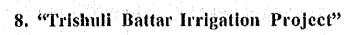
## Part A





थी ५ को सरकार

पिंचार महा

टेतेक्स मं. २३१२ पोष्ट बनस तं. २१५७

सिहदरबार कारमाधी नेवास ।

पत्र संह्या :- २ ग्र-६१०५८। ५६ न प्राप्त पत्र संस्था र मिति !--

विषय !- (ऋार्ज) सहुदार (धेवार्ष गोजगा ।

भी पिवार किमा। पानी पार्शी। ुर्डु, भी भाग विम प्राप्तिसम्, दवाँ भाग ।

> उर्शुंच अवस्था सिवार किया बाट असि ०४८ १ शरद मा पत्र गुर्ग मस्पो टियागी कारता वार्वाही हुंदा भी र की बक्ता राज्य मंत्री सा बाट मिति ० अदा १२। ह मा निम्न बेदा निर्मा विवेश निर्मा हो अनुरोध मिलिह ।

बटाचा सिनार्थका जाम त्रित्रुती देखिताट खिलार आयोजनामा एक्ताउस्ट नै ३ भन्दा मार्था देउस पुत्र बाद ३ क्यूबर भागिको बावश्यत पर्ने मुख्या प्रदेश प्रवृत्य रिराजी सर्वे भे वि.प्रा. हे बावश्वव 'व्यवसार प्री

र, यो भवा बायावनाको एकपाईनया छता प्राह्मतीन प धर्चमा धर्म मुर्च प्राह्म वान भर यो बार महारी गर्न सहित्य । विका बैंड सी परामई गरी में, वि, प्रा, 8 पन्न गर्ने ।

ार्रवार्थ किया बाट त्यम पछि महर त्यार गरी पानी बितरण गर्न आयश्वत बार्मत 108C192198 प्रोतमा अम्बर्ग गरी वार्ष क्रिया ।

A-81

#### His Majesty's Government Ministry of Water Resources (Irrigation Division)

Reference: 2-ga-6/048/49

Subject: Trishuli Battar Irrigation Project

Department of Irrigation Panipokhari.

Nepal Electricity Authority Durbar Marg

As regards the Report File on the above mentioned subject submitted by the DOI on March 1992, a HMG/N decision (State Ministerial level) has been made on March 1992 and accordingly you are requested to execute the following points.

- 1. The NEA should make the necessary provision for providing 3 m3/s of water, which may be necessary for irrigating in Battar from the Trishuli Devighat Hydropower Upgrading Project head race canal upstream of aqueduct no. 2.
- 2. If upon doing so, some revisions have to be made in the design and if this entails additional expenditure, NEA should consult the World Bank on this matter and submit a report.
- 3. The DOI should then start making arrangements for the budget to construct the canal system required to distribute the irrigation water.

/Signed/

9. Report on Water Availability for Trisuli Irrigation Project from Trisuli Hydro Project Februay 1997

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- 4. ASSESSMENT OF INTAKE AND WATER CONVEYANCE STRUCTURES
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#### ANNEX:

- a) Generation Record for January 1996 to January 1997
- b) Load curve of the system
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#### LIST OF REFERENCES

- a) Executive Summary of Trisuli Devighat Rehabilitation Project Concept
- b) Project Completion Report
- c) Trisuli Power House Log sheet from January 1996 to January 1997
- d) Hydrological data from 1967 to 1993
- e) NEA Annual Report 1996

#### 1. SUMMARY AND RECOMMENDATION:

- 1.1. Water conveyance system from he regulator to balancing reservoir can safely carry a discharge of 48.6 m/sec. So tapping to m/sec water from balancing reservoir is possible for irrigation purpose.
- 1.2. NEA has upgraded and rehabilitated Trishuli Devighat Hydropower Plants for optimum utilization of available water resources. So NEA will agree to provide the spilled over water only for irrigation purposes.
- 1.3 From the hydrological and water demand analysis it is apparent that during normal year extra water for irrigation is available during whole year. During dry year 4 month from January to April are critical. But probability of such critical year is only 15%.
- 1.4. By the year 2000/2001 the power demand scenario will be changed to better situation and demand to Trishuli will be less than now. Moreover, it will take at least 2 years to complete the proposed irrigation project and water will be required only after that period.
- 1.5. The balancing reservoir should be cleaned to its full storage capacity which will provide better operational flexibility for both irrigation and hydropower plants.
- 1.6. Department of Irrigation should assure NEA that they will be tapping only additional water which is not been used for generation of hydropower. The intake at balancing reservoir will be constructed accordingly.
- 1.7. Low flow study of Trishuli River should be carried out for 2-3 year, to have more realistic data on low flow.
- 1.8. Department of Irrigation should look into the possibility of construction of a small balancing reservoir required for storage of Irrigation water for 4-6 hours (21600 32,400) m<sup>3</sup> at suitable elevation and place.

#### 2. ASSESMENT OF AVAILABILITY OF WATER FOR IRRIGATION

- 2.1. Department of Irrigation (DOI) is planning to rehabilitate Trisuli Irrigation Project having a command area of about 1000 ha. The water demand is 1.5 m<sup>3</sup>/sec for about 12 hours in a day. DOI wishes to study whether the possibility exist or not to meet the irrigation water demand from Trisuli Hydro-power Project (THP) balancing reservoir without hampering NEA energy generation demand.
- 2.2. The flow of the river is tapped by Trisuli Hydropower Project (THP) and developed in two stages and completed in 1970 after the installation of the last unit. The plant utilizes 31.15 m/sec, of water which is being 90% dependable flow of the river. The catchment of Trisuli at headworks is 4640 sq. km.
  - In 1984 the tail race water of the plant was further recoursed through 4.5 km conveyance system to Devighat for generating electrical power from Devighat Hydropower Plant (DHP).
- 2.3. For better utilization of available water resources along with rehabilitation and maintenance Trisuli Devighat Upgrading and Rehabilitation Project was launched in May 1992. NEA completed the project in December 1995 (through the loan assistance of IDA under Power System Efficiency Project) increasing 31.15 m/sec. canal base flow to 90% dependable

discharge of 45.6 cumes by rehabilitating original intake and conveyance system upto the Balancing Reservoir. However, the water conveyance downstream of reservoir is maintained same. The electro-mechanical empipment are also revamped for enhancing the generation capability of the Trisuli Plant F = 21 MW to 24 MW.

To meet the requirement of Irrigation and others instead of originally designed 1.32 m, the weir was raised 1.52 to 2.80 m height. Weir was designed for 100 year flood discharge 2780 m<sup>3</sup>/sec.

- 2.4. After completion of the major civil upgrading works water was successfully released on 3rd June, 1995. Whereas, revamping of electromechanical components was completed on 25th December, 1995 and thereby started generating anticipated average yearly energy of 163.8 GWh against previous record of 105 GWh through one number of 3.5 MW and 6 number of 3.25 MW generators.
- 2.5. The irrigation command area is located at low level (elevation) and downstream of the Balancing Reservoir, so the excess water accumulated in the Balancing Reservoir or spilled over could be well used for irrigation.
- 2.6. The THP revamped units have rated flow of 7.61 m³/s each and are operated in the base part of INPS load curve because of its run-off river characteristics. As per upgraded design 45.66 cumees continuous base flow can be make available to the plant. The irrigation requirement of 1.5 cumees is 3.2% of the continuous base flow. Considering the 255000 m³ capacity of Balancing Reservoir and peaking nature of load irrigation water requirement could be made available fully in normal year and partially during dry year.
- 2.7. From the mean monthly and yearly discharge of Trisuli river at Betrawati gauging station, No.
  447 four months (January, February, March & April) of the year are observed to be critical.
  So, detail low flow study of Trisuli river is recommended.
- 2.8. The intake and conveyance system upto the Balancing Reservoir is actually capable of carrying 48.5 m/sec, discharge (See daily log sheet from 25th Dec. 1995 to 8th Jan. 1996 and 30th Dec. 1996 Annex e 16 pages) without detrimental effect to the civil structures. Hence, additional 1.5 m/sec, discharge required for irrigation on top of 45.66 m/sec, base flow through the existing conveyance system right from Intake to Balancing Reservoir is safely allowable throughout the year.
- 2.9 The irrigation water requirement lies within 5% of the hydrological error margin. Therefore the study on low flow for accessing the availability of enough water during critical months of the year is essential.

#### 3. LOW FLOW ANALYSIS OF TRISULI RIVER (AT BETRAWATI)

#### 3.1 Introduction

Trishuli river is a snow fed river. The river flow is monitored since 1967 by the Department of Hydrology and Meteorology (DHM): There is a permanent hydrological station at Betravati. The river water is utilized by Nepal Electricity Authority for hydropower production at Trishuli Bazar. The flow of 45.66 m<sup>3</sup>/sec. is the present requirement to the hydropower plant.

The following hydrological study is focused to assess the low (winter) flow and find out the excess water which can be used for irrigation. The present minimum water requirement for irrigation is about 1.5 m<sup>3</sup>/s for 12 hours a day.

#### 3.2 Long-term Averages:

The mean monthly flow of Trishuli river from 1967 to 1993 is collected from DHM publication. The long-term as trages are computed and these values are given in Table 3.1 mg. presented in Figure 3.1 beam in following pages.

Four month of the year, January, February, March and April are seen to be the critical months with low flow. The river flow in April and February is less than 45.5 m<sup>3</sup>/s.

#### 3.3 Low flow recession (Annual minimum):

Annual minimum flow from 1967 to 1993 are arranged in descending order and plotted using Weibull plotting formula. It has been seen that the present water requirement (45.5 m<sup>3</sup>/s.) for hydropower production is not available for all the periods. Table 3.2 contains minimum flows during the months February, March and April.

#### 3.4 Low flow recession (Daily ):

The daily river flows of selected year was analyzed. According to the statistic there was below normal rainfall during 1982/83 and 1978/79. The daily flow hydrograph from November to May of these years were analyzed (Table 3.3).

The following relationships were established for daily flows from November to February.

For 1978-79:

$$Q_c = 117e^{-0.010} T_d$$

$$[R^2 = 0.95]$$

For 1982-83:

$$Q_1 = 84e^{-0.0087} T_d$$

$$[R^2 = 0.93]$$

For 1983-84:

$$Q_t = 109e^{-0.0112} T_d$$

$$1R^2 = 0.961$$

Where,

Qt is the river flow on Tdlh day from first of November and Td is the number of days from the first day of November.

The above equation are valid only from November to February.

#### 3.5 Snow melt effect :

It has been found from low flow analysis that snow melt starts contributing to the flows from February or March. However, there seems no appreciable change of flow in April. Flow starts increasing generally from May (Figure 3.2).

#### 3.6 Flow duration curve:

Two low flow duration curves were prepared [Figure 3.3] - one for normal year (Wet year) and another for dry year. Table 3.4 lists flows and the corresponding percent of time. 45.6 m³/sec. is found to be 65 % dependable flow in dry year and 85 % in normal year. Table 3.4 contains daily mean river flows. Generally in February, March and April flow is less than 45.5 m³/s during normal year. However, it is further low in January, February March and April during dry year.

Longterm mean monthly flows

Trishuli at Betra	avati .			-								
Tustion or pear		Feb	Mar		May	Jun	Jul	Aug	Sep	Oct	Nov I	Dec
Period	Jan	700	Will	_Apr_	MILL	000 30	407.63		271 48	160 12		
1967-93	44.58	39.27	40.51	49.41	82.26	222.78	497.03	979.19	371,46	190.19	02.03	30.25
							Į.		<i>'</i>			i
1:27 Yrs)				ł		L,						

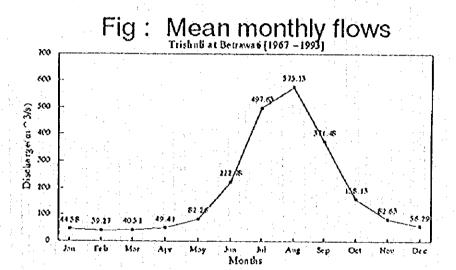


Fig. 3.1:

Table 3.2

Trishuli river at Betrawati : Area : 4110

Prob.%	Min-Q	Est-Q	(+3-Q	Mar-Q	Apr-Q	Year
96.4	24.5	26.1	28.0	24.5	26.0	1993
92.9	26.5	26.7	26.5	26.5	27.0	1992
89.3	28.4	27.3	30.5	28.4	28.4	1970
85.7	29.1	28.0	29.8	29.1	30.5	1968
82.1	29.1	28.6	37.0	34.0	29.1	1967
78.6	30.5	29.3	33.3	30.5	32.6	1969
75.0	30.5	29.9	31.0	30.5	31.5	1991
71.4	30.9	30,6	31.8	31.8	30.9	1984
67.9	31.3	31.3	31.3	37.1	38.0	1985
64.3	32.7	32.0	32.7	34.5	34.5	1983
60.7	33.6	32.7	33.6	33.6	35.4	1981
57.1	33.7	33.5	36.1	33.7	35.3	1974
53.6	33,7	34.3	35.1	33.7	33.7	1990
50.0	35.4	35.0	35.4	35.4	38.6	1972
46.4	35.4	35.8	35,4	36.0	43.3	1973
42.9	35.9	36.6	36.7	35.9	37.5	1971
39.3	36.1	37.5	38.8	37.9	36.1	1976
35.7	36.3	38.3	38.1	36.3	42.3	1980
32.1	37.2	39.2	37.2	44.5	67.6	1982
28,6	40.8	40.1	40.8	42.9	42.9	1978
25.0	41.5	41.0	41.5	41.5	42.9	1979
21.4	43.3	41.9	44.2	43.3	44.2	1975
17.9	43.6	42.9	43.6	45.8	49.0	1977
14.3	44.0	43.9	44.8	44.0	51.0	1989
10.7	44.5	44.9	47.8	44.5	49.0	1987
7.1	45.6	45.9	45.6	47.2	55.0	1988
3.6	50.9	46.9	50.9	50.9	52.0	1986



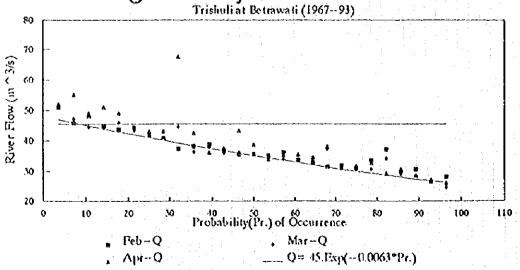


Fig. 3.2

Flows of Trishull river at Betrawati

November					Available	Elavi	Balance F	Elou	<u> </u>
5		wir ye:	4002/04			Normal	Dry	Norma.	
Day	1978/79	1887/83	1983/84		Dry	Nonnai	Uly	MOTHE.	
Nov-1	140	91	123		91	140	46.0	95.0	
5	123	86	112		86	123	41.0	78.0	
10	112	81	102		81	112	36.0	67.0	
15	104	76	92		76	104	31.0	59.0	
20	97	71	84		71	97	26.0	52.0	}
25	90	66	78		66	90	21.0	45.0	]
30	84	62	72		62	84	17.0	39.0	]
Dec-5	79	58.5	68		58.5	79	13.5	34.0	]
10	74	56	63		56	74	11.0	29.0	
15	70	53	58		53	70	8.0	25.0	]
20	66	50	55		50	66	5.0	21.0	]
25	62	48	51		48	62	3.0	17.0	]
30	59	45.5	48		45.5	59	0.5	14.0	
Jan-5	56	43	45		43	56	-2.0	11.0	
10	53	41	42		41	53	-4.0	8.0	
15	50	40	40		40	50	-5.0	5.0	]
20	48	38	38		38	48	-7.0	3.0	]
25	46	36	36		36	46	-9.0	1.0	]
30	45	35	35		35	45	-10.0	0.0	
Feb-5	43.5	33.5	32,5	175	32.5	44	12.5	-1.5	
10	43.3	33	31.5	1	31.5	43	-13.5	-1.7	]
15	43	34	33.5		33.5	43	-11.5	-2.0	
20	12.5	36	34		34		-11.0	-2.5	
25	42	35.5	33	1 1	33	42	-12.0	-3.0	
Mar-1	41.5	35	32	:	32	42	-13.0	-3.5	
5	44	34.5	33.5		33.5	44	-11.5	-1.0	
10	45	37	33	1	33	45	-12.0	0.0	
15	44.5	36.5	33,5		33.5	45	-11.5	-0.5	1
20	44	36.3	34		34	44	-11.0	-1.0	
25	43.5	36	35		35	44	•10.0	-1.5	
30	43		34,5		34.5		-10.5	-2.0	
Apr-5	44.5				33		-12.0	-0.5	]
10	46	35,3	32,5		32.5	46	-12.5	1.0	
15					32		-13.0	3.5	
20			+		34		-11.0	6.0	Į
25					36		-9.0	11.0	
30	1			L	38		-7.0	17.0	<b>.</b>
May-5	70				42		-3.0	25.0	
10					46		1.0	35.0	{
15					55		10.0	40.0	
[20	90	60	100	l	60	100	15,0	55.0	L

Table 3.4

Flow Duration Curve

Percent	River Disc	haror	NEA	Balance C	ischarge	T
of Time	Dry	Noma	needs	Dry	Normal	1
			110000			1
45.1	91	140	45.5	45.5	94.5	
46.2	86	123	45.5	40.5	77.5	
47.5	81	112	45.5	35.5	66.5	:
48.9	76	104	45.5	30.5	58,5	
50.3	71	100	45,5	25.5	54.5	}
51.6	66	97	45.5	20.5	51.5	
53.0	62	90	45.5	16.5	44.5	* 1
54.4	60	85	45.5	14.5	39.5	
55,7	58.5	84	45.5	13.0	38.5	
57,1	56	80	45.5	10.5	34.5	
58.5	55	79	45.5	9.5	33.5	
59.8	53	74	45.5	7.5	28.5	
61.2	50	70	45.5	4.5	24.5	
62.8	48	70	45.5	2.5	24.5	
64.2	46	66	45.5	0.5	20.5	
65,6	45.5	62	45.5	0.0	16.5	
66.9	43	62	45.5	-2.5	16.5	
68.3	42	59	45.5	3.5	13.5	
69.7	41	56	45.5	4.5	10.5	
71.3	40	56	45.5	-5.5	10.5	
72.7	38	53	45.5	7.5	7.5	
74.0	38	51	45.5	7.5	5.5	
75.4	36	50	45.5	9.5	4.5	0.00
76.8	36	49	45.5	-9.5	3.0	
77.6	35	48	45.5	-10.5	2.5	
79.0	35	46	45.5	-10.5	0.5	
80.3	34.5	46	45.5	11.0	0.5	
81.7	34	45	45.5	-11.5	-0.5	
83.1	34	45	45.5	-11.5	-0.5	
84.4	34	45	45.5	-11.5	-1.0	
85.8	33.5	- 45	45.5	-12.0	-1.0	7 2 7 4 8
87.4	33.5	44	45.5	-12.0	-1.5	
88.8	33.5	44	45.5	-12.0	-1.5	
90.2	33	44	45.5	-12.5	-2.0	
91.5	33	44	45.5	-12.5	-2.0	
92.9	33	43	45.5	-12.5	2.2	
94.3	32.5	43	45.5	-13.0	-2.5	
95.6	32.5	43	45.5	-13.0	-2.5	
97.0	32	43	45.5	-13.5	-3.0	
98.4	32	42	45.5	-13.5	-3.5	
99.7	31.5	42	45.5	-14.0	-4.0	

# Fig: Flow Duration Curve



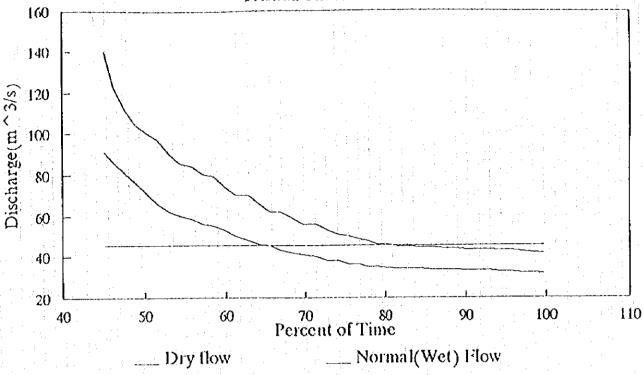


Fig. 3.3

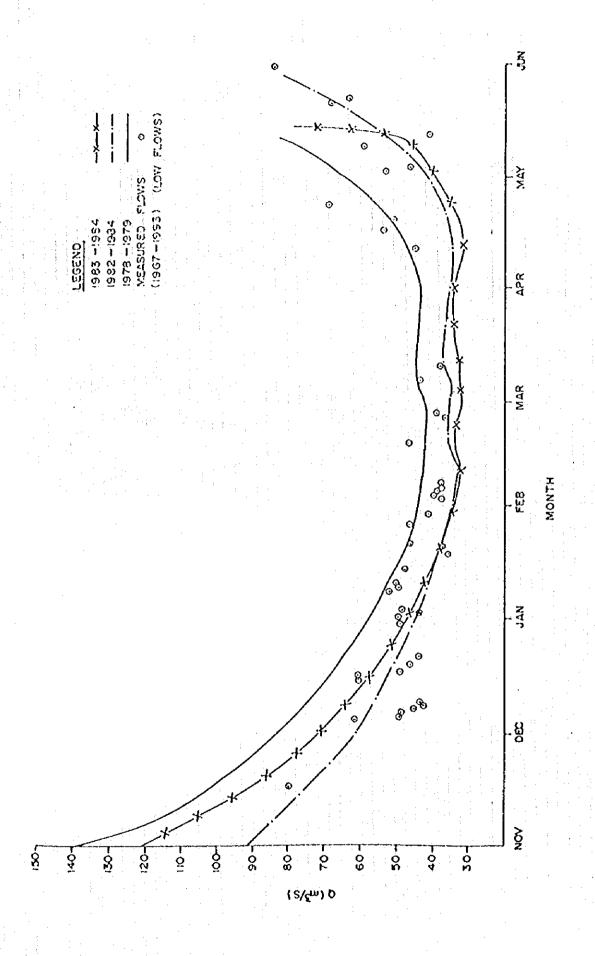


FIG 3.4 - LOW FLOW RECESSION CURVES

Table 3.5

Station No River

: 447

: Trisuli

Station Name : Betrawati

## MEAN MONTHLY AND YEARLY DISCHARGES - FUR TRISHULI RIVER AT BETRAWATI

5	ÆAR	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP	ОСТ	NOV	DEC	YEAR
	1967	47.4	38	35.6	41.2	63.7	137	429	518	326	199	66.9	48.2	162.5
	1968	37.9	32.1	31	38.7	64.9	220	486	479	281	170	75.8	51	164.0
	1969	40.5	35.5	33.2	38.1	62.3	133	398	451	335	126	67.7	47.4	147.3
	1970	36.7	31.9	29.9	40.5	65.3	162	468	512	290	152	83.6	57.9	160.8
	1971	45	38.6	36.8	46.5	65.1	348	448	549	320	154	81.5	54.8	182.3
	1972	42.9	37.3	38.9	42.9	101	163	431	473	283	106	62.5	43	152.0
r	1973	37.3	36.8	40.2	60.4	96.6	366	510	663	525	288	100	65.7	232.4
Ī	1974	50.8	39.7	36.2	53.8	82.2	200	546	640	390	106	86.4	60.6	191.0
	1975	51.2	46.2	44.2	60.2	91.6	264	548	561	516	222	110	69.8	215.4
	1976	49.3	41.9	40.3	46.6	85.2	207	367	487	372	165.8	99	65.5	168.9
	1977	48.1	46.5	49.8	54.9	77.8	192	591	609	368	164	94	61.7	196.4
	1978	46.6	44.1	45.4	57.4	151	335	520	625	332	215	108	70.1	212.5
	1979	51.3	43.5	43.9	52.4	91.9	185	468	504	280	137	83	55.6	166.3
Γ	1980	43.5	40.2	40.8	59.4	80.6	276	663	678	390	162	91	58.9	215.3
1	1981	42.5	36.4	37.2	50	85	290	714	619	369	140	87.1	56	210.5
	1982	43.8	39.8	56	74.6	81,4	212	407	596	393	120	77.5	54.1	179.6
Ī	1983	40.1	34.8	36.3	38.5	69.9	161	377	506	438	200	94.9	60.4	171.4
	1984	46.6	33.4	34.2	34.5	107	298	619	517	417	117	69.5	48.9	195.2
	1985	35.6	33.8	41.9	46.3	54.6	156	486	432	369	200	91.7	69.1	168.0
	1986	55.1	54.5	53.9	64.6	77.1	319	694	595	454	169	96.4	67.9	225.0
	1987	54.2	50.1	48.8	56.7	69.9	177	396		•	116	78.7	61,8	110.9
ſ	1988	52.2	48.5	49.3	59.7	86.3	178	488	566	260	109	71.8	58.8	169.0
	1989	53,8	47.5	48.4	57.2	105	188	356	441	282	120	65.1	• :	160.4
	1990		37.1	36.1	47.9	91.2	315	815	650	452	167	77.1	49.1	248.9
	1991	38.5	32.5	32.5	38	86.4	210	448	802				45.1	192.6
l	1992	35.4	29.1	28.6	34.6	43.1	122	325	709	410	140	66.5	41.8	165.4
	1993	33.2	30.5	27.2	38.2	84.5	192	446	728	422	166	67.7	42.1	189.8
	AVG	44.60	39.27	39.87	49.40	82.24	222.44	497.93	573.46	370.96	158.88	82,82	56.36	184.9
	MAX	55.1	54.5	56	74.6	151	366	815	802	525	288	110	70.1	248.9
	MIN	33.2	29.1	27.2	34.5	43,1	122	325	432	260	106	62.5	41.8	110,9

#### 3.7 Conclusion & Recommendations:

From the present data it has been seen that generally there is no surplus of water in February to April for irrigation when Hydropower and runs in its full capacity at 45.6 m<sup>3</sup>/s. However, this is not the actual case. Due to fluctuation in load, the generation also fluctuates and water demand varies accordingly.

The water requirement for irrigation is only about 3.5 % of the water requirement for hydropower plant and this amount falls within the error of data collection - staff gauge readings and discharge measurements as well as rating curve fitting. Hence, to have a clear picture it is strongly recommended to have low flow data collection for two to three years and its analysis. Such study would help to revise the low flow part of the rating curves and find out the actual amount of water available for irrigation use.

#### ASSESSMENT OF INTAKE AND WATER CONVEYANCE STRUCTURES

The Trisuli Hydropower plant is located about 75 km North West of Kathmandu which is constructed on Trisuli river. The installed capacity of this power plant was 21 MW and was completed in year 1970. It is a run-off the river scheme with a Balancing Reservoir of 2,55,000 m<sup>3</sup> capacity designed to provide peaking for about 4.5 hours daily.

With the aim to better utilize the available water resources a rehabilitation and upgrading program was launched. Trisuli was upgraded to 24 MW installed capacity. The following changes are made in various existing hydraulic structures.

For hydropower upgrading, weir height was planned to raise 1.32 m only. But considering irrigation and other demand, the crest elevation of the existing weir was raised by 1.5m to divert the whole dry weather flow through the head regulator. The number of head regulator has been increased to six from original three.

The closed reinforced concrete ducts (Twin Barrel) installed after the head regulator was cleaned during upgrading work and is presently being used under pressure flow to carry the designed discharge.

The depth of the power channel between the twin barrel and the desilting basin has been increased between 0.4 to 1.4 m to make it capable to carry the designed discharge 45.6 m/s. The energy generation log sheet shows that the water conveyance system from head regulator to balancing reservoir can safely carry a discharge of 48.6 m/s.

Two new aqueduct of similar capacity has been constructed in parallel to the existing aqueduct No. 1 and No. 2 to carry the designed discharge after upgrading.

The Balancing Reservoir was nearly filled and cleaned partially during upgrading work. It is again filled during last year monsoon. About 250 lps of discharge was found leaking from the flushing structure. The balance reservoir was sufficient to provide about 4.5 hours of daily peak load operation. At present there are four lateral Kholas which drain into the balancing reservoir bringing substantial load of sediment. The measure taken to check the flow of debris into reservoir was not fully successful.

The existing seasonal stream called Khahare on west side of the Balancing Reservoir is bringing a huge amount of sand and debris flow during monsoon. The debris flow have to be controlled by constructing several check dams upstream of those Kholas.

Regular cleaning of the balancing reservoir will enhance live storage capacity, limit the abrasion damage to hydraulic turbines and limit the accumulation of bed load at the head race forchay. Enhancement of storage capacity of the balancing reservoir will provide better flexibility of water availability for irrigation.

#### 5. POWER GENERATION PATTERN OF TRISULI

- Power generation pattern of 3. 'I Power House after the completion of upgrading and rehabilitation works are studied in sugh daily log sheets of power house from 1st Jan. 1996 till 12th Jan. 1997. The study indicate that the power plant is used as a base load station in the operation of Integrated Nepal Power System (INPS).
- Even during the dry season (Jan, Feb, March and April) of the year, the maximum load registered varies from 17.2 MW to 22 MW, while the minimum load is from 0.2 MW to 10 MW with monthly load factor well above 77%. The least monthly energy generation recorded is more than 10.7 GWh.
- 5.3 Similarly during the wet season (July, Aug. Sept, Oct) maximum demand has registered as high as 22.8 MW and minimum demand 0.4 MW with load factor well above 77%. The monthly energy generation is highest in the month of October attaining 14.4 GWh.
- 5.4. The plant is designed for average yearly generation of 163.8 GWh with annual firm energy as 144.2 GWh. The annual energy generation of 1996 is 153.94 GWh which is already 94% of the designed figure. The plant will generate 100% of the average yearly energy in 1997 with plant load factor registering well above 75%.
- 5.5 The study of daily log sheets confirms that the plant will share its optimum role in INPS load distribution curve in terms of capacity and energy till 2000/2001.

#### 6. ANTICIPATED FUTURE OPERATION REGIME

- 6.1 Trisuli and Devighat hydropower stations together are operated as base load stations to meet the daily load demands of INPS. They are run-off-river type plants with balancing reservoir for daily peak hours.
- 6.2 Load dispatching center of NEA has sensible link to interacts with Trisuli-Devighat complex for the daily operation of power stations, so is making optimum generation for meeting the anticipated daily demands of the system regularly.
- The typical daily load distribution curve of INPS is presented in Annex b. The curve shows that Trisuli-Devighat power generation pattern lies in the base part of the INPS load curve. The operational regime of Trisuli-Devighat is assumed to remain the same in the subsequent years.

The following committed or anticipated new generation is considered for assessing the future operation regime of Trisuli-hydro power plant.

- Puwa Khola Hydro power project under construction by NEA, a nun-off-river type, with 2000 cu.m. capacity balancing reservoir for average annual generation of 48 GWh and deliver winter peak power of 6.2 MW is expected to commence generation by February 1998.
- Modi Khola Hydropower Project also is under construction by NEA; a run-off-river type, having 26640 cu.m. capacity regulating pondage for annual average energy generation of 91 GWh and deliver a winter peak of 14 MW is scheduled to be commissioned by FY 2054/55 (1997/98).
- Chilime Hydropower Project is under construction by a public company is scheduled to be commissioned by the middle of 1998. The project is with an installed capacity of 20 MW for average annual energy generation 137 GWh.

- Kali Gandaki A. a priority hydropower project is of the run-off-river type having a daily pendage with a peaking capacity of about 6 hours. This project will generate 840 GWh in an average year and deliver a winter peak power of 144 MW to the INPS from 'uly 2000 only.
- Khinti Khola Hydro power project a rapid action project, under construction by Himal Power Ltd under the Power Purchase Agreement by NEA, is of the run-off-river type. It is scheduled to join INPS from June 2000 on wards. The project is expected to generate 350 GWh in an average year and deliver a winter peak power of 60 MW.

#### 6.4 Energy

During 1996/97 winter NBA could manage to meet the electrical power demands without having to resort to load shedding inside the Valley like last year and year before. It was possible due to import of power and operation of Diesel generators at Duhabi, Biratnatgar and Hetauda.

The hydro power generation scenario is presented below which explains the likely hydro power deficit till the year 2000 both in peak power as well as in terms of energy.

The assessment of energy is done based on expected energy generation capability of INPS against the anticipated increase of loads over the future years.

The attached table 6.1 shows the energy generation capability of the various power stations based on their past performances. Long term maintenance of any hydro stations has not been considered.

Table 6.1

						2000
1113	1144	1147	1327	1418	1768	2608
						840
						91
						136
			44	44	44	44
			•		330	244
, , ,	• • • •	147	, (	70		350
						78
						20
						50 20
						50 50
					and the second s	109
						160
						65
						165
						430
	430 165 65 139 96 50 20 20 78	430 430 165 165 65 65 139 157 96 109 50 50 20 20 20 20 78 78	430 430 430 165 165 165 65 65 65 139 157 160 96 109 109 50 50 50 50 50 50 20 20 20 20 20 20 78 78 78	430	430	430       460       65

Note: The gap between supply and demand is within manageable limit of 20%, which can be met by import or generation by stand by diesel generators.

#### 6.4.1 Hydro Energy Deficit

The energy generation scenario is liewed through table 6.1 and NEA load forecast (F.Y. 95/96 A year in Review). It is evice at that energy deficit persists for few years and improves after Kaligandaki-A joins in the national grid in the year 2000.

The results are summarised below:

Ί	<b>`</b> _	ŀ,	ı	6	7

	and the second s	I WOLC O'R		
		Hydro	Deficit Surp	
1996	(G.Wh).	(GWb)	:::::(GWh)::::::::::(GW	U. S. A. A.
1997	1392	1147	245	
1998 1999	1530 1688	1327 1418	203 270	
2000 2001	1860 2047	1768 2608	92 561	

After Kaligandaki-A joining the system, the energy surplus is apparent and present operation regime of Trisuli will be relieved from hard pressed situation after 2001 and thereafter. Till 2001 both Trisuli and Devighat will be generating the energy to maximum possible limit of 160 GWh and 118 GWh annually.

#### 6.4.2 Peak Power

1

The attached table 6.3 shows the available hydro peak power likely to be generated in the coming years. No provision is made of the major maintenance during this period.

Table 6.3

Peaks Station				ık Capaci	ly (MIV)		
	1995	1996	1997	1998	1999	2000	2001
Marsyangdi	69	69	69	69	69	69	69
Kulekhani I	60	60	60	- 60	- 60	60	- 60
Kulekhani 2	28	28	28				28
Trisuli	18	22	- 22	22	22	22	22
Devighat	14	14	14	14	14	14	14
Sunkoshi	7	7	7	7	7	. 7	7
Gandaki	8	8	8	8	8	8	8
Small Hydro	4	4	4	4	4	4	4
Áodhí Khola	4	4	4	4	4	4	4
Jhimruk	5	5	5	5	5	- 5	5
Khimti						60	60
Upper Bhote Koshi				:			36
, Puiva				36	36	36	36
Chilime				20	20	20	- 20
Modi					1 14	14	14
Upper Modi							
Kaligandaki-A					<u> </u>		144
Total	217	221	221	277	291	351	531
Compared to NEA forecast	244	275	303	328	- 354	383	413
Peak Balance	Deficit	Deficit	Deficit	Deficit	Deficit	Surplus	Surplus

#### 6.4.4 Water Balance Status after upgrading of Trishuli Power House

Installated Capacity

- 24 MW

Firm Capacity

- 19.5 MW

Annual Firm

- 144.2 GWh

Secondary

- 19.6 GWh

Yearly Average

- 163.8 GWh

Plant Flow

- 45.66 m<sup>3</sup>/sec

Firm Flow

- 31.15 m<sup>3</sup>/sec

### Monthly average water Balance for 1996 in m<sup>3</sup>/sec

Month	Energy	Average Generation	Average water consumtion	From hyd records of 27	_ 1	Bala m³/s			f hourly d to meet
	Generation in	MW .	correspoing to		1			irrigatio	n dernand
1996	KWh		energy gen.					:.	
		10.1	m³/sec	Min	Average	Min	Average	Min	Aver
January	12,757,140	17.13	37.20	33,20	44.60	(4.04)	7.40	-	133.26
February	10,506,160	15.09	32.77	29.10	39,27	(3.70)	6.50	-	117.06
March	10,730,580	14.72	31.96	27.20	39.87	(4.80)	7.91	-	142.32
April	11,859,030	16.5	<b>35</b> .83	34.50	49.40	(1.30)	13.57		244.29
Мау	12,786,600	17.16	37.26	43,10	82.24	5.84	44.98	3.08	809.61
June	13,667,570	18.97	41.19	122.00	222.44	80.81	181.25	0.22	3262.46
July	14,106,726	18.89	41.02	325.00	497.93	283.98	456.91	0.06	8224.41
August	12,962,910	17.7	38 43	432.00	573.46	393,57	535.03	0.05	9630.46
Sept.	12,701,500	17.72	38.48	260.00	370.96	221,52	332.48	0.08	5984.68
Oct.	14,498,000	19.52	42.39	106.00	158.88	63.61	116.49	0.28	2096.89
Nov.:	13,890,600	18.47	40.11	62.50	82.82	22,39	42.71	0.80	768.85
Dec.	13,480,700	18.01	39.11	41.80	56.36	2.69	17.25	6.69	310.55
Total	153,947,516								· · · · · · · · · · · · · · · · · · ·

To generate 21 MW the water

requirement is 45.6 m³/sec.

Daily water demand for irrigation

= 1.5x12x60x60 m<sup>3</sup>

 $= 64,800 \,\mathrm{m}^3$ 

Result:

If water demand forecast is based on minimum discharge only, then from January to April no water is available for irrigation purposes. Based on average discharge water is available throughout the year.

## JANUARY, 1996

Date	Max Load (MW)	Min Load (MW)	Avg. Load (MW)	Load Factor (%)	Genera- tion (KWH)
1	20.3	15	17.85	89.25	428430
2	22	16.2	17.5	79.58	420200
3	20	16	17.88	89.41	429180
4	20	17	18.14	90.7	435390
5	21	15.8	17.87	85.09	428890
6	21	17	17.69	84.24	424600
7	21	15	17 35	82.63	416500
8	21	15.1	17.27	82.26	414620
9	21	15	17.27	82.25	414550
10	20	15	16.69	83.45	400600
11	21	4	16.03	76,37	384910
12	21	14.7	17,11	81.49	410740
13	20	16	17.02	85.12	408590
14	21	15	16.97	80.85	407510
15	22	15	17.47	79.41	419290
16	21	18	18.72	89.18	449490
17	19.5	15	17.38	89.14	417190
18	19.5	18	18.24	93.56	437900
19	19.6	16	17.36	88.58	416720
20	20	14.9	17.29	86.45	414990
21	20	15.5	17.76	87.35	419320
22	19.5	15	17.26	88.54	414400
23	19.5	16	16.72	85.76	401370
24	19.7	15.2	16.68	84.69	400430
25	19	16	16.07	87.77	400240
26	19	15	16.22	85,37	389290
27	16.8	16	16.37	97.44	392900
28	17.3	16	16.45	95 13	395020
29	17	16.6	16.14	94.97	387480
30	17	16.7	16.16	95.11	388050
31	17.2	15.2	16.18	94.07	388350
Average	of Avg. Load		17.13		
Total Mo	nth Generation	1	: : · · ·		12757140

## FEBRUARY, 1996

Date	Max Load (MW)	Min Load (MW)	Avg. Load (MW)	Load Factor (%)	Genera- tion (KWH)
1	17	15.5	15.88	93.45	388350
2	17	15.5	15.86	93.33	380790
3	17	10.5	14.76	86.86	35439(
4	17	15	15.36	90.38	368780
5	17	15	15.61	90.23	374660
6	17.1	13.5	15.53	90.86	372930
7	17.5	,15	15.42	88.15	370240
8	18	15	15.42	85.71	370280
9	17	14	15.19	89.39	364750
10	16.7	14.4	15.6	93,45	374570
11	.17	15.5	15.66	92,11	375840
12	17.6	16	16.08	91.37	385950
13	17	13.5	14.67	86.33	352260
14	17	14	15.17	89.25	364160
15	17	14	15.19	89.37	364650
16	17.6	14	15.34	83,98	35476
17	18.1	13	14.44	79.83	34679
18	18	13	14.31	79.54	343650
19	18	10	14	77.79	336060
20	18	14	15,49	86.08	37190
21	18	13	14.49	80.54	347940
22	17.5	13	14.32	81.55	34380
23	17.5	13	14.39	82.25	345460
24	17.5	13	14.58	83.36	350120
25	17.5	13	14.55	83.15	349270
26	17.5	14	15.29	87.42	367190
27	18	13.8	15.17	84.28	364100
28	18	14	15.22	84.56	365340
29	18	14	14.88	82.68	357180
verage o	f Avg. Load		15.09896552		

MARCH, 1996

. ≀ate	Max Load (MW)	Min Load (MW)	Avg. Load (MW)	Loss Factor (%)	Genera- tion (KWH)
1	17	14	14.71	86.54	353110
2	17	14	14.71	85.7	349670
3	14.8	14.8	14.30	96.88	344140
4	14.8	14.8	14.8	95.94	340800
5	14.8	14.6	14.32	96.77	343730
6	14.8	12.9	13.87	93.76	333070
7	14,1	13.8	13.38	94.89	321120
8	14.5	14	13.68	94.37	328440
9	14.5	14	13.58	93.67	326000
10	16	14	14.03	87.74	336950
11	16.5	16	15.24	92.4	365940
12	16.9	16	15.67	92.74	376160
13	17	12	15.78	92.84	378800
14	17	16	15.95	93.85	382910
15	17	0.5	13.55	76.42	311800
-16	17	16	15.07	88.66	361740
17	16.8	16	15.61	92.93	374710
18	17.2	15.8	15.98	92.92	383580
19	17.1	17.1	17.1		389090
20	17.2	16.5	16.12	93.74	386990
21	17	16.5	15,98	94.05	383730
22	17	16.5	15.7		377070
23	17.3	17	15.98	93.1	386590
24	17.1	14.5	15.32	89.63	364710
25	17	14	15.17	89.38	364710
26	17.1	13.2	14.85	86.89	356620
27	16	16	15.03	93.93	360720
28	16	16	<b>15</b> .1	94.41	362550
29	16	0.2	7.54	47.17	18140
30	16	0.2	14.05	87,81	337210
31	16	10.6	13.74	85.88	329780
Average o	f Avg. Load		14.72		
rotal Mon	th Generation				10730580

APRIL, 1996

Date	Max Load (MW)	Min Load (MW)	Avg. Load (MW)	Load Factor (%)	Genera- tion (KWH)
1	16	0.4	13.7	85.65	328920
2	16	16	15	93.91	360620
3	16	15.7	15.1	94.39	362490
4	16	15.4	15.05	95.25	358910
5	16	15.7	15.96	94.14	361520
6	16	11.2	14,84	92.77	356250
7	16	16	15.11	94.49	362860
8	16	12.7	14.7	91.88	352820
9	16	7	14.84	92.75	356190
10	17.6	16	15.59	88.58	374170
11	18	10	16.58	92,12	397960
12	18	16.8	16.99	94.4	407840
13	18.2	0.4	16.62	90.81	398880
14	18	17.7	17	94.44	408020
15	18.1	3.4	16.79	92.79	403110
16	18	18	16,74	93	401760
17	18	17.8	17.11	95.08	410760
18	18	13	16.42	91.24	394180
19	18	8.9	16,47	91.52	395390
20	19	12.6	16.8	88.46	403390
21	19	7	17.12	90.12	410970
22	19.5	17	17.84	91.5	428230
23	19	13	17.08	89.91	410040
24	19.5	7	17.53	89.92	420860
25	20.5	17.5	17.74	86.56	425880
26	19.5	18	17.99	92.3	431990
27	19.5	18	18.33	94.04	440150
28	19.5	18	17.88	91,71	429240
29	20	7	17.79	88.98	427140
30	20	18	18.27	91.35	438490
werage V	alue		16.50		
otal Mon	lh Generation				11859030

MAY, 1996

<del></del>	: 	·					
Date	Date Max Load (MW)		Load Load Loa		Avg. Load (MW)	Load Factor (%)	Genera- tion (KWH)
1	19	13	17.56	92.42	421460		
2	19	7	14.47	91.95	419300		
3	19	17.5	17.57	92.52	421900		
4	18.3	6.3	14.76	80.65	295210		
5	14.5	0.7	12.1	83.5	290580		
6	18	13	14.77	82.08	354610		
7	18	10	15.49	86.05	371770		
8	18.2	14.8	16.11	88.53	386700		
9	18	13.5	16.57	92.1	397910		
10	18.3	14.8	17.04	93.14	409100		
11	19.5	0.2	14.87	76.27	356970		
12	20.9	12.5	17,61	84.28	422770		
13	19.5	15.7	17.53	89.93	420910		
14	18.3	18.3	17.37	94.96	417090		
15	19.6	18.3	17.67	90.18	424240		
16	18.8	7	17.21	91.56	413130		
17	19.4	18.3	17.42	89.81	418160		
18	19.4	18.5	18.02	92.92	432660		
19	19.5	8	17.73	90.95	425670		
20	20	19,3	18.92	94.63	454260		
21	20.5	19.2	19.04	92.9	457070		
22	21	18	19.5	92.86	468040		
23	21.5	21	20.26	94.24	486290		
24	21.5	. 21	20.47	92.2	491500		
25	21.5	0.6	16.6	77.21	398450		
26	21	4.6	13.23	63.03	317680		
27	22	18	19.05	86.61	457310		
28	22	19	19.88	90,39	477300		
29	22	0.4	16.24	73.87	389780		
30	21	0.4	17.16		411900		
31	22	18	19.87	90.31	476880		
Average Va	lue		17.16				
Total Monti	n Generation				12786600		

JUNE 1996

Date	Max Load (MW)	Min Load (MW)	Avg. Load (MW)	Load Factor (%)	Genera- tion (KWR)
1	22	7	15.63	71.05	375150
2	22	19	19.68	89.49	472540
3	21	18	18.72	88.14	449290
4	22	19	19.34	87.95	464390
5	21.3	12	19.33	90.79	464150
6	22	10	18.58	84.49	446120
7	22	18.8	19.53	96.81	468930
8	22	18	19.97	90.72	479030
9	- 22	18	19.49	88.6	467820
10	22	19	19.7	89.57	477960
11	22.5	8.5	18.94	84.18	454620
12	22.5	19	20.02	88.99	480550
13	22	19	19.22	87.4	461490
14	21.4	9	18.56	86.73	445470
15	22	0.8	19.05	86,68	457380
16	22	18.7	19.78	89.94	474900
17	22	9.6	19.43	88.33	46639
18	22	19.4	20.21	91.88	48514
19	22.5	11	19.54	86.88	46918
20	22.5	16.4	19.27	85.66	462600
21	22	9.2	18.26	83.03	43841
22	22	13.3	17.98	81.76	431710
23	21	13	18.35	87.41	44057
24	22	10	17.37	78.99	417110
25	22	11	18.84	85.67	45234
26	22.5	12	18.29	81.3	439050
27	22.5	12	19.11	89.96	458830
28	22.1	18	19.33	87.46	463930
29	22	10	18.27	87.06	43860
30	22.5	12	19.33	85.91	46392
Average	Value		18.97		
Total Mo	nth Generation				1366757

**JULY 1996** 

Date	Max Load (MW)	Min Load (MW)	Avg. Load (MW)	Load Factor (%)	Genera- tion (KWH)
1	22.5	13.0	19.63	87.26	471240
2	22.5	12.0	18.84	83.76	452310
3	22	12.4	19.09	86.8	458350
4	22.6	12.0	19.11	84.56	458680
- 5	22	13.0	18.8	85.49	451400
6	22	10.0	19.34	87.94	464350
7	22	12.5	19.45	88.43	466960
8	22	0.4	18.21	82	437250
9	22	13.0	19.11	86.86	458670
10	22	12.0	19.3	87.75	463350
11	22	12.0	18.8	85.47	451320
12	22	14.0	19.7	89.56	472910
13	22	12.0	19.58	89.01	469990
14	22	12.0	19.53	88.77	468740
15	22	12.0	17.63	80.17	423310
16	22	14.0	18.89	85.89	453520
17	21	3.0	17.51	92.47	466090
18	22	10.0	18.26	83.01	438320
19	22	14.0	19.57	88.98	469840
20	22	13.4	19,33	80.88	464040
21	22	13.0	19,55	88,86	469200
22	22.5	4.0	16.53	73.46	396720
23	22	10.5	18.5	84.09	444000
24	22	13.0	18.83	85.63	452140
25	22	13.0	19.01	86.41	456290
26	21.3	12.0	19,11	89.76	458866
27	22	12.0	18.51	84.17	444420
28	22	12.0	18.21	86.33	436880
29	22	12.0	19.01	86.45	456460
30	22	12.0	19.74	89.72	473770
- 31	22	14.0	19.05	86.61	457340
Average	Value		18.89		r
Total Mo	nth Generation				14106726

## AUGUST 1996

L	Max oad MW)	Min Load (MW)	Avg. Łoad (MW)	Load Factor (%)	Genera- tion (KWH)
1	22	14	19.7	89.56	47288
2	22	14	18.44	83.83	44264
3	22	12	17,43	79.23	41837
4	22	14	18.69	84.96	44864
5	22	14	18.17	82.6	43614
6	22	12	18.93	86.04	45433
7	22	13	18.85	85.69	45248
8	22	12	18.43	83.88	44251
9	22	14	18.92	86.04	45431
10	22	10	18.87	85.8	45303
11	22	14	19.29	87.69	46302
12	21	12	19.68	97.73	47241
13	21	1.1	16.7	79.52	40081
14	20	. 12	15.77	78.88	36289
15	21	11	18.54	88.31	44510
16	21	12	19.17	91,33	46031
17	21	12	17.94	85.46	43074
18	21.6	13	18.16	84.1	43600
19	21,3	12	18.4	85.92	41500
20	22	0.6	16.5	75	39600
21	21	10	16.74	79,72	40180
22	22	12	16.09	73.14	38620
`23	21	11	16.35	77.87	39250
24	19	12	15.84	83.44	38050
25	19	10	16.75	88.15	40200
26	19.5	11	16.58	85.04	39800
27	19	12	15.33	80.69	33730
28	19		15.7	82.67	36130
29	21	13	17.61	83.88	42280
30	21	10.2	17.73	84.44	42560
31	22.8	5	17.48	76.7	29730
age Value			17.70		
<del></del>		1		76.7	1

## SEPTEMBER, 1996

<b>Date</b>	Max Load (MW)	Min Load (MW)	Avg. Load (MW)	Loan Factor (%)	Genera- tion (KWH)
					·
1	22.5	13	17.52	77.90	420700
2	23	10	17.85	77 67	428500
3	21	12	17.96	85.55	431200
4	21	12	17.88	85.17	429300
5	22	12	18.18	82.67	436500
6	22	14	18.6	84.58	446600
7	21	11	18.32	87.26	439800
8	22	13	18.92	86.04	454300
9	22	11	18.34	83.39	440300
10	21	13	18.2	86.7	437000
11	21	3	13.31	63.39	213000
12	18	13	16.59	91.89	397000
13	20	12	17.22	86.14	41350
14	20	9.5	16.82	84.1	40370
15	20.5	10.8	18.47	90.1	44330
16	20.5	13	17.83	8.01	428100
17	21	12	18.1	86.19	434400
. 18	21	14	17.85	85.01	428500
19	21	12	17.79	84.72	427000
20	21.5	13	18.13	84.34	425200
21	21	10	17.65	84.08	423800
22	20	13	17.39	86.97	471500
23	20	12	18.27	91.35	438500
24	21	17	17.56	83.63	421500
25	21	4	17,79	54.72	427000
26	21	12	17.79	55,59	431400
27	21	12	18.54	88.29	445000
28	. 21	55	16.38	77.99	393100
28	21	11	18 24	86.88	437900
30	21	12	18.07	86.09	433900
\verage	Value		17.72		
Fotal Mo	nth Generation	) 1		11	12701500

## OCTOBER, 1996

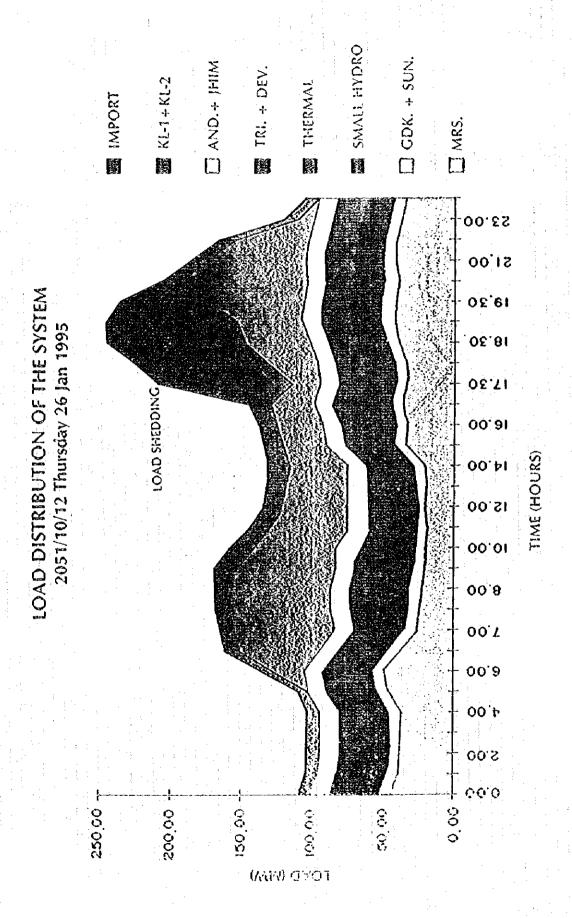
Date	Max Load (MW)	Min Load (MW)	Avg. Load (MW)	Load Factor (%)	Genera- tion (KWH)
1	19.2	8	19.39	8,181.00	37700
2	22	12	17.22	78.31	41350
3	21.6	18	19.2	88.92	46100
4	22	17.2	19,4	88.18	46560
5	21	10.5	17.97	85,59	43140
6	22	17	19	86.36	45600
7	22	17	19.02	86.45	45650
8	22	19	19.84	90.41	47740
9	22	21	20.81	94.62	49960
10	22	21	20.56	93.46	49350
11	22	21	21.06	95.73	50550
12	22	11	20.96	87.4	46150
13	22	18	20.03	91.07	48090
14	21.5	19	19.16	89.14	46000
15	21.5	18.6	19.47	90.58	46740
16	22	18.5	20.07	90.96	48030
17	22	19	20.57	93.5	49370
18	22	18	20.48	93.12	49170
19	22	12	19.84	90.18	47620
20	22	21	21.06	92.48	48830
21	22	18	16.76	89.82	47430
22	22	18	20.32	92.38	48780
23	22	11	19.66	89.37	47190
24	22	21	20.84	94.75	50030
25	22	18	20.25	92.08	48620
26	22	18.8	20.52	93.27	49250
27	22	18.9	19.33	87.87	46400
28	,19	13	17.95	94.51	43100
29	21	18	18.56	88.39	44550
30	21	14	18.95	90.27	45500
31	20.5	17.3	18.85	91.97	45250
verage Va	llue		19.58		+5250
otal Mont	h Generation				1449800

## DECEMPTR, 1996

			<del></del>		
Date	Max Load (MW)	Min Load ∷(MW)	Avg. Load (MW)	Load Factor (%)	Genera- tion (KWH)
				0.01	160000
1	20.5	19.5	16.78	91 64	450900
2	21,5	19.5	19.65	91.41	471700
3	22	19.5	19.55	88.86 95.27	469200
4	19.5	19.5	18.57 18.84	96.9	445900 453500
5	19.5	19.5		85.01	453500 397850
6	19.5 17.5	8	16.57 16.49	94.25	395850
	18.5	16.6 17.5	16.93	91.53	406400
8 9	19		16.5	91.33	419400
10	18.5	17.5 18.5	17.84	96.44	428200
11	18.5	8.5	17.42	94.18	418200
12	18.2	18.2	17.09	93.93	410300
13	18.2	17	17.33	95.26	416100
14	18	8.5	15.99	88.86	383900
15	18	1.7	17.1	95.02	410500
16	18	8.5	16.56	92.01	397500
17	20	17.7	17.4	87.02	417700
18	20	19	18.3	91.82	439300
19	20	17	17.95	89.75	430800
20	20	18.7	16.65	93.27	447700
21	20.5	19.5	19.43	94.81	466500
22	20.5	18.5	18.75	91.46	450000
23	20.2	19	19.92	93.67	454100
24	20.6	18.6	18.49	89.76	443800
25	19.7	18.5	18.38	93.33	441300
26	20	18.5	18.53	92.66	444800
27	20.2	12.2	18.52	91.68	444500
28	20.3	19	18.62	91.74	447000
29	20.5	19.3	19.06	92.98	457500
30	20.5	19.2	19.15	93.45	459800
31	20.8	19.2	19.8	92 24	460500
Average \	Value		18.01		
Total Mo	nth Generation	and the second s			13480700

## 'ANUARY, 1997

Date	Max Load (MW)	Min Load (MW)	Avg. Load (MW)	Load Factor (%)	Genera- tion (KWH)
1	21	17.6	19.32	92.04	463900
2	20	19	19.47	92.71	467300
3	21	19.5	19.5	92.85	468000
4	21	18.8	19.33	92.06	464000
5	20	19	19.34	92.12	464300
6	21	19	18.88	89.92	453200
7	21	18	18.52	88.19	444500
8	20	18	18.5	92.54	444200
9	21	12.3	17.89	85.19	429400
10	21	18	17.96	85.53	431100
11	21	14	18.01	85.77	432300
12	21	17	17.89	85.21	429500
Average Va	ılue	. !	18.72		,,,,,,,,,
Total Mont	h Generation	1			5391700



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## Annex C

RAPID ACTION PROJECTS					
Khimti Khola	60 MW 350 GWh	IPP, Himal Power Ltd. MOU on 2nd ammendment to PPA	June 2000		
Upper Bhote Khosi	36 MW 244 GWh	IPP, Himal International Power Co. MOU for Project development sigend	July 2000		
PUWA	6,2 MW 44 GWh	NEA Tender awarded for civil works	Feb. 1998		
Chilime	20 MW 136 GWh	NEA/privated Joint Venture Preparatory works for construction started	July 1998		
Modi Khola	14 MW 91 GWh	NEA/private Joint Venture Preparation works for construction	July 1998		
Upper Modi Khola	14 MW 84 GWh	NEA/private Joint Venture Feasibility study completed and solicitation for Project Development published.			
Middle Marsyangdi	43 MW 255 GWh	Germany Feasibility study completed, MOU signed	permitten Committee and Commit		
Upper Marsyangdi	42 MW 350 GWh	IPP Matthews International Canada Feasibility study being made. MOU for project development sigend	And the second s		
Lower Bhote Koshi	35 MW 253 GWh	Feasibility study to be made	orm de maries i ministra i inclusiva produce de la distribución de la decensión de la companya de desegra-		
Kaligandaki - A	144 MW 840 GWh	A priority project under construction by NEA financed by loan assistance by AD8 and another donor agency	July 2000		

## त्रिशुली देबीघा जल बिद्युत सुदृढिकरण आयोजना एक परिचय

काठमाण्डौबाट ७५ कि.मी. उत्तर पश्चिम दिशामा रहेको नुवाकोट जिल्लाको बिदुर न.पा. मा त्रिशुली नदीको किनारामा अबस्थित त्रिशुली तथा देबीघाट जल विद्युत केन्द्रहरू नेपालक पुराना जल विद्युत केन्द्रहरू मध्ये पर्वछन । यि जल विद्युत केन्द्रहरू मध्ये त्रिशुली जल विद्युत केन्द्रबाट ई.सं. १९५६ देखि ई.सं. १९७० भित्र सर्वप्रथम ३ वटा युगोरलाभियामा निर्मित ३ मे.वा धमतामा युनिटहरू र पुनः पछि ४ वटा (फुंजी कम्पनी) जापानमा निर्मित ३ मेगावाट विद्युत धमताका युनिटहरू (टवाईन र जेनेरेटर सेट) जडान गरि विद्युत उत्पादन शुरू गरिएको थियो । ति सात युनिटहरू मध्ये एउटा आकस्मिक प्रयोगको लागि जगेडा राखि बांकी ६ वटा युनिटहरूबाट विद्युत उत्पादन गरिएको थियो । ई.सं. १९८४ सालमा त्रिशुली जल विद्युत केन्द्रले प्रयोग गरि यसको टेलरेस पण्डमा खसालेको पानीलाई नदीमा नखसाली सिधा पण्डवाटै खुला नहर र सुरुङ्गको माध्यमबाट देवीघाट पुरयाई त्यहां अर्को जल बिद्युत केन्द्रको स्थापना भगो । त्यस विद्युत केन्द्रको धमता भारतमा निर्मित ३ वटा प्रत्येक ४.७ मे.वा. धमताका (टवाईन जेनेरेटर सेट) युनिटहरूको मद्धतले कुल १४.१ मे.वा. रहेको थियो ।

आर्थिक दृष्टिकोणबाट कुनै पनि जलविद्युत केन्द्रहरूको उभेर सरदर २५ देखि ३० वर्षको हुन्छ । तत्पश्चात यदि त्यसलाई तुरुन्तै सुदृढिकरण नगरिएको खण्डमा त्यो टुटफुट हुने, विग्रिने र त्यसको आर्थिक प्रयोजननै समाप्त भएर अन्त्यमा त्यसको परिणाम शुन्य सालभेज मुल्य (salvage value) हुन जाने हुन्छ ।

विगतका अनुभवहरुवाट के पनि देखिएको छ भने पुराना ज.वि.केन्द्रका विभिन्न संरचनाहरु तथा ईलेक्ट्रोभेकानीकल पार्टपुर्जाहरुको समय समयमा गरिरहनु पर्ने मर्मत खर्च त्यस जलबियुत केन्द्रको वार्षिक संचालन बजेटको निकै प्रतिशत हुनुको साथ साथ ती ज.वि. केन्द्रहरुको समयमै सम्पूर्ण रूपमा पर्मत सभार (Rehabilitation) नभएको खण्डमा तिनीहरूको थप २०-२४ वर्ष लम्बीन सबने आर्थिक जीवन (Economic Life) मा नकारात्मक असर पर्नुको साथै तिनीहरू बन्दनै हुन सबने अवस्थामा पनि पुग्न सब्धन् ।

त्रिशुली जल त्रिशुत केन्द्र करिव २५ वर्ष पुरानो भई आफनो आर्थिक जीवन समाप्त गरिसकेको परिपेक्षमा पि दुत्रै जल विश्वत केन्द्रहरुवाट निम्न कारणहरूले गर्दा तिनको बास्तबीक डिजाईन क्षमताको दुई तिहाई शक्ति मात्र उत्पादन गरिरहेका थिए ।

- क) नहरको मुहान देखि टर्बाईनको पेनस्टक पाईप सम्म पानी लैजादां भएका अत्यधीक हेडलस, चुहाबट तथा बालुबा र ग्रेगर जम्नु र रिजम्भायरमा बालुबा तथा माटो जम्नाले यसले आफ्नो सम्पूर्ण Live Storage Capacity गुमाई मात्र ३१.२ क्युमेक पानी जलविद्युत केन्द्रहरुमा आपूर्ती पर्न सबनु । यसबाट त्रिशुली केन्द्रले १२ मेगाबाट (उत्पादन गर्नु पर्ने १८ मे बा.) र देबीघाट ज.बि. केन्द्रले १.४ मे.बा. (उत्पादन गर्नु पर्ने १४.१ मे.बा.) मात्र भरपर्दो बिद्युत शिक्त उत्पादन गर्नु स्वादन स्वादन गर्नु स्वादन गर्नु स्वादन गर्नु स्वादन गर्नु स्वादन गर्नु स्वादन स
- ख। देबीघाट जल विद्युत संरचनामा भएको अत्यधिक हेडलस, लिकेज तथा माटो बालुवा जम्नाले डिजाईन ४५.६ क्युमेक क्षमतामा ३५ क्युमेक मात्र पानी जाने स्थिति हुनु ।

ग। त्रिशाली जल त्रियुत केन्द्रमा रहेका टर्बाईनहरु धेरै पुरानो भईसकेकोले त्यसको रनरहरु खिईसकेको र तिनमा लिकेज हुनको साथै तिनको न्युन Efficiency भएको कारणले तिनीहरूको शमतामा हास हुनु, : १ ।

आयोजनाको उद्धेशयः

- क) त्रिशुली तथा देवीघाट केन्द्रहरूको उत्पादन क्षमतामा आएको झस हटाउनुको (rehabilitation) साथ साथै क्षमतामा बृद्धि (upgrading) समेत गर्न त्रिशुली केन्द्रको ईन्ट्रेक देखि रिजॉम्बायर सम्म पानी लैजाने नहरको क्षमता बृद्धि गरि ३९.२ ब्युमेक बाट ४५.६ ब्युमेक बेस फलो कायम गर्ने ।
- ख) रिजीभ्वायरमा भईरहेको माटो तथा बानुवा जम्ने प्रकृषा रोवन यसमा वर्ग खोलाहरूमा ग्रामेल दाप (Gravel Trop) बनाउने ।
- ग) डिसेण्डर (Desander) पोखरीको Efficiency अभिवृद्धि गर्न त्यहां एउटा Tranquilizer Rack राख्ने
- घ) त्रिशुली तथा देवीधाट ज.वि.के. हरुको Electro-Mechanical equipment हरुको मर्भत संभार तथा आवश्यक नयां पार्टपुजीहरु संगेत फेरेर त्यसको उत्पादन क्षमतामा बृद्धि गर्ने ।
- ड.) आयोजनाको डिजाईन तथा निर्माण सुपरिबेक्षण कार्यहरूमा कन्सलटेण्टहरू सित अधिकतम नेपाली ईन्जिनियरहरूलाई संलग्न गराएर तत्सम्बन्धी टेबनीकल कार्य बारे बढि से बढि अनुभव हासील गराई भविश्यमा यस्ता किसिमका साना तथा मंभौता आयोजनाहरू नेपाल बिद्युत प्राधिकरणते आफनै जनशक्ति प्रयोग गरि परामर्शदात् बिदेशी संघ संस्थाहरूको सहयोग नितिएरै सम्पन्न गर्न सबने क्षमता विकास गर्ने आदी ।

### आयोजनाको छोटो बिबरण:

76)	कार्यको किसिम	मर्भत सम्भार तथा सुदृढिकरण (Rehabilitation & Upgrading)
खा	ठेकेदार	
4		i) तिमिल कार्य तर्फ लार्सन एण्ड दुवों लि. (ईण्डीया)
		ii) इतिबदोमेकानीकल कार्य तर्फ मल्टीपावर हाईडो
		ईलेक्ट्रीक डेभलपमेण्ट कर्पोरेशन (चाईना) ।
ग)	परामर्शदात् संस्था :	वयानेडियन ईण्टरनेशनस वाटर एण्ड ईनजी कन्सल्टेण्ट (CIWEC)
ម)	आयोजना प्रमुख	गणेश बहादर श्रेष्ठ
ड.)	कार्यकारी संस्था(Executive Agency	): ने.बि.प्रा. ।
च)		बिश्व बैंक (IDA), श्री ४ को सरकार तथा ने बि.प्रा.
<b>ij</b> )	विद्युत शक्ति उत्पादन :	

#### 5. ENERGY SENARIO :

A)	Condition Before Ungrading:	
$\Lambda$ )	Condition Before Operating:	

• •	Capacity ( MW)		Energy (GWH) Canal Capacity		
Trishuli	Installed 21.0	Firm 12.0	Annual Firm	Yearly Avera	nge 31,2
Devighat	 14.1	9.4	82.0	82.0	morace
Total	35.1 ×	21.4	187.0	187.0	

8) Condition After Upgrading:
Capacity (MW)

Annual Energy (GWII) Canal Capacity

	*								
	Trishufi Devigbat Total	Installed 24.0 44.1 38.1	Firm 19.5 14.1 33.6		al Firm 144.2 109.7 253.7	Secondary 19.6 8.8 28.4	Yearly	Average 163.8 118.5 282.3	45.6 m3/sec
C)	A CONTRACTOR OF THE PROPERTY O	irm Power: in Energy:	33.6 - 282.3 - [		# #	12.2 MW 95.3 GWH			
	Incremental	Energy Cost:	US \$ 0.04 present 6			NRs. 2.39 per	KWh at	power sta	tion at
			ND: 19	) nor K'	What f	ontract Awar	a A TUS S	i = NRc J	2.80)
<b>4.</b> ************************************	Marginal be	NE/	k system :		2.7 3.2				
6.	SAILENT F	<u>EATURES</u> :							
Λ.	TRISHULI Location Plant type		huli River -of-the river t	pe with	peaking	, capability util	izing	Balanc	ing
Reservo									
	<u>HEAD</u>			Before	Upgrac	ling	Afte	er Upgradi	ng
	Gross Head Net Head				54.0 m 51.4 m				
flood)		length ign discharge			139.00 3770 m	the state of the s		39.50 m 13/s (1 in	100 years
	Upstream T	raining wall						:	
		e of structure	gabion w	all of 68	3.5 m len	gth 67.7	m Conc.	wall	
		gth 100 Years Flood ilor	discharge				9 <b>2</b> ,0 3770	m m3/sec	
		of intake ening of intakes			3 2.88 m(	(w)x 2.8m(h)	3,05m(	2 old + 4 n w)x2 8m (i & new gate:	h) for both
	Conveyance	System			\$,40 kii	1		. •	
		nal Bank raising anel (Twin Barre		ir	1330 m	m to 300 min			

Closed reinforced concrete ducts

Length 1036.32 m Size 3.35 X 3.35 m Number Superpass Flume RCC(old) RCC(new) 4m wide x 2.4 m high Aqueduct # 1- Bridge A new bridge has been constructed at u/s of and parallel to the old bridge. Type of abutment a coordination of old wing wall of existing RRM masonry and a new RCC wall & deadman beam behind the wing wall at each bank. Well foundation The new bridge foundation on left & right bank were installed in the form of sunk wells. Central Pier Type of structure RRM Mass cone, with stone facing connected flexibly with old pier Scour protection around Old blocks repaired and new blocks added with an increased height. **Bridge Type** Old bridge -New bridge-Steel truss 59.50 in span stee! truss and 29.10 self supporting Pre-Stressed Concrete Flume. Steel liner in existing flume The old existing flume overlopped regularly at the u/s end when flow in the canal were above 35 cusees. Thus, a steel liner was installed in the flume. The width of the flume was reduced to 2.8 m from 3.66 m and height increased to 3.6 m from 3.09 m. PSC Flume 29.10 m (a new technology applied) RCC Flume 0 2.8m (w)x3.7 m (h) Aqueduct #11 A new bridge paralle to the old bridge has been constructed at the ws of the old bridge. Type of abutment **RMM** a coordination of old wing wall of existing RRM masonry and a new RCC wall & deadman beam behind the wing wall at each bank. Well foundation The new bridge foundation on left & right bank were installed in the form of sunk wells. Central pier

#11

Old bridge

RRM

Mass cone, with stone facing connected flexibly with old pier.

New bridge

Type of structure

Bridge type

		Steel truss	Steel truss
		double span of 57.95 m o	f double span, of 60,13m and
	•	first and 27.42 m second	
	Panatuati nima	mst and 27.42 in second	Sp.in. 27,00 iii
. :	o Penstock pipe		
	a) Old steel & con	crete pipe	
	Diameter (insid		3,66m
	- Steel pipe lengt		87.68 m
	- Concrete pipe l		63.92 m (L/B)and 101.94m (R/B)
	- Concrete Infici	Cugui	but the but th
	100 miles		
	b) New steel pipe	The state of the s	
	- Inside diameter		2.6m
	- Longth	可以 医原基征 性炎血病	253.433
	Desanding Basin		
	o Infet gates	2 nos	3 nos of 2.67m width and 1.3m
			increased ht with new hoisting
			mechanism and hoisting bridge
			mechanism and mosting orrage
1.1	Tranquilizer rack		
1.35	o Type of structure	Baffle deflector	Steel racks supported on RCC foundation.
	o Outlet gates	2 nos of 2.67m (w)	New hoisting mechanism and
hoisting			bridge with stream lined curve at
outlet to			reduce head loss.
100	o Flushing channel		Repair with new cut off wall and a new
	o Hosining Chainet		layer of 300 mm thick RCC placed
			over old surface.
			over old surface.
1	Flushing chute		
	o Gates		New hoisting mechanism new steel plat
			form provided.
	Overflow Spillway at Bypass Ca	anal	
* .	o Type of structure	Mass concrete	
	o Objective - As a		oing of the canal through accidental closure of
			oning of the canal through accidental crossic of
the		nder Intake Gates.	
	Balancing Reservoir		
1.5			
	e Length	1219,2m	
	o Gross	765,000 m3	
	o Net	255,000 m3	
	Reclaimed	147,000 cum	
	Reclassifica	147, Chin Chin	
	Forebay and Intake		
		•	
	o Forchay	Trapez	oidal canal with 4 intakes
	Penstocks		
F 4 F 4 F		3 + 1	
	o No.		& 1.52 m
	o Diameter		
	o Length	118 m	(& 125 m / 1 / 1 / 1 / 1 / 1 / 1 / 1 / 1 / 1 /
1.5	Powerhouse		
	The state of the s		
	Malaki	15.691	m
	o lleight	13.071	
	o No of generating units	i di di kacamatan d	

		the second			
0	Capacity of units	3 MV	v	3.5 N	41.U
0	Plant flow	• • •	Lin3/s	1 1	mMs
0	- Fire You	4,2,3	1 10.73		11/s
			v -		.,,,
The state	to a 1 de			•	
0	ine + Generators				
0 0	Yugoslav make	3 nos		Refurbished by	
0	Japanese make (Fuji) Capacity of each units	4 nos		Refurbished by	
0	Turbine type		•	3.5 N	1 VV
o	Rated head	: Fan 51.4			
0	Rated flow		m Lm3/s	* ;	4
0	Generator rated capac	rite 7,300	. 3.75 MV	7.6	3.89 MV/
o	Power factor	0.8	.,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	D.90	3.07 141 47
				0.70	1 (
Taily	rater Conditions				
0	Minimum elevation	149.0	-		
O	Normal elevation	150,2			149,007
0 <del>3</del> 9	Maximum elevation	150,8	87 m		
i he r	ight bank retaining wall wa	is modified from 90 degro	e bend to a s	niooth curved t	ransition.
<u>Paint</u>	ina				
		nead to mitte its star as	· · · · · · · · · · · · · · · · · · ·		
· I Pri	optiate types of paints were	used to paint all the steel	sinicuires as	well as power	nouse buildi
B, DE	EVIGHAT				
			44		
<u>Plant</u>	Type	Ron of river w	ith peaking	capability	
0.	Gross Head	40 m			
0	Net Head	39 m			e g the
<i>(</i>					
	cyance System (total lengt	<u>(h)</u>	4.50 Km		
0 .	Cut and Cover (J reac		600 m		
0	Tunnel (2 reaches)	Length (total)	2000 m		
O	Syphons (4 No.)	Length (a) 60 m each	240 m	The state of the s	. 1
Forel	19 V	Section 1985 and the second			
· VICE					
Eleva	tion		1420		
	mation		147.0 m		
*****	:- <b>::::::::::::::</b>		IVOOT CU	ım (twice reclai	mea)
Penst	<u>ock</u>				
0	No.		3	1 a 2 to 2	
0	Diameter	:	2.5 m		·
0	length		65 m		
	•				
Powe	rhouse				
0	Length	30.3 (	n		
0 -	Width	13.7			100
0 1	No. of units	3			
0	Capacity of unit	4.7 M			
0 -	Firm Now		m3/s		
0 :	Plant flow	45.31	m3/s	45.66	m3/s
		the second second second second			

Turbines and Generators

O Turbine type
O 16% I head 38 m
O 220 J flow 15.3 m3/s
O Generator rated capacity 5.875 MVA
O Power factor 0.8

Tailwater Conditions
O Minimum elevation 104.80 m

Tailwater Conditions104.80 m0Minimum elevation105.30 m0Maximum elevation115.80 m

## GENERATION DEPARTMENT TRISULI DIVISION

x0.00x

DAILY GENERATION REPORT

Station: TRISULI POWER HOUSE

Dated OS 2 .. 9-11

Remarks ushing head gate Time s Sector brs Discharge casec Sec 5.0 D. tank CVC Load ĭ. W. ٥ ئ Time in ırs 0200 0300 0400 0500 300 0000 000 001 200 400 301 2002 Station consumption in kwh. (1) USE A... (2) UUCE SAU Peak load in MW: 31. 4. time, 21.4.c. to . 22 a 1. hrs Maximum load in MW: 2/25 minium load in MW/8.5 Average lond in MW: 18, 25, Toad factor % - 3, 3, 5, 6 Des. 11 kv local supply in kwh.......2..12.5 (D... Synchroniizng Time (hrs) AVAILABLE MACHINES CONDITION: Total Energy Generation in kwh: ......\.\.S.Y.S. LOIF HAS Sent out Energy in kwh. ...... Barrage colony consumption in kwh. Trisuli River Discharge in cusec: ... 4 canal Discharge in cusec: .. /. Trisuli River level in feet ...... Total Discharge in Cusec: \_\_ Particulars Stopped time (hrs) 8080 Unit No Running Unit No. 7.4 ۲ ۲ 7.7 イイ

Prepared by:

7

Total

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## ARPAL BERGRAUGE AUTHORITY

GENERATION DEPARTMENT TRISULL DIVISION

Station: TRISULI POWER HOUSE

DAILY GENERATION REPORT

Dated. 6.5.2-9-13

## GENERATION DEPARTMENT TRISULI DIVISION

## DAILY GENERATION REPORT

Station: J'RISULI POWER HOUSE

Dated 652-9-14\_

Remarks gate Flushing head in a Discharge cusec D. tank level 30 0 ∞. M. W. Load Time in hrs 0010 88 Station consumption in kwh.: (1) .419.4...(2) .412...(2) Maximum load in MW: 2.3. minium load in MW 15:3 canal Discharge in cused: ....../2.45....... 3. time..... to ...... Des, 11 ky local supply in kwh......, 2,13,26,5.... 7.7 Average load in MW: 1/2... S.C. load factor % Sent out Energy in kwh. . ..... Barrage colony consumption in kwh. Trisuli River Discharge in cusec: Particulars Total Energy Generation in kwh: Trisuli River level in feet Peak load in MW: 3

Unit No	t No Running Unit No.	Unit No.	Stopped time (hrs)	Synchroniizng Time (hrs)
	\$ \{\}			
2	<u>ئ</u> م	1	1	1
3	2.4			
4	24			
2	70			
9	13			
7	3			
Total	29/			

AVAILABLE MACHINES CONDITION:

Total Discharge in Cusec:

Prepared by:

## GENERATION DEPARTMENT TRISULI DIVISION

I

97-6-250 Dated.

DAILY GENERATION REPORT Station: TRISULI POWER HOUSE

Remarks Flushing head gate Ime Discharge cusec 3 D. tank level Load × × Time in 0400 <u>@</u> 400 0700 000 9 Maximum load in MW-2013, minimum load in MW 126.129 Station consumption in kwh. (1) 46.24...(2) 44.2 = 300 Peak load in MW: 13. 0. time, 27.3.2. to 9.6.3. 2. hrs Average load in MW: 17.15.6. load factor 1/2.5.22.2. 7.12867 Total Discharge in Cusec: Trisuli River Discharge in cusec: AVAILABLE MACHINES CONDITION: canal Discharge in cused: .....(5.17) Des, 11 ky local supply in kwh..... Barrage colony consumption in kwh. Trisuli River level in feet: Particulars Total Energy-Generation in kwh: Sent out Energy in kwh.

Synchroniizng Time (hrs)	(025 m.
Stopped time (brs)	7.50
Unit No.	404
Running hours	257 24 24 27 27 27 27 27 27 27 27 27 27 27 27 27
Unit No Running Unit No.	1 3 4 4 5 6 6 7 7 7

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A-123

## NEPAL ELECTRICITY AUTHORITY GENERATION DEPARTMENT TRISULI DIVISION

# CALLY GENERATION REPORT

Dated 052-9-18

Station: TRISULI POWER HOUSE

Remarks Flushing bead gate Time Discharge cusec D. tank [eve] Load Time in 080 0000 Average load in MW: (F.: ST. load factor% 25.:53. Peak load in MW...\$2.5. time\_/2.25 to <u>25.55.</u> hrs Maximum load in MW.\$2.2. minium load in MW (4.2 Des, 11 ky local supply in kwh...... Seat out Energy in kwh.: ..... Barrage colony consumption in kwh. Particulars Total Energy Generation in kwh:

1900

009

Prepared by:

ニケックシン

1013

A-124

Trisuli River Discharge in cuseci

## Marin And State of Country Adjusted States of Country o

GENERATION DEPARTMENT TRISULI DIVISION

## DAILY GENERATION REPORT

Station; TRISULI POWER HOUSE

Dated 052-3-20

Remarks head gate Flushing Time Discharge Cusac 3 D. tank level 6 Load M. W. c o Time in 001 500 0900 800 888 canal Discharge in cusec: \_\_\_\_/&\_\_\_\_\_\_\_\_\_\_\_ Maximum load in MW: 25.5 minium load in MW. Peak load in MW: As 2. time 123.2 to 22.95 Total Discharge in Cusec: Average load in MW: 1817. Toad factor % Station consumption in kwh. (1) 440.1 Trisuli River Discharge in cusec: Total Energy Generation in kwh: ...4.3.5 Des, 11 ky local supply in kwh..... Barrage colony consumption in kwh. Sent out Energy in kwh., ..... Particulars

			- :			-		:
Synchroniizng Time (brs)	1110 mg.			1525				
(Stopped tune (hrs)	1	(00)	ļ	1				
Jait No Running Unit No.	3	J	·	<u>۲</u>				
Running hours	2.7	77	7.3	23	ング	24	27	ĺ
Jnit No		2	3	4	5	9	7	

AVAILABLE MACHINES CONDITION:

2400

Prepared by:

A-125

## PROPERTY OF THE PROPERTY OF TH

## GENERATION DEPARTMENT TRISULI DIVISION

## DAILY GENERATION REPORT

Station: TRISULI POWER HOUSE

Dated. 0.52-1-9-2 1966

Remarks Flushing head gate Discharge Cusco 5 D. tank level Load ĭ. Kan Time in Irs 0400 Maximum load in MW 2/2.8 minium load in MW 15-8 Average load in MW: 13:182: 10ad factor %... Sti. 22. Peak load in MW: 2/2 time/2-4% to And Station consumption in kwh. (1) 455 £ Total Energy Generation in kwhr ..... 5 Des, 11 ky local supply in kwh-..... Barrage colony consumption in kwin Trisuli River level in feet Particulars Sent out Energy in kwh.1

ن د

1500

AVAILABLE MACHINES CONDITION:

Total Discharge in Cusec: canal Discharge in cusec: .../ @ 20

Trisuli River Discharge in cusec: ..

8

Synchroniizng Time (hrs)	1412 tm.	
(Stopped time (hrs)	0/01	
init No Running Unit No.	001 ×	
Runniag	27.7.2.2.2.2.2.2.2.2.2.2.2.2.2.2.2.2.2.	16/
Init No	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	7 Total

2000 2100 2200 2300 2400

# NEIN BENEVILLORING AUTHORITY GENERATION DEPARTMENT TRISULI DIVISION

DAILY GENERATION REPORT

		000		Disid	Discharge	head	
Particulars	Time in	Z E	D. tank level	casec	Jime Free	gate	Remarks
87 > 26/ or 756/ sime /3 / WM at 102 / 1/2	0010	7.4.	(,)	0			
WM ni beol muin	0200	0.3/	7.79				:
	0300	5 37	8 77				·
Station consumption in kwh. (1) 422 + (2) 4/25 83	0400	16.3	6.93			:	
-:	0500	2.8/	67.0				
14.V.	0090	0 . %	63. c				
1	1	3/.3	\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\				
	- <del></del> -	20.€	7.5				
	0000	18.3	2.5	-			
Irishli River level in feet:	1000	2 3/	0			-	
	001	17.0	66.4				
canal Discharge in cusec:	: 	3.6	66.3				
Total Discharge in Cusec.	1300	0.6	トン				•
AVAILABLE MACHINES CONDITION:	1400	n -5/	66.5				
Stonney	1500	ري. د	وو٠ ي				
Their No Running It no Studies Synchronitzng	1600	15.0	67.0				
	- ' <u>'</u>	20.02	69.2				
1 21 2 0/83		0.%	07.0			•	:
2 (9 2		~	4.37				
3 24 2 13/5		20-0	7.79				
4 24 1		Ī	7.93				
5 20	2200	13.0					
22 2 6830	23()()	19.0	0.19				
7 21 6 10/0 m.	2400	0:6/	660				
				  -			

Prepared by:

## A CONTROL OF THE PROPERTY OF T GENERATION DEPARTMENT TRISULI DIVISION

DAILY GENERATION REPORT

Station: TRISULI POWER HOUSE

8-1-9561 Dated: 0.572...9...22

Remarks head gat: Flush. ime Discharge cascc D. tank cvel 0 ر ز Load Time in brs S Station consumption in kwh. (1) 32a.t...(2) 39a.B. Peak load in MW: 2/ 2... time 15:50. to . 212 c. hrs Average load in MW: 17:27-load factor 1/2-22:26. Maximum load in MW-2/1.2. minium load in MW /5. Total Discharge in Cusec: Des, 11 ky local supply in kwh..... Barrage colony consumption in kwl Trisuli River level in fect: Particulars Total Energy Generation in kwh: Trisuli River Discharge in cusec: Sent out Energy in kwh., .....

AVAILABLE MACHINES CONDITION Synchroniizng Time (hrs) 1035 50/ Stopped time (hrs) **ふから**/ Unit No Runniag Unit No.

2002

8

2300

12

500

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900

Prepared by:

Total

Total

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Annex c -16

Station: TRISULI POWER HOUSE

DAILY GENERATION REPORT

Dated. 653-9-15

Remarks Flushing head Discharge cusco D. tank level 4.22 6.33 66.7 5.09 Load ĭ. ¥. 20.5 7 . 51 \`. 0\\ i O Time in 2007 500 0061 0.010 0000 11.S 0300 06.30 0700 008 9 `≎ ≊ 0060 000 700 0500 900 500 8 2300 2000 2400 Peak load in MW: 2014.. time\_1805... to ...21.52. hrs Station consumption in kwh.: (1) 42 cf...(2) 1102 83. Maximum load in MW: 2.4.15. minium load in MW 19. Average load in MW: 19.11. load factor 7. . 23.14. 5 Synchroniizng Time (hrs) AVAILABLE MACHINES CONDITION: Des, 11 ky local supply in kwh...... Total Discharge in Cusec:\_\_\_ Barrage colony consumption in kwh Trisuli River level in feet: Trisuli River Discharge in cusec: .... Particulars Total Energy Generation in kwh: Stopped time (brs) Sent out Energy in kwh.. ... Unit No Running Unit No. hours S ξ,

## Part A

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10. M/M on the Progress Report (March 19, 1997)

## MINUTES OF MEETING ON PROGRESS REPORT OF

## FEASIBILITY STUDY ON TRISHULI IRRIGATION PROJECT

Date: March 19, 1997

Place: Meeting Room of the Department of Irrigation, Ministry of Water Resources

The JICA Study Team submitted the PROGRESS REPORT of the Feasibility Study on Trishuli Irrigation Project in twenty(20) copies with two(2) sets of Topographical Maps on March 19, 1997 to the Department of Irrigation, Ministry of Water Resources.

Subsequently, Mr. Masamitsu Fujioka, Team Leader of the JICA Study Team explained the contents of the Report to the concerned officials of the Department of Irrigation.

Y. L. Vaidya

Ţ

Director General,

Department of Irrigation,

Ministry of Water Resources,

Kathmandu, Nepal

M. Fujioka

Team Leader of JICA Study Team,

Chuo Kaihatsu Corporation,

Tokyo, Japan

Witnessed by:

A. Uchida

Assistant Representative,

JICA Kathmandu Office,

Nepal

Part A

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11. "Shifting the Intake Site of Trishuli Irrigation Project"

## HIS MAJESTY'S GOVERNMENT MINISTRY OF WATER RESOURCES

Ph. No. S 4-11686

j 4-11119

Fax No. : 418152

## DEPARTMENT OF IRRIGATION

Maharajgunj Kathmandu, Nepal.

Ref. No.

TIP/ | /053.54 - | 2.8%

XIr. M. Fujioka
Team Leader,
IICA Study Team
Trishuli Irrigation Project

Subject: Shifting the Intake Site of Trisuli Irrigation Project

Dear Mr. Fujioka

With regard to the shifting of the intake point to Alternative I site as proposed by you, we had thorough discussion with the NEA management. They suggested to look into the following 3 options

Option 1

To fix the intake point before the desanding basin with separate desander for Irrigation. This will not hamper any facility of Trishuli Hydropower System.

Option 2

Fixing of intake point between the balancing reservoir and the desanding basin.

In this case the desanding basin has to accommodate more sand than the designed capacity. So the capacity of desander has to be checked and proper facility to be provided to desand the extra silt which deposits in the desanding basin with the extra Irrigational water of 1.5 cumees.

Option 3

Fixing the intake point at the end of balancing reservoir.

This option seems to be most suitable as the intake point for Irrigation. But due to this the desander will be overloaded and more silt will be deposited in the balancing reservoir. This might reduce the generating capacity of the Trishuli Hydropower Station. So for this option the capacity of the desanding basin has to be checked as in the case of "Option 2". Besides this the balancing reservoir has to be cleaned/desilted regularly from the additional silt. For this some desilting device has to be installed.



## HIS MAJESTY'S GOVERNMENT MINISTRY OF WATER RESOURCES PEPARTMENT OF IRRIGATION

4-13/33 4-12374 Ph. No. {4-11686 {4-11119

Fax No.: 418152

Maharajgunj Kathmandu, Nepal.

Date .....

Ref. No.

So during the preparation of your draft report please consider the above options and suggest the most optimum solution so that both the Hydropower station and the irrigation systems runs without interfering each other.

Thanking you

With regards.

Sincerely yours

(N. N. Vaidya)

Superintending Engineer

C.C.

 Ministry of Water Resources Singh Durbar

enclosure:

one copy of Progress report submitted by JICA Study Team on Trishuli Irrigation Project

2. The Managing Director NEA, Kathmandu

enclosure:

one copy of Progress report submitted by JICA Study Team on Trishuli Irrigation Project

12. "Request for agreement for shifting of intake of TIP"

Director General, Department of Irrigation, Ministry of Water Resources, Jawalakhel, Lalitpur, Nepal

Subject: Request for agreement for shifting of intake of TIP

Date: 21/3/1997

Dear Sir,

We acknowledge the receipt of your letter dated 19/3/1997 regarding the discussion that you had with NEA in connection with the shifting of the intake site for Trisuli Irrigation Project (TIP). We would like to request you to make decision to fix the intake site as proposed in option 3 of your letter, i.e. down stream of the balancing reservior of Trisuli Hydel Project as we recommended earlier. We also appreciate your opinion about the appropriateness of the proposed intake (option 3). We would like to inform you that we will proceed our further study regarding the irrigation system planning based on the intake site of the option 3.

I would like to inform you that there was a discussion between the Managing Director of NEA and Mr. N. N. Vaidya of DOI along with Mr. A. Uchida, Assistant Representative, JICA and the Mr. Fujioka, Team Leader, JICA Study Team regarding the shifting of the intake for TIP. The Study Team Leader requested to both Managing Director and Mr. Vaidya to specify mutually agreeable terms and the conditions on option 3 of the shifting of intake within one month period.

Please get agreed the terms and conditions with Nepal Electricity Authority (NEA) for shifting of the intake as recommended. Please inform us the progress regarding the terms and conditions within one month to our CKC office in Tokyo.

Regards

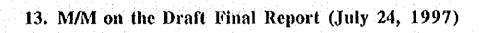
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Sincerely Yours.

Masamitsu Fujioka

Team Leader, JICA Study Team

## PartA



## Minutes of Meeting for Draft Final Report

## Feasibility Study on Trisuli Irrigation Project in the Kingdom of Nepal

Date:

July 24, 1997

Place:

Meeting Room of the Director General of the Department of

Irrigation, Ministry of Water Resources.

Attendants: As listed in the table attached hereto

JICA Study Team submitted the Draft Final Report on the Feasibility Study on Trisuli Irrigation Project in twenty (20) copies in total to the Department of Irrigation, Ministry of Water Resources on July 13 and 17, 1997.

On July 17, 1997, JICA Study Team explained to the officials concerned of the Department of Irrigation and other line agencies on the contents of the Draft Final Report at the conference room of the Department stressing that the project would address poverty alleviation through intensive agricultural development in the project area. At the meeting, high construction cost was pointed out by the Department of Irrigation, compared to other similar hill projects. In reply to the question, JICA Study Team justified its high construction cost due to geographical constraints in the hills, introduction of a pipeline system and limited construction period. The Study Team also explained that the shifting of the original intake from Aqueduct No. 2 to the downstream portion of the balancing reservoir (option 3) as proposed is the best alternative for the project.

As a result of discussions both parties have reached the following concurrence:

1. The Draft Final Report was in general accepted by the Department of Irrigation.

M.G. (g)

- 2. The Department of Irrigation agreed to provide comments on the Draft Final Report if any to JICA within one (1) month after receiving the Draft Final Report.
- 3. JICA Study Team agreed to submit the Final Report in fifty (50) copies to the Department of Irrigation through JICA within two (2) months after the receipt of comments from the Department.
- 4. The Final Report shall be opened to public whenever it is requested.

M.N.Aryal

**Director General** 

Department of Irrigation

Ministry of Water Resources

M.Fujioka

JICA Team Leader

Witnessed by

M. Kitanaka

Deputy Director

Agricultural Development Study Division Agriculture, Forestry and Fisheries Development Study Department

Japan International Cooperation Agency (JICA)

Full Name (in Block Letters)	Occupational Status	Name of Organization	Signature
Mahmers. N. Aryal	sirector General	DOZ	Le-
Mizing Na InVoisy	s.E.	Dos	6-51
	tori Chief Hydrogeologic	2 DOI	skymant of
Sungay Man Shrish		307	loc /
Maked Ber Parten	(Enginer (Engine)	DoI	NAMON
Navenda Gurun	Sr. Program. Officer	JICH	Amy
Atsushi Uchida	A.R.R.	JZCA	Ouchde
Yatsuhira CHIB	A JICA expect to DO.I		y.cliba
	-		
MAKOTO KITANAKA	Deputy Director	JICA	hiber
. MASAMITSU FUJIOKA	Leader of JICA Study Team		My Sport
KUNIKI IWATA	Member of HCA Study Team		Twool
NOBUKI TOYOOKA	Member of HCA Study Team		1 sul

## List of Attendants

Full Name	Occupational Status	Name of Organization
M.N Aryal	D.G	DOI
N.N Vaidya	S.E	DOI
T.S Bhandari	Chief Hydrogeologist	DOI
S.M Shrestha	DDG	DOI
M.B Pradhan	Engineer	DOI
N. Gurung	Senior Program Officer	JICA
A. Uchida	A.R.R	JICA
Y. Chiba	JICA expert	DOI
M. Kitanaka	Deputy Director	JICA
M. Fujioka	Team Leader	JICA Study Team
K. Iwata	Irrigation and Drainage	JICA Study Team
N. Toyooka	Agro Economy / Project Evaluation	