Manning's roughness coefficient was assumed at 0.025 taking into account of smooth river bed condition of uniform sandy materials.

3.3 SEDIMENT TRANSPORT CAPACITY AT SELECTED LOCATIONS

Sediment load was calculated by applying the above formulas at Maskup near outlet of Sacobia valley and outlet of the sand pocket in Sacobia river, at Malonzo, San Francisco bridge and Confluence of Rio Chico in Bamban/Parua river and at Friendship bridge and San Jose Malino in the Abacan river. In the study, the Bamban river system includes Marimla and Sapang Cauayang rivers but Sacobia river is separated from the Bamban river.

Table G.1 shows the sediment load and concentration for selected flow rates between 10 m^3 /s and 1000 m^3 /s and they are summarized that the sediment concentration was estimated at 0.7 - 7% at Maskup in the Sacobia river, 0.05 - 0.5% at San Francisco bridge in the Bamban river, and 0.2 - 1.7% at Friendship bridge in the Abacan river, respectively.

3.4 ANNUAL SEDIMENT LOAD AT SELECTED LOCATIONS

Annual sediment load was calculated at the same locations as those selected above. Since no runoff record was available in the study area, runoff discharge was generated from daily rainfall record in the following manner.

(1) Runoff

A long term daily rainfall record is not available in the study area. A short term daily rainfall record is available at Clerk Air Base for three years, although annual rainfall data is available for more than 30 years. A long term daily rainfall data is available at Dagupan located 100 km northwest of the study area. Since the average annual rainfall depth of Dagupan (2086 mm) is almost same, only 5 % higher than that of Clerk Air Base (1987 mm) and the monthly rainfall pattern is similar each other as shown in Fig G.1 and in Appendix D "Meteo-Hydrology", the daily rainfall data of Dagupan was used for generating rumoff.

The average year of rainfall depth was selected as 1989 which was the 5th wettest year within the recent 10 years record. The maximum annual rainfall was recorded in 1972 during the observation period. Tables G.2 and G.3 shows daily rainfall record at Dagupan in 1989 and 1972, respectively.

Then, hourly runoff hydrograph was generated on the basis of 1989 rainfall record at Dagupan assuming runoff coefficient of 0.8 for daily rainfall depth larger than 20 mm. The effective rainfall was calculated at 1,567 mm and they were converted to runoff having triangular distribution for 5 hours a day.

On the other hand, rainfall pattern at Clerk Air Base might be more reliable for the study area. The 1993 rainfall record gives approximately same annual effective rainfall of 1560 mm as Dagupan assuming 0.85 of runoff coefficient for daily rainfall larger than 20 mm. Annual sediment was also estimated for the Clerk Air Base rainfall in 1993.

(2) Annual Sediment Transport Capacity

Table G.4 shows the annual sediment transport capacity at the selected locations for the average rainfall year. Sediment deposit volume was estimated assuming 40 % of porosity. The results show that the annual sediment deposits delivered from Maskup is 3.3 million m^3 which is well coincide with the 3-4 million m^3 sediment delivery from the Sacobia valley in normal flow condition in 1994 which was estimated by the study

team based on the field survey (refer to Appendix F "Sediment Balance and Monitoring").

G.4 Long Term Forecast of river bed movement

4.1 INTRODUCTION

A long term sediment movement in river channels was calculated in this section for evaluating changes in river bed elevation as time passes which would affect flood flowing capacity of the river channels. The annual changes were evaluated for 10 years based on topographic data obtained in Mar. 1994 as initial conditions.

The study was applied to three river reaches, Bamban/Parua river channels from Bamban to the confluence of Rio Chico, Sacobia valley from Mactan gate to Maskup gorge and the Abacan river channels from Friendship bridge to the confluence of San Fernando river at Mexico.

4.2 METHODOLÓGY

(1) Models

One-dimensional river bed movement model was applied in this study. Sediment load rate was given by Brown's equation while flow conditions such as depth and velocity were calculated by the momentum equation of nonuniform flow as shown below,

$\partial z/\partial x + \partial h/\partial x + 1/2g \cdot \partial (v)^2/\partial x = If$

where x : vertical distance

- z : river bed elevation
- h : flow depth
- If : friction loss given by Manning's equation $=n^2v^2/h^{2/3}$

The procedure of calculation is given as follows for each cross section of the river channel.

- 1) Set initial cross section of channel
- 2) Set runoff and boundary condition
- 3) Calculation of water level given by nonuniform flow equation
- 4) Calculation of sediment load rate given by Brown's equation
- 5) River bed changes due to sediment balance
- 6) Next time step and back to 2)

(2) River cross section

Cross sectional data of the channel were prepared on the basis of the results of the cross sectional survey of 600 m interval carried out by the Survey Team in Mar. 1994 for Bamban/Parua river and Abacan river while they were produced for every 400 m from DTM (Digital Topographic Map of 1:10,000 scale) for Sacobia valley. Typical cross sections are shown in Figures G.2, G3 and G4.

(3) Runoff and Parameters

The same runoff data and parameters as the sediment transport capacity study in the previous chapter were applied in this study. Porosity of sediment deposits was assumed to be 40 %.

4.3 LONG TERM RIVER BED CHANGES IN SACOBIA RIVER CHANNEL

(1) Model and Boundary Conditions

In the Sacobia model, the upstream end of the Sacobia river was set at Mactan gate located northwest of the Clerk Air Base. Although the drainage basin of Mactan gate site is 18 km^2 and the remaining basin of the Sacobia valley between Mactan and Maskup is 20 km^2 , runoff from the drainage area of 38 km^2 was input into the entrance of the river channel at Mactan for simplicity of calculation.

Erosible depth of each section was set at the original river bed elevation of pre-cruption and sediment supply from the upstream entrance was given by sediment transport capacity at the section.

Since the model does not consider any lateral erosion rate, downcutting of the river channel would be overestimated than the actual condition if channel width is given as 100-150 m of normal channel condition. The cross section of the Sacobia valley forms double channels of a narrow low channel of 100-150 m wide and an inner terrace of some 400-600 m wide and 5-10 m deep. Then the cross section was assumed to be rectangular with the width of the terrace.

The downstream end was fixed at the present elevation of Maskup Gorge assuming consolidation works will be provided at the site.

(2) River Bed Movement for 10 years

Tables G.5 and G.6 show the changes in river bed elevation and sediment movement of the Sacobia river for 10 years under the 1989 rainfall condition which is considered to be average climatic condition in the study area. Fig G.5 shows the river bed profiles of 4 cases, 1994, one year, 5 year and 10 year afterward. This calculation does not consider the lahars from the basins upstream of Mactan. The simulation shows that river bed erosion will first occur in the highest section and propagate downstream. Then the sections in the upper half reaches will be eroded to the original river bed in 10 years. The annual sediment volume to be delivered from Maskup, the lowest outlet of the valley, is almost constant for 10 years since the slope of the 1994 condition is almost uniform along the valley and the lower portion will not be affected by the changes of the upstream sections in 10 years.

Since the model did not consider tributary flows and lateral erosions, it seems to be too simple to represent the actual conditions, then the results can be considered only as reference. In the study of sediment transport capacity, it was estimated that the annual rate was about 2.1 million m^3 at Maskup site, the actual sediment transport rate at the site would be between 1.2 and 2.1 million m^3 . A two-dimensional study in the valley was also carried out in this study and the results are described in Section 5.9.

4.4 LONG TERM RIVER BED CHANGES IN BAMBAN RIVER CHANNEL

(1) Model and Boundary Conditions

The Bamban/Parua river collects water from two river systems of the Marimla river (drainage area of 65 km^2) and the Sapang Cauayang river (21 km^2). In the model, the upstream end of the Bamban/Parua river was set at 3 km upstream of the previous Bamban bridge site and it would receive runoff from the drainage area of 86 km^2 while flow from the remaining basin of 12 km^2 along the river channel was not considered in the model.

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For calculation of river bed movement, the initial elevation of each cross section was set at the Mar. 1994 condition and the river bed was allowed to be eroded or aggradated by river flow. Since the upstream end of the calculation was set at No.40, about 3 km upstream of the Bamban bridge site where the river bed was El.85.0 m and not affected by the eruption, any erosion was not allowed at the upstream end. In consideration of normal vegetation coverage of the Marimla and Sapang Cauayang river basins, 1 mm/year of an average erosion rate or 86,000 m³/year in volume was considered as an allowable sediment production rate from the upstream basins. The water level at the downstream end was set at El.11.0 m which was average water table of the Rio Chico river in the rainy season.

(2) River Bed Movement for 10 years

Tables G.7 and G.8 show the changes in river bed elevation and sediment movement of the Bamban/Parua river for 10 years under the 1989 rainfall condition. In the table, Section No.181 is the downstream end, No.40 is the upstream end and the San Francisco bridge is located approximately at No. 100.

Fig G.6 shows the longitudinal profile of the river bed in 1994, in one year, 5 years and 10 years afterward. The down cutting of the river bed is obvious by 5 to 10 meters in upstream 5-6 km stretches and is lowered to the original river bed in pre-eruption condition within 8-10 years. On the contrary, river bed would be elevated by some 2.5 meters in 10 years from 1994 near the San Francisco bridge. The speed of aggradation would be high in a few years and gradually slowing down afterward. The downstream reaches would be little affected by sediment transport and the effect would appear in longer time.

According to this simulation, it might be said that the river bed movement of the Bamban/Parua river would be serious within the next 10 years and then gradually stabilized to the original river bed elevation in pre-eruption condition. The maximum aggradation in long term period would be some 3 meters in middle reaches and 1 meter in the lower reaches.

Under the most critical condition of climate, however, 1 meter of aggradation within 1 year would be possible in middle and lower reaches according to the simulation based on 1972 rainfall condition which was the largest rainfall of 4670 mm during 30 years of record period at Dagupan. Table G.9 shows the river bed profile under 1972 condition. According to the observation in 1995, the riverbed movement at the Bamban bridge site was estimated at about 1 meter during 1994-1995 period, while the freeboard of the San Francisco bridge was 0.67 m on an average at the middle of October 1995 although the river channel was excavated about 500 m long, 100 m wide and 2 m deep before onset of the 1995 rainy season and it was obvious that the reaches near the San Francisco bridge was in a stage of river bed aggradation. The results of the simulation give generally good estimates.

4.5 EFFECT OF SACOBIA RIVER DIVERSION

(1) Model and Boundary Conditions

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The Sacobia river is now flowing down on the flood plain developing in the right bank of the Bamban river below Maskup. In the master plan, the Sacobia river is planned to be diverted to the Bamban river by constructing a diversion channel from Maskup to Malonzo about 5 km upstream of the San Francisco bridge. The riverbed fluctuation of the diversion channel and the effect of the diversion to the Bamban river were evaluated in this section. It was assumed that the diversion works were completed in 1999 following to the construction schedule given in the feasibility study. A consolidation dam is planned to be placed at the entrance of the diversion channel to stabilize the riverbed elevation at Maskup where is the outlet of the Sacobia valley and the river forms a narrow gorge. The crest elevation of the consolidation dam would be set at El.111.0 m. A diversion channel would be excavated from Maskup to Malonzo. The diversion channel would be a 5.5 km long artificial straight channel with a 1/180 of constant slope having a 150 m wide and 2.5 to 6 m deep single trapezoid section. The river bed of the channel is protected by a series of groundsill with a 500 m interval in the upper half reaches.

In the Sediment Balance Study, the sediment supply from the Sacobia valley was assumed to be 1.8 million m^3 per year in 1999 and afterwards (0.4 million m^3 from upper Sacobia and 1.4 million m^3 from the spindle valley between Mactan and Maskup). It is very difficult to estimate how long the erosion from the spindle valley would last. In this study two alternative cases were examined; 1) the sediment supply of 1.8 million m^3 is constant for 10 years, and 2) the sediment supply is gradually decreased from 1.8 million m^3 to 0.4 million m^3 for 10 years from 1999.

On the other hand, two alternative conditions of river training works were also examined for the Bamban river channel; a) with no dredging work in the Bamban river, and b) with dredging works of 0.5 m deep in a 6 km long stretch downstream of the San Francisco bridge every year. In both cases, it is assumed that the river training works of the Bamban river channel is completed in 1999 and the riverbed profile is the same as that in 1994.

(2) River Bed Movement for 10 years

(Case 1-a) Constant sediment supply and no dredging

In the case that no dredging works is carried out in the Bamban river after the Sacobia river is diverted to the Bamban river, the river bed of the Bamban river would be locally aggradated by another 1 meter at maximum in the lower reaches compared with the case of no diversion work but the effect is not generally significant. Fig G.7 shows the river bed profile of the Bamban river with the Sacobia diversion and without dredging in the Bamban river channel. Fig G.8 shows the river bed profile of the same case. Some 2 m high sediment deposit would be expected in the upper half reaches of the diversion channel if sediment supply would be as high as 180 million m^3 throughout the 10 year period. Tables G.10 and G.11 show riverbed profile and sediment transport of the Bamban river channel while Tables G.12 and G.13 show those of the diversion channel.

(Case 1-b) Constant sediment supply and with dredging

In the case that dredging works would be carried out in the Bamban river channel, the aggradation of the river bed in the Bamban river would be maintained to be some 1.5 meters on an average in the middle and lower reaches of the Bamban river as shown in Fig.G.9. Since the river bed at the confluence of the diversion channel and the Bamban river is affected little by the dredging works, the river bed profile of the diversion channel is almost the same as that of (Case 1-a) as shown in Fig.G.10. Tables G.14 and G.15 show riverbed profile and sediment transport of the Bamban river channel while Tables G.16 and G.17 show those of the diversion channel.

(Case 2-a) Declining sediment supply and no dredging

If the sediment supply is declining year by year from 1.8 to 0.4 million m3 from 1999, the river bed aggradation is 2 meters less than Case 1-a in the middle reaches and 1 meter in the lower reaches as shown in Fig. G.11 and Tables G.18 and G.19. Consequently, the lower river bed condition at the confluence of the diversion channel

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and the Bamban river would affect the river bed profile of the diversion channel. Downcutting of the diversion channel is expected in its lower stretch as shown in Fig.G.12 and Table G.20 and G.21. Groundsill should be extended to the lower stretch in 10 years.

(Case 2-b) Declining sediment supply and with dredging

Fig. G.13 and Tables G.22 and G.23 show the riverbed profile of the Bamban river in the case that dredging works would be carried out in the Bamban river channel under the condition of declining sediment supply rate. The aggradation of the river bed in the Bamban river would be almost the same as that in (Case 1-b). Downcutting of the diversion channel would be a little deeper than that of (Case 2-a) as shown in Fig.G.14 and Table G.24 and G.25. Extension of groundsill would be needed in the lower stretch.

4.6 LONG TERM RIVER BED CHANGES IN ABACAN RIVER CHANNEL

(1) Model and Boundary Conditions

In the Abacan model, the upstream end of the model was set at the section just below Friendship bridge and the downstream end was set at the confluence of the San Fernando river. The total length of the river channel was 25 km. The drainage area upstream of Friendship bridge was 33 km^2 and the following three tributaries join along the main channel in the model.

Loca	tion	Distance	Drainage area
Friendship bridge	(Upstream end)	25 km	33 km ²
Capaya bridge	(Tributary-1)	19 km	49 km ²
Culubasa	(Tributary-2)	15 km	62 km ²
San Jose Malino	(Tributary-3)	10 km	77 km²
Confluence	· · · · · · · · · · · · · · · · · · ·	0 km	77 km ²

The downstream end was located in lowlying area where was usually inundated during rainy season and the constant water table of EL.5 m was assumed.

Runoff pattern in the basin is different from the Sacobia-Bamban river because major sub basins are located lower slopes of the alluvial fans and therefore flood runoff duration was assumed 15 hours.

Sediment field in the upstream reaches was estimated at 40,000 m3 per annum (see Appendix F "Sediment Balance" and 1 meter deep of sediment deposit in the channel was assumed as allowable erosion depth.

(2) River Bed Movement for 10 years

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Tables G.26 and G.27 show the changes in river bed elevation and sediment movement of the Abacan river for 10 years under the 1989 rainfall condition. Fig G.15 shows the river bed profiles of 4 cases, 1994, one year, 5 year and 10 year afterward.

Because of a concave sharp of river bed profile of the Abacan river, sediment transport capacity of the channel gradually decreases from upstream to downstream and sediment deposit is expected in the lower 5 km reaches. Desilting in the lower reaches would be needed.

G.5 Two-dimensional Flood Inundation Analysis

5.1 INTRODUCTION

The purpose of two-dimensional analysis was to assess the inundation area, flow depth and duration, and sediment deposits depth in flood prone areas due to the overflow of streams due to large scale flood runoff for flood damage analysis as well as to assess the flood flowing capacity and topographic changes of sand pocket with the condition of control structures

The overflow of stream under the 1991 and 1994 topographic conditions in the Bamban river basin were firstly simulated to evaluate applicability of the model. Then flood flow and sediment movement in the sand pocket area were simulated under 1994 topographic condition and the effect of the structures proposed in the sand pocket was assessed. The potential hazard areas were also calculated based on probable flood hydrographs for Bamban/Parua river and Abacan river.

For the numerical simulation, the study applied the computer program of twodimensional flood and mudflow analysis developed by Public Works Research Institute and Sabo and Landslide Technical Center of Japan.

5.2 METHODOLOGY

(1) Models

Basic equations of an unsteady and two-dimensional flow model are expressed as follows:

1) Momentum Equations of Water Flow

x direction:

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\partial M/\partial t + \beta x \cdot \partial (M \cdot u)/\partial x + \beta y \cdot \partial (M \cdot v)/\partial y = -g \cdot h \cdot \partial H/\partial x - t x/\rho
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y direction:

 $\partial N/\partial t + \beta x \cdot \partial (N \cdot u)/\partial x + \beta y \cdot \partial (N \cdot v)/\partial y = -g \cdot h \cdot \partial H/\partial y - \tau y/\rho$

where:	M,N u,v		flow flux of x, y direction velocity of x, y direction
	βx,βy	: -	coefficient of x, y direction
	βx,βy h		flow depth
	H	:	elevation of water table
на стана 1971 -	τχ,τγ	:	friction force, $\tau x/\rho = g \cdot n^2 \cdot u \cdot (u^2 + v^2)^{1/2}/h^{1/3}$
1.1		•	$\tau y/\rho = g \cdot n^2 \cdot v \cdot (u^2 + v^2)^{1/2}/h^{1/3}$

2) Water Flow Continuity

 $\partial h/\partial t + \beta x \cdot \partial M/\partial x + N/\partial y = -0$

3) Sediment Flow Continuity

 $\frac{\partial z}{\partial l+1}(1-\lambda) \cdot \frac{\partial (C_M \cdot M)}{\partial x+1}(1-\lambda) \cdot \frac{\partial (C_N \cdot N)}{\partial y=0}$

where: C_M, C_N : sediment concentration of x, y direction

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4) Sediment load rate is given by Brown's equation

The objective study area is divided into meshes of appropriate size depending on the purpose of the study and allowable number of meshes determined by computation capacity. For each time step, flow rate, flow depth, sediment load, changes in elevation are calculated by the above equations for every meshes of the model and the process is repeated in the next time step.

(2) Topographic data

Topographic data were produced from DTM (Digital Topographic Map of 1:10,000 scale) in 1991 pre-cruption and 1994 prepared by the Survey Team for Sacobia valley, Bamban river basin and Abacan river basin.

(3) Parameters

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The same parameters as the sediment transport capacity study in the previous chapter were applied in this study.

5.3 FLOOD UNDER 1991 TOPOGRAPHIC CONDITION IN BAMBAN RIVER

The flood flow and sediment transport in 1991 was simulated under the following condition.

 Topographic data Hydrograph	DTM in 1991 Area of analysis Mesh size Peak discharge		14.5 km x 6 km 50 m x 50 m 1,000 m ³ /s (including
sediment)	Duration Sediment concentration		5 hours 50 % by volume

The peak discharge of 1,000 m^3 /s with 50% of sediment concentration was considered as a typical lahar occurred in 1994, which was derived from effective rainfall of 100 mm at Mactan.

Fig. G.16 shows the distribution of the maximum water depth on the topographic map and Fig. G.17 shows the maximum water depth and sediment deposits of every meshes over the simulation area. The results well represented the situation that the original river channel was filled by sediment but water did not overtop the channel near the Bamban bridge while major flow overtopped at the section downstream of San Francisco Bridge toward left bank area.

5.4 FLOOD UNDER 1994 TOPOGRAPHIC CONDITION IN BAMBAN RIVER

The flow and sediment transport in 1994 was simulated under the following condition.

Topographic data	DTM in 1994Area of analysisMesh size		14.5 km x 6 km 50 m x 50 m
Hydrograph	Peak dischargeDurationSediment concentration	==	500 m ³ /s (including sediment) 5 hours 10 % by volume

Fig. G.18 shows the distribution of the maximum water depth on the topographic map and Fig. G.19 shows the maximum water depth and sediment deposits of every meshes over the simulation area in 1994. The results can be compared with the chronological changes of Sacobia-Bamban river course in 1994 shown in Fig. G.20 which was reproduced from Appendix G. The stream lines shown in the simulation results well represent the actual water course observed in 1994 although quantitative analysis has not been well carried out.

5.5 POTENTIAL INUNDATION STUDY IN BAMBAN RIVER BASIN

Potential flood inundation over flood prone areas in the Bamban/Parua river basin was simulated under the following condition for 6 cases of probable flood hydroghaphs in different return periods. The flood runoff was input at two locations, Maskup point in the Sacobia river and a confluence of the Marimla and Sapang Cauayang rivers upstream of Bamban bridge in the Bamban river assuming flood events of the same return period occur simultaneously in these basins.

Topographic data	:	DTM in 1994	
101		Area of analysis	20 km x 8 km
		Mesh size	100 m x 100 m

Hydrographs at Maskup and Marimla+Sapang Cauayang (see Fig. G21)

	Peak discharge (m ³ /s)			
	Maskup	Marimla+Cauayang		
2-year flood	125	270		
5-year flood	170	360		
10-year flood	230	490		
20-year flood	270	580		
50-year flood	330	690		
100-year flood	370	800		

Sediment concentration is given by Brown's equation for the initial discharge

The distribution of maximum water depth of every meshes is illustrated on topographic maps produced by GIS as shown in Figure G. 23 for 100 year floods. Figures G.24 to G.27 show the distribution of maximum water depth and sediment deposits of every meshes for 5, 20, 50 and 100-year floods.

The potential inundation areas estimated in this study are summarized as follows:

Water depth	2-year	5-year	10-year	20-year	50-year	100-year
0 < h < 0.2	8,360	8,150	7,860	7,500	7,580	7,590
0.2 < h <1.0	1,390	1,930	2,530	3,000	3,130	3,160
1.0 < h	70	80	110	150	170	200
Total	9,20	10,160	10,500	10,650	10,80	10,950

Inundation Area in Bamban river basin (unit: ha)

Total area of analysis : 16,000 ha

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5.6 POTENTIAL INUNDATION STUDY IN ABACAN RIVER BASIN

Potential flood inundation over flood prone areas in the Abacan river basin was simulated under the following condition. The flood runoff was input at Abacan bridge point.

Topographic data: DTM in 1994: Area of analysis20 km x 11 km: Mesh size100 m x 100 m

Hydrographs at Capaya Bridge (see Fig. G22)

Return period	Peak discharge (m ³ /s)
2-year flood	180
5-year flood	240
10-year flood	310
20-year flood	370
50-year flood	430
100-year flood	490

Sediment concentration is given by Brown's equation for the initial discharge

The distribution of maximum water depth of every meshes is illustrated on topographic maps produced by GIS as shown in Figure G. 28 for 100 year floods. Figures G.29 to G.32 show the distribution of maximum water depth and sediment deposits of every meshes for 5, 20, 50 and 100-year floods.

The potential inundation areas estimated in this study are summarized as follows:

Inundation Area in Bamban river basin (unit: ha)

Water depth	2-year	5-year	10-year	20-year	50-year	100-year
0 < h < 0.2	6,080	6,170	6,120	6,010	6,00	6,940
0.2 < h <1.0	810	1,170	1,680	2,060	2,350	2,600
1.0 < h	20	20	30	40	50	60
Total	6,910	7,360	7,830	8,110	8,800	9,600

Total area of analysis : 22,000 ha

5.7 EFFECT OF CONTROL STRUCTURES IN SAND POCKET

The effects of proposed control structures in sand pocket were examined under 1994 topographic conditions for the 20-year flood. As the control structures, separation dikes between Sacobia and Bamban river, lateral dikes in lower end of sand pocket, sump and 329 highway elevated by 5 meters and collect canals are considered.

;	Topographic data	:	DTM in 1994		· · · ·
		:	Area of analysis		14.5 km x 6 km
		:	Mesh size		50 m x 50 m
	Hydrograph	:	Peak discharge	=	270 m³/s (Sacobia)
			(20-year flood)	Ħ	580 m ³ /s (Bamban)
		:	Duration	C3	24 hours
		:	Sediment concentration	53	10 % by volume
		•	Scument concentration	22	to 70 by volume

Fig.G.33 shows the distribution of maximum flow depth on topographic map and Fig.G 34 shows the maximum flow depth and sediment deposits of every meshes. In the simulation, the flood flows in sand pocket were well drained into the Sapang Balem river and sediment materials were trapped at the lower lateral dike and sump. Most of sediment deposits were deposited in upper part of the sand pocket. It means that sediment materials will not be transported downstream by a single flood event although it might be gradually transported by normal flows.

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A more detailed examination was carried out to evaluate the effect of the lateral dike system in the lower part of the sand pocket which was proposed by the feasibility study. Fig. G.35 shows the water depth and deposit distribution on a 20 meter mesh map due to the 5 year flood hydrograph. The figure shows that sediment materials are trapped by the lateral dike system.

5.8 LONG TERM TOPOGRAPHIC CHANGES IN SAND POCKET

For the purpose of examining the long term changes of topography of the sand pocket, constant runoff of 500 m³/s was input at Maskup contiguously for 120 hours which is equivalent to 5 years runoff volume from the Sacobia river at Maskup.

Topographic data	:	DTM in 1994		
	:	Area of analysis		14.5 km x 6 km
	:	Mesh size		-50 m x 50 m
Hydrograph	:	Peak discharge	=	500 m ³ /s
	:	Duration	==	120 hours

Figures G.36 and G.37 show the maximum flow depth and sediment deposits in 1 year, 3 years and 5 years. Because of the siltation of collect canals and sump located lower end of sand pocket, overflow of the canal and river channel occurred in the Sapang Balem river. In actual situation, desilting in the canal and channels should be carried out. The sediment deposit is obvious in the upper half of the sand pocket as well as north east corner of the sand pocket. According to this long term changes and flow conditions due to the probable floods, major flood flow in the sand pocket would run along the separation dike between the sand pocket and the Bamban river. Reinforcement of the dikes along the possible water way should be considered.

Figs. G.38 and 39 shows the longitudinal profile of the sand pocket along the stream lines from Maskup to outlet structures of the sand pocket in 1, 3 and 5 years. The sediment materials would deposit so as to fill the depressions along the slopes and create smoother slopes. The depth of deposit would be some 1-3 meters in maximum in upper reaches and less than 1 m in the lower reaches.

The sand pocket would effectively work to retain sediment in the sand pocket area with in 5 years.

5.9 TOPOGRAPHIC CHANGES IN SACOBIA VALLEY

The topographic changes of the Sacobia valley between Mactan and Maskup would be caused by lateral as well as vertical erosions of the water courses. For the purpose the purpose of evaluating the effects of lateral erosions, a simulation study was carried out by applying a two dimensional model in the valley.

Topographic data		DTM in 1994		
	•:	Area of analysis	•	10 km x 2.8 km
	: • `	Mesh size		50 m x 50 m
Hydrograph	•	Peak discharge	_	1000 m ³ /s
· · · ·		Duration		5 hours
	•	Sediment concentration	=	10 % by volume

In the simulation, a flood hydrograph having 1,000 m³/s of peak discharge was input at Mactan gate site. Fig. G.40 shows the maximum water depth on the topographic map and Fig.G.41 shows the maximum flow depth, deposit and erosion of every meshes. The flow water overtopped from the low water channel of 100-200 m wide and create a wider water courses and sub channels outside of the lower channels. Then erosion and

deposit occurred in the wider water courses. In this single event of flood, large amount of sediment would first deposit in upper reaches of the channels and then eroded and transported downstream.

G.6 Pasig river Inundation Analysis

6.1 INTRODUCTION

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Active lahar events caused drastic topographic changes in the Pasig river basin even in the 1995 rainy season. This study simulated the process of lahar events and the resulting topographic changes by applying the two-dimensional simulation model. Although the study only shows general process of the topographic changes and not reproduce exactly the actual situation in the field, it could be useful to evaluate the possible flood/mudflow inundation areas in coming years.

The study area of the simulation study covers 27 km x 10 km area from the outlet of the Timbu river to the alluvial plain near Delta 5 watching point to 4 km downstream of GSO (Gapan San Fernando Olongapo) highway The original river channel of the Pasig river is placed almost in the center of the simulation area.

Topographic data were produced from DTM (Digital Topographic Map of 1:10,000 scale) based on the aerophotographs taken in Mar.1994.

The study reproduced the topograpy in the end of 1994 assuming sediment supply by lahar events during the 1994 rainy season. Then the topograpy in the end of 1995 was again reproduced assuming sediment supply by lahar events during the 1995 rainy season on the basis of the reproduced topography at the end of 1994.

The potential flood inundation area was estimated for 20 and 100 year probable flood events on the basis of the reproduced topography at the end of 1995.

6.2 LAHARS UNDER 1994 TOPOGRAPHIC CONDITION IN PASIG RIVER

The flow and sediment transport in 1994 was simulated under the following condition.

Topographic data	: DTM in Mar.1994	
• • F • 9 • • F • • • • •	: Area of analysis	27 km x 10 km
	: Mesh size	100 m x 100 m
Hydrograph	: Combination of th	ree hydrographs
	Peak discharges	1000 m ³ /s, 500 m ³ /s and 100
m ³ /s		
	🔆 Triangular shape o	of 5 hours duration for each hydrograph
	: One year simulation	on equivalent to 5 cycles
	: Sediment concenti	ation = 50 % for 1000 m^3/s ,
$(1, \dots, n) = (1, \dots, n)$		30 % for 500 m ³ /s and
		10 % for 100 m ³ /s
	: Grain size	0.7 mm uniform

The total flow amount in this study was equivalent to 14.4 million m^3 including 70 million m^3 of sediment and 74 million m^3 of water in volume. Fig. G.42 shows the distribution of the maximum water depth and sediment deposits of every meshes over the simulation area in the end of 1994. The results can be compared with the chronological changes of Pasig in 1994 shown in Fig. G.43 which was reproduced from Appendix G.

The flow and sediment transport in 1995 was simulated based on the topographic condition simulated in the previous section under the following condition.

Topographic data	:	DTM produced in the 19	94 sir	nulation study
	:	Area of analysis		km x 10 km
	:	Mesh size	10	0 m x 100 m
Hydrograph	:	Combination of three hydrogeneity	drogra	aphs
		Peak discharges	1000	m ³ /s, 500 m ³ /s and 100
m ³ /s		v		- ·
	:			aration for each hydrograph
	:	One year simulation equ	iivale	nt to 5 cycles
	:	Sediment concentration	==	30 % for 1000 m ³ /s,
	·			10 % for 500 m ³ /s and
				5 % for 100 m ³ /s

On the basis of the field observation in 1995, it was assumed that the sediment concentrations of lahars were lower in 1995 than those in 1994. The total flow amount in this study was equivalent to 14.4 million m^3 including 32 million m^3 of sediment and 112 million m^3 of water in volume. Fig. G.44 shows the distribution of the maximum water depth on the topographic map and Fig. G.45 shows the maximum water depth and sediment deposits of every meshes over the simulation area in 1995. The results can also be compared with the chronological changes of Pasig in 1995. The lahar flows were shifting towards the left hand direction downstream of the Angeles Porac road in 1995 and the situation was well expressed in the simulation and inundation in the lowest reaches was not well simulated in the study.

6.4 POTENTIAL INUNDATION STUDY IN PASIG RIVER BASIN

Potential flood inundation over flood prone areas in the Pasig river basin was simulated under the following condition for 2 cases of probable flood hydroghaphs in different return periods which were given by the study of USACE (Ref.G.4). The flood runoff was input at the outlet of the Timbu river.

Topographic data	:	DTM produced by	y the 1995 simulation study
		Area of analysis	27 km x 10 km
		Mesh size	100 m x 100 m

Return period	Peak discharge (m ³ /s)
20-year flood	430
100-year flood	490

Hydrographs at Delta 5

Sediment concentration is given by Brown's equation for the initial discharge

The distribution of maximum water depth of every meshes is illustrated on topographic maps produced by GIS as shown in Figure G.46 for 20 year flood and Figures G.47 for 100 year flood. Figures G.48 and G.49 show the distribution of maximum water depth and sediment deposits of every meshes for 20 and 100-year floods.

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Water Depth	20-Year Flood	100-Year Flood
0 < h < 0.2	3,596	2,434
0.2 < h < 1.0	2,246	3,922
1.0 < h	224	487
Total	6,066	6,843

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The potential inundation areas estimated in this study are summarized as follows:

Inundation Area in Pasig river basin (unit: ha)

Total area of analysis : 27000 ha

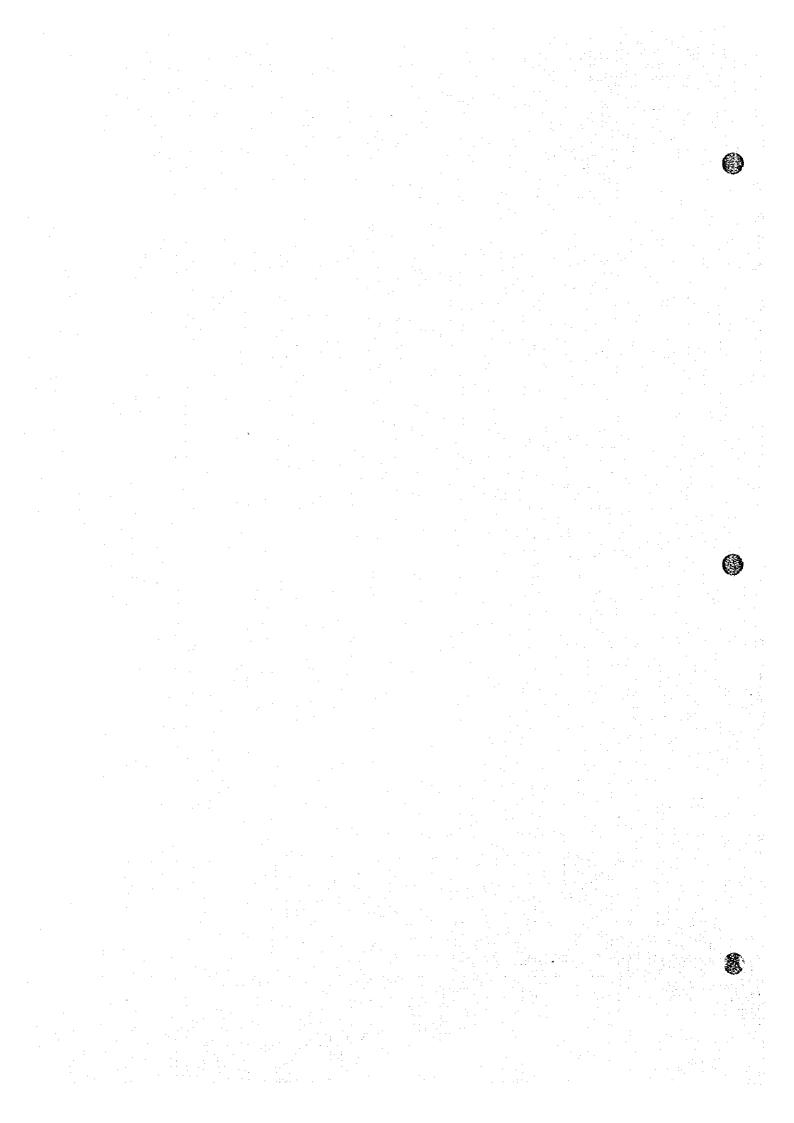
REFERENCE

Ref. No.

Title

- G.1 Swiss Disaster Relief, "Lahars in the O'Donnell river system, Mt. Pinatubo, Philippines, Final Report", January 1994
- G.2 PHIVOLCS-USGS, Progress Report of Joint PHIVOLCS-USGS collaboration, 1992 through mid-1994, Serious but Rapidly Diminishing Hazards at Mount Pinatubo, August, 1994
- G.3 T.C. Pierson, R.J. Janda, J.V. Umbal and A.S. Daag, PHIVOLCS-USGS, Immediate and long-term hazards from lahars and excess sedimentation in rivers draining Mt. Pinatubo, Philippines, 1992
- G.4 US Army Corps of Engineers, Mount Pinatubo Recovery Action Plan Long. Term Report, Eight River Basins Republic of the Philippines, March 1994





Location	Catch- ment area (km2)	Hydraulic gradient	Width (m)	Discharge (m3/s)	Depth (m)	Sediment transport (m3/s)	Concentration
(1) O	(KIIIZ)	·		(11.3.0)			
(1) Sacobia 1) Maskup	38	0.0100	100	10	0.11	0.07	0.69%
т) маякир	50	0.0100	100	50	1		
				100			1
				500			4.88%
			:	1000			
2) Sand Pocket	61	0.0048	100				
- 2) Sand Forker		0.0010		50			0.42%
				100			1
				500			
				1000		1	1.88%
(2) Bamban		·					
(2) Damban 1) Malonzo	93	0.0038	200	10	0.10	0.01	0.09%
1) Million20		0.000.0		50			
				100			
			1.	500			
·				1000		1	
2)San Francisco	94	0.0027	200				0.05%
Bridge				.50	1 ·	0.06	0.119
111080				100		0.16	0.16%
			· · ·	500			0.359
				1000	1.69	4.95	0.499
3)Confluence	98	0.0015	200	10	0.13	0.00	0.029
5,000				50	0.33	0.02	0.04%
				100	0.50	0.06	0.06%
		· ·	÷ .	500) 1.32	2 0.65	0.139
				1000	2.00	1.85	0.189
(3)Abacan							
1)Friendship	33	0.0067	200	10	0.08	0.02	0.179
Bridge				50			
Ding	1			100			
				500			1.199
		1		1000	1	1	
2)San Jose	77	0.0040) 150				
Malino				50	1	•	
				100			
	1	1 · · · · · · · · · · · · · · · · · · ·	1	500			
				1000			

Table G.1 Sediment Transportation Capacity

* Depth

2

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* Sediment

* Diamenter of sediment

: Uniform Flow (Manning's Formula)

; n=0.025

: Brown's Formula

: 0.7 mm in Sacobia-Bamban river

: 1.0 mm in Abacan river

Density=2.60

									Station: Year:			UPAN 1989
	(Unit ; n				r	1			Agency		r	GASA
	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec
1	0	0	2	0	0	<u> </u>	0	2	15	0	0	0
2	0	0	0	3	1	0	0	39	1	9	0	0
3	0	0	0	12	1	0	0	69	3	5	0	0
4	0	0	0	0	25	23	0	19	44	0	0	0
5	0	0	0	0	0	0	0	19	4	0	0	0
6	0	0	0	0	0	2	1	42	1	: 5	0	0
7	0	0	0	0	0	4	0	- 39	0	- 5	0	0
8	0	0	0	l	0	. 0	56	9	28	2	0	0
9	0	0	0	0	0	0	2	1	208	0	0	0
10	0	0	0	1	2	- 8	0	2	62	3	0	0
- 11	0	0	0	0	0	1	3	0	34	5	0	0
12	0	0	0	0	0	1	0	16	14	0	0	0
13	0	0	0	0	0	0	1	29	17	0	0	0
14	0	0	• 2	0	53	- 2	0	2	8	0	0	0
15	0	0	45	0	1	. 0	81	36	2	0	0	0
:16	0	0	0	0	0	0	137	1	19	0	0	1
17	0	0	0	0	28	: 0	14	4	21	0	0	0
18	0	.0	0	. 0	12	- 0	-3	30	· · , 0	. 0	0	0
19	0	0	0	0	0	0	1	49	0	199	0	0
20	14	0	7	0	9	70	6	10	0	6	0	0
21	0	- 0	- 35	0	. 0	13	8	. 3	0	0	2	. 0
22	0	0	. 0	0	0	42	0	11	1	3	52	0
23	0	0	4	0	0	14	0	17	2	. 0	0	0
24	0	0	0	0	8	30	0	0	0	0	0	0
25	0	0	: 0	0	0	0	4	0	0	. 0	0	0
26	0	0	0	0	- 9	0	59	0	2	0	0	0
. 27	0	0	0	0	4	0	3	0	0	0	0	0
28	.0	8	0	0	0	. 0	2	0	35	0	0	0
29	0		0	1	0	31	116	0	19	0	0	0
30	0		0	0	2	4	89	0	1	0	0	0
31	0		1		0		34	0		0		0
								1 E		:		
Total	- 14	8	97	18	156	247	619	449	541	241	54	1
										Annual	Total =	2446
Daily Max	14	8	45	12	53	70	137	69	208	199	52	1
Rainy Day	- 1	1	7	5		15	19		22	10		1

Table G.2 Daily Rainfall at Dagupan in 1989



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										Station: Year:			UPAN 1972
	I	(Unit : n	າຫາ)					<u></u>		Agency:		T	GASA
Γ		Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec
F	^{; ,} 1	0	0	17	0	6	2	5	108	0	10	0	0
	2	0	0	3	0	:]	0	3	107		1	0	0
	3	0	0	0	0	0	24	69	155	15	0	0	0
	4	0	0	0	0	0	0	1	131	- 2	2	0	0
ł	5	0	0	0	0	1	0	23	36	0	0	3	. 2
F	6	0	0	0	0	0	40	70	19	0	1	0	0
ł	7	0	0	0	0	5	64	224	6	0	0	0	1
	8	0	0	0	0	0	28	77	0	0	0	0	0
Ì	9	5	0	0	0	4	31	110	0	0	0	0	0
F	10	0	0	0	0	3	23	86	2	21	22	0	0
ľ	- 11	0	• 0	0	0	0	22	58	0	0	0	0	0
	12	2	0	0	0	0	0	42	4	60	0	0	14
F	- 13	2	0	0	0	0	0	21	37	0	0	0	0
-	14	0	0	0	1	0	. 0	64	18	. 0	0	0	0
F	15	0	0	0	0	0	0	15	202	5	. 0	0	0
. [16	0	0	0	. 0	0	0	31	203	0	10	0	0
	. 17	0	0	· 0	0	0	0	148	87	22	- 0	0	0
Ì	18	0	0	0	0	1	3	171	5	2	0	0	0
3	19	0	- 0	10	0	14	0	157	0	0	0	· 0	0
Ĩ	20	0	0	0	0	6	4	183	1	0	0	0	0
:	21	0	0	0	0	0	- 8	65		30	§	0	. : 0
Ì	22	0	0	0	0	9	26	1	19	3	·	0	0
	23	.0	0	0	0	0	0	18				0	0
ľ	24	0	0	. 0	25	0	0	26				0	. 0
	25	.0	0	0	0	2	7	81	22			0	. 0
	26	0	0	0	0	1	3	77					0
Ì	27	0	0	0	0	16	4			↓		· · · · · ·	0
	28	. 0	1	0	<u> </u>) 1	2	313	+	+ · · · · · · · · · · · · · · · · · · ·			0
	29	0	1	0 1	· () 11	0	136	5	{			0
:.	30	5		0	() 0	7	104		0	. 0	0	
	31	0		0		0	<u> 2888</u>	132	3		<u> 1</u>	<u> 46 9 8</u>	0
:		[·			<u> </u>			
	Tota	14	2	30	20	5 80	296	2659	1275	201		J	1
		·									-	Total =	4657
	Daily Max	: .	5	17	2	5 16	61						14
	Rainy Day	/ 4	1	2 3		2 15	17	31	27	14	9	1	3

Table G.3 Daily Rainfall at Dagupan in 1972

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	Location	Catch- ment area (km2)	Hydraulic gradient	Rainfall	Annual runoff (million m3)	Annual Trans Cap (million m3)	Concen- tration	Annual deposit (million m3
1) Sa	cobia							
	1) Maskup	-38	0.0111	Dagupan	59.9	2.0	3.4%	3.4
			(1/90)	Clerk AB	.59.6	2.1	3.5%	3.5
	2) Sand Pocket	61	0.0048	Dagupan	96.1	0.9	1.0%	1.6
	_,		(1/210)	Clerk AB	95.7	1.0	1.0%	1.6
2) Ba	ເກເຽລກ		· · · · · · · · · · · · · · · · · · ·					
	 Malonzo 	93	0.0038	Dagupan	146.6		0.6%	1.4
			(1/260)	Clerk AB	146.0	0.9	0.6%	1.6
	2) Sanfrancisco	94	0.0027	Dagupan	148.1	0.5	0.3%	0.8
•	Bridge		(1/370)	Clerk AB	147.5	0.5	0.4%	0.9
	3) Confluence	98	0.0015	Dagupan	154.4	0.2	0.1%	0.3
	.,		(1/650)	Clerk AB	153.7	0.2	0.2%	0.4
3)Ab								·
•	1) Friendship	- 33	0.0067	Dagupan	51.2	0.3	0.6%	0.6
	Bridge		(1/150)	Clerk AB	48.0	0.3	0.6%	0.5
· · ·	2) San Jose	77	0.0040	Dagupan	119.6	0.6	0.5%	0.9
	Malino		(1/250)	Clerk AB	112.2	0.5	0.5%	0.9

Table G.4 Annual Sediment Transportation Amount

* Depth

* Sediment

* Diamenter of sediment

*Discharge

: Uniform Flow (Manning's Formula) : n=0.025 0

: Brown's Formula

: 0.7 mm in Sacobia-Bamban river

: 1.0 mm in Abacan river Density=2.60 Average Year =1989

							(1989 га	ainfall co	nditon)		(ELm)		
ſ	Section	distance	El in 94	I year	2 year	3 year	4 year	5 year	6 year	7 year	8 усаг	9 year	10 year
ľ	No52	0	99.00	99.00	99.00	99.00	99.00	99.00	99.00	99.00	99.00	99.00	99.0 0
-	No50	400	103.00	102.64	102.58	102.56	102.55	102.54	102.53	102.52	102.51	102.51	102.50
ſ	No48	800	107.00	107.18	107.07	107.01	106.96	106.93	106.90	106.87	106.85	106.83	106.81
	No16	1200	111.00	111.07	110.98	110,89	110.83	110.78	110.73	110.69	110.66	110.63	110.60
	No14	1600	115.00	114.88	114.82	114.74	114.67	114.60	114.55	114.50	114.45	114.41	114.38
	No42	2000	119.00	118.90	118.83	118.76	118.69	118.62	118.56	118.50	118.45	118.40	118.36
Ī	No40	2400	123.00	122.91	122.82	122.76	122.69	122.62	122.56	122.50	122.45	122.40	122.36
Ĩ	No38	2800	127.00	126.94	126.87	126.80	126.74	126.68	126.63	126.57	126.52	126.48	126.43
	No36	3200	131.00	130.97	130.93	130.87	130.82	130.77	130.72	130.68	130.64	130.60	130.55
	No34	3600	135.00	134.99	134.95	134.91	134.87	134.83	134.79	134.76	134,73	134.70	134.66
ľ	No32	4000	139.00	138.97	138.93	138.90	138.87	138.85	138.83	138.81	138.79	138.76	138.71
Ē	No30	4400	143.00	142.94	142.91	142.89	142.87	142.87	142.87	142.86	142.85	142.81	142.74
	No28	4800	147.00	146.93	146.90	146.89	146.90	146.92	146.94	146.95	146.93	146.87	146.75
Ĩ	No26	5200	151.00	150.98	150.98	151.01	151.06	151.11	151.14	151.15	151.10	150.95	150.73
ľ	No24	5600	155.00	155.03	155.12	155.21	155.31	155.39	155.44	155.39	155.22	154.91	154.51
Ĩ	No22	6000	159.00	159.21	159.41	159.59	159.73	159.83	159.84	159.61	159.17	158.57	157.96
_	No20	6400	163.00	163.46	163.77	164.02	164.24	164.30	164.01	163.38	162.47	161.60	
	No18	6800	168.00	168.26	168.58	168.86	169.04	168.77	167.81	166.72	165.23	164.02	163.00
	No16	7200	174.00	174.00	174.06	174.19	173.91	172.60	170.63	169.15	169.00		169.00
·	No14	7600	180.00	180.08	180.10	179.75	178.04	175.46	175.00	175.00	175.00	175.00	
	No12	8000	186.00	186.11	186.00	183.98	181.00	181.00			·		+
	No10	8400	192.00	192.01	190.38	187.00	187.00	187.00	187.00	187.00	187.00		
ľ	No8	8800	198.00	197.22	193.34	193.00	193.00	193.00	193.00		193.00		
[No6	9200	204.00	200.27	199.00	199.00	199.00	199.00				· · · · · · · · · · · · · · · · · · ·	
÷ [No4	9600	210.00	205.00	205.00	205.00		205.00			205.00		
	No2	10000	215.99	211.00	···-	211.00	t						
	Nol	10200	216.00	216.00	216.00	216.00	216.00	216.00	216.00	216.00	216.00	216.00	216.00

Table G.5 River Bed Profile of Sacobia River under 1989 Rainfall Condition

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	(1989 rainfall conditon) (El.m)											
Section	distance	1 year	2 year	3 year	4 year	5 year	6 year	7 year	8 year	9 усаг	10 year	
No52	0		,	- 1000000, 41-2,749 P								
No50	400	1,206	1,171	1,155	1,146	1,140	1,134	1,129	1,125	1,121	1,118	
No18	800	1,101	1,155	1,149	1,142	1,136	1,131	1,127	1,123	1,120	1,116	
No16	1200	1,143	1,129	1,134	1,132	1,128	1,124	1,121	1,118	1,115	1,113	
No14	1600	1,154	1,115	1,121	1,122	1,120	1,117	1,115	1,113	1,110	1,108	
No42	2000	1,129	1,104	1,105	1,107	1,107	1,106	1,105	1,104	1,102	1,101	
No40	2400	1,106	1,090	1,089	1,091	1,092	1,092	1,092	1,093	1,092	1,092	
No38	2800	1,087	1,070	1,074	1,075	1,077	1,078	1,079	1.081	1,081	1,082	
No36	3200	1,072	1,050	1,057	1,060	1,061	1,064	1,066	1,068	1,070	1.070	
No34	3600	1,065	1,040	1,043	1.046	1,048	1.051	1,054	1,057	1,059	1,059	
No32	4000	1,061	1,029	1,031	1,035	1,037	1,041	1,044	1,048	1,049	1,048	
No30	4400	1,052	1,020	1,023	1,027	1,031	1,036	1,039	1,042	1,041	1,035	
No28	4800	1,037	1,011	1,017	1,024	1,030	1,036	1,039	1,039	1,032	1,017	
No26	5200	1,018	1,002	1,015	1,027	1,035	1,041	1,041	1,034	1,013	983	
No24	5600	1,010	1,004	1,025	1,041	1,050	1,053	1,043	1,016	968	911	
No22	6000	1,021	1,031	1,055	1,071	1,077	1,068	1,029	962	867	784	
No20	6400	1,092	1,102	1,116	1,120	1,112	.		809	659		
No18	6800	1,221	1,188	1,187	1,181	1,128	988			414		
No16	7200	1,310	- 1,300	1,282	1,243	1,038	ŧ	402	41	0		
No14	7600	1,311	1,319	1,317	1,166	682	120					
No12	8000	1,333	1,324	1,226	714	+	<u>↓</u>		· · · · · · · · · · · · · · · · · · ·			
No10	. 8400	1,359	1,297	741	0	0	0			<u> </u>	<u>+</u>	
No8	8800	1,363	970		f					·····		
No6	9200	1,211	208	0	0				· · · · · · · · · · · · · · · · · · ·			
No4	9600	600	0 0									
No2	10000	-		·	+		· • ·····			· · · · · · · · · · · · · · · · · · ·	+ · · · · · · · · · · · · · · · · · · ·	
Nol	10200							0	. 0	0	0	

Table G.6 Sediment Transport of Sacobia River under 1989 Rainfall Condition

Ø

						(1989 r	ainfall c	onditio	n)	(EL.m)		
Section	distance	EL in94	1 year	2 year	3 year	4 year	5 year	6 year	7 year	8 year	9 year	10year
No181	0	7.01	7.01	7.01	7.01	7.01	7.01	7.01	7.01	7.01	7.01	7.01
No178	600	10.41	9.65	9.52	9.49	9.53	9.59	9.64	9.68	9.70	9.71	9.73
No175	1200	11.39	10.53	10.33	10.30	10.37	10.47	10.55	10.60	10.63	10.66	10.68
No172	1800	11.53	11.28	10.97	10.96	11.11	11.28	11.42	11.51	11.58	11.62	11.67
No169	2400	12.18	12.11	11.75	11.83	12.05	12.27	12,44	12.55	12.63	12.69	12.75
No166	3000	13.22	13.00	13.00	13.00	13.00	13.02	13.08	13.13	13.22	13.29	13.36
No163	3600	13.95	14.00	14.19	14.28	14.22	14.19	14.22	14.27	14.36	14.45	14.53
No160	4200	15.04	15.00	15.00	15.00	15.01	15.03	15.07	15.17	15.29	15.41	15.54
No157	4800	16.19	16.01	16.08	16.15	16.24	16.30	16.36	16.47	16.59	16.72	16.86
No154	5400	16.82	16.70	16.89	17.08	17.24	17.36	17.46	17.58	17.71	17.84	17.98
No151	6000	16.32	17.10	17.47	17.82	18.07	18.23	18.36	18.50	18.64	18.78	18.93
No148	6600	17.69	18.05	18.56	19.02	19.29	19.48	19.64	19.80	<u> 19.95</u>	20.11	20.27
No145	7200	18.57	18.91	19.71	20.19	20.50	20.72	20,92	21.10	21.27	21.44	21.61
No142	7800	19.55	19.90	20.84	21.31	21.64	21.89	22.12	22.31	22.50	22.68	22.86
No139	8400	20.50	20.57	22.08	22.47	22.80	23.06	23,31	23.52	23.73	23.92	24.12
No136	9000	23.09	23.00		23.73	24.04	24.33	24.59	24.84	25.06	25.27	25.47
No133	9600	24.08	24.92	24.80	25.08	25.36	25.66	25.94	26.20	26.44	26.66	26.86
No130	10200	24.70	26.33	26.29	26.56	26.80	27.11	27.39	27.66	27.90	28.12	28.32
No127	10800	27.30	27.67	27.74	27.97	28.20	28.50	28,78	29.05	29.29	29.52	29.72
No124	11400	28.20	28.99	29.25	29.43	29.67	30.00	30.28	30.55	30.80	31.02	31.23
No121	12000	29.70	29.95	30.47	30.93	31.40	31.67	31.94	32.21	32.46	32.68	32.89
No118	12600	31.00	31.76	32.29	32.67	32.94	33.16	33.43	33.67	33.90	34.12	34.32
No115	13200	33.00	33.40	33.84	34.15	34.50	34.75	35.01	35.24	35.46	35.67	35.86
No112	13800	34.80	35.07	35.47	35.78	36.16	36.45	36.70	36.92	37.14	37.35	37.55
No109	14400	36.40	36.70	37.06	37.32	37.68	38.01	38.28	38.49	38.70	38.89	39.08
No106	15000	37.60	38.34	38.71	38.90	39.36	39.68	39.94	40.14	40.32	40.51	40.68
No103	15600	39.50	39.42	40.03	40.73	41.14	41.41	41.63	41.82	42.00	42.17	42.32
No100	16200	39.70	41.51	42.20	42.74	42.88	43.13	43.33	43.52	43.68	43.85	44.00
No97	16800	43.00	43.93	44.17	44.54	44.76	44.98	45.16	45.33	45.48	45.63	45.76
No94	17400	45.20	46.12	46.05	46.37	46.62	46.81	46.96	47.10	47.23	47.37	47.49
No91	18000	48.30	48.06	48.16	48.44	48.72	48.86	48.97	49.08	49.19		49.42
No88	18600	50.00	50.02	50.14	50.42	50.71	50.85	50.97	51.08			51.30
No85	19200	52.50	52.38	52.45	52.67	52.90	53.03	53.14	53.21	53.26	53.29	53.37
No82	19800	55.00	55.53			55.82	55.94	56.01			1	r1
No79	20400	57.00	57.55	57.33	57.76						****	· ····
No76	21000	60.00	60.07	60.36			1	<u> </u>		1	1	59.41
No73	21600	63.00					1			· · · · · · · · · · · · · · · · · · ·		62.26
No70	22200	66.50	67.31	r			1					63.46
No67	22800	70.80	70.62	70.94	70.95			1	1		1	
No64	23400	74.30	T	1								
No61	24000	77.80		+							1	
No58	24600	80.30		1								
No55	25200	82.70		1	1		7			1		
No52	25800	84.20		1			1			1	1	1
No49	26400	85.20	84.62									
No46	27000		1				· · · · · · · · · · · · · · · · · · ·					
No43	27600				81.01							
No40	28200	85.00	85.00	85.00	85.00	85.00	85.00	85.00	85.00	\$5.00	85.00	85.00

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the set of the second second second second					(1989 ra	vinfall co	ndition)		(1000 n	13)	
Section	distance	l year	2 year	3 year	4 year	5 year	6 year	7 year	8 year	9 year	10year
No181	0	16	16	16	16	16	16	16	16	16	16
No178	600	618	388	329	338	379	421	452	473	490	503
No175	1200	469	363	323	347	390	431	458	478	493	507
No172	1800	313	329	316	360	408	445	467	484	498	511
No169	2400	275	279	315	383	434	467	482	494	505	518
No166	3000	262	220	327	420	471	495	502	507	516	527
No163	3600	235	220	327	421	479	510	514	- 517	525	536
No160	4200	236	249	341	412	474	514	522	530	538	548
No157	4800	233	249	346	417	479	521	534	545	554	564
No154	5400	202	261	358	432	489	531	552	566	577	587
No151	6000	180	296	391	462	512	548	574	590	601	612
No148	6600	290	348	440	498	537	568	595	613	625	635
No145	7200	354	438	521	548	574	599	626	644	656	667
No142	7800	408	567	617	610	621	640	662	680	693	703
No139	8400	463	722	710	675	672	686	702	719	731	741
No136	9000	471	890	782	735	723	733	- 745	759	770	779
No133	9600	456	969	841	793	778	783	792	803	812	819
No130	10200	615	946	895	846	835	836	843	849	856	861
No127	10800	869	938	946	892	899	894	899	. 900	903	905
No124	11400	935	950	988	932	956	949	952	948	947	946
No121	12000	1,082	997	1,021	975	1,017	1,003	1,007	998	993	989
Nol18	12600	1,109	1,052	1,071	1,026	1,072	1,060	1,066	1,052	1,043	1,036
No115	13200	1,242	1,144	1,138	1,073	1,111	1,110	-1,111	1,096	1,084	1,075
No112	13800	1,312	1,219	1,191	1,134	1,155	1,156	1,154	1,138	1,124	1,113
No109	14400	1,362	1,293	1,247	1,203	1,208	1,203	1,195	1,180	1,164	1,151
No106	15000	1,412	1,353	1,289	1,262	1,262	1,247	1,230	1,215	1,197	1,182
No103	15600	1,529	1,410	1,319	1,334	1,315	1,291	1,265	1,248	1,229	1,212
No100	16200	1,523	1,461	1,378	1,381	1,360	1,330	1,297	1,278	1,259	1,239
No97	16800	1,711	1,563	1,463	1,404	1,399	1,364	1,328	1,306	1,286	1,265
No94	17400	1,779	1,598	1,527	1,442	1,438	1,397	1,358	1,333	1,314	1,291
No91	18000	1,930	1,587	1,582	1,486	1,471	1,423	1,383	1,356	1,338	1,313
No88	18600	1,883	1,606	1,639	1,540	1,500	1,448	1,407	1,380		1,337
No85	19200	1,885	1,628	1,689	1,592	1,524	1,471	1,427	1,394	1,378	1,355
No82	19800	1,856	1,645	1,742	1,647	1,556	1,496	1,445	1,406		1,375
No79	20400	1,961	1,645	1,844	1,706	1,620	1,538	1,453	1,444	1,389	1,414
No76	21000	2,043	1,613	1,909	1,653	1,630	1,541	1,576	1,461	1,387	1,310
No73	21600	2,054	1,656	1,831	1,705	1,641	1,576	1,647	1,508	1,366	1,052
No70	22200	2,369	1,747	1,761	1,710	1,634	1,651	1,602	1,396	1,111	796
No67	22800	2,525	1,876	1,868	1,602	1,607	1,675	1,359	1,271	872	538
No64	23400	2,477	1,960	1,871	1,619	1,557	1,600	998	957	470	49
No61	24000	2,274	1,906	1,924	1,549	1,480	1,535	796	608	75	50
No58	24600	2,002	1,824	1,908	1,494	1,399	1,429	608	393	75	50
No55	25200	1,594	1,513	1,734	1,335	1,124	862	44	78	79	76
No52	25800	1,310	1,257	1,532	1,026	702	35	79	79	79	76
No49	26400	1,121	1,029	1,263	413	28	77	79	79	79	76
No46	27000	1,007	825	376	27	77	79	.79	79	79	76
No43	27600	936	16	25	57	77	79	79	79	79	76
No40	28200	19	19	56	79	77	79	79	79	79	76

Tabel G.8 Sediment Transport of Bamban/River under 1989 Rainfall Condition

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Table G.9	River Bed Profile of	Bamban/ River unde	er 1972 Rainfall Condition
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						(1972 r	ainfall c	condition	1)	(EL.m)		
Section	distance	EL in94	1 year	2 year	3 year	4 year	5 year	6 year	7 year	8 year	9 year	10year
No181	0	7.01	7.01	7.01	7.01	7.01	7.01	7.01	7.01	7.01	7.01	7.01
No178	600	10.41	9.56	9.65	9.81	9.88	9.91	9.94	9.98	10.01	10.03	10.05
No175	1200	11.39	10.20	10.33	10.53	10.62	10.68	10.73	10.79	10.85	10.89	10.92
No172	1200	11.53	10.82	11.12	11.40	11.53	11.62	11.69	11.78	11.86	11.93	11.97
No169	2400	12.18	11.59	12.10		12.56		12.78	12.91	13.03	13.13	13.18
No166	3000		13.00			13.38		13.67	13.84	14.00	14.11	14.18
No163	3600	13.95	14.38	14.20	****	14.45			14.99	15.18	15.31	15.39
No160	4200		15.04	15.14	15.21	15.44		15.98	16.25	16.46	16.61	16.68
No157	4800	16.19	16.04	16.29	16.43	16.70		17.33	17.57	17.77	17.92	17.99
No154	5400		16.95	17.37	17.60	17.91	18.24	18.56	18.81	19.02	19.17	19.23
No151	6000		17.70			18.96	19.30		19.91	20.13	20.27	20.32
No148	6600		18.75	19.46		20.22	20.59	20.94	21.24	21.47	21.60	21.62
No145	7200		19.91	20.66		21.53	21.93		22.62	22.85	22.97	22.96
	7200		21.07	21.86		22.83			23.96	24.20	24.28	24.23
No142			22.26	·		24.10				25.49	25.54	25.43
No139	<u>8400</u> 9000		23.55	23.04		25.44			26.64		26.81	26.64
No136	9600		23.33	25.60		26.83	27.30		28.04			27.85
No133				27.04		28.29	*		29.47	29.55		29.04
No130	10200		26.38	28.47		29.72	30.20		30.86	fra +	30.58	****
No127	10800		27.84			31.23	31.69		32.30			31.21
<u>No124</u>	11400			31.59		32.84	33.31	33.68	33.83			32.30
<u>No121</u>	12000					34.34		35.14	35.19	*	33.99	33.13
<u>No118</u>	12600					35.90			36.57	<u> </u>		33.86
No115	13200	*					}	1	37.98		35.71	34.57
No112	13800			36.40					39.25	+		35.02
No109	14400				1				40.50			35.31
No106	15000			+	40.20	40.72			41.66		f	37.00
<u>No103</u>	15600					42.30		1	42.71	40.00	f	40.00
No100	16200	1					46.00	+	43.44			42.00
No97	16800					47.58	t		44.43			
No94	17400								46.00			
No91	18000			+			1				• · · · · · · · · · · · · · · · · · · ·	
No88	18600	a second a second								50.00		
<u>No85</u>	19200		T							52.00		
No82	19800									1		
No79	20400		1	1			1					
No76	21000					*				1	1	
No73	21600	· · · · · · · · · · · · · · · · · · ·										
No70	22200	the second se			+	1					T	
No67	22800						1		+	1		
No64	23400		1									
No61	24000											
No58	24600						+				1	1
No55	25200						1					
No52	25800					1						
No49	26400											
<u>No46</u>	27000											
No43	27600						85.00			85.00		
No40	28200	85.00	85.00	85.00	1 03.00	1 03.00	1 0.1.0	1 03.00	1.0.3.00	1 0.1.00	1.05.00	1.02.00

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						(1989 г	ainfal <mark>l</mark> c	onditio	n)	(EL.m)		
Section	distance	EL in99	1 year	2 year	3 year	4 year	5 year	6 year	7 year	8 year	9 year	10year
No181	0	7.01	7.01	7.01	7.01	7.01	7.01	7.01	7.01	7.01	7.01	7.01
No178	600	10.41	9.71	9.66	9.64	9.64	9.65	9.66	9.68	9.69	9.71	9.72
No175	1200	11.39	10.64	10.57	10.54	10.54	10.55	10.58	10.60	10.62	10.65	10.67
No172	1800	11.53	11.56	11.44	11.40	11.41	11.43	11.47	11.51	11.56	11.60	11.64
No169	2400	12.18	12.55	12.43	12.39	12.40	12.44	12.49	12.55	12.60	12.66	12.72
No166	3000	13.22	12.98	12.88	12.87	12.90	12.95	13.02	13.10	13.18	13.25	13.33
No163	3600		14.01	13.93	13.93	13.98	14.05	14.14	14.22	14.31	14.40	14.49
No160	4200	15.04	14.61	14.58	14.63	14.72	14.83	14.96	15.08	15.21	15.35	15.48
No157	4800	16.19	15,74	15.77	15.86	15.97	16.10	16.24	16.37	16.51	16.65	16.79
No154	5400	16.82	16.63	16.74	16.88	17.02	17.18		17,48	17,63	17.77	17.92
No151	6000	16.32	17.20		17.67	17.88	18.07	18.23	18.39	18.55	18.71	18.87
No148	6600	17.69	18.29	18.61	18.92	19.16	19.35	19.52	19.69	19.86	20.03	20.20
No145	7200	18.57	19.38	19.81	20.15	20.40	20.60	20.80	20.99	21.18	21.36	21.54
No142	7800		20.49	20.93	21.29	21.56	21.79	22.01	22.21	22.41	22.60	22.79
No139	8400		21.55	22.13	22.45	22.74	22.98	23.21	23.43	23.65	23.85	24.05
No136	9000		22.98	23.40		23.99	24.25		24.75	24.98	25.20	25.42
No133	9600		24.31	24.74	25.04	25.32	25.59		26.12	26.37	26.60	26.82
No130	10200		25.74	26.17	26.51	26.76	27.04	27.32	27.59	27.84	28.07	28.29
No127	10800		27.17	27.57	27.90		28.43		28.99	29.24	29.48	29.70
No124	11400		28.69	29.06			29.93	t	30.51	30.76	31.00	31.22
No121	12000		29.74		t	31.34	31.61	t			h	32.89
No118	12600		31.61	32.04	32.50		33.12		33.66			34.35
No115	13200		33.35	1		34,39	34.71	35.00	35.24	35.47	35.70	35.90
No112	13800	1			f	36.06			36.94	1	1	
No109	14400							1	-38.53			
No106	15000			F			}	1	40.20		40.60	40.77
No103	15600			t			41.43	1		42.11	42.28	
No100	16200				1			1	43.65			1
No97	16800			44.26	1			<u> </u>	45.49	45.66	45.80	f
No94	17400			46.15	*	1			47.30	******	+	
No91	18000	· · · · · · · · · · · · · · · · · · ·			+			t	49.33	49.44	49.52	49.65
No88	18600						····	51.21	51.32	51.39	51.43	51.58
No85	19200			1			1		1		1	1
No82	19800				1				56.56	56.66	56.77	56.80
No79	20400										1	
No76	21000					1 ~ ~ ~ ~ ~ ~		1			1	
No73	21600			1	1							
No70	22200			1			1	1				1
No67	22800		· · · · · · · · · · · · · · · · · · ·						·		1	
No64	23400		¢									1
No61	24000						P	1				
No58	24600		1									
No55	25200		1									
No52	25800										1	
No49	26400			1				1				
No46	27000			1			1					· · · · · · · · · · · · · · · · · · ·
No43	27600						1		1			
	28200			85.00								

Table G.10 River Bed Profile of Bamban River under 1989 Rainfall Condition Case 1-a ; constant sediment supply and no dredging

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					(1989 ra	infall co	ndition)		(1000 m	3)	
Section	distance	l year	2 year	3 year	4 year	5 year	6 year	7 year	8 year	9 year	10year
No181	0	16	16	16	16	16	16	16	16	16	16
No178	600	637	469	444	436	439	447	458	470	484	497
No175	1200	500	460	440	436	440	449	461	474	487	500
No172	1800	366	447	435	435	442	453	465	478	491	504
No169	2400	369	429	429	436	446	459	472	485	498	511
No166	3000	431	409	423	437	452	467	481	495	508	521
No163	3600	402	396	421	441	459	476	491	505	518	530
No160	4200	410	385	421	448	470	489	504	518	531	543
No157	4800	357	381	426	459	484	505	520	534	548	559
No154	5400	280	387	441	478	507	528	543	558	572	583
No151	6000	245	406	467	504	536	556	569	586	598	609
No148	6600	369	438	500	534	564	581	595	611	624	634
No145	7200	474	495	554	578	598	615	629	645	657	667
No142	7800	603	569	622	630	639	656	669	684	696	705
No139	8400	753	648	691	685	685	699	711	725	736	745
No136	9000	861	724	751	739	730	744	755	767	777	786
No133	9600	840	804	808	792	779	792	802	812	821	828
No130	10200	883	885	866	844	830	844	853	860	866	872
	10200	1,081	967	930	893	888	901	<u> </u>	<u>912</u>	916	918
No127	11400		1,037	990	933	946	956	965	962	962	962
No124		1,059		1,047	955	1,020	1,011	1,021	1,015	1,011	1,007
No121	12000	1,149	1,105		1,030	1,020	1,011	1,021	1,013	1,064	1,056
<u>No118</u>	12600	1,152	1,151	1,106				1,130	1,117	1,107	1,097
No115	13200	1,259		1,186	1,106	1,103	1,121	1,150	1,163	1,150	1,136
No112	13800	1,320	1,284	1,245	1,170	1,158	1,171		1,209	1,193	1,176
No109	14400	1,365	1,341	1,303	1,241	1,225	1,223	1,219 1,259	1,209	1,228	1,208
No106	15000	1,412	}	1,343	1,302	1,288	1,270		1,247	1,220	1,200
No103	15600	1,530		1,370		1,348	}	1,299		1,202	1,263
No100	16200	1,527	1,498	1,425	1,421	1,398		1,338	1,319 1,351	1,320	1,205
No97	16800	1,727	1,598	1,507	1,443	1,439	t	1,376		1,345	
No94	17400	1,812	+	1,569	1 481	1,477	1,443	1,415	1,382	1,345	1,310 1,331
No91	18000	1,962					1,480	1,451	1,408		
No88	18600			<u>+</u>							
No85	19200			t					1,450		
No82	19800				f				1,484		
No79	20400					1,684			1,546		1,434
No76	21000				h			1,625		1,530	1,489
No73	21600			<u>}</u>	f		·} ·····			245	201
No70	22200					402	{				156
No67	22800			*							
<u>No64</u>	23400		*		+					1	
No61	24000			1	+			36	*		ŧ
<u>No58</u>	24600									↓ •	
No55	25200	↓ ~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~						}			
No52	25800	{		· · · · · · · · · · · · · · · · · · ·		· · · · · · · · · · · · · · · · · · ·			ŧ		
No49	26400									-	
No46	27000			+			·				
No43	27600							1			
No40	28200	79	77	79	79	78	76	76	77	78	78
					G - 3						

 Tabel G.11
 Sediment
 Transport of Bamban River under 1989 Rainfall Condition

 Case 1-a; constant sediment supply and no dredging

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						(1989 r	ainfall c	onditio	л)	(EL.m)	•	
Section	distance	EL in99	l year	2 year	3 year	4 year	5 year	6 year	7 year	8 year	9 year	10year
N49	0	63.20	62.72	63.27	63.48	63.55	63.59	63.61	63.62	63.63	63.64	Contraction of the second second
N50	280	64.75	64.30	64.90	65.17	65.26	65.32	65.34	65.36	65.38	65.39	65.39
N51	560	66.30	66.11	66.72	67.01	67.12	67.19	67.21	67.22	67.24	67.25	67.26
N52	840	67.85	67.86	68.45	68.77	68.85	68.91	68.96	68.98	69.00	69.02	69.03
N53	1120	69.40	69.67	70.27	70,59	70.66	70.71	70.78	70.79	70.80	70.82	70.84
N54	1400	70.95	71.48	72.05	72.35	72.47	72.50	72.55	72.57	72.61	72.64	72.66
N55	1680	72.50	73.22	73.79	74.07	74.24	74.25	74.28	74.36	74.43	74.50	74.53
N56	1960	74.05	75.01	75.57	75.86	76.12	76.16	76.17	76.32	76.43	76.52	76.61
N57	2240	75.60	76.82	77.47	77.64	78.18	78.64	78.80	78.93	79.01	79.09	79.16
N58	2520	77.15	78.68	<u>79.72</u>	80.46	80.80	81.00	81.23	81.33	81.42	81.52	81.62
N59	2800	78.70	81.50	82.41	82.97	83.33	83.45	83.72	83.82	83.92	84.04	84.14
<u>N60</u>	2850	80.90		82.82	83.15	83.48	84.00	84.18	84.26	84.36	84.48	84.54
<u>N61</u>	3075	82.15		84.41	85:31	85.69	86.02	86.25	86.27	86.41	86.54	86.64
N62	3300	83.40	85.95	86.84	87.26	87.77	87.86	88.26	88.30	88.46	88.62	88.77
<u>N63</u>	3350	85.70	86.43	87.27	87.53	87.99	88.17	88.59	88.92	88.99	89.07	89.19
N64	3575	86.95	88.31	89.01	89.07	89.74	90.33	90.60	90.95	91.05	91.14	91.25
<u>N65</u>	3800	88.20		90.84	91.37	91.96	-92.43	92.57	92.82	93.10	93.20	93.32
<u>N66</u>	3850	90.50		91.13	91.68	92.15	92.51	92.84	93.12	93.55	93.74	93.82
N67	4075	91.75	92.71	92.87	93.41	93.75	94.10	<u>94.76</u>	95.21	95.58	<u>95.79</u>	95.89
<u>N68</u>	4300	93.00		93.98	95.00		95.86	96.82	97.23	97.45	97.85	97.95
<u>N69</u>	4350			95.30			96.26	97.01	97.43	97.76	98.36	
<u>N70</u>	4575	t		97.00							100.40	
N71	4800			98.29							102.28	
N72	4850						100.10					
<u>N73</u>	5075						101.88					
N74	5300						106.64					· · · · · · · · · · · · · · · · · · ·
N75	5350						106.98					
N76 N77	<u>5475</u> 5600		a and a state of the state of t		and the second sec		108.02 113.09					
N78	5650						113.09 113.44					
N79	5900						115.44 116.47					a constant of the second
N80	6150						118.94					
N81	6400	a series of the second s	and the second second second			to all the set of the set	121.39					
N82	6650						124.06					
N83	6900						126.47					
N84	7150					a management	128.46					
N85	7650						132.17					
N86	8150				The Restance on Description		136.33					
N87	8650						141.00					
N88	9150						146.05					
N89	9650						151.00					

Table G.12 River Bed Profile of Diversion Channel under 1989 Rainfall Condition Case 1-a; constant sediment supply and no dredging

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					(1989 ra	infall co	ndition)		(1000 m	an a	****
Section	distance	l year	2 year	3 year	4 year	5 year	6 year	7 year	8 year	9 year	10yea
N49	0	1,185	1,195	1,256	1,284	1,298	1,299	1,300	1,303	1,305	1,30
N50	280	1,166	1,217	1,265	1,287	1,300	1,299	1,301	1,303	1,305	1,30
N51	560	1,148	1,241	1,275	1,290	1,302	1,300	1,302	1,304	1,305	1,30
N52	840	1,140	1,265	1,286	1,295	1,305	1,301	1,302	1,305	1,306	1,31
N53	1120	1,141	1,288	1,299	1,298	1,307	1,303	1,303	1,305	1,307	1,31
N54	1400	1,151	1,312	1,312	1,301	1,309	1,305	1,304	1,306	1,308	1,31
N55	1680	1,172	1,335	1,324	1,305	1,310	1,307	1,305	1,307	1,309	1,31
N56	1960	1,200	1,357	1,335	1,312	1,310	1,309	1,308	1,310	1,312	1,31
N57	2240	1,239	1,379	1,347	1,322	1,312	1,309	1,314	1,315	1,315	1,31
N58	2520	1,287	1,405	1,353	1,344	1,331	1,316	1,319	1,318	1,319	1,32
N59	2800	1,347	1,446	1,384	1,361	1,343	1,330	1,327	1,325	1,326	1,32
N60	2850	1,367	1,453	1,391	1,365	1,345	1,335	1,328	1,326	1,328	1,33
N61	3075	1,403	1,478	1,401	1,376	1,362	1,341	1,331	1,330	1,333	1,33
N62	3300	1,458	1,495	1,431	1,390	1,377	1,353	1,332	1,338	1,341	1,34
N63	3350	1,476	1,502	1,435	1,397	1,378	1,359	1,333	1,340	1,344	1,34
N64	3575	1,500	1,529	1,443	1,411	1,384	1,373	1,344	1,343	1,347	1,34
N65	3800	1,543	1,551	1,445	1,433	1,404	1,384	1,361	1,348	1,352	1,3
N66	3850		1,556	1,449	1,438	1,409	1,386	1,364	1,352	1,353	1,3
N67	4075		1,570	1,466	1,453	1,421	1,396	1,373	1,367	1,360	1,3
N68	4300			1,483	1,464	1,432	1,418	1,389	1,383	1,370	1,3
N69	4350			1,492		1,436	1,426	1,394	1,386	1,376	1,3
N70	4575			1,498	1,480	1,455	1,449	1,407	1,397	1,396	
N71	4800		·····	1	1,488	1,477	1,469	1,421	1,431	1,415	1,3
N72	4850			1,497	1,498	1,486	1,480	1,427	1,442	1,417	
N73	5075		1,575	1,498	1,498	1,493	1,492	1,460	1,468	1,430	
N74	5300	· · · · · · · · · · · · · · · · · · ·	1,559	1,498	3 1,498	1,493	1,496	1,493	1,494	1,463	
N75	5350			1,499	1,497	1,494	1,494	1,493	1,494	1,467	
N76	5475		1,611	1,50	1,496	5 1,49 4	1,495	1,494	1,494	1,470	
N77	5600				1 1,495	5 1,494	1,494	1,494	1,495		
N78	5650					1,494	1,494	1,493			
N79	5900				1,493	3 1,491	1,496	1,491	1,506	5 1,482	
N80	6150				1,513	3 1,492	2 1,497	1,488	+		
N81	6400		1,754	1,682	2 1,549	9 1,497	7 1,496	1,49.	1,494	1,49	
N82	6650			5 1,72	7 1,602	2 1,50	1 1,499	1,488	1,492		
N83	6900			5 1,75	3 1,650	5 1,530	0 1,483	1,484	1,505		
N84	7150			+	7 1,690	5 1,58	2 1,452	1,482			
N85	7650					5 1,68	9 1,516	1,40	1,562		
N86	815					2 1,75	4 1,642	1,432	2 1,424		
N87	865						7 1,730	5 1,557			
- N88	915					5 1,77	7 1,776				
N89	965			~			1 1,778	3 1,72	1,55	0 1,42	4 1,4

 Tabel G.13
 Sediment
 Transport of Diversion Channel under 1989 Rainfall Condition

 Case 1-a ; constant sediment supply and no dredging

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						(1989 r	ainfall c	onditio	n)	(EL.m))	
Section	distance	EL in99	1 year	2 year	3 year	4 year	5 year	6 year	7 year	8 year	9 year	10year
No181	0	7.01	7.01	7.01	7.01	7.01	7.01	7.01	7.01	7.01	7.01	7.01
No178	600	10.41	9.71	9.66	9.64	9.64	9.65	9.66	9.67	9.68	9.69	9.70
No175	1200	11.39	10.64	10.57	10.54	10.54	10.55	10.57	10.59	10.61	10.63	10.64
No172	1800	11.53	11.56	11.44	11.40	11.40	11.43	11.46	11.49	11.53	11.56	11.59
No169	2400	12.18	12.55	12.43	12.39	12.40	12.43	12.47	12.52	12.57	12.61	12.65
No166	3000	13.22	12.98	12.88	12.87	12.89	12.94	13.00	13.06	13.12	13.18	13.23
No163	3600	13.95	14.01	13.93	13.93	13.97	14.03	14.11	14.18	14.24	14.31	14.36
No160	4200	15.04	14.61	14.58	14.63	14.71	14.80	14.91	15.01	15.10	15.19	15.27
No157	4800	16.19	15.74	15.77	15.85	15.95	16.06	16.18	16.28	16.38	16.47	16.56
No154	5400	16.82	16.63	16.74	16.87	17.00	17.13	17.25	17.36	17.46	17.55	17.64
No151	6000	16.32	17.20	17.43	17.65	17.84	18.00	18.13	18.24	18.35		18.54
No148	6600	17.69	18.29	18.61	18.89		19.27	19.39	19.51	19.61		19.81
No145	7200	18.57	19.38	19.80	20.11	20.33	20.49	20.63	20.75	20.87	20.97	21.06
No142	7800	19.55	20.49	20.91	21.23	21.46	21.64	21.78	21.90	22.02	22.12	22.21
No139	8400	20.50		22.10	22.37	22.59	22.77	22.91	23.03	23.15		23.34
No136	9000	23.09	22.98	23.37	23.60		23.96	24.11	24.24	24.35		24.54
No133	9600	24.08	24.31	24.68	24.90		25.22	25.36	25.49	25.59		25.78
No130	10200		25.74	26.09	26.30		26.61	26.73	26.84	26.93	27.02	27.10
No127	10800		27.17	27.46	27.65		27.92	28.03	28.12	28.21	28.29	28.34
No124	11400		28.69	28.92	29.09		29.31	29.41	29.49	29.55	29.60	29.61
No121	12000		29.74	29.95	30.12		30.41	30.59	30.76	30.94		31.29
<u>No118</u>	12600		31.61	31.80	31.95		32.19		32.44	32.56		32.79
No115	13200		33.35	33.47	33.56		33.73		33.91	33.99	34.07	34.15
No112	13800			35.14	35.22		35.38			35.62	35.69	35.76
No109	14400		36.68	36.78	36.86	f	37.00		37.13	37.18	•	37.28
No106	15000	1	38.35	38.47	38.55	f	38.69		38.79	38.83		38.87
No103	15600	T	39.46	39.67	39.82	39.98	40.08	40.20	40.32	40.44		40.67
<u>No100</u>	16200	1		41.92	42.08	· · · · · · · · · · · · · · · · · · ·	42.31	42.40		42.57		42.74
<u>No97</u>	16800			44.07	44.18		44,33		44.44	44.49		44.59
<u>No94</u>	17400	1		46.07	46.15		46.26		46.35	46.39		46.47
No91	18000		48.14	48.28	48.38	48.42	48.46		48.53	48.56		48.62
No88	18600	1				1						
No85	19200					· · · · · · · · · · · · · · · · · · ·					· · · · · · · · · · · · · · · · · · ·	53.02
<u>No82</u>	19800	1		· · · · · · · · · · · · · · · · · · ·		+					t	1
<u>No79</u> No76	20400		<u>58.45</u> 61.37			1	58.78		58.73 61.81	<u>58.69</u> 61.74	The second s	<u>58.60</u> 61.59
<u>No76</u> No73	21000 21600	[f			61.90 64.71			فأخرف فستعدد والمع	1	63.74
No70	21000			t		+						64.07
No67	22200	1								1		65.04
No64	22800	The second se	71.04		1							
No61	23400		72.43				69.00					69.00
No58	24600		74.49	§							1	****
No55	25200		73.01	73.00	·	1		£				
No52	25800				· · · · · · · · · · · · · · · · · · ·	1		and the second s				
No49	25800	1									1	
No46	27000	1				1			· · · ·	<u></u>		
No43	27600	2 ·····	}							i		
No40	28200			F						• ·····		
No40	28200	85.00	85.00	85.00	85.00	85.00	85.00	85.00	85.00	85.00	85.00	85.00

 Table G.14
 River Bcd Profile of Bamban River under 1989 Rainfall Condition

 Case 1-b ; constant sediment supply and dredging works

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Quation	d:	1	1	2		infall co	LOCAL DIVISION OF TAXABLE	7	(1000 m	and Area and Area	10.000
Section	CHARGE IN CONTRACT	<u>1 year</u>	2 year	3 year	4 year	5 year	6 year	7 year	8 year	9 year	10year
No181	0	16	16	16	16	16	16	16	16	16	16
No178	600	637	469	444	436	438	445	454	465	475	484
No175	1200	500	460	440	435	439	447	457	467	477	486 489
No172	1800	366	447	435	435	441	450	460	471	480	409
No169	2400	369	429	429	435 437	<u>444</u> 450	455 462	466 474	476 484	<u>486</u> 493	501
No166 No163	<u>3000</u> 3600	431	409 396	<u>423</u> 420	440	456	402	474	404	500	507
No160	4200	402	385	420	446	450	470	492	501	509	516
No157	4200	357	381	420	440	405	401	505	513	520	526
	5400	280	386	440	450	477	514	523	529	535	541
No154	6000	260	406	440	475	520	537	542	547	552	558
No151	6600	369	400	404	523	543	557	560	564	567	572
No148	7200		457	545	562	572	581	583	585	587	590
No145 No142	7800	<u>474</u> 603	495 565	608	606	605	608	608	609	608	609
										629	627
No139	8400	753	640	670	<u>652</u> 693	<u> </u>	$\frac{636}{662}$	<u>634</u> 658	<u>632</u> 655	649	644
No136	9000	861	712	719			689	683	677	668	661
No133	9600	840	786	762	731	704	716	706	697	686	679
No130	10200	883	857	803	705	<u>732</u> 759	739	700		703	697
No127	10800	1,081	923	844			848	835	716 821	806	795
No124	11400	1,059	951	907	<u> </u>	869 980	958	941	925	906	889
No121	12000	1,149	1,083	1,031				1,014	923	900	961
Nol18	12600	1,152	1,110	1,076	1,060	1,049	1,031				1,068
No115	13200	1,259	1,229	1,190	1,169	1,156 1,256	1,141	1,123 1,225	1,106	1,088	1,008
No112	13800	1,320	1,310	1,287	1,271			1,332		1,100	
No109	14400	1,365	1,372	1,363	1,364	1,361	1,351		1,313	1,295	1,273
No106	15000	1,412	1,434	1,438	1,453	1,453 1,540	1,444	1,425	1,404		1,444
No103	15600	1,530		1,530	1,545	1,540	1,532 1,583	1,511	1,489	1,467	1,496
No100	16200	1,527	1,546	1,557	1,585			1,563		1,609	1,586
No97	16800	1,727	1,665	1,654	1,681	1,678	1,674	1,653	1,632		1,681
No94	17400	1,812	1,749	1,716	1,751	1,751	1,753 1,761	1,737	1,729	1,703 1,710	1,688
No91	18000	1,962	1,742 1,769	1,729	1,763	1,760 1,768					
No88	18600							· · · · · · · · · · · · · · · · ·		1,720	
No85	19200	1,955	<u>}</u>		1,779						
No82	19800	1,942		1,840			1,778				1,691
No79	20400	2,073		1.951	1,830	1,801			t		1,676
No76	21000	2,299						<u>1,749</u> 383		307	275
No73	21600	1,585			· · · · · · · · · · · · · · · · · · ·	501 389	438 320		<u> </u>	201	178
No70	22200	2,096		<u>877</u> 821	<u>519</u> 456			213	}	152	131
No67	22800	2,086		f		188		115		62	58
No64	23400	1,622				75	66				
No61	24000	1,426				f				63	58
No58	24600		.				¢	<u>↓</u>		- 78	• • • • • • • • • • • • • • • • • • •
No55	25200							*		78	
No52	25800							<u>}</u>	···	78	
No49	26400					····		ð		78	{
No46	27000			<u>}</u>							· · · · · · · · · · · · · · · · · · ·
No43	27600								+	78	
No40	28200	79	L	<u>19</u>	<u>19</u>	<u>/8</u>	/0	/0	//	/8	

Tabel G.15 Sediment Transport of Bamban River under 1989 Rainfall Condition Case 1-b ; constant sediment supply and dredging works

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						(1989 i	ainfall (conditio	m)	(EL.m))	
Section	distance	EL in99	l year	2 year	3 year	4 year	5 year	6 year	7 year	8 year	9 year	10year
N49	0	63.20	62.72	63.27	63.49	63.51	63.51	63.49	63.43	63.37	63.30	63.24
<u>N50</u>	280	64.75	64.30	64.89	65.18	65.24	65.26	65.24	65.20	65.16	65.11	65.06
<u>N51</u>	560	66.30	66.11	66.72	67.02	67.11	67.14	67.14	67.10	67.07	67.03	66.99
N52	840	67.85	67.86	68.45	68.74	68.85	68.88	68.88	68.85	68.85	68.82	68.79
<u>N53</u>	1120	69.40	69.67	70.25	70.56	70.67	70.70	70.71	70.69	70.69	70.67	70.65
N54	1400	70.95	71.48	72.04	72.36	72.48	72.50	72.52	72.51	72.47	72.46	72.44
N55	1680	72.50	73.22	73.77	74.07	74.25	74.26	74.27	74.28	74.27	74.26	74.23
N56	1960	74.05	75.01	75.57	75.85	76.12	76.13	76.13	76.16	76.19	76.19	76.16
N57	2240	75.60	76.82	77.46	77.66	78.19	78.60	78.75	78.85	78.90	78.94	78.97
<u>N58</u>	2520	77.15	78.68	79.67	80.45	80.77	80.98	81.17	81.30	81.36	81.43	81.46
<u>N59</u>	2800	78.70	81.50	82.32	82.98	83.30	83.45	83.65	83.82	83.89	84.00	84.04
<u>N60</u>	2850	80.90	82.05	82.72	83.15	83.52	83.99	84.17	84.25	84.33	84.40	84.45
<u>N61</u>		82.15		84.37	85.30	85.69	86.00		86.27	86.37	86.45	86.51
N62	3300			86.76		87.74	87.84	88.22	88.30	88.42	88.52	88.59
N63	3350			87.22	87.46	87.96	88.15	88.55	88.91	88.99	89.02	89.08
<u>N64</u>	3575	86.95	88.31	88.97	89.11	89.54			90.94	91.06	91.08	91.15
<u>N65</u>	3800	88.20		90.84	91.22	91.78	92.24	92.67	92.80	93.11	93.13	93.22
N66	3850	90.50		91.22	91.63	92.18	92.51	92.86	93.11	93.46	93.73	93.77
N67	4075	91.75	92,71	<u>92.86</u>	93.27	93.75	93.97	94.62	95.22	95.45	95.80	95.83
N68	4300	93.00	94.85	<u>94.06</u>	94.23	95.27	95.74	96.64	97.26	<u>97.44</u>	97.77	97.88
<u>N69</u>	4350	95.30	95.31	95.30	<u>95.30</u>	95.68	96.18	96.93	97.35	97.64	98.11	98.50
N70	4575	96.55		97.01	96.99	<u>96.97</u>	<u>97.89</u>		99.02		100.16	
<u>N71</u>	4800	97.80		98.31	98.26	97.81	98.88		100.46			
<u>N72</u>	4850			100.10								
N73	5075			101.88								
N74	5300			106.62								
<u>N75</u>	5350			106.96								
N76	5475			108.01								
<u>N77</u>	5600			112.07			a second s				the second second	
<u>N78</u>	5650			112.42		and here there is a second to be						
N79	5900			114.25								
<u>N80</u>	6150			116.34								
N81	6400			118.54								
<u>N82</u>	6650			121.00								
N83	6900			123.50								
N84	7150			126.00								
<u>N85</u>	7650			131.00								
<u>N86</u>	8150			136.01								
<u>N87</u>	8650			141.00								
<u>N88</u>	9150			146.03								
N89	9650	151.00	151.00	151.00	131.00	121.00	191'00	151.00	151.00	151.00	151.00	151.00

Table G.16 River Bed Profile of Diversion Channel under 1989 Rainfall ConditionCase 1-b ; constant sediment supply and dredging works

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					(1989 га	infall co	ndition)		(1000 n	13)	andre beserview
Section	distance	1 year	2 year	3 year	4 year	5 year	6 year	7 year	8 year	9 year	10year
N49	0	1,185	1,201	1,258	1,292	1,311	1,320	1,330	1,341	1,352	1,361
N50	280	1,166	1,223	1,267	1,293	1,311	1,319	1,327	1,339	1,349	1,359
N51	560	1,148	1,247	1,278	1,295	1,311	1,318	1,326	1,337	1,347	1,357
N52	840	1,140	1,271	1,290	1,298	1,313	1,318	1,324	1,336	1,346	1,355
N53	1120	1,141	1,294	1,302	1,302	1,314	1,318	1,323	1,335	1,344	1,35
N54	1400	1,151	1,317	1,314	1,307	1,315	1,318	1,323	1,336	1,343	1,35
N55	1680	1,172	1,339	1,326	1,311	1,316	1,319	1,322	1,334	1,343	1,35
N56	1960	1,200	1,361	1,338	1,318	1,317	1,319	1,323	1,333	1,342	1,35
N57	2240	1,239	1,383	1,349	1,329	1,317	1,319	1,324	1,334	1,342	1,35
N58	2520	1,287	1,408	1,357	1,350	1,334	1,325	1,329	1,336	1,345	1,35
N59	2800	1,347	1,448	1,390	1,366	1,347	1,337	1,337	1,340	1,349	1,35
N60	2850	1,367	1,454	1,397	1,370	1,349	1,340	1,340	1,342	1,351	1,35
N61	3075	1,403	1,475	1,411	1,382	1,364	1,346	1,343	1,345	1,354	1,35
N62	3300	1,458	1,492	1,441	1,397	1,378	1,359	1,345	1,350	1,359	
N63	3350	1,476	1,498	1,447	1,401	1,380	1,364	1,346	1,352	1,361	1,36
N64	3575	1,500	1,523	1,455	1,417	1,385	1,377	1,358	1,355	1,362	1,36
N65	3800	1,543	1,544	1,459	1,431	1,410	1,390	1,376	1,361	1,363	1,36
N66	3850	1,557	1,549	1,462	1,435	1,414	1,396	1,378	1,366	1,364	1,37
N67	4075	1,563	1,566	1,475	1,453	1,425	1,407	1,386	1,377	1,372	1,37
N68	4300	1,594	1,570		1,468	1,432	1,428	1,406	1,387	1,390	1,37
N69	4350	1,607	1,566	1,490	1,476	1,435	1,435	1,413	1,390	1,395	1,37
N70	4575	1,608	1,567	1,493	1,497	1,451	1,459	1,427	1,399	1,410	1,38
N71	4800	1,630	1,560	1,492	1,496	1,481	1,482	1,440	1,423	1,425	1,40
N72	4850	1,642	1,563	1,492	1,500	1,494	1,489	1,443	1,433	1,432	1,41
N73	5075	1,651	1,568	1,499	1,500	1,494	1,500	1,464	1,468	1,444	1,43
N74	5300		1,552	1,499	1,500	1,494	1,496	1,493	1,496	1,480	
N75	5350	1,696	1,571	1,500	1,499	1,493	1,495	1,493	1,497	1,483	1,4
N76	5475	1,698	1,607	1,502	1,498	1,494	1,494	1,494	1,495	1,488	1,40
N77	5600	1,719	1,629	1,504	1,496	1,492	1,495	1,494	1,494	1,491	1,49
N78	5650		1,670	1,522	1,501	1,493	1,492	1,497	1,492	1,494	1,49
N79	5900		1,720	1,569	1,493	1,491	1,503	1,487	1,502	1,481	1,49
N80	6150	1,740	1,743	1,627	1,514	1,489	1,501	1,489	1,499	1,485	
N81	6400	1,744	1,755	1,679	1,548	1,497	1,492	1,501			
N82	6650	1,744	1,757	1,723	1,601	1,505	1,489	1,498	1,489	1,500	1,49
N83	6900			1,750	1,655	1,530	1,476	1,489	1,50€	1,501	1,4
N84	7150		*			5 1,581	1,448	1,484	1,540	1,500	1,4
N85	7650	and the second designed in				5 1,686	1,508	1,407	1,57	1,476	1,4
N86	8150						1,634	1,429	1,42	1,442	1,4
N87	8650		· · · · · · · · · · · · · · · · · · ·				1,731	1,549	1,410	1,434	
N88	9150							1,660	5 1,4 7	1,429	
N89	9650						and the second sec	1,727	1,535	1,427	1,4

Tabel G.17 Sediment Transport of Diversion Channel under 1989 Rainfall ConditionCase 1-b ; constant sediment supply and dredging works

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1996 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 -	uan a coasta ta forma a forma	en en en stat de stat de Stat de Stat				(1989 r	ainfall c	a di setta di secola da la fina di secola di secol		(EL.m)		antan acit watan wa
Section	distance	EL in99	l year	2 year	3 year	4 year	5 year	6 year	7 year	8 year	9 year	10year
<u>No181</u>	0	7.01	7.01	7.01	7.01	7.01	7.01	7.01	7.01	7.01	7.01	7.01
No178	600	10.41	9.71	9.66	9.64	9.6 4	9.65	9.66	9.68	9.69	9.71	9.72
No175	1200	11.39	10.64	10.57	10.54	10.54	10.56	10.58	10.60	10.62	10.65	10.67
No172	1800	11.53	11.56	11.44	11.40	11.41	11.43	11.47	11.51	11.56	11.60	11.64
No169	2400	12.18	12.55	12.43	12.39	12.40	12.44	12.49	12.55	12.60	12.66	12.72
No166	3000	13.22	12.98	12.88	12.87	12.90	12.95	13.02	<u>13.10</u>	13.18	13.25	13.33
No163	3600	13.95	14.01	13.93	13.93	13.98	14.05	14.14	14.22	14.31	14.40	14.49
No160	4200	15.04	14.61	14.58	14.63	14.72	14.83	14.96	15.08	15.21	15.35	15.48
No157	4800	16.19	15.74	15.77	15.86	15.97	16.10	16.24	16.37	16.51	16.65	16.79
No154	5400	16.82	16.63	16.74	16.88	17.02	17.18	17.33	17.48	17.63	17.77	17.92
No151	6000	16.32	17.20	17.43	17.67	17.88	18.07	18.23	18.39	18.55	18.71	18.87
No148	6600	17.69	18.29	18.61	18.92	19.16	19.35	19.52	19.69	19.86	20.03	20.20
No145	7200	18.57	19.38	19.81	20.15	20.40	20.60	20.80	20.99	21.18	21.36	21.54
No142	7800	19.55	20.49	20.93	21.29	21.56	21.79	22.01	22.22	22.41	22.60	22.79
No139	8400	20.50	21.55	22.13	22.45	22.74	22.98	23.21	23.43	23.65	23.85	24.05
No136	9000	23.09	22.98	23.40	23.71	23.99	24.25	24.50	24.75	24.98	25.20	25.42
No133	9600	24.08	24.31	24.74	25.04	25.32	25.59	25.86	26.12	26.37	26.60	26.82
No130	10200	24.70	25.74	26.17	26.51	26.76	27.04	27.32	27.59	27.84	28.07	28.29
No127	10800	27.30	27.17	27.57	27.90	28.12	28.43	28.71	28.99	29.24	29.48	29.70
No124	11400	28.20	28.69	29.06	29.37	29.53	29.93	30.23	30.51	30.76	31.00	31.22
No121	12000	29.70	29.74	30.16	30.71	31.34	31.61	31.90	32.18	32.44	32.67	32.89
No118	12600		31.61	32.04	32.50	32.94	33.12	33.40	33.66	33.90	34.13	34.35
No115	13200		33.35	33.69	34.03	T	34.71	35.00	35.24	35.47	35.70	35.90
No112	13800					f	-36.42	36.71	36.94	37.18	37.40	37.60
No109	14400			37.00			38.00	38.29	38.53	38.76	38.97	39.15
No106	15000		38.35				39.69	39.97	40.20	40.41	40.60	40.77
No103	15600	T	39.46	**************************************				41.69	41.91	42.11	42.28	42.44
No100	16200	1	41.59	42.26	42.78	42.92	43.17	43.42	43.65	43.83	44.00	44.13
No97	16800			44.26		44.83	45.05	45.27	45.49	45.66	45.80	45.91
No94	17400			46.15			46.90	47.11	47.30		47.56	47.66
No91	18000			48.30	48.59	48.84	48.97	49.16	49.33	49.44	49.52	49.59
No88	18600		t		1			51.21	51.32	51.39	51.43	51.47
No85	19200	F				1		1		2	53.79	53.83
No82	19800	1				**************************************	1					
No79	20400								T			
No76	21000				1				1		61.74	
No73	21600		1						_	1	·····	63.51
No70	22200		1						1			
No67	22800		1		T	1						
No64	23400				2				1			
No61	24000	1	1		1			1	1	1		
No58	24600		t		*****				1			1
No55	25200		1									*
No52	25800	T										
No49	26400	f	fr		1		1					
No46	27000	£	2					1				· · · · · · · · · · · · · · · · · · ·
No43	27600		1									
No40	28200											

Table G.18River Bed Profile of Bamban River under 1989 Rainfall Condition
Case 2-a; declining sediment supply and no dredging

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						(1989 ra	infall co	ndition)	((1000 m	3)	
Sec	tion	distance	1 year	2 year	3 year	4 year	5 year	6 year	7 year	8 year	9 year	10year
) - marine	181	0	16	16	16	16	16	16	16	16	16	16
	178	600	637	469	444	436	439	447	458	471	484	497
	175	1200	500	460	440	436	440	449	461	474	487	500
No	172	1800	366	447	435	435	442	453	465	478	491	504
No	5169	2400	369	429	429	436	446	459	472	485	498	511
No	0166	3000	431	409	423	437	452	467	481	495	508	521
No	5163	3600	402	396	421	441	459	476	491	505	518	530
No	5160	4200	410	385	421	448	470	489	50-1	518	531	543
No	5157	4800	357	381	426	459	484	505	520	534	548	559
N	5154	5400	280	387	441	478	507	528	543	558	572	583
No	0151	6000	245	406	467	504	536	556	569	586	598	609
No	o148	6600	369	438	500	534	564	581	595	611	624	635
N	o145	7200	474	495	554	578	598	615	629	645	657	667
N	0142	7800	603	569	622	630	639	656	669	684	696	706
N	o139	8400	753	648	691	685	685	699	711	725	736	745
N	0136	9000	861	724	751	739	730	744	755	767	777	786
N	o133	9600	840	804	808	792	779	792	802	812	821	828
N	o130	10200	883	885	866	844	830	844	853	860	867	872
· · · · · ·	0127	10800	1,081	967	930	893	888	901	910	912	916	918
	0124	11400	1,059	1,037	990	933	946	956	965	962	962	962
	0121	12000	1,149	1,105	1,047	962	1.020	1,011	1,021	1.015	- 1,011	1,007
	0118	12600	1,152	1,151	1,106	1,030	1,070	1,071	1,081	1,071	1,064	1,056
	0115	13200	1,259	1,226	1,186	1,106	1,103	1,121	1,130	1,117	1,107	1,097
	0112	13800			1,245	1,170	1,158	1,172	1,175	-1,163	1,150	1,136
3	o109	14400			1,303	1,241	1,225	1,223	1,219	1,209	1,193	1,176
i	0106	15000	1,412	1,394	1,343	1,302	1,288	1,270	1,259	1,247	1,228	1,208
	0103	15600		1,447	1,370	1,374	1,348	1,318	1,299	1,284	1,262	1,238
N	o100	16200			1,425	1,421	1,398	1,362	1,338	1,318	1,292	1
	1097	16800	1,727	1,598	1,507	1,443	1,439	1,402	1,376	1,350	1,319	4
	1094	17400	1,812	1,641	1,569	1,481	1,477	1,444	1,415	1,381		
1	1091	18000	1,962	1,648	1,623	1,528	1,507	1,480	1,450			
	1088	18600	1,931	1,680	1,681	1,577	1,534	1,522	1,487	1,433	1,384	
	No85	19200	1,955	1,717	1,737	1,620	1,562	1,548	1,510			
	1082	19800	1,942	1,777	1,799	1,686	5 1,602	1,578	1,538	1,474		
	No79	20400	2,073	1,921	1,913	1,791	1,684	1,637	1,589	1,510		
	No76	21000			1,928	1,828	1,737	1,668				
	No73	21600			734	570	i 49 5			1		
1	No70	22200			850	53	401	322				
	No67	22800	2,086	1,364	792	47	336					
1	No64	23400	1,622	2 1,220	638	32	5 195					
	No61	24000	1,420	5 1,050	474	35						
	No58	24600	1,288	8 849	328	9						
	No55	25200			3 79	7						
	No52	25800		3 75	3 79) 7					- 4	
	No49	26400		5 7	7 79	> 7						
	No46	27000	0 79	9 7	7 79			-+			-+	
	No43	2760	0 79	9 7								
	No40	2820	0 7	0 7	7 79	7	9 7	8 70	6 76	5 7	7 7	8 78

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Tabel G.19 Sediment Transport of Bamban River under 1989 Rainfall ConditionCase 2-a ; declining sediment supply and no dredging

						(1989 r	ainfall c	onditio	n)	(EL.m)	000 NO 10002008.4	- Andrew The Mandam Local
Section	distance	EL in99	l year	2 year	3 year	4 year	5 year	6 year	7 year	8 year	9 year	10year
N49	0	63.20	62.72	63.27	63.48	63.55	63.60	63.59	63,42	63.17	62.90	62.62
N50	280	64.75	64.30	64.91	65.17	65.26	65.32	65.28	64.94	64.53	64.18	63.77
N51	560	66.30	66.11	66.73	67.01	67.13	67.18	67.11	66.60	66.06	65.60	65.08
N52	840	67.85	67.86	68.45	68.75	68.86	68.91	68.83	68.18	67.57	67.00	66.38
N53	1120	69.4 0	69.67	70.27	70.58	70.67	70.71	70.61	<u>69.79</u>	69.12	68.42	<u>67.68</u>
N54	1400	70.95	71.48	72.05	72.37	72.47	72.51	72.31	71.32	70.56	69.72	68.82
N55	1680	72.50	73.22	73.77	74.06	74.24	74.27	73.94	72.82	71.91	70.96	69.87
N56	1960	74.05	75.01	75.58	75.85	76.12	76.13	75.57	74.34	73.19	72.10	70.81
N57	2240	75.60	76.82	77.47	77.68	78.16	78.08	77.15	75.69	74.29	72.97	71.62
N58	2520	77.15	78.68	79.68	80.46	80.74	80.41	78.78	76.86	75.24	73.57	72.32
<u>N59</u>	2800	78.70	81.50	82.40	82.97	83.27	82.72	80.68	78.70	78.70	78.70	78.70
N60	2850	80.90	82.05	82.80	83.17	83.33	82.91	80.95	80.90	80.90	80.90	
N61	3075	82.15	83.86	84.39	85.39	85.48	84.35	82.05	82.01	81.97	81.88	<u>81.86</u>
N62	3300	83.40	85.95	86.72	87.46	87.40	85.83	83.40	83.40	83.40	83.40	83.40
<u>N63</u>	3350	85.70	86.43	87.20	87.57	87.60	86.04	85.70	85.70	85.70	85.70	85.70
<u> </u>	3575	86.95	88.31	88.90	89.18	89.08	87.04	86.92	86.79	86.74	86.67	86.63
<u>N65</u>	3800	88.20	90.15	91.54	91.44	90.80	88.20	88.20	88.20	88.20	88.20	88.20
N66	3850	90.50	90.68	92.03	91.79	90.70	90.50	90.50	90.50	90.50	90.50	90.50
N67	4075	91.75	92.71	93.66	93.50		·		91.66	91.58	91.50	91.42
N68	4300	93.00	94.85	95.28	94.91	93.00	93.00			93.00	93.00	93.00
<u>N69</u>	4350		95.31	95.64	95.39	95.30					95.30	t (
N70	4575	96.55	97.25	97.34	97.04	96.86			96.62	96.54	96.43	<u>96.30</u>
<u>N71</u>	4800										97.80	(
N72	4850	· · · · · · · · · · · · · · · · · · ·				100.10						
N73	5075	101.35					T					101.30
N74	5300											102.60
<u>N75</u>	5350	f				<u>104.90</u>	f					
N76	5475			****		105.92						
N77	5600	· · · · · · · · · · · · · · · · · · ·										106.30
N78	5650				t	1						111.00
<u>N79</u>	5900						· · · · · ·					113.50
<u>N80</u>	6150						1	1	F	1		116.00
<u>N81</u>	6400									7		118.50
<u>N82</u>	6650											121.00
<u>N83</u>	6900								· · · · · · · · · · · · · · · · · · ·			123.50
<u>N84</u>	7150			1								126.00
N85	7650		••••••		<u>}</u>	<u>} · · · · · · · · · · · · · · · · · · ·</u>		;	}			131.00
<u>N86</u>	8150								*			136.00
<u>N87</u>	8650				·							141.00
<u>N88</u>	9150											146.00
<u>N89</u>	9650	L 121'00	[151.00	[151.00	1151.00	<u>1151.00</u>	1151.00	1151.00	[151.00	1151.00	1151.00	151.00

 Table G.20 River Bed Profile of Diversion Channel under 1989 Rainfall Condition

 Case 2-a ; declining sediment supply and no dredging

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						(1989 ra	infall co	ndition)		(1000 n	13)	
Se	ction	distance	l year	2 year	3 year	4 year	5 уеаг	6 year	7 year	8 year	9 year	10year
1	349	0	1,185	1,195	1,257	1,284		1,289	1,205	1,062	933	826
1	150	280	1,166	1,217	1,266	1,287	1,298	1,288	1,198	1,052	923	815
1	151	560	1,148	1,241	1,276	1,290	1,301	1,286	1,185	1,036	909	799
N	152	840	1,140	1,266	1,287	1,295	1,303	1,284	1,165	1,014	891	778
N	153	1120	1,141	1,289	1,299	1,299	1,305	1,281	1,139	990	868	754
N	154	1400	1,151	1,313	1,311	1,303	1,307	1,276	1,107	964	840	725
N	155	1680	1,172	1,336	1,323	1,307	1,308	1,268	1,068	933	807	690
N	156	1960	1,200	1,357	1,335	1,314	1,309	1,256	1,024	897	<u> </u>	646
	\$57	2240	1,239	1,380	1,346	1,324	- 1,310	1,234	975	852	727	595
N	158	2520	1,287	1,405	1,354	1,343	1,306	1,197	918	797	675	542
	159	2800	1,347	1,445	1,387	1,357	1,290	1,131	841	733	609	493
N	160	2850	1,367	1,453	1,393	1,361	1,283	1,115	841	733	609	493
N	161	3075	1,403	1,476	1,405	1,367	1,270	1,054	841	733	609	493
N	162	3300	1,458	1,493	1,438	1,370		981	840	731	606	
1	163	3350	1,476	1,499	1,445	1,369	1,219	976	850	737	613	496
N	164	3575	1,500	1,523	1,457	1,370	1,174	973	850	737	613	496
Ν	165	3800	1,543	1,542	1,466	1,367	1,109	969	846	735	611	495
N	166	3850	1,557	1,552	1,465	1,362	1,113	978	857	745	624	501
Ν	N67.	4075	1,563	1,595	1,458	1,328	1,111	978	857	745	624	501
1	168	4300	1,594	1,625	1,453	1,269	1,118	974	855	742	621	: 499
1	169	4350	1,607	1,628	1,451	1,271	1,129	989	863	749	630	
1	170	4575	1,608	1,639	1,444	1,272	1,129	989	863	749	630	
1	171	4800	1,630	1,642	1,435	1,266	1,126	986	861	746		504
1	172	4850	1,642	1,638	1,443	1,282	1,147	1,006	880	760		
1	173	5075	1,651	1,630	1,443	1,282	1,147	1,006	880	760	636	
1	174	5300	1,684	1,616	1,439	1,280	1,144	1,003	878	759	634	510
1	175	5350	1,696	1,619	1,468	1,325	1,184	1,030	903	772	645	
1	<u>176</u>	5475	1,698	1,632	1,468	1,325	1,184	1,030	903	772	645	
1	177	5600	1,719	1,620	1,466	1,324	1,183	1,029	902	772	· ····································	516
1	178	5650	1,737	1,608	1,470	1,335	1,197	1,058	923	789		
1	179	5900	1,737	1,608	1,470	1,335		1,058	923	789	655	521
1	480	6150	1,740	1,606	1,470	1,335			923			
1	181	6400	1,744	1,606				1,058	923			
1	182	6650	1,744	·····				1,058	923			
1	183	6900	1,744	1,606	1,470	·····	<u>}</u>	1,058	923	789		
1	N84	7150	1,745	1,606	1,470			1,058	923			
1	N85	7650	1,745	1,606	1,470	1,335	1,197	1,058	923	789		
	N86	8150	1,745	1,606	1,470			~	923	789		
1	N87	8650	1,745	1,606				1,058	923			
	88	9150				<u> </u>	.		923	4		
1	N89	9650	1,748	1,606	1,470	1,335	1,197	1,058	923	789	655	521

Tabel G.21 Sediment Transport of Diversion Channel under 1989 Rainfall Condition Case 2-a ; declining sediment supply and no dredging

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						(1989 r	ainfall o	conditio	n)	(EL.m))	
Section	distance	EL in99	1 year	2 year	3 year	4 year	5 year	6 year	7 year	8 year	9 year	10year
No181	0	7.01	7.01	7.01	7.01	7.01	7.01	7.01	7.01	7.01	7.01	7.01
No178	600	10.41	9.71	9.66	9.64	9.64	9.65	9.66	9.67	9.68	9.69	9.70
No175	1200	11.39	10.64	10.57	10.54	10.54	10.55	10.57	10.59	10.61	10.63	10.64
No172	1800		11.56	11.44	11.40	11.40	11.43	11.46	11.49	11.53	11.56	11.59
No169	2400		12.55	12.43	12.39	12.40	12.43	12.47	12.52	12.57	12.61	12.65
No166	3000	13.22	12.98	12.88	12.87	12.89	12.94	13.00	13.06	13.12	13.18	13.23
No163	3600	13.95	14.01	13.93	13.93	13.97	14.03	14.11	14.18	14.24	14.31	14.36
No160	4200	15.04	14.61	14.58	14.63	14.71	14.80	14.91	15.01	15.10	15.19	15.27
No157	4800	16.19	15.74	15.77	15.85	15.95	16.06	16.18	16.28	16.38		16.56
No154	5400	16.82	16.63	16.74	16.87	17.00	17.13	17.25	17.36	17.46	17.55	17.64
No151	6000	16.32	17.20	17.43	17.65	17.84	18.00	18.13	18.24	18.35	18.45	18.54
No148	6600	17.69	18.29	18.61	18.89	19.11	19.27	19.39	19.51	19.61	19.71	19.81
No145	7200	18.57	19.38	19.80	20.11	20.33	20.49	20.63	20.75	20.87	20.97	21.06
No142	7800		20.49	20.91	21.23	21.46	21.64	21.78	21.90	22.02	22.12	22.21
No139	8400	20.50	21.55	22.10	22.37	22.59	22.77	22.91	23.03	23.15	23.25	23.34
No136	9000	23.09	22.98	23.37	23.60	23.80	23.96	24.11	24.24	24.35	24.45	24.54
No133	9600	24.08	24.31	24.68	24.90	25.08	25.22	25.36	25.49	25.59	25.69	25.78
No130	10200		25.74	26.09	26.30	26.47	26.61	26.73	26.84	26.93	27.02	27.10
No127	10800	27.30	27.17	27.46	27.65	27.80	27.92	28.03	28.12	28.21	28.29	28.34
No124	11400	28.20	28.69	28.92	29.09	29.21	29.31	29.41	29.49	29.55	29.60	29.61
No121	12000	29.70	29.74		30.12	30.27	30.41	30.59	30.76	30.94	31.12	31.28
No118	12600	31.00	31.61	31.80	31.95	32.07	32.19	32.32	32.44	32.56	32.68	32.79
No115	13200	33.00	33.35		33.56	-33.65	33.73	33.82	33.91	33.99	34.07	34.15
No112	13800	34.80	35.05	35.14	35.22	35.30	35.38	35.46	35.54	35.62	35.69	35.76
No109	14400		36.68	36.78	36.85	36.94	37.00		37.13	37.18	37.23	37.27
No106	15000		38.35	38.47	38.55	38.64	38.69	38.74	38.79		38.85	38.87
No103	15600		39.46	39.67	39.82	39.98	40.08	40.20	40.32	40.43	40.54	40.63
No100	16200		41.59	41.92	42.07	42.22	42.30	42.40	42.49	42.57	42.64	42.70
No97	16800		44.01	44.07	44.18	44.27	44.33	44.39	44.44	44.49	44.52	44.56
No94	17400		46.11	46.07	46.15	46.21	46.26	46.31	46.35	46.39	46.41	46.43
No91	18000		48.14		48.38	48.42	48.46	48.49	48.52	48.55	48.55	48.56
No88	18600					50.54				50.64		
No85	19200		52.44			52.93			52.98		52.93	52.89
No82	19800					56.05						
No79	20400						58.78	58.76	58.69			58.50
No76	21000								61.72	61.51	61.30	
No73	21600					64.85	64.71	64.55	64.33	64.01	63.60	
No70	22200		66.45			65.74	65.40		64.82	64.53	64.25	
No67	22800		69.06		67.93	67.29	66.78	66.37	66.00			64.00
No64	23400		71.04			67.89	67.22	66.70	66.24	*****		
No61	24000		72.43		69.28	69.00			69.00	69.00		
No58	24600		74.49	71.85	71.00	71.00	71.01	71.01	71.00	71.00	71.01	71.02
No55	25200		73.01	73.00		73.00			73.00	73.00		73.00
No52	25800	75.00		75.00		75.00			· · · · · · · · · · · · · · · · · · ·	75.00		75.00
No49	26400		77.00	77.00		77.00	77.00		77.00	77.00		77.00
No46	27000		79.00	79.00		79.00	79.00		79.00	79.00		
No43	27600			81.00		81.00			81.00	81.00		
No40	28200		85.00	85.00		85.00			85.00			85.00

Table G.22River Bed Profile of Bamban River under 1989 Rainfall Condition
Case 2-b; declining sediment supply and dredging works

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Section	distance	l year	2 year	3 year	4 year	5 year	6 year	7 year	8 year	9 year	10year
No181	- 0	16	16	16	16	16	16	16		16	16
No178	600	637	469	444	436	438	445	454	465	475	484
No175	1200	500	460	440	435	439	447	457	467	477	486
No172	1800	366	447	435	435	441	450	460	47 1	480	489
No169	2400	369	429	429	435	444	455	466	476	486	494
No166	3000	431	409	423	437	450	462	474	484	493	501
No163		402	396	420	440	456	470	482	491	500	507
No160	4200	410	385	420	446	465	481	492	501	509	516
No157	4800	357	381	426	456	477	494	505	513	520	526
No154	5400	280	386	440	473	496	514	523	529	535	541
No151	6000	245	406	464	497	520	537	542	- 547	552	558
No148	6600	369	437	495	523	543	557	560	564	567	572
No145	7200	474	493	545	562	572	581	583	585	587	590
No142	7800	603	565	608	606	605	608	608	609	608	609
No139	8400	753	640	670	652	640	636	634	632	629	627
No136	9000	861	712	719	693	673	662	658	655	649	644
No133	9600	840	786	762	731	704	689	683	676	668	661
No130	10200	883	857	803	765	732	716	706	697	686	678
No127	10800	1,081	923	844	797	759	739	728	716	703	697
No124	11400	1,059	951	907	885	869	- 848	835	821	806	795
No121	12000	1,149	1,083	1.031	999	980	958	94 1	925	906	889
No118	12600	1,152	1,110	1,076	1,060	1,049	1,031	1,014	998	980	961
No115	13200	1,259	1,229	1,190	1,169	1,155	1,141	1,123	1,106	1,087	1,067
No112	13800	1,320	1,310	1,287	1,271	1,256	1,244	1,225	1,207	1,188	1,168
No109	14400	1,365	1,372	1,363	1,364	1,360	1,351	1,332	1,313	1,293	1,272
No106	15000	1,412	1,434	1,438	1,453	1,452	1,444	1,425	1,404	1,383	1,361
No103	15600	1,530	1,531	1,530	1,545	1,539	1,532	1,511	1,489	1,466	1,443
No100	16200	1,527	1,545	1,557	1,586	1,588	1,584	1,563	1,541	1,516	1,493
No97	16800	1,727	1,665	1,654	1,682	1,677	1,674	1,653	1,631	1,604	1,581
No94	17400	1,812	1,749	1,716	1,752	1,750	1,753	1,737	1,720	1,696	1,673
No91	18000	1,962	1,742	1,729	1,763	1,758	1,761	1,744	1,726	1,699	1,676
No88	18600	1,931	1,768	1,748	1,772	1,766	1,768	1,750	1,730	1,700	1,677
No85	19200	1,955	1,799	1,783	1,780	1,774	1,774	1,755	1,731	1,697	1,674
No82	19800	1,942	1,847	1,840	1,790	1,782	1,778	1,756	1,724	1,691	1,665
No79	20400	2,073	1,963	1,951	1,830	1,801	1,781	1,749	1,680	1,685	1,632
No76	21000	2,299	2,020	1,963	1,854	1,810	1,774	1,726	1,632	1,690	1,614
No73	21600	1,585	1,046	766	- 575	501	441	411	425	625	714
No70	22200	2,096	1,399	877	519	389	319	265	241	479	467
No67	22800	2,086	1,382	821	456	323	260	212	187	426	96
No64	23400	1,622	1,235	670	287	188	152	115	95	57	39
No61	24000	1,426	1,050	499	112	75	66	37	57	57	39
No58	24600	1,288	838	332	79	75	66	37	57	57	39
No55	25200	872	- 44	79	79	78	76	76	77	77	76
No52	25800	43	76	79	79	78	76	76		77	76
No49	26400	76	77	79	79	78	76	76	77	77	76
No46	27000	79	77	79	79	78	76	76		77	76
No43	27600	79	77	79	79	78	76	76		77	76
No40	28200	79	77	79	79		76	76		77	76

Tabel G.23 Sediment Transport of Bamban River under 1989 Rainfall ConditionCase 2-b ; declining sediment supply and dredging works

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Section	distance	EL in99	1 year	2 year	3 year	4 year	5 year	6 year	7 year	8 year	9 year	10year
N49	0	63.20	62.72	63.27	63.49	63.51	63.51	63.45	63.19	62.90	62.53	62.31
<u>N50</u>	280	64.75	64.30	64.89	65.18	65.24	65.26	65.17	64.74	64.32	63.82	63.47
<u>N51</u>	560	66.30	66.11	66.72	67.02	67.11	67.14	67.01	66.42	65.87	65.26	64.77
N52	840	67.85	67.86	68.45	68.74	68.85	68.88	68.71	68.00	67.34	66.65	66.06
N53	1120	69.40	69.67	70.25	70.56	70.67	70.71	70.49	69.66	68.88	68.12	67.36
N54	1400	70.95	71.48	72.04	72.35	72.48	72.52	72.21	71.22	70.30	69.44	68.49
N55	1680	72.50	73.22	73.76	74.07	74.22	74.28	73.83	72.67	71.63	70.65	69.52
N56	1960	74.05	75.01	75.57	75.85	76.11	76.12	75.49	74.17	72.94	71.75	70.46
<u>N57</u>	2240	75.60	76.82	77.48	77.67	78.17	78.03	77.04	75.50	74.05	72.64	71.23
N58	2520	77.15	78.68	79.69	80.45	80.73	80.49	78.49	76.70	75.01	73.38	71.83
N59	2800	78.70	81.50	82.42	82.98	83.22	82.83	79.77	78.70	78.70	78.70	78.70
N60	2850	80.90	82.05	82.80	83.17	83.35	83.00	80.90	80.90	80.90	80.90	80.90
<u>N61</u>	3075	82.15	83.86	84.40	85.41	85.48	84.45	82.18	82.02	81.97	81.88	81.86
<u>N62</u>	3300	83.40	85.95	86.77	87.47	87.47	85.50	83.40	83.40	83.40	83.40	83.40
N63	3350	85.70	86.43	87.17	87.59	87.52	85.89	85.70	85.70	85.70	85.70	85.70
N64	3575	86.95	88.31	88.88	89.18	89.00	86.66	86.92	86.79	86,74	86.67	86.63
N65	3800	88.20	90.15	91.46	91.45	90.30	88.20	88.20	88.20	88.20	88.20	88.20
N66	3850	90.50	90.68	91.96	<u>91.83</u>	90.71	90.50	90.50	90.50	90.50	90.50	90.50
N67	4075	91.75	92.71	93.59	93.45	91.94	91.82	91.74	91.66	91.58	91.50	91.42
N68	4300	93.00	94.85	95.34	95.11	93.00	93.00	93.00	93.00	93.00	93.00	93.00
N69	4350	<u>95.30</u>	95.31	95.62	95.31	95.30	95.30	95.30	95.30	<u>95.30</u>	<u>95.30</u>	95.30
N70	4575	96.55	97.25	97.32	96.93	96.81	96.71	96.67	96.62	96.54	96.43	96.30
<u>N71</u>	4800	97.80	99.43	98.69	97.90	97.82	97.80	97.80	97.80	97.80	97.80	97.80
N72	4850									100.10		
<u>N73</u>	5075									101.49		
N74	5300									102.60		
N75	5350									104.90		
N76	5475									105.76		
<u>N77</u>	5600									106.37		
<u>N78</u>	5650									111.00		
N79 ·	5900	113.50	113.58	<u>113.50</u>	113,50	113.50	113.50	113.50	113.50	113.50	113.50	113.50
N80	6150		116.02	116.00	116.00	116.00	116.00	116.00	116.00	116.00	116.00	116.00
<u>N81</u>	6400									118.50		
<u>N82</u>	6650									121.00		
N83	6900									123.50		
N84	7150									126.00		
<u>N85</u>	7650									131.00		
<u>N86</u>	8150									136.00		
<u>N87</u>	8650									141.00		
<u>N88</u>	9150	146.00	146.00	146.00	146.00	146.00	146.00	146.00	146.00	146.00	146.00	146.00
<u>N89</u>	9650	151.00	151.00	151.00	151.00	151.00	151.00	151.00	151.00	151.00	151.00	151.00

Table G.24 River Bed Profile of Diversion Channel under 1989 Rainfall ConditionCase 2-b ; declining sediment supply and dredging works

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					(1989 ra	ainfall co	ndition)		(1000 n	13)	
Section	distance	l year	2 year	3 year	4 year	5 уеаг	6 year	7 year	8 year	9 year	10year
N49	0	1,185	1,201	1,257	1,292	1,310	1,307	1,231	1,088	964	838
N50	280	1,166	1,223	1,266	1,293	1,310	1,305	1,221	1,076	950	829
N51	560	1,148	1,247	1,277	1,295	1,311	1,301	1,204	1,060	930	816
N52	840	1,140	1,271	1,289	1,299	1,312	1,296	1,180	1,038	906	796
N53	1120	1,141	1,294	1,301	1,303	1,313	1,290	1,152	1,012	879	773
N54	1400	1,151	1,317	1,313	1,307	1,315	1,281	1,119	982	849	743
N55	1680	1,172	1,339	1,325	1,312	1,316	1,269	1,080	945	815	705
N56	1960	1,200	1,360	1,337	1,318	1,319	1,251	1,034	904	776	660
N57	2240	1,239	1,382	1,348	1,329	1,319	1,226	982	856	729	610
N58	2520	1,287	1,408	1,356	1,349	1,313	1,187	921	799	673	554
N59	2800	1,347	1,449	1,387	1,362	1,301	1,106	851	732	609	493
N60	2850	1,367	1,456	1,394	1,366	1,296	1,082	849	732	609	493
N61	3075	1,403	1,480	1,406	1,371	1,285	1,022	849	732	609	493
N62	3300	1,458	1,497	1,439	1,374	1,251	950	844	731	606	492
N63	3350	1,476	1,503	1,446	1,374	1,234	951	851	736	614	496
N64	3575	1,500	1,527	1,459	1,372	1,195	950	851	736	614	496
N65	3800	1,543	1,545	1,469	1,366	1,120	958	847	734	611	495
N66	3850	1,557	1,554	1,469	1,357	1,128	971	859	744	624	501
N67	4075	1,563	1,595	1,465	1,322	1,122	971	859	744	624	501
N68	4300	1,594	1,622	1,460	1,274	1,118	969	856	742	621	499
N69	4350	1,607	1,626	1,459	1,275	1,127	984	866	748	630	508
N70	4575	1,608	1,638	1,451	1,278	1,127	984	866	748	630	508
N71	4800	1,630	1,640	1,439	1,274	1,123	982	864	745	627	504
N72	4850	1,642	1,636	1,448	1,289	1,146	1,004	882	759	636	514
N73	5075	1,651	1,629	1,448	1,289	1,146	1,004	882	759	636	514
N74	5300	1,684	1,615	1,444	1,287	1,144	1,001	880	758	634	510
N75	5350	1,696	1,617	1,467	1,333	1,178	1,031	903	771	645	517
N76	5475	1,698	1,632	1,467	1,333	1,178	1,031	903	771	645	517
N77	5600	1,719	1,619	1,465	1,333	1,177	1,030	902	771	645	516
N78	5650	1,737	1,608	1,470	1,335	1,197	1,058	923	789	655	521
N79	5900	1,737	1,608	1,470	1,335	1,197	1,058	923	789	655	521
N80	6150	1,740	1,606	1,470	1,335	1,197	1,058	923	789	655	521
N81	6400	1,744	1,606	1,470	1,335	1,197	1,058	923	789	655	521
N82	6650	1,744	1,606	1,470	1,335	1,197	1,058	923	789	655	521
N83	6900	1,744	1,606	1,470	1,335	1,197	1,058	923	789	655	521
N84	7150	1,745	1,606	1,470	1,335	1,197	1,058	923	789	655	521
N85	7650	1,745	1,606	1,470	1,335	1,197	1,058	923	789	655	521
N86	8150	1,745	1,606	1,470	1,335	1,197	1,058	923	789	655	521
N87	8650	1,745	1,606	1,470	1,335	1,197	1,058	923	789	655	521
N88	9150	1,746	1,606	1,470	1,335	1,197	1,058	923	789	655	521
N89	9650	1,748	1,606	1,470	1,335	1,197	1,058	923	789	655	521

Tabel G.25 Sediment Transport of Diversion Channel under 1989 Rainfall ConditionCase 2-b; declining sediment supply and dredging works

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Table G.26 River Bed Profile of Abacan River under 1989 Rainfall Condition
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						(1989 ra	infall co	nditon)		(El.m)		
Section	distance	El in 94	1 year	2 year	3 year	4 year	5 year	6 year	7 year	8 year	9 year	10 year
KP0.0	0	2.50	2.50	2.50	2.50	2.50	2.50	2.50	2.50	2.50	2.50	2.50
KP0.6	600	4.10	3.89	3.91	3.95	3.99	4.02	4.05	4.08	4.10	4 13	4.15
KP1.2	1200	4.46	4.68	4.75	4.82	4.89	4.94	5.00	5.04	5.08	5.12	5.16
KP1.8	1800	5,14	5.24	5.34	5.43	5.52	5.60	5.67	5.73	5.79	5.84	5.90
KP2.4	2400	5.97	5.99	6.12	6.23	6.34	6.43	6.52	6.59	6.67	6.75	6.83
KP3.0	3000	6.54	6.74	6.88	7.02	7.15	7.26	7.36	7.47	7.58	7.68	7.78
KP3.6	3600	7.36	7.57	7.73	7.88	8.03	8.16	8.28	8.42	8.56	8.68	8.78
KP4.2	4200	8.20	8.44	8.62	8.79	8.97	9.12	9.29	9.46	9.59	9.71	9.82
KP4.8	4800		9.32	9.52	9.73	9.96	10.20	10.42	10.58	10.72	10.84	10.93
KP5.4	5400		10.42	10.61	10.82	11.04	11.26	11.47	11.62	11.75	11.87	11.95
KP6.0	6000	11.35	11.51	11.71	11.94	12.17	12.39	12.61	12.77	12.90	12.99	13.05
KP6.6	6600	∦ -	12.58	12.83	13.09	13.36	13.63	13.88	14.06	14.16	14.23	14.25
KP7.2	7200		13.82	14.11	14.41	14.73	15.03	15.26	15.41	15.48	15.49	15.46
KP7.8	7800	+	15.07	15.42	15.75	16.07	16.33	16.52	16.61	16.62	16.56	16.47
KP8.4	8400		16.45	16.84	17.19	17.51	17.78	17.91	17.93	17.85	17.71	17.53
KP9.0	9000		17.90	18.32	18.70	19.02	19.24	19.30	19.19	19.00	18.75	18.49
KP9.6	9600		19.49	19.96	20.36	20.66	20.81	20.73	20.47	20.14	19.75	19.38
KP10.2	10200	· · · · · · · · · · · · · · · · · · ·	21.49	21.98	22.36	22.62	22.64	22.35	21.86	21.36	20.80	20.37
KP10.8	10800		23.66	24.10	24.46	24.62	24.40	23.79	22.99	22.36	22.23	22.23
KP11.4	11400		25.91	26.29	26.62	26.63	26.07	25.04	24.57	24.57	24.57	24.57
KP12.0	12000	1	28.37	28.72	29.00	28.73	27.58	27.05	27.05	27.05	27.05	27.05
KP12.6	12600		30.95	31.24	31.41	30.68	29.79	29.79	29.79	29.79		29.79
KP13.2	13200		33.62	33.88	33.82	32.32	32.13	32.13	32.13	32.13	32.13	32.13
KP13.8	13800		36.28	36.53	35.90	34.68	34.68	34.68	34.68	34.68	34.68	34.68
KP14.4	14400	38.94	39.02	39.18	37.94	37.94	37.94	37.94	37.94	37.94	37.94	37.94
KP15.0	15000	41.86	41.71	41.77	40.86	40.86	40.86	40.86	40.86			40.86
KP15.6	15600	44.67	44.90	44.80	.43.67	43.67	43.67	43.67	43.67	43.67		43.67
KP16.2	1620	48.01	48.42	48.08	47.01	47.01	47.01	47.01	47.01	47.01		47.01
KP16.8	1680	51.93	51.81	51.07	50.93	50.93	50.93	50.93				50.93
KP17.4	1740	56.19	55.45	55.19	55.19	55.19	55.19	\$5.19	55.19			
KP18.0	1800	60.18	59.18	59.18	59.18	59.18	59.18	59.18	1			59.18
KP18.6	1860		1	61.86	61.74	61.74	61.74	61.74	61.74			
KP19.2	1920	0 67.0 2	66.56	66.02	66.02	66.02	66.02					
KP19.8	1980	0 70.28	70.86	69.29	69.29	69.28	69.28		1			
KP20.4	2040	0 74.33	74.95	73.33	73.33	73.33	73.33				-1	
KP21.0	2100	0 79,17	78.34	78.17	78.17	78.17	78.17	7 78.17				
KP21.6	2160	0 84.51	83.51	83.51	83.51	83.51	83.5	83.5				· · · · · · · · · · · · · · · · · · ·
KP22.2		0 88.54	87.54	87.54	87.54	87.54	87.5					
KP22.8		0 94.33	93.33	93.33	93.33	93.33	93.3	93.33				
KP23.4	2340		97.85	97.85	97.85	97.85	97.8	5 97.85	97.85	97.8		
KP24.0			1	102.87	102.87	102.87	102.8	7 102.87	+			
KP24.6				109.40	109.40	0 109.40	109.40	0 109.40) 109.4(109.40	0 109.40	109.40

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						(1989 r	ainfall co	nditon)	(1000 m3)	
ſ	Section	distance	1 year	2 year	3 year	4 year	5 year	6 уеаг	7 year	8 year	9 year	10 year
ľ	KP0.0	0	97	97	97	97	97	97	97	97	97	97
	KP0.6	600	372	321	351	387	423	459	492	521	549	575
	KP1.2	1200	200	339	381	417	452	485	514	541	567	593
ľ	KP1.8	1800	422	411	449	482	511	537	559	581	605	629
	KP2.4	2400	507	495	524	552	576	594	609	629	652	674
	KP3.0	3000	528	607	628	649	665	672	680	702	725	743
ł	KP3.6	3600	698	728	744	759	765	756	768	797	815	827
ł	KP4.2	4200	871	856	\$68	878	871	851	885	915	920	922
·	KP4.8	4800	1,043	984	995	1,002	983	970	1,017	1,032	1,028	1,015
ł	KP5.4	5400	1,078	1,105	1,128	1,144	1,132	1,117	1,147	1,150	1,136	1,106
	KP6.0	6000	1,263	1,237	1,274	1,293	1,281	1,262	1,258	1,247	1,217	1,169
ľ	KP6.6	6600	1,366	1,373	1,425	1,444	1,426	1,411	1,373	1,339	1,284	1,212
Ī	KP7.2	7200	1,254	1,521	1,585	1,603	1,590	1,566	1,491	1,415	1,329	1,229
	KP7.8	7800	1,505	1,706	1,776	1,804	1,786	1,734	1,606	1,466	1,339	1,207
Ì	KP8.4	8400	1,747	1,918	1,976	1,999	1,952	1,850	1,668	1,471	1,302	1,146
	KP9.0	9000	2,081	2,164	2,197	2,204	2,124	1,940	1,681	1,418	1,208	1,031
	KP9.6	9600	2,568	2,419	2,425	2,401	2.261	1,971	1,619	1,296	1,059	872
	KP10.2	10200	2,866	2,708	2,678	2,603	2,359	-1,916	1,447	1,079	818	649
	KP10.8	10800	3,141	3,003	2,916	2,770	2,368	1,731	1,144	775	478	389
	KP11.4	11400	3,400	3,268	3,132	2,869	2,238	1,364	660	389	402	
	KP12.0	12000	3,603	3,501	3,334	2,874	1,895	742	379	389		389
	KP12.6	12600	3,816	3,728	3,521	2,695	1,134	395	379	389		
1	KP13.2	13200	3,930	3,931	3,637	2,185	513	395	379	389	402	
	KP13.8	13800	4,276	4,115	3,599	1,124	375	397	379	389	· · · · · · · · · · · · · · · · · · ·	
	KP14.4	14400	4,640	4,268	3,217	382	375	397	379	389	+	
	KP15,0	15000	4,690	4,374	2,412	383	375	397	379			
	KP15.6	15600	4,608	4,407	1,903	t			379	- 389		
	KP16.2	16200	4,743	4,352	1,231	390			379	389		
	KP16.8	16800	5,002	4,137					379	389		1
	KP17.4	17400	4,929			· · · · · · · · · · · · · · · · · · ·	+		379	389	+	+
	KP18.0	18000	4,307			4				389	· • ······	- · · · · · · · · · · · · · · · · · · ·
	KP18.6	18600	3,703	1		t				1	-	
	KP19.2	19200	3,661	2,897							+	
1.	KP19.8	19800		+	· · · · · · · · · · · · · · · · · · ·					1		
·	KP20.4	20400				1						
	KP21.0	21000		<u> </u>							· [·}
	KP21.6	21600										·
	KP22.2	22200		ł		1						
•	KP22.8	22800				+						
	KP23.4	23400		+		· • · · · · · · · · · · · · · · · · · ·						
	KP24.0	24000	1	1								
	KP24.6	24600	453	460	473	473	47.	473	473	473	<u>473</u>	473

Table G.27 Sediment Transport of Abacan River under 1989 Rainfall Condition

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