new pond however, was more constant, at an average level of 430 m³/day, with a maximum and minimum flow of 478 and 340 m³/day respectively. This suggests a reduction in flow which is far higher than can be attributed to evaporation (estimated at approximately 30 m3/day). Although there would be initial seepage losses from the new pond, there was no evidence that this was significant after the initial period of pond filling. The explanation is probably due to the high level of fluctuations of the inflow. and that the days during which measurements took place (which were not consecutive days), coincided with days of higher than average inflows. Due to the flow attenuation provided by the storage in the pond system, the outflow from the new pond is likely to provide a far more accurate indication of the average flow conditions during this period. Therefore, assuming that there are no other significant losses, the average inflow into the system over the period would have been equal to the average outflow, plus an allowance for evaporation, which comes to approximately 460 m3/day.

A total or 84 samples were analysed for 12 constituents. Samples were taken at 8-00, 12-00, 16-00 and 20-00 hours on nine different days at the inlet to the old works and at the outlet to the new pond, and on three days at the inlet to the new pond.

The results for the first day of monitoring has been taken as being unrepresentative due to the low flows, and the TSS and Faecal Coliform counts for February 26th appear to be anomalous, and have not been included in *Table H-4*:

Table H-4 Summarised, Average Results of Wastewater Monitoring at Meru

Parameter	Unit	Inict to old pond	Inlet to new pond	Outlet from new pond	% reduction in old pond	% reduction in new pond
Flow	m3/d	1011	512	430	49%	16%
Adjusted flow						
(1)	m3/d	460	440	430	4%	2%
BOD	mg/l	563	185	124	67%	33%
TSS	mg/l	536	201	114	68%	43% (2)
EC	Nr/100ml	812	175	104	81%	41% (2)

Notes: (1) Inflow adjusted due to high level of variations, by adding an allowance to the outfall flows.

(2) Omitting results for 26/2/97 which were anomalous

The results indicate that, considering the overloaded state of the ponds, their performance, apart from FC removal, appears to be creditable. The FC removal was much lower than what would normally be expected, due to the unusually low levels recorded at the inlet to the ponds.

The average loading rate for the initial pond comes to approximately 1,300 kg BOD/ha/d at 1,010 m3/d, and 600 kg BOD/ha/d at 460 m3/d. This has to be compared with the normal recommended design loading range for facultative ponds of between 100 and 400 kg/ha/d. The pond is thus severely overloaded and, especially at the inlet zone of the pond, it will be tending to act as an Anaerobic pond, which are usually loaded at a rate of over 3,000 kg/ha/d. Improved treatment could be gained by constructing a proper anaerobic pond about 3 m deep upstream of the original pond, with a retention capacity of about 1 day. This would have the effect of reducing the loading rate on the original pond to close to the usually accepted design loading rates for facultative ponds.

Although the monitoring results are useful to give a good indication of the extent of overloading and treatment efficiency, a number of factors combined to reduce to benefits of the monitoring exercise of the pilot pond:

Daily flows varied from 300 to 1,700 m3/day due to blockages in the reticulation system. The system was not therefore in equilibrium.

- The original intention of the monitoring exercise was to load the new pond at different levels to determine its efficiency at different loading rates. However the fact that the flows varied by such a high degree made this impossible to achieve.
- Pond systems require time for the necessary microbial populations to build up to effect waste stabilisation. The low flows during filling the pond, and during the initial monitoring period will have hindered this process.
- The faecal coliform counts of both influent and effluent were far lower than for



normal sewage flows, indicating either high levels of dilution or errors in collection, transportation or analysis. Whatever the reasons, it means that the results can not be used to estimate the first order reaction constant for the die-off rate for E-Coli.

Pond effluents contain large concentrations of algae which are essential for the treatment processes and which are not necessarily detrimental to the downstream eco-system. To determine the efficiency of removal of soluble BOD it is necessary to analyse filtered effluent samples. As the analysis was conducted on un-filtered samples, the actual soluble BOD removal efficiency will be considerably higher than indicated by the results.

5. CONCLUSIONS

The existing pond system, even with the added pilot pond is, totally overloaded and can not produce effluent of acceptable quality.

The system urgently needs to be extended to provide effluent in accordance with normal standards for discharge to inland water courses as shown in *Table H-5*:

Table H-5 Recommended Effluent Standards

Parameter	Recommended Effluent Standards*
BOD ₅ (MG/L)	<20
E.Coli (Nr/100 ml)	<3000
Total Suspended Solids (mg/l)	<20

^{*} Source: Water Pollution Control Office, Nairobi.

The location of the existing ponds however, is unacceptable for long term planning, as it lies adjacent to commercial and domestic housing, and is at a higher elevation to much of the Town, which can not therefore be connected without pumping. Long term planning should provide for gravity flow from the whole of the town, with a treatment plant suitably located away from any areas of housing density.

The results of the monitoring exercise generally support the use of existing design criteria for waste stabilization ponds as proposed by Mara. (Waste Stabilization Ponds - A design manual for Eastern Africa; ODA; 1992). The use of which is proposed for the design of a new pond system, or extensions to the existing system.

For illustrative purposes, a preliminary design of a waste stabilization system has been prepared based on the above design recommendations, and the assumptions as shown in *Table H-6*:







Table H-6 Wastewater Treatment Design Assumptions

Criteria	Design Assumption	Unit				
Design population	100,000					
Design flow rate	10,000	m3/d				
BOD of raw sewage	563	mg/l				
FC content of raw sewage	100,000,000	per 100 ml				
Required effluent BOD	20	mg/l (filtered sample)				
Required effluent FC	1,000	per 100 ml				
Mean temp of coldest month	15	deg C				
Nr of parallel Anaerobic ponds	6					
Nr of parallel Facultative ponds	3					
Nr of parolee Maturation ponds	3					
Anaerobic loading rate	200	g/m3/d				
Facultative loading rate	167	kg/ha/d				
1st Maturation pond loading	125	kg/ha/d				
FC removal rate	1.09	d-1				

The preliminary design uses an equivalent domestic population of 100,000. Before finalisation of the design it will be necessary to undertake a consumer survey of the population to determine the likely affordability of households, and to determine the extent of contributions from institutions, industry and commerce. The results of the design exercise are summarised in *Table H-7*:

Table H-7 Outline Design of Future Waste Stabilization Ponds for Meru

Pond	Depth	Total volume (m3)	Total Retention (days)	Operational ponds	Length of single pond (2) (m)	Breadth of single pond (2) (m)
Anaerobic	3.0	28,150	2.8	6	40	40
Facultative	1.5	253,300	26.7	3	336	169
1st Maturation	1.5	175,000	20.3	3	279	139
2nd Maturation	1.5	37,325	4.5	3	129	64
3rd Maturation	1.5	37,325	4.5	3	129	64

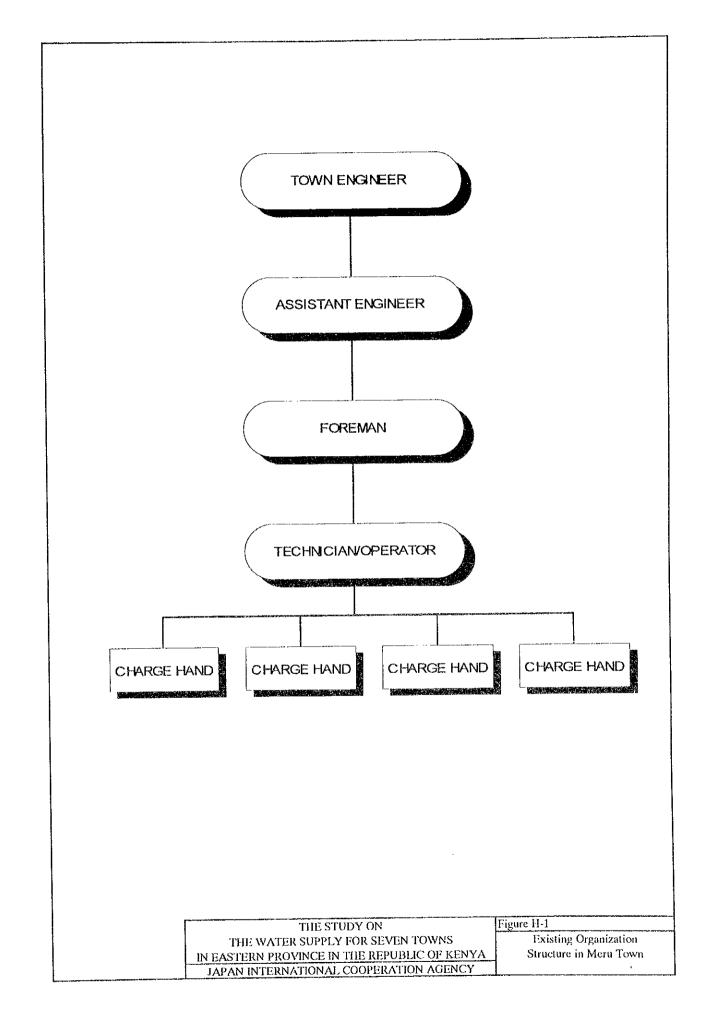
Note: (1) Total number of anaerobic ponds to be increased to 9, to allow for annual desludging. (2) Mid depth dimensions

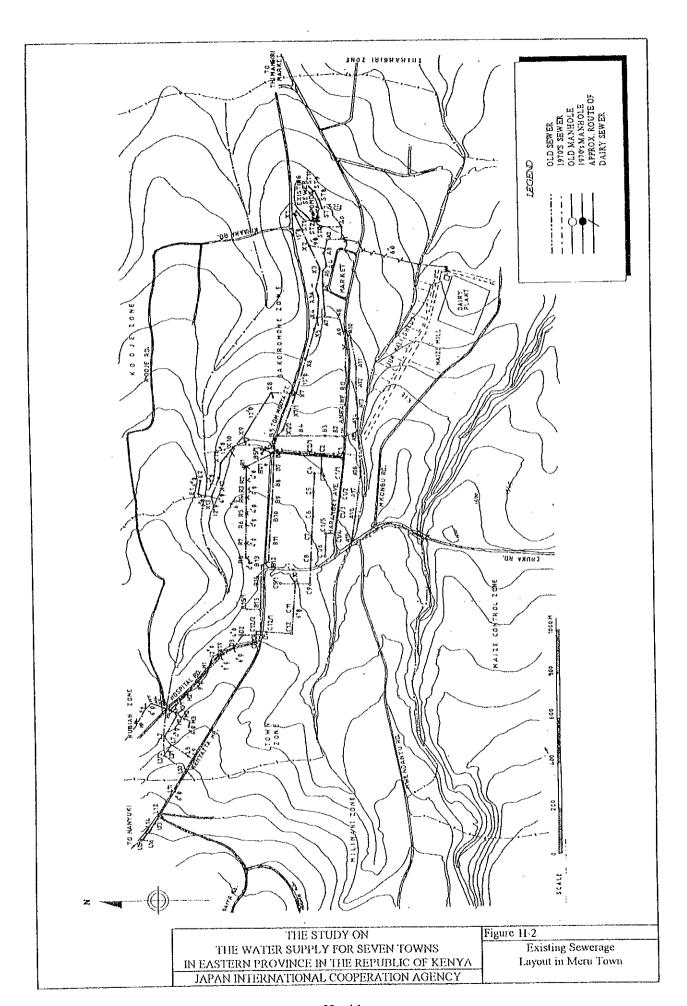
A conceptual general layout plan for the pond system is indicated on *Figure H-6*. This indicates the requirement for a site measuring approximately 1,000m x 650 m, and clearly illustrates that the land available at the existing wastewater treatment plant site is totally inadequate.

The acquisition of a suitable piece of land for the construction of a wastewater treatment plant is a priority.

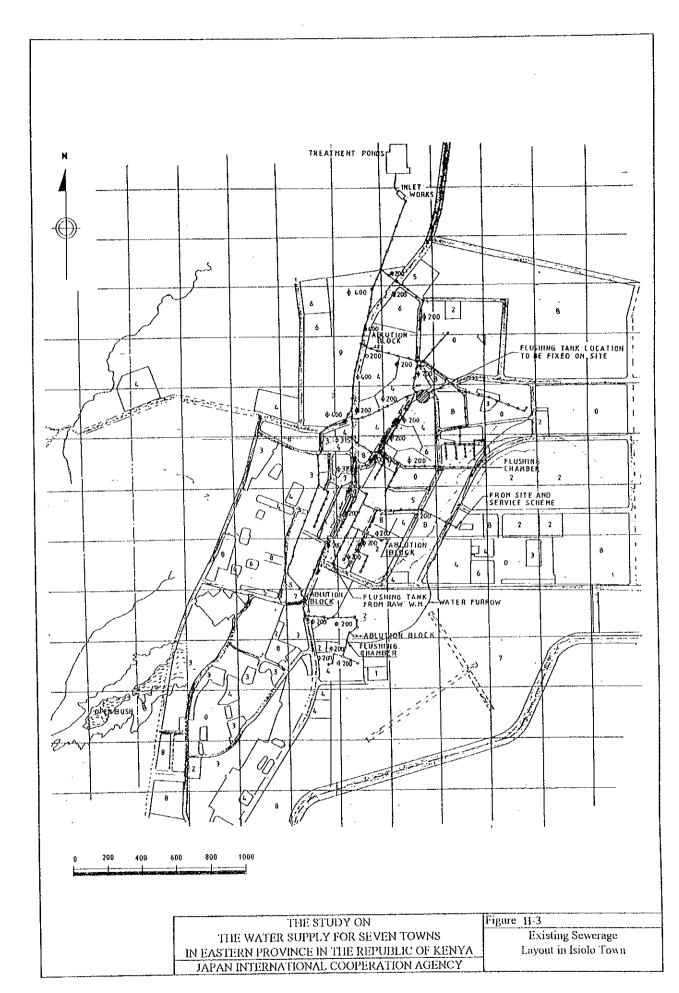


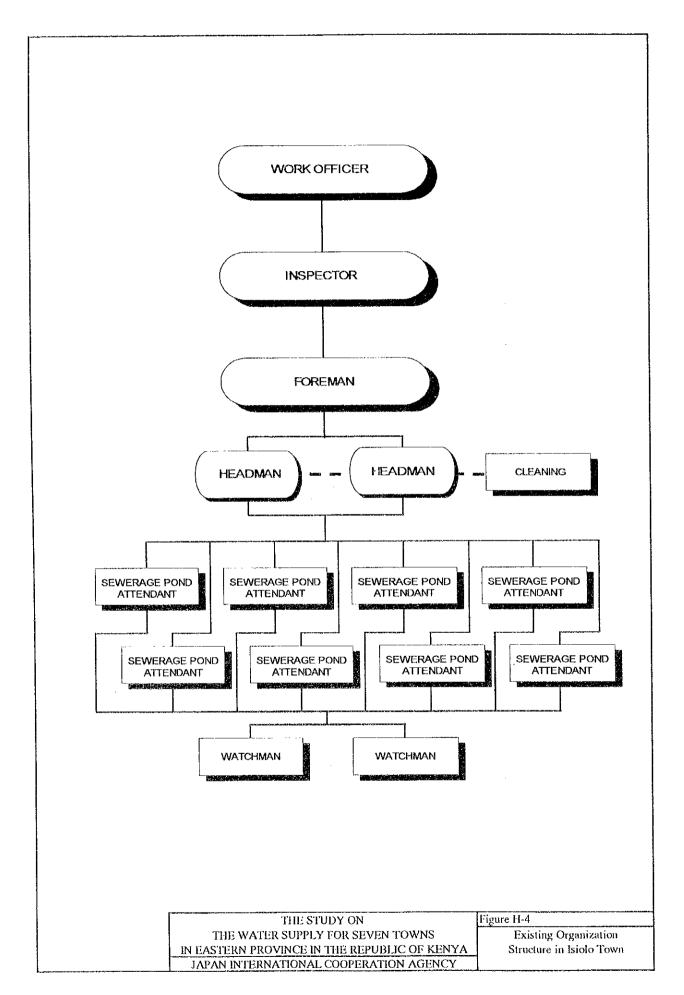
FIGURES

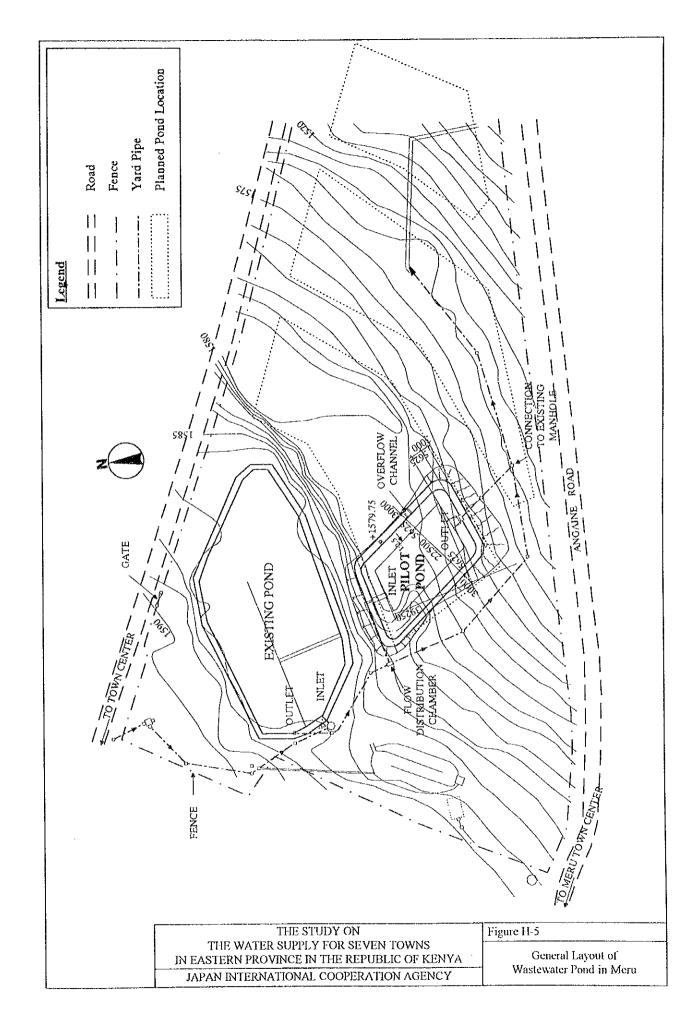




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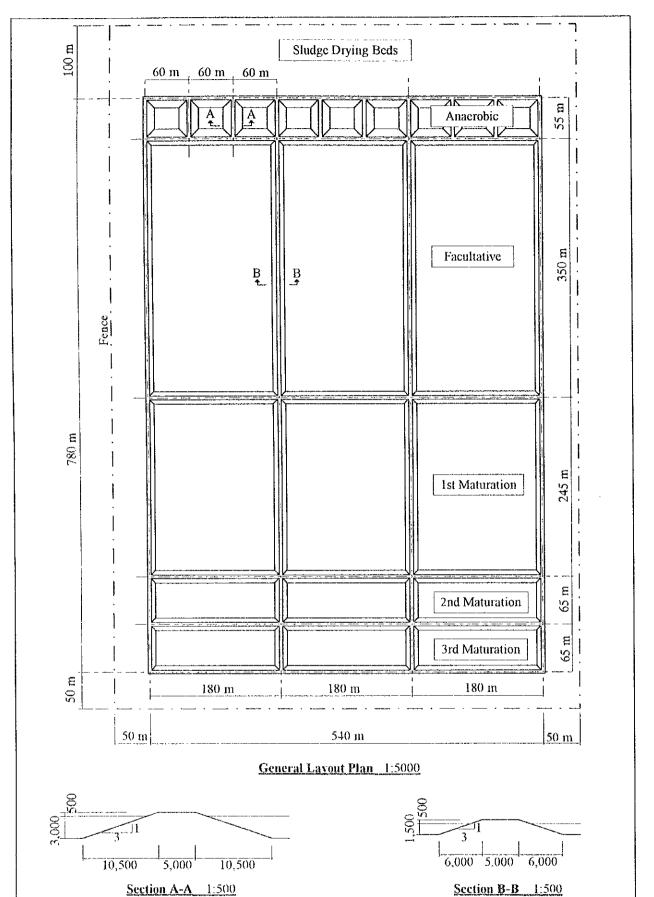




Figure H-6

General Layout for Future Meru

Waste Stabilization Ponds

THE STUDY ON

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JAPAN INTERNATIONAL COOPERATION AGENCY

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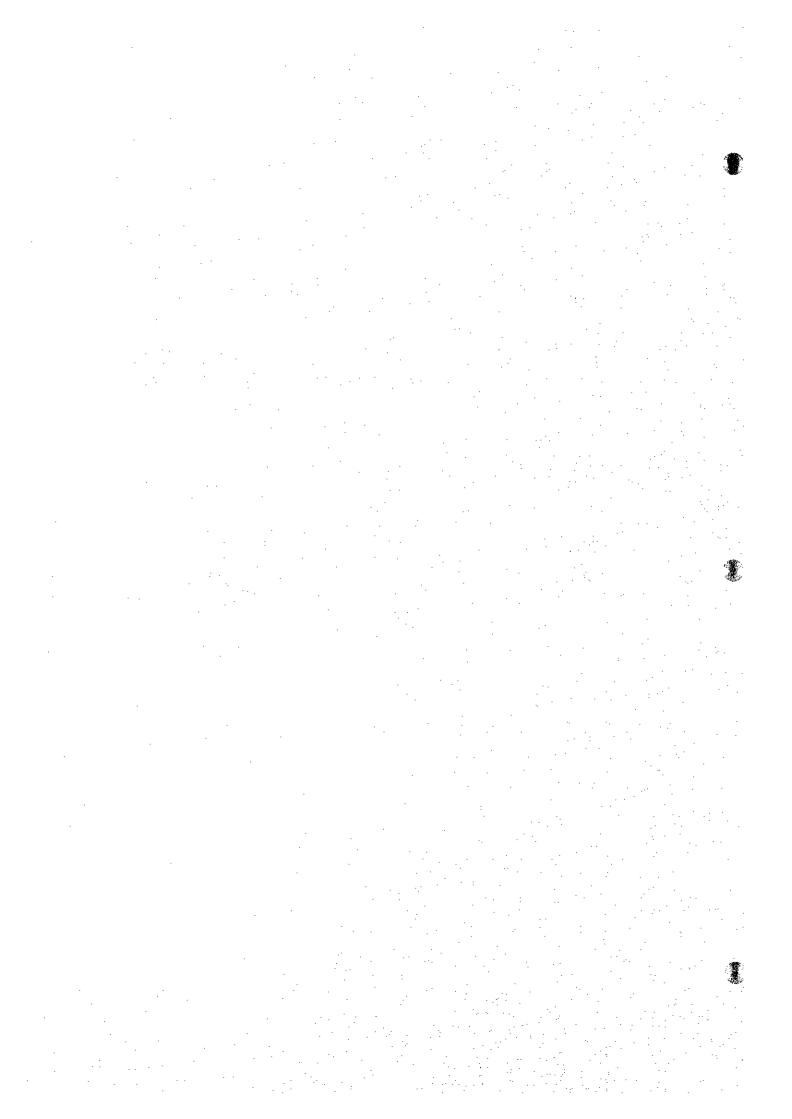


Table H-1 Flow measurement result in Meru (1/4)

Location Base reading = Cd average =	Inlet 0 c 0.5135	em			date : Sep/5/'96
Time	Reading	Flow Height	Discharge	Discharge	Discharge
	cm	m	m3/sec	lit./sec	m3/hr
8;00	11.50	0.115	0.00544	5.44	19.58
9:00	10.60	0.106	0.00444	4.44	15.98
10:00	10.00	0.100	0.00384	3.84	13.82
11:00	9.00	0.090	0.00295	2.95	10.62
12:00	12.30	0.123	0.00644	6.44	23.18
13:00	11.40	0.114	0.00532	5.32	19.15
14:00	11.50	0.115	0.00544	5.44	19.58
15:00	10.90	0.109	0.00476	4.76	17.14
16:00	8.90	0.089	0.00287	2.87	10.33
17:00	10.70	0.107	0.00454	4.54	16.34
18:00	10.00	0.100	0.00384	3.84	13.82
19:00	9.30	0.093	0.00320	3.20	11.52
20:00	7.90	0.079	0.00213	2.13	7.67
21:00	8.00	0.080	0.00220	2.20	7.92
22:00	7.80	0.078	0.00206	2.06	7.42
23:00	7.10	0.071	0.00163	1.63	5.87
0:00	6.50	0.065	0.00131	1.31	4.72
1:00	5,90	0.059	0.00103	1.03	3.71
2:00	5.60	0.056	0.00090	0.90	3.24
3:00	6.10	0.061	0.00111	1.11	4,00
4:00	5.40	0.054	0.00022	0.22	0.79
5:00	5.60	0.056	0.00090	0.90	3.24
6:00	9.20	0.092	0.00311	3.11	11.20
7:00	11.00	0.110	0.00487	4.87	17.53

Table H-1 Flow measurement result in Meru (2/4)

Total

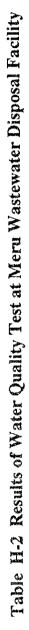
Location	Outlet				
Base reading =	0.27	cm			date : Sep/5/'96
Cd average ≠	0.8462				····
Time	Reading	Flow Height	Discharge	Discharge	Discharge
	cm	m	m3/sec	lit./sec	m3/hr
8:00	4.51	0.042	0.00074	0.74	2.66
9:00	5.53	0.053	0.00137	1.37	4.93
10:00	6.30	0.060	0.00178	1.78	6.41
11:00	6.09	0.058	0.00163	1.63	5.87
12:00	6.76	0.065	0.00215	2.15	7.74
13:00	6.57	0.063	0.00200	2.00	7.20
14:00	6.40	0.061	0.00186	1.86	6.70
15:00	6.43	0.062	0.00188	1.88	6.77
16:00	6.46	0.062	0.00191	1.91	6.88
17:00	6.74	0.065	0.00213	2.13	7.67
18:00	6.79	0.065	0.00217	2.17	7.81
19:00	6.85	0.066	0.00222	2.22	7.99
20:00	5.67	0.054	0.00135	1.35	4.86
21:00	6.10	0.058	0.00164	1.64	5.90
22:00	6.27	0.060	0.00176	1.76	6.34
23:00	5.67	0.054	0.00135	1.35	4.86
0:00	5.89	0.056	0.00150	1.50	5.40
1:00	5.63	0.054	0.00133	1,33	4.79
2:00	5.09	0.048	0.00102	1.02	3.67
3:00	4.89	0.046	0.00092	0.92	3.31
4:00	4.59	0.043	0.00078	0.78	2.81
5:00	4.37	0.041	0.00068	0.68	2.45
6:00	4.08	0.038	0.00057	0.57	2.05
7:00	4.19	0.039	18000.0	0.61	2.20
Total					127.26

Location	Inlet				
Base reading =	0 (em		(late: Sep/12/'96
Cd average =	0.5135				
Time	Reading	Flow Height	Discharge	Discharge	Discharge
	cm	m	m3/sec	lit./sec	m3/hr
8:00	10.90	0.109	0.00476	4.76	17.14
9:00	11.50	0.115	0.00544	5.44	19.58
10:00	11.20	0.112	0.00509	5.09	18.32
11:00	10.20	0.102	0.00403	4.03	14.51
12:00	10.80	0.108	0.00465	4.65	16.74
13:00	9.20	0.092	0.00311	3.11	11.20
14:00	10.00	0.100	0.00384	3.84	13.82
15:00	9.80	0.098	0.00365	3.65	13.14
16:00	13.20	0.132	0.00768	7.68	27.65
17:00	15.90	0.159	0.01223	12.23	44.03
18:00	8.20	0.082	0.00234	2.34	8.42
19:00	9.00	0.090	0.00295	2.95	10.62
20:00	8.10	0.081	0.00227	2.27	8.17
21:00	7.40	0.074	0.00181	1.81	6.52
22:00	8.20	0.082	0.00234	2.34	8.42
23:00	6.10	0.061	0.00111	1.11	4.00
0:00	6.40	0.064	0.00126	1.26	4.54
1:00	6.30	0.063	0.00121	1.21	4.36
2:00	6.00	0.060	0.00107	1.07	3.85
3:00	5.90	0.059	0.00103	1.03	3.71
4:00	5.90	0.059	0.00103	1.03	3.71
5:00	6.00	0.060	0.00107	1.07	3.85
6:00	6.00	0.060	0.00107	1.07	3.85
7:00	8.80	0.088	0.00279	2.79	10.04
Total					280.19

Table H-1 Flow measurement result in Meru (4/4)

Location	Outlet				
Base reading =	0.27	cm			
Cd average =	0.8462				
Time	Reading	Flow Height	Discharge	Discharge	Discharge
	cm	m	m3/sec	lit./sec	m3/hr
8:00	4.60	0.043	0.00078	0.78	2.81
9:00	5.29	0.050	0.00113	1.13	4.07
10:00	5.25	0.050	0.00111	1.11	4.00
11:00	5.25	0.050	0.00111	1.11	4.00
12:00	5.72	0.055	0.00139	1.39	5.00
13:00	5.91	0.056	0.00151	1.51	5.44
14:00	5.79	0.055	0.00143	1.43	5.15
15:00	5.68	0.054	0.00136	1.36	4.90
16:00	5.97	0.057	0,00155	1.55	5,58
17:00	5.75	0.055	0.00141	1.41	5.08
18:00	6.21	0.059	0.00172	1.72	6.19
19:00	6.04	0.058	0.00160	1.60	5.76
20:00	4.78	0.045	0.00086	0.86	3.10
21:00	5.65	0.054	0.00154	1.54	5.54
22:00	5.14	0.049	0.00105	1.05	3.78
23:00	5.36	0.051	0.00117	1.17	4.21
0:00	5.14	0.049	0.00105	1.05	3.78
1:00	4.87	0.046	0.00091	0.91	3.28
2:00	4.85	0.046	0.00090	0.90	3.24
3:00	4.68	0.044	0.00082	0.82	2.95
4:00	4.34	0.041	0.00067	0.67	2.41
5:00	4.27	0.040	0.00064	0.64	2.30
6:00	4.08	0.038	0.00057	0.57	2.05
7:00	4.13	0.039	0.00057	0.59	2.12
Total	4,13	0.057	0.00000		96.73
LOTAL					





					5th Sept /96	ot /96							12th Sept /96	pt /96			
PARAMETER	Unit		Inlet	i.			outlet	et			Inlet	ابيد			outlet	t e	
		8:00	12:00	16:00	20:00	8:00	12:00	16:00	20:00	8:00	12:00	16:00	20:00	8:00	12:00	16:00	20:00
Air Temperature	ЭC	18	29	27	21	18	29	25	20	#	18	25	19	11	18	25	6
Water Temperature) n	20	21	21	20	19	25	28	24	14	16	19	30	13	16	21	21
pří		7.7	7.8	7.9	7.8	7.3	8.2	8.5	8.3	8.0	7.0	8.2	7.9	7.0	7.2	7.9	8.1
вор	mg/l	530	725	375	515	146	151	137	146	496	493	463	470	96	128	250	225
Faecal coliform	No./100mil	800	spoilt	1200	1500	200	250	290	350	009	200	700	200	215	235	250	225
Nitrates	ng/l	0.12	0.09	0.35	0.19	0.42	0.31	0.75	0.55	Ë	ri u	n	Eju.	0.18	0.55	0.25	0.53
Nitrite	l/gim	0.32	0.77	1.00	0.70	0.11	0.36	0.02	0.01	lin Lin	lia	lin	lin	0.001	0.011	0.025	0.003
Ammonia	∫⁄giu	Įį a	fin	lin	liu	lin	0.02	[id	liu	lin	0.023	0.038	0.11	lin	lin	ni	iia
Sulfide	mg∕/	0.496	0.453	0.391	0.269	0.185	0.169	0.210	0.101	0.700	0.516	0.569	0.352	0.01	0.015	0.03	0.05
Phosphorus (anino- naphthol-sulfonic method)	mg/]	0.0325	0.0225	0.0375	0.0200	0.0150	0.0188	0.0100	0.0113	0.03	0.03	0.03	0.02	0.010	0.015	0.030	0.050
Phenois	നളവ	0.700	0.270	0.160	0.123	0.30	0.15	0.10	0.10	0.070	0.025	0.010	0.012	90.0	0.05	0.01	0.01
Phosphorus	ற்தி	0.03	0.02	0.04	0.02	0.015	0.020	0.010	0.010	0.0275	0.0225	0.0315	0.0195	0.0125	0.0135	0.0275	0.0450
Total suspended solids	mg/I	500	675	535	445	233	268	215	253	685	710	795	625	30	125	309	144

Table H-3 Flow mearurement result in Isiolo (1/4)

Location	Inlet				
Base reading =	40.7	em			date: Sep/5/'96
Cd average =	0.6545				
Time	Reading	Flow Height	Discharge	Discharge	Discharge
	cm	m	m3/sec	lit./sec	m3/hr
8:00	54.50	0.138	0.01094	10.94	39.38
9:00	54.30	0.136	0.01055	10.55	37.98
10:00	54.00	0.133	0.00997	9.97	35.89
11:00	54.00	0.133	0.00997	9.97	35.89
12:00	53.20	0.125	0.00854	8.54	30.74
13:00	52.60	0.119	0.00755	7.55	27.18
14:00	53,00	0.123	0.00820	8.20	29.52
15:00	52.90	0.122	0.00804	8.04	28.94
16:00	52.50	0.118	0.00740	7.40	26.64
17:00	54.50	0.138	0.01094	10.94	39,38
18:00	50.40	0.097	0.00453	4.53	16.31
19:00	51.10	0.104	0.00539	5.39	19.40
20:00	52.00	0.113	0.00664	6.64	23.90
21:00	52.50	0.118	0.00740	7.40	26.64
22:00	53.40	0.127	0.00890	8.90	32.04
23:00	52.30	0.116	0.00710	7.10	25.56
0:00	52.30	0.116	0.00710	7.10	25.56
1:00	51.80	0.111	0.00635	6.35	22.86
2:00	51.70	0.110	0.00621	6.21	22.34
3:00	50.00	0.109	0.00607	6.07	21.83
4:00	51.80	0.093	0.00410	4.10	14.76
5:00	51.80	0.111	0.00635	6.35	22.86
6:00	51.70	0.110	0.00621	6.21	22,34
7:00	52.50	0.118	0.00740	7.40	26.64
Total					654.61

Table H-3 Flow mearurement result in Isiolo (2/4)

Location Base reading =	Outlet 0.8 c	em			date : Sep/5/'96
Cd average =	0.4				•
Time	Reading	Flow Height	Discharge	Discharge	Discharge
	em	m	m3/sec	lit./sec	m3/hr
8:00	16.80	0.160	0.00472	4.72	16.99
9:00	16.30	0.155	0.00436	4.36	15.70
10:00	14.80	0.140	0.00338	3.38	12.17
11:00	16.80	0.160	0.00472	4.72	16.99
12:00	17.20	0.164	0.00502	5.02	18.07
13:00	15.50	0.147	0.00382	3.82	13.75
14:00	17.40	0.166	0.00518	5.18	18.65
15:00	17.30	0.165	0.00510	5.10	18.36
16:00	17.30	0.165	0.00510	5.10	18.36
17:00	17.40	0.166	0.00518	5.18	18.65
18:00	17.50	0.167	0.00525	5.25	18.90
19:00	18.00	0.172	0.00566	5.66	20.38
20:00	18.40	0.176	0.00600	6.00	21.60
21:00	18.50	0.177	0.00608	6.08	21.89
22:00	18.40	0.176	0.00600	6.00	21.60
23:00	18.40	0.176	0.00600	6.00	21.60
0:00	18.30	0.175	0.00591	5.91	21.28
1:00	18.30	0.175	0.00591	5.91	21.28
2:00	18.40	0.176	0.00600	6.00	21.60
3:00	18.30	0.175	0.00591	5.91	21.28
4:00	17.40	0.166	0.00518	5.18	18.65
5:00	17.40	0.166	0.00518	5.18	18.65
6:00	18.50	0.177	0.00608	6.08	21.89
7:00	19.00	0.182	0.00652	6.52	23.45
Total					461.72

Table H-3 Flow mearurement result in Isiolo (3/4)

Location	Inlet	0 #2#0/
Base reading =	0 cm	date: Sep/12/'96
Cd average =	0.5135	

Cd average =	0.5135				
Time	Reading	Flow Height	Discharge	Discharge	Discharge
	cm	m	m3/sec	lit./sec	m3/hr
8:00	53.60	0.129	0.00924	9.24	33.26
9:00	55.50	0.148	0.01303	13.03	46.91
10:00		0.127	0.00880	8.80	31.68
11:00		0.121	0.00780	7.80	28.08
12:00		0.121	0.00780	7.80	28.08
13:00		0.128	0.00906	9.06	32.62
14:00		0.132	0.00980	9.80	35.28
15:00		0.116	0.00710	7.10	25.56
16:00		0.101	0.00501	5.01	18.04
17:00		0.101	0.00501	5.01	18.04
18:00		0.116	0.00709	7.09	25.52
19:00		0.113	0.00664	6.64	23.90
20:00		0.111	0.00635	6.35	22.86
21:00		0.123	0.00820	8.20	29.53
22:00		0.113	0.00664	6.64	23.90
23:00		0.121	0.00787	7.87	28.33
0:00		0.106	0,00566	5.66	20.38
1:00		0.108	0.00593	5.93	21.35
2:00		0.093	0.00408	4.08	14.69
3:00	and the second s	0.092	0.00397	3.97	14.29
4:00		0.093	0.00408	4.08	14.69
5:00		0.093	0.00408	4.08	14.69
6:00	-	0.107	0.00579	5.79	20.84
7:00		0.130	0.00942	9.42	33.91
Total					606.43

Table H-3 Flow mearurement result in Isiolo (4/4)

Location Base reading =	Outlet 0,27 c 0,8462	em			
Cd average = Time	Reading	Flow Height	Discharge	Discharge	Discharge
) time	cm	m	m3/sec	lit./sec	m3/hr
8:00	16.10	0.153	0.00422	4.22	15.19
9:00	16.20	0.154	0.00429	4.29	15.44
10:00	16.00	0.152	0.00415	4.15	14.94
11:00	15.90	0.151	0.00408	4.08	14.69
12:00	16.10	0.153	0.00422	4.22	15.19
13:00	16.20	0.154	0.00429	4.29	15.44
14:00	16.50	0.157	0.00450	4.50	16.20
15:00	16.50	0.157	0.00450	4.50	16.20
16:00	18.80	0.180	0.00634	6.34	22.82
17:00	18.30	0.175	0.00591	5.91	21.28
18:00	17.70	0.169	0.00541	5.41	19.48
19:00	17.60	0.168	0.00533	5.33	19.19
20:00	17.50	0.167	0.00525	5.25	18.90
21:00	18.00	0.172	0.00566	5.66	20.38
22:00	17.40	0.166	0.00518	5.18	18.65
23:00	17.60	0.168	0.00533	5.33	19.19
0:00	17.50	0.167	0.00525	5.25	18.90
1:00	17.40	0.166	0.00518	5.18	18.65
2:00	17.40	0.166	0.00518	5.18	18.65
3:00	16.50	0.157	0.00450	4.50	16.20
4:00	16.80	0.160	0.00472	4.72	16.99
5:00	17.60	0.168	0.00533	5,33	19.19
6:00	17.80	0.170	0.00549	5.49	19.76
7:00	18.20	0.174	0.00582	5.82	20.95
Total	10.50				432.47
17/101					

Table H-4 Results of Water Quality Test at Isiolo Wastewaater Disposal Facility

					5th Sept /96	96/ 1d							12th Sept /96	pt /96			
PARAMETER	Unit		Inlet	et			outlet	et			Inlet				outlet	et	
		8:00	12:00	16:00	20:00	8:00	12:00	16:00	20:00	8:00	12:00	16:00	20:00	8:00	12:00	16:00	20:00
Air Temperature	л С	21	35	4.	27	21	35	£	27	21	32	59	56	21	32	53	79
Water Temperature) C	18	25	25	23	18	23	24	24	24	26	25	25	16	22	25	23
Нd		8.0	7.7	7.5	7.6	8.5	8.5	0.6	8.5	8.0	7.5	7.9	7.5	∞ ₁Ú	8.5	8.9	8.5
ВОО	mg/l	755	775	069	725	156	130	118	125	522	426	491	456	153	169	200	160
Faecal coliform	No./100ml	300	450	550	9009	175	150	225	250	550	450	200	300	100	160	200	160
Nitrates	mg//	0.38	0.42	0.10	0.07	0.20	0.16	0.15	0.17	nii	0.01	0.03	0.03	0.43	0.13	0.20	lin
Nitrite	mg/l	0.16	0.28	0.55	0.07	1.47	2.18	0.15	05.0	0.01	0.03	lin	Lin.	2.540	3.690	0.300	0.275
Ammonia	mg/l	Ţī.	0.005	lin	lia	iin	lin	liu	0.001	liu	Lin	liu	liu	niI	liu	lia	Lic
Sulfide	l/gin	0.970	0.764	0.679	0.678	0.205	0.198	0.134	0.184	0.533	0.436	0.620	0.734	0.36	0.392	0.38	0.29
Phosphorus (anmo- naphthol-sultonic method)	∏g/π	0.0275	0.0215	0.0525	0.0500	0.0250	0.0175	0.0180	0.020.0	0.02	0.02	0.02	0.02	0.025	0.023	0.016	0.020
Phenols	l/gm	0.600	0.250	0.150	0.100	0.02	0.02	0.02	0.02	0.550	0.350	0.250	0.300	0.50	0.40	0.30	0.30
Phosphorus	ng/l	0.03	0.02	0.04	0.04	0.020	0.015	0.020	0.020	0.0140	0.0200	0.0200	0.0200	0.0200	0.0200	0.0200	0.0200
Total suspended solids	l/gm	460	513	385	440	343	270	223	186	930	830	805	865	47	125	63	104

Manager (c)

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Table H-5 Flow Measurement Monitoring Result (1/6)

Unit: m3/hr

				Omi: m3/m
Date		5/'96	Dec/1	
Location [Old Pond Inlet	New Pond Outlet	Old Pond Inlet	New Pond Outlet
Base reading =	-	-	-	-
Cd average =	0.5135	0.402	0.5135	0.402
8:00	24.12	0.36	51.84	8.28
9:00	19.44	0.36	110.88	8.28
10:00	15.48	0.36	60.12	9.36
11:00	15.48	0.36	88.20	10.80
12:00	19.44	0.36	19.44	12.24
13:00	24.12	0.36	19.44	13.68
14:00	26.64	0.36	17.64	16.92
15:00	19.44	0.36	13.68	18.72
16:00	13.68	0.36	17.64	15.48
17:00	12.24	0.36	13.68	18.72
18:00	12.24	0.36	17.64	16.92
19:00	13.68	0.36	17.64	16.92
20:00	5.76	0.36	78.12	18.72
21:00	5.76	0.72	68.76	18.72
22:00	15.48	1.08	68.76	16.92
23:00	13.68	1.08	68.76	18.72
0:00	6.84	1.44	51.84	16.92
1:00	5.76	1.08	51.84	15.48
2:00	5.76	0.72	17.64	15.48
3:00	3.96	0.36	44.64	10.80
4:00	3.96	0.36	44.64	12.24
5:00		0.36	60.12	9.36
6:00	3.96	0.36	60.12	9.36
7:00	5.76	0.00	48.24	9.36
Total	295.20	12.24	1111.32	338.40

Table H-5 Flow Measurement Monitoring Result (2/6)

Unit: m3/hr

	Jan/.	2/'97	Jan/1	
Location	Old Pond Inlet	New Pond Outlet	Old Pond Inlet	New Pond Outlet
Base reading =	-	-	-	~
Cd average =	0.5135	0.402	0.5135	0.402
8:00	51.84	7.20	73.44	13.68
9:00	60.12	8.28	83.16	15,48
10:00		8.28	78.12	20.88
11:00		9.36	68.76	18.72
12:00	44.64	10.80	60.12	16.92
13:00	44.64	12.24	51.84	18.72.
14:00	60.12	15.48	44.64	20.88
15:00	44.64	13.68	38.16	25.20
16:00	44.64	15.48	44.64	25.20
17:00	48.24	16.92	41.40	20.88
18:00	44.64	16.92	38.16	20.88
19:00	44.64	18.72	44.64	25.20
20:00	44.64	18.72	51.84	27.36
21:00	48.24	20.88	44.64	20.88
22:00	44.64	43.92	44.64	18.72
23:00	44.64	43.92	44.64	16.92
0:00	44.64	37.80	38.16	16.92
1:00	55.80	37.80	38.16	16.92
2:00		16.92	38.16	16.92
3:00	41.40	18.72	38.16	16.92
4:00		16.92	32.04	16.92
5:00		13.68	38.16	13.68
6:00		10.80	44.64	16.92
7:00		10.80	51.84	16.92
Total	1086.84	444.24	1172.16	458.64



Table H-5 Flow Measurement Monitoring Result (3/6)

Unit: m3/hr

Date	Jan/2		Feb/1	
Location	Old Pond Inlet	New Pond Outlet	Old Pond Inlet	New Pond Outlet
Base reading =	-	-	-	-
Cd average =	0.5135	0.402	0.5135	0.402
8:00	78.12	16.92	68.76	12.24
9:00	78.12	20.88	78.12	13.68
10:00	73.44	25.20	83.16	15.48
11:00	68.76	20.88	60.12	18.72
12:00	68.76	16.92	51.84	16.92
13:00	78.12	25.20	51.84	16.92
14:00		25.20	44.64	23.04
15:00	78.12	23.04	44.64	25.20
16:00	78.12	16.92	41.40	25.20
17:00	68.76	13.68	38.16	23.04
18:00	68.76	16.92	44.64	20.88
19:00	78.12	20.88	41.40	25.20
20:00		25.20	38.16	23.04
21:00	88.20	20.88	38.16	20.88
22:00		18.72	44.64	23.04
23:00	64.08	16.92	51.84	16.92
0:00	51.84	13.68	44.64	20.88
1:00	51.84	13.68	44.64	25.20
2:00		10.80	41.40	25.20
3:00		10.80	44.64	20.88
4:00		8.28	38.16	18.72
5:00		8.28	38.16	16.92
6:00		12.24	38.16	13.68
7:00		13.68	38.16	16.92
Total	1708.92	415.80	1149.48	478.80

Table H-5 Flow Measurement Monitoring Result (4/6)

Unit: m3/hr

			Unit: m3/nr
		Feb/26/'97	
Location	Old Pond Inlet	New Pond Intlet	New Pond Outlet
Base reading =	-	-	-
Cd average =	0.5135	0.63	0.402
8:00	26.64	15.48	10.80
9:00	51.84	15.48	10.80
10:00	51.84	17.64	12.24
11:00	32.04	19.80	13.68
12:00	32.04	22.32	15.48
13:00	38.16	22.32	16.92
[14:00	26.64	22.32	18.72
15:00	26.64	19.80	20.88
16:00	29.16	22.32	20.88
17:00	32.04	22.32	23.04
18:00	13.68	22.32	25.20
19:00	15.48	17.64	16.92
20:00	13.68	22.32	16.92
21:00	17.64	22.32	18.72
22:00	21.60	22.32	20.88
23:00	26.64	22.32	20.88
0:00	38.16	17.64	23.04
1:00	44.64	17.64	20.88
2:00	32.04	17.64	16.92
3:00	26.64	17.64	18.72
4:00	13.68	15.48	16.92
5:00		15.48	13.68
6:00	21.60	15.48	13.68
7:00	0.00	15.48	13.68
Total	646.20	461.52	420.48







Table 5-5 Flow Measurement Monitoring Result (5/6)

Unit: m3/hr

		51.05105	Ont . m./m
		Feb/27/'97	
Location	Old Pond Inlet	New Pond Intlet	New Pond Outlet
Base reading =	-	•	-
Cd average =	0.5135	0.63	0.402
8:00	21.60	13.32	10.80
9:00	21.60	13.32	10.80
10:00	38.16	13.32	10.80
11:00	32.04	17.64	13.68
12:00	26.64	22.32	13.68
13:00	38.16	27.72	16.92
14:00	38.16	27.72	23.04
15:00	32.04	22.32	20.88
16:00	32.04	27.72	23.04
17:00	38.16	27.72	23.04
18:00	44.64	27.72	25.20
19:00	29.16	27.72	25.20
20:00	29.16	33.84	27.36
21:00	26.64	33,84	27.36
22:00	32.04	33.84	25.20
23:00	38.16	33.84	25.20
0:00	44.64	27.72	23.04
1:00	32.04	27.72	25.20
2:00	32.04	27.72	20.88
3:00	26.64	24.84	20.88
4:00	26.64	24.84	16.92
5:00		22.32	16.92
6:00	i	17.64	13.68
7:00	0.00	17.64	16.92
Total	726.12	594.36	476.64

Table H-5 Flow Measurement Monitoring Result (6/6)

Unit: m3/hr

		Feb/28/'97	Olik i mo/m
<u>.</u>	OU David Liles 1	New Pond Intlet	New Pond Outlet
Location	Old Pond Inlet	New Pond Inner	Mem Long Ogner
Base reading =		- 0.73	0.402
Cd average =	0.5135	0.63	
8:00	34.92	17.64	16.92
9:00	68.76	22.32	16.92
10:00	41.40	33.84	23.04
11:00	41.40	33.84	25.20
12:00	38.16	33.84	20.88
13:00	44.64	33.84	23.04
14:00	3.96	33.84	23.04
15:00	7.92	33.84	25.20
16:00	3.96	24.84	27.36
17:00	3.96	17.64	25.20
18:00	5.76	15.48	23.04
19:00	7.92	13.32	23.04
20:00	5.76	13.32	25.20
21:00	3.96	13.32	25.20
22:00	2.52	15.48	20.88
23:00	1.44	15.48	20.88
0:00		15.48	16.92
1:00		3.24	16.92
2:00	ł <u> </u>	13.32	15.48
3:00		11.52	13.68
4:00	•	11.52	10.80
5:00	2	8.64	8.28
6:00		8.64	6.12
7:00		17.64	1.08
Total	335.16	461.88	454.32

Table H-6 Results of Water Quality Monitoring at Meru Pilot Pond (1/6)

					5th De	e /96							19th D	ec /96			
PARAMETER	Unit		Old Por	nd Inlet			New Pon	d Outlet			Old Por	nd Inlet		n=-	New Pon	d Outlet	
		8:00	12:00	16:00	20:00	8:00	12:00	16:00	20:00	8:00	12:00	16:00	20:00	8:00	12:00	16:00	20:00
Air Temperature	∍c,	20	29	27	22	20	29	27	22	21	28	28	20	20	25	26	26
Water Temperature	2C	20	21	21	20	19	25	23	22	20	21	20	20	20	21	21	22
pH		7.7	7.8	7.8	7 .9	7.3	8.2	8.3	8.4	7.5	7.8	7.7	7.6	8.3	8.3	8.0	δ.3
BOD	mgl	500	700	450	480	130	150	120	120	525	650	525	500	126	160	140	135
Faccal coliform	No./100ml	850	1100	1200	1500	150	200	260	300	800	1000	900	850	135	220	240	250
Nitrates	mg/l	0.14	0.10	0.09	0.12	0.45	0.30	0.70	0.55	0.15	0.13	0.10	0.14	0.40	0.33	0.63	Ð.50
Nivite	mg/l	0.30	0.70	0.90	0.60	0.11	0.40	0.20	0.25	0,28	0.65	0.83	0.58	0.13	0.35	0.25	0.28
Ammonia	mg/l	3.50	3.85	3.75	4,00	2.75	3.00	2.50	3.25	3.25	3.50	3.50	3.75	3.75	2.50	2.75	2.86
Sulfide	mg/l	0.510	0.458	0,390	0.275	0.405	0.350	0.278	0.175	0,490	0.427	0.415	0.260	0.053	0.145	0.055	0.055
Phenois	mg·l	0.700	0.300	0.400	0.250	0.275	0.150	0.125	0.125	0.650	0.425	0.400	0.350	0.225	0.125	0.150	0.150
Phosphorus	mg/l	0.033	0.028	0.035	0.040	0.015	0,620	0.010	0.010	0.040	0.038	0.035	0.030	0.020	0.020	0.015	810.0
Total suspended solids	mg/l	525	670	500	450	200	240	260	250	530	690	525	500	225	200	250	260

Table H-6 Results of Water Quality Monitoring at Meru Pilot Pond (2/6)

					2nd Ja	n /97							16th J	an /97			
PARAMETER	Unit		Old Pon	d Injet			New Pon	d Outlet			Old Por	nd inlet		<u> </u>	New Por	d Outlet	
		8:00	12:00	16:00	20:00	8:00	12:00	16:00	20:00	8:00	12:00	16:00	20:00	8:00	12:00	16:00	20:00
Air Temperature	∍c	22	32	30	24	22	32	30	25	23	32	29	22	21	32	30	23
Water Temperature	эc	21	21	22	21	22	21	22	21	20	21	22	21	20	22	22	21
ρΗ	Í	7.5	7.7	7.8	7.9	8.3	8.4	8.0	8.3	7.8	7.9	7.8	7.8	8.2	8.5	8.0	8.4
вор	ng:l	500	510	550	525	110	120	160	140	505	65ù	575	500	120	140	135	130
Faeca) coliform	No./100mil	850	825	815	800	135	140	160	150	820	1120	1100	1300	125	180	260	180
Nitrates	mg/l	0.14	0.15	0.12	0.10	0.45	0,38	0.35	0.40	0.13	0.10	0.12	0.11	0.43	0.30	0.25	0.45
Nitrite	mg:1	0.35	0,45	0.43	0.50	0.18	0.20	0.25	0.15	0.30	0.50	0.45	0.55	0.13	0.15	0.20	0.10
Аштоліа	ıng∕l	3.80	3.75	3.60	3.50	2.50	2.15	3.25	3.00	3.75	3.60	3.80	3.75	2.00	2.50	3,00	3.80
Sulfide	mgl	0.550	0.484	0.375	0.400	0.035	0.030	0.028	0.033	0.500	0.450	0.400	0.325	0.038	0.035	0,040	0.040
Phenois	mg1	0.700	0.450	0.450	0.500	0.250	0.250	0.300	0.180	0.650	0.350	0.350	0.300	0.200	0.225	0.275	0.175
Phosphorus	nig/l	0.622	0.030	0.030	0.030	0.015	0.020	0.018	0.020	0.030	0.030	0.035	0.033	0.013	0.015	0.018	0.015
Fotal suspended solals	mg)	525	625	600	575	156	160	140	150	530	650	525	500	180	150	180	200



Table H-6 Results of Water Quality Monitoring at Meru Pilot Pond (3/6)

					29th Ja	an /97							15th F	eb /97			
PARAMETER	Unit		Old Por	d Inlet		_	New Pon	d Outlet			Old Per	d Inlet			New Por	d Outlet	
		\$:00	12:60	16:00	20:00	8:00	12:00	16:00	20:00	5:00	12:00	16:00	20:00	8:00	12:00	16:00	20:00
Air Temperature	³C	22	33	30	24	22	33	30	24	23	33	31	23	23	33	31	23
Water Temperature	⊅C	21	20	20	22	20	22	22	21	22	23	22	20	22	23	22	20
рH		7.2	7.4	7.5	7.6	8.3	8.5	8.3	8.2	7.3	7.5	7.4	7.8	8.4	8.2	8.5	8.3
вор	mg.1	450	480	450	500	125	130	140	120	495	500	500	530	120	140	120	150
Faecal coliform	No./100ml	850	900	850	900	120	160	180	140	900	850	800	825	120	150	160	180
Nărates	mg/l	0.14	0.13	0.13	0.12	0.40	0.33	0.20	0.50	0.15	0.13	0.12	0.14	0.43	0.33	0.20	0.40
Nitrite	fgm	0.25	0.35	0.20	0,20	0.15	0.10	0.25	0.15	0.30	0.40	0.38	0,45	0.10	0.10	0.15	0.15
Ammonia	mg/l	4.00	4.50	3.00	2.50	2.50	1.50	2.00	3.00	4.50	5.00	4.25	3.75	2.75	2.50	2.75	2.00
Sulīvie	mg/l	0.450	0,400	0.300	0.250	0.030	0.033	0,028	0.025	0.500	0.450	0.380	0.480	0.035	0.030	0.033	0.035
Phenols	mg/l	0.450	0.400	0.450	0.450	0.150	0.200	0.200	0.275	0.650	0.380	0.500	0.450	0.250	0.275	0.200	0.200
Phosphorus	mg/l	0.030	0.025	0.020	0.020	0.010	0.010	0.020	0.015	0,025	0.028	0.020	0.025	0.010	0.015	0.010	0.015
Total suspended solids	lgm	480	550	575	500	160	140	180	180	530	600	650	575	175	150	160	180



Table II-6 Results of Water Quality Monitoring at Meru Pilot Pond (4/6)

							26th Fe	eb /97						
PARAMETER	Unit		Old Por	d Inlet			Old Pon	d Outlet		New Pond Outlet				
		8:00	12:00	16:00	20:00	6:00	12:00	16:00	20:00	8:00	12:00	16.00	20:00	
Air Temperature	.²C.	21	33	30	23	21	33	30	23	21	33	30	2.	
Water Temperature	∍c	21	21	21	21	21	21	21	21	21	21	21	21	
pН		7.3	7.4	7.2	7.5	8.6	8.2	8.3	8.4	8.1	7.9	8.2	8.0	
BOD	mg/l	635	630	590	550	200	240	120	115	200	180	220	250	
Faecal coliform	No./100ml	600	650	700	750	90	80	120	140	150	200	180	220	
Nitrates	mg/l	0.12	0.13	0.10	0.12	0.30	0.30	0.20	0.30	0.20	0.18	0.15	0.10	
Nitrite	mg/l	0.25	0.28	0.30	0.30	0.10	0.13	0.18	0.20	0.18	0.20	0.23	0.20	
Ammonia	mg/l	4.00	4.25	3.50	3.85	2.25	1.75	2.75	1.50	3.00	3.50	2.75	2.6	
Sulfide	mg-1	0.524	0.493	0.381	0.599	0.089	0.093	0.098	0.091	0.113	0.099	0.108	0.09	
Phenels	lgm	0.148	0.300	0.462	0.700	0.061	0.156	0.050	0.197	0.070	0.174	0.050	0.20	
Phosphorus	mg/l	0.025	0.020	0.025	0.028	0.010	0.015	0.013	0.010	0.015	0.015	0.020	0.01	
Total suspended solids	mg/l	520	500	525	480	120	110	150	106	200	180	220	25	



Table H-6 Results of Water Quality Monitoring at Meru Pilot Pond (5/6)

							27th F	cb /97						
PARAMETER	Unit		Old Per	ıd İrdet			Old Pon	d Outlet		New Pond Outlet				
		8:00	12:00	16:00	20;00	8:00	12:00	16:00	20:00	8:00	12:00	16:00	20:00	
Air Temperature	∍c	22	32	30	24	22	32	30	24	22	32	30	24	
Water Temperature	эC	22	21	22	21	22	21	22	21	22	2.1	22	21	
рН		7.5	7.4	7.3	7.6	8.0	8.0	8.1	δ.2	8.5	8.2	8.4	8.6	
вор	mg/l	695	590	680	660	175	100	120	235	75	90	90	120	
Farcal coliform	No./100ml	500	800	720	580	150	120	180	160	100	80	120	110	
Nitrates	mg/l	0.14	0.13	0.12	0.10	0.13	0.18	0,20	0.23	0.10	0.25	0.30	0.20	
Niurite	mg/l	0.20	0.30	0.30	0.28	0.15	0.10	0.15	0.10	0.10	0.05	0.15	0.10	
Aumonia	mg/l	4.75	4.00	4.80	3.90	3.75	3.00	2.00	2.50	3.00	1.50	1.75	2.00	
Sulfide	mg/l	0.513	0.361	0.400	0.599	0.106	0.082	0.100	0.090	0.076	0.087	0.095	0,090	
Phenols	mg/l	0.585	0.510	0.300	0.549	0.159	0.033	0.185	0.390	0.048	0.000	0.160	0.187	
Phosphorus	mg/l	0.020	0.015	0.025	0.020	0.015	0.610	0.015	0.010	0.605	0.013	0.003	0.018	
Total suspended solids	mg/l	500	510	590	550	200	150	180	250	150	100	120	86	

Table H-6 Results of Water Quality Monitoring at Meru Pilot Pond (6/6)

							28th Fe	·b /97						
PARAMETER	Unit		Old Por	id Inlet			Old Pone	Outlet		New Pond Outlet				
		8:00	12:00	16:00	20:00	8:00	12:00	16:00	20:00	8:00	12:00	16:00	20:00	
Air Temperature	³C	23	33	31	23	23	33	31	2.3	23	33	31	23	
Water Temperature	эC	21	22	21	20	21	22	20	20	21	22	21	20	
рН		7.2	7.6	7.4	7.3	8.0	7.9	7.8	8.1	8,6	8.2	8.4	8.1	
BOD	mgł	675	700	555	700	240	160	190	290	130	110	120	160	
Faecal coliform	No./160ml	625	610	585	575	200	250	180	160	100	120	80	120	
Nittates	ությ	0.10	0.11	0.13	0.10	0.13	0.15	0.10	0.14	0.25	0.23	0.20	0.20	
Nitrite	mg1	0.23	0.28	0.27	0.25	0.17	0.13	0.15	0.14	0.15	0.10	0.14	0.15	
Ammonia	mg/ì	4.80	3.75	3.50	3.00	3.00	2.75	2.85	2.50	2.50	2.00	2.75	2.25	
Sulfide	mg/l	0.520	0.321	0.265	0.209	0.167	0.125	0.101	0.106	030.0	0.092	0.077	0.084	
Phenols	mgA	0.405	0.630	0.288	0.486	0.097	0.107	0.156	0.079	0.037	0.152	0.105	0.070	
Phosphorus	ուբյ	0.010	0.018	0.020	0.019	0.013	0.010	0.018	0.016	6.015	0,010	0.020	0.012	
Total suspended solids	ingl	525	575	500	510	250	200	225	150	100	120	106	136	