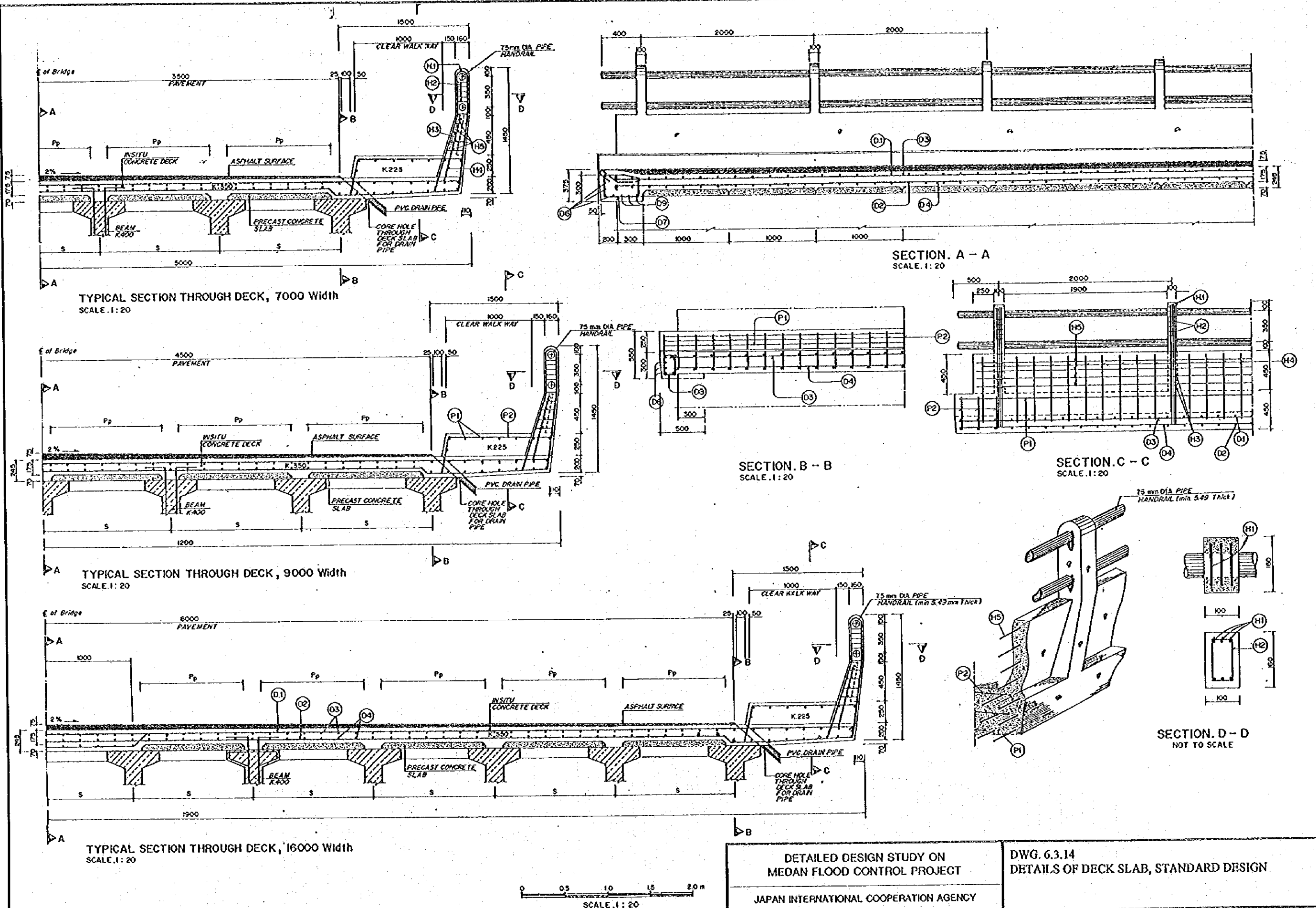


<p>DETAILED DESIGN STUDY ON MEDAN FLOOD CONTROL PROJECT</p> <p>JAPAN INTERNATIONAL COOPERATION AGENCY</p>	<p>DWG. 6.3.13 GENERAL PLAN OF JL. SMA - 12 BRIDGE (F7)</p>
---	---



TYPICAL SECTION THROUGH DECK, 7000 Width  
SCALE 1:20

TYPICAL SECTION THROUGH DECK, 9000 Width  
SCALE 1:20

TYPICAL SECTION THROUGH DECK, 16000 Width  
SCALE 1:20

SECTION A - A  
SCALE 1:20

SECTION B - B  
SCALE 1:20

SECTION C - C  
SCALE 1:20

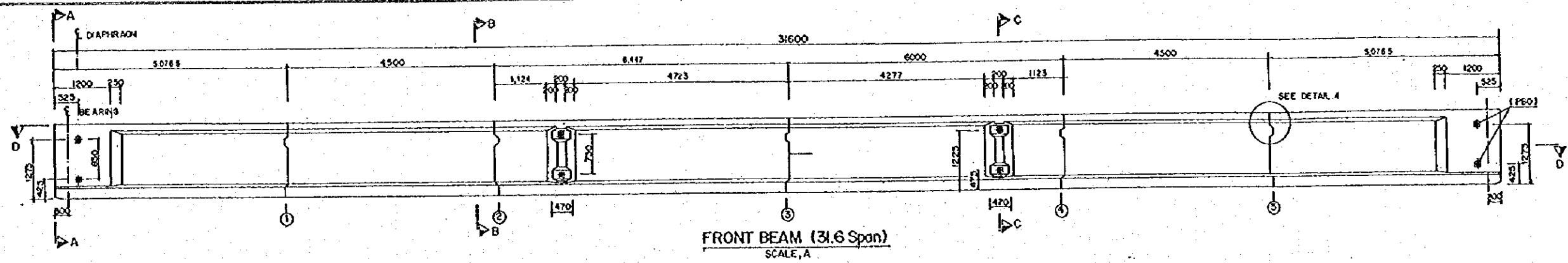
SECTION D - D  
NOT TO SCALE

DETAILED DESIGN STUDY ON  
MEDAN FLOOD CONTROL PROJECT  
JAPAN INTERNATIONAL COOPERATION AGENCY

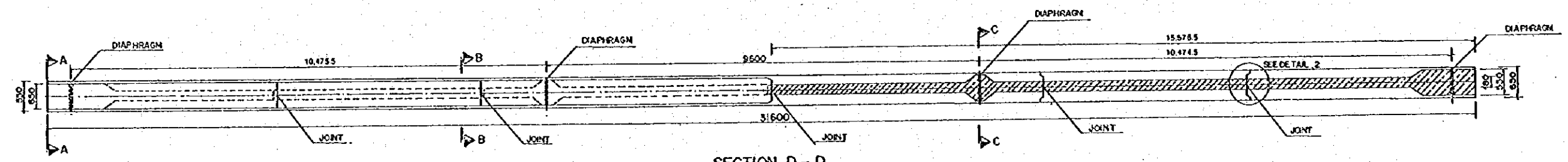
DWG. 6.3.14  
DETAILS OF DECK SLAB, STANDARD DESIGN

6-59

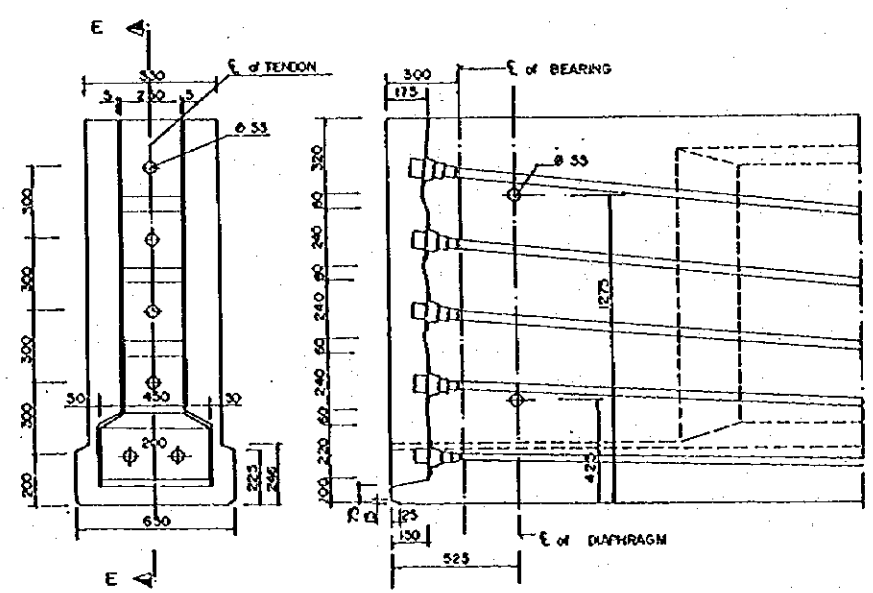
D - 6 - 59



FRONT BEAM (31.6 Span)  
SCALE, A

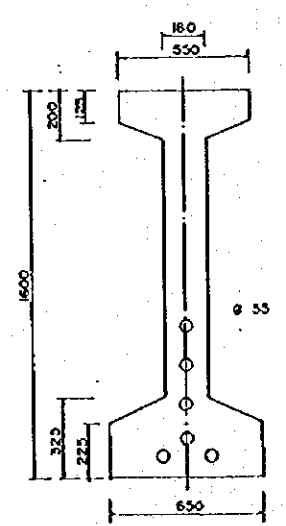


SECTION D - D  
SCALE, A

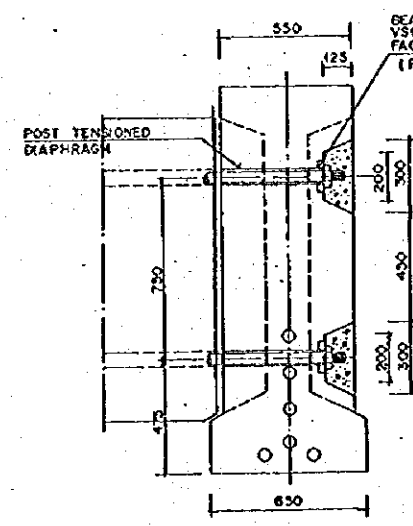


SECTION A - A  
SCALE, B

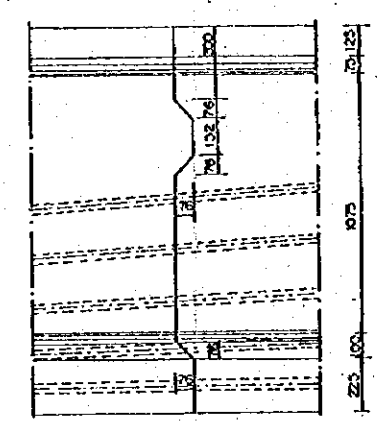
SECTION E - E  
SCALE, B



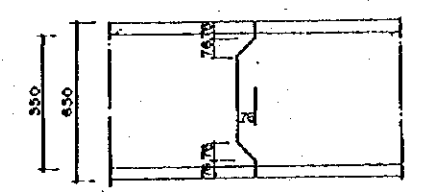
SECTION B - B  
SCALE, B



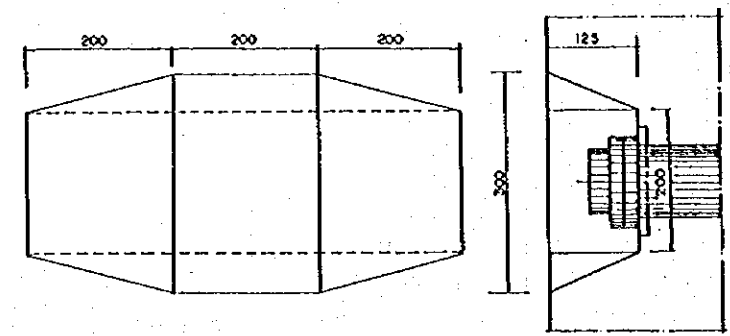
SECTION C - C  
SCALE, B



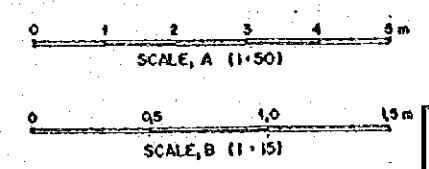
DETAIL 1  
SCALE, B



DETAIL 2  
SCALE, B

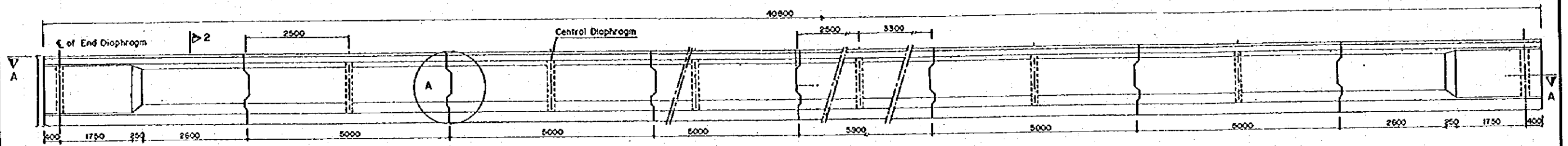


PRECAST OF COVERING FACE EXTERIOR BEAMS

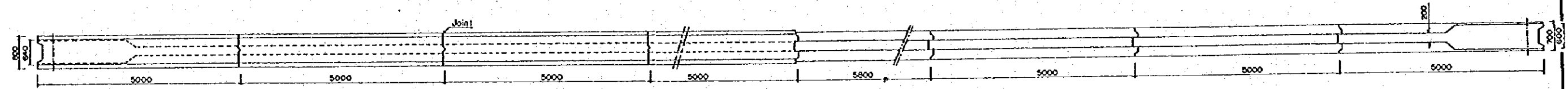


DETAILED DESIGN STUDY ON  
MEDAN FLOOD CONTROL PROJECT  
JAPAN INTERNATIONAL COOPERATION AGENCY

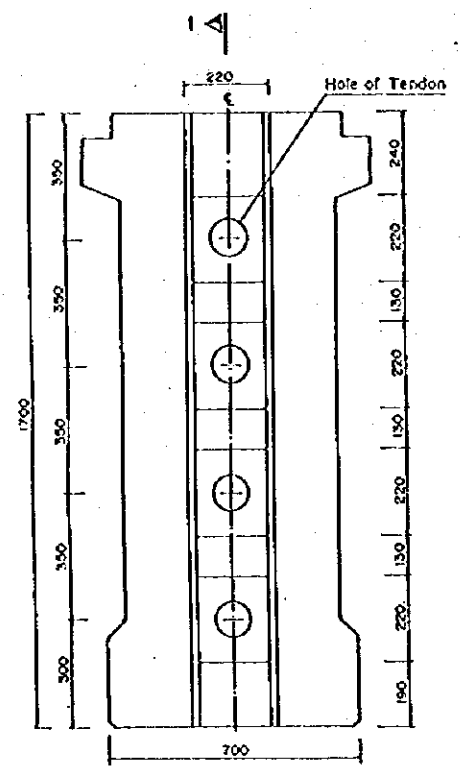
DWG. 6.3.15  
DETAILS OF PC-GIRDER, STANDARD DESIGN,  
L = 31.6 M



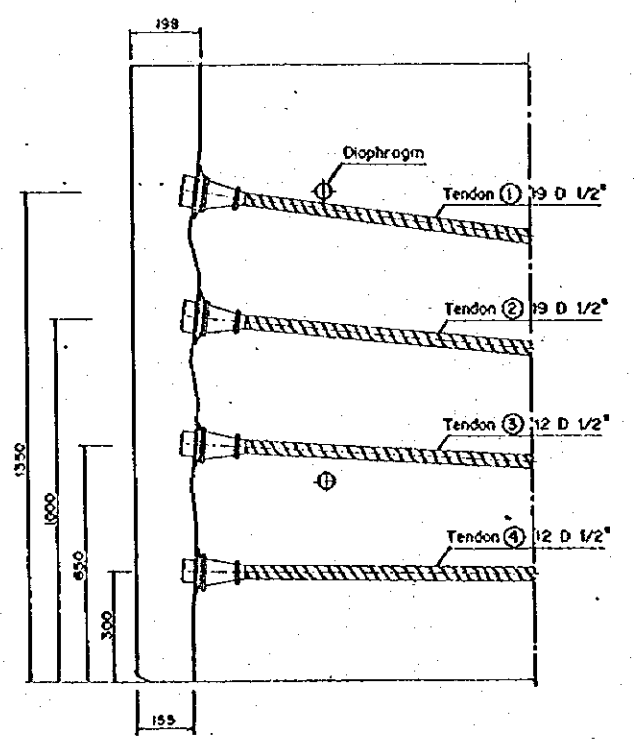
FRONT BEAM (40.80 Span)  
SCALE, A



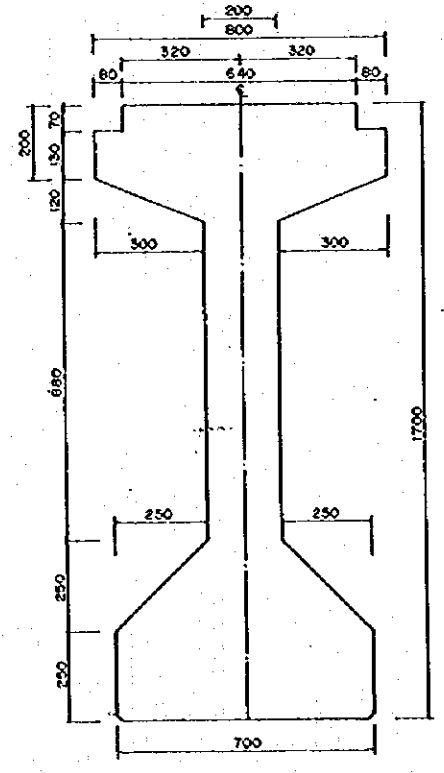
SECTION, A - A  
SCALE, A



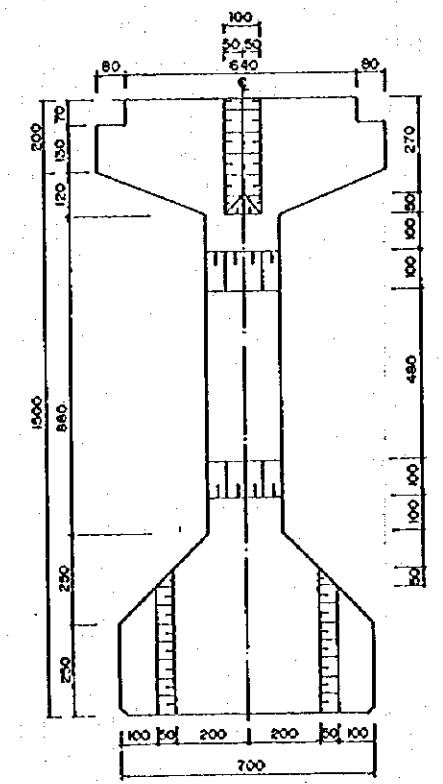
END VIEW  
SCALE, B



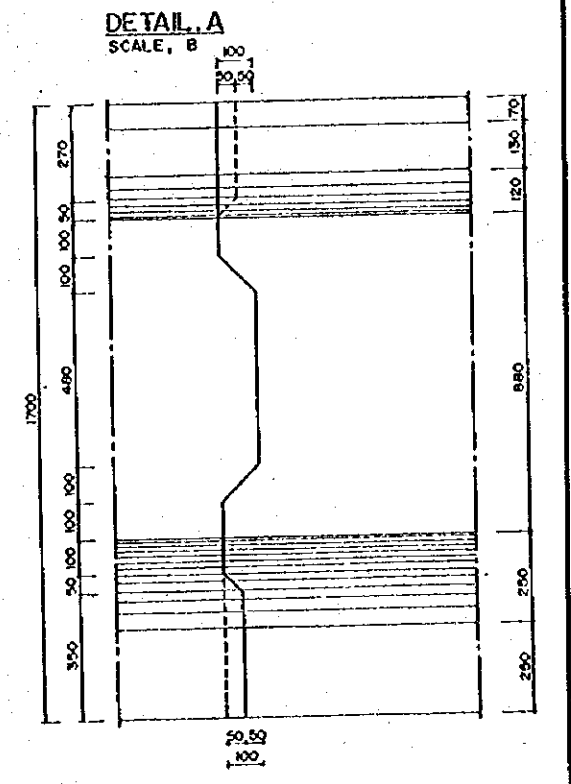
SECTION, 1  
SCALE, B



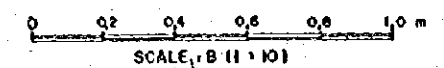
SECTION, 2  
SCALE, B



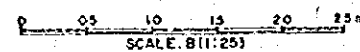
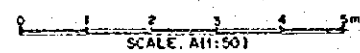
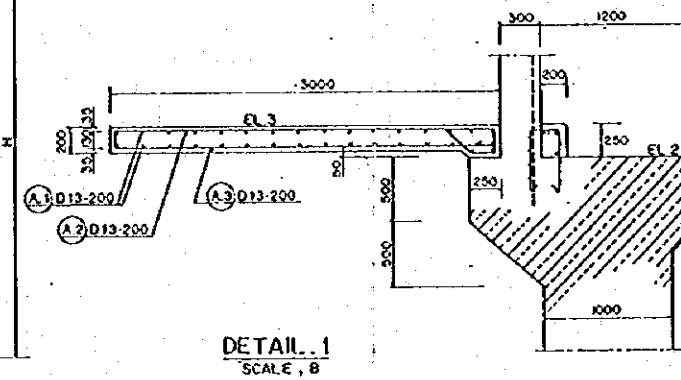
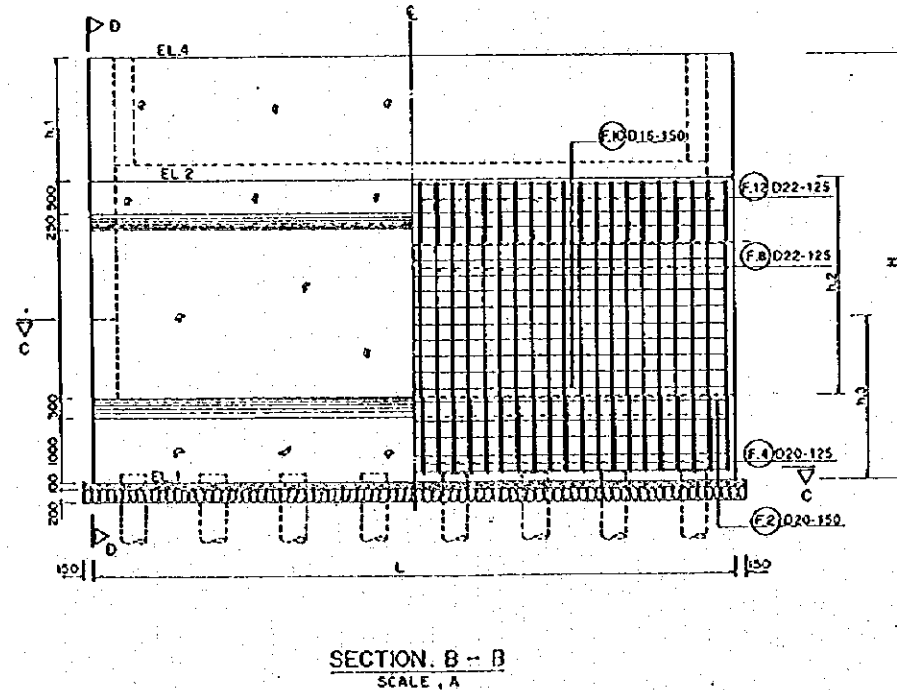
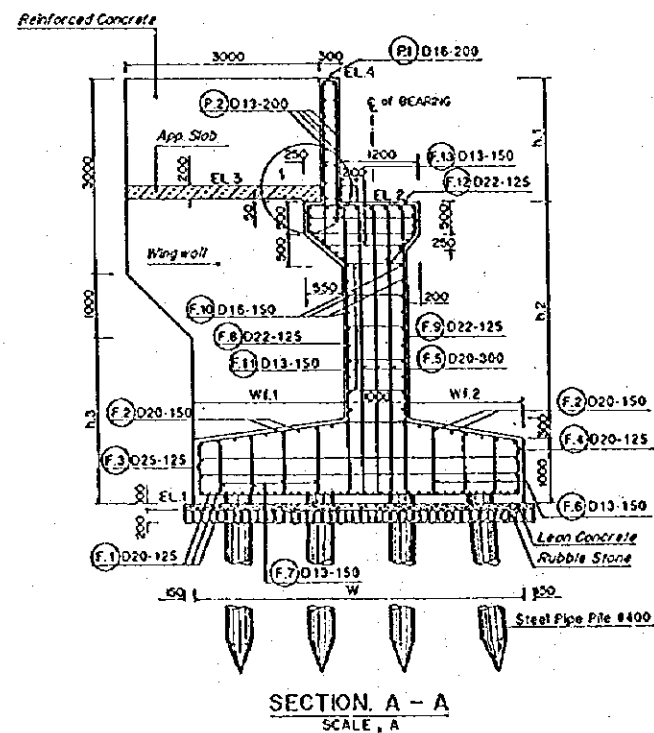
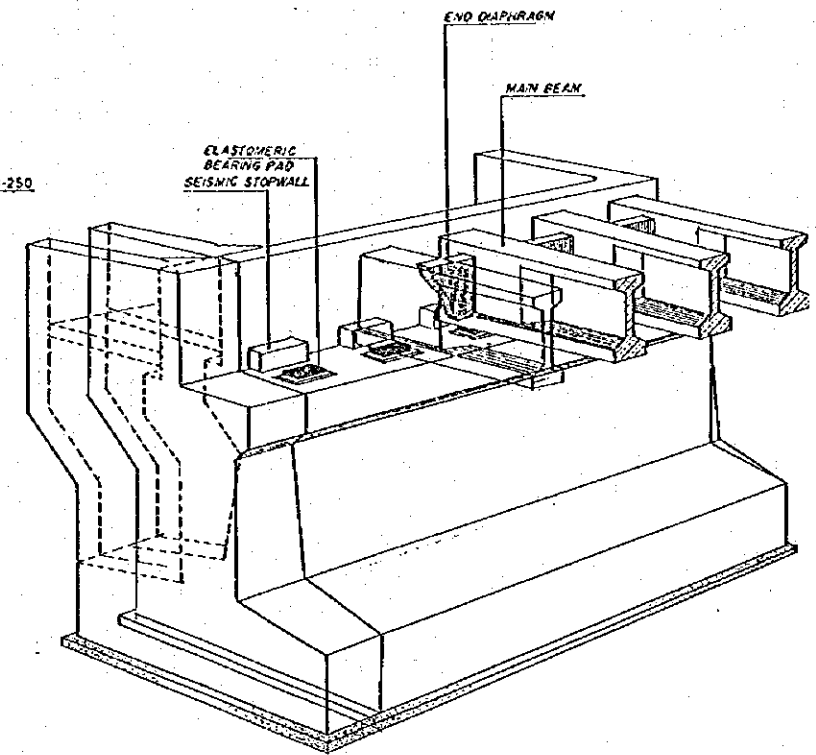
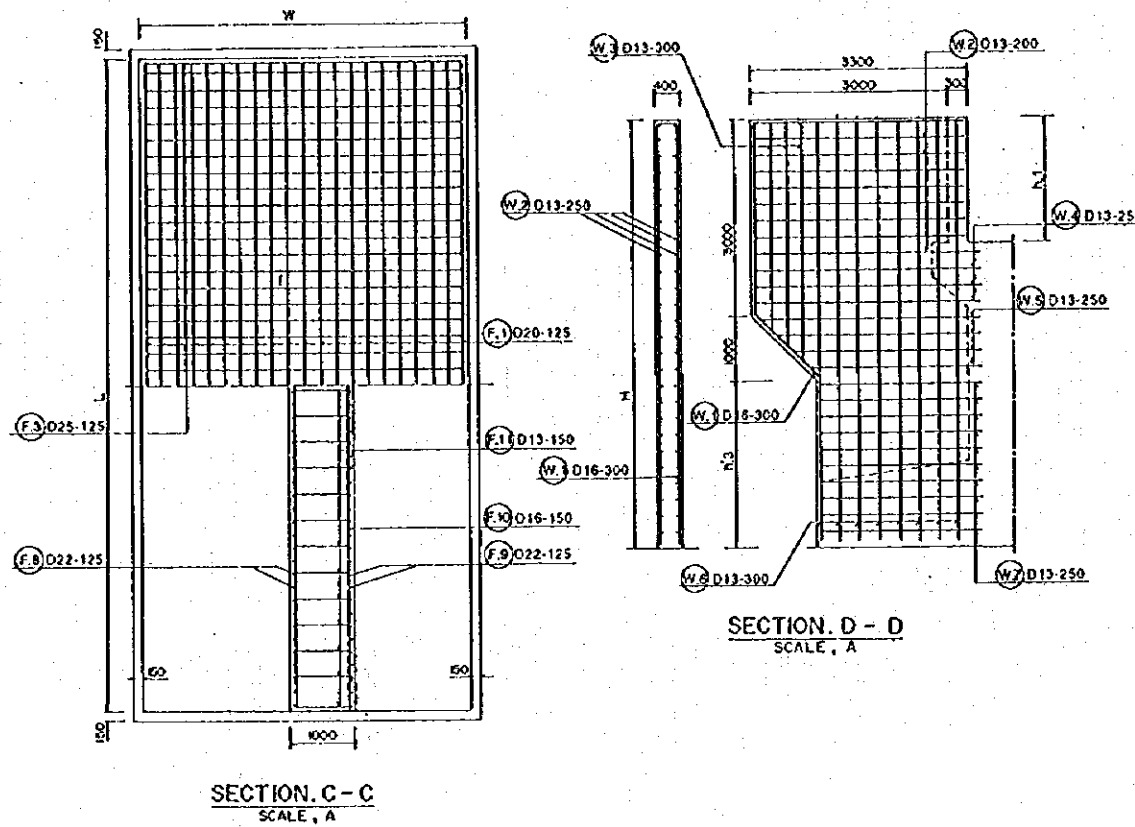
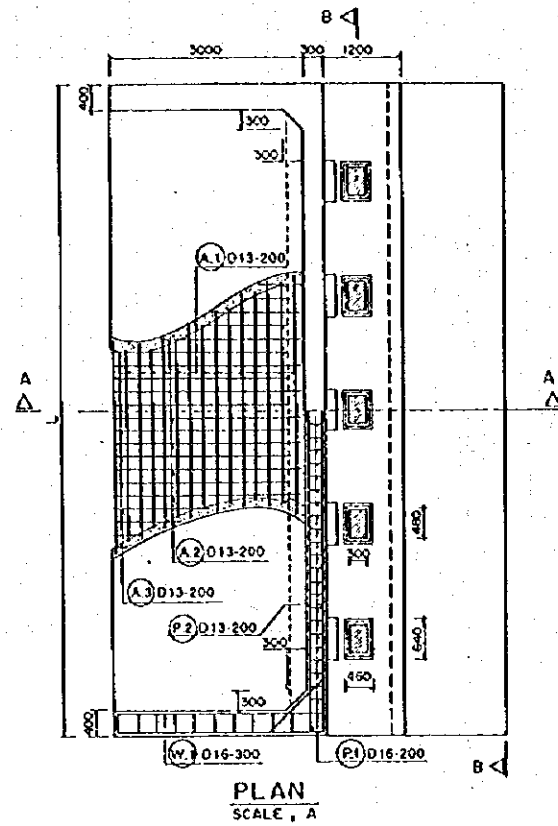
VIEW ON JOINT  
SCALE, B



DETAIL, A  
SCALE, B

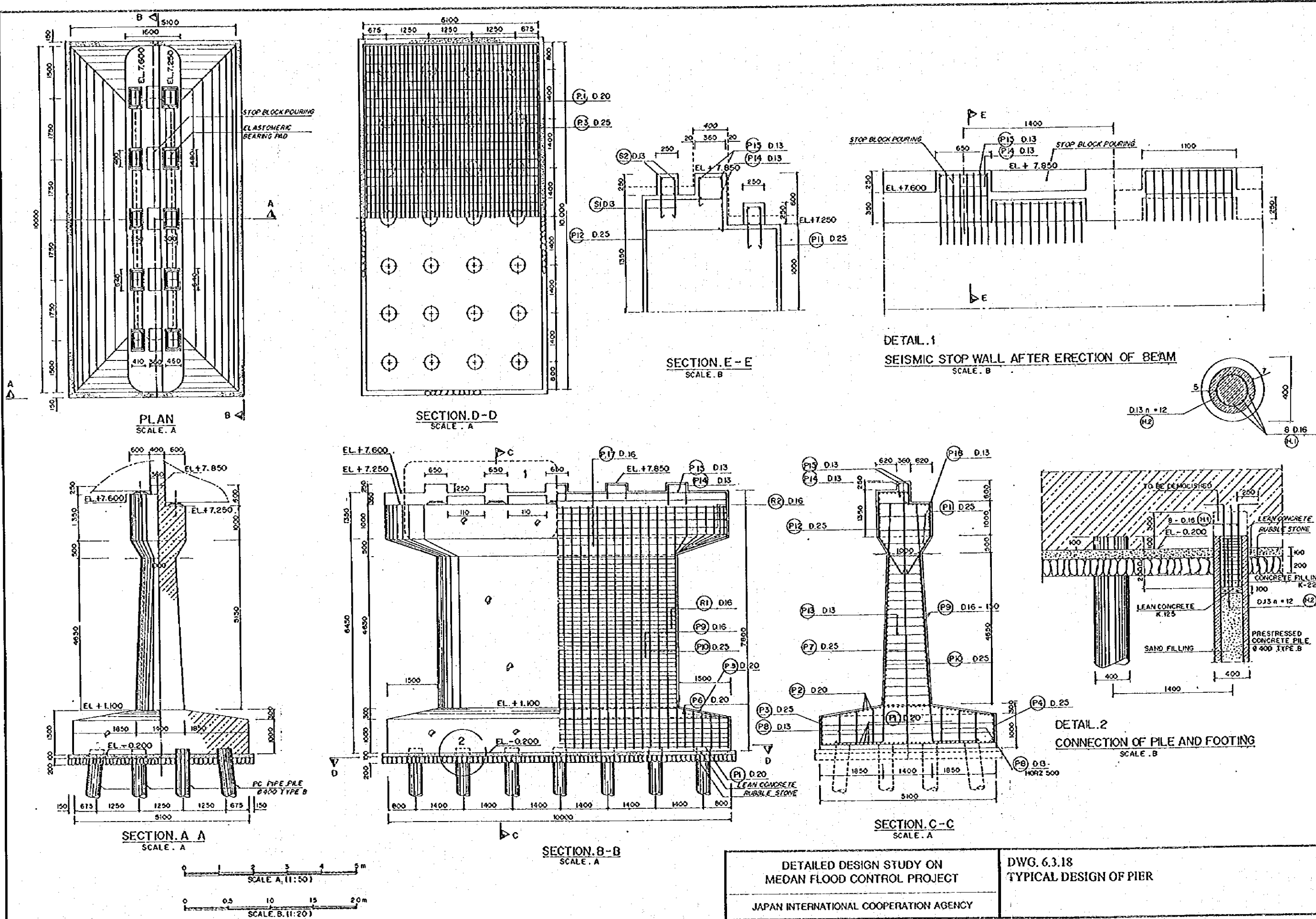


<p>DETAILED DESIGN STUDY ON MEDAN FLOOD CONTROL PROJECT</p> <p>JAPAN INTERNATIONAL COOPERATION AGENCY</p>	<p>DWG. 6.3.16 DETAILS OF PC-GIRDER, STANDARD DESIGN, L = 40.8 M</p>
---	--



DETAILED DESIGN STUDY ON  
MEDAN FLOOD CONTROL PROJECT  
JAPAN INTERNATIONAL COOPERATION AGENCY

DWG. 6.3.17  
TYPICAL DESIGN OF ABUTMENT



PLAN  
SCALE . A

SECTION D-D  
SCALE . A

SECTION E-E  
SCALE . B

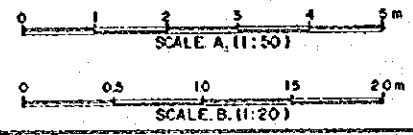
DETAIL 1  
SEISMIC STOP WALL AFTER ERECTION OF BEAM  
SCALE . B

SECTION A-A  
SCALE . A

SECTION B-B  
SCALE . A

SECTION C-C  
SCALE . A

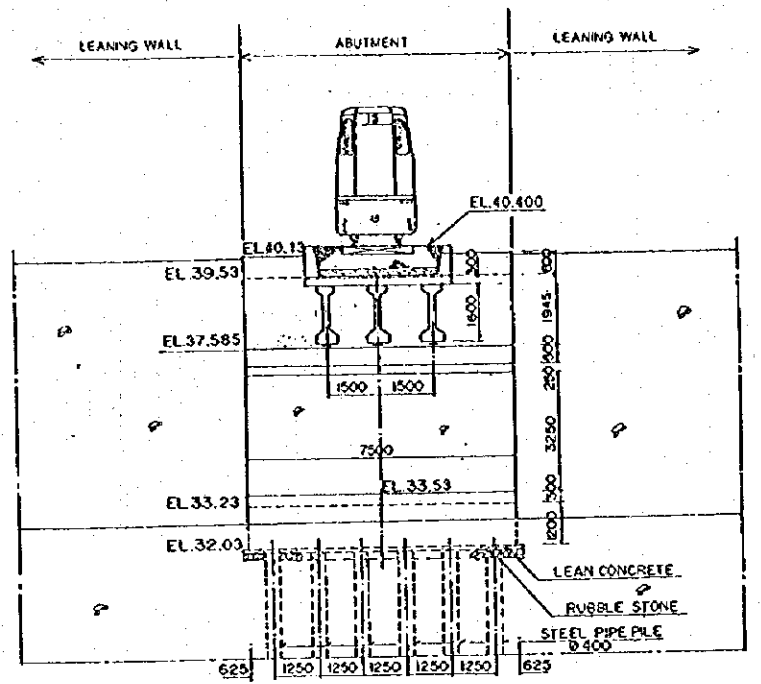
DETAIL 2  
CONNECTION OF PILE AND FOOTING  
SCALE . B



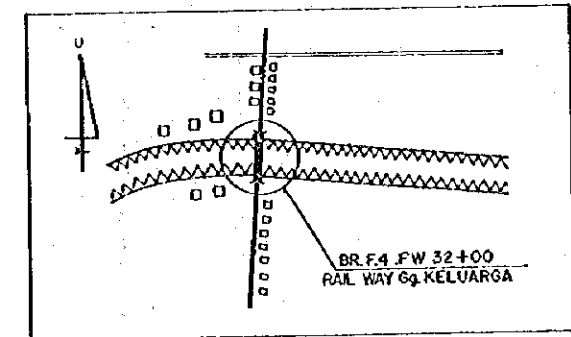
DETAILED DESIGN STUDY ON  
MEDAN FLOOD CONTROL PROJECT

JAPAN INTERNATIONAL COOPERATION AGENCY

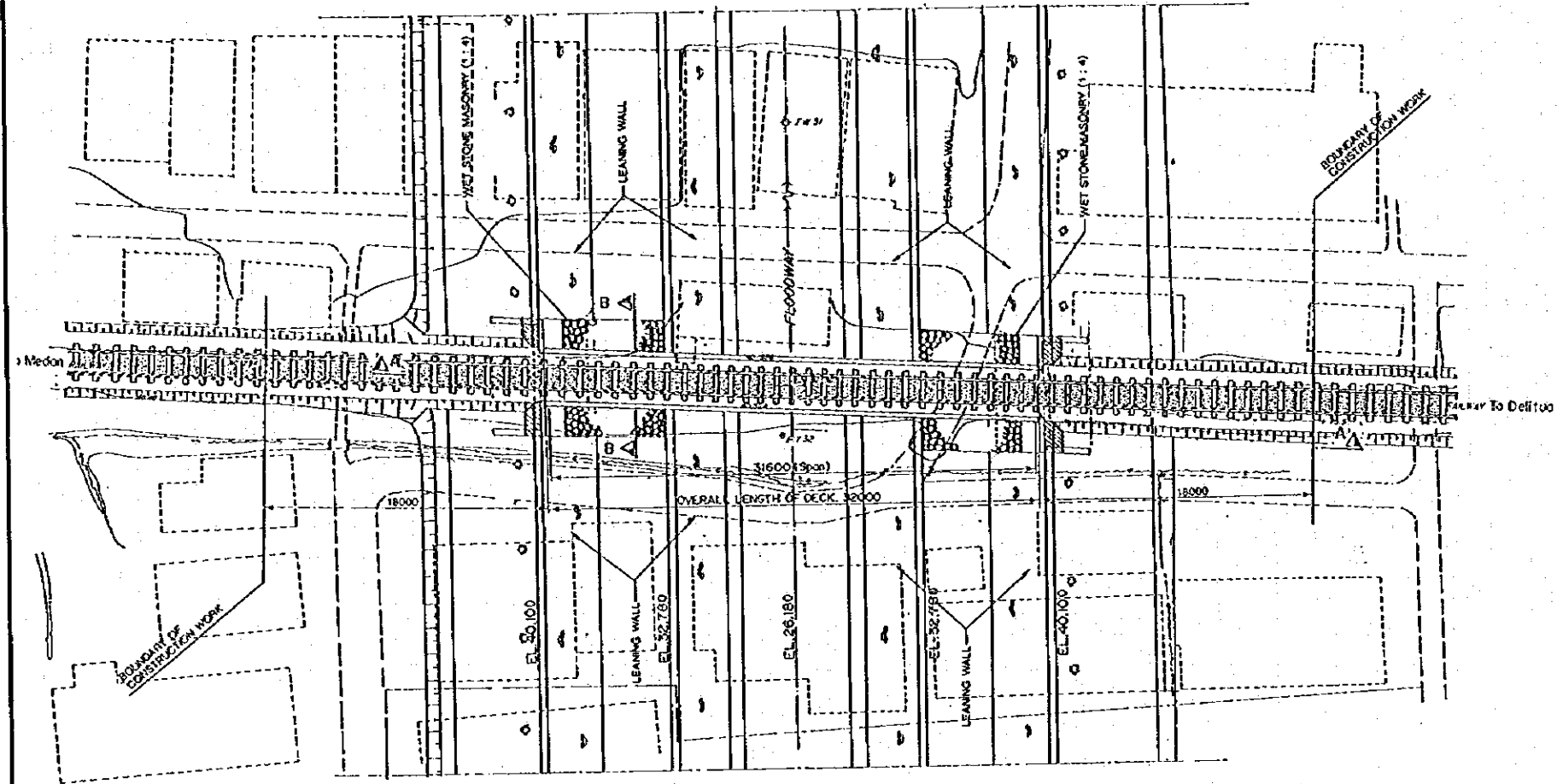
DWG. 6.3.18  
TYPICAL DESIGN OF PIER



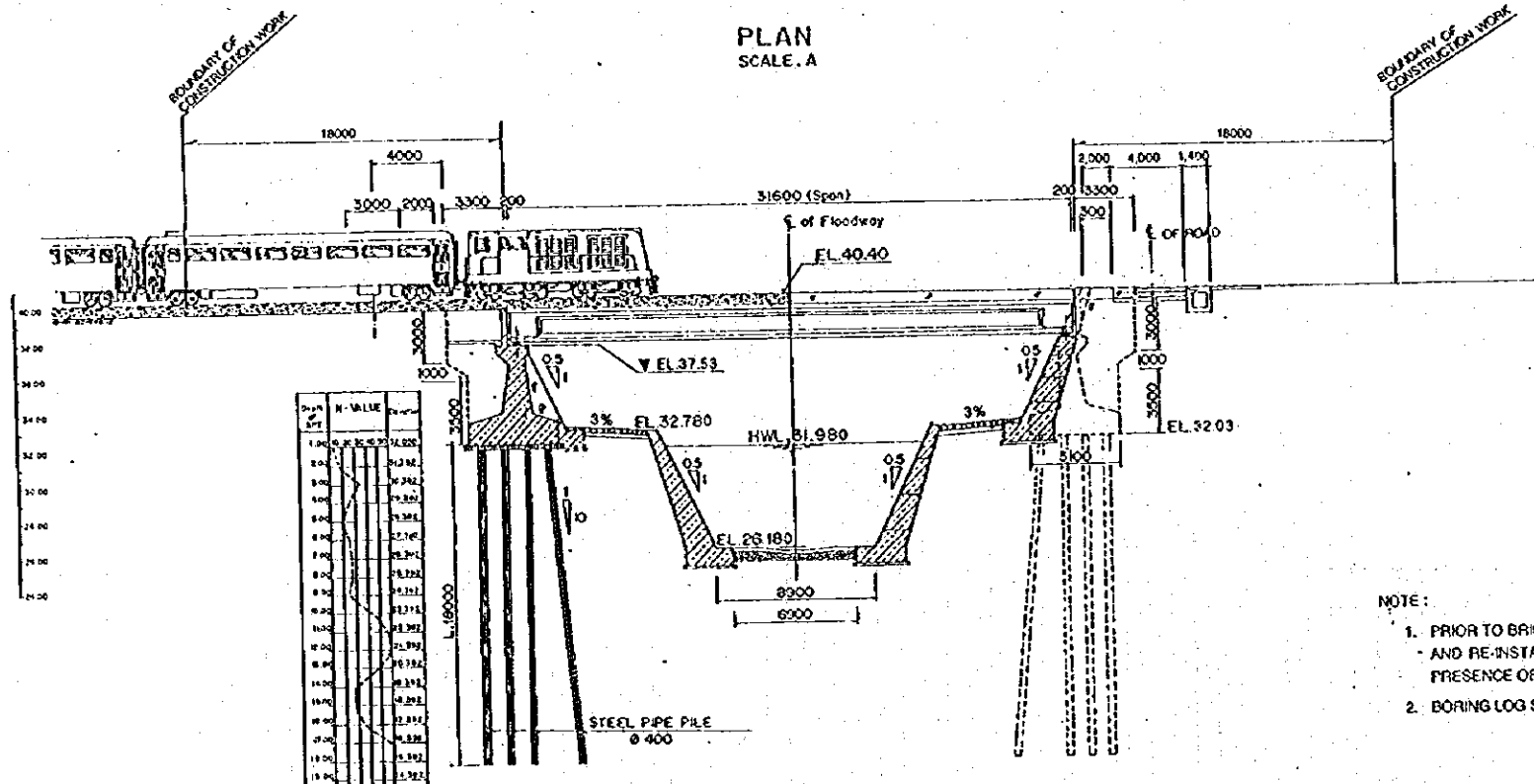
SECTION B-B  
SCALE: B



KEY PLAN  
NOT TO SCALE



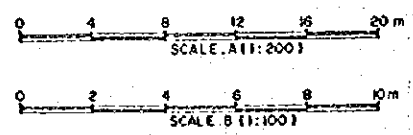
PLAN  
SCALE: A



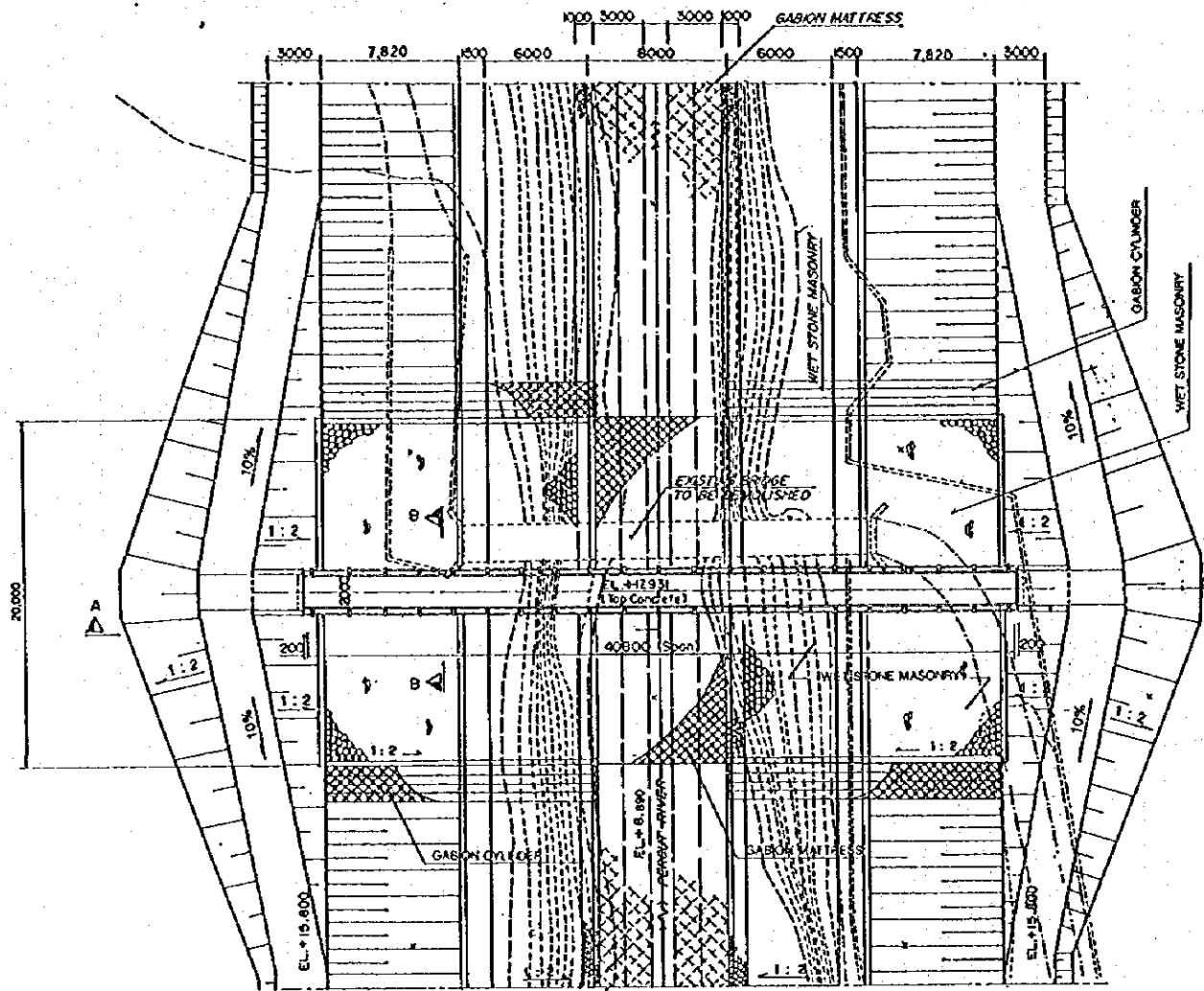
SECTION A-A  
SCALE: A

NOTE:

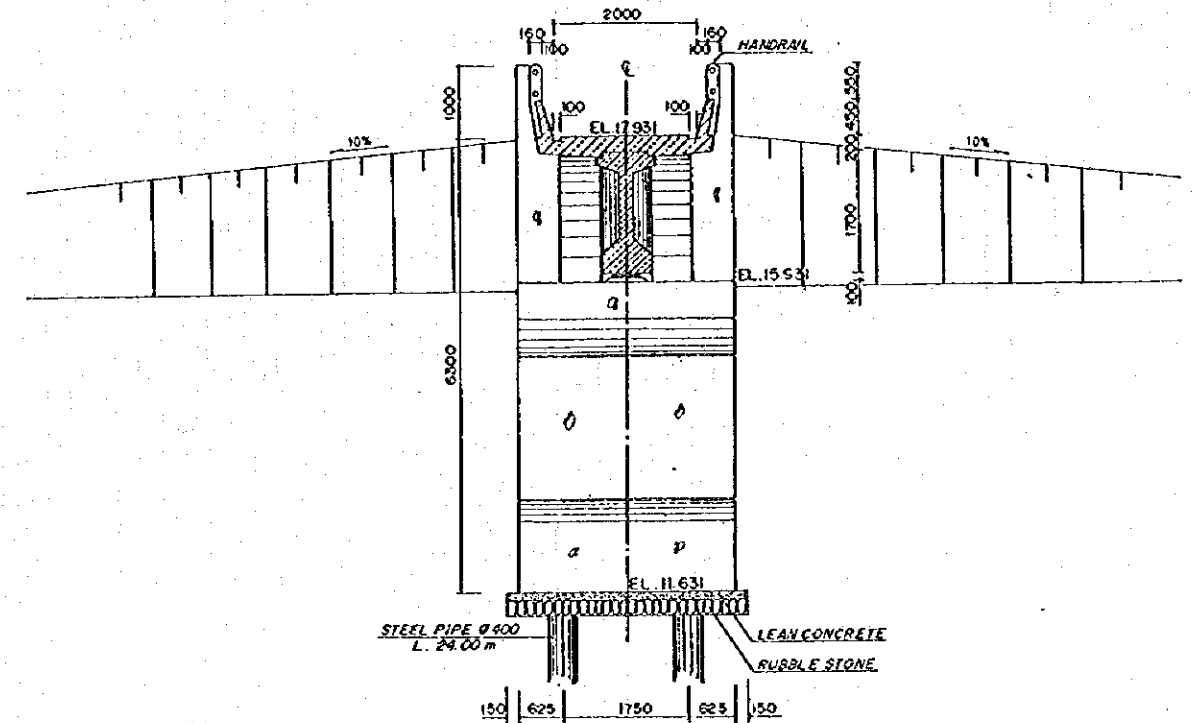
1. PRIOR TO BRIDGE WORK, THE EXISTING RAILS IN THE AREA DESIGNATED AND SHOWN ON THE DRAWING SHALL BE REMOVED, AND RE-INSTALLED ON THE BALLASTED FLOOR OF THE NEWLY BUILT BRIDGE. THE WORK SHALL BE PERFORMED IN THE PRESENCE OF THE ENGINEER OF PJK (PERUSAHAAN JAWATAN KERETA API).
2. BORING LOG SHOWN ON THIS DRAWING IS FOR REFERENCE ONLY.



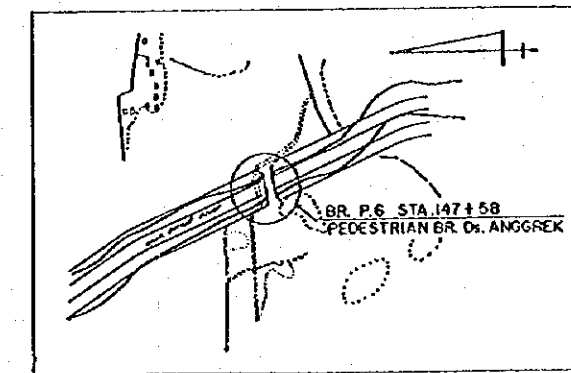
<p>DETAILED DESIGN STUDY ON MEDAN FLOOD CONTROL PROJECT</p>	<p>DWG. 6.3.19 GENERAL PLAN OF RAILWAY BRIDGE (F4)</p>
<p>JAPAN INTERNATIONAL COOPERATION AGENCY</p>	



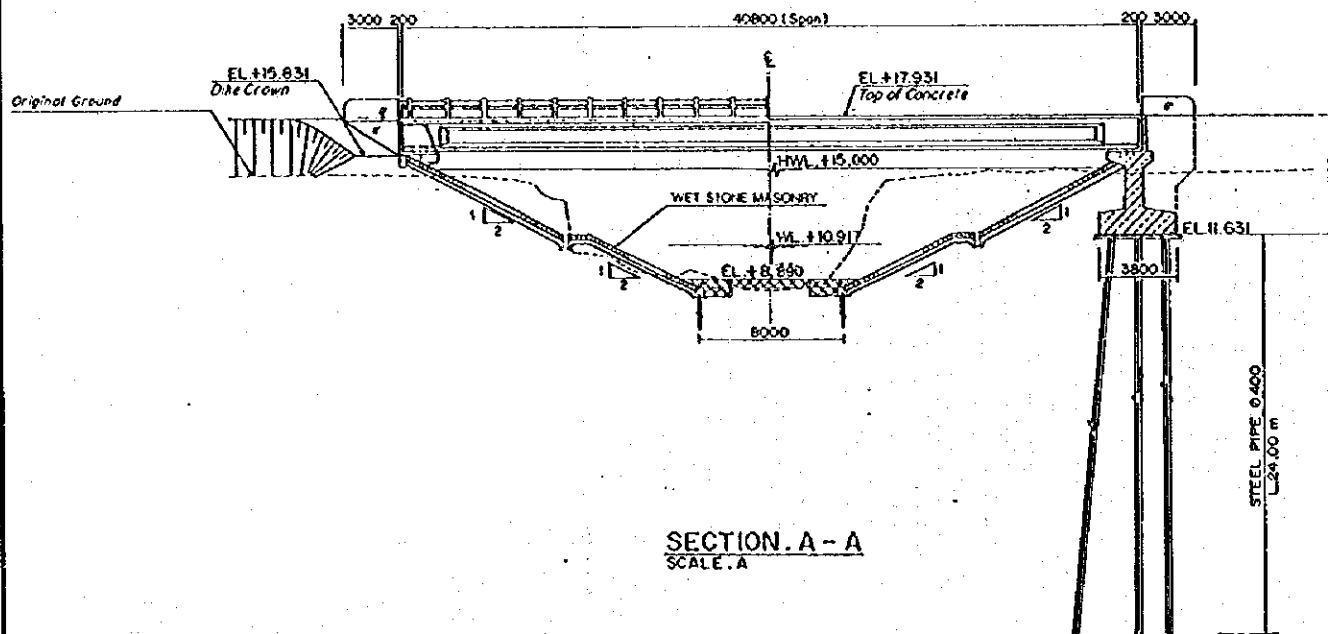
PLAN  
SCALE: A



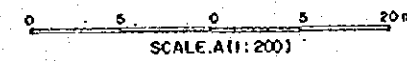
SECTION B - B  
SCALE: B



KEY PLAN  
NOT TO SCALE



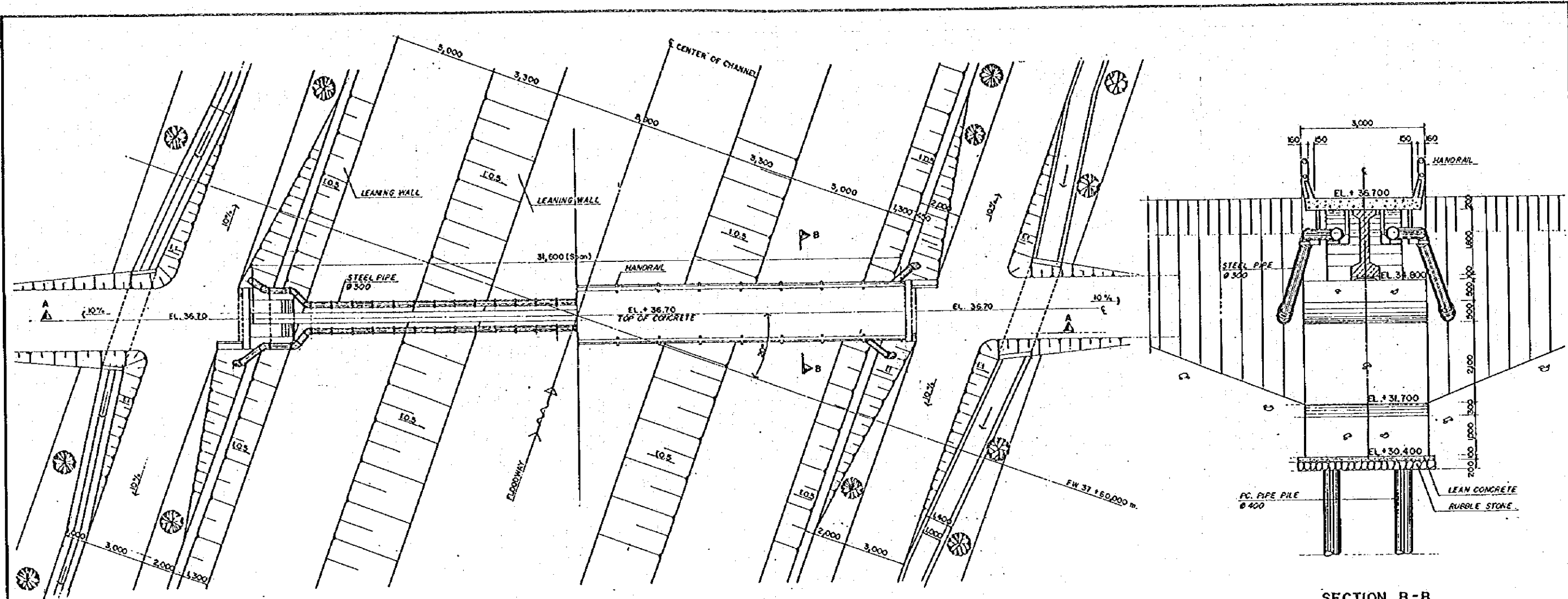
SECTION A - A  
SCALE: A



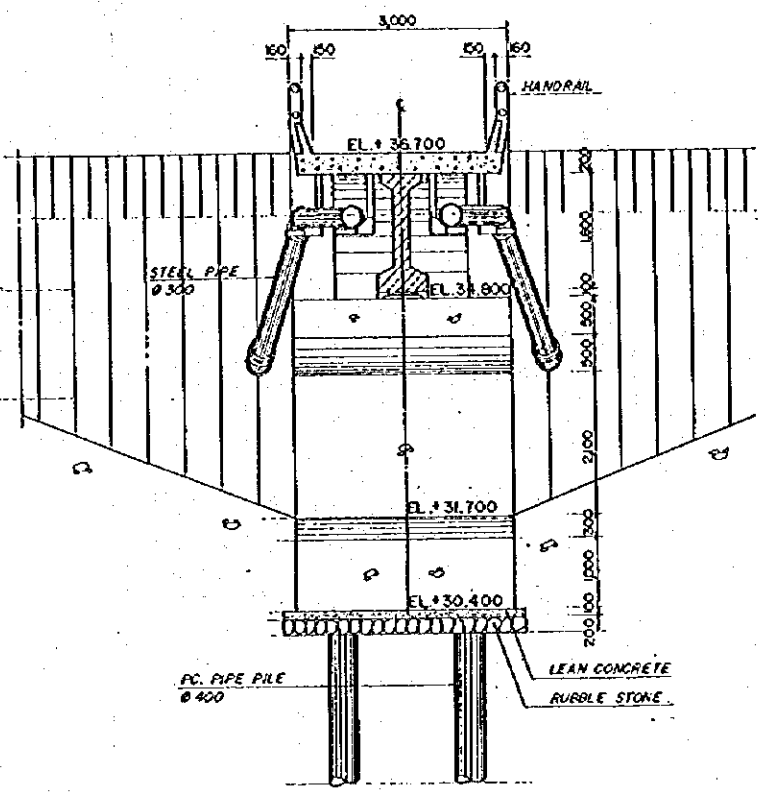
DETAILED DESIGN STUDY ON  
MEDAN FLOOD CONTROL PROJECT  
JAPAN INTERNATIONAL COOPERATION AGENCY

DWG. 6.3.20  
GENERAL PLAN OF DUSUN ANGGREK BRIDGE  
(P6, PEDESTRIAN BR.)

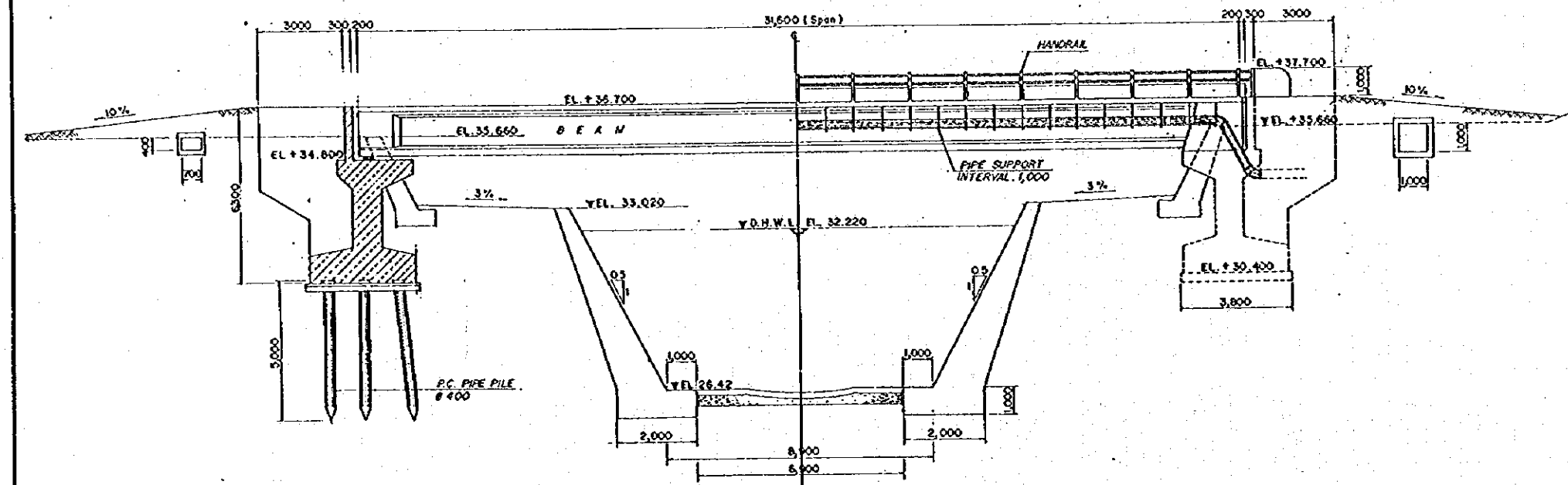




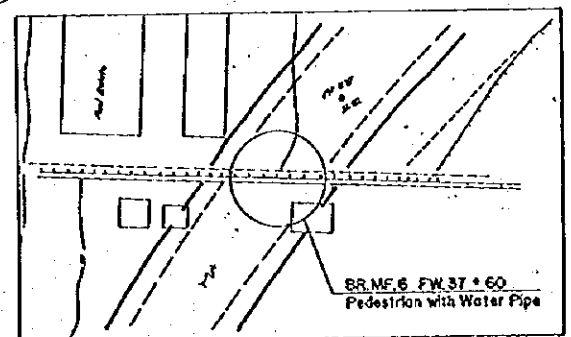
PLAN  
SCALE: A



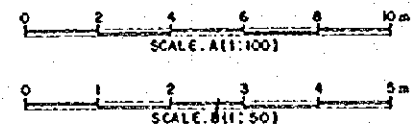
SECTION. B-B  
SCALE: B



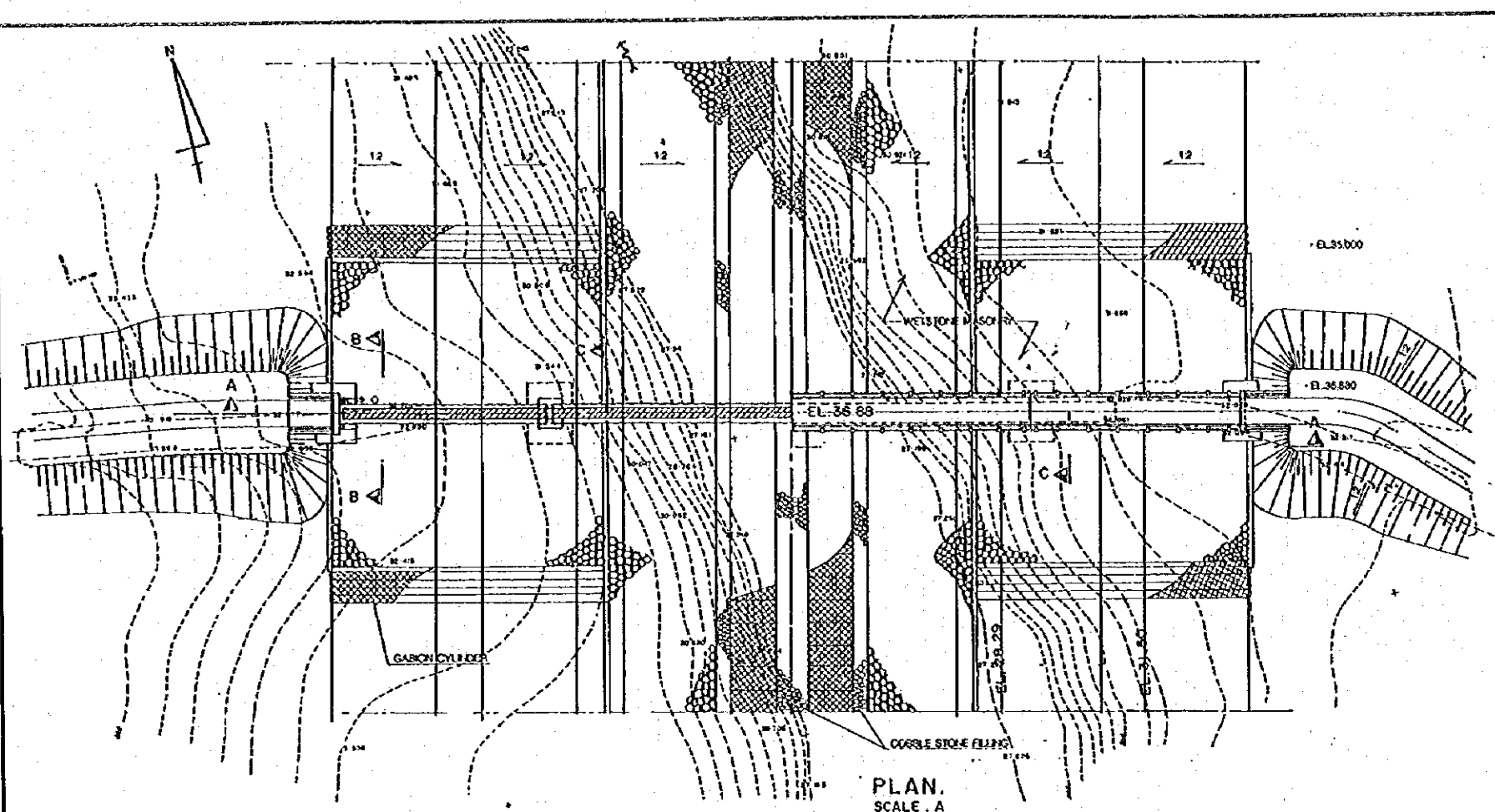
SECTION. A-A  
SCALE: A



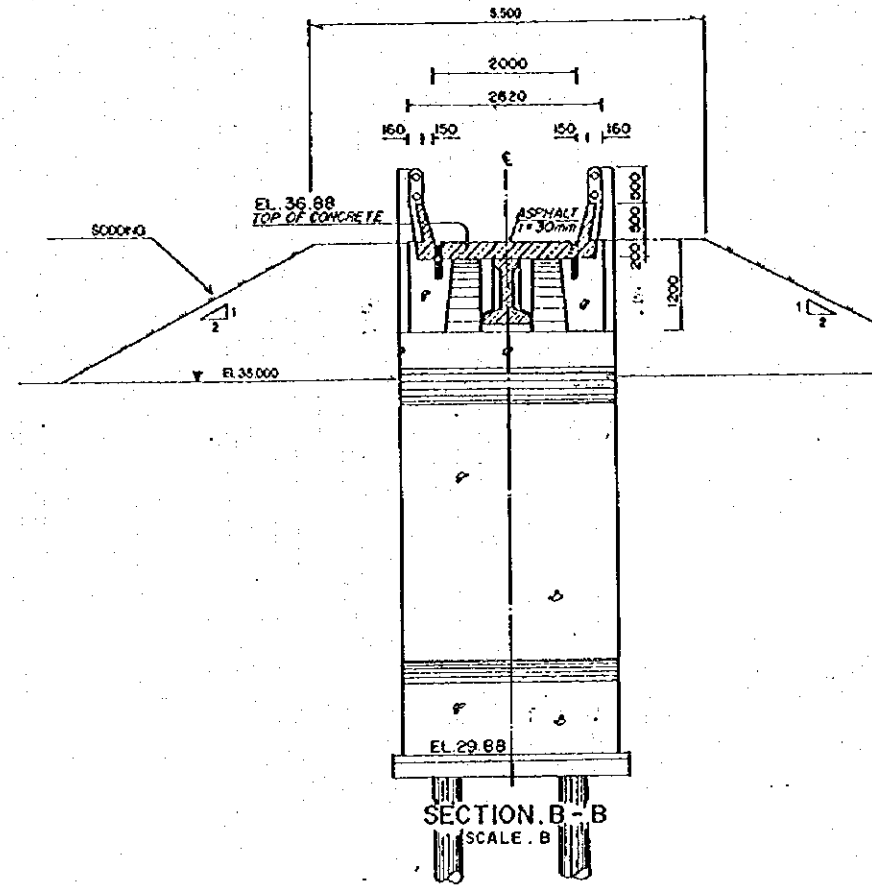
KEY PLAN  
NOT TO SCALE



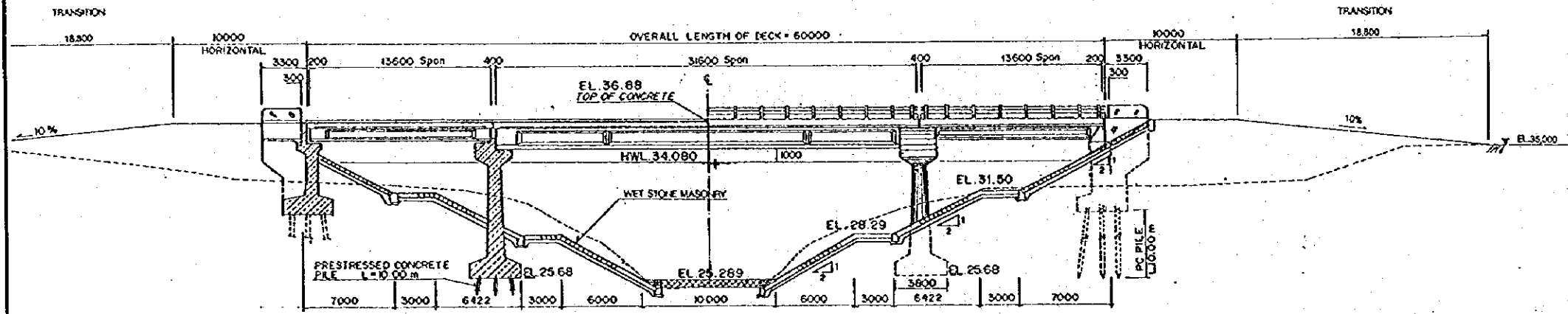
<p>DETAILED DESIGN STUDY ON MEDAN FLOOD CONTROL PROJECT</p>	<p>DWG. 6.3.21 GENERAL PLAN OF PEDESTRIAN BRIDGE (F6)</p>
<p>JAPAN INTERNATIONAL COOPERATION AGENCY</p>	



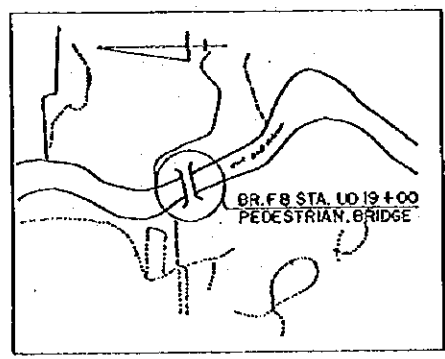
PLAN.  
SCALE . A



SECTION B-B  
SCALE . B



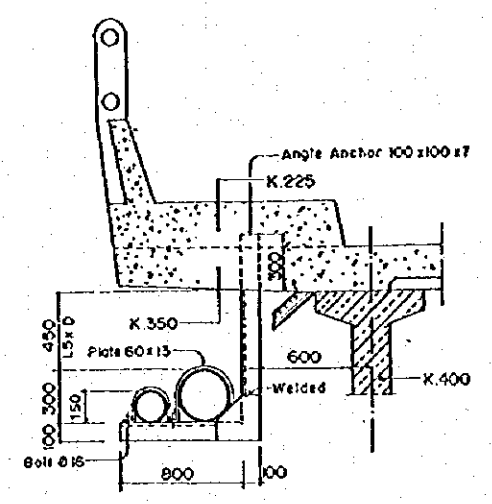
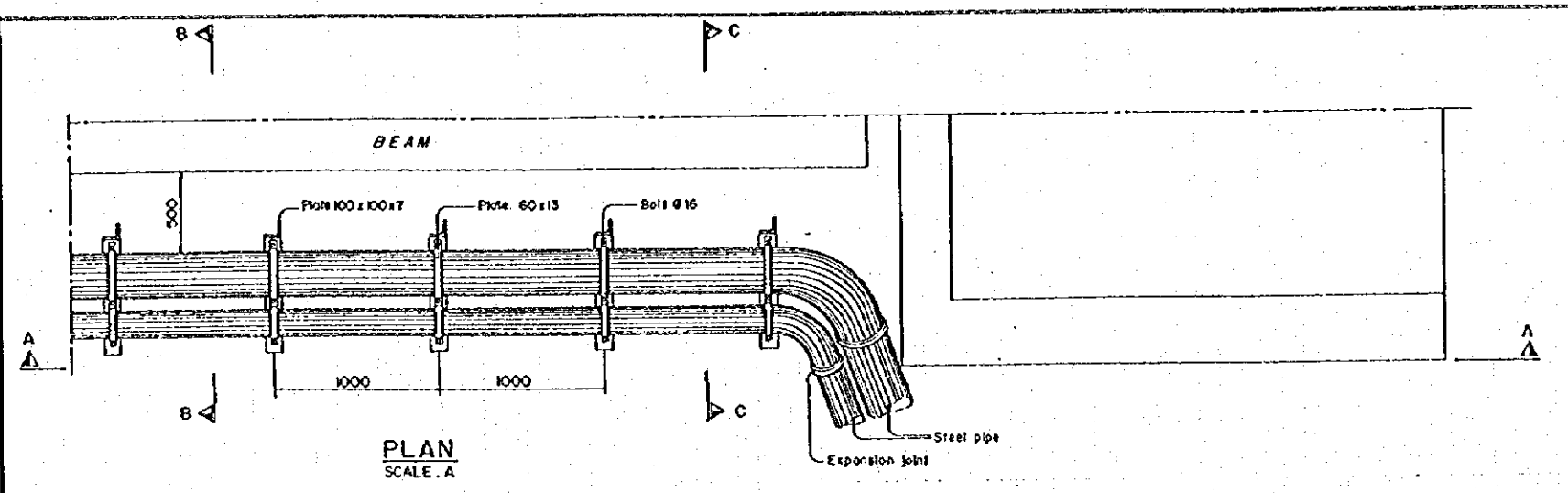
SECTION A-A  
SCALE . A



KEY PLAN  
NOT TO SCALE



<p>DETAILED DESIGN STUDY ON MEDAN FLOOD CONTROL PROJECT</p>	<p>DWG. 6.3.22 GENERAL PLAN OF GG. SEKSAMA BRIDGE (F8, PEDESTRIAN BR.)</p>
<p>JAPAN INTERNATIONAL COOPERATION AGENCY</p>	

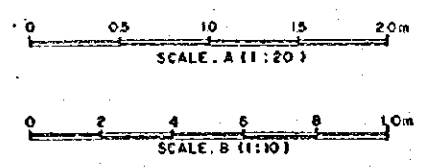
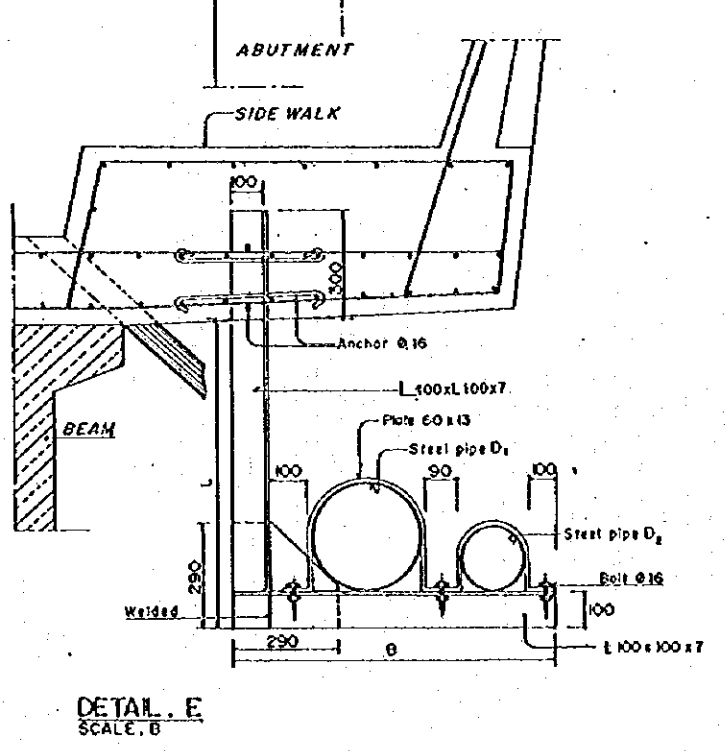
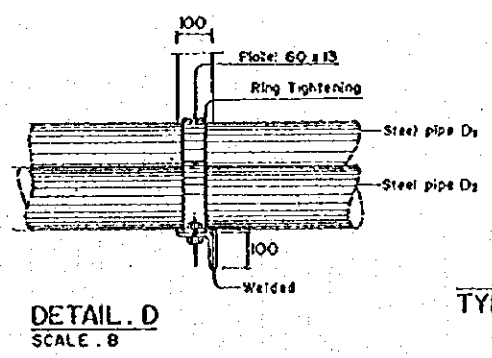
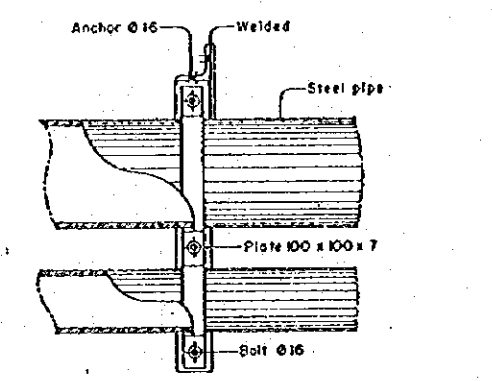
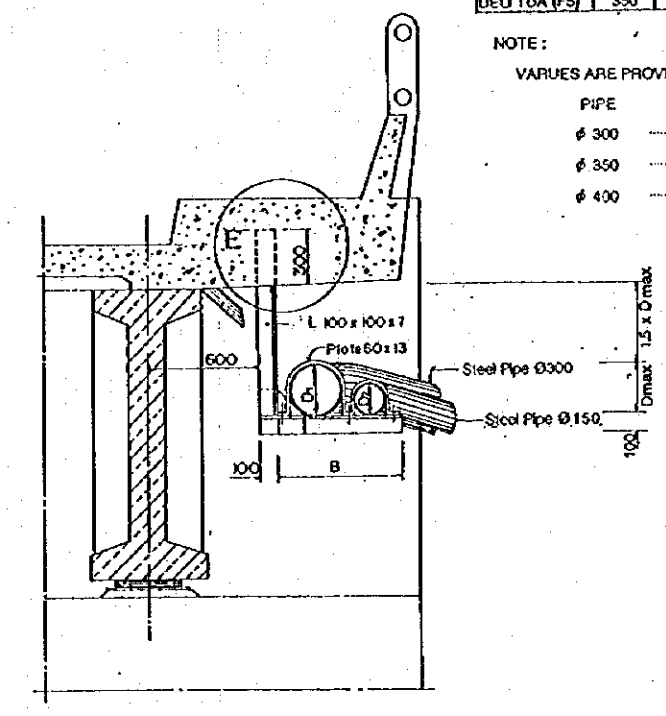
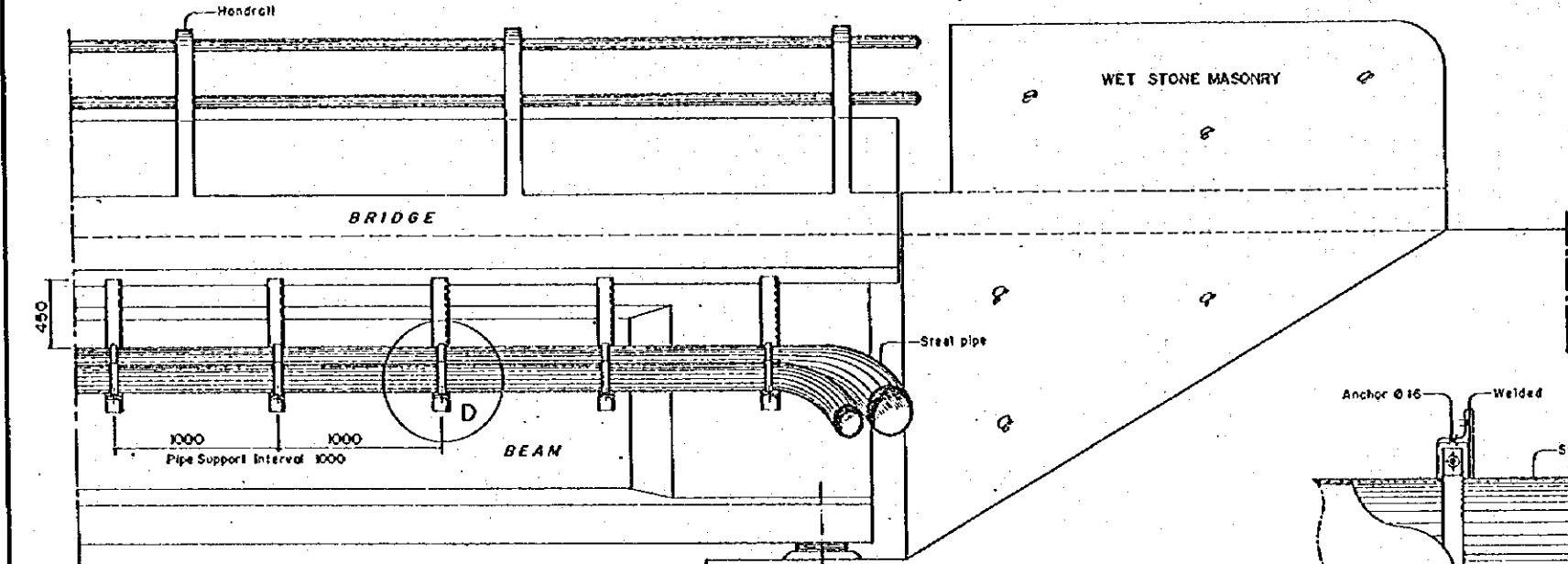


DIMENSIONS OF PIPE AND HANG SUPPORT

NAME OF BRIDGE	DIAMETER OF PIPE (mm)		HANG SUPPORT (mm)	
	Ø1	D2	L	B
MEDAN TEMBUNG (P7)	100	-	350	400
MEDAN DENAI (P9)	150	100	500	650
BINJAI (P11)	400	150	1,100	1,000
AMPLAS (P13)	300	150	850	800
DEU TUA (FS)	350	-	600	450

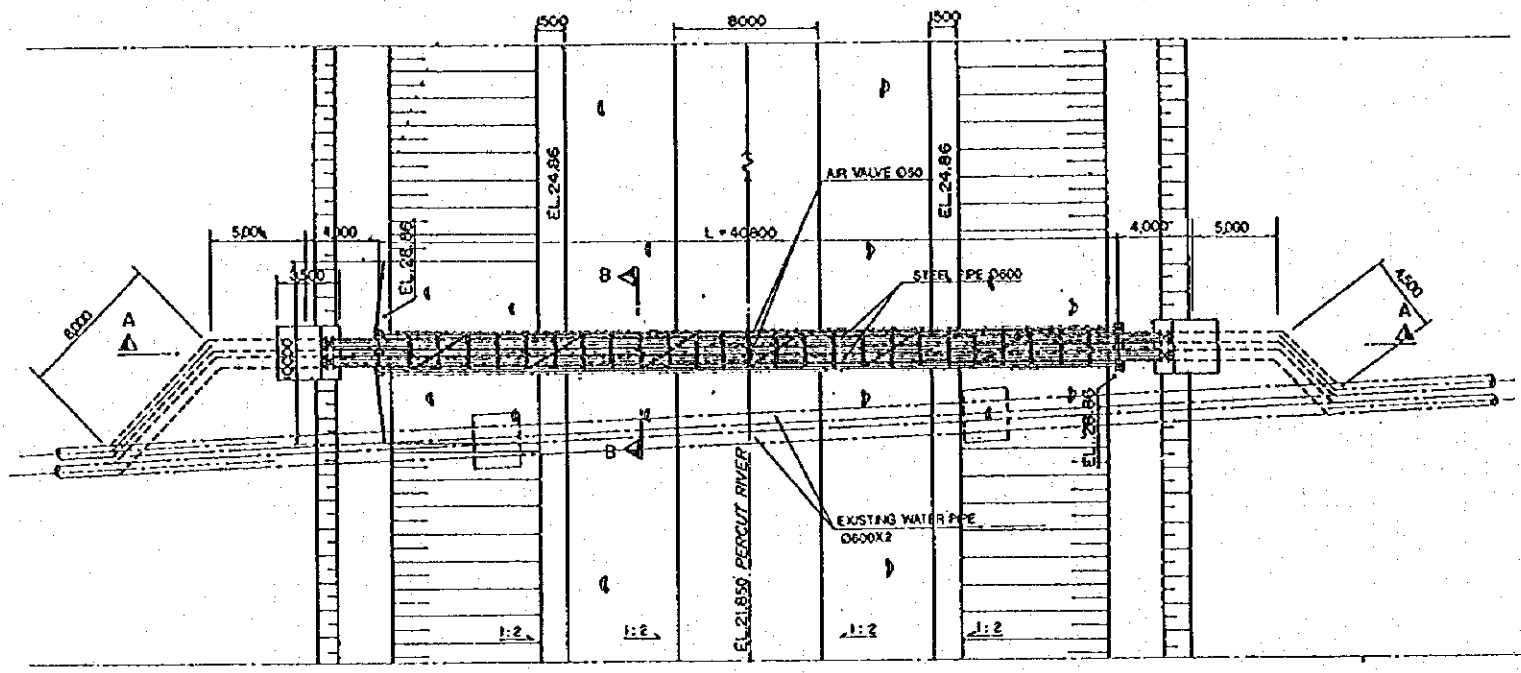
NOTE:  
VALUES ARE PROVIDED AS FOLLOWS:

PIPE	VALVE
Ø 300	Ø 25
Ø 350	Ø 25
Ø 400	Ø 50

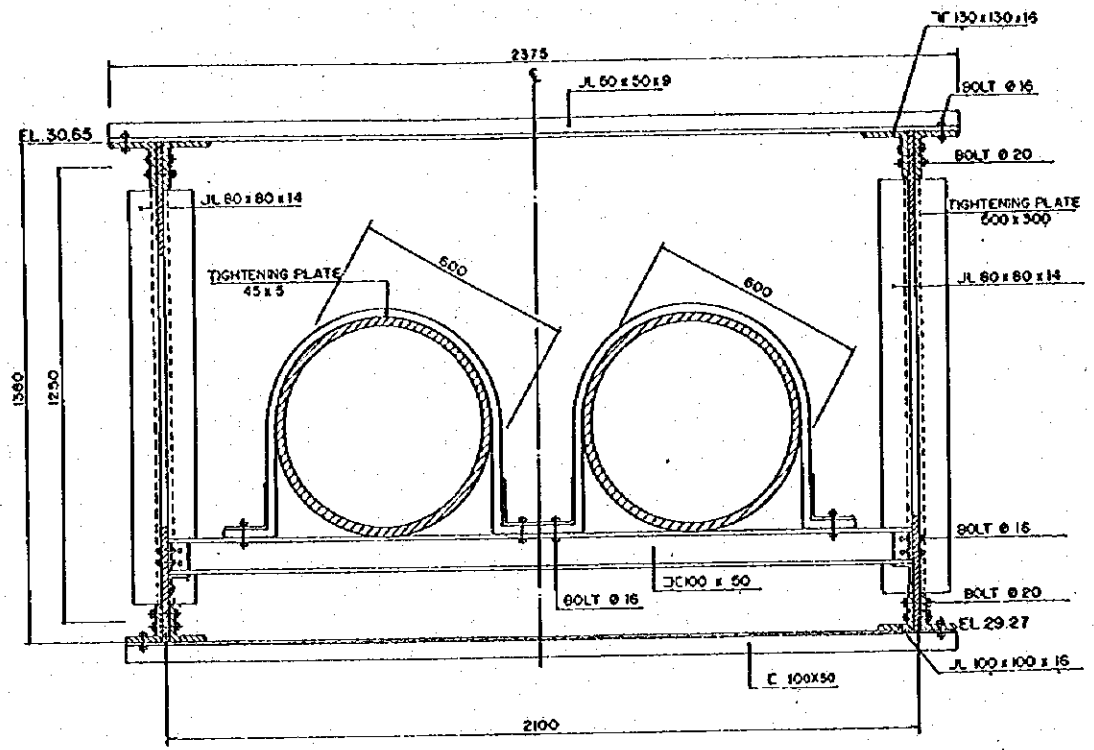


DETAILED DESIGN STUDY ON MEDAN FLOOD CONTROL PROJECT  
JAPAN INTERNATIONAL COOPERATION AGENCY

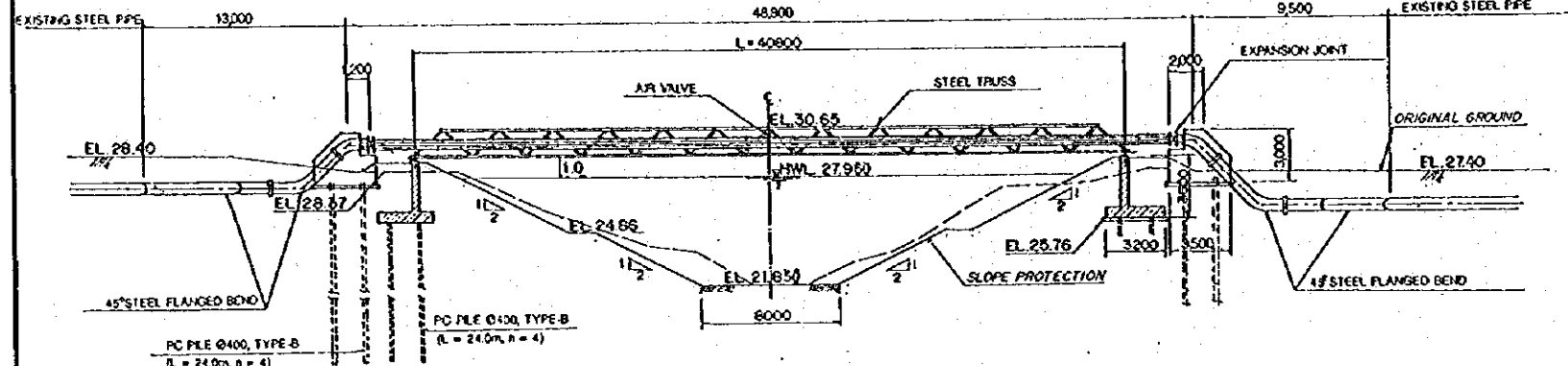
DWG. 6.3.23  
DETAILS OF PIPE HANGER UNDER BRIDGE



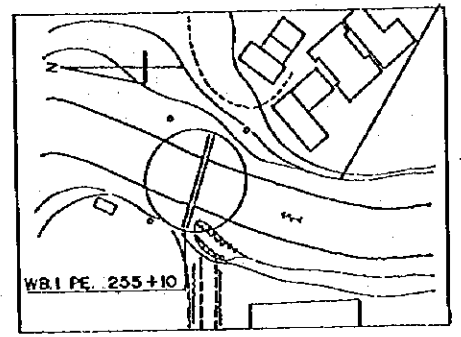
PLAN  
SCALE: A



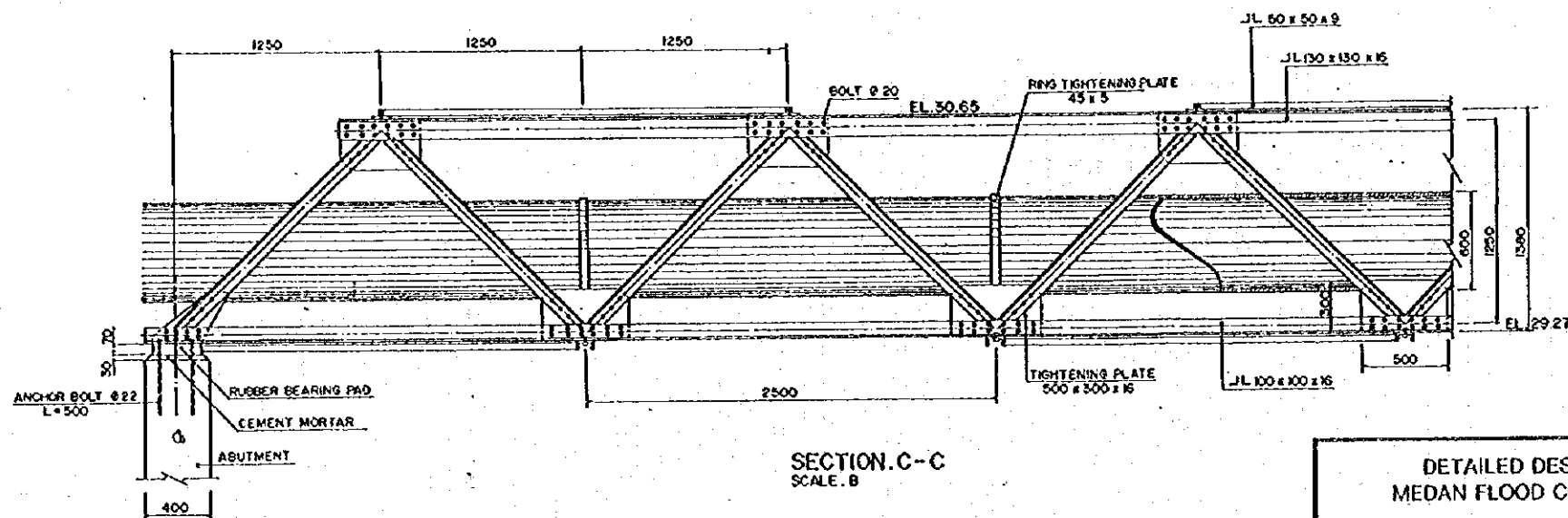
SECTION B-B  
SCALE: C



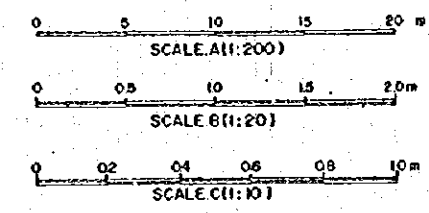
SECTION A-A  
SCALE: A



KEY PLAN  
NOT TO SCALE

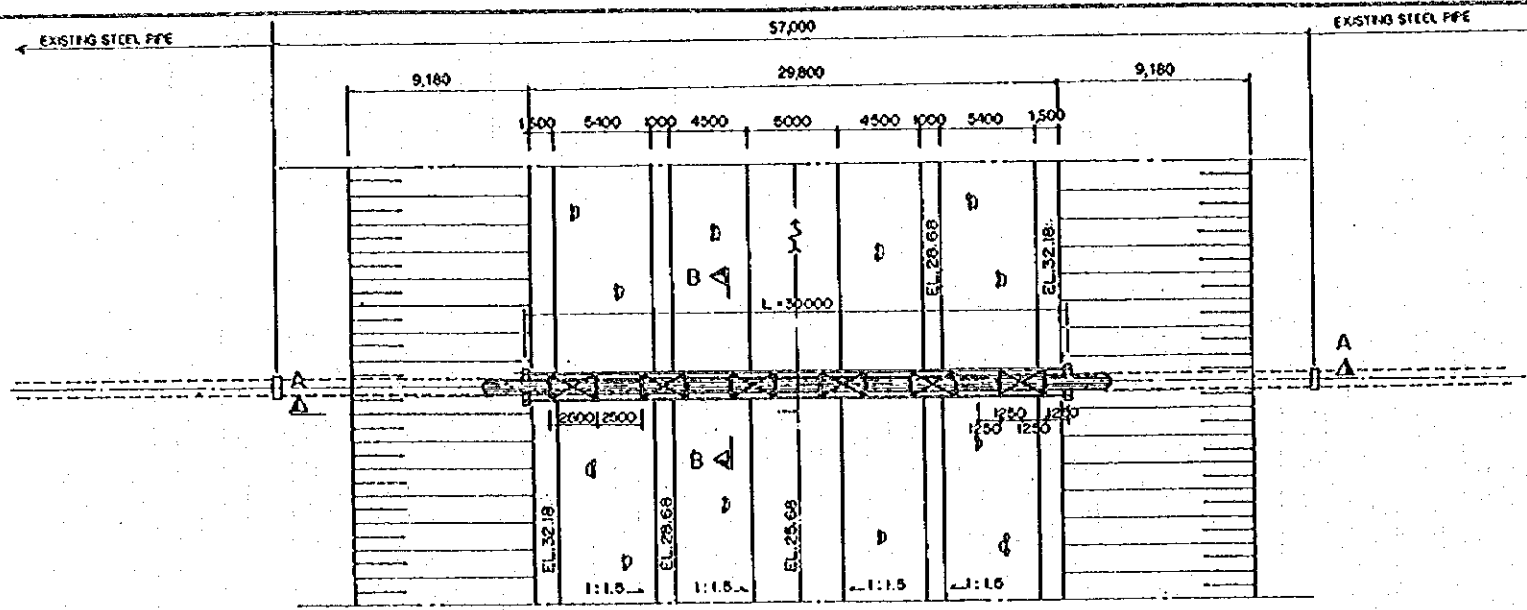


SECTION C-C  
SCALE: B

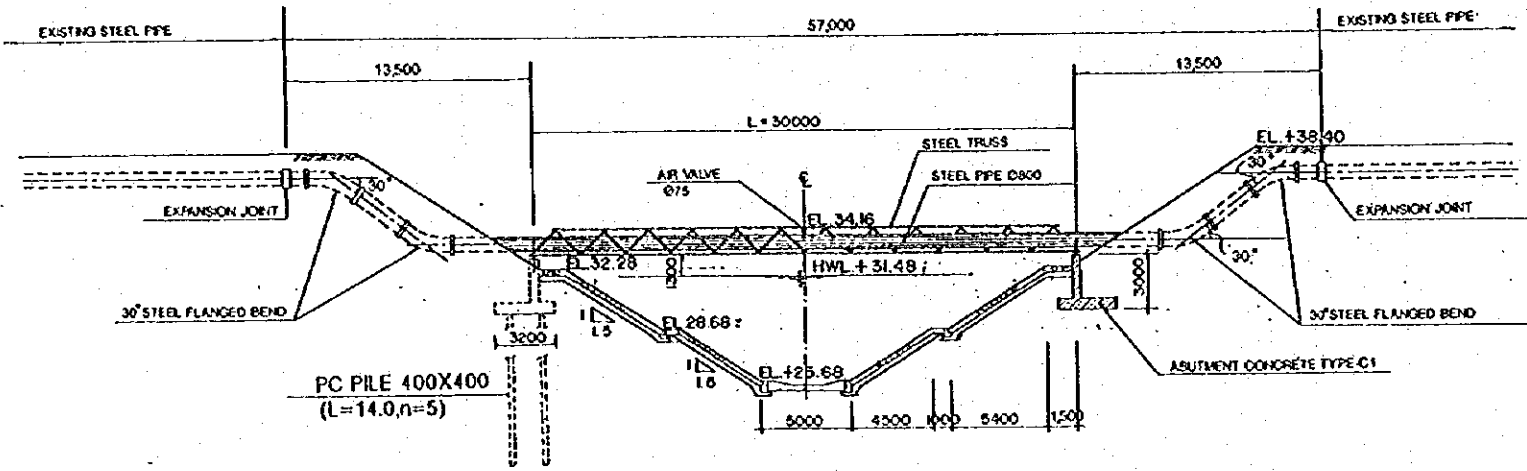


<p>DETAILED DESIGN STUDY ON MEDAN FLOOD CONTROL PROJECT</p> <p>JAPAN INTERNATIONAL COOPERATION AGENCY</p>	<p>DWG. 6.3.24 GENERAL PLAN OF WATER PIPE BRIDGE (WB 1)</p>
---	---

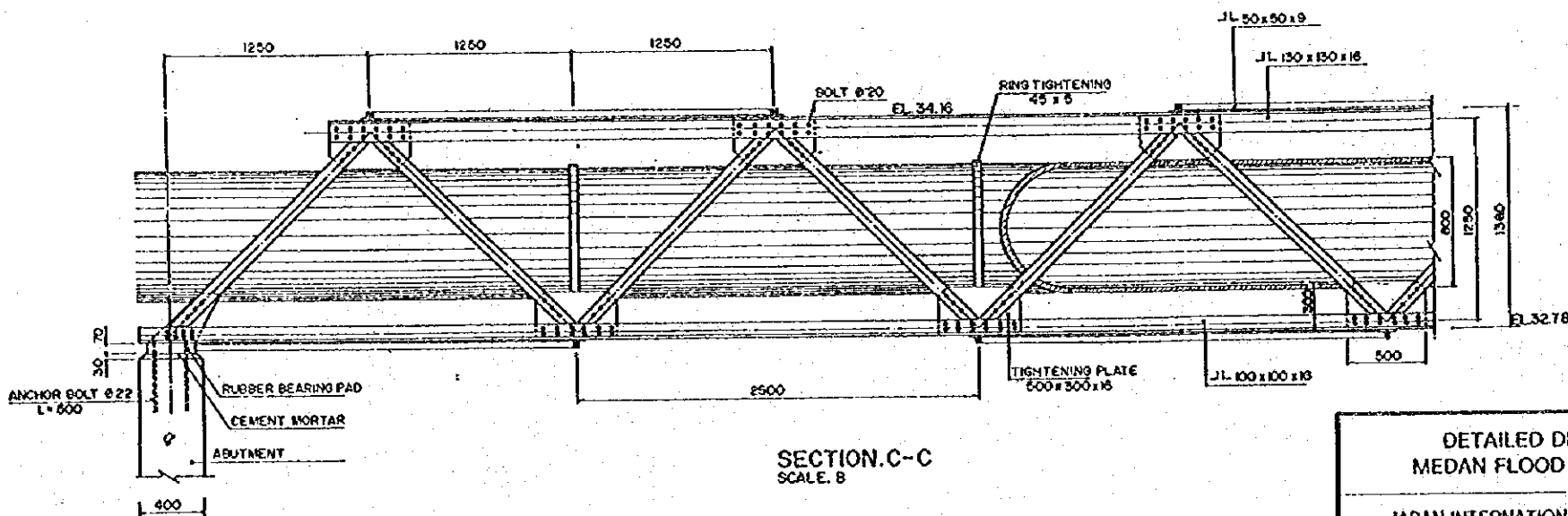
6-69



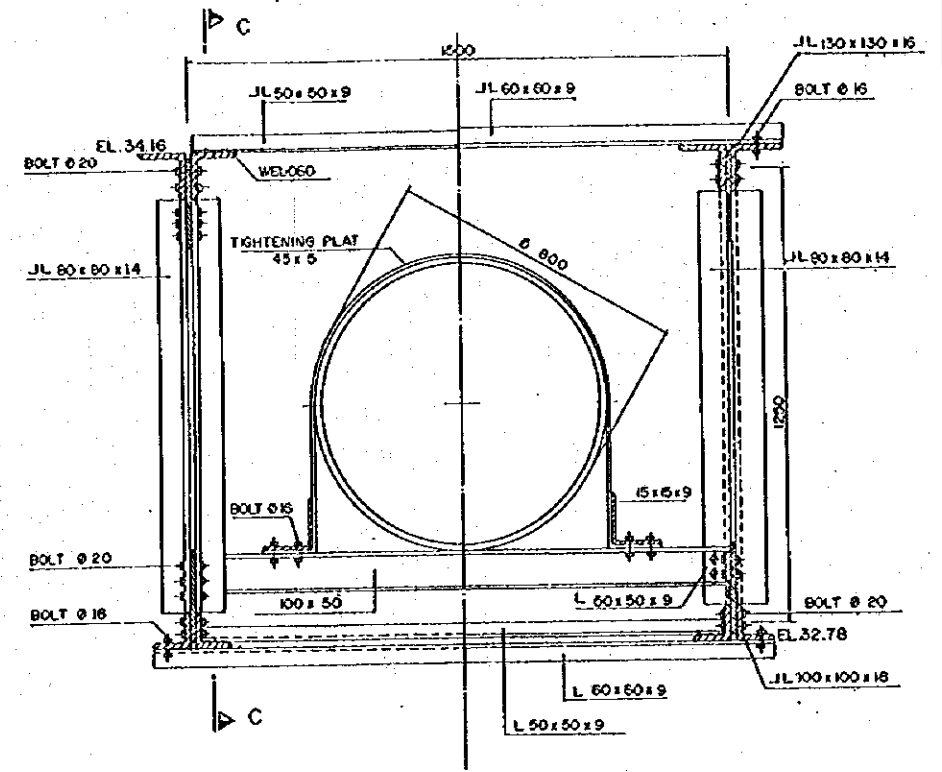
PLAN  
SCALE .A



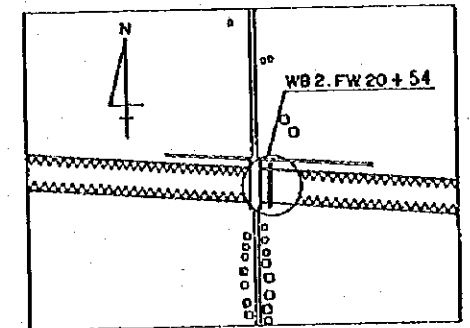
SECTION A-A  
SCALE .A



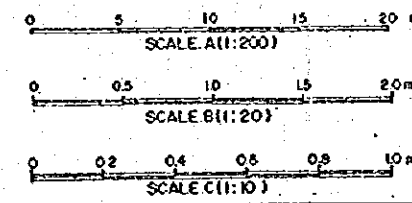
SECTION C-C  
SCALE .B



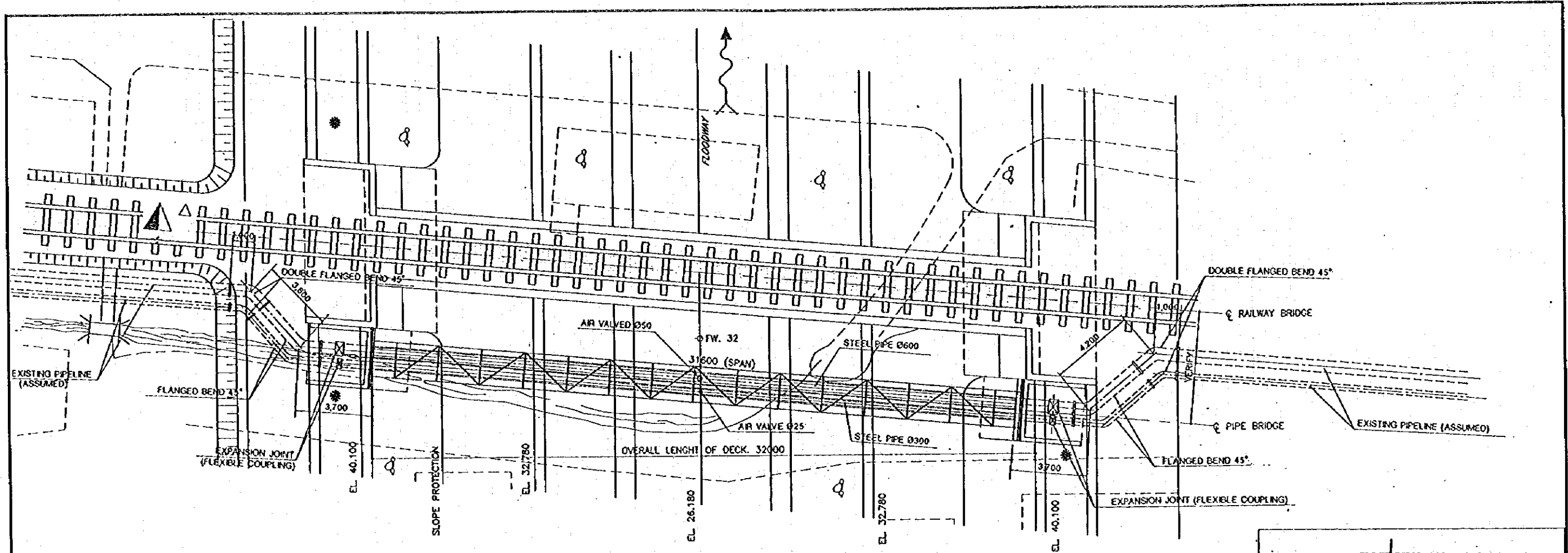
SECTION B-B  
SCALE .C



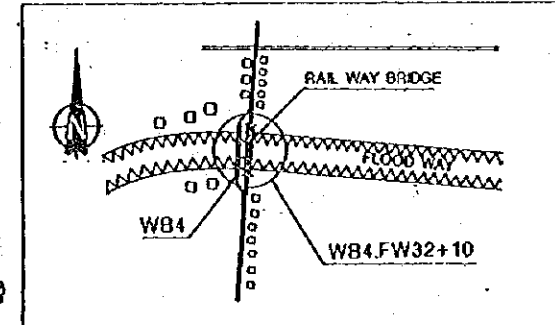
KEY PLAN  
NOT TO SCALE



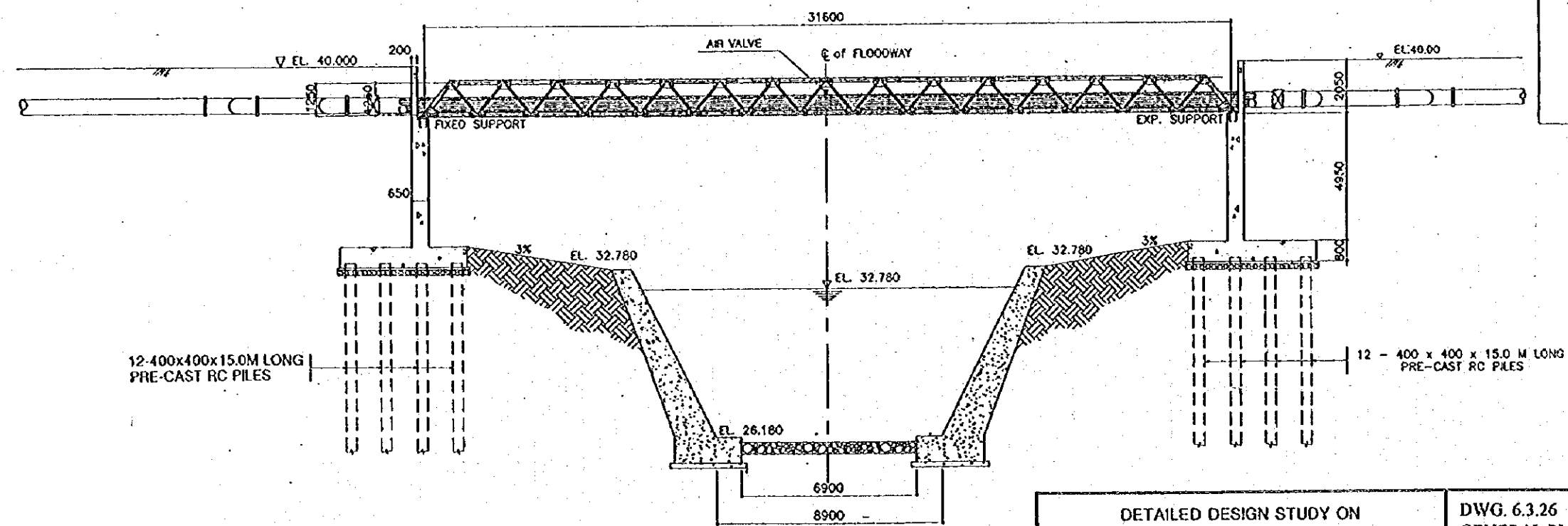
<p>DETAILED DESIGN STUDY ON MEDAN FLOOD CONTROL PROJECT</p> <p>JAPAN INTERNATIONAL COOPERATION AGENCY</p>	<p>DWG. 6.3.25 GENERAL PLAN OF WATER PIPE BRIDGE (WB 2)</p>
---	---



**PLAN OF PIPE BRIDGE**  
 SCALE 1:100



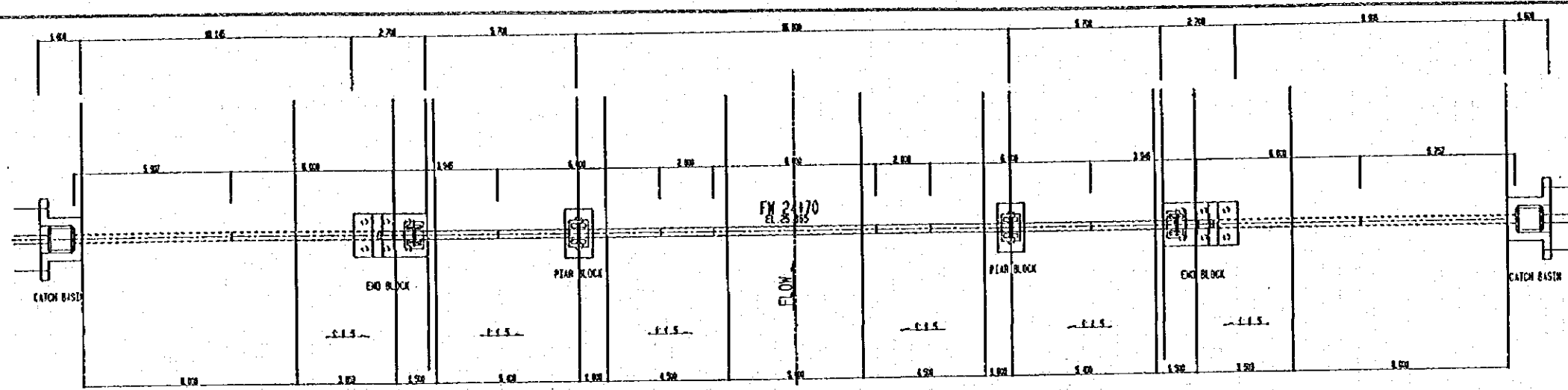
SCALE 0 5 10 15 20 25m  
 SCALE 1:100



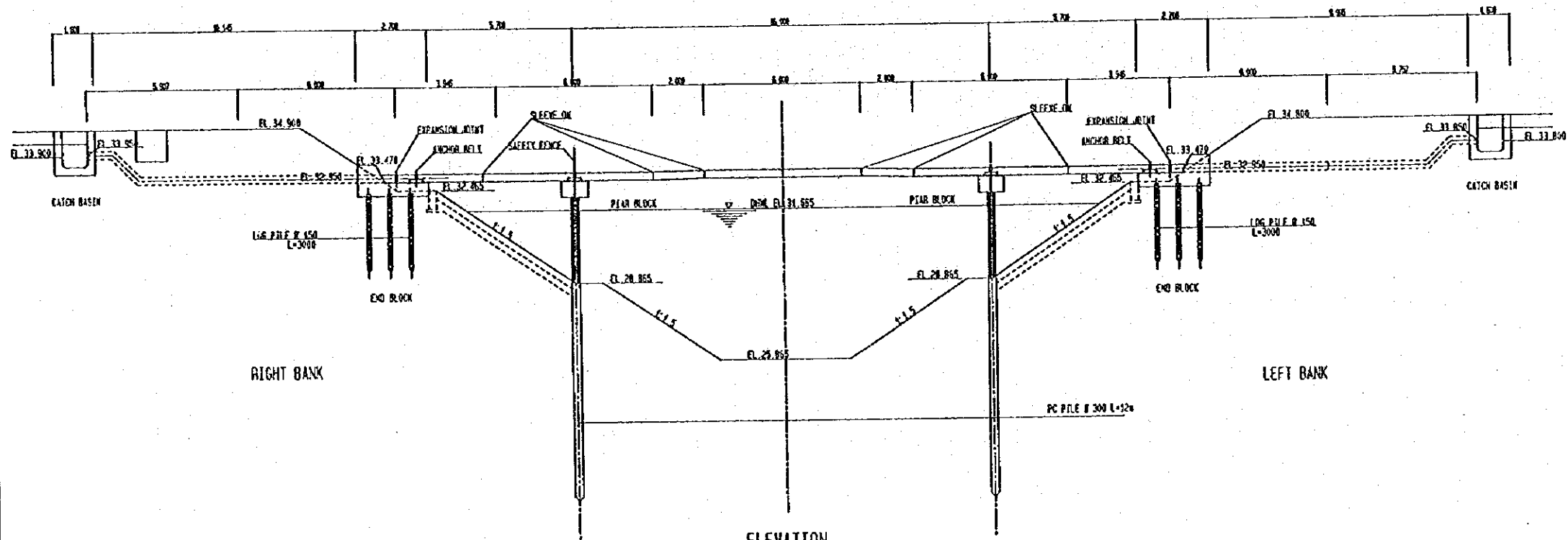
**GENERAL PLAN AND ELEVATION**

**ELEVATION**  
 SCALE 1:100

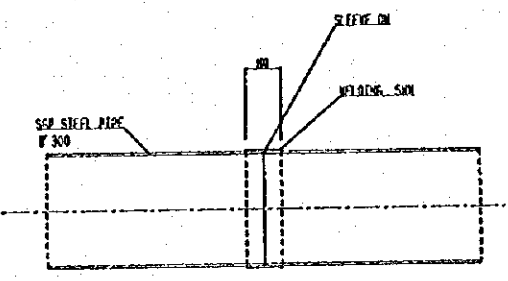
DETAILED DESIGN STUDY ON MEDAN FLOOD CONTROL PROJECT JAPAN INTERNATIONAL COOPERATION AGENCY	DWG. 6.3.26 GENERAL PLAN OF WATER PIPE BRIDGE (WB 4)
---	---



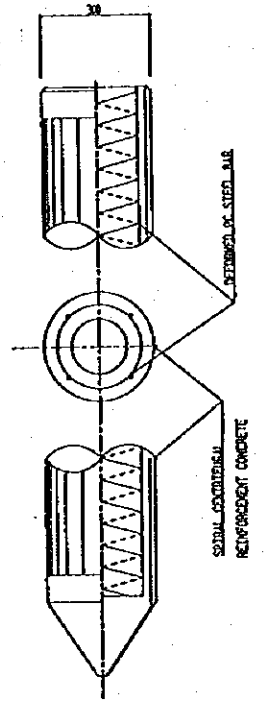
PLAN  
SCALE 1



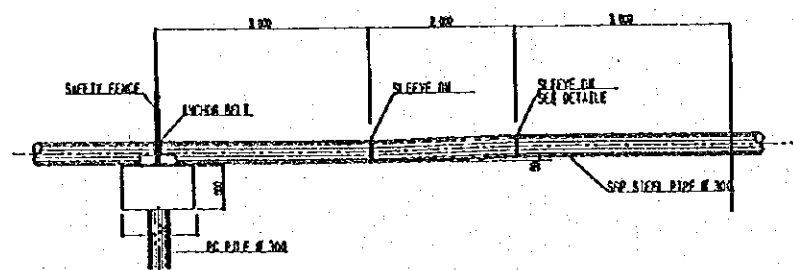
ELEVATION  
SCALE 1



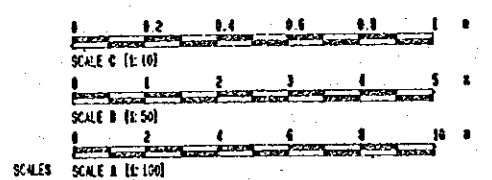
DETAIL OF SLEEVE ON  
SCALE C



DETAIL OF PILE  
SCALE C



DETAIL OF PIPE JOINT  
SCALE B



DETAILED DESIGN STUDY ON  
MEDAN FLOOD CONTROL PROJECT

DWG. 6.3.27  
GENERAL PLAN OF WATER PIPE BRIDGE  
FOR IRRIGATION (WB 3)

JAPAN INTERNATIONAL COOPERATION AGENCY

**CHAPTER 7**

**CONSTRUCTION PLAN AND COST ESTIMATE**



## CHAPTER 7. CONSTRUCTION PLAN AND COST ESTIMATE

### 7.1 Construction Plan

#### 7.1.1 Planning Condition

##### Condition for Construction Works

For construction planning and scheduling, the following basic concepts were taken into consideration:

##### (1) Working Days

The estimation of working days in Indonesia is based on the daily rainfall data at Balai Penelitian Station for the last ten years from 1984 to 1993, as well as the number of Sundays and holidays. Daily rainfall is classified into three ranges; namely, less than 3 mm/day, 3 to 10 mm/day, and more than 10 mm/day. The average number of days in each rainfall is as estimated below:

Less than 3 mm/day	261 days
Less than 10 mm/day	298 days
More than 10 mm/day	67 days

Embankment work can be executed only under the weather condition having rainfall of less than 3 mm/day. Furthermore, embankment work shall not commence at least half a day after a rainfall of more than 10 mm/day to assure dry-up of embankment materials. Dredging may be executed under any weather condition.

Construction works that are affected by river flow such as river channel excavation, ground sill, bridge foundation/substructure and revetment can be executed under rainfall of less than 3 mm/day. On the other hand, other works that are not affected by river flow such as common excavation, bridge superstructure and concreting of structures outside of river channel can be executed under rainfall of less than 10 mm/day.

The non-working national holidays and Sundays are estimated at 65 days per year in total. Therefore, by deducting the non-working holidays and Sundays from the number of days of the respective rainfall conditions, the annual workable days for each work item are estimated as follows:

Civil Works Item	Workable Days
Embankment	187 days
River Channel Excavation, Groundsill and Revetment	215 days
Bridge Substructure/Foundation and Protection Works	215 days
Common Excavation	245 days
Bridge Superstructure and Drainage Outlet	245 days
Dredging	300 days

(2) Construction Materials, Labor and Equipment

Major construction materials are aggregate and stone, cement and asphalt, lumber/timber, steel bar and pile, prestressed concrete pile, girder and oil. These construction materials can be obtained mostly in the domestic market in Indonesia. The promising supply sources of construction materials are shown below.

Material	Source
Aggregate/Stone	Rivers in Medan and Binjai City
Cement/Asphalt	Medan City
Ready-Mixed Concrete	Medan City
Lumber/Timber	Medan City
Steel Bar	Medan City
Steel Sheet Pile	Jakarta or Surabaya City
PC Pile/Girder	Medan City
Combustibles (Oil, Grease, etc.)	Medan City

Common labor is easily recruited in and around the construction sites, and skilled labor is abundant in Medan City. Major equipment with the standard or average capacity required for construction works are mostly earth-moving, piling, concreting and loading equipment. Most of these construction equipment are available in the domestic market, usually used in similar construction projects in and around the project area.

(3) Access to the Site

There is one main national road in the project area, which is a trunk route linking Belawan to Tebing Tinggi crossing Deli River and Percut River at PE206+20. The seaport is located at Belawan, and this is the biggest port in North Sumatra for export-import, as well as domestic trade. A tollway, 25 km long, connects the project area to the Belawan Port.

(4) Power Supply

In the project area, PLN supplies electric power which is available for the construction works of the Project. Diesel engine generator can be provided at the construction site, where connection to the PLN power line may be costly.

(5) Construction Method and Type of Construction Equipment

The construction of river improvement works employ conventional and effective styles. Construction equipment utilizes the standard and common equipment at the site.

(6) Earth Volume Change Factor

Volume change factors are used for the calculation of production rate of equipment for earthwork. The conversion factor for loose and compacted conditions are assumed as 1.10 and 0.91, based on the geological survey results.

**Main Construction Works**

(1) Construction Schedule

The Project consists mainly of the Percut River Improvement Works (28.23 km) and the construction of the Floodway (3.92 km) including the improvement of Upper Deli River (0.95 km). For implementation, the construction works are divided into seven (7) portions, as tabulated below and delineated in Fig. 7.1.1. The apportioning of construction works is made in consideration of the suitable and practical volume of civil works contracts as practiced and experienced in similar projects in Indonesia.

Work Portion	Station No.	Object Distance
MFC-1	PE0-200 ~ PE46	5,040 m
MFC-2	PE46 ~ PE129	8,270 m
MFC-3	PE129 ~ PE210	8,100 m
MFC-4	PE210 ~ PE274	6,500 m
MFC-5	FW0 ~ FW24 & PE274 ~ PE274+320	2,680 m
MFC-6	FW24 ~ FW34	1,010 m
MFC-7	FW34 ~ FW39+50 & UD12-85.0 ~ UD23	1,500 m

(2) Arrangement of Spoil Bank

As shown in Fig. 7.1.2, some spoil banks for excavation material are mainly situated in swamp and lowland areas. The capacities of the spoil areas are as shown below.

Spoil Bank	Area (in ha)	Embankment Height (m)	Spoil Volume (m <sup>3</sup> )
1	300m x 400m = 12.0 ha	1.5	180,000
2	150m x 1,800m = 27.0 ha	2.0	540,000
3	200m x 600m = 12.0 ha	2.0	240,000
4	150m x 500m = 7.5 ha	3.0	225,000
5	300m x 500m = 15.0 ha	3.0	450,000
6	200m x 1,000m = 20.0 ha	2.0-4.0	800,000
Total	93.5 ha	.	2,435,000

The hauling distance from each portion of construction work to the spoil area is calculated by the spoil volume to each area and the distance, and the average distance is as shown below. The spoil volume is considered as the surplus of excavated and embankment soil.

Portion	Spoil Area	Average Distance	Spoil Area	Average Distance	Spoil Area	Average Distance	Estimated Distance
MFC-1	1	2 km	2	2 km			2 km
MFC-2	1	5 km	2	5 km			5 km
MFC-3	2	11 km	3	2 km	4	2 km	6.5 km
MFC-4	4	6.5 km	4	4 km			5 km
MFC-5	5	1 km	6	6 km			5 km
MFC-6	5	5 km					5 km
MFC-7	6	4 km					4 km

**Construction Method of Major Work Items**

The construction method of major work items are as described below.

(1) Excavation

A common excavation is defined as the execution of excavation works on ground higher than the elevation of 4 m below the design high water level, while a river channel excavation is the excavation on ground lower than the said elevation. The common excavation is assumed to use 0.6 m<sup>3</sup> backhoe, 12 ton bulldozer, 1.2 m<sup>3</sup> wheel loader and 8-ton dump truck.

The common excavation in the Floodway can be performed with a larger capacity of equipment than those in Percut River. River channel excavation is mainly carried out by using backhoe and clamshell for dumping the excavated materials in the inland side, and a dump truck for hauling them to the spoil area.

Dredging work is employed only for the stretch from the river mouth (PE0-200) to PE14 of Percut River. Spoil areas for dredging materials are provided with a sedimentation pond with dike coffering to prevent the outflow of sand and silt into the river. Besides, excavated material from PE0-200 to PE1.0 is highly silty soil which is planned to be spoiled to the right side at around PE0.0 with coffering dike. (refer to Fig. 7.1.3)

(2) Embankment

Embankment of dike is carried out mainly with suitable material from the excavated soil. The embankment works are performed by 12 ton bulldozer, 0.4 m<sup>3</sup> backhoe and compactors. The thickness of each layer of embankment is preferably less than 30 cm.

The materials for embankment will depend on the results of survey and soil test. The ratio of embankment suitable volume to the total volume of excavated materials is estimated for each portion as below.

Portion	Common	River Channel	Dredging
MFC-1	40%	10%	0%
MFC-2 to MFC-4, MFC-7	90%	20%	-
MFC-5 to MFC-6	90%	-	-

(3) Concrete Works

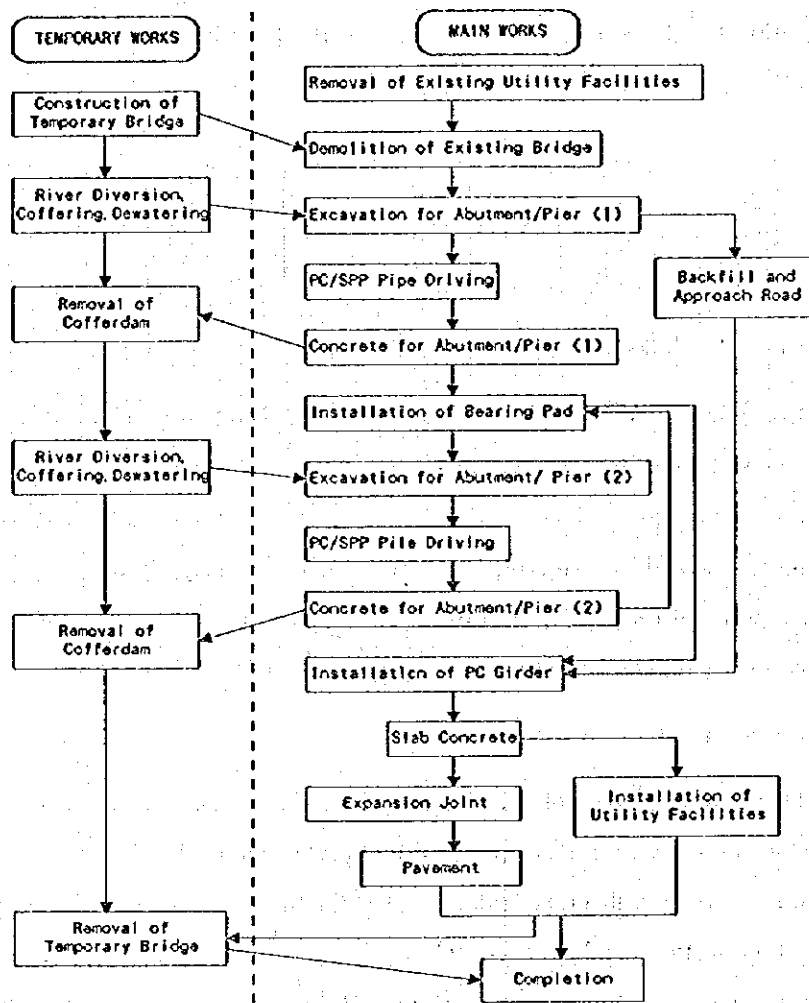
Concrete works are applied to the reinforced concrete for weirs and piles, bridge slab and structures where ready-mixed concrete is to be procured from suppliers. A portable concrete mixer is used for small concrete structures such as partition wall of revetment and drainage ditch. Formwork for concrete is of two types, form with support and form only, each of which is made up of 2.5 m in height. Formwork is necessary for concrete structures having a slope of 1 : 1.5 and above.

(4) Revetment

There are two types of revetment, wet stone masonry type and concrete type. Wet stone masonry type is composed of foot protection, cobblestone filling, base concrete, wet masonry with gravel filling, and palm filter. Foot protection is provided with gabion mattress and cylinder. The coffering of the work site is made of log piles and sandbags. Concrete type is composed of rubble stone bedding, base concrete, backfilling with soil and gravel, main concrete and weep hole. Weep hole is made of PVC pipe and filter cloth.

(5) Bridge Works

Bridge works are composed of excavation, piling, reinforced bar, concrete, girder stressing and loading, pipe and pavement works. Designed bridges are of five types, pedestrian, water pipe, railway, road and maintenance, depending on purpose. Precast prestressed concrete girder for the bridges is commonly designed as a main and cross beam. Loading work for bridge girder is executed by portal crane which is a common method in Indonesia (refer to Fig. 7.1.4). A typical work sequence of bridge construction is as illustrated below.



(6) Intake Weir (Inflatable Rubber-Made Dam)

The major works of new intake weir construction are composed of inflatable rubber-made dam, irrigation channel, revetment, box culvert and intake gate. After diverting river flow from the existing low water channel to the newly excavated low water channel connected to the new intake weir, the existing weir will be demolished. Work schedules for the main body are the installation of anchor bolt, embedded plate and piping, and the construction of control house and installation of operating equipment. Testing will be the last work after placing the inflatable rubber main body on foundation and clamps.

(7) Preparatory Works

The preparatory works consist of mobilization and demobilization of construction equipment as well as labor force, preparation and demolition of temporary building and

yards, clearing and grubbing of spoil area, installation and removal of water, fuel, electric power supply system, survey works for the preparation of working drawings and so on. These works are to be carried out in the beginning and the end of the construction period. Preparatory works include improvement and maintenance of existing road, construction, maintenance and subsequent demolition of the temporary access road.

Immediately after the receipt of Notice to Proceed, the Contractor will commence with the mobilization of work force, construction equipment and preparation of temporary facilities such as site office, workshop, laboratory, etc., including water, telecommunication and power supply system. Other requirements for the construction works such as survey, soil investigation, submittal of construction program for approval, working drawings and other necessary documentation have to be carried out smoothly based on the stipulation of the contract. The preparatory works mentioned above are common for each portion of the project.

(8) Temporary Works

Temporary works include temporary bridges, temporary channels, and coffering and dewatering. The reconstruction of bridges require temporary bridge to minimize inconvenience to the usual activities of people around the construction site during the construction period. A temporary channel for river diversion work is also necessary for the construction of the Deli Diversion Weir. Dewatering and coffering works are required for the construction works of ground sill, bridge protection, dredging, jetty parapet, riverbed protection and excavation.

(a) Coffering and Dewatering

Cofferdam will be provided in riverside construction works using sandbags with excavated materials and one or two lines of log piles. The cofferdam is designed with a crest width of 1.0 m and a height of 2.0 m. The materials for a cofferdam can be reused for the next cofferdam. In the dry season, a small sized cofferdam may be constructed considering water level and rainfall. However, at the end of the dry season, October and November, the size of cofferdam should be reinforced to the same dimension mentioned above in preparation for the coming rainy season.

Throughout the construction period, dewatering work is indispensable to construct all structures in dry condition, especially during the construction of revetment,

bridge protection, ground sill, weir and excavation of floodway. Excavation works at the floodway may percolate groundwater to the construction site. The contractor has to employ enough number of submersible pumps and engine generators to cope with dewatering.

(b) Temporary Bridge

The temporary bridge for detouring is constructed of coconut piles with steel beam and wooden board (refer to Fig. 7.1.5). Equipment for loading and materials for temporary bridge are assumed to be reused from one bridge construction to another. The width of the temporary bridge is 3.0 m to 7.0 m based on traffic condition. The dimensions of temporary bridge are as shown below.

Name of Bridge	Width (m)	Length (m)	Area (m <sup>2</sup> )
Br. P-1, Titi Besi Bridge	6.0	80.0	480.0
Br. P-7, Medan Tembung Bridge	7.0	30.0	210.0
Br. P-9, Medan Denai Bridge	4.0	60.0	240.0
Br. P-11, Binjai Bridge	7.0	30.0	210.0
Br. P-13, Amplas Bridge	6.0	50.0	300.0
Br. F-8, Gg. Sekasama Bridge	3.0	50.0	150.0
MFC-7, Hauling Bridge	3.0	50.0	150.0

(c) Temporary Channel

A temporary channel is proposed to divert the river discharge during the construction of Deli Diversion Weir. The channel can be either one of two alternative types: (1) temporary diversion channel by open-cut method with protection work, and (2) river channel with coffering of steel structure. The comparison of these two types was made as to construction period, workability of site, execution and safety, as tabulated below.

Alternative 1 (Diversion Channel)		Alternative 2 (Coffering)	
Dimension		Dimension	
- Riverbed Width	15.0 m	- Coffering Length	80.0 m
- Channel Length	225.0 m	- Piling Length (two sides)	10 m
Work Volume		Work Volume	
- Excavation	23,241 m <sup>3</sup>	- Steel Structure	40,000 kg
- Gabion Mattress	1,353 m <sup>3</sup>	- Dewatering	300 set/day
- Coffering	330 m	- Coffering	380 m
- Embankment	25,566 m <sup>3</sup>	- Excavation	3,960 m <sup>3</sup>
		- Embankment	4,356 m <sup>3</sup>
Construction Period	11 months	Construction Period	13 months
Construction Cost	Rp 475,000,000	Construction Cost	Rp 604,000,000



Based on the above comparison, Alternative 1 is more advantageous than Alternative 2. Especially, Alternative 2 is required to shift coffering to the opposite side in the rainy season since the construction period of Deli River Weir is estimated to be one year. The coffering work is difficult and dangerous in the rainy season because the left side bank of the construction site has a cliff profile and a small working area.

**Work Capacity**

The quantity of work for major work items is calculated in consideration of construction method suitable to the work. Daily work volume is estimated on the basis of "set" estimated on workability of major equipment required for each item of work. The daily work volume per set of main work item is estimated as follows:

Work Item	Unit	Daily Work Volume/Set
Excavation (Common)	m <sup>3</sup>	1,120
Excavation (Riverbed)	m <sup>3</sup>	480
Excavation (Floodway)	m <sup>3</sup>	1,440
Embankment	m <sup>3</sup>	800
Revetment (Wet Stone Masonry)	m <sup>3</sup>	17.5
Revetment (Concrete)	m <sup>3</sup>	15
Bridge	m <sup>2</sup>	5
Sodding	m <sup>2</sup>	400
Clearing/Grubbing	m <sup>2</sup>	960
Stripping	m <sup>3</sup>	1,150
Dredging	m <sup>3</sup>	400

A set of equipment for each work item is composed as follows:

Work Item	Equipment	Number of Units
Excavation (Common)	Bulldozer, 12 ton	1
	Backhoe, 0.6 m <sup>3</sup>	1
	Wheel-Loader, 1.2 m <sup>3</sup>	1
	Dump Truck, 8-ton	14
Excavation (River Channel)	Backhoe, 0.6 m <sup>3</sup>	2
	Dump Truck, 8-ton	6
Excavation (Floodway)	Bulldozer, 15-ton	1
	Backhoe, 0.6 m <sup>3</sup>	1
	Wheel-Loader, 1.2 m <sup>3</sup>	1
	Dump Truck, 8-ton	18
Dredging	Dredger, 200 ps	1
Embankment	Bulldozer, 12-ton	1
	Backhoe, 0.4 m <sup>3</sup>	1
	Tire Roller	1
Clearing and Grubbing	Bulldozer, 12-ton	1
	Wheel-Loader, 1.2 m <sup>3</sup>	1
	Dump Truck, 8-ton	12
Stripping	Bulldozer, 15-ton	2
	Wheel-Loader, 1.2 m <sup>3</sup>	1
	Dump Truck, 8-ton	6

### 7.1.2 Construction Plan of Work Portions

The construction works of portion MFC-1 to MFC-7 are scheduled to commence within three years. River improvement is generally executed from the downstream to upstream, but some embankment works upstream may be conducted before excavation works of the downstream on condition that the work shall not bring an imbalance of the flow capacities between the up and downstream stretches.

#### MFC-1

MFC-1 is the portion nearest to the river mouth and the improvement stretch is 5,040 m long from PE0-200 to PE46.

#### (1) Work Quantity

Based on the design of river improvement, the work volumes for MFC-1 are as calculated below.

Work item	Unit	Quantity
<b>River Works</b>		
- Clearing and Grubbing	m <sup>2</sup>	290,600
- Stripping	m <sup>3</sup>	36,500
- Dredging	m <sup>3</sup>	82,600
- Excavation (Common)	m <sup>3</sup>	139,600
- Excavation (River Channel)	m <sup>3</sup>	164,600
- Embankment	m <sup>3</sup>	263,800
- Parapet Wall	m <sup>3</sup>	1,560
- Revetment (Wet Stone Masonry)	m <sup>3</sup>	2,560
- Sodding	m <sup>2</sup>	86,100
<b>Other Works</b>		
- Jetty (Landing Stage)	L.S.	1
- Channel Excavation for Sungai Latang (PE0-60 ~ PE8+50)	m	910
- Relocation of Kabupaten Road (PE14+80 ~ PE33+100)	m	1,920
- Intake Sluice Gate for Fishpond	nos.	2

#### (2) Site Condition

In the construction stage, the following matters shall be considered:

- (a) Dredged materials are hauled to the spoil area where dike and sedimentation pond for sand and silt are provided, and two locations of spoil area for dredged materials are proposed, the right side area at PE0.0 and Spoil Area No. 2 (refer to Fig. 7.1.2). Since the right side area has a length of 200 m and a width of about 150 m, the dredged volume from PE0.0-200 to PE1.0 is estimated to have a depth of 0.5 m. It is not necessary to build a sedimentation pond because the materials

dredged from the estuary are of the same quality as the materials in the spoil area. On the other hand, the dredged materials from PE1.0 to PE14.0 are less silty and easier to settle. The excavated soils shall be dried up in the sedimentation pond for a few days and then mixed with other soils to obtain a stiffer material.

- (b) A temporary jetty is required during the construction of a new jetty. It is necessary for the coffering and dewatering of piling works, wet stone masonry and concrete works, because the left side of Percut River downstream is always inundated by tide. The construction period is estimated to be 4.5 months during the dry season. The height of coffering shall conform with the tidal and water levels. The concrete sheet pile work shall be finished before the onset of the rainy season.
- (c) The excavated materials in work portion MFC-1 does not meet the required volume of embankment material. The materials for embankment shall be supplied directly from the other work portions (MFC-2 or MFC-3) or from the stockyard. (refer to Fig. 7.1.6). The right-of-way will be utilized as transportation road.
- (d) Since Spoil Area No. 1 and No. 2 are located near the estuary and earth works are to be started from the beginning of each portion especially MFC-1 and MFC-2, the transportation road to the spoil area shall be executed first from the beginning of the construction period in both sides of the river.
- (e) Channel excavation for Sungai Lalang is also planned to start early in order to complete diking work on the right side bank from PE9.0 to PE0.0 before the rainy season. Besides, the right side dike will be used as the transportation road for rubble mound construction.

(3) Number of Working Sets and Period

The number of sets of major civil works are as estimated below, and the construction schedule is shown in Fig. 7.1.7:

Work Item	No. of Sets	Period (month)
Clearing and Grubbing	3	5.0
Stripping	2	1.0
Excavation (Common)	2	3.5
Excavation (River Channel)	3	6.0
Dredging	1	9.5
Embankment	1	11.0
Parápet	1	2.0
Revetment (Wet Stone Masonry)	1	7.5
Sodding	2	4.5

**MFC-2**

MFC-2 is located upstream of MFC-1 with a stretch of 8,270 m from PE46 to PE129.

**(1) Work Quantity**

The work volumes for MFC-2 are as calculated below.

Work Item	Unit	Quantity
<b>River Works</b>		
- Clearing and Grubbing	m <sup>2</sup>	528,000
- Stripping	m <sup>3</sup>	61,000
- Excavation (Common)	m <sup>3</sup>	340,900
- Excavation (River Channel)	m <sup>3</sup>	166,200
- Embankment	m <sup>3</sup>	338,000
- Bandar Sidoras Intake Weir	L.S.	1
- Groundsill	L.S.	1
- Revetment (Wet Stone Masonry)	m <sup>3</sup>	2,680
- Sodding	m <sup>2</sup>	175,400
<b>Bridge Works</b>		
- Titi Besi Bridge, W=7.0 m, L=82.8 m	L.S.	1
- Perkebunan Bridge, W=7.0 m, L=104.0 m	L.S.	1
- Titi Gantung Bridge, W=7.0 m, L=57.4 m	L.S.	1
<b>Drainage Works</b>		
- Drainage Gate, Flap Dia 600 mm	L.S.	1
- Drainage Gate, 2.0 m x 1.5 m x 2	L.S.	1
<b>Other Works</b>		
- Relocation of Existing Road (PE82 ~ PE97)	m	1,500
- Relocation of Irrigation Channel for SL2 (PE95+35 ~ PE103+10)	m	710

**(2) Site Condition**

- (a) The irrigation intake gate at Bandar Sidoras Intake Weir is fully opened twice a year, April to May and October to November. River diversion work is accordingly executed at those times (refer to Fig. 7.1.8). There is a large hole dugged in front of the existing gate and in order to maintain the smooth flow, the demolished concrete block of the existing weir will be used to backfill this place.
- (b) The construction of Titi Besi Bridge (P1) requires a temporary bridge as well as coffering and dewatering for foundation works. The electric pole on the bridge is also required to be relocated. The temporary bridge is 6.0 m wide and 80.0 m long, and it is wider than the existing bridge for construction use. Coffering is necessary during the foundation work on piers but not so much. Perkebunan Bridge (P2) and Titi Gantung Bridge (P3) are planned to be constructed upstream

of the existing bridges. The existing bridges will be used during the construction period.

- (c) Groundsill work shall be executed with exact construction timing together with the temporary works. Coffering and dewatering is carried out by diking with log piles and sandbags. Construction may be started from one side, diverting river flow to the other side and vice versa.. Therefore, construction of the groundsill shall be executed in the dry season.
- (d) A specified skill and technique is required to install and adjust the control equipment of the inflatable rubber-made dam. Electric power is required for the adjustment and test after installation.

(3) Number of Working Sets and Period

The number of sets for major civil works are as estimated below, and the construction schedule is shown in Fig. 7.1.9.

Work Item	No. of Sets	Period (month)
Clearing and Grubbing	3	9.0
Stripping	3	1.0
Excavation (Common)	2	7.5
Excavation (River Channel)	2	8.5
Embankment	3	9.0
Bandar Sidoras Intake Weir	1	10.0
Revetment (Wet Stone Masonry)	1	7.5
Sodding	4	5.5

**MFC-3**

MFC-3 is located upstream of MFC-2, with a length of 8,100 m covering the stretch from PE129 to PE210.

(1) Work Quantity

Based on the design of the river improvement, the work volumes for MFC-3 are as calculated below.

Work Item	Unit	Quantity
River Works		
- Clearing and Grubbing	m <sup>2</sup>	332,400
- Stripping	m <sup>3</sup>	16,300
- Excavation (Common)	m <sup>3</sup>	626,300
- Excavation (River Channel)	m <sup>3</sup>	161,900
- Embankment	m <sup>3</sup>	95,200
- Revetment (Wet Stone Masonry)	m <sup>3</sup>	10,200
- Sodding	m <sup>2</sup>	88,500

<b>Protection Works</b>		
- Titi Runtuh Bridge	L.S.	1
- Railway Bridge	L.S.	1
<b>Bridge Works</b>		
- Payung Bridge, W=7.0 m, L=40.8 m	L.S.	1
- Pedestrian Bridge, W=2.0 m, L=40.8 m	L.S.	1
- Medan Tembung Bridge, W=9.0 m, L=40.8 m	L.S.	1
- Medan Denai Bridge, W=16.0 m, L=40.8 m	L.S.	1
<b>Drainage Works</b>		
- Drainage Gate, 1.5 m x 1.5 m	site	2
- Drainage Gate, 2.0 m x 1.5 m	site	1
- Drainage Box Culvert, 2.0 m x 1.5 m	site	1
- Drainage Pipe, Dia 1,000 mm x 2	site	1
- Drainage Pipe, Dia 1,000 mm	site	1
- Drainage Pipe, Dia 800 mm x 2	site	2
- Drainage Pipe, Dia 800 mm	site	1
- Drainage Pipe, Dia 600 mm	site	3
- Drainage Ditch, W=1,000 mm	site	1
- Drainage Ditch, W=600 mm	site	1

(2) Site Condition

- (a) Protection works for bridge pier require coffering and dewatering during the construction period. Riverbed protection is made by concrete lining and placing of concrete block and gabion mattress, or a combination of the above. Construction period shall be within the dry season.
- (b) Excavated materials are classified as embankment materials and others. Embankment materials are temporarily stored in a spoil area near the construction site, and the materials for embankment of MFC-1 portions are hauled to Spoil Area No. 1 and No. 2, because the volume of excavation exceeds the embankment volume.
- (c) Relocation of water level gauging station shall consist only of the construction of gauging well and observation house. The existing gauging equipment is transferred to the new station. The new well of the water level station will be made of steel pipe with a diameter of 600 mm and with two inlet pipes of 400 mm. Leveling and protection works shall be executed.
- (d) The construction of Medan Tembung Bridge (P7) and Medan Denai Bridge (P9) requires temporary bridges during the construction period. Medan Denai Bridge (P9) is planned to be constructed prior to Medan Tembung Bridge (P7) which is started after completion of Medan Denai Bridge (P9), so that materials for the temporary bridge could be reused. The locations of bridge abutments in MFC-3 are on the land-side so that coffering work is expected to be not so much.

## (3) Number of Working Sets and Period

The number of sets of major civil works are as estimated below, and the construction plan is shown in Fig. 7.1.10.

Work Item	No. of Sets	Period (month)
Clearing and Grubbing	3	6.0
Stripping	1	1.0
Excavation (Common)	3	9.5
Excavation (River Channel)	2	8.5
Embankment	1	8.0
Revetment (Wet Stone Masonry)	4	7.5
Sodding	2	5.5

**MFC-4**

MFC-4 is located upstream of MFC-3, with a length of 6,500 m from PE210 to PE274.

## (1) Work Quantity

Based on the design of river improvement, the work volumes for MFC-4 are as calculated below.

Work Item	Unit	Quantity
<b>River Works</b>		
- Clearing and Grubbing	m <sup>3</sup>	228,700
- Stripping	m <sup>3</sup>	8,750
- Excavation (Common)	m <sup>3</sup>	342,900
- Excavation (River Channel)	m <sup>3</sup>	99,500
- Embankment	m <sup>3</sup>	59,200
- Revetment (Wet Stone Masonry)	m <sup>3</sup>	5,880
- Sodding	m <sup>2</sup>	65,500
<b>Protection Works</b>		
- National Road Bridge	L.S.	1
<b>Bridge Works</b>		
- Binjai Bridge, W=16.0 m, L=40.8 m	L.S.	1
- Amplas Bridge, W=16.0 m, L=40.8 m	L.S.	1
<b>Drainage Works</b>		
- Drainage Box Culvert, 1.5 m x 1.5 m x 2	site	1
- Drainage Box Culvert, 2.0 m x 2.0 m	site	1
- Drainage Box Culvert, 2.0 m x 2.0 m x 2	site	1
- Drainage Box Culvert, 2.1 m x 2.4 m x 2	site	1
- Drainage Pipe, Dia 1,000 mm x 2	site	1
- Drainage Pipe, Dia 1,000 mm	site	2
- Drainage Pipe, Dia 800 mm	site	4
- Drainage Pipe, Dia 600 mm	site	6
- Drainage Ditch, W=600 mm	site	3

## (2) Site Condition

- (a) The construction of drainage outlets, which are located beside the bridge, is carried out after the bridge work.

- (b) Denai Bridge and Amplas Bridge require temporary bridges at the downstream sections of the existing bridge sites. Coffering length for bridge abutment could be reduced by pushing soil to the riverside.
- (c) Protection works of the National Road Bridge requires coffering and dewatering works. Concrete blocks, 1.0 × 1.0 × 0.5 m, are planned for riverbed protection. The placement of concrete blocks requires a crane pontoon at the middle of the river channel, and placement on either side of the river could be handled by a crawler crane. Coffering and dewatering is required for revetment work; cofferdam should be higher than the normal water level and execution is in the dry season.

(3) Number of Working Sets and Period

The number of sets of major civil works are as estimated below, and the construction schedule is shown in Fig. 7.1.11.

Work Item	No. of Sets	Period (month)
Clearing and Grubbing	3	4.0
Stripping	1	0.5
Excavation (Common)	2	7.5
Excavation (River Channel)	1	10.5
Embankment	1	5.0
Revetment (Wet Stone Masonry)	3	5.5
Sodding	2	4.0

**MFC-5**

MFC-5 covers the stretch of 2,680 m from PE274 to PE274+320 and from FW0 to FW24.

(1) Work Quantity

Based on the design of river improvement, the work volumes for MFC-5 are as calculated below.

Work Item	Unit	Quantity
<b>River Works</b>		
- Clearing and Grubbing	m <sup>2</sup>	152,900
- Stripping	m <sup>3</sup>	4,210
- Excavation (Floodway)	m <sup>3</sup>	646,300
- Embankment	m <sup>3</sup>	15,800
- Floodway Groundsill	L.S.	1
- Revetment (Wet Stone Masonry)	m <sup>3</sup>	19,100
- Sodding	m <sup>2</sup>	35,200
<b>Bridge Works</b>		
- Jalan Bajak Bridge, W=7.0 m, L=31.6 m	L.S.	1
- PTP-IX Bridge, W=9.0 m, L=31.6 m	L.S.	1
- Pipe Bridge, Dia 800 mm	L.S.	1



Drainage Works		
- Drainage Box Culvert, 2.0 m x 2.0 m	site	1
- Drainage Pipe, Dia 1,000 mm	site	3
- Drainage Pipe, Dia 800 mm	site	1

## (2) Site Condition

- (a) Loading equipment (patrol crane) for bridge work can be reused for another bridge construction.
- (b) Dewatering work is executed before the rainy season and all local excavation for bridge foundation is provided with dewatering channel or pumping system.
- (c) Hauling road for excavation materials is temporarily placed inside the Floodway area to avoid disturbance to the local traffic system.
- (d) Bridge work requires a site yard for erection and tensioning of PC beam girder segments. The site yard may be used as a stockyard for materials such as concrete beams, cables, and anchors. Since the main beam is 31.6 m long, the site yard shall at least be 60.0 m long by 30.0 m wide.
- (e) Excavation of the Floodway is executed adopting a bench-cut method, and the progress of excavation is adjusted with the related works such as bridge foundation works. Excavation work will be executed from the downstream to upstream, and left side ditch excavation for dewatering fishpond is planned in the beginning of the construction period.
- (f) Fishponds at the left side of the Floodway are promising spoil areas. Dewatering of fishponds shall discharge into the floodway channel. The surface layer of the bottom of fishponds which may be muddy and silty is required to dry-up and be separated from backfill soil.

## (3) Number of Working Sets and Period

The number of sets of major civil works are as estimated below, and the construction schedule is shown in Fig. 7.1.12.

Work Item	No. of Set	Period (month)
Clearing and Grubbing	3	3.0
Stripping	0.5	0.5
Excavation (Floodway)	3	7.5
Embankment	1	1.5
Revelment (Wet Stone Masonry)	6	9.0
Sodding	1	4.5

**MFC-6**

MFC-6 is the stretch of 1,010 m from FW24 to FW34.

(1) Work Quantity

Based on the design of river improvement, the work volumes for MFC-6 are as calculated below.

Work Item	Unit	Quantity
<b>River Works</b>		
- Clearing and Grubbing	m <sup>2</sup>	63,200
- Stripping	m <sup>3</sup>	2,270
- Excavation (Floodway)	m <sup>3</sup>	290,200
- Embankment	m <sup>3</sup>	4,620
- Revetment (Concrete)	m <sup>3</sup>	29,800
- Revetment (Wet Stone Masonry)	m <sup>3</sup>	3,140
- Sodding	m <sup>2</sup>	5,680
<b>Bridge Works</b>		
- STM Ujung Bridge, W=9.0 m, L=31.6 m	L.S.	1
- Railway Bridge, W=3.0 m, L=31.6 m	L.S.	1
- Deli Dua Bridge, W=16.0 m, L=31.6 m	L.S.	1
- Pipe Bridge, Dia 300 mm	L.S.	1
- Pipe Bridge, Dia 600, 300 mm	L.S.	1
<b>Drainage Works</b>		
- Batuan River Box Culvert, 2.0 m x 2.0 m x 2	site	1
- Drainage Pipe, Dia 1,000 mm	site	1

(2) Site Condition

- (a) Dewatering work is executed before the rainy season and all local excavation for bridge foundations is provided with a dewatering ditch or pumping system. Since Batuan River will be the dewatering channel for MFC-6, dewatering ditches shall be constructed along both sides of the floodway.
- (b) Jalan Deli Tua is one of the main roads in Titi Kuning. STM Ujung Bridge will be constructed before detouring Jalan Deli Tua.
- (c) Loading equipment for bridge works can be sequentially used for another bridge construction.
- (d) Hauling road for excavation materials can be located in the area of the floodway to avoid disturbance to the local traffic system.
- (e) Bridge work requires a site yard for erection and tensioning of the PC girder segment.
- (f) Excavation of the floodway is executed adopting a bench-cut method, and the progress of excavation is adjusted with the related works such as bridge foundation works.
- (g) Revetment work in MFC-6 has two types, wet stone masonry type and concrete type. Leaning wall of concrete type revetment shall be constructed by the optimum

working procedure of concreting, because wall height is from 2.0 m to 7.0 m and the concreting height for one placement is 1.5 m to 2.0 m on account of the daily work volume.

(3) Number of Working Sets and Period

The number of sets of major civil works are as estimated below, and the construction schedule is shown in Fig. 7.1.13.

Work Item	No. of Sets	Period (month)
Clearing and Grubbing	1.5	2.5
Stripping	0.5	0.5
Excavation (Floodway)	1	10.0
Embankment	0.5	1.0
Revetment (Concrete)	6	16.5
Revetment (Wet Stone Masonry)	2	4.5
Sodding	1	1.0

**MFC-7**

MFC-7 is the stretch of FW34 to FW39+50 in the Floodway and DU10+46.5 to DU23.0 in the Upper Deli River, with a total length of 1,500 m.

(1) Work Quantity

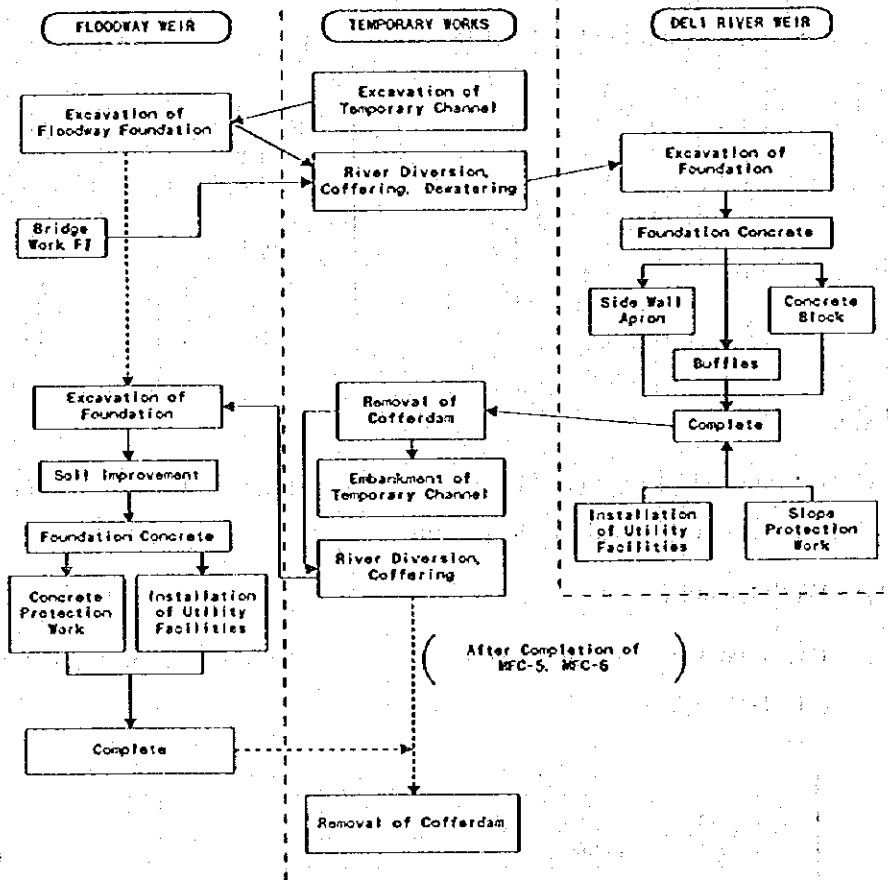
Based on the design of river improvement, the work volumes for MFC-7 are as calculated below.

Work Item	Unit	Quantity
<b>River Works</b>		
- Clearing and Grubbing	m <sup>2</sup>	143,700
- Stripping	m <sup>3</sup>	24,800
- Excavation (Common)	m <sup>3</sup>	40,500
- Excavation (Floodway)	m <sup>3</sup>	137,000
- Excavation (River Channel)	m <sup>3</sup>	13,000
- Embankment	m <sup>3</sup>	267,700
- Floodway Weir	L.S.	1
- Deli River Weir	L.S.	1
- Revetment (Concrete)	m <sup>3</sup>	22,600
- Revetment (Wet Stone Masonry)	m <sup>3</sup>	5,120
- Sodding	m <sup>2</sup>	31,700
<b>Bridge Works</b>		
- Pipe & Pedestrian Bridge, W=3.0 m, L=31.6 m	L.S.	1
- Jalan SMA Bridge, W=4.5 m, L=16.6 m	L.S.	1
- Gg. Seksama Bridge, W=2.0 m, L=58.8 m	L.S.	1
<b>Drainage Works</b>		
- Drainage Pipe, Dia 1,000 mm	site	1

(2) Site Condition

(a) Deli River Weir is located in the narrow valley-shaped section, therefore, it is difficult to execute coffering in the river channel and, further, costly to carry out

coffering within the river channel. A temporary diversion channel is proposed from the Floodway Weir to the section downstream of Deli River Weir (refer to Fig. 7.1.14). The sequence of construction work on the Floodway Weir and the Deli River Weir is as illustrated below.



(b) The embankment work at the left side of retarding channel requires a temporary bridge for crossing Deli River. This temporary bridge can be combined with the detour bridge for Gg. Seksama Bridge (F8).

(c) Miscellaneous works consist of tree planting along the Floodway, installation of roofed bench with related facilities, and installation of guard fence, information board and inspection road.

(3) Number of Working Sets and Period

The number of sets for major civil works are as estimated below, and the construction schedule is shown in Fig. 7.1.15.

Work Item	No. of Sets	Period (month)
Clearing and Grubbing	1.5	5.0
Stripping	1	1.5
Excavation (Common)	1	2.0
Excavation (Floodway)	1	5.0
Excavation (River Channel)	1	1.5
Embankment	2	11.0
Revetment (Concrete)	7	11.0
Revetment (Wet Stone Masonry)	3	5.0
Sodding	2	2.0

### 7.1.3 Overall Construction Plan

The overall construction plan of the Medan Flood Control Project is prepared by integrating all of the construction plans of the respective portions in consideration of the following items:

- (1) The river improvement works are generally executed from the downstream to upstream.
- (2) To assure the smooth and enough supply of construction materials as well as procurement of construction equipment and labor, the same type of civil works shall be sequentially arranged.
- (3) The embankment works shall be carried out in the dry season for higher work efficiency. Further, the embankment works shall be well arranged with the excavation works so as to minimize hauling cost and storage cost of the materials.
- (4) Reconstruction or new construction of bridges shall be carefully examined whether or not the work may disturb or deteriorate the local traffic system. The necessity of temporary bridge and detouring shall also be examined to find the best arrangement or coordination of the work.

Based on the examination of the aforementioned conditions, the overall construction schedule is prepared, as shown in Fig. 7.1.16. The monthly number of construction equipment and labor for each portion of work are shown in Tables 7.1.1 to 7.1.7.

### 7.1.4 Soil Balance

#### Embankment Materials

Embankment materials are mainly supplied from the excavated soils of construction works through the selection of suitable soils for embankment. Based on the results of the geological investigation and soil mechanics tests, it has been identified that the lower reaches of Percut River is covered by thin clay layer and thick, poor grade sand layer, and that silty and sandy

clay layer increases in thickness at the upper reaches from PE50. The silty sand around Bandar Sidoras Intake Weir should then be suitable for embankment materials.

The estimation of available volume for embankment is made for each portion of the Project. Based on the above study, MFC-1 and MFC-7 do not have enough volume for embankment works (refer to Table 7.1.8). Embankment materials for MFC-1 are brought from MFC-2 and MFC-3 and their quantities are estimated at 2,050 m<sup>3</sup> and 189,450m<sup>3</sup>, respectively. The embankment materials for MFC-7 is provided by 94,830 m<sup>3</sup> from MFC-6.

### Volume for Spoil Area

In the balance calculation of embankment volume among portions, the surplus soil volume will be hauled to the spoil area. The surplus soil is mostly brought by dredging, stripping and excess soil from embankment. The materials produced by clearing and grubbing works will be arranged at the site. The surplus soil volume for each portion is calculated as shown in Table 7.1.9. Consequently, the total volume for the spoil area is estimated at 2,373,600 m<sup>3</sup>.

## **7.2 Cost Estimate**

### **7.2.1 Cost Estimation Conditions**

Project cost is estimated on the basis of the design, the construction plan, and the following assumptions and conditions.

(1) Price Level

Price level is as of November 1995.

(2) Currency Conversion Rate

Currency conversion rates among U.S. Dollar (US\$), Indonesian Rupiah (Rp.) and Japanese Yen (¥) are: US\$1.00 = Rp 2,285 = ¥103.6.

(3) Currency of Cost Estimate

Construction cost is estimated in foreign and local currencies. Both of the estimated costs are expressed in Indonesian Rupiah using the currency conversion rates stated above.

### **7.2.2 Cost Estimation Method**

Costs for civil works are estimated on the unit price basis. The unit prices are composed of the unit costs of labor, material, equipment, miscellaneous and contractor's indirect cost.

(1) Unit Price

(a) Labor Cost

The unit costs of labor are estimated using the local basic wage in Medan for 1995-1996 and are calculated based on the regulations for employment (HIMPUNAN PERATURAN, 1988) in Indonesia. The estimated costs of labor per day are shown in Table 7.2.1.

(b) Material Cost

The unit costs of construction materials are estimated based on the data collected at the project site at November 1995 prices. The cost of material which has to be supplied from outside of Medan area shall include transportation and insurance costs to the site. The materials which have to be imported shall consider insurance, freight to port of destination, transport to port and transport to the site. The estimated costs of materials are shown in Table 7.2.2.

(c) Equipment Cost

The unit costs of construction equipment are estimated based on the local government prices of 1991 and consider the price escalation of 4% for each year. The price of equipment is calculated on rental basis per hour. The estimated costs of equipment per hour are shown in Table 7.2.3.

(d) Indirect Cost

The Contractor's indirect cost is computed in percentage of each unit cost. Indirect cost is estimated at 7% for contractor's profit and 8% for overhead and other incidentals.

(2) Value Added Tax

Value Added Tax is estimated at 10% and included in the construction cost, except for the direct foreign currency.

**7.2.3 Bill of Quantities**

Based on the structural design, a draft Bill of Quantities for items of civil works is prepared as hereto attached (see ANNEX).

**TABLES**

**CHAPTER 7**

**CONSTRUCTION PLAN AND COST ESTIMATE**

















Table 7.1.8 BALANCE OF SOIL VOLUME FOR EACH WORK PORTION (FOR EMBANKMENT)

(UNIT : m<sup>3</sup>)

WORK ITEM	MFC-1	MFC-2	MFC-3	MFC-4	MFC-5	MFC-6	MFC-7	TOTAL
I. Excavation (Common)	139,600	340,900	626,300	342,900	654,000	290,200	189,300	2,543,000
2. Excavation (Riverbed)	164,600	166,200	161,900	99,500	0	0	13,000	608,000
I. Volume can be used for Embankment	72,300	340,050	596,050	328,510	588,600	261,180	172,970	2,359,660
3. Embankment	263,800	338,000	95,200	59,200	15,800	4,600	267,800	912,000
II. Remained Volume after Embankment	231,900	167,050	503,550	383,200	638,200	190,770	29,330	2,144,000
4. Dredging	82,600	0	0	0	0	0	0	84,000
5. Clearing and Grubbing (m <sup>2</sup> )	273,700	528,000	332,400	228,700	159,100	63,200	143,700	1,764,000
6. Stripping	32,500	61,100	16,300	8,700	4,200	2,300	21,900	148,000
Total of Remained Volume (II + 4. +6.)	347,000	228,150	519,850	391,900	642,400	193,070	51,230	2,373,600

Note : Clearing and Grubbing Volume will is not counted in the remained volume.



Table 7.1.9 BALANCE OF SOIL VOLUME FOR EACH WORK PORTION (FOR SPOIL AREA)

UNIT : m<sup>3</sup>

WORK ITEM	MFC-1	MFC-2	MFC-3	MFC-4	MFC-5	MFC-6	MFC-7	TOTAL
1. Excavation (Common)								
Right	105,100	153,100	345,900	159,000	654,000	290,200	170,900	1,878,200
Left	34,500	187,800	280,400	183,900	0	0	18,400	705,000
Sub-Total	139,600	340,900	626,300	342,900	654,000	290,200	189,300	2,583,200
Satisfied Volume for Embankment								
Right	42,040	137,790	311,310	143,100	588,600	261,180	153,810	1,637,830
Left	13,800	169,020	252,360	165,510	0	0	16,560	617,250
Sub-Total	55,840	306,810	563,670	308,610	588,600	261,180	170,370	2,255,080
2. Excavation (Riverbed)								
Right	129,100	63,400	91,800	43,400	0	0	8,200	335,900
Left	35,500	102,800	70,100	56,100	0	0	4,800	269,300
Sub-Total	164,600	166,200	161,900	99,500	0	0	13,000	605,200
Satisfied Volume for Embankment								
Right	12,910	12,680	18,360	8,680	0	0	1,640	54,270
Left	3,550	20,560	14,020	11,220	0	0	960	50,310
Sub-Total	16,460	33,240	32,380	19,900	0	0	2,600	104,580
3. Embankment								
Right	179,500	184,400	45,000	30,800	15,800	4,600	161,700	621,800
Left	84,300	153,600	50,200	28,400	0	0	106,100	422,600
Sub-Total	263,800	338,000	95,200	59,200	15,800	4,600	267,800	1,044,400
Volume Balance after Embankment								
Right	-124,550	-33,930	284,670	120,980	572,800	256,580	-6,250	1,070,300
Left	-66,950	35,980	216,180	148,330	0	0	-88,580	244,960
Sub-Total	-191,500	2,050	500,850	269,310	572,800	256,580	-94,830	1,315,260
Remained Volume	0	0	311,400	269,310	572,800	161,750	0	1,315,260
Embankment Material from other Package								
MFC-1	0	2,050	189,450	0	0	0	0	0
MFC-2	-2,050	0	0	0	0	0	0	0
MFC-3	-189,450	0	0	0	0	0	0	0
MFC-4	0	0	0	0	0	0	0	0
MFC-5	0	0	0	0	0	0	0	0
MFC-6	0	0	0	0	0	0	0	0
MFC-7	0	0	0	0	0	94,830	0	0

Legend :  
 - minus get from other Package  
 - plus produce to other Package

Table 7.2.1 BASIC UNIT COSTS OF LABOUR AND COMPUTATION OF LABOUR COSTS IN KAB. DELI SERDANG 1995-1996

(Unit : Rupiah)

Qualification	Basic Wage	Additional Cost (Monthly Base)				COST PER DAY	Rounded
	(1) Daily	(2) Overtime	(3) Leave	(4) Bonus	(5) Others		
1 Foreman	10,000	5,000	833	833	2,500	19,167	19,200
2 Operator	10,000	5,000	833	833	2,500	19,167	19,200
3 Electrician	9,000	4,500	750	750	2,250	17,250	17,300
4 Mechanic	9,000	4,500	750	750	2,250	17,250	17,300
5 Welder	9,000	4,500	750	750	2,250	17,250	17,300
6 Driver	7,500	3,750	625	625	1,875	14,375	14,400
7 Mason	8,000	4,000	667	667	2,000	15,333	15,300
8 Carpenter	9,000	4,500	750	750	2,250	17,250	17,300
9 Scaffolding Man	8,000	4,000	667	667	2,000	15,333	15,300
10 Plumber	8,000	4,000	667	667	2,000	15,333	15,300
11 Steel Worker	8,000	4,000	667	667	2,000	15,333	15,300
12 Concrete Worker	8,000	4,000	667	667	2,000	15,333	15,300
13 Painter	8,000	4,000	667	667	2,000	15,333	15,300
14 Plasterer	8,000	4,000	667	667	2,000	15,333	15,300
15 Asphalt Worker	8,000	4,000	667	667	2,000	15,333	15,300
16 Skilled Labour	6,000	3,000	500	500	1,500	11,500	11,500
17 Common Labour	5,000	2,500	417	417	1,250	9,583	9,600
18 Watchman	5,500	2,750	458	458	1,375	10,542	10,500

(1) : SOURCE; DAFTAR HARGA SATUAN BAHAN - UPAH KERJA  
TAHUN ANGGARAN 1995-1996

(2) : 7 Basic Working Hours + 2 Hours Overtime +  
1 Hour Rest Time

- Overtime, 1st Hour; Basic Wage / 7 \* 1.5
- Overtime, 2nd Hour; Basic Wage / 7 \* 2.0

(3) : Vacation and Sick Leave ;

Basic Wage / 12

(4) : Bonus or 13th Month Pay ;

Basic Wage / 12

(5) : Food, Transportation and other Allowances +  
Taxes and Insurance

((1) + (2) + (3) + (4)) \* 0.15

**Table 7.2.2 (1/3) BASIC UNIT COSTS OF CONSTRUCTION MATERIALS  
IN MEDAN CITY**

(Unit: Rp.)

No.	Materials	Unit	Local Currency (Rp.)	Foreign Currency (Rp.)	Total (Rp.)
<b>A. Combustibles</b>					
1	Gasoline	lit.	560	140	700
2	Light Oil (Diesel Oil)	lit.	304	76	380
3	Kerosene	lit.	210	140	350
4	Propane Gas	kg	600	150	750
5	Oxygen (big tube)	7m <sup>3</sup>	45,000	5,000	50,000
6	Grease	kg	3,600	400	4,000
<b>B. Sand and Stones</b>					
1	Fine Aggregate (Washed Sand) (10km)	m <sup>3</sup>	11,880	1,320	13,200
2	Sand for Filling and Base Course (10 km)	m <sup>3</sup>	9,900	1,100	11,000
3	Cobble Stone (Average Delivery Dis. 20km)	m <sup>3</sup>	13,500	1,500	15,000
4	Crasher Run (Average Delivery Dis. 20 km)	m <sup>3</sup>	31,500	3,500	35,000
5	River Gravel (Average Delivery Dis. 25 km)	m <sup>3</sup>	18,000	2,000	20,000
6	Boulder (Average Delivery Dis. 25km)	m <sup>3</sup>	19,800	2,200	22,000
<b>C. Concrete and Asphalt</b>					
1	Portland Cement	bag	5,100	3,400	8,500
2	Cut-back Asphalt	kg	480	320	800
3	Asphalt Concrete (Hot Mix)	ton	66,000	44,000	110,000
4	Asphalt Tack Coat	lit	720	480	1,200
5	Asphalt Prime Coat	lit	600	400	1,000
6	Ready Mixed Concrete; 400 kg/cm <sup>2</sup> , 25 mm (A)	m <sup>3</sup>	109,560	73,040	182,600
7	Ready Mixed Concrete; 350 kg/cm <sup>2</sup> , 25 mm (B)	m <sup>3</sup>	104,940	69,960	174,900
8	Ready Mixed Concrete; 225 kg/cm <sup>2</sup> , 25 mm (C1)	m <sup>3</sup>	95,040	63,360	158,400
9	Ready Mixed Concrete; 225 kg/cm <sup>2</sup> , 10 mm (C2)	m <sup>3</sup>	90,720	60,480	151,200
10	Ready Mixed Concrete; 175 kg/cm <sup>2</sup> , 40 mm (D)	m <sup>3</sup>	87,420	58,280	145,700
11	Ready Mixed Concrete; 120 kg/cm <sup>2</sup> , 25 mm (E)	m <sup>3</sup>	80,160	53,440	133,600
12	Prestressed Concrete Pile Dia. 300 mm A	m	23,800	10,200	34,000
13	Prestressed Concrete Pile Dia. 300 mm AB	m	28,700	12,300	41,000
14	Prestressed Concrete Pile Dia. 400 mm B	m	52,500	22,500	75,000
15	Prestressed Concrete Pile Dia. 400 mm AB	m	48,300	20,700	69,000
16	Prestressed Concrete Pile Dia. 500 mm AB	m	73,500	31,500	105,000
17	Prestressed Concrete Pile Dia. 600 mm AB	m	99,400	42,600	142,000
18	Concrete Pipe (Without Re-bar) Dia. 600 mm	m	18,200	7,800	26,000
19	Concrete Pipe (Without Re-bar) Dia. 400 mm	m	8,400	3,600	12,000
20	Reinforced Concrete Pipe, Dia. 300 mm	m	17,400	11,600	29,000
21	Reinforced Concrete Pipe, Dia. 400 mm	m	27,000	18,000	45,000
22	Reinforced Concrete Pipe, Dia. 500 mm	m	43,200	28,800	72,000
23	Reinforced Concrete Pipe, Dia. 600 mm	m	58,800	39,200	98,000
24	Reinforced Concrete Pipe, Dia. 800 mm	m	104,400	69,600	174,000
25	Reinforced Concrete Pipe, Dia. 1000 mm	m	162,600	108,400	271,000
26	Concrete Block for Pavement : 21 x 10.5 x 8 cm	pcs	440	110	550
27	Concrete Hollow Block : 40 x 20 x 10 cm	pcs	496	124	620
28	Form Tie	pcs	3,000	2,000	5,000
29	Non Shrinkage Mortar	m <sup>3</sup>	41,500	41,500	83,000
30	Sealant	m <sup>3</sup>	40,000	40,000	80,000
31	Prestressed Concrete Sheet Pile	m	86,064	57,376	143,440
32	Precast Prestressed Concrete Main Beam	m <sup>3</sup>	1,010,400	673,600	1,684,000
33	Precast Prestressed Panel	m <sup>3</sup>	280,000	280,000	560,000
34	Precast Prestressed Concrete Diaphragm	m <sup>3</sup>	339,200	339,200	678,400
<b>D. Log and Timber</b>					
1	Log Pile, Dia. 15 cm	m	1,980	220	2,200
2	Log Pile, Dia. 10 cm	m	990	110	1,100
3	Timber	m <sup>3</sup>	337,500	37,500	375,000
4	Plywood, t = 12 mm	m <sup>2</sup>	12,600	1,400	14,000
5	Door incl. Frame Accessories, 2.0 x 0.8 m	nos.	110,880	12,320	123,200
6	Form Timber	m <sup>3</sup>	405,000	45,000	450,000
7	Coconut Pile, Dia. 25 cm, 10-12 m	nos.	45,000	5,000	50,000

Table 7.2.2 (2/3) BASIC UNIT COSTS OF CONSTRUCTION MATERIALS  
IN MEDAN CITY

(Unit : Rp.)

No.	Materials	Unit	Local Currency (Rp.)	Foreign Currency (Rp.)	Total (Rp.)
<b>E. Iron</b>					
1	Reinforcing Bars, Round	kg	165	1,485	1,650
2	Reinforcing Bars, Deformed	kg	165	1,485	1,650
3	Structural Steel	kg	315	2,835	3,150
4	Steel Plate	kg	225	2,025	2,250
5	Steel Pipe, Dia. 800 mm incl. Coating & Lining	m	79,940	719,460	799,400
6	Steel Pipe, Dia. 600 mm incl. Coating & Lining	m	69,750	627,750	697,500
7	Steel Pipe, Dia. 400 mm incl. Coating & Lining	m	46,500	418,500	465,000
8	Steel Pipe, Dia. 300 mm incl. Coating & Lining	m	41,000	369,000	410,000
9	Steel Pipe, Dia. 150 mm incl. Coating & Lining	m	18,500	166,500	185,000
10	Steel Pipe, Dia. 125 mm incl. Coating & Lining	m	15,500	139,500	155,000
11	Steel Pipe, Dia. 100 mm incl. Coating & Lining	m	9,300	83,700	93,000
12	Steel Pipe, Dia. 38 mm (1.5 inch)	m	2,025	18,225	20,250
13	Steel Pipe Pile, Dia. 600 mm (spiral welded)	m	36,000	324,000	360,000
14	Steel Pipe Pile, Dia. 400 mm (spiral welded)	m	23,800	214,200	238,000
15	Galvanized Steel Pipe, Dia. 40 mm	m	2,213	19,912	22,125
16	Galvanized Steel Pipe, Dia. 50 mm	m	2,775	24,975	27,750
17	Galvanized Steel Pipe, Dia. 75 mm	m	4,500	40,500	45,000
18	Galvanized Steel Pipe, Dia. 100 mm	m	6,000	54,000	60,000
19	Steel Door, 40 mm thick, 2.10 x 1.70 m	pes	285,000	2,565,000	2,850,000
20	Steel Sheet Pile	ton	247,500	2,227,500	2,475,000
21	Expansion Joint, Steel Profile L-75 x 6 mm	m	705	6,345	7,050
22	Anchor, Steel Bar (Dia. 32 & 22) incl. PVC Pipe	nos	3,000	27,000	30,000
23	Galvanized Steel Wire	kg	500	2,000	2,500
24	Bolt and Nut	kg	3,750	33,750	37,500
25	Welding Rod	kg	1,950	7,800	9,750
26	Galvanized Steel Fence, H = 1.75 m	m	20,580	82,320	102,900
27	Gabion Mattress; 4 mm, 1.5 x 3.0 x 0.5 m	pes	34,000	136,000	170,000
28	Gabion Cylinder; 4 mm, Dia. = 50 cm	m	5,000	20,000	25,000
29	Zinc Roof	m <sup>2</sup>	1,800	7,200	9,000
30	Checkered Steel Plate, 6 mm thick	kg	450	1,800	2,250
31	Live and Anchorage	set	189,000	441,000	630,000
32	Steel Pipe, Dia. 350 mm incl. Coating & Lining	m	43,750	393,750	437,500
33	Steel Pipe, Dia. 200 mm incl. Coating & Lining	m	25,150	226,350	251,500
<b>F. Valves</b>					
1	Air Valve, Dia 25 mm	set	124,200	496,800	621,000
2	Sluice Valve for 400 mm Dia. Pipe	set	1,725,000	6,900,000	8,625,000
3	Counterflow Prevention Valve for 100 mm Dia. Pipe	set	0	45,423	45,423
4	Butterfly Valve for 600 mm Dia. Pipe	set	2,250,000	9,000,000	11,250,000
5	Butterfly Valve for 400 mm Dia. Pipe	set	1,500,000	6,000,000	7,500,000
6	Flap Gate 1000 mm Dia	set	4,684,800	3,123,200	7,808,000
7	Flap Gate 600 mm Dia	set	2,812,800	1,875,200	4,688,000
8	Air Valve Dia. 75 mm	set	903,600	602,400	1,506,000
9	Air Valve, Dia. 50 mm	set	638,400	425,600	1,064,000
10	Steel Gate 2.0 * 2.0 m (Slide Gate Type)	set	9,000,000	6,000,000	15,000,000
11	Steel Gate 2.0 * 1.5 m (Slide Gate Type)	set	6,969,600	4,646,400	11,616,000
12	Steel Gate 1.5 * 1.5 m (Slide Gate Type)	set	5,270,400	3,513,600	8,784,000
13	Steel Gate 1.0 * 1.25 m (Slide Gate Type)	set	4,128,000	2,752,000	6,880,000
14	Steel Gate 1.0 * 1.0 m (Slide Gate Type)	set	3,600,000	2,400,000	6,000,000
15	Expansion Joint for Pipe, Dia. 100 mm	nos.	852,000	1,988,000	2,840,000
16	Expansion Joint for Pipe, Dia. 125 mm	nos.	952,500	2,222,500	3,175,000
17	Expansion Joint for Pipe, Dia. 150 mm	nos.	1,047,900	2,445,100	3,493,000
18	Expansion Joint for Pipe, Dia. 300 mm	nos.	1,092,900	2,550,100	3,643,000
19	Expansion Joint for Pipe, Dia. 350 mm	nos.	1,293,900	3,019,100	4,313,000
20	Expansion Joint for Pipe, Dia. 400 mm	nos.	2,119,500	4,945,500	7,065,000
21	Expansion Joint for Pipe, Dia. 600 mm	nos.	2,338,500	5,456,500	7,795,000
22	Expansion Joint for Pipe, Dia. 800 mm	nos.	5,268,300	12,292,700	17,561,000

Table 7.2.2 (3/3) BASIC UNIT COSTS OF CONSTRUCTION MATERIALS  
IN MEDAN CITY

(unit : Rp.)

No.	Materials	Unit	Local Currency (Rp.)	Foreign Currency (Rp.)	Total Nov.1.95
<b>G. Chemicals</b>					
1	PVC Pipe, Dia. 150 mm	m	29,925	12,825	42,750
2	PVC Pipe, Dia. 100 mm	m	14,490	6,210	20,700
3	PVC Pipe, Dia. 75 mm	m	7,350	3,150	10,500
4	PVC Pipe, Dia. 50 mm	m	5,250	2,250	7,500
5	PVC Air Vent Pipe, Dia. 75 mm, 80 cm Long	pcs	24,500	10,500	35,000
6	Elastic Joint Filler 10 mm thick	m <sup>2</sup>	17,500	7,500	25,000
7	Geotextile	m <sup>2</sup>	5,250	2,250	7,500
8	Waterstop, B = 300 mm	m	31,500	13,500	45,000
9	Elastomeric Bearing, 300 x 300 x 60 mm	pcs	175,000	75,000	250,000
10	Elastomeric Bearing, 406 x 280 x 67 mm	pcs	218,750	93,750	312,500
11	Elastomeric Bearing, 480 x 300 x 67 mm	pcs	280,000	120,000	400,000
12	PVC Pipe, Dia. 200 mm	m	48,825	20,925	69,750
<b>II. Rubber Dam</b>					
1	Rubber Body (6.315*32.33 m)	L.S	0	595,350,000	595,350,000
2	Fixing Materials (Anchor Bolts, Plate, etc.)	L.S	0	529,200,000	529,200,000
3	Spacer Parts and Pipelines	L.S	0	66,150,000	66,150,000
4	Operation Equipment	L.S	0	132,300,000	132,300,000
<b>I. Plants and Grass</b>					
1	Acasia Petandra	tree	7,000	0	7,000
2	Fillisium	tree	10,500	0	10,500
3	Bougainvillea	tree	5,000	0	5,000
4	Cemara Kipas	tree	35,000	0	35,000
5	Cemara Lilin	tree	35,000	0	35,000
6	Palem Hijau	tree	15,000	0	15,000
7	Tanjung	tree	10,500	0	10,500
8	Cendrawasih/Taiwan Lila	tree	105	0	105
9	Soka	tree	250	0	250
10	Filling of Fertilized Soil	kg	1,200	0	1,200
<b>J. Others</b>					
1	Palm Fiber, 20 mm thick	m <sup>2</sup>	18,000	2,000	20,000
2	Brick; 23 x 11.5 x 5.5 cm	pcs	90	10	100
3	Floor Tile	m <sup>2</sup>	16,650	1,850	18,500
4	Wall Tile	m <sup>2</sup>	15,750	1,750	17,500
5	Mosaic Tile	m <sup>2</sup>	20,790	2,310	23,100
6	Roof Tile	m <sup>2</sup>	16,650	1,850	18,500
7	Wall Paint	m <sup>2</sup>	6,750	750	7,500
8	Window with Accessory, 0.6 x 1.20 m	m <sup>2</sup>	36,000	4,000	40,000
9	Synthetic Shell (5 m <sup>3</sup> / kg)	kg	3,125	3,125	6,250
10	Water Proofing Coat	m <sup>2</sup>	10,200	10,200	20,400
11	Staff Gauge (5.0 m)	nos	140,000	60,000	200,000
12	Asbestos Cemct, 6 mm thick	m <sup>2</sup>	875	875	1,750
13	Aluminium Frame	m <sup>2</sup>	125,000	125,000	250,000
14	Cast-iron Cover, Dia.60 cm	pcs	400,000	400,000	800,000
15	Nails	kg	1,200	800	2,000
16	Handy Talky	set	240,000	160,000	400,000
17	Trash Can	nos	28,000	7,000	35,000
18	Trash Carrier	nos	128,000	32,000	160,000
19	Water Tank, 5.0 m <sup>3</sup>	nos	2,500,000	625,000	3,125,000
20	Maintenance Post Marker	nos	39,000	26,000	65,000
21	Submerged Motor Pump, 0.1 m <sup>3</sup> /min. 2.2 kw	nos	3,525,000	2,350,000	5,875,000
22	Submerged Motor Pump, 0.1 m <sup>3</sup> /min. 1.5 kw	nos	2,550,000	1,700,000	4,250,000
23	Name Plate (marble)	m <sup>2</sup>	360,000	40,000	400,000
24	Bench (wooden)	nos.	165,000	110,000	275,000
25	Bench (steel)	nos.	200,000	0	200,000
26	Sodding Grass	m <sup>2</sup>	500	0	500

Source:

1 DAFTAR HARGA SATUAN BAHAN - UPAHKERJA, TAHUN ANGGARAN 1995-1996  
DI DAFTAR II KABUPATEN DELI SERDANG.

2 JURUAL HARGA SATUAN BAHAN BANGUNAN DAN KONSTRUKSI, EDISI III September 1995

Table 7.2.3 BASIC UNIT COSTS OF CONSTRUCTION EQUIPMENT

Construction Equipment	Capacity/ Specification	Power	Type	Economic Life (Year)	Yearly Working (hour)	Basic Price (Rp./hour)
1. Bulldozer A	15 ton	145 PS	D58E-1	5	2,000	74,900
2. Bulldozer B	12 ton	100 PS		5	2,000	60,200
3. Bulldozer C	6 ton	71 PS		5	2,000	37,700
4. Bulldozer with Ripper	21 ton	225 PS	D53A-17	5	2,000	119,400
5. Back hoe / Excavator A	0.60 m3	120 PS	PC200-5	4	2,000	65,700
6. Back hoe / Excavator B	0.40 m3	88 PS	PC100-5	4	2,000	43,000
7. Crawler-Type Loader	1.20 m3	93 PS	951C	5	2,000	49,500
8. Wheel-Type Loader	1.20 m3	81 PS	920	5	2,000	55,600
9. Dump Truck A	8 ton	253 PS	TDZ-60	5	2,000	33,000
10. Dump Truck B	6 ton	188 PS	TSD-6	5	2,000	16,500
11. Ordinary Truck	4 ton	180 PS	TE-100	5	2,000	12,300
12. Wheel Crane A	10 ton	160 PS	KB-122	5	2,000	38,200
13. Wheel Crane B	4.9 ton	50 PS	TSO45	5	2,000	33,400
14. Truck with Crane	6 ton	190 PS	TXO-60	5	2,000	19,400
15. Truck with Crane	4 ton	162 PS	TS-3	5	2,000	12,900
16. Trailer A	35 ton	320 PS	VOZ-441	5	2,000	61,700
17. Trailer B	20 ton	320 PS	F-2100	5	2,000	43,500
18. Truck Mixer	4.5 ton	290 PS	TDJ-50	5	2,000	49,700
19. Concrete Pump Truck	55 m3/hr	164 PS	NCP-900	4	2,000	62,400
20. Water Tanker	4 m3		4862-WD	5	2,000	14,100
21. Tamper	80/100 kg	4 PS	BS-60Y	3	1,600	5,500
22. Vibrator Roller A	1 ton	8 PS	SV-10	5	1,600	13,100
23. Vibrator Roller B	4 ton	27.3 PS	SV-25	5	1,600	26,700
24. Vibrator Roller C	12 ton	91 PS	SV-900	6	2,000	66,500
25. Tire Roller	8/12 ton	99 PS	TS-150	7	1,600	36,500
26. Tandem Roller	8/12 ton	99 PS		5	2,000	39,200
27. Motor Grader	2.8 m	94 PS	GD510R-1	6	2,000	49,500
28. Crawler Crane A	50 ton	156 PS	TO-500	6	1,400	146,600
29. Crawler Crane B	35 ton	117 PS	DH-350	6	1,400	115,500
30. Crawler Crane C	15 ton	96 PS	421D	6	1,400	64,600
31. Portable Engine Compressor	3.5m3/min.	36 PS	EC-50Hz	6	1,200	16,300
32. Diesel Engine Generator A	100KVA	120 PS	3304	6	1,600	40,400
33. Diesel Engine Generator B	35KVA	42 PS	DCA-35	6	1,600	15,100
34. Diesel Engine Generator C	15KVA	20 PS	DCA-21	6	1,600	8,400
35. Diesel Pile Hammer	2.5 ton	102 PS	335-A	4	1,400	28,500
36. Vibro Hammer	23/24 ton	30 KW		4	1,400	11,200
37. Drop Hammer	0.5 ton	9 PS		4	1,400	5,000
38. Concrete Vibrator		1 KW	Dia.0-13	3	1,600	1,300
39. Portable Concrete Mixer	0.5 m3		SM-500	3	1,600	28,900
40. Portable Concrete Mixer	0.2 m3		KNP-7US	3	1,600	9,200
41. Asphalt Plant	30 ton/hr	110 KW	BDM-50	5	1,400	192,500
42. Asphalt Finisher	2.4 m	33 PS	MP-30-2	5	1,600	41,400
43. Asphalt Sprayer	30 Lit./min.	5.5 PS	BAD-200L	5	2,000	26,300
44. Concrete Breaker A	600 kg	108 PS	HB-700	3	1,200	29,100
45. Pneumatic Hand Breaker	20 kg	8/10 PS	CB-21	3	1,200	2,000
46. Submergible Pump A	D 150mm	7.5 KW		5	1,200	3,500
47. Submergible Pump B	D 50 mm	0.75 KW		5	1,200	2,000
48. Engine Welder	250 AMP	19.1 PS	250 MK	6	1,200	7,500
49. Chain Saw		0.7 PS		6	1,200	2,400
50. Drag Line	0.8 m3			5	2,000	51,400
51. Bar Cutting Machine				6		1,100
52. Bar Bending Machine				6		3,000
53. Portal Crane	25 ton			5	2,000	56,700
54. Dredger		200 PS		5	2,000	71,600

Note :

1. Hourly costs of equipment are estimated based on owning, operation and maintenance cost system in Indonesia
2. Operator and labour cost are excluded.
3. Mobilization and demobilization cost is not included.
4. Price Escalation is considered 4 % for every year.

Source of Data :

1. Keputusan Menteri Pekerjaan Umum Nomor : 167/KPTS/1991