CHAPTER 7 DESIGN STANDARD

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Appendix A - 7.1 Materials and Basic Strength

(1) General

The strength of materials to be used for the structural design are shown as follows reference to AASHTO, ASTAM and JIS which refer to the latest revisions of the specifications or method test.

(2) Concrete

The use of each class of concrete and required strengths are shown in Tables A - 7.1.1 and A - 7.1.2 respectively.

Table A - 7.1.1 Concrete Classes and Their Use

Class of	Use of Each Class of Concrete
Concrete	
A - 1.	Precast prestressed concrete box girders
A - 2	Prestressed concrete box girders
A - 3	Precast prestressed concrete I - girders
A - 4	Precast prestressed concrete U - girders
A - 5	Prestressed concrete hollow slabs
A - 6	Precast prestressed concrete hollow core slab units
B - 1	Reinforced concrete slabs and cross beams of prestressed concrete bridges
B - 2	Reinforced concrete for pier columns and cantilevered pier heads except
	for pedestrian bridges, abutment
B - 3	Cast - in - place reinforced concrete piles
B - 4	Precast reinforced concrete pile
C - 1	Piers (except for columns) approach slabs, retaining walls, on Stair
	embankment and foundation of street lighting poles
C - 2	Box culverts (including wing walls) and encasement of pile
C - 3	Curbs reinforced
C - 4	Stair of pedestrian bridges
C - 5	Piers for pedestrian bridges
D	Gravity - type retaining wall, concrete footpaths, head walls curbs (non -
	reinforced) and cradles of pile culverts
E	Leveling concrete, backfield concrete in masonry structure and as
	specified drawings
AA	Prestressed concrete piles

Table A - 7.1.2 Strength of Concrete (UNIT: kg/cm2)

Class of Concrete	Minimum Compressive Strength at 28 Days by Cylinder Test
A1 to A4	400
A5 to A6	346
B1 to B4	290
C1 to C5	210
D	130
E	80
AA	500

(3) Reinforcing Steel

Type, designation and yield strengths of reinforceing steel for concrete structures are specified in Table A - 7.1.3.

Table A - 7.1.3 Reinforcing Steel Strength (UNIT: kg/cm2)

Type	JISG 3	3112	ASTMA	N - 61 5	Distance
	Designation	Yield Strength	Designation	Yield Strength	Standard
Road bar	SR 235	24	Grade 40	28	Applicable
Deformed bar	SD 295	30	Grade 60	42	Applicable

(4) Prestressing Steel

Notation, nominal diameter and yield and breaking Strengths of prestressing steel are as shown in Table A - 7.1.4.

Notation	Utilization	Nonational Diameter (mm)	Yield Strength (kg/cm2)	Breaking Strength (kg/cm2)	Applicable Standard ASTM
PC Wire SWPR1	PC I - girder	Dia.7	13.500	15.500	A 421
PC Wire SWPR1	Diaphragm for PC box girder	Dia.8	13.000	15.000	A 421
PC7 - Wire Strand SWPR7A	PC I girder and hollow slab	Т 12.4	15.000	175.000	A 416
PC 7 - Wire Strand SWPR 7B	PC box girder and PC hollow core slab unit	Т 12.7	16.000	19.000	A 416
PC 19 - Wire Strand SWPR 19	Diaphragm for PC I - girder	T 19.3	16.200	189.000	A 416
PC 19 - Wire Strand SWPR 19	Slab for PC box girder	T 21.8	16.100	187.000	Λ 416
PC bar SBPR 80/95	Diaphragm for box girder PC I - girder	Dia. 23	8.000	9.500	A 722

CHAPTER 8

OPTIMUM SOLUTION OF HIGHWAY NO.18 IMPROVEMENT

Appendix A - 8.1

WORKING PAPER ON ALTERNATE HIGHWAY

JAPAN INTERNATIONAL COOPERATION AGENCY (JICA)

MINISTRY OF TRANSPORT
THE SOCIALIST REPUBLIC OF VIET NAM

FEASIBILITY STUDY ON THE HIGHWAY NO. 18 IMPROVEMENT IN VIET NAM

WORKING PAPER ON ALTERNATE HIGHWAY BETWEEN NOI BAI AND CHI LINH

JANUARY 1996

PACIFIC CONSULTANTS INTERNATIONAL ORIENTAL CONSULTANTS CO., LTD.

FEASIBILITY STUDY ON THE HIGHWAY NO. 18 IMPROVEMENT IN VIET NAM

WORKING PAPER ON ALTERNATE HIGHWAY BETWEEN NOI BAI AND CHI LINH

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FEASIBILITY STUDY ON THE HIGHWAY NO. 18 IMPROVEMENT IN VIETNAM

WORKING PAPER ON ALTERNATE HIGHWAY BETWEEN NOI BAI AND CHI LINH

1. Introduction

1.1 Background

In response to the request of No. 18 Projects Management Unit, Ministry of Transport of the Government, the JICA Study Team has conducted the further study concerning the alternate highway between Noi Bai and Chi Linh.

The main purposes of the further study have been aimed at:

- Comparison of the alternate highway routes which were selected by JICA Study team and Transport Engineering Design Inc., Ministry of Transport (hereinafter referred to as 'TEDI") between Noi Bai and Bac Ninh; and
- Inception study of the alternate highway between Bac Ninh and Chi Linh.

1.2 Necessity of the Alternate Highway and the Government's Highway Development Policy

The necessity of improving the road network in the Red River Delta by providing new highway system (i.e. alternate highway) is primarily due to the recent rapid economic development and forecast drastic increase in future vehicle traffic demand.

To cope with this situation, the Government recently decided to develop an alternate highway in the Ha Noi - Ha Long corridor.

The following future development had been envisaged by the Government in the Ha Noi - Ha Long corridor:

- Major urban development (Ha Noi, Hai Phong and Ha Long);
- Areas of mineral extraction (Uong Bi, Bai Chay, Cua Ong);
- Industrial concentrations (Ha Noi, Pha Lai and Ha Long); and
- Tourism development area (Ha Noi and Ha Long).

2. Basic Concept of Ha Nol - Ha Long Alternate Highway

2.1 Planning of Alternate Highway

The alternate highway network will be developed with a basic understanding that the service of rural collector roads and alternate highways are differ from each other. Vehicles with shorter trips would use rural collector roads and the service by alternate highways would be for the vehicles with relatively longer trips.

2.2 Network of Alternate Highway

Presently the Government is intending to provide the alternate highways in the corridors of the following national highways:

- National Highway No.1 in Ha Noi and its vicinity areas;
- National Highway No. 286 (i.e. Noi Bai Bac Ninh Section of Highway No.18) and;
- National Highway No. 18 between Bac Ninh Ha Long.

2.3 Service Level

High capacity and high speed as on the regional expressway will be maintained for the alternate highways.

2.4 Contribution to Development Projects

The alternate highways will render the services to the new airport, urban and industrial development areas and their vicinities as well as to the projects arising from Ha Long seaboard.

3. Basic Policies of Further Study

3.1 Objective Alternate Highway Section

The study sections comprise:

Noi Bai - Bac Ninh (approximately 33 kilometers); and

Bac Ninh - Chi Linh (approximately 35 kilometers).

3.2 Execution of Further Study

The initial phase of the further study essentially consists of the clarification of the following conditions which were studied in the framework of "Feasibility Study on the Highway No.18 Improvement":

- Socio-economic characteristics;
- Forecast traffic demand; and
- Various design standard.

Two routes, namely Noi Bai - Bac Ninh route (hereinafter referred to as "the West Route") and Bac Ninh - Chi Linh route (hereinafter referred to as "the East Route") will be selected and further the East Route (Bac Ninh - Chi Linh Alternate Highway) will be analyzed comprising the following works:

- Inception engineering study;
- Magnitude project cost estimates; and
- Inception economic analysis.

4. Socio-Economic Characteristics

4.1 Influence Area and Direct Influence Zone

The influence area and direct influence zone of Ha Noi - Ha Long Alternate Highway coincides with the same regions which were established in the "Feasibility Study on the Highway No.18 Improvement".

(1) Influence Area

The influence area is defined as the northern part of Viet Nam which covers twenty provinces. This area can be divided into two distinct sub-regions when described in terms of geographical and socio-economic characteristics. There are:

- The North Mountain and Midland Region; and
- The Red River Delta.

The major characteristics of the North Mountain and Midland Region are a low population density coupled with a relatively low per capita income level, while the Red River Delta has relatively high levels of both population density and per capita income. A third important feature of the influence area is the existence of urban and industrial agglomeration centers in the triangle bounded by Ha Noi, Hai Phong and Quang Ninh.

This triangle, which geographically forms part of the Red River Delta, has been designated for development purposes as "The Northern Focal Economic Area" in view of its high potential for industrial development, its outstanding urbanization rate and existing level of agricultural diversification.

(2) Direct Influence Zone

The Direct Influence Zone (Study Area) is defined as the direct influence area of the alternate highway project which includes such administrative districts as provinces of Ha Noi, Hai Phong, Hai Hung, Ha Bac and Quang Ninh through which the alternate highway passes and its environs. Ha Noi city functions as a primary center of regional activities in the administrative, social, economic, cultural and educational field.

4.2 Northern Focal Economic Area and Red River Delta Region

It is the intention of the Government to establish the Northern Focal Economic Area (NFEA) mainly comprising Ha Noi, Hai Phong and Quang Ninh. Hence, integration of different transport modes within this area as well as integration of the NEFA with the whole Influence Area are important to ensure a smooth transport of cargo and people. The NEFA falls within the same administrative boundaries of five provinces which are included in the Direct Influence Zone.

4.3 Development Plans

(1) Regional Structure in Red River Delta

A number of development centers are recognized as sub-regional growth poles endowed with different economic potentials for industrial, commercial and tourism development in the Red River Delta Region, all of which need to be integrated into the entire regional economy with adequate road transport network.

Still more, in order to encourage foreign direct investment in the industrial sector a number of new industrial zones to accommodate industrial estate projects, including estate planning zone (EPZ) have been planned in the region.

(2) Development Plans in Ha Noi Area

Hanoi People's Committee has a Ha Noi City Master Plan, 2010. Some projects in this master plan have been approved by the Government for the implementation and some developments are ongoing. The most vital issues in the master plan are the road transport network development and new land development in order to meet the demand of recent urbanization and industrialization.

1) A new Urban Structure (2010)

A new urban structure had been formulated together with a planned Ha Noi Ring Road which is expected:

- To provide better access for new urban development areas and industrial zones; and
- To provide land access to Noi Bai international airport.

2) New Industrial Development Areas

New industrial development are planned mainly in the north of Red River, in Thang Long, Soc Son, Sai Dong, Xuan Mai and Hoa Lac (Refer to subsection 7.4 for development projects of industrial areas).

5. Traffic Demand

5.1 Forecast Traffic Demand in Noi Bai - Chi Linh Section

The forecast traffic volumes for the target years of 2005 and 2015 were calculated as described in the Interim Report of the "Feasibility Study on the Highway No.18 Improvement" and are summarized in Table 2 together existing traffic volumes at the year 1995.

Highway Section Noi Bai (2) Year 1995 2,800 5,500 Traffic 1,800 Volume 7,600 2005 14,800 26,500 (PCU/day) 22,700 37,100 2015 58,200

Table 1 Summary of Traffic Volume

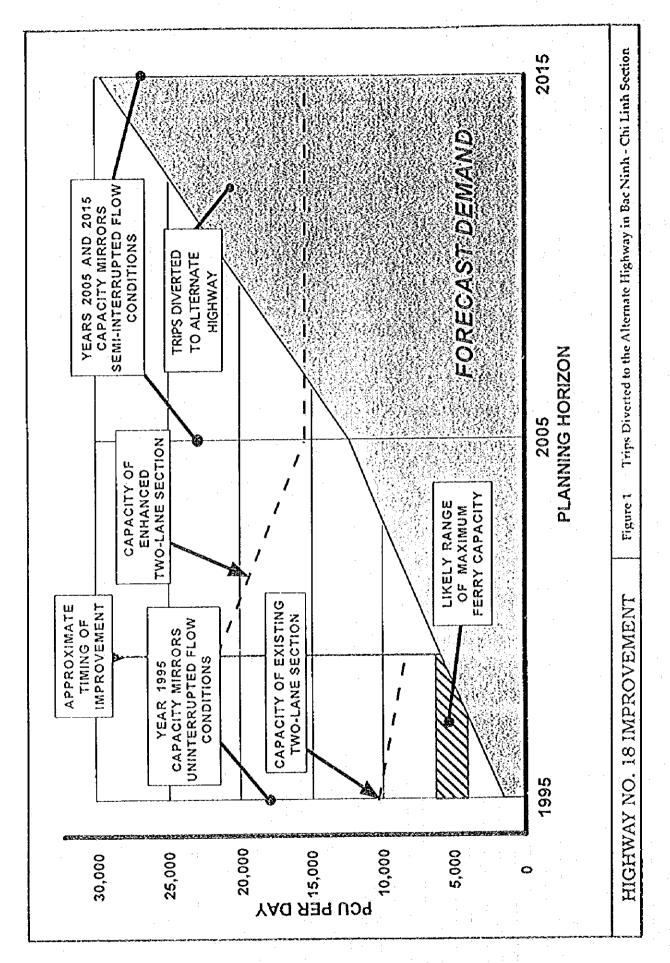
Notes:

- (1) Traffic volume at the West of Bac Ninh
- (2) Traffic volume at the East of Bac Ninh

5.2 Trips Diverted to the Alternate Highway in Bac Ninh-Chi Linh Section

The improvement of Highway No.18 in Bac Ninh - Chi Linh (Section 2) will be improved by adopting 2 - lane 2 - way highway. Once the said section was improved, it is likely that additional capacity by providing alternate highway will not be required until near year 2007 (refer to Figure 1, the diagram shows a concept only).

A - 8.11



5.3 Further Descriptions on Traffic Demand

(1) Noi Bai - Bac Ninh Section

The existing capacity of section 1 (Noi Bai - Bac Ninh) is very low due to the poor state of Highway 286. Following implementation of an enhanced two-lane section, capacity will increase to in excess of 20,000 pcu per day. This should be sufficient for approximately ten years after completion of construction, at which time the limits of semi-interrupted capacity will likely be reached. At that point vehicle trips would be forced to divert to additional highway facilities if levels of service implied by semi-interrupted capacity are to be maintained. The growth rate of demand suggests that additional road capacity (i.e. highway widening) must be in-place near year 2008. In view of conservative future traffic volumes, staged construction approach should be employed in the development to ensure the viability of the project.

(2) Bac Ninh - Chi Linh Section

The 2-lane 2-way improved Highway No.18 will reach to its capacity near year 2007 and will require an alternate highway. The forecast traffic growth suggests the construction of 4-lane divided highway at the initial stage, rather than the adoption of staged construction approach (2-lane 2-way in the initial stage construction and 4-lane divided highway at the final stage), to enhance high speed and to attract more traffic.

6. Outline Design Standard

6.1 General

The Government's standards are to be used to a maximum extent where available. The American and Japanese standards can be referred to the items not covered in the Government's standard.

6.2 Geometric Design Standard

There exist the Government's standard related to the design of new construction, improvement and rehabilitation of public highways, namely "Viet Nam Highway Design Standards, TCVN 4054-85". This standard will be followed for the maximum extent.

An adoption of 120 Km/hr design speed is recommended for the design of Noi Bai - Chi Linh alternate highway (hereinafter referred to as " the Alternative Highway"), taking into account the role of alternate highway system which should have high-speed and high-capacity services. Favorable terrain conditions in the project area will permit high-speed design.

Design speed is the maximum safe speed that can be maintained over a specified section of the Alternate Highway. The design speed will directly affect

many geometric elements i.e. horizontal and vertical alignments, sight distance, provision of super elevation, etc. Other features such as lane width and roadside clearance are influenced by design speed but to a lesser degree.

Further discussions will be made in the Section 8 in this paper.

6.3 Bridge Design Standard

Following is an outline specifications of the bridge design, please refer to the Interim Report of the "Feasibility Study on the Highway No.18 Improvement" for further details.

(1) Loading Specifications

Vietnamese Bridge Design Code (Specifications 2057/QD-KT4-1979) will be followed together with AASHTO specifications.

Design load to be applied is 125% of HS 20-44 (Truck loading) or 125% of HS 20-44 (Lane Loading) whichever produces the maximum stress.

(2) Seismic Design

The Direct Influence Zone is covered by Region 6 or Region 7 in accordance with the Seismic Map of Viet Nam and the adoption of the following acceleration coefficients are suggested.

Seismic Intensity

Scale (region) : 6 7 8
Acceleration Coefficient : 0.04 0.07 0.17

The following table shows the relation among seismic intensity scale, acceleration coefficient and seismic performance category.

Seismic Intensity Scale shown in the Map	6	7	8
Acceleration Coefficient	0.04	0.07	0.17
Seismic Performance Cetegory	Α	. A	В

In the region which falls in the Seismic Performance Category A, no detailed analysis will be required for any bridge structures, except the longitudinal connections of girders must be designed for specified forces.

6.4 Flood and Navigation Clearances

(1) Frequency of Design Flood

The frequencies of design flood will be determined in accordance with Article 1.29 of Vietnam Bridge Design Code 2057/QD - KT4 - 1979 and Article 4.6 of Design Criteria of Highway TCVN - 4054-85. Following table gives the frequencies of design flood by size of bridge.

Size of Bridge	Design Frequency
All sizes of Bridges	
L ≥ 25m	1%
Small Bridges L < 25m	2%

(2) Flood Clearance

The clearances for the design of bridges are given as shown in the following table in case the river is not utilized for navigation (Article 1.27 of Vietnam Bridge Design Code 2075/QD-KT4-1979).

	Structure		Minimum clearance above Design flood level (m) for Highway Bridges
	Water level rising by	Less than 1m	0.50
Girder	influence of piers	over 1m	0.50
	Flooded Material	Wooden logs and debris	1.00
		Roll stone	1.00
Bearing plate			0.25

(3) Navigation Clearances

Classification of waterways (i.e. rivers and canals) shall be determined in accordance with TCVN-5664-1992. The classification of waterways and navigation clearances are to be given in accordance with the following table.

	7	Waterway size				Navigation Clearance			
Class		Natural River		Canal		Curvature	Horizontal		Vertical
	·.	Water	Width of	Water	Width of		Diaman	Const	
ĺ		Depth	Water Surface	Depth	Water Surface		River	Canal	
	I	> 3.0	> 90	>4.0	> 50	> 700	80	50	10
	II	2.0 - 3.0	70 - 90	3.0 - 4.0	40 - 50	500 - 700	60	40	9
	III	1.5 - 2.0	50 - 70	2.5 - 3.0	30 - 50	300 - 500	50	: 30	7
	IV.	1.2 - 1.5	30 - 50	2.0 - 2.5	20 - 30	200 - 300	40	25	6 (5)
	V.	1.0 - 1.2	20 - 30	1.2 - 2.0	10 - 20	100 - 200	25	20	3.5
	VI	< 1.0	10 - 20	< 1.2	10	60 - 150	15	10	2.5

Notes:

1. Figure in () may be applied with approval of official agencies.

2. Horizontal clearance : span length

Vertical clearance : height between water level and bottom of girder.

3. Water depth and width of water surface shown on the above table are based on low water level of waterway with a frequency of 95% in dry season.

6.5 Pavement Design Standard

Presently production of portland cement in Viet Nam is limited therefore the adoption of flexible design will be recommended to attain economical and rapid construction.

The thickness design of the pavement will be carried out based on the "AASHTO Interim Guide for Design of Pavement Structures, 1972". The "AASHTO Guide for Design of Pavement Structures, 1986" will also be used to determine percent of 18-kip ESAL in design lane.

7. Route Study

7.1 Bases of Route Study

(1) Studies of TEDI Concerning Route Location

Pre-feasibility Study for the Ha Noi - Ha Long Alternate Highway (Nghien Cuu Tien Kha Thi, Duong Cao Toc Ha Noi - Ha Long, sponsored by PMU No.18) is ongoing by TEDI.

TEDI's preliminary report on the above study was provided by PMU No.18 and the JICA Study Team interviewed TEDI for their opinions about the route selection of the Alternate Highway.

(2) Socio-Economic Condition

Comprehensive Socio-Economic study had been carried out in the influence and direct influence zone (Study Area) of the Highway No.18 improvement

project, in the framework of JICA Study. Since the influence and direct influence zone of the above Ha Noi- Ha Long Alternate Highway coincides with the JICA Study, the findings of the said study have been reflected to the selection of the optimum route.

(3) Traffic Condition

Traffic demand projected by JICA Study Team was used for the maximum intent (refer to Section 5. Traffic Demand in this paper for further details).

(4) Design Speed

A 120 Km/hr design speed was applied taking into account the role of the Alternate Highway (refer to Subsection 6.2 for further descriptions).

7.2 Basic Data for Route Study

(1) Topographical Maps

Current topographical maps to a scale of 1:50,000 were used to proceed with the route location in the context of the total road network in the project area, to check the topographic conditions along the corridor.

Topographical maps to a scale of 1: 5,000 were available between Noi Bai and about 2 Km West of Pha Lai river and these maps were made use of. The crossings with Song Cau (Cau river) and Song Thuong (Thuong river) were checked and confirmed with 1: 25,000 topographical maps and on-the-ground field investigations.

(2) Information on the Geological Conditions

The findings of JICA Study together with soils survey (i.e. borings and laboratory testing) data are utilized. Geologically, the flat terrain in Noi Bai - Chi Linh area is of alluvium or diluvium formation of Holocence Age or Pleistocene Age composed of alluvial or diluvial soils of gravel, sand, loam, silt and clay.

(3) Hydrological Study

The findings of JICA Study was made use of. To obtain the hydrological information on Cau and Thuong rivers an additional data collection had been conducted.

7.3 Consideration for Environmental Impact

Environmental survey data is available only for the following subcorridors:

- Noi Bai Bac Ninh (Highway No.18 improvement route); and
- Bac Ninh Chi Linh (Highway No.18 improvement route).

Environmental study for Bac Ninh - Chi Linh alternate highway would involve numerous variables which require through and lengthy works. No specific survey or study was carried out in the Further Study, however, the experience gained through the study on the Highway No.18 improvement has been applied as necessary for the comparison of alternative routes.

7.4 Development Projects of Industrial Areas

Information on the size and location of future industrial area will influence the utilization of Alternate Highway. Table 2 shows the summary of future industrial development areas together with possible route to connect Ha Noi with future Cai Lan port or Hai Phong port.

Table 2 Summary of Future Industrial Areas and Possible Route Utilization

Industrial	Major Types of	Area	Possible Route
Zones	Industries		
North Thang	Electric and electronic	300 ha	AH (Noi Bai - Bac
Long	appliances; High-quality		Ninh - Chi Linh)
	construction materials;	11	
	Optical products.		
Soc Son	Electric and electronic	300 -	AH (Noi Bai - Bac
Industrial	appliances/ parts;	400 ha	Ninh - Chi Linh)
Zone	Computer-related		
	industries; Optical		:
	products; Precision		
	machinery; Toys.		
Sai Dong - Gia	Electric products/	500 ha	NH No.1. AH (Bac
Lam	appliances; Packaging		Ninh - Chi Linh)
	industries; Machinery/		
·	parts; Food processing;	-	
	Bevarages.	- 1:	
Xuan Mai	Construction materials;	300 ha	NH No.21, NH
	Sanitary fixtures;		No.1, AH (Bac
	Ceramics.	4 2	Ninh - Chi Linh)
Hoa Lac	Electronic appliances;	1,600	NH No.32, NH
	Precision machinery;	ha	No.1, AH (Bac
THE RESIDENCE DAY OF SHARE SHA	Automobile parts.		Ninh - Chi Linh)

Note: NH and AH denote National Highway and Alternate Highway respectively.

7.5 Selection of the Optimum Alternate Highway Route (Noi Bai - Bac Ninh)

(1) Topographic Condition

Topographic condition is favorable but there are several locations of low laying areas.

(2) Geological and Soils Conditions

The corridor is covered by the alluvium formation of Holocene Age. According to the results of the soils survey carried out by the JICA Study Team, the soil condition is not favorable and bearing strata for bridge foundation piling are situated in the following depths from the existing ground level:

Name of Bridge or Viaduct	Depth of Bearing Strata
Ca Lo Bridge	.35m
Ngu Huyen Khe Bridge	38m
Highway No.1 Viaduct	49m

However, the soft ground layer (N-value less than 4) was not identified in the JICA route.

(3) Hydrological Condition

The corridor develops along the Ca Lo and Ngu Huyen Khe rivers. No flood is recorded in recent years, but there was observed retained water areas in limited locations.

(4) Existing Highway Network

The pattern of the highway network in the corridor is not complicated as far as the route location of the Alternate Road is concerned. The connections with the existing national roads are given full consideration to enhance regional development as well as to obtain the long-term and short-term viability of the Alternate Highway.

The highway links which are important in the route selection are as shown in

the following table:

Name of Highway	Highway Link		
National Highway No.1	Ha Nam - Ha Noi - Bac Giang		
National Highway No. 2	Phu Lo - Viet Tri		
National Highway No.3	Yen Vien - Da Phuc		
National Highway No.286	Noi Bai - Bac Ninh		
Provincial Highway No. 295	Tien Son - Yen Phong - Mai Dinh		
Thang Long - Airport Highway	Thang Long - Noi Bai Airport		

(5) Railway Line

There are two existing railway lines, Ha Noi - Thai Nguyen Line which runs in the north-south direction in Phu Lo and Ha Noi - Bac Giang - Dong Dang which runs parallel with Highway No.1.

(6) Noi Bai International Airport

Various forecasts suggested that the demand at NBIA would reach at 4 to 6 million passengers per annum level by 2005. Thus a necessity arose to apply world class standard for Noi Bai International Airport to meet the requirement compatible to the national gateway airport in the capital. Provision of land access from the east region was took into account in the route selection.

(7) Industrial Development Areas

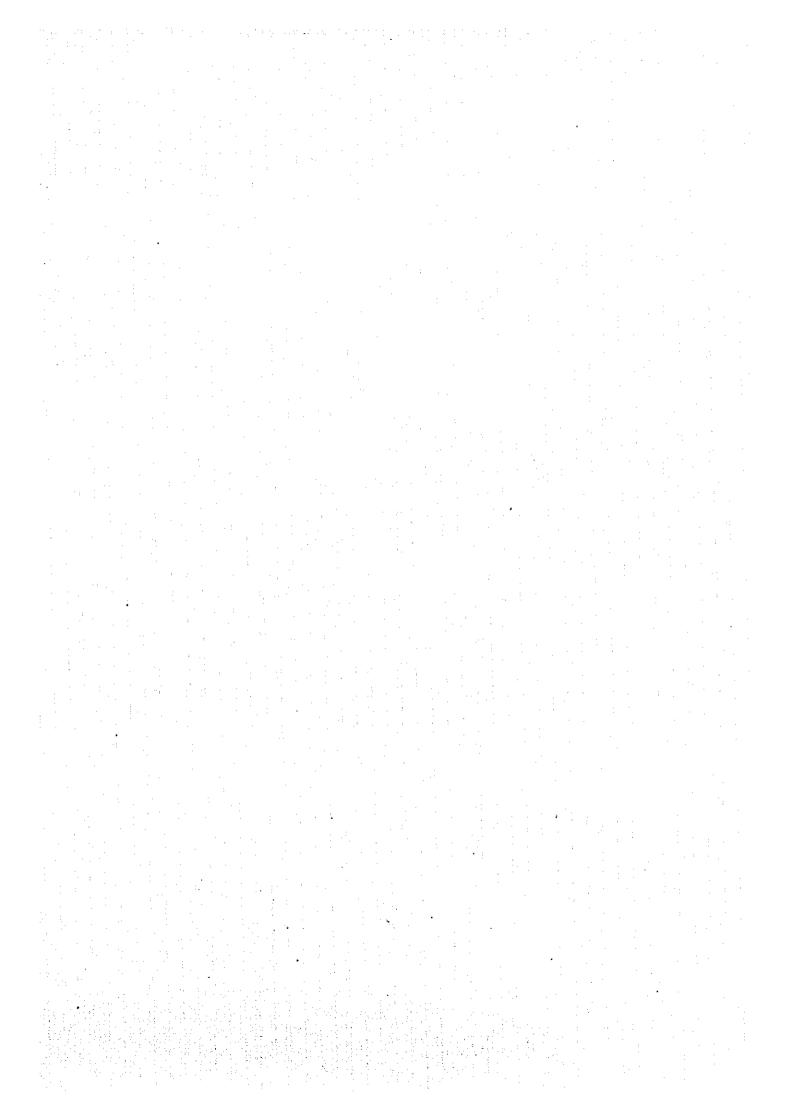
Industrial areas are planned in Ha Noi city and its surrounded areas as discussed in Subsection 7.4. The enhancement of accessibilities to these industrial areas has been considered in the study.

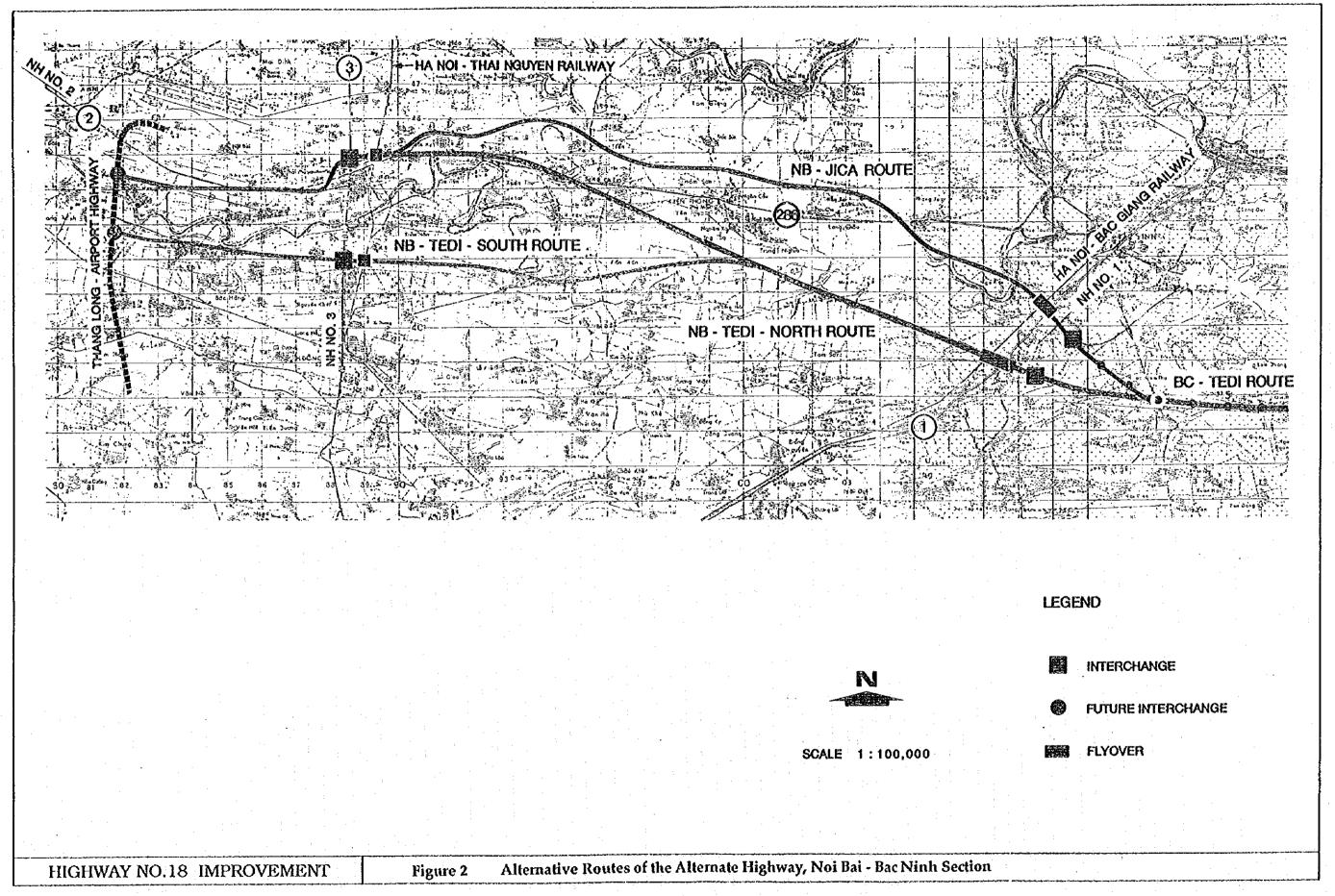
(8) Interchanges

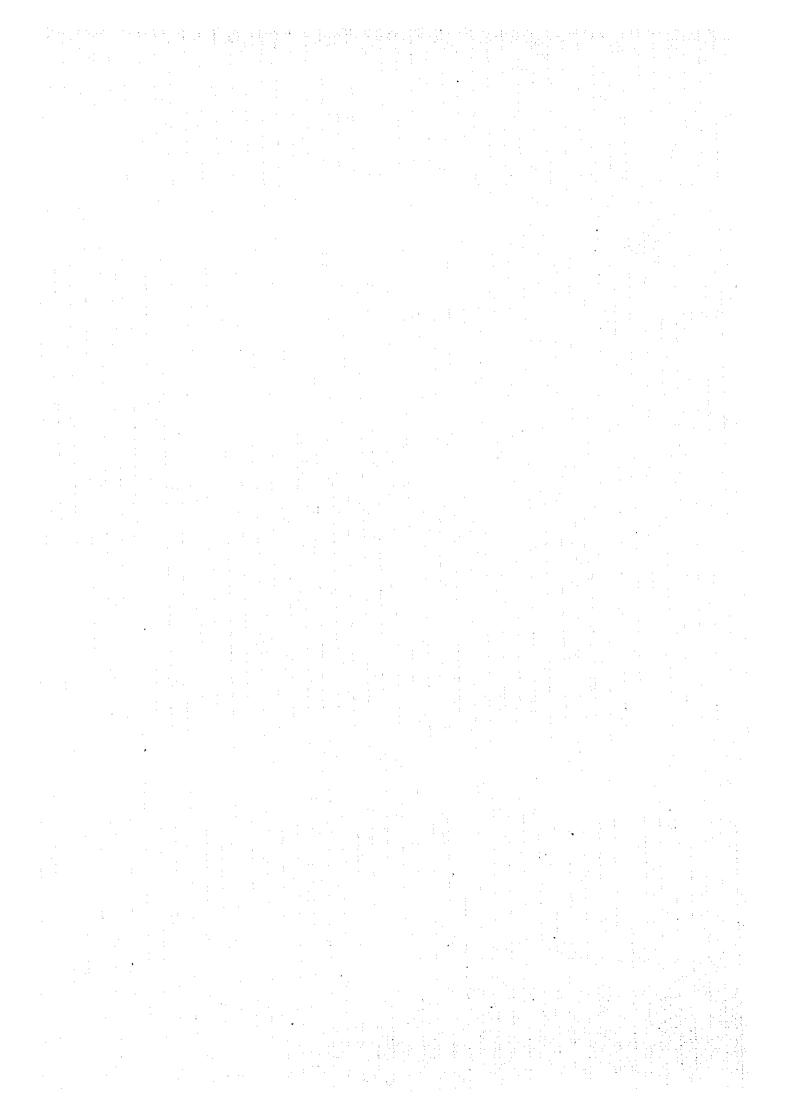
Selection of the locations of interchanges is one of the major governing factors in the route selection of the Alternate Highway. Along the corridor, the connection with Thang Long - Airport highway, national highway No.3 (NH No.3) and national highway No.1 (NH No.1) are the major interchanges to be considered in the route selection. Once the locations of interchanges are determined, the general route alignment of the Alternate Highway is almost fixed.

(9) Description of Alternative Routes

A total of three (3) route alternatives are compared. These alternatives are designated Alternative NB - JICA Route, NB - TEDI North Route and NB - TEDI South Route (Figure 2). The following is brief description of each route alternative.







1) Alternative NB - JICA Route

This alternative is basically identical to the JICA route shown in the Interim Report of the "Feasibility Study on the Highway No.18 Improvement". It aims at less adverse impact on the existing socioeconomic environment and passes through undeveloped areas. The route starts from the existing Thang Long - Airport Highway at the nearest point to connect with Noi Bai International Airport.

Starting at the Noi Bai terminus on Thang Long - Airport Highway, about 0.5 kilometer south of planned airport gate, the route runs to the east and meets the existing NH No. 2 and runs on the NH No.2 for about 3 km. Thereafter the route turns to the northeast, runs in an undeveloped area for one (1) kilometer and then gradually turns the direction to the east. After running about one (1) kilometer to the east again gradually turns the direction to the north east and passes the north of Xuan Ky village for about 1.5 km. In the east of Xuan Ky village there is Kim Lu low laying area. The route avoids this area and passes through the northern edge of the wet land. Thereafter the route runs toward the general direction of southeast by east (SEE) and reaches NH No. 286 near Dong Yen village. After crossing NH No. 286 the route turns the direction to the southeast and crosses Ngu Huyen Khe river, Ha Noi - Bac Giang - Dong Dang railway line/ existing NH No.1 and reaches the planned alternative highway of NH No.1 (Km Post 144).

2) Alternative NB - TEDI - North Route

This alternative route has been established by TEDI together with TEDI - South Route in the framework of "Pre-Feasibility Study on the Ha Noi - Ha Long Alternate Highway Project". According to the preliminary report on the said pre-feasibility study, TEDI subdivided Noi Bai - Bac Ninh section into the following two (2) stretches:

- Noi Bai Dong Mai; and
- Dong Mai Bac Ninh.

In Noi Bai - Dong Mai stretch TEDI assumed that there are two choices of route location namely the north route and the south route (Figure 2).

Noi Bai terminous of Alternative NB - TEDI - North Route is identical with before mentioned Alternative NB - JICA Route and follows to the JICA Route up to Phu Tho village. From Phu Tho village, TEDI's route continues to run eastward up to Xuan Duong (approximately 3 km east of Phu To) and then gradually turns the direction southeasterly to pass through the south of Yen Phong and reaches Dong Mai.

TEDI Selected the location of the overcrossing with NH No.1 at about 5 km southwest of Bac Ninh (Kilometer Post 146) and the above mentioned Dong Mai and the location of the said overcrossing with a straight line

and determined the location of Bac Ninh Interchange (Bac Ninh terminus) on the extension of this straight line.

3) Alternative NB - TEDI - South Route

This alternative aims at the selection of a more straight connection between Thang Long - Airport Highway and Bac Ninh terminus.

Starting at the Noi Bai terminus on Thang Long - Airport Highway, about 2 km south of planned airport entrance, the route runs to the general direction of east for about 18 km and reaches Dong Mai which is the meeting point with TEDI - North Route. The other characteristics of this route alternative are identical to TEDI - North Route.

(10) Selection of the Optimum Route

1) General

Formation of a consensus amongst the government agencies concerned on the development policies of the alternate highway, in particular, to fix an optimum route of the alternate highway and interchange locations, is indispensable. This paragraph contains the comparison of route alternatives on the following bases:

- Technical aspects;
- Environmental impact; and
- Transportation aspects.

2) Technical Aspects

Technical aspects of each alternative are compared in the form of various aspects of i. Length of the Alternate Route; ii. Connection with NH No. 2; iii. Disturbance of existing social settings at the crossing with existing NH No.1; iv. River crossings; and v. Interference to the operation of existing highway system.

i. Length of route

When future widening of NH No.2 to 4-lane was considered, the length of route can be compared with the distance between future Noi Bai Interchange and the crossing location with Provincial Highway No. 288 which is the meeting point of three alternative routes. The route length in the pre-feasibility study generally offers rather shorter distances, since the avoidance of difficult places are not considered seriously. The following is the distances measured on the preliminary design drawings (JICA Route) and on the 1: 50,000 maps (TEDI Route).

Alternative	Route Length (km)		
IICA Route	33		
TEDI North Route	31.5		
TEDI South Route	32.5		

ii. Connection with NH No.2

JICA Route and TEDI - North Routes would provide direct connection with NH No.2 when future interchange will be realized on Thang Long - Airport Highway. In case of TEDI-South route, the widening of Thang Long-Airport Highway to 6-lane for about 1.5 km including the widening of an existing bridge crossing Ca Lo river.

iii. Crossing with NH No.1

The location of existing NH No.1 crossing point was determined based on careful site surveys in case of JICA Route. Roadsides are already developed along the existing NH No.1 at the crossing area of TEDI's Route, therefore the change of route selection may be necessary to avoid adverse social impacts associated with land acquisition and resettlement.

vi. River Crossings

Three (3) alternative routes cross the following two (2) rivers and the situations are considered almost equal to all alternatives:

- Ca Lo river (Bridge widening in case of TEDI-South Route); and
- Ngu Huyen Khe.

v. Interference to the Operation of Existing Highway System

It is noted that Alternative NB - TEDI-North Route will disturb and damage the function of existing NH No. 286 between Phu Tho and Kim Lu Thuong. This problem is serious in case that the Alternate Highway will come to the full operation and charge toll levy, since the Alternate Highway operator must provide frontage road(s) for about 4km stretch to provide other choice to use toll-free road.

Environmental Impact

Since there found no preserved area, important vegetation and wildlife in the direct influence zone, comparison of natural surroundings is omitted and land acquisition and resettlement matters are focused in the comparison. The loss of agricultural land is assumed the same for all alternatives. The following table shows the comparison of alternatives from the viewpoint of adverse social impact.

Description		TEDI-North Route	TEDI-South Route	
Densely/ sparsely	0.3	1.3	2.3	
inhabited villages				

Note: The table shows the villages crossed by each alternative route, in km, measured on the 1:50,000 scale maps.

The comparison of route alternatives from the viewpoint of environmental impact revealed that JICA Route is superior to the other alternatives.

4) Transportation Aspects

i. Road Network

As discussed before JICA Route and TEDI-North Route enable a direct connection with NH No.2 and the entrance of planned Noi Bai International Airport. The terminous of TEDI-South Route is located a little remotely from these points.

TEDI-North Route will disturb the local transportation on the existing NH No. 286 in Xuang Duon area as mentioned in the above.

Consequently, from the road network point of view, JICA Route is preferable to the other alternatives.

ii. Impact on Regional Development

As discussed in Subsection 7.4, North Thang Long industrial area and Soc Son industrial area are major future development projects. Both industrial area will have a similar scale of 300 ha and utilize NH No.3 and Noi Bai - Bac Ninh Alternate Highway to go and come from Future Cai Lan Port.

Since Thang Long industrial area is located in the south of TEDI-South Route and Soc Son industrial area is located in the north of JICA Route the accessibility to these industrial areas will be considered equal in both routes.

iii. Conclusion

Overview of the issues discussed above will conclude that JICA Route is recommendable from the viewpoint of transportation aspects.

5) The Optimum Route

Order of priority to be taken up in technical, environmental impact and transportation aspects among the route alternatives is summarized in Table 3.

Table 3 Preferable Priority in Three Major Aspects for Each Alternative Route

Major Aspects for	Order of Priority to Adopt			
Comparison	JICA Route	TEDI-North Route	TEDI-South Route	
Technical Aspects	1	3	2	
Environmental Impact	1	2	3	
Transportation Aspects	1	2	3	

The JICA Study Team and TEDI strived to satisfy optimally the needs of road users while maintaining the integrity of the environment of each route alternative. However the preferable priority in three major aspects differentiated as seen in Table 3 in the above.

The priority, as a total evaluation for JICA Route, will not be changed even if different weighting factors are adopted for these three elements of technical, environment and transportation, thus the Study Team recommends JICA Route as the optimum route in Noi Bai - Bac Ninh section of the Alternate Highway.

7.6 Selection of the Optimum Alternate Highway Route (Bac Ninh - Chi Linh)

(1) Topographic Condition

There are three (3) kinds of topographic conditions in Bac Ninh - Chi Linh section:

- Favorable flat area between Bac Ninh and Dong Du;
- Low lying wet area in Thai Binh and its tributaries basins; and
- Hilly areas in Pha Lai and east of Pha Lai.

There are several hills between Bac Ninh and Dong Du but these areas can be easily avoided in the route selection and could be utilized as the embankment material sources for the Alternative Highway construction.

(2) Geological and soils Conditions

The corridor is basically of alluvium formation of Holocene Age or Pleistocene Age composed of alluvium or diluvial soils of clay, silt, loam, sand, gravel and sandstone.

As a result of the soils survey conducted by JICA Study Team, the following soils condition was revealed along proposed Pha Lai bridge which was studied for the Highway No.18 improvement.

Boring Location	Depth of Bearing	
	Strata	
West bank of Thai Binh river	10m	
East bank of Thai Binh river	14m	

The bearing strata consists of conglomerated silty sand/gravel or weathered sandstone. No soils data is available at the crossings of Cau and Thuong rivers.

(3) Hydrological Condition

No difficult hydrological condition exists between Bac Ninh and Dong Du. However, in the east of Dong Du, the Alternate Highway must pass flooding areas of the Thai Binh, Cau and Thuong rivers for about 5 km. The Cau and Thuong rivers are the tributaries of the Thai Binh river which diverge to the northwest and northeast respectively at one (1) kilometer north of existing ferry crossing. All these rivers are provided with flood protection dikes but the occurrence of flood in the west of the Cau and Pha Lai rivers is common.

(4) Existing Highway Network

The pattern of the highway network in the corridor is rather simple. The connections with national highways are given full consideration to obtain the long-term and short-term viability of the Alternate Highway. Following highways are considered in the route selection:

Name of Highway	Highway Link
National Highway No.18	Bac Ninh - Ha Long
Provincial Highway No.288	Bac Ninh - Cam Binh
Local Road	Dai Lien - Vu Duong
Local Road	Pha Lai - Vuon Dao
Provincial Highway No.376	Binh Giang - Vuon Dao
National Highway No.183	Hai Duong - Chi Linh
National Highway No.379	Chi Linh - Thai Nguyen

(5) Railway Line

Chi Linh - Pha Lai railway line (spur of Kep - Bai Chay railway line) is the only railway line which affects the route selection of the Alternate Highway.

(6) Interchanges

Interchanges may be provided at Bac Ninh, Chi Linh and near the middle point of Bac Ninh - Chi Linh section. Normally the following standard range is adopted in Japan for the spacing of interchanges:

_	Urban areas, major industrial areas	5 - 10km
_	Flat area with scatterd small towns	15 - 25 km
_:	Rural areas, mountainous area	25 - 30 km

(7) Description of Alternative Routes

According to the preliminary report on the Pre-Feasibility Study, TEDI divided the Bac Ninh - Chi Linh section into two (2) sections, Bac Ninh - Dong Du and Dong Du - Chi Linh, and established South Route and North Route in the said Dong Du - Chi Linh section. As the result of a site investigation these alternative routes are well selected and the JICA Study Team felt that the set up of additional alternative route is unlikely.

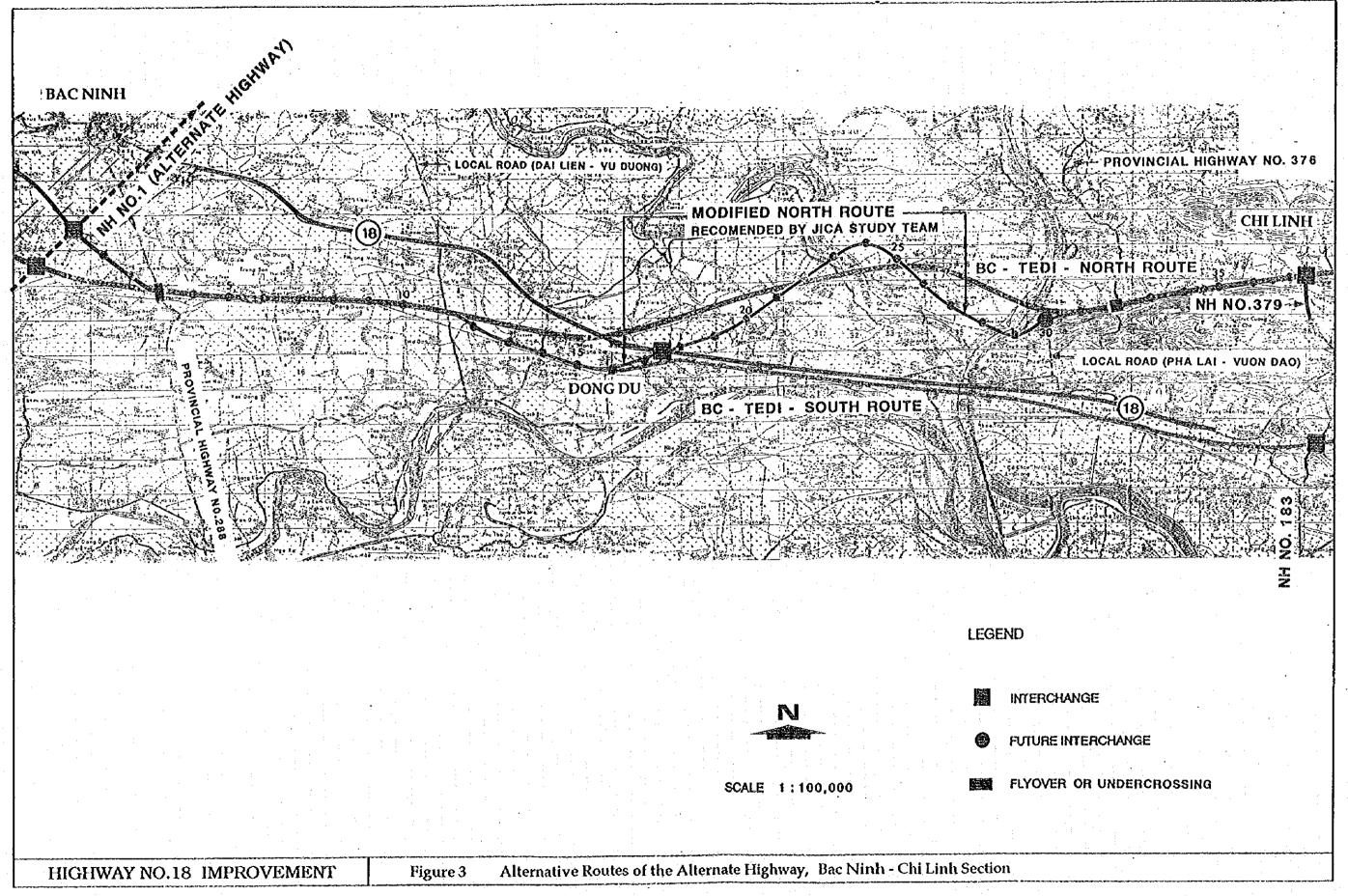
Alternative BC - TEDI - South Route

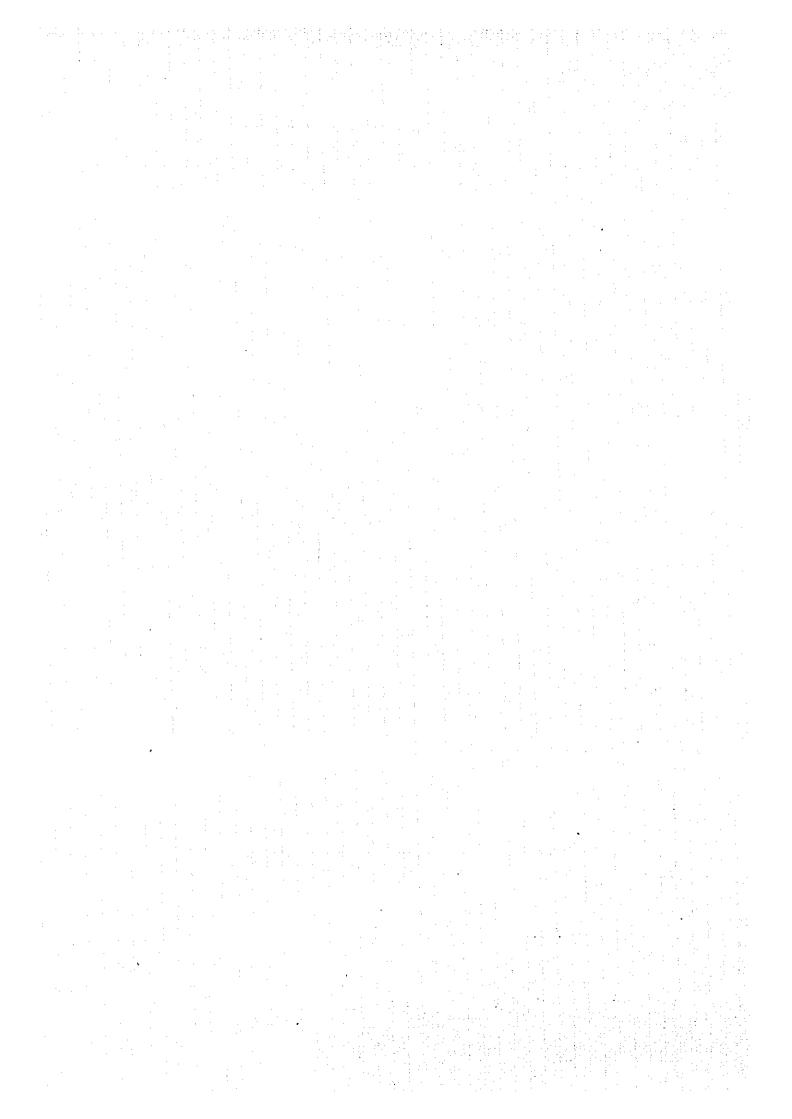
As a highway alignment the South Route offers ideal features. Starting at Bac Ninh Interchange the Route runs straight to the east and directly reaches Chi Linh Interchange which is the terminous of the Bac Ninh - Chi Linh section. Pha Lai Bridge is planned at about 500m down stream of existing ferry crossing.

2) Alternative BC - TEDI - North Route

This alternative route is identical with the South Route between Bac Ninh and Dong Du. From Dong Du, the North Route veers to the east-northeast (ENE) and runs straight for about 7.5 km. Thereafter the route crosses the Cau river and gradually turns the direction to the east-southeast in the Cao Thai area and soon crosses the Thuong river (Figure 3).

After crossed the Thuong river, the route turns the direction to the east-northeast to pass through the northern foot of N. Duong Tau, Nui Doc Nung and Dong Sanh hills and runs about 7 km with fairly straight line and then reaches Chi Linh Terminous where Chi Linh Interchange is located to connect with National Highway No. 379.





(8) Selection of the Optimum Route

General

Formation of a consensus amongst the Government agencies concerned on the development policies of the Alternate Highway is important as mentioned before in Paragraph 7.5 (10).

However, the real comparison and finalisation of the route will require more studies on the various aspects in the similar depth for both alternative routes because of the presence of the Thai Binh, Cau and Thuong rivers and a number of villages and towns in the corridor.

Since the development plan of the Alternate Highway is still in the prefeasibility study phase, many important data for comparison such as soils, hydrological, social environment and detailed traffic data are not in hand at this moment.

A full scale comparison of the South Route and North Route would involve numerous variables which require through and lengthy studies, which is considered to be beyond the control of the Study Team and to be considered beyond the scope of work which was agreed upon between the Government and the JICA Preliminary Study Team in January 1995.

Under such circumstances, the conclusion which are derived from this route study would still be of a tentative nature. The Study Team is leaving room for the discussion of future investigations in the establishment of the optimum route of the Alternative Highway between Bac Ninh and Chi Linh. It is hoped that the appropriate agencies will pick up and continue this endeavor. This paragraph contains the inception comparison which is carried out based on the TEDI's Study, on the following aspects:

- Technical;
- Environmental impact; and
- Transportation.

2) Technical Aspects

Technical aspects of each alternative are compared in the from of various aspects of i. Length of Alternative Route; ii. Interrelation with existing highways; iii. River crossings; and iv. Embankment construction.

i) Length of Route

The Western terminous is Bac Ninh Interchange and the eastern terminous is planned Chi Linh Interchange.

Alternative	Route Length (km)
TEDI - South Route	36.9 km
TEDI - North Route	37.5 km

ii) Interrelation with existing Highways

TEDI Proposed three (3) interchanges for the South Route and four (4) interchanges for the North Route. However, the Study Team considers that an interchange is neccessary also in case of the South Route near Dong Du to enhance the use of the Alternate Highway.

An addition of one (1) overcrossing by provision of box culvert may be considered at about 5 km west of Dong Du (presently a local road for dry season is provided between Dai Lien and Vu Duong).

iii) River Crossings (Bridges)

Table 4 shows the general information concerning Thai Binh (Pha Lai), Cau and Thuong rivers:

Table 4 General Information Thai Binh, Cau and Thuong Rivers

Descriptions	Unit	Thai Binh R.	Cau R.	Thuong R.
Class of River	<u>-</u>	III	Ш	Ш
Water Depth	m	> 4.0	> 4.0	> 4.0
Flood Discharge (Q1%)	m ³ /sec	4,380	3,120	3,450
Flood W. L (H1%)	m	7.94	8.49	7.55
Flood W. L (H5%)	m	7.05	7.56	7.35
Navigation Clearance				•
Horizontal	m	50	50	50
Vertical (above H 5%)	m	7.0	7.0	7.0
High of Dike			,	
East Bank	m	7.7 - 8.0	6.8	N.1
West Bank	m	6.3	N.1	6.2 - 6.7
Subsurface soils at		Fair	N.1	N.1
Bridge Site				

Notes: 1. N.1 denotes no information.
2. R. denotes the river.

TEDI proposed the bridge lengths for the above river crossings in his preliminary report on the Pre-Feasibility Study. The Study Team checked these proposed bridge lengths based on 1: 25,000 scale map and site investigation. Since the maximum embankment height is preferable less than 7m the bridge length can not be determined by the distance between two dikes (east and west) only, but should have about 100m of allowance (50m for each approach behind the dike to keep 7m embankment height) for each river crossing, in case of the North Route.

Considering this situation for the North Route, as well as based on the study result obtained through Highway No.18 Improvement, the Study Team checked the TEDI's preliminary findings (Table 5). As seen in Table 5, it may be said that TEDI assumes considerably shorter bridge lengths compared with the realistic bridge lengths.

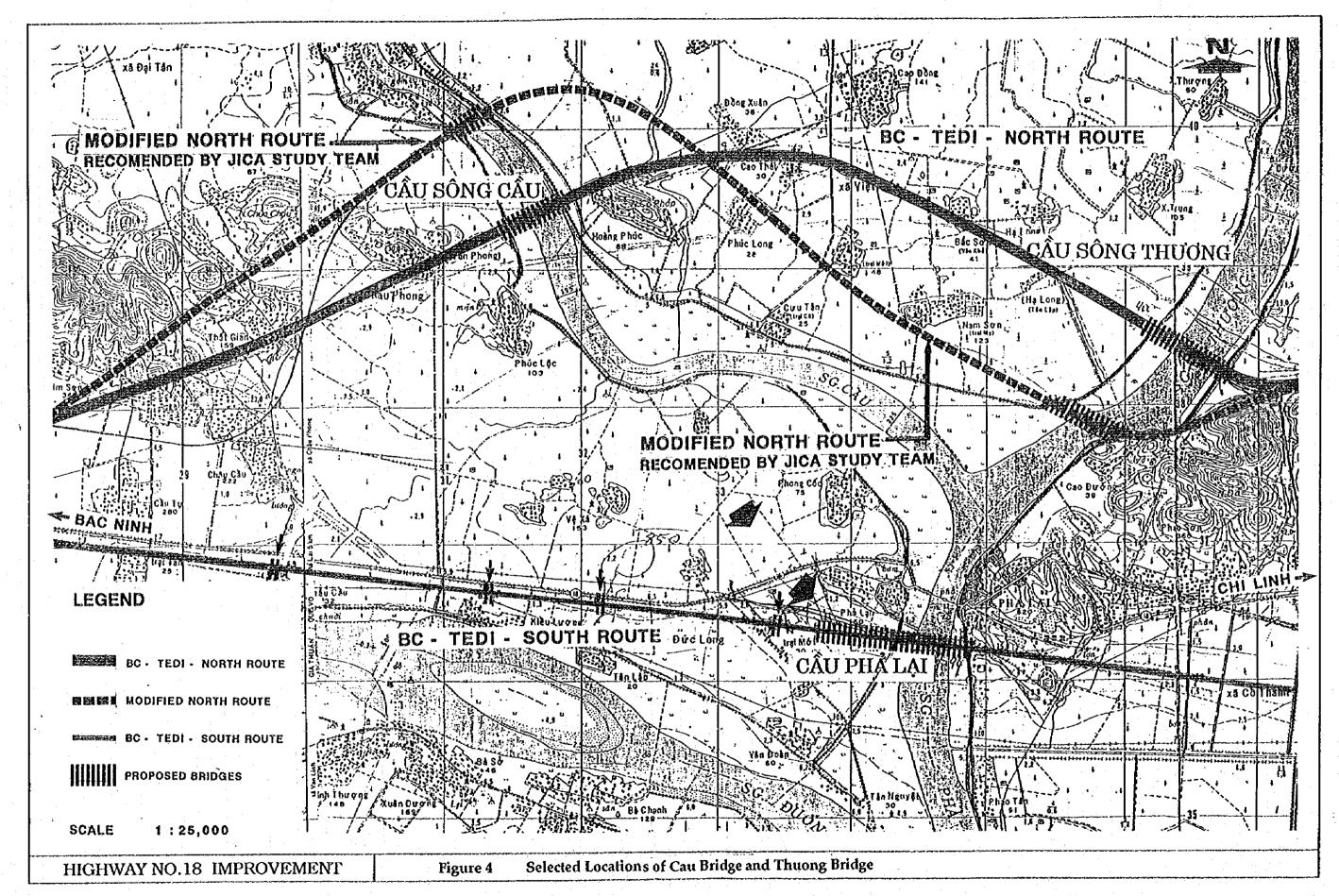
Table 5 Bridge Length by Alternative Route

Unit: in meter

Bridge Name	South Route		No	rth Route
	TEDI's	Checked by	TEDI's	Checked by
	Study	JICA Study	Study	JICA Study
		Team		Team
Pha Lai Bridge (main)	550	580		
Pha Lai Bridge (approach)	300	860		
Cau Bridge			350	500+100=600*
Thuong Bridge			450	600+100=700*
Total Length	850	1,440	800	1,300

* Note : TEDI's bridge lengths have been checked with 1:25,000 scale map and site investigation.

Finalization of TEDI's alignment of North Route was carried out by the Study Team with a site investigation by using an engine boat, to try the reduction of the bridge lengths. Narrower possible river crossings were found at about 1 km upstream of TEDI's Cau Bridge site and at about 1 km downstream of TEDI's Thuong Bridge site (Figure 3). When the bridge lengths were measured on the 1:25,000 scale map the required bridge lengths at new sites would become as shown in Table 6.



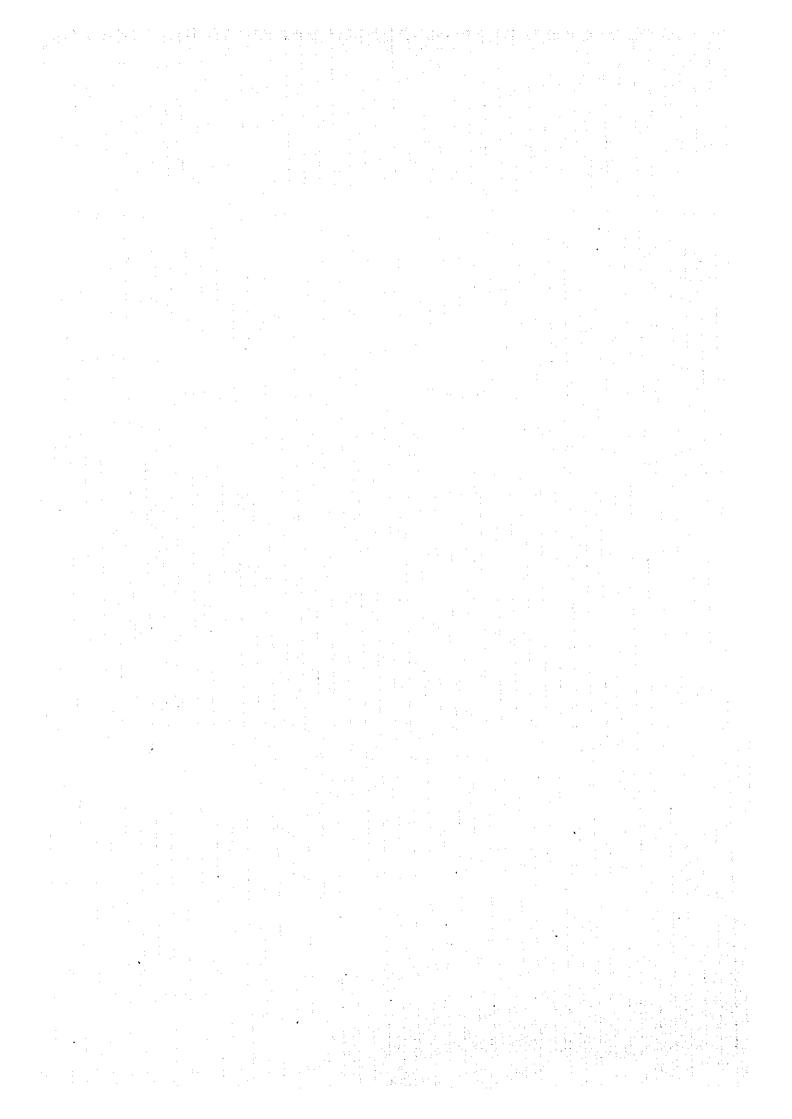


Table 6 Bridge Length by Alternative Route (JICA Study Team)

i .	Un	it : in meter
Bridge Name	South Route	North Route
Pha Lai Bridge (main)	580	
Pha Lai Bridge (approach)	860	
Cau Bridge		350+100=450
Thuong Bridge		500+70=570
Total Length	1,440	1,020
	The second secon	

Note

The Study Team recommends the subsurface soils investigations and on-the-ground topographical survey together with environmental survey (resettlement and existence of historic structures such as temples especially in the east bank of the Thuong river, the northeast of Cao Duong) along the TEDI and JICA Study Team routes to confirm the actual site conditions.

Overview of the issues discussed above will conclude that the North Route is recommended from the view point of river crossings aspects.

iv) Embankment Construction

In the west area of the Cau-Thai Binh rivers axis, the North Route is advantageous compared with the South Route in the following points:

- Avoidance of low-lying area;
- Reduction of hauling distance of embankment materials;
- Contribution to the flood fighting policy (future dike construction plan) of the region; and
- Embankment construction in dry conditions.

Since the Study Team shifted the Cau river crossing to the north, the TEDI's route section between Dong Du and the Cau bridge can be modified as shown in Figure 4.

The North Route will path the Duong Mai - Trai Ma - Bac Luong triangle area which is blocked by the Cau river and the Thuong river. The general ground elevation of this triangle is about 2.0m along the Route and no sustained water in dry season.

The length of an east approach has been reduced to 20m due to the existence of rock outcrops in the area.

The hauling of embankment materials across the Cau river or Thuong river will result expensive disposition, the utilization of pump dredge would result adverse environmental impact by mud flow and this dredging sometimes changes the alignment of river course.

Therefore, the embankment materials should be obtained in the said triangle area. Topographical map shows high spot areas near Ban Tuong, but topographical survey, soils and materials survey and environmental investigations will be necessary during the further steps of the Alternate Highway development. The triangle area is well protected by higher dikes and the embankment work will be possible in dry conditions.

The result of above study suggests an adoption of the North Route from the view point of embankment construction.

3) Environmental Aspects

The most significant environmental condition which affects the planning and design of the Alternate Highway is the social settings especially resettlement of residents and loss of agricultural land. Ha Bac province holds the second large population of 2.26 million in 1993 followed by the province of Ha Noi (2.15 million, 1993) in the direct influence zone of the Alternate Highway. However, when Ha Bac province is viewed from the development of agricultural land, the percentage of agricultural land is low compared with other provinces as shown in the following table:

Province	Agricultural Land (ha)	Percentage of Agricultural Land
Hai Hung	158,702	62.23%
Ha Noi	44,188	48.00%
Hai Phong	68,061	45.27%
На Вас	146,691	31.78%
Quang Ninh	57,364	9.66%

The environmental aspects are viewed from i. Land acquisition and resettlement; and ii. favorable impact on socio-economic development.

i. Land Acquisition and Resettlement

The South Route passes the low-lying wet land between Dong Du and Pha Lai which is sparsely inhabited. While the North Route passes hilly area where many houses are scatterd for about 2 km stretch, therefore, a careful alignment selection is still needed, but the number of affected houses may not exceed around 100.

The North Route passes through rice paddy area between Dong Du and Thuong Bridge. Since the Alternate Highway will adopt high embankment, the loss of agricultural land will be considerably large since the bottom width of the embankment would become of about 100m.

In the eastern area, the South Route passes through the technical irrigated area between Hou Loc and Chi Linh, therefore, the loss of agricultural land in valuable area should be in mind in the case of the South Route.

ii. Favorable Impact on Socio-Economic Development

The South Route will give a favorable impact to the southern flat area of Pha Lai/Hou Loc, and the landuse potential in the area will be greatly enhanced. Higher landuse potential will cause an increase in development demand and thus increase land values.

The major advantages of the North Route from a view point of socio-economic development can be described as follows:

- The construction of the elevated Alternate Highway on high embankment will contribute to the establishment of strengthened flood control system in the region; and
- Development of Duong Mai-Tri Ma-Bang Luong triangle area which is presently isolated from Highway No.18 will be enhanced by the provision of the connections (frontage road) between west dike and east dike for the Cau river and the Thuong river.

4) Transportation Aspects

i) Road Network Pattern and Impact on Regional Development

The western region of Ha Bac province has been suffered from the low density of road network. The reasons are mainly due to the existence of major rivers which cross the region. The Duong river, the Cau river and the Thuong river are flowing in the west-east direction. When the Alternate Highway between Noi Bai - Bac Ninh - Chi Linh was developed, this new highway will greatly contribute to the strengthening of the highway network in the western region of Ha Bac.

In the other hand, when viewed the development of the Alternate Highway from the situation of road network pattern, it is preferable that the subcorridors of the Alternate Highway and Highway No.18 should not be overlapped in the similar subcorridors to cover more larger area to obtain larger impact on the regional development.

Since the South Route follows the alignment of Highway No.18 the adoption of this alternative is doubtful from the stand point of road network pattern.

ii) Traffic Demand

The calculation of detailed average cross sectional traffic volumes and average number of the Alternate Highway users are impossible at this moment but it can be said that there is no significant change in two alternatives from the traffic demand point of view.

iii) Conclusion

Overview of the issues discussed above will conclude that the North Route is recommended from the transportation aspects.

5) The Optimum Route

Order of priority to be taken up in technical, environmental and transportation aspects in the two (2) route alternatives is summarized in Table 7. The basic premises to the comparison is that the Highway No.18 Improvement (including the rehabilitation approach, but with the construction of Pha Lai Bridge) will be executed before the development of the Alternate Highway, since the expected traffic volume on the Alternate Highway will never reach to the presumed level which satisfies the economic viability without abovementioned improvement.

Table 7 Preferable Priority in Three Major Aspects for Each Alternative Route

Major Aspects for	Order of Priority to Adopt		
Comparison	North Route	South Route	
Technical Aspects	1	2	
Environmental Impact	1	2	
Transportation Aspects	1	1	

Both alternative routes are technically feasible and no serious difficulties will be encountered in the construction of the Alternate Highway, but modified North Route showed shorter bridge length of about 1,020m compared with the South Route (total estimated total bridge length = 1,440m).

Land acquisition and resettlement is advantageous in the South Route. However, the North Route will create enormous flood protected area by the construction of high embankment highway in the north-east axis and contributes to the implementation of further regional flood control scheme.

Projected future traffic demand will be similar in both alternative routes but the North Route will provide better road network pattern and larger favorable impact on regional development.

The Study Team recommends the adoption of North Route in the Bac Ninh - Chi Linh section of the Alternate Highway based on the above conclusions of the route study.

8. Inception Engineering Study of Bac Ninh - Chi Linh Alternate Highway

8.1 Geometric Design Standard

- (1) Geometric Standard for Throughway
 - 1) Highway Type

Full access controlled inter-city or inter-region high type highway through sparsely inhabited area.

2) Design Speed

The design speed has a direct effect on the minimum standards of such features as sight distance, horizontal curvature, super-elevation, ect. The higher design speed of 120 Km/hr was adopted for the entire section. Adoption of higher design speed will not seriously affect to the construction cost since the terrain condition is very flat and landuse pattern is simple.

3) Lane Width

A lane width of 3.5m is recommended in lieu of 3.75 which is specified in the Government standard.

4) Shoulder Width

A 3.0m outer shoulder and a 1.25m in inner shoulder are to be adopted considering design policies of the Government and Japanese design practice and AASHTO's design policy.

5) Median Width

The median width id expressed as the dimension between the throughlane edges and includes inner shoulders.

It is recommended to provide a 5.0m median including 1.25m inner shoulders and with 2.5 m width raised. A 2.5m width is generally

sufficient for the construction of pier columns of grade separation structure including space for protection with guardrail.

6) Summary of Geometric Design Standard for Throughway.

The summary of geometric deign standard for throughway is shown in Table 8.

Table 8 Geometric Design for Throughway

					CONTRACTOR OF THE PARTY OF THE
Item	Unit	Throughway Geometric Design Standards			
Standard		Government's	Japanese	AASHTO	Recommended
		Standard	Standard	Standard	Standard
Terrain	-	Flat	Flat	Flat	Flat
Design Speed	km/h	120	120	112	120
Stopping Sight Distance	m	175	210	190	210
Lane Width	m	3.75	3,5	3.66	3.5
Median Width	m	_	4.5 (3)	4.3(1)	5.0(1)
Inner Shoulder Width	m	-	1.25	1.2 (2)	1,25
Outer Shoulder Width	m	3.0	2.5 (3)	3.0 (3)	3.0
Minimum Radii	m	600	710	500	710
Minimum Radius not	m	-	4,000	-	4,000
Requiring Transition Curve					
Minimum Redius not	m	`. -	7,500		7,500
Requiring Super elevation				:	
Maximum Gradient	%	4.0	2.0	3.0	3.0 - 4.0
Crossfall Carriageway	%	2.0	2.0	1.5 - 2.0	2.0
Crossfall of outer Shoulder	%	3.0 (4)	4.0(4)	4.0-6.0	2.0 - 4.0
	l			(4)	
Maximum Superelevation	%	-	6.0	4.0 - 6.0	6.0

Note

- (1) Total of raised median and inner shoulders
- (2) Paved shoulder for 4-lane divided highway
- (3) Usable shoulder
- (4) Gravel or crushed rock shoulder

High type highway requires junctions (alternate highway to alternate highway interchange) and interchanges (alternate highway to national highway or access road) in many cases.

Refer to interim report of the "Feasibility Study on the Highway No.18 Improvement" (hereinafter referred to as the "Interim Report") for the design standard for throughway in the vicinity of junction or interchange ramp terminal.

(2) Geometric Design Standard for Junction or Interchange Ramps

The recommended geometric design standard for junction or Interchange ramps of the Alternate Highway is shown in Table 9.

Table 9 Geometric Design Standard for Interchange Ramps

Item	Unit	Junction or Inter	change Ramps
Design Speed	km/h	40	50
Sight Distance	m	40	55
Lane Width	m	3.5	3.5
Median Width, Raised	m	2.5	2.5
Inner Shoulder Width	m	0.75	0.75
Outer Shoulder Width	m	3.0	3.0
Minimum Radii	m	50	90
Minimum Radius for Curve	m	140	220
not Requiring Transition			
Curve			·
Minimum Radius for Curve	m	800	1,300
not Requiring Superelevation			
Maximum Gradient	%	6 (8)	5.5 (7.5)
Minimum Vertical Curve	m	40	50
Length			
Crossfall of Pavement	%	2	2
Crossfall of Shoulder	%	4	4
Maximum Superelavation	%	10	10

Note: () shows absolute minimum values.

8.2 Typical Cross Sections

(1) Number of Traffic Lanes

4-lane divided highway will be adopted for the Alternate Highway.

(2) Cross Section Elements

The major cross sectional elements are:

- Number of lanes;
- Lane width;

- Widths of shoulders;
- Median;
- Side slopes; and
- Drainage channels.

(3) Side slopes, Drainage Channels and Erosion Control

1) Side Slopes in Embankment

Site conditions do not permit the use of flat or gentle slopes. A side slope of 1:1.5 will be generally adopted in the design.

2) Side slope in cut section

A back slope of 1:1.0 will be adopted based on evaluation regarding the slope stability (i.e. silty sand), and normal highway construction practices in the region.

3) Drainage Channels

The design incorporates safety, and economy of maintenance. Provision of intercepting ditches is considered in the design where long distance overland flow is anticipated.

4) Erosion Control

Rainwater which falls on the traveledway flows away laterally under the influence of the cross slope or superelevation. A common rule for drainage where the traveledway are situated on earth embankment is to let the flow continue off the shoulders and down the side slopes to the side ditches. Protection with sodding (i.e. strip sodding in the construction) will be also adopted in the design. Cut slopes are designed in similar manner.

(4) Typical Cross section and Right-of-Way Width

1) Clear Area and Right-of-Way

The Decree 203-HDBT dated 21-12-1982 indicates a right-of-way (ROW) of 20 meters for highway from the lower edge of fill or from the outer edge of longitudinal ditches.

The Circular, MOTAC 704/GTTB dated 16-06-199 stipulates that the edge of clear area shall be 7 meters from the toe of embankment or the end of cut slope to consider traffic safety and future upgrading and pavement widening.

Any kind of houses and buildings within clear area shall be demolished and compensated. Farmers will, however, be able to continue to cultivate in the clear area.

The existing houses and buildings in the area between clear area and right-of-way edge lines will be maintained as they are, but new houses or buildings will not be allowed to be constructed.

2) Typical Cross Sections

The typical cross section of the Alternate Highway has been proposed as shown in the Interim Report (Figure 8.2(1), Type D).

(5) Special Considerations

1) Cut Area at That Phong

As before-mentioned (7.6. (8).2).iv.), the Alternate Highway will path Van Phong, Xa Chau Phong and Xa Ngoc Xa low-lying area with high embankment, and borrow materials are intended to obtain in That Giang hilly area (Figure 4). Since a large volume of borrow materials is necessary for the embankment construction, it is beneficial to select the route which passes the hilly areas which will create enormous cut volume. It is recommended to adopt ultimate 6-lane Alternate Highway (Figure 5) in that hilly areas based on the following reasons:

- To provide enough volume of borrow materials; and
- To avoid the future difficulty in the highway widening.

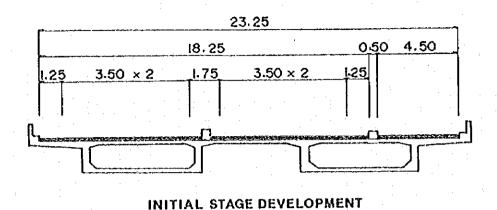
2) Cau and Thuong Bridges

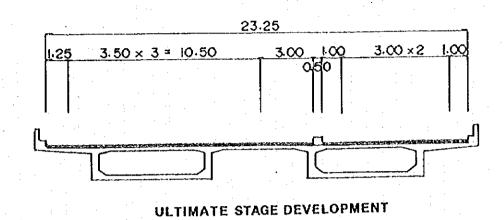
The Alternate Highway is envisaged 6-lane in the ultimate stage. Once 4-lane bridges were constructed for the Cau river and Thuong river, the major difficulties would occur in the future in the widening of these bridges.

The ideal Alternate Highway development is to provide 6-lane bridges and 4-lane paved road in the embankment sections in the initial stage but the economic viability situation will not justify this method. In the other hand the PMU No.18 suggested the provision of an additional 4.5m wide carriageway (i.e. side lane) to the Cau and Thuong bridges for the utilization by local traffic.

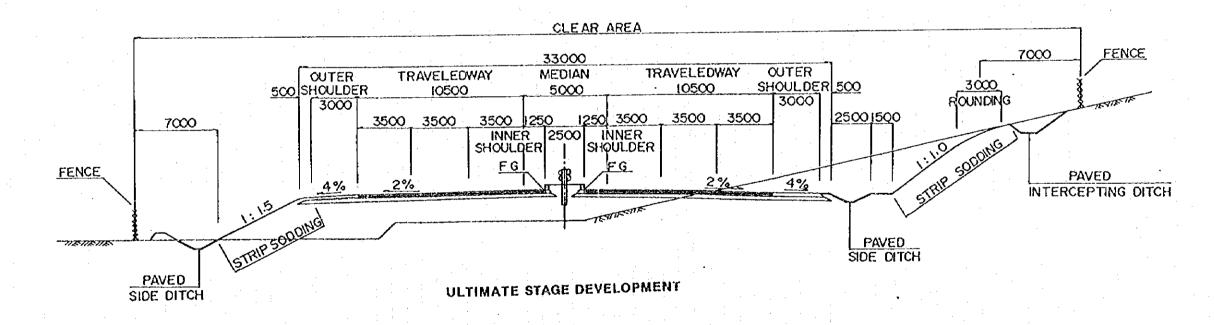
To cope with the above situations the Study Team decided to recommend the following countermeasures:

Planned bridges in the ultimate stage status will be constructed for Ha Noi bound only (3-lane carriageway) in the initial stage;

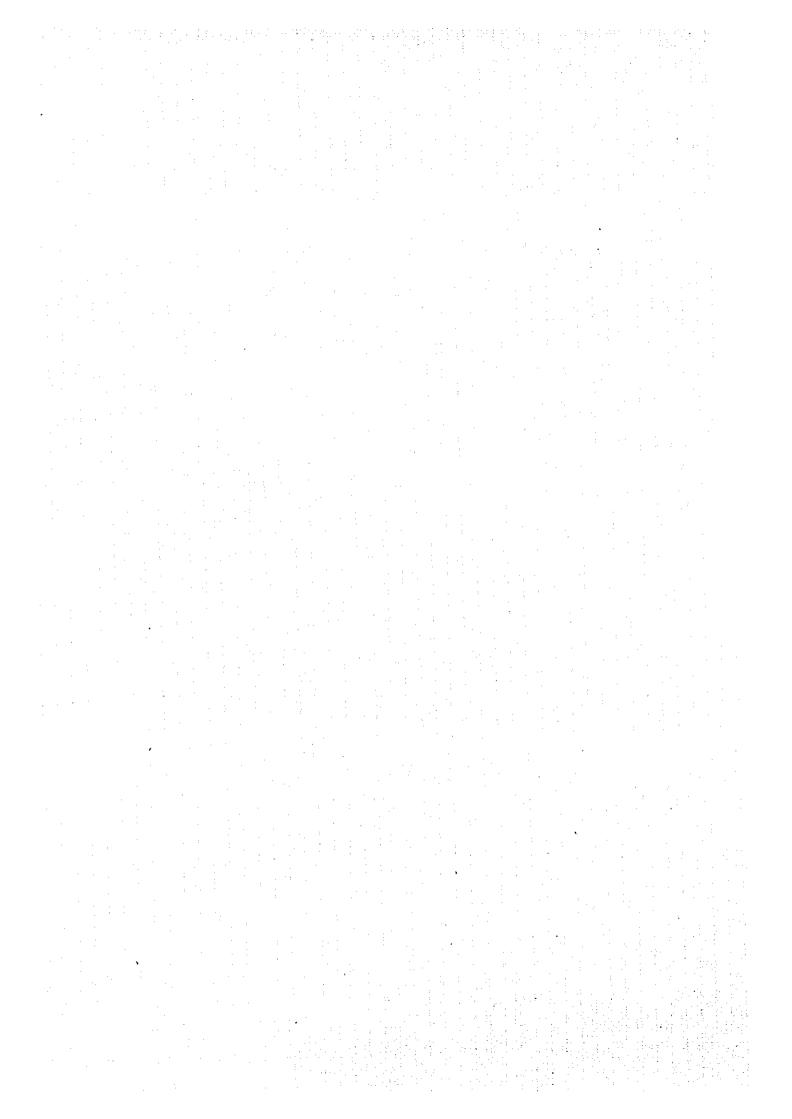




TYPICAL CROSS SECTION (CAU BRIDGE AND THUONG BRIDGE)



TYPICAL CROSS SECTION (EARTH WORK SECTION)



- A 4.5m wide side lane will be provided at the outside of the Alternate Highway carriageway in the initial stage;
- Constructed Ha Noi bound bridges will be operated as a 4-lane divided highway applying slightly lower design speed of 100 km/hr in lieu of 120 km/hr until the ultimate bridges (additional 3-lane carriageway) will be realised;
- The total width of 3-lane carriageway including shoulders in normal design is 14.75m and this carriageway width is not sufficient for 4-lane operation which is able to keep 100 km/hr and traffic safety, therefore, the initial stage Alternate Highway carriageway shall have 18.25m of width (Figure 5);
- Since an extra width of 3.5m (18.25m 14.75m = 3.5m) will be provided for the Alternate Highway carriageway in the initial stage, these extra width can be transferred to the 4.5m side lane after the realization of ultimate stage development of bridges, the total width of the side lane will become 8.0m and 2-lane 2-way operation will become possible.

The recommended Alternate Highway typical cross section for ultimate stage in the embankment or cut section is shown in Figure 5.

8.3 Interchanges

(1) General

Interchanges at the ultimate status are tentatively planned at the crossings with National Highway Nos.1 (alternate highway), 18 and 379 and Provincial Highway No.376 which connects Binh- Giang - Vuon Dao to collect/distribute the Alternate Highway traffic from/to the highway network in the corridor, normally with the consideration given to the provision of possible toll gate.

(2) Toll Levy System

TEDI's study suggests the toll levy for the Alternate Highway. Therefore the Study Team will follow this basic policy. It can be understood that basically the Alternate Highway will be operated under a distance-proportional toll levy system.

(3) Location of Interchanges

Four (4) interchanges (Table 10) were planned as mentioned in the above, however really needed and important interchanges in the initial stage are Bac Ninh Interchange and Chi Linh Interchange.

Table 10 List of Interchange

No.	Name of Interchange	Distance (km)	Connection
1.	Bac Ninh	16.0	Alternate NH No.1
2.	Dong Du	14.5	Highway No.18
3.	Vuon Dao	7.0	Provincial Highway No.376
4.	Chi Linh		National Highway No.379

Justification of Vuon Dao interchange is doubtful since the normal intervals of interchange in toll levy highways under a distance-proportional toll levy system in Japan suggests the following:

Standard Interval of Interchange

- Urban areas, major industrial areas...... 5 10km; and
- Flat area with scattered small towns 15 25km.

Dong Du interchange requires a considerable amount of initial investment, therefore only the at-grade intersection with toll gate may be provided in the initial stage.

(4) Selection of Type of Interchange

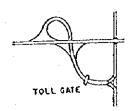
There are several basic types of interchange and patterns of ramps for vehicle maneuvering at a grade separation, and each type of interchange has inherent advantages and disadvantages. The Study Team selects a trumpet type of interchange considering volume of traffic, topography, availability of land, location of tollgate and construction cost.

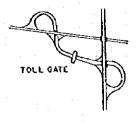
In consideration of the toll levy system for the connection between a toll road and a toll free road, trumpet interchange has a definite advantage as compared with other types of interchange.

This advantage is to be able to gather all toll plazas into one place, and thus, the toll levy administration is to be easy and economical. Therefore, trumpet type interchange has been adopted for all interchanges in consideration of the inherent advantages to a trumpet mentioned above.

(5) Layout of Interchange

To manage larger on/off ramp traffic, a double trumpet type may be selected for Bac Ninh interchange. But for Chi Linh interchange the construction of second trumpet will not be necessary in the initial stage development but the securing of right-of-way should consider the ultimate stage development.





Double Trumpet Type (Initial Stage)

Single Trumpet Type (Ultimate Stage)

(6) Toll Collection Facilities

Under the distance-proportional toll levy system, it is necessary to provide toll gates, on-ramp gates for issuing cards and off-ramp gates for collecting toll. The type of interchange which provides integrated on/off ramp gates at one locations is advantageous as it is economical and efficient for toll levy highway operation and management. The type which allows such arrangement is normally a trumpet type, single or double trumpet depending on the volume of on/off traffic which has been applied for all the interchanges.

The major roles of toll collection facilities are collection of the distanceproportional tariff and recording the number of vehicles by class. The functions of each tollgate and toll office are as follows:

Tollgate

- Toll collection and issue of receipts;
- Vehicle counting by loop detectors, where required; and
- Registration of the number and types of vehicles.

Toll Office

- Total and check the toll account daily;
- Total the number and types of vehicles daily;
- Check for mis-registration and discrepancies; and
- Real time audit.

(7) Tollgate

Since the forecast on/off traffic are not available at this phase of the study, the further discussions on number of toll booth are omitted.

8.4 Major Bridges

(1) Design Standard

Vietnamese Bridge Design Code (Specifications 2057/QD-KT4-1979) will be followed together with AASHTO specifications. Refer to subsections 6.3 and 6.4 for further descriptions on bridge design standard.

(2) General Information on the Cau River and Thuong River

Refer to subparagraph 7.6 (8).2).

(3) Design Features of Major Bridges

Design features of highway crossing and river crossing are summarized in Table 11.

Table 11 Design Features of Highway and River Crossing Bridges

No.	Approximate Station (km)	Name of Bridge	Total Length (m)	Bridge Width (m)
1.	0.5	Bac Ninh Interchange	40	14
2.	. 3	PH No.288 Under crossing	80	12
3.	14	Drainage Channel No.3	30	11.25x2=22.5
4.	15.5	Dong Du Interchange	40	12
5.	17.5	NH No.18 Under crossing	250	12
6.	23	Cau River	450	23.25
7.	28	Thuong River	570	23.25
8.	30	Vuon Dao Interchange	40	12
9.	32	PH No.376 Crossing	30	11.25x2=22.5
10.	37.5	Chi Linh Interchange	20	11.25x2=22.5
NED 201-73		Total Area of Bridges		30,996m2

8.5 Pavement

Adoption of an uniform layer thickness for the Alternate Highway will be recommended, although traffic characteristics varies by each subsection.

(1) Design Conditions

- Pavement design

Pavement design Strategy

- Service life of the first stage

Flexible pavement design

Two stages

10 years

_	Service life of second stage	10 years
-	Soil support value	3.7 (=CBR 4)
	Number of lanes	4
	ESAL in the first stage	7.4x 10 ⁶
	Regional factor	2.5
: -	Terminal serviceability	2.5

(2) Design Thickness

Table 12 shows the results of flexible pavement design for the first stage.

Table 12 Result of Flexible Pavement Design of the Alternate Highway

	Design Features
Service Life (years)	10
Surface Course Thickness (cm)	10
Asphalt Treated Base Course Thickness (cm)	20
Aggregate Subbase Course Thickness (cm)	30

An overlay of pavement (10cm thickness) will be required after 10 years of the opening of the Alternate Highway (2016).

9. Magnitude Project Cost

9.1 General

The estimates of the project cost were based on the result of the inception engineering study. The project cost which dealt in this Section consists of the following items:

- Construction;
- Land acquisition and resettlement;
- Engineering services;
- Supervision services; and
- Physical contingency.

The unit price of each cost component was determined based on the economic conditions prevailing in January 1996. The project cost was estimated in financial and economic prices.

9.2 Construction Cost

(1) Initial Investment Cost

The estimated construction cost at January 1996 prices is shown in Table 13. 13.

(2) Overlay Cost

For overlay of asphalt concrete pavement, it is assumed that the overlay will be executed, once life span of the project, in 2016. The cost was estimated for the purpose of economic project evaluations. The estimated construction cost is Dong 84,869 million.

9.3 Land Acquisition and Resettlement Cost

No tax component is included in the land acquisition and resettlement cost. The cost was estimated based on the data of the "Feasibility Study on Noi Bai - Bac Ninh Highway, by TEDI, in 1994". To consider the ultimate stage status of the Alternate Highway, TEDI's cost was doubled and estimated at Dong 55,270 million.

9.4 Estimated Project Cost

(1) Initial Investment Cost

The estimated initial investment cost is Dong 1,295,500 million in financial cost as summarized in Table 14.

(2) Additional Investment Cost

Only an overlay cost above mentioned was considered in the estimates.

Table 14 Summary of Project Costs in 1996 Prices

	Description	l'inancial Cost
 		(Million Dong)
(1)	Construction	1,018,406
(2)	Land Acquisition and Resettlement	55,270
(3)	Engineering Services	42,947
(4)	Supervision Services	61,104
(5)	Physical Contingency (10%)	117,773
	Total	1,295,500

Table 13 Estimated Construction Cost, in January 1996 Prices

Alternate Highway between Bac Ninh and Chi Linh

Item	Unit	Unit Cost (Dong)	Quantity	Total (1000 Dong)
I) General	LS			29,662,31
	,			
2) Earthwork			. =	
Clearing & Grubbing	sq.m	2,000	1,760,000	3,520,00
Soil Excavation	cum	18,000	0	
Rock Excavation	, çu.m	120,000	0	
Embankment (Borrow Material)	cu.m	45,000	1,645,103	74,029,63
Embankment (Borrow Material)	cu m	40,000	3,829,126	153,165,04
Sand Mat (t=50cm)	sq.m	45,000	0	
Sub Total		*		230,714,67
3) Pavement				
Removal of Existing Asphalt Pavement	cum	90,000		
Subgrade Preparation	sq m	6,000	882,156	5,292,93
Aggregate Subbase Course	cu m	100,000	317,625	31,762,50
Asphalt Treated Base Course	cum	750,000	118,180	83,635,00
Prime Coat (1.0 liter/sq.m)	sq.m	5,000	801,456	4,007,26
Tack Coat (0.5 liter/sq.m)	sq.m	3,300	1,266,708	4,180,13
Asphalt Concrete Surface (t=5cm)	sq.m	55,000	. 0	
- ditto - (t=10cm)	sq.m	110,000	733,536	80,688,96
Concrete Side walks	sq.m	140,000	0	
Overlay (t=10cm)	sq m	130,000	0	ì
Sub Total				214,566,8
Coo Food				
4) Drainage Structures				
Pipe Culvert (D=0.8m)	l.m	1,200,000	83	99,60
Pipe Culvert (D=1.0m)	l.m	1,700,000	935	1,589,50
Pipe Culvert (D=1.5m)	Lm	2,700,000	327	882,9
Box Culvert (1.5 x 1.5)	l.m	3,400,000	277	941,8
- ditto - (4.0x 2.5)	l m	30,000,000	277	8,310,0
Street Pipe Drain	l.m	1,500,000	0	
Street U-Ditch	l.m	500,000	0	
Sub Total	1			11,823,8
Sub i Otal			:	.,,,,
(5) Bridges	63 m	500,000	0	
Removal of Existing Bridge	sq.m	6,000,000	o	٠.
RC Slab Bridge (L<10m)	sq.m	8,000,000	0	
- ditto - (with Piling)	sq.m		. 0	
RC Hollow Stab Bridge (10m <l<20m)< td=""><td>sq m</td><td>8,000,000</td><td>0</td><td></td></l<20m)<>	sq m	8,000,000	0	
- ditto - (with Piling)	sq m	10,000,000	0	
RC T-Beam Bridge (10m <l<20m)< td=""><td>sq.m</td><td>8,000,000</td><td>v_o</td><td>1</td></l<20m)<>	sq.m	8,000,000	v _o	1
- ditto - (with Piling)	sq m	10,000,000	0	;
PC I-Beam Bridge (20m <l)< td=""><td>sq m</td><td>11,000,000</td><td>0</td><td>212.070.0</td></l)<>	sq m	11,000,000	0	212.070.0
- ditto - (with Piling)	sq.m	13,000,000	24,020	312,260,0
Long Span Bridge	sq.m	26,000,000	6,975	181,350,0
Sub Total				493,610,0
		:		1 1
(6) Miscellaneous Works	L.S	<u> </u>		38,028,6
	1	· I		} ·

10. Implementation Schedule

Assuming realization of Highway No.18 improvement, it is recommended that the construction of the Alternate Highway be completed by the end of 2006 before the capacity of Highway No.18 will be saturated.

Taking into account the scale of the construction and the urgency of the Alternate Highway development, the maximum possible construction period was set at 2.5 years (Figure 6).

The Alternate Highway project implementation schedule has been prepared as shown in Figure 3, assuming the following implementation schedule:

Feasibility Study and Detail Design	2001 - 2003 (2.5 years)
Detail Design	
Land Acquisition/ Resettlement	2003 - 2004 (1.5years)
Construction	2004 - 2006 (2.5 years)

11. Inception Economic Analysis

The Alternate Highway development will have to alternative cases:

Scheme - 1: Alternate Highway development with the improvement of Highway No.18.

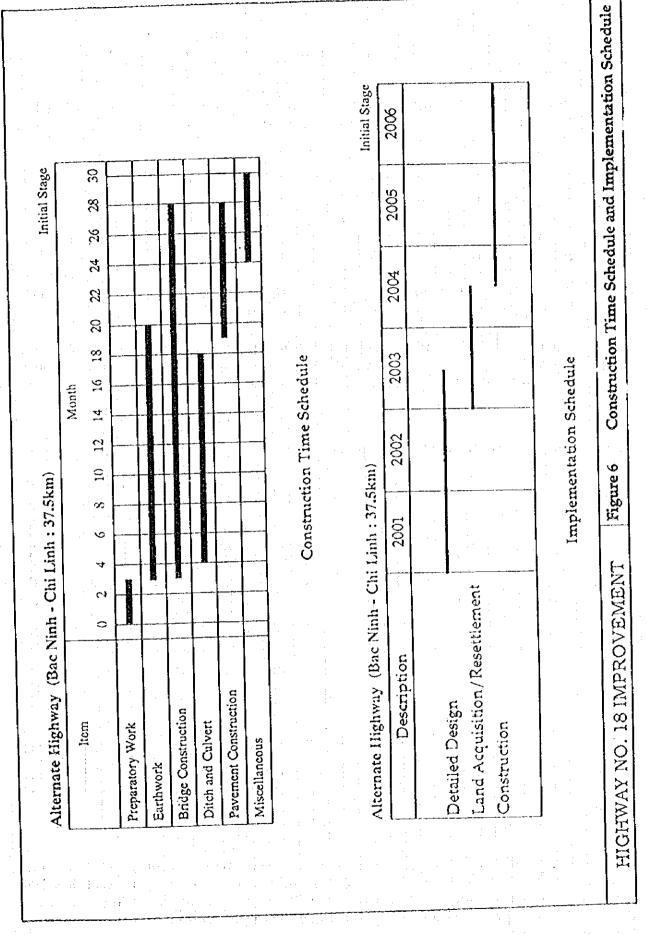
Scheme - 2: Alternate Highway with the rehabilitation (only overlay of asphalt pavement) of Highway No.18.

The economic indicators for the Alternate Highway development alternatives are shown in Table 15.

Table 15 Economic Indicators

Scheme No.	Development Alternative	Present Worth of Benefit at i = 10 % (in million Dong)	B/C Ratio at i = 10 %	IRR (%)
S-1	With Improvement of Highway No.18	489,616	2.00	15.04
S-2	With Rehabilitation of Highway No.18	351,639	1.40	13.23

It is found that the Alternate Highway development with in provement of Highway No.18 (S - 1) indicates higher return. The difference compared with rehabilitation scheme (S - 2) is quite small since the construction of Pha Lai bridge is included in both schemes of S - 1 and S - 2. The adoption of S - 1 is



recommended for conceivable Alternate Highway development. Refer to Tables 16 and 17 for the cost benefit analysis of schemes S - 1 and S - 2.

Further, the economic indicators for only the Alternate Highway portion has been analyzed for reference (Table 18). The result of analysis showed the following economic indicators:

IRR = 13.72 %

Present worth of benefit = 193,334 at i = 10 % (in million Dong)

B/C ratio = 1.40

12. Annual Fund Requirement

A tentative disbursement schedule for the implementation of the Alternate Highway development is prepared as shown in Table 19.

For the calculation of annual fund required, the escalation allowances of 1 percent per annum and 10 percent per annum for the foreign currency component and the local currency component are applied respectively.

Table 19 Annual Fund Requirement

Alternate Highway Development

Bac Ninh - Chi Linh Al	ternate	Highway					J)	Jnit: Milli	on Dong)
		Costin			Curren	Price	٠.	:	Total
		1996 Price	2001	2002	2003	2004	2005	2006	;
Detailed Design	F/C	33,069	13,902	14,041	7,091	0	0	0	35,034
	L/C	14,172	9,130	10,043	5,523	0	0	0	24,696
2. Land Acquisition	F/C	0	. 0	0	o	. 0	0	0	, . c
and Resettlement	L/C	60,797	0	0	59,238	65,162	0	0	124,400
3. Construction	F/C	672,148	0	o	0	72,784	367,559	296,988	737,331
	L/C	443,098	0	0	0	96,051	528,296	464,900	1,089,256
4. Supervision	F/C	47,050	0	0	0	5,095	25,729	20,789	51,613
Services	L/C	20,164	0	0	0	4,322	23773	20,920	49,01
Sub Total	F/C	752,267	13,902	14,041	7,091	77,879	393,288	317,777	823,978
<u></u>	L/C	543,231	9,130	10,043	64,761	165,538	552,069	485,820	1,287,36
Total		1,295,498	23,032	24,084	71,852	243,417	945,357	803,597	2,111,339

13. Conclusions

The results of the study suggest that the project will be technically sound and economically feasible. Taking into account the direct and indirect benefits towards regional development other than the quantified savings in travel costs, the project should be initiated at earlier opportunity.

Cost Benefit Analysis of Alternate Highway with Improvement of Highway No.18

: 2 Lauc + Alernate Highway

15, 04%	Killion Dong	S	Benefit	Value	0	0	6	Ö	23, 481	20,858	26. 20	23,5,57	2000 100	200.12	52.411	60, 404	30.820	31.851	31, 031	67,9,67	23.800	28.50	20 00 00 00 00 00 00 00 00 00 00 00 00 0	STO .C7	707 400	27. 100 20. 20.	18.58	100 01	071.71	670 CT	74.000	13, 484	765.71	CTT -TT	000	202, 303
361	Unit: N	B/C Analysi	Cost	Value	16,		136.	94.	8, 451		į ·	· ·	2	104, 400				200	•	~i			L	ი	C.		:	2 5			28		2 2	1,3/2	010	610.672
			Discout	Factor	0.8693		0.6563	0.5710	o ·	0. 4314	ડે લ	5 6		0.240	oʻ (<u> </u>	o.	o	o o		o' (o ·	5	o' i		.	<u> </u>	> <	-	<u> </u>	o ·		ن د			Ö
-			,	Total	0	<u> </u>			47, 311	48, 345	59, 054	78, 443	94, 347	110.012	151, 36	163, 347	190. 486	226, 47	253, 82,	279.27	323, 32,	252, 229	33. 75. 75. 75. 75. 75.	412, 33	444, 79	474.61	510,62	530, 44	569, 36	604, 45		7	724, 192			9, 107, 350
			Ferry	Cost Saving	0	0	0	O		5, 470	5, 836	11, 402	10,214	14,826	16, 232	11, 598	12,684	13, 296	16, 982	16, 788	26,000	21, 732	22, 824	27, 716	25, 328	28, 260	35, 712	31,084	31,550	32, 024	32, 504		33, 486	•		570.712
			Saving	Diverted Traffic	0	0	0	0	630	916	1,285	1,711	2, 158	2, 452	3, 538	3,861	4,675	5.677	6,486	7, 008	7,546	8,147	8, 764	9.237	9, 700	10,217	10, 721	11, 243	11, 785	12, 347	12, 928	13, 528	14, 147			667 50
	٠.	Benefit	Tine	Normal Traffic	0	0	0	O	1, 757	2, 295	જાં	က ံ	4	ហ		ော်	10.	, 13	16,	20,	27,	34,600	ᅉ	42,	51.	SS.		ල <u>්</u>	<u>.</u> .	77.	84.	91,	99, 924			200 407
٠			Saving	Diverted Traffic	0	o		0	17.	25.	×	46,	8	67,	90	100	117	133.	155	167,	181	5 195, 443	210,	222	234	246,	260,	273	288	303	319	336	8 353, 917	372.		510 700 1 999 15E
	Highway ka		7. O. C	Normal Traffic				:	12,	14.636	14	15	17,	20.	33	88	44,	51,	85	67.	8	93,	103,	110	123	133	(2)	157	165	178	192	206	222	222		c
	+ Alernate 36.4			Total	19, 450	27. 275	207, 103	165, 682	17,027	17.027	39, 125	128, 455	488, 581	424, 072	1.425	1, 425						1, 425								T-1				91, 793		1 017 6948
Highway No.18	oueri 2 : L	Cost	Kainte-	nance	C	5	• C	• C	٠.		425				1,425	1,425	1,425	1.425	1,425	1,425	1,425	1,425	1, 425	1,425	1,425	1, 425	1, 425	1,425	1, 425	1,425	1,425		,-i	1, 425		050 050
-	Caso Design length	יייייייייייייייייייייייייייייייייייייי		Capital	19 450	276 77	207 103	165 629	16,602	16,602	38, 700	128, 030	483, 156	423, 647	0	Ö			0	18,217	Ċ	0	0	90,368	0	18, 217	0	0	0	0		18,217	-	90,363		100 000
· · · · · · · · · · · · · · · · · · ·	U F			Year	1007	1000	2000	0000	2001	2002 9	7 2003	8 2004	2005	10 2006	11 2007					16 2012		8 2014	19 2015		21 2017			2020	25 2021			٠.	29 2025	30 2026		
	٠.				1_									۱۱	-	, F	•	-	B22-7	(9 € ######	- -	- 1(- 1(~~	Ľ	~~					~~			_		J.

Cost Benefit Analysis of Alternate Highway with Rehabilitation of Highway No.18 Table 17

13, 23% Willian Don	VSIS	Benefit	<u> </u>	C		C			1 16,850																3 27,585							17, 115	14, 121		751, 275
IRE :	B/C Analy	<u> </u>	Ľ		120, 56	106, 475	23	86	231.			i i						4						2,	88		3	<u>ما</u>	51	4, 254	7	~~	~~~		750, 964
	_	Discout	Factor	0 8839	0, 7800	0.6888	0,6084	0, 5373		0	0	ं	o'	0	0	င	o'	o	o.	o,	O	୕	0	്	ं	0,0574	0.0507	0.0448	0, 0395	0.0349	0.0308	0. 0272	0.0241		
			Total					36, 990	35, 512	45,942	90, 597	114, 784	134,013	150, 259	162, 175	139, 259	225, 193	252, 462	273, 201	305, 185	325, 817	253, 996	379, 397	398, 554	424, 446	457, 336	477,277			563, 225		628, 435			8, 243, 652
)		Ferry Cost	Saving	0	0	0		15, 184	5, 470	5,836	11.402	10.	14,	15,	Ţ	12,	Š	16,	16.	26.	21.	22.	27.	25,	28.260	ૹ	ij	ਲ	83	32,	32.	쫎	ģ		570,712
		ne Saving	Diverted Traffic	0	0	0	0	338	513	769	1.587	2, 795	3, 201	3, 532	3,884	4,746	5,810	6.662	7, 198	7.750	8,366	9,013	9, 513	10,003	10,550	11,084	11, 639	12, 218	12.817	13, 434	14,068	14, 719	15, 386		201, 994
	Benefit	Time	Normal Traffic	0	0		O	1,363	1,862	2,301	3,869	4, 753	5, 785	7.060	8, 630	10,573	12,987	15, 994	17,949	20.143	22, 606	25, 371	27, 553	29, 895	32, 437	35, 195	38, 188	41,376	44,826	48, 559	52, 598	56, 967	56, 796		625, 637
		Saving	Diverted Traffic	0	0	0		13, 101	133 133	27.	ਫ਼	22	31	8	100	117	137,	155	167.	180	195,	210.	222.	234		259	273	287.	305	တ်	334,	82	369,		4,823,042
<u>\$</u>		V. O. C	Normal Traffic	0	0	0		7,004	оо _.	9. 983	21,350	27	28	33	3	43	ည်	57	8		-	86.074	92, 493	99, 278	106, 568	115, 574	122.815	131, 393	140,524	150, 237	160, 564	171,538	111,583		2, 022, 267
Overley + Highway 36.4			Total	0	154, 572	154, 572	38, 983	128, 313	488, 439	390, 807	1, 283	31, 475	1, 233	1, 283	1, 283	1, 283	1, 283	31, 475	1, 283	91,651	1, 283	1.283	1, 233	31, 475	1, 283	1, 283	1, 283	1, 283	7. 283	121,843	1.283	1, 283	I, 283		1. 586, 699
	Cost	Mainte-	nance	0	0	6	283	88	283	283	1, 283	1.283	1, 283	1, 283	1, 283	1,283	1, 283	1, 283	1. 283	1, 283			1, 283	1.283	282.	7,7	38	7.783	1, 283	1.283	207	1. 283	1, 233	90	30.04
Case Design length)		Capital	0	154, 572	154, 572	38, 700	128, 030	488, 156	390, 524	0	30, 192	0	Ö		0	Ö	30, 192	Ö	90, 368	0	0	0	30, 152	5 6		-	5		120, 560	Ö	00	0	0.00	7, 656, 058
			Year	1997	1998	1999	2000	2001	2002	2003	2007		2006											2017		8102						2025	•		lotal g
	Laction .		-		c ₁	~~~ (1)	~		φ.	- 1		3 0 •	31		77			5	97	T-1	00	6	₹.	7 2	3 2	3 5	7 .	3 8	2 6	7	3 8	\$ 6	3]		_]

Cost Benefit Analysis of Alternate Highway based on Scheme S-2 Table 18

	Cost	ame		83	Benefit					B/C Amalysi	
	Hainte-	ac _{sec} .	V. O. C. Sav	Saving	Time	Saving	Ferry Cost		Discout	Cost	Benefit
Capital	nance	Total	Normal Traffic	Diverted Traffic	Normal Traffic	Diverted Traffic	Saving	Total	Factor	Present Value	Present Value
0	0	0	0	0	0	0	0	0	7618 0	0	0
O		0	O	0	0	0	0	0	0, 7733	0	ō
O			0	-	0	0	0	0	0.6800	0	ਨ
0	0	0	0	<u></u>	0	0	0	0	0,5979	0	
602	0	16, 602	0	6	0	0	0	0	0, 5258	8, 729	O
602	0	16,602	0	0	0	0	0	0	0.4624	7,676	0
38, 700	0	38, 700	0	0	0	0	0	0	0.4066	15, 735	O
030	0	128,030	0	0	0	0		0	0.3575	45, 774	
156	0	488, 156	0	0	0	0	0	0	0.3144	153, 471	
524	0	390, 524	0	Ö	0	0	0		o	107,964	0
lo	1, 532	1.532	20, 706		4,447	2, 123	9, 739	91,	o	372	22, 258
Ö	1,532	1,532	23, 858	9	5, 428	2,316	6,959	86 86	ं	328	21, 135
Ö	1, 532	1,532	27, 479	70, 757	6,641	5	7.610	•	O	288	21, 673
0	1.532	1,532	31,651	8	8, 147		10,978	137,	ਂ	253	22, 655
0	1.532	1,532	36, 426	93	10,021	က	10, 189	153,	o`	223	22, 336
0	1,532	1.532	41,912		12, 373	4, 205	10,073	169,	o'	196	21.622
0	1, 532	1, 532	50, 180	108,	17,046	4,	15,600		o	172	22.027
0	1,532	1,532	57,675	117.	21,388	4	13,039		Ö	151	21, 177
0	1.532	1,532	63, 772	126.	24,004	ഗ	13,694		ਂ	183	20,276
90, 368	1,532	91, 900	68, 525	133,	26,069	ស	16,630		0.0764	7,024	19, 121
0	1.532	1, 532	76, 555	140.	31.895	c,	15, 197		o'	103	18, 150
0	1,532	1,532	82, 308	148,	34, 639	6, 130	16,956	288	o'	9	17,033
0	1, 532	1,532	88, 516	156.	37,620	ဏ်	21.427	310,	ਂ	80	16,115
0	1,532	1, 532	95, 217	164.	40.858	တ်	18,650	325,	ਂ	02	14.892
0	1,532	1,532	102, 452	173	44,377	<i>د</i>	18,930	345, 928	ď	62	13,902
O	1,532	1, 532	110, 262	182, 234	48.199	7,	19,214	367.	o ·	22	12,980
0	1,532	1,532	118, 703	191,810	52, 352		19,502	•	o	48	12, 123
0	1, 532	1.532		201, 843	56,864	တ်	19, 795	414,	0.0273	75	11,325
0	1, 532	1,532		212, 350	61,766		20, 082	440	o'	37	10,581
90,363	1,532	91,900	137, 661	223, 348	61, 766	∞	20, 393		ं	1.942	9, 551
											
1.259.350	30,640	1, 289, 990	1, 499, 346	499, 346 2, 741, 932	605, 900	111,803	304, 668	5, 263, 650		351.016	350, 931

CHAPTER 13 PROJECT COST ESTIMATE

Appendix 13.1 Estimated Construction Cost (1)

Section 1 : Noi Bai - Bac Ninh (Initial Stage Construction)

Item	Unit	Unit Cost	Quantity	Total
itent		(Dong)		(1000 Dong)
and the state of t	L.S	The state of the s		15,081,22
) General	1		·	
!) Earthwork				•
Clearing & Grubbing	.sq.m	2,000	702,031	1,404,06
Soil Excavation	cu.m	18,000	0	The state of the s
Rock Excavation	cu.m	120,000	0	The first of the
Embankment (Borrow Material)	cum	60,000	1,465,737	87,944,22
Sand Mat (t=50cm)	sqm	45,000	0	
Sub Total				89,348,28
) Pavement				
Removal of Existing Asphalt Pavement	co.m	90,000	0	
Subgrade Preparation	sq m	6,000	440,537	2,643,27
Aggreyate Subbase Course	cu m	100,000	158,018	15,801,80
Asphalt Treated Base Course	cu m	750,000	53,631	40,223,2
Prime Coat (1.0 liter/sq.m)	sq.m	5,000	370,307	1,851,5
Tack Coat (0.5 liter/sq.m)	sq m	3,300	616,114	2,033,1
Asphalt Concrete Surface (t=10cm)	sq.m	110,000	354,345	38,977,9
Concrete Sidewalks	sq.m	140,000	0	
Overlay (t=10cm)	sq m	130,600	0	
Sub Total	1 '			101,530,9
See Texat			-	
4) Drainage Structures				
Pipe Culvert (D=0.8m)	Lm	1,200,000	0	
- ditto - (D=1.0m)	Lm	1,700,000	799	1,358,3
- ditto - (D=1.5m)	l.m	2,700,000	238	642,6
Box Culvert (1.5 x 1.5)	1.m	3,400,000	26	
- ditto - (3.0 x 3.0)	.tm	10,300,000	720	7,416,0
Street Pipe Drain	l.m	1,500,000	0	
Street U-Ditch	Lm	500,000	0	and the second
Sub Total				9,505,
oub rotal				
5) Bridges			٠	
Removal of Existing Bridge	sqini	500,000		1
RC Stab Bridge (L<10m)	sq.m	6,000,000	· c	· ·
ditto (with Piling)	sq.m	8,000,000	C)
RC Hollow Slab Bridge (10m <l<20m)< td=""><td>sq.m</td><td>8,000,000</td><td></td><td>)</td></l<20m)<>	sq.m	8,000,000)
- ditto (with Piling)	sq.m	10,000,000)
RC T-Beam Bridge (10m <l<20m)< td=""><td>sq.m</td><td>8,000,000</td><td></td><td>1 -</td></l<20m)<>	sq.m	8,000,000		1 -
ditto (with Piling)	sq.m	10,000,000	731	7,310,
PC I-Beam Bridge (20m <l)< td=""><td>m.ps</td><td>11,000,000</td><td></td><td></td></l)<>	m.ps	11,000,000		
ditto - (with Piling)	squi	13,000,000	21,200	275,678,
	sq.m	30,000,000		D
Long Span Bridge Sub Total				282,988,
Sub Iotal				
en Maria Works	LS		·	19,334,
(6) Miscellaneous Works	 			
Total	1		· ·	517,788,

Appendix 13.1 Estimated Construction Cost (2)

Section 1: Noi Bai - Bac Ninh (Final Stage Construction)

Seil Exeavation Cu in 18,000 0 Rock Excavation Cu in 120,000 0 Embankment (Borrow Material) Cu in 60,000 1,159,340 69,550,4 Sand Mat (t=50cm) Sqm 45,000 0 70,496,4 (3) Pavement Removal of Existing Asphalt Pavement Cu in 50,000 0 Subgrade Preparation Sqm 6,000 411,807 2,470,8 Aggregate Subbase Course Cu in 750,000 148,442 14,844,2 Asphalt Treated Base Course Cu in 750,000 55,546 41,659,5 Prime Coal (10) Rier/sq m) Sqm 3,300 595,364 1,964,7 Asphalt Concrete Surface (t=10cm) Sqm 33,00 595,364 1,964,7 Asphalt Concrete Surface (t=10cm) Sqm 110,000 344,768 37,924,4 Concrete Sidewalks Sqm 140,000 0 Sub Total In 1,200,000 0 Sub Total In 1,200,000 728 1,237,6 Aitto (D=1,5m) In 1,700,000 728 1,237,6 Aitto (D=1,5m) In 1,700,000 14 47,6 Aitto (D=1,5m) In 3,400,000 14 47,6 Aitto (T=5,x-5) In 6,000,000 0 Sub Total In 1,500,000 0 Sub Total Sqm 5,000,000 0 Sub Total Sqm 5,000,000 0 Aitto (with Pling) Sqm 8,000,000 0 Aitto (with Pling) Sqm	Item	Unit	Unit Cost	Quantity	Total
22 Earthwork Clearing & Grubbing Sq.m 2,000 468,020 936,05	gygy saman ny gadupana matuma andro ventramanani w die antonier let 16 februarie en 16 februarie 16 februarie		(Dong)		
Clearing & Grubbing Sq.m 2,000 468,020 936,05 Soil Excavation Cum 13,000 0 Embankment (Borrow Material) Cum 60,000 1,159,340 69,550,4 Sand Mat (t=50km) Sq.m 45,000 0 Sub Total Cum 70,000 1,159,340 69,550,4 Sub Total Cum 70,000 0 Subgrade Preparation Sq.m 6,000 411,807 2,470,8 Aggregate Subbase Course Cum 750,000 148,442 14,844,2 Asphalt Treated Base Course Cum 750,000 376,591 1,834,3 Asphalt Treated Base Course Cum 750,000 376,591 1,834,7 Asphalt Concrete Surface (t=10cm) Sq.m 110,000 344,768 37,924,4 Concrete Surface (t=10cm) Sq.m 110,000 0 0 Overlay (t=10cm) Sq.m 130,000 0 0 Sub Total Clear Clear (t=10cm) Clear Clear (t=10cm) Clear (t=10	(1) General	L.S			14,228,63
Clearing & Grubbing Sq.m 2,000 468,020 936,05 Soil Excavation Cu.m 120,000 0 Embankment (Berrow Material) Cu.m 60,000 1,159,340 69,560,4 Sand Mat (1-50cm) Sq.m 45,000 0 Sub Total Cu.m 70,000 0 Subgrade Preparation Sq.m 6,000 411,807 2,470,8 Aggregate Subbase Course Cu.m 750,000 36,560,4 Aggregate Subbase Course Cu.m 750,000 36,560,4 Aggregate Subbase Course Cu.m 750,000 376,591 1,833,4 Tack Ceat (0.5 liter/sq.m) Sq.m 5,000 376,591 1,834,4 Asphalt Concrete Surface (t=10cm) Sq.m 110,000 344,768 37,924,4 Concrete Sidewalks Sq.m 110,000 34,768 37,924,4 Concrete Sidewalks Sq.m 110,000 34,768 37,924,4 Overlay (t=10cm) Sq.m 130,000 0 Sub Total Cl. Total Cl. Total Cl. Total (4) Drainage Structures Pipe Culvert (D=0,8m) Lm 1,200,000 0 ditto (D=1,5m) Lm 2,700,000 196 529,2 Box Culvert (1,5 x 1,5) Lm 3,400,000 0 ditto (with Piling) Sq.m 5,000,000 0 Sub Total Sub Total Cl. Total Cl. Total Cl. Total (5) Bridges Removal of Existing Bridge Sq.m 5,000,000 0 ditto (with Piling) Sq.m 5,000,000 0 ditto (with Piling) Sq.m 5,000,000 0 ditto (with Piling) Sq.m 8,000,000 0 ditto (with Piling) Sq.m 10,000,000 0	(2) Earthwork		!	1	
Soil Execavation Cu.m 18,000 0 Rock Excivation Cu.m 120,000 0 Rock Excivation Cu.m 120,000 0 Sand Mat (ta-50cm) Sq.m 45,000 0 Sub Total		sq.m	2,000	468,020	936,040
Rock Excavation	· · · · · · · · · · · · · · · · · · ·		1	o	(
Embankment (Borrow Material) Cum Sq.m 45,000 0 70,495.40 69,569.41 1,159,340 69,569.		1	1	o	(
Sand Mat (t=50cm) Sub Total 70,496.4		çu.m		1,159,340	69,560,406
Sub Total	i	sq.m	1 1	o	
Removal of Existing Asphalt Pavement Cu.m 90,000 0		•			70,496,44
Removal of Existing Asphalt Pavement Cu.m 90,000 0	(3) Pavement			•	· ·
Subgrade Preparation		CILID	90.000	a	
Aggregate Subbase Course	-		1	411 807	2.470.842
Asphalt Treated Base Course					14,844,200
Prime Coat (1.0 liter/sq m) sq.m 5,000 376,691 1,883,4 Tack Coat (0.5 liter/sq m) sq.m 3,300 595,364 1,964,7 Asphalt Concrete Surface (t=10cm) sq.m 110,000 344,768 37,924,4 Concrete Sidewalks sq.m 140,000 0 0 0 Overlay (t=10cm) sq.m 130,000 0 0 100,247,1 (4) Drainage Structores Fipe Culvert (D=0.8m) l.m 1,700,000 0 728 1,237,6 ditto · (D=1.6m) l.m 1,700,000 728 1,237,6 1,237,6 ditto · (D=1.5m) l.m 1,700,000 196 529,2 1,237,6 1,2	00 0				41,659,500
Tack Coat (0.5 liter/sq.m) sq.m 3,300 595,364 1,964,7 Asphalt Concrete Surface (1=10cm) sq.m 110,000 344,768 37,924,4 Concrete Sidewalks sq.m 140,000 0 Overlay (1=10cm) sq.m 130,000 0 Sub Tetal			I		1,883,45
Asphait Concrete Surface (t=10cm) Sq.m 110,000 344,768 37,924,4	•	=			The second secon
Concrete Sidewalks		· ·			· · · · · · · · · · · · · · · · · · ·
Overlay (t=10cm) Sub Total Sq.m 130,000 0 100,747,1			! 1	0	
Sub Total 100,747,1 100,			! !	ان ا	
A) Drainage Structures Dipe Culvert (D=0.8m) Dim Dim	· · · · · · · · · · · · · · · · · · ·	54		Ĭ	100.747.17
Pipe Culvert (D=0.8m)					
Pipe Culvert (D=0.8m)	(4) Drainage Structures			<i>*</i>	
ditto - (D=1.0m)	- · · · · · · · · · · · · · · · · · · ·	Lm	1,200,000	ol	er en
- ditto - (D=1.5m)	-	l.m		728	1,237,600
Box Culvert (1.5 x 1.5)]	. 1	529,20
- ditto - (2.5 x 2.5)				: I	47,60
Street Pipe Drain Lm 1,500,000 0 Street U-Ditch Lm 500,000 0 Sub Total		l.m	1		, ,
Street U-Ditch Sub Total		l m	I I	0	
Sub Total		l.m	l : B	o	
(5) Bridges Removal of Existing Bridge					1,814,400
Removal of Existing Bridge					
Removal of Existing Bridge	(5) Bridges	·		ŀ	t -
RC Slab Bridge (L<10m)	_	sq.m	500,000	· 0	v
- ditto - (with Piling)		sq.m	6,000,000	o	- 1 t
RC Hollow Stab Bridge (10m < L < 20m)	•		8,000,000	. • 0	
- ditto - (with Piling) sq.m 10,000,000 0 RC T-Beam Bridge (10m <l<20m) (20m<l)="" (with="" -="" 0="" 10,000,000="" 11,000,000="" 13,000,000="" 21,206="" 275,678,0="" 282,988,0<="" 30,000,000="" 7,310,0="" 731="" 8,000,000="" bridge="" ditto="" i-beam="" long="" pc="" piling)="" span="" sq.m="" sub="" td="" total=""><td></td><td></td><td>8,000,000</td><td>o</td><td></td></l<20m)>			8,000,000	o	
RC T-Beam Bridge (10m <l<20m)< td=""> sq.m 8,000,000 0 ditto (with Piling) sq.m 10,000,000 731 7,310,0 PC I-Beam Bridge (20m<l)< td=""> sq.m 11,000,000 0 21,206 275,678,0 Long Span Bridge sq.m 30,000,000 0 282,988,0 Sub Tetal L.S 18,241,8</l)<></l<20m)<>		1	10,000,000	o	
- ditto - (with Piling)			8,000,000	0	
PC I-Beam Bridge (20m <l) 0="" 10,000="" 10,00<="" 11,000,000="" 13,000,000="" 21,206="" 275,678,0="" sq.m="" td=""><td></td><td>1 -</td><td>10,000,000</td><td>731</td><td>7,310,000</td></l)>		1 -	10,000,000	731	7,310,000
ditto - (with Piling) sq.m 13,000,000 21,206 275,678,0 Long Span Bridge sq.m 30,000,000 0 Sub Total 282,988,0 (6) Miscellaneous Works L.S 18,241,8	· · · · · · · · · · · · · · · · · · ·			ol	
Long Span Bridge sq.m 30,000,000 0 Sub Total 282,988,0 (6) Miscellaneous Works L.S 18,241,8		1		21,206	275,678,000
Sub Total 282,988,0 [6] Miscellaneous Works L.S 18,241,8		1	•	o	
(6) Miscellaneous Works L.S 18,241,8	-				282,988,00
				- · · ·	
T.1.1	(6) Miscellaneous Works	LS			18,241,841
	Total			*	488,516,499

Appendix 13.1 Estimated Construction Cost (3)

Section 2: Bac Ninh - Chi Linh (Initial Stage Construction)

ltem	Unit	Unit Cost (Dong)	Quantity	Total (1000 Dong)
	L.S	(Dong)		12,584,36
i) General	L.S		1	12,301,00
	:			· .
2) Earthwork	- i	2,000	74,822	149,64
Clearing & Grubbing	sq.m	18,000	74,022	,.
Soil Excavation	cum		ď	
Rock Excavation	cum	120,000	569,205	34,152,3
Embankment (Borrow Material)	cum	60,000	49,500	2,227,5
Sand Mat (t=50cm)	sq.m	45,000	49,300	36,529,4
Sub Total				30,027,4
		·		
3) Pavement				*
Removal of Existing Asphalt Pavement	cu.m	90,000		2.722.0
Subgrade Preparation	sq.m	6,000	453,827	2,722,9
Aggregate Subbase Course	ca m	100,000	162,787	16,278,7
Asphalt Treated Base Course	cu.m	750,000	55,248	41,436,0
Prime Coat (1.0 liter/sq.m)	sq.m	5,000	381,478	1,907,3
Tack Coat (0.5 liter/sq.m)	sq.m	3,300	634,700	2,094,5
Asphalt Concrete Surface (t=10cm)	sq.m	110,000	365,035	40,153,8
Concrete Sidewalks	sq.m	140,000	0	1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 -
Overlay (t=10cm)	sq.m	130,000	0	
Sub Total	•			104,593,4
340 1014				
(4) Drainage Structures	: 1			
Pipe Culvert (D=0.8m)	i.m	1,200,000	35	42,0
- ditto - (D=1.0m)	Lm	1,700,000	603	1,025,
	Lin	2,700,000	208	561,0
- ditto - (D=1.5m)	l.m	3,400,000	132	448,8
Box Culvert (1.5 x 1.5)	l.m	6,000,000	9	51,0
- ditto - (2.5 x 2.5)	Lm	1,500,000	0	
Street Pipe Drain		500,000	0	· · · · · · · · · · · · · · · · · · ·
Street U-Ditch	Lm	300,000		2,131,
Sub Total	1		; [†]	
(5) Bridges		500.000	573	286,3
Removal of Existing Bridge	sq.m	500,000		200,
RC Slab Bridge (L<10m)	sq.m	6,000,000	0	
ditto - (with Piling)	sq m	8,000,000	0	
RC Hollow Slab Bridge (10m <l<20m)< td=""><td>sq.m</td><td>8,000,000</td><td>0</td><td>100</td></l<20m)<>	sq.m	8,000,000	0	100
ditto (with Piling)	sq m	10,000,000	U	
RC T-Beam Bridge (10m <l<20m)< td=""><td>sq.m</td><td>3,000,000</td><td></td><td>1 1 1 1</td></l<20m)<>	sq.m	3,000,000		1 1 1 1
ditto (with Piling)	sq.m	10,000,000		1,210,
PC 1-Beam Bridge (20m <l)< td=""><td>sq m</td><td>11,000,000</td><td></td><td></td></l)<>	sq m	11,000,000		
ditto (with Piling)	sq.m	13,000,000	14,388	
Long Span Bridge	sq m	30,000,000	2,385	71,550,
Sub Total	1			260,090,
+ ·		1		
(6) Miscellaneous Works	LS			16,133,
(a) phacelidireory (1018)	1-:			
Total	1		e de la companya de l	432,063,

Appendix 13.1 Estimated Construction Cost (4)

Section 2: Bac Ninh - Chi Linh (Final Stage Construction)

Item	Unit	Unit Cost	Quantity	Total
		(Dong)		(1000 Dong)
(1) General	LS			12,114,25
(2) Earthwork				
Clearing & Grubbing	sqm	2,000	279,531	559,06
Soil Excavation	cu.m	18,000	15,456	278,20
Rock Excavation	cu.m	120,000	0	
Embankment (Borrow Material)	cum	60,000	307,190	18,431.40
Sand Mat (t=50cm)	sq.m	45,0 00	12,750	573,75
Sub Total				19,842,42
				1
3) Pavement				
Removal of Existing Asphalt Pavement	cu.m	90,000	6,742	606,78
Subgrade Preparation	sq.m	6,000	279,531	1,677,18
Aggregate Subbase Course	cu.m	100,000	155,468	15,546,80
Asphalt Treated Base Course	cu.m	750,000	52,289	39,216,75
Prime Coat (1.0 liter/sq m)	sq.m	5,000	330,504	1,652,52
Tack Coat (0.5 liter/sq.m)	sq.m	3,300	531,109	1,752,66
Asphalt Concrete Surface (t=10cm)	sq.m	110,000	297,618	32,737,98
Concrete Sidewalks	sq.m	140,000	0	
Overlay (t=10cm)	sq.m	130,000	0	
Sub Total				93,190,67
4) Drainage Structures		ļ.		
Pipe Culvert (D=0.8m)	l.m.	1,200,000	48	57,60
Pipe Culvert (D=1.0m)	l.m	1,700,000	332	564,40
Pipe Culvert (D=1.5m)		2,700,000	119	321,30
Box Culvert (1.5 x 1.5)	l.m	3,400,000	145	493,00
- ditto - (2.5 x 2.5)	l.m	6,000,000	9	54,00
Street Pipe Drain	l m	1,500,000	0	
Street U-Ditch	l.m	500,000	o	1
Sub Total				1,490,30
5) Bridges				:
Removal of Existing Bridge	sq.m	500,000	o	
RC Slab Bridge (L<10m)	sqm	6,000,000	o	
- ditto - (with Piling)	sq.m	8,000,000	ő	· ·
RC Hollow Slab Bridge (10m <l<20m)< td=""><td>sq.m</td><td>8,000,000</td><td>ő</td><td></td></l<20m)<>	sq.m	8,000,000	ő	
- ditto - (with Piling)	sqm	10,000,000	ň	
RC T-Beam Bridge (10m <l<20m)< td=""><td>sq.m</td><td>8,000,000</td><td>ň</td><td></td></l<20m)<>	sq.m	8,000,000	ň	
- ditto - (with Piling)	sq.m	10,000,000	121	1,210,00
PC I-Beam Bridge (20m <l)< td=""><td>sq.m</td><td>11,000,000</td><td>""</td><td>1,210,00</td></l)<>	sq.m	11,000,000	""	1,210,00
- ditto - (with Piling)		13,000,000	14,388	187,044,00
	sq.m sa.m	30,000,000	2,850	85,500,00
Long Span Bridge	sq.m	30,000,000	2,000	
Sub Total				273,754,0
(() k#:N	,	4 4		10 031 0
(6) Miscellaneous Works	L.S	 		15,531,09
Total]		415,922,74

Appendix 13.1 Estimated Construction Cost (5)

Section 3: Hong Gal - Cua Ong (Initial Stage Construction)

Item	Unit	Unit Cost	Quantity	Total
		(Dong)		(1000 Dong)
(1) General	L.S			17,790,20
(2) Earthwork			• .	
Clearing & Grubbing	0.2 12	2,000	200.211	700 41
Soil Excavation	sq.m	1	390,211	780,42
Rock Excavation	cu.m	18,000	716,316	12,893,68
Embankment (Borrow Material)	cu.m	120,000	32,432	3,891,84
Sand Mat (t=50cm)	cu m	60,000 45,000	945,876	56,752,56
Sub Total	sq m	45,000	4	74 210 51
Suo Tetai		•		74,318,5
(3) Pavement		i :		:
		20,000	5040	527.43
Removal of Existing Asphalt Pavement	cu.m	90,000	5,849	526,41
Subgrade Preparation	sq.m	6,000	728,382	4,370,29
Aggregate Subbase Course	cu m	100,000	252,716	25,271,60
Asphalt Treated Base Course Prime Coat (1.0 liter/sq.m)	cu.m	750,000	117,870	88,402,50
· · · · · · · · · · · · · · · · · · ·	sq.m	5,000	745,027	3,725,13
Tack Coat (0.5 liter/sq.m)	sqm	3,300	1,193,525	3,938,63
Asphalt Concrete Surface (t=10cm) Concrete Sidewalks	sq.m	110,000	667,188	73,390,6
	sq.m	140,000	280,920	39,328,8
Overlay (t=10cm)	sq.m	130,000	38,000	4,940,0
Sub Total				243,894,03
A) Designate Charles		.1		
4) Drainage Structures] .	1,202,000	4.5	
Pipe Culvert (D=0.8m)	Lm	1,200,000	415	498,00
- ditto - (D=1.0m)	I.m	1,700,000	384	652,80
- ditto - (D=1.5m)	l.m	2,700,000	0	1 400 0
Box Culvert (1.5 x 1.5)	l.m	3,400,000	420	1,428,00
- ditto - (2.5 x 2.5)	l m	6,000,000	71	426,00
Street Pipe Drain	lm	1,500,000	23,000	34,500,00
Street U-Ditch Sub Total	l.m	500,000	46,320	23,160,00
Sub Total				60,664,80
Et Dailage		Ì		
5) Bridges Removal of Existing Bridge		500,000	461	225.50
The state of the s	sq m		451	225,50 5,352,00
RC Słab Bridge (L<10m) - ditto - (with Piling)	sq.m	6,000,000 8,000,000	892	
	sq.m	and the second s	1,144	9,152,00
RC Hollow Slab Bridge (10m <l<20m)< td=""><td>sq m</td><td>8,000,000</td><td>201</td><td>0.010.00</td></l<20m)<>	sq m	8,000,000	201	0.010.00
ditto (with Piling)	sq.m	10,000,000	901	9,010,00
RC T-Beam Bridge (10m <l<20m)< td=""><td>sq m</td><td>8,000,000</td><td>, (2)</td><td>14 730 00</td></l<20m)<>	sq m	8,000,000	, (2)	14 730 00
- ditto - (with Piling)	sqm	10,000,000	1,673	16,730,00
PC I-Beam Bridge (20m < L)	sqm	11,000,000	7,137	78,507,00
ditto (with Piling)	sqm	13,000,000	5,565	72,345,00
Long Span Bridge	sq.m	30,000,000	0	101 001 50
Sub Total		: 1		191,321,50
(CAN to a Manager Marke				23 007 05
(6) Miscellancous Works	L.S			22,807,95
			+	£10.707.01
Total	l		·	610,797,01

Appendix 13.1 Estimated Construction Cost (6)

Section 4 : Cua Ong - Tien Yen (Initial Stage Construction)

Item	Unit	Unit Cost	Quantity	Total
		(Dong)		(1000 Dong)
1) General	L.S			1,491,6
m m d I				
(2) Earthwork		1 2000		
Clearing & Grubbing	sq.m	2,000	0	
Soil Excavation	cu.m	18,000	0	. :
Rock Excavation	cu.m	120,000	0	
Embankment (Borrow Material)	cum	60,000	0	
Sand Mat (t=50cm)	sq.m	45,000	0	
Sub Total	:			:
3) Pavement				
Removal of Existing Asphalt Pavement	cu.an	90,000	o	
Subgrade Preparation	sq.m	6,000	0	
Aggregate Subbase Course	cu.m	100,000	0	
Asphalt Treated Base Course	cum	750,000	, o	
Prime Coat (1.0 liter/sq.m)		5,000	ام	
	sq.m	3,300	0 0	
Tack Coat (0.5 liter/sq.m)	sq.m	1 : 1	V	
Asphalt Concrete Surface (t=10cm)	sq.m	110,000	ار	and the state of
Concrete Sidewalks	sq.m	140,000	105.015	24.160.0
Overlay (t=10cm)	sq.m	130,000	185,915	24,168,9
Sub Total		·		. 24,168,9
(4) Drainage Structures		1,000,000		
Pipe Culvert (D=0.8m)	l.m	1,200,000	.0	
- ditto - (D=1.0m)	Lm	1,700,000	0	
- ditto - (D=1.5m)	l.m	2,700,000	0	:
Box Culvert (1.5 x 1.5)	1 m	3,400,000	0	
- ditto - (2.5 x 2.5)	l n	6,000,000	. 0	
Street Pipe Drain	Lm	1,500,006	0	100
Street U-Ditch	Lm	500,000	0	
Sub Total				
			<u>:</u>	
5) Bridges		1		
Removal of Existing Bridge	sq.m	500,000	1,274	637,0
RC Slab Bridge (L<10m)	sq.m	6,000,000	356	2,136,0
- ditto - (with Piling)	so m	8,000,000		
RC Hollow Slab Bridge (10m <l<20m)< td=""><td>sq.m</td><td>8,000,000</td><td>120</td><td>960,0</td></l<20m)<>	sq.m	8,000,000	120	960,0
- ditto - (with Piling)	. sq.m	10,000,000	407	4,070,0
RC T-Beam Bridge (10m <l<20m)< td=""><td>sq m</td><td>8,000,000</td><td>104</td><td>832,0</td></l<20m)<>	sq m	8,000,000	104	832,0
ditto (with Piling)	sq.m	10,000,000	0	
PC I-Beam Bridge (20m <l)< td=""><td>sq.m</td><td>11,000,000</td><td>1,364</td><td>15,004,0</td></l)<>	sq.m	11,000,000	1,364	15,004,0
- ditto - (with Piling)	sq m	13,000,000	0	
Long Span Bridge	sq.m	30,000,000	0	
Sub Total	[23,639,0
			* :	
(6) Miscellaneous Works	L.S		: ' .	1,912,3
· · · · · · · · · · · · · · · · · · ·	 			*************************************
Total	1 1			51,211,8

Appendix 13.1 Estimated Construction Cost (7)

Section 4 : Cua Ong - Tien Yen (Final Stage Construction)

Iten)	Unit	Unit Cost	Quantity	Total
		(Dong)	. •	(1000 Dong)
(I) General	L.S		assessandine videntilla lähtävällipalisistenin priiteiva	5,056,851
(2) Earthwork			÷ .	
Clearing & Grubbing	sq.m	2,000	216,528	433,056
Soil Excavation	cu m	18,000	1,233,706	22,206,70
Rock Excavation	çu.m	120,000	0	
Embankment (Borrow Material)	cu.m	60,000	203,862	12,231,720
Sand Mat (t=50cm)	sq.m	45,000	0	1
Sub Total			i i	34,871,48
(3) Pavement			1 1	
Removal of Existing Asphalt Pavement	cum	90,000	2,408	216,72
Subgrade Preparation	sq.m	6,000	443,935	2,663,61
Aggregate Subbase Course	cu.m	100,000	161,523	16,152,300
Asphalt Treated Base Course	cu.m	750,000	58,271	43,703,25
Prime Coat (1.0 liter/sq.m)	sq.m	5,000	393,976	1,969,88
Tack Coat (0.5 liter/sq.m)	sq.m	3,300	655,192	and the second s
Asphalt Concrete Surface (t=10cm)	sq.m	110,000	372,449	40,969,39
Concrete Sidewalks	sq.m	140,000	0	1
Overlay (t=10cm)	sq.m	130,000	0	. '
Sub Total			•	107,837,28
(4) Drainage Structures				I
Pipe Culvert (D=0.8m)	Lm	1,200,000	302	362,400
- ditto - (D=1.0m)	1.m	1,700,000	15 9	270,300
- ditto - (D=1.5m)	1m	2,700,000	.13	35,100
Box Culvert (1.5 x 1.5)	Lm	3,400,000	180	612,000
- ditto - (2.5 x 2.5)	Lm	6,000,000	0	
Street Pipe Drain	Lm	1,500,000	0	(
Street U-Ditch	l.m	500,000	36,180	18,090,000
Sub Total			* : .	19,369,80
(5) Bridges				
Removal of Existing Bridge	sq.m	500,000	0	. (
RC Slab Bridge (L<10m)	sq.m	6,000,000	0	. (
- ditto - (with Piling)	sq.ភា	8,000,000	0	(
RC Hollow Slab Bridge (10m <l<20m)< td=""><td>sq m</td><td>8,000,000</td><td>0</td><td>•</td></l<20m)<>	sq m	8,000,000	0	•
- ditto - (with Piling)	şq.m	19,009,000	0	(
RC T-Beam Bridge (10m <l<20m)< td=""><td>sq m</td><td>8,000,000</td><td>0</td><td></td></l<20m)<>	sq m	8,000,000	0	
- ditto - (with Piling)	squii	10,000,000	0	(
PC 1-Beam Bridge (20m <l)< td=""><td>sq m</td><td>11,000,000</td><td>0</td><td>(</td></l)<>	sq m	11,000,000	0	(
- ditto - (with Piling)	sqm	13,000,000	0	(
Long Span Bridge	sq.m	30,000,000	o	· · · · · · · · · · · · · · · · · · ·
Sub Total	[
]		j	
(6) Miscellaneous Works	L.S			6,483,143
Total]	· ·		173,618,562

Appendix 13.1 Estimated Construction Cost (8)

Section 5: Tien Yen - Bac Luan (Initial Stage Construction)

Item	Unit	Unit Cost	Quantity	Total
መመመመ መመመመ መመመመ መመመመ የመመመመ መመመመ መመመመ መመ		(Dong)		(1000 Dong)
(1) General	LS			3,557,58
(2) Earthwork				000 77
Clearing & Grubbing	sq.m	2,000	110,376	220,75
Soil Excavation	cu.m	18,000	31,020	558,30
Rock Excavation	çu m	120,000	150 500	0.002.00
Embankment (Borrow Material)	cu.m	60,000	158,722	9,523,30
Sand Mat (t=50cm)	sq.m	45,000	ď	•••••
Sub Total				10,302,43
3) Pavement				
Removal of Existing Asphalt Pavement	Cit.m	90,000	0 130	50.4 O/
Subgrade Preparation	sq.m	6,000	87,480	524,85
Aggregate Subbase Course	¢u.m	100,000	30,942	3,094,20
Asphalt Treated Base Course	cu.m	750,000	10,368	7,776,00
Prime Coat (1.0 liter/sq.m)	sq.กา	5,000	69,660	348,30
Tack Coat (0.5 liter/sq.m)	sq m	3,300	115,830	382,23
Asphalt Concrete Surface (t=10cm)	sq.m	110,000	68,040	7,484,40
Concrete Sidewalks	sq.m	140,000	0	
Overlay (t=10cm)	sq.m	130,000	297,987	38,738,3
Sub Total		·		58,348,3
(4) Drainage Structures			i	
Pipe Culvert (D=0.8m)	l m	1,200,000	251	: 301,20
- ditto - (D=1.0m)	l m	1,700,000	24	40,80
ditto - (D=1.5m)	l m	2,700,000	0]	
Box Culvert (1.5 x 1.5)	1.m	3,400,000	67	227,80
- ditto - (2.5 x 2.5)	l.m	6,000,000	12	72,00
Street Pipe Drain	l.m	1,500,000	0	
Street U-Ditch	l.m	500,000	2,820	1,410,00
Sub Total			1 + 1 - +	2,051,80
			• .	
(5) Bridges				
Removal of Existing Bridge	sq m	500,000	1,893	946,50
RC Slab Bridge (L<10m)	sq.m	6,000,000	688	4,128,00
- ditto (with Piling)	sq.m	8,000,000	0]	
RC Hollow Stab Bridge (10m <l<20m)< td=""><td>sq.m</td><td>8,000,000</td><td>816</td><td>6,528,00</td></l<20m)<>	sq.m	8,000,000	816	6,528,00
- ditto · (with Piling)	sq.m	19,000,000	0	
RC T-Beam Bridge (10m <l<20m)< td=""><td>sq.m</td><td>8,000,000</td><td>104</td><td>832,00</td></l<20m)<>	sq.m	8,000,000	104	832,00
- ditto - (with Piling)	sq.m	10,000,000	0	
PC 1-Beam Bridge (20m <l)< td=""><td>sq.m</td><td>11,000,000</td><td>1,872</td><td>20,592,00</td></l)<>	sq.m	11,000,000	1,872	20,592,00
- ditto - (with Piling)	sq m	13,000,000	792	10,296,00
Long Span Bridge	sq m	30,000,000	0	
Sub Total		· [,	43,322,50
(6) Miscellaneous Works	L.S]		4,561,0
			·	
Total				122,143,6

Appendix 13.1 Estimated Construction Cost (9)

Section 5: Tien Yen - Bac Luan (Final Stage Construction)

Îtem	Unit	Unit Cost	Quantity	Total
		(Dong)		(1000 Dong)
1) General	L.S			10,583,06
2) Earthwork	,		,	
Clearing & Grubbing	sq.m	2,000	503,456	1,006,91
Soil Excavation	cu.m	18,000	581,502	10,467,03
Rock Excavation	cu.m	120,000	0	
Embankment (Borrow Material)	cu.m	60,000	561,618	33,697,08
Sand Mat (t=50cm)	sq m	45,000	0	
Sub Total				45,171,02
(3) Pavement				
Removal of Existing Asphalt Pavement	çu.m	. 90,000	9,594	863,46
Subgrade Preparation	sq.m	6,000	771,120	4,626,72
Aggregate Subbase Course	cum	100,000	278,764	27,876,40
Asphalt Treated Base Course	cú.m	750,000	98,018	73,536,00
Prime Coat (1.0 liter/sq.m)	sq.m	5,000	658,760	3,293,8
Tack Coat (0.5 liter/sq.m)	sq.m	3,300	1,095,380	3,614,7
Asphalt Concrete Surface (t=10cm)	so.m	110,000	620,460	68,250,60
Concrete Sidewalks	sq.m	140,000	. 0	1 4 1
Overlay (t=10cm)	Sq.ភា)	130,000	. 0	1
Sub Total				182,061,73
940 1044				,
4) Drainage Structures		·		
Pipe Culvert (D=0.8m)	l m	1,200,000	596	715,20
- ditto - (D=1.0m)	1 m	1,700,000	778	1,322,60
- ditto - (D=1.5m)	1 m	2,700,000	261	per per di
Box Culvert (1.5 x 1.5)	l m	3,400,000	953	3,240,20
- ditto - (2.5 x 2.5)	l.m	6,000,000	65	390,00
Street Pipe Drain	Ì.m	1,500,000	. 0	
Street U-Ditch	1 m	500,000	37,480	18,740,0
Sub Total			·	24,408,0
(5) Bridges				
Removal of Existing Bridge	sq.m	500,000	0	:
RC Slab Bridge (L<10m)	sq.m	6,000,000	. 0	
- ditto - (with Piling)	sq.m	8,000,000	. 0	
RC Hollow Slab Bridge (10m <l<20m)< td=""><td>sq.m</td><td>8,000,000</td><td>0</td><td></td></l<20m)<>	sq.m	8,000,000	0	
- ditto - (with Piling)	sq.m	10,000,000	0	
RC T-Beam Bridge (10m <l<20m)< td=""><td>sq.m</td><td>8,000,000</td><td>0</td><td>1.1</td></l<20m)<>	sq.m	8,000,000	0	1.1
- ditto - (with Piling)	sq.m	10,000,000	0	•
PC I Beam Bridge (20m <l)< td=""><td>sq.m</td><td>11,000,000</td><td>7,960</td><td>87,560,0</td></l)<>	sq.m	11,000,000	7,960	87,560,0
- ditto - (with Piling)	sq.m	13,000,000	0	
Long Span Bridge	sq.m	30,000,000	o	•
Long эрап впаде Sub Total	34			87,560,0
Suo Itali				
WAS - Name and Morks	L.S			13,568,0
(6) Miscellaneous Works				
Total		· !		363,351,8

Appendix 13.2 Estimated Land Acquisition and Resettlement Cost (1)

Section 1: Noi Bai - Bac Ninh (Initial Stage)

Description	Unit	Unit Cost (Dong)	Quantity	Total (1000 Dong)
(1) Land Acquisition/Compensation				and the second s
Inhabited Land along Existing Road	sq.m	400,000	2834	1,133,600
Inhabited Land in Rural Area	sq.m	38,600	11,337	437,608
Rice Field/Cultivated Area	sq.m	19,300	1,155,880	22,308,484
Forest Area	sq.m	14,000	0	0
Subtotal			ļ	23,879,692
		ŀ		
(2) Crop Compensation				=
Rice	sq.m	7,200	1,155,880	8,322,336
Subtotal				8,322,336
		1		
(3) Resettlement/Compensation				
Permanent House	sq.m	1,500,000	580	870,000
Semi-Permanent House	sq m	1,200,000	0	0
Temporary House	sq.m	500,000	. 0	·
Subtotal				870,000
				1
(4) Other Compensation				
Cost Relevant to Relocating house	nos.	6,000,000	3	18,000
Subtotal				18,000
Total		[:]		33,090,028

Section 2: Bac Ninh - Chi Linh (Initial Stage)

Description	Unit	Unit Cost	Quantity	Total
		(Dong)		(1000 Dong)
(1) Land Acquisition/Compensation				
Inhabited Land along Existing Road	sq.m	400,000	4604	1,841,600
Inhabited Land in Rural Area	sq.m	38,600	O.	0
Rice Field/Cultivated Area	sq.m	19,300	44,557	859,950
Forest Area	sq.m	14,000	25661	359,254
Subtotal		;		3,060,804
(2) Crop Compensation				
Rice	sq.m	7,200	44,557	320,810
Subtotal				320,810
				1. T
(3) Resettlement/Compensation			. ,	
Permanent House	sq.m	1,500,000	2653	3,979,500
Semi-Permanent House	sq.m	1,200,000	, 0	0
Temporary House	sq.mi	500,000	0	C
Subtotal	· .		:	3,979,500
(4) Other Compensation	1		100	
Cost Relevant to Relocating house	nos.	6,000,000	32	192,000
Subtotal		.i •		192,000
Total				7,553,114

Appendix 13.2 Estimated Land Acquisition and Resettlement Cost (2)

Section 2: Bac Ninh - Chi Linh (Final Stage)

Description	Unit	Unit Cost	Quantity	Total
•		(Dong)		(1000 Dong)
1) Land Acquisition/Compensation				•
Inhabited Land along Existing Road	sq.m	400,000	39,134	15,653,600
Inhabited Land in Rural Area	sq.m	38,600	0	0
Rice Field/Cultivated Area	sq.m	19,300	240,397	4,639,662
Forest Area	sq.m	14,000	0	. 0
Subtotal				20,293,262
				. :
Crop Compensation Rice	sq.m	7,200	240,397	1,730,858
Subtotal	1			1,730,858
				:
(3) Resettlement/Compensation				
Permanent House	sq.m	1,500,000	2,486	3,729,000
Semi-Permanent House	sq.m	1,200,000	. 0	
Temporary House	sq.nv	500,000	0	
Subtotal				3,729,000
				•
(4) Other Compensation		6,000,000	40	240,000
Cost Relevant to Relocating house Subtotal	nos.	0,000,000	10	240,000
Total			NAME OF THE OWNER, WHEN	25,993,120

Section 3: Hong Gai - Cua Ong (Final Stage)

Description	Unit	Unit Cost (Dong)	Quantity	Total (1000 Dong)
(1) Land Acquisition/Compensation				
Inhabited Land along Existing Road	sq.m	245,000	126,691	31,039,295
Inhabited Land in Rural Area	sq.m	24,200	173,240	4,192,408
Rice Field/Cultivated Area	sq.m	12,100	45,140	546,194
Forest Area	sq m	8,900	45,140	401,746
Subtotal				36,179,64
(2) Crop Compensation				
Rice	m.pa	7,200	45,140	325,00
Subtotal				325,00
(3) Resettlement/Compensation				
Permanent House	sq m	1,500,000	22,646	33,969,00
Semi-Permanent House	sqm	900,000	11,323	10,190,70
Temporary House	sq.m	500,000	3,774	1,887,00
Subtotal				46,016,70
(4) Other Compensation				
Cost Relevant to Relocating house	nos.	6,000,000	368	2,208,00
Subtotal				2,208,00
	1			
Total				81,759,3

Appendix 13.2 Estimated Land Acquisition and Resettlement Cost (3)

Section 4 : Cua Ong - Tien yen (Final Stage)

Description	Unit	Unit Cost	Quantity	Tetal
		(Dong)		(1000 Dong)
(1) Land Acquisition/Compensation				
Inhabited Land along Existing Road	sq.m	150,000	2,519	377,850
Inhabited Land in Rural Area	sq.m	19,400	0	0
Rice Field/Cultivated Area	. sq.n\	12,100	30,538	369,510
Forest Area	so m	8,920	183,471	1,636,561
Subtotal				2,383,921
(2) Crop Compensation				
Rice	sq.m	7,200	30,538	219,874
Subtotal				219,874
(2) Parallement (Comparation				
(3) Resettlement/Compensation Permanent House	.:	1 200 000	3,143	4.005.000
Semi-Permanent House	sq.m	1,300,000	1,571	4,085,900 1,413,900
Temporary House	sq m sq m	490,000	524	209,600
Subtotal	5Q.III	450,000	324	5,709,400
			: -	
(4) Other Compensation	•			
Cost Relevant to Relocating house	nos.	6,000,000	o	e
Subtotal				0
Total				8,313,195

Section 5: Tien Yen - Bac Luan (Initial Stage)

Unit	Unit Cost	Quantity	Total
	(Dong)		(1000 Dong)
sq.m	150,000	0	1
sq.m	19,400	0	•
sq.m	12,100	55,188	667,77
sq m	8,920	55,188	492,277
			1,160,052
		* :	:
sq m	7,200	55,188	397,35
			397,35
sg.m	1,300,000	607	789,10
sq.m	. ,	304	273,60
! .	1 1	101	40,40
			1,103,10
			•
nos.	6,000,000	15	90,00
1			
 	 		90,00
1		-	•
	sq.m sq.m sq.m sq.m	(Dong) sq.m 150,000 sq.m 19,400 sq.m 12,100 sq.m 8,920 sq.m 7,200 sq.m 7,200 sq.m 900,000 sq.m 900,000 sq.m 400,000	(Dong) sq.m 150,000 0 sq.m 19,400 0 sq.m 12,100 55,188 sq.m 8,920 55,188 sq.m 7,200 55,188 sq.m 1,300,000 607 sq.m 900,000 304 sq.m 400,000 101

Appendix 13.2 Estimated Land Acquisition and Resettlement Cost (4)

Section 5 : Tien Yen - Bac Luan (Final Stage)

Description	Unit	Unit Cost (Dong)	Quantity	Total (1000 Dong)
		\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\		
(1) Land Acquisition/Compensation		150,000	4,345	651,750
Inhabited Land along Existing Road	sq.m	-	75,184	1,458,570
Inhabited Land in Rural Area	sq.m	19,400	1	
Rice Field/Cultivated Area	sq.m	12,100	200,344	2,424,162
Forest Area	sq.m	8,920	223,583	1,994,360
Subtotal				6,528,842
		•	•	
(2) Crop Compensation		*		
Rice	m.pa	7,200	200,344	1,442,477
Subtotal				1,442,477
			1000	
(3) Resettlement/Compensation				
Permanent House	sq.m	1,300,000	1,803	2,343,900
Semi-Permanent House	รสุ.กา	900,000	902	811,800
	sq.m	400,000	300	120,000
Temporary House	54,111	100,000	•	3,275,700
Subtotal			i	0,2,0,,0
	, i			
(4) Other Compensation				. 244.00
Cost Relevant to Relocating house	nos.	6,000,000	44	264,000
Subtotal	<u></u>	<u> </u>		264,000
Total	[·			11,511,01

CHAPTER 14 ECONOMIC PROJECT ANALYSIS

Appendix A - 14.1 Income Structure in Study Area/Month

Income Class	Average	Percentage of Total
Upper class	445,690 Dong	2%
Lower upper class	173,750 Dong	15%
Middle class	94,540 Dong	37%
Lower middle class	62,050 Dong	25%
Lower class	38,980 Dong	21%
Total	815,010 Dong	100%

Source: Statistical data on labor and social affairs 1994

Appendix A - 14.2 Vehicle Price for Vehicle Operating Cost Calculation (Financial price in Table - 1)

					Unit: Million dong
Passenger Car Import Knock-down	Component 40% 60%	Average Value 270 160	Weighed Price 108 204 96	Registration Fee 5%	Financial Price 214
<u>Bus</u>	Registered Amount	%	Average Value	Weighed Price	Financial Cost Registration Fee 5%
less than 12 seats 12 - 36 seats more than 36 seats	941 644 2,259	60% 40%	370 100 900	222 40	265 945
Truck less than 1 ton 1 - 3 tons	1,048 680	11% 7%	240 240	26 17	
3 - 5 tons	7,728 9,456 16,875	82% 100%	270 560	221 246	277
5 - 7 tons 7 - 10 tons more than 10 tons	1,556 230	8% 2%	560 560	<i>E</i> 20	588
Van	18,661	100%	225	560 225	236
Motorcycle Honda "Dream" 70cc 50cc		20% 50% 30%	22.5 18.0 17.5	6 9 4	
	t i e i			19	19.8

Authorized Value of Vehicle: Vehicle value to levy for registration tax by Ha Noi Vehicle Taxation Department, Vietnam State Treasury.

Averaged class 1 and class 2. Value revised every 2 years

Appendix A - 14.3 Procedure and Equation for Unit Vehicle Operating Cost Calculation

B-1 Fuel Cost

Financial Fuel Cost = Financial Fuel Price x Fuel Consumption (I/km)

Economic Fuel Cost = Economic Fuel Price x Fuel Consumption (I/km)

B-2 Lubricant Oil Cost

Financial Lubricant Oil Cost = Financial Lubricant Oil Price (liter) x Lubricant Oil Consumption (1/100km)

Economic Lubricant oil Cost = Economic Lubricant Oil Price (liter) x Lubricant Oil Consumption (1/100km)

B-3 Tyre Cost

Financial Tyre Cost = Financial Tyre Unit Price x Number of Tyre + Tyre Life km

Economic Tyre Cost = Economic Tyre Unit Price x Number of Tyre + Tyre Life km

B-4 Maintenance Cost

- Parts
Financial Maintenance Parts Costs = Financial Vehicle Price + Vehicle Life Km x Maintenance Spare Parts per year (%)
Economic Maintenance Parts Costs = Economic Vehicle Price + Vehicle Life Km x Maintenance Spare Parts per Year (%)

- Labor

Financial Maintenance Labor Costs = Financial Maintenance Labor Cost (per hour) x Maintenance Labor Working Hour (per year) + Vehicle Annual Km Economic Maintenance Labor Costs = Economic Maintenance Labor Cost (per hour) x Maintenance Labor Working Hour (per year) + Vehicle Annual Km

B-5 Depreciation Cost (A part of Capital Cost) - distance related portion

Financial Depreciation cost = Financial vehicle price + Vehicle life Km x depreciation distance related (60%)

Economic Depreciation cost = Economic vehicle price + Vehicle life Km x depreciation distance related (60%)

Capital Cost

Capital cost consists of depreciation cost and interest cost. The depreciation cost is composed of two parts: distance-related portion and time-related portion. For the estimation of capital cost: price of vehicle cost has to estimate in advance both its financial and economic cost.

	Depreciation - distance related	Running cost
Capital Cost	Depreciation - time related	Fixed cost
	Interest	Fixed cost

B-6 Depreciation - time related

Financial Depreciation Cost = Financial vehicle price ÷ Vehicle life Km x depreciation distance related (60%)

Economic Depreciation Cost = Economic vehicle price ÷ Vehicle life Km x depreciation distance related (60%)

B-8 Crew Costs

Crew number (1) x Financial crew cost per hour = Financia	al crew cost
Crew number (1) \times Economic crew cost per hour = Financia	al crew cost

B-9 Overhead Cost

This component is the most difficult to estimate; often a certain percentage (between 10 and 20%) of total annual cost is applied.

B - 10 Registration Cost (License and Insurance)

Registration tax per year + Vehicle annual operating hours = Financial registration costs

0 = Economic costs

B-11 Factor

Pleet reduction factor = Productivity of Saving hour (%)

B - 12 Equation of Vehicle Operating Costs

Passenger car	$y = 0.514 x^2 - 79.84 x + 4828$
Bus	$y = 0.745 x^2 - 93.19 x + 6186$
Truck	$y = 0.972 x^2 - 126.88 + 6952$
Motor Cycle	$y = 0.0083 x^2 - 7.53 x + 462$

Where: y = Vehicle Operating Costx = Running Speed (km/h)

Appendix A - 14.4 Explanation and Calculation of Benefits Resulting from Ferry Saving Cost

(1) Methodology

- 1. First, the total present value of the total waiting time of ferry traffic will be calculated, discounted at 12% of the capital.
- 2. The total present value of ferry's additional investment cost discounted at capital opportunity cost (12%) is then calculated.
- 3. When both totals are equal to each other, investment is feasible. The investment scale of ferry boat, proven economically feasible, will be calculated as a benefit of the bridge (As ferry boat will not be used if bridge is constructed).

(2) Amount of Ferry Traffic in the Case Without Bridge

- 1. All traffic on route 18 uses the ferry. For example in year 2000, the passenger car unit (PCU) will be 3,830 per day.
- 2. Traffic volume at peak hour will be 383 per hour; 10% of the total traffic volume.
- 3. Minimum number of ferries required:
- Ferry capacity of 20 PCU per boat
- Ferry trips twice in one hour: so that it carries 40 PCU per hour.
- During the peak hour; number of ferry vessels required will be 10: 383 + 40 = 10.
- The average ferry traffic volume per hour is 4 vessels; 383 vehicles + 24 hours = 160 vessels + 40 = 4 vessels.
- 4. Thus, minimum number of ferry vessels required is four (4), and at peak hours ten (10). For the present calculation, the average figure of 7 was used. By the year 2019 the daily ferry traffic would require 67 vessels.

5. Number of Ferry Boats Needed and the Cost.

The life span of a ferry is 6 years. The average yearly amount of purchase is 3 vessels. A ferry costs 12 million dong, and it is traded in for a new one at 20% six year later.

There are other costs; (1) harbor, (2) ticket sales office and waiting facilities, (3) operational costs such as personnel expenses, and fuel expenses, (4) management costs. Ferry costs by the year 2019 will be 374.168 million dong.

Table A - 14.4.1 Section 2 Pha Lai Ferry Projection (for Comparison with Bridge Substitution)

Traffic = Passenger Car Unit

SAN WAR THE COLUMN THE SERVICE OF	Total	Peak	Max.	Hour	Min.	Ave.
	a	$b = a \times 10\%$	c = b/40	d = a/24	e = d/40	f = (d+f)/2
	Vehicle	Vehicle	Ship	Vehicle	Ship	Ship
2000	3,830	383	10	160	4	7
2001	4,839	484	12	202	2	9
2002	6,118	612	15	255	6	11
2003	7742	774	19	323	8	14
2004	9805	981	25	409	10	17
2005	12427	1243	31	518	13	22
2006	13530	1353	34	564	14	24
2007	14736	1474	37	614	15	26
2008	16050	1605	40	669	17	28
2009	17485	1749	44	729	18	31
2010	19049	1905	48	794	20	34
2011	20575	2058	51	857	21	. 36
2012	22621	2262	57	943	24	40
2013	24656	2466	62	1027	26	44
2014	26878	2688	67	1120	28	48
2015	29306	2931	73	1221	31	52
2016	31211	3121	78	1300	33	55
2017	33240	3324	83	1385	35	59
2018	35400	3540	89	1475	37	63
2019	37701	3770	94	1571	39	67
2020	40152	4015	100	1673	42	71
2021	42762	4276	107	1782	45	76
2022	45541	4554	114	1898	47	81
2023	48501	4850	121	2021	51	86
2024	51654	5165	129	2152	54	91
2025	55011	5501	138	2292	57	97
2026	58587	5859	146	2441	61	104

(3) Benefits of Waiting Time Saved

Large ferry cost should be analyzed feasible for investment. Benefits of ferry investment: (1) saving of waiting time, (2) year-round river crossing made possible (including flood periods).

For the calculation of waiting time saving, normal traffic and generated traffic is included. Diverted traffic is not counted.

About 12% of the normal and generated traffic have to wait in line for 2.5 hours during the peak hours.

For instance, in year 2000 normal traffic volume of passenger car per day will be 219.

- Cars waiting in line is 26 vehicles; 12%

- Time value per hour is 11,048 dong a vehicle and 7,730 dong per passenger = 18778 dong.

Therefore, when 219 vehicles wait in line for one hour, the cost will be 493,486 dong.

Waiting time value of the generated traffic is a half.

(4) Flood Benefit

Flood occurs 2-3 times every year. Once in 8 years, water level reaches to P=12%, and it becomes impossible to cross for 5-7 days. Users of the ferry make a detour of 25km to National Highway No.1, and of 80.8 km to Highways Nos.5 and 183.

Driving cost and time benefit is calculated for the case of ferry stop for 3.5 day a year. The analysis includes only the normal traffic.

(5) Investment feasibility analysis

Table 14.28 is the analysis of cost and benefit comparison for the case of ferry investment. Discounted at 12% of the capital opportunity cost. Benefit shows more than cost.

Thus the ferry investment is feasible from a national viewpoint. Lastly, this investment amount is regarded as a benefit of bridge construction, because ferry investment will be saved by the bridge.