sustainable growth including the maximization of positive impacts of development and the minimization of potential environmental hazards. The NWRMP including the need for an integrated environmental protection program and related strategies and countermeasures are suggested in some detail in the Chapters as mentioned above. In particular, the provision for fish ladders and other passage ways for fish movement up and down stream should be carefully examined during the stage of project identification and preparation, and when a negative conclusion is a given to the project economic viability as a whole, this project may be subject to suspension or cancellation.

For reference, a proposal on the technical cooperation program "EIA Study and Environmental Monitoring Program for Dam Projects" to be assisted by the external agencies which should be carried out during an early period of the National Water Master Action Plan by 2000 is compiled in Appendix 11-9 of Chapter 11.

# 4.5 NATIONAL WATER MASTER ACTION PLAN TOWARDS THE YEAR 2000

# 4. 5. 1 BMR Program for the Existing Dams

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# (1) Establishment of Integrated Water Management Rule in the North Region

The surface water of the Hadejia and Rima basins in the North region has been mostly developed by the existing dams. The outline of water resources development and service area under the existing dams is summarized as follows:

700	Barriel Street	Item		Hadejia Bas	in Sokoto-Rima
1	Potential su	rface water	(MCM)	1,700	2,500
2.		xisting Dams	(No)	14	8
3.	さいいく かいしょうい げんだま				
		eservoir Capacity	(MCM)	3,010	1,660
		e Reservoir Water		1,130	910
4.	Public Irriga				
	(1) Potentia	l Area	(10³ha)	and the $41\%$	an an 60
	(2) Actual I	rrigation Area	(10 <sup>3</sup> ha)	15	6
5.	Private Irrig	gation	in the second se		
11.	(1) Potentia	l Area	(10³ha)	66	, , , , , , , , , , , , , , , , , , ,
	(2) Actual I	rrigation Area	(10³ha)	34	16
6.	Water Supp	ly by Surface Wate	er		
٠.,	(1) Actual 8	upply in 1991	(MCM)	71	68
	(2) Water I	emand in 2020	(MCM)	253	82

As is clear in the above table, the Hadejia and the Rima basin have large available reservoir water of 1,130 MCM and 910 MCM respectively under the large active reservoir capacity. The available reservoir water, however, has not been used effectively for irrigated agriculture due to inadequate irrigation system in the service area. The actual public irrigation area in both basins is 21,000 ha against the potential area of 91,000 ha. Although the domestic water demand towards the year 2020 will increase to 335 MCM a year in both basins, this could be satisfied by the available water of the existing dams. FMWRRD intends to implement the irrigation system at the undeveloped service area towards the year 2000 and the water supply agencies will implement the waterworks to respond to the increasing domestic water demand towards the year 2020. The water management in both basins for the effective use of reservoir water, however, is rather difficult as compared with that in the other basins of the Central and South regions due to the following reasons:

8

- Potential surface water decreases by Sahelian Drought.
- Active reservoir capacity in the dams of Tiga, Challawa and Goronyo is very large against their reservoir inflow.
- Some of the existing reservoir dams located in the upstream basin of the Challawa and Goronyo use the surface water preferentially for the service area, as a result the inflow of the Challawa and Goronyo will be decreased.
- Large Fadama area lying in the downstream of damsites requires the water to be released from the reservoir.
- Large seepage loss will take place along the rivers between damsite and Fadama area.

If the integrated water management rule is not set up by solving the above problems, the reservoir water will be released randomly, as a result the water management in the irrigation system and waterworks in the service area could not be made adequately.

With this background, the integrated water management rule in both basins shall be urgently set up as explained in 4.4.3. The water management study will be commenced firstly in the Hadejia basin with the following items:

- (a) Data Collection Related to the Study by the Inventory Survey
- (b) Installation of Gaging Stations

Site	Rainfall/Evaporation	Surface Water	Sub-Surface <u>Groundwater</u>
Damsite1	2	4	-
Along River	_	6	
Wet Land	· <u>-</u>	-	5
Total	2	10	5

- (c) Topographical survey and discharge measurement at gaging stations.
- (d) Hydrological analysis by the runoff model at the representative damsites.
- (e) Water balance study at the wetlands.
- (f) Water demand study for irrigation and water supply in the service area as well as for the wetlands.
- (g) Integrated reservoir operation study for Tiga, Challawa, Watari and the other medium scale dams.
- (h) Rehabilitation plan for the existing dams, irrigation system and waterworks.
- (i) River improvement plan to minimize water loss.
- (j) Preliminary design for the above rehabilitation and river improvement works.
- (k) Formulation of the integrated water management rule and OM plan including OM organization.
- (l) Environmental Impact
- (m) Implementation program

The guideline for the integrated water management study is as shown in Appendix 4-1. Work schedule for the above study is as follows:

Item	1996	1997	1998	1999	2000
1. Hadejia Basin	<del></del>		<del>&gt;</del>		
(1) Review & Analyse of Data	<u></u>				
(2) Data Collection		1.			
(3) Site Survey			N 12		
(4) Installation of Gaging	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1				
Stations		15.		State of Maria	
(5) Water Management practice					
(6) Formulation of Management Plan		\$   **			
2. Sokoto-Rima Basin			•		· · · · · · · · · · · · · · · · · · ·

#### (2) Rehabilitation Works

#### (a) Ordinary Rehabilitation Works

The ordinary rehabilitation works will be composed of the items as mentioned in 4.4.2 (3). Dam and Reservoir Operation Department in FMWRRD has carried out the survey to identify the defects of the existing dams. Accordingly the rehabilitation program for the defects will be urgently set up in the following manner:

- Review of inventory survey result by site survey and identification of the size of rehabilitation works.
- Classification of the rehabilitation works from view point of size, urgency and execution method by RBDA force account or by the contractor on contract basis.
- Detailed design and construction plan for the rehabilitation works.
- Provision of construction equipment, if RBDA or OM office implement the works.
- Execution of the works.

#### (b) Particular Rehabilitation Works

As for the particular rehabilitation works in the Goronyo and Bakolori dams, the survey, analysis, detailed design, construction method, specifications for construction shall be either executed or prepared by recruiting the consultants.

The rehabilitation works will be executed by the contractor under the supervision of the consultants.

## (c) Rehabilitation Work Schedule

The rehabilitation work schedule is set up as follows;

Item		1996	1997	1998	1999	2000
1. Ordinary Rehabilitati	on W	orks				
(1) Review of Inventory Survey Result						
(2) Detailed Design/ Construction Plan		-				:
(3) Procurement of Equipment			<u> </u>			
(4) Execution of Works	Programme Total	1 19 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1				
2. Particular Rehabilitation Works	4.					
(1) Survey and Design (2) Execution of Works		3. 31 3 <u>1 3.54</u>				

# 4.5.2 Preparation of the Medium and Small Package Program

# (1) Preparation Method

The medium and small dam package program will be prepared by the year 2000 aiming to implement the program after the year 2000. One model river basin with priority to prepare the program will be firstly selected by each RBDA and FMWRRD, although JICA Team proposes tentatively the selection guideline and the proposed basin at each RBDA as described in 4.4.4 (6). The preparation work of the program after selection of the model basin will be carried out by recruiting the consultants.

# (a) Master Plan or Pre-Feasibility Study

The basinwide master plan or the pre-feasibility study will be made prior to the feasibility study with the following purposes:

- Identification of a number of the proposed dams and summarization of their outline.
- Analysis of the potential surface water resources.
- Study of the water balance of the potential water resources and the available water to be developed by the proposed dams.
- Selection of the priority tributary basin and the representative dams for the feasibility study.
- Formulation of survey and study items for the feasibility study.

## (b) Feasibility Study

The feasibility study will be performed in the above selected tributary basin and the representative dams. The representative dams will be composed of three to five medium and small dams with different size. The study will be made with the international feasibility study level including hydrological analysis, reservoir plan, preliminary design, cost estimation, environment impact, project evaluation, etc.

## (c) Formulation of Package Program

In accordance with the feasibility study result, the package program is formulated in the following manner;

- Outline of all proposed dams identified in the master plan will be reviewed based on the data of the feasibility study for the representative dams.
- The proposed dams to be implemented with priority are selected based on the above reviewing result and taking into account the project and budget size, project economy, urgency of each sub project, etc.
- Package program will be classified into the sub-package program consisting of three to five proposed dams.

## (2) Particular Attention for the Study

## (a) Reservoir

- Active reservoir capacity will be planned with 60 to 70 percent of average annual inflow. The reservoir with large active capacity against average annual inflow will not be filled up to the full water level in dry year, as a result the reservoir operation will be difficult.
- Damsite to be able to obtain the large reservoir capacity by low dam height and short dam length will be selected with the priority for implementation.
- Dead water capacity in the reservoir will be carefully studied taking into consideration sediment transport based on the size of the catchment area. Small dams under the large catchment area shall not be selected.
- The service area shall be located near damsite.

## (b) Dam Standard Section

The center core type fill dam as shown in the Database Drawings (24) will be applied for the proposed dams taking into account the prevention of seepage water through dam foundation and the stability of dam body.

# (c) Spillway

Spillway will be planned with the non controlled weir without gate. The emergency spillway will be planned, if there should be a suitable site.

## (d) Intake and Outlet

- Intake elevation shall be placed so as to carry out the gravity irrigation from the reservoir to the service area.
- Intake will be designed with drop inlet or inclined inlet instead of the intake tower in order to minimize the construction cost.
- Outlet conduit will be designed by reinforced concrete without steel pipes, because of the low reservoir water head of 15 to 25m (medium and small dams).

#### (e) Construction Period

The medium dams will be constructed in two years, while the small dams in one year.

## (3) Work Schedule for the Study

Work schedule for the study of the package program will be as follows:

Item	1996	1997	1998	1999	2000
1. Model Basin with Priority					
(1) Master Plan Study	a liver in the second				
Data Collection &	the diameter		4 4	10000	
Review 197	-		er in the	44	·
Site Survey				. 7.6	
Analysis & Study					
(2) Feasibility Study					
Topo-Survey					
Geological Investigation					
Construction Material					in the state
Survey					
Analysis and Study	5 1 k jags	·			1: -
(3) Preparation of					
Implementation				<del></del>	garanti (f
2. Additional Basin Study		1 10 13	<u> </u>		

# 4.5.3 Guideline for Water Management, Dam Planning and OM Work

The integrated water management study in the North region, the rehabilitation works under OM office and dam planning study for the medium and small dam package program will be mainly implemented by the year 2000. In order to carry out the above study and works smoothly and properly, the guideline is prepared and given under Appendices 4-1, 4-2 and 4-3.

It is necessary that NWRI in Kaduna shall emphasize the study to formulate more detailed guideline for the planning and designing and to upgrade the dam engineering through staff training. It is also necessary to request the foreign technical aid to fulfill the above work in NWRI.

# 4. 5. 4 Implementation Program

The implementation program for the study and works by the year 2000 is summarized as follows:

Item	1996	1997	1998	1999	2000
Integrated Water     Management in the north region					
(1) Hadejia Basin (2) Sokoto Rima Basin					
2. Rehabilitation Works (1) Ordinary					
Rehabilitation (2) Particular Rehabilitation				·	
3. Preparation of Medium and Small Dam Package				: .	
Program (1) Model Package Program					
(2) Second Package Program					

# 4.5.5 Terms of Reference for Water Resources Management and Medium/Small Dam Package Program

Terms of Reference for the water resources management in the upper Hadejia and for the medium/small dams package program is prepared as shown in Appendices 4-4 and 4-5.

TABLE 4.2.1 NUMBER BY SIZE OF EXISTING DAMS

	1	II .	Ш	Ŋ	v	VI	W	VII	Total
1. Dam Height									
>50 m	1	1	0	Ö	. 0	0	0	0	2
30∼50 m	1	6	4	0	0	2	0	3	16
15~30 m	7	12	8.	4	0	4	1	9	45
<15 m	11	13	9	9	7	26	11	11	97
Total	20	32	21	13	7	32	12	23	160
2. Dam Length									1
>3 km	4	2	1	0	. 0	0	0	8	15
3~2 km	1	2	0	0	0		0	7	10
1~2 km	0	8	- 5	1	0	3	0	3	20
0.5~1 km	6	7	4	4	0	10	0	1	32
<0.5 km	• 9	8	8	8	0	8	5	3	49
Total	20	27	18	13	0	21	5	22	126
3. Reservoir Capacity									
>1,000 MCM	1	2	1	0	0	0	0	2	6
500~1,000 MCM	1	0	1	0	0,	. 0	0.	1	3
100~500 MCM	3	3	0	0	0	2	0	3	11
10~100 MCM	3	14	9	1	0	5	1	11	44
5~10 MCM	2	3	0	2	0	1	0	3	11
<5 MCM	8	8	5	8	1	24	1	2	57
Total	18	30	16	11	1	32	2	22	132
4. Reservoir Area									
>100 km²	3	3	2	1	0	0	0	3	12
50~100 km²	1	0	0	0	0	1	0	0	2
10~50 km²	3	4	1	0	0	2	<b>O</b> jj	8	18
5~10 km²	2	6	3	0	0	1	0	4	16
<5 km²	6	4	11	10	0	8	1	6	46
Total	15	17	17	11	0	12	1	21	94

TABLE 4.2.2 EXISTING NOTABLE DAMS

Н.А	Dam	Active Capacity	Dam Héight	Objective	Completed Year
		(MCM)	(MCM)		
1	Zibiya	121	21.5	Irri/W.S	1990
1	Zobe	170	18.9	Irri/W.S	1983
1	Bakolori	403	48.0	Irr/H.P	1982
1	Goronyo	833	20.0	Irri/W.S	1984
1	Kainji	11,500	65.5	н.р	1968
1	Kubli	62	23.0	Irri	1992
2	Kontagora	200	32.0	Irri	U.C
2	Asa	344	27.0	W.S	?
2	Kagara	39	31.0	W.S	U.C
2	Jebba	1,000	40.0	Н.Р	1983
2	Omi	220	43.0	Irri	U.C
2	Zaria	29.8	15.0	W.S	1974
2	Kangimi	59.3	19.2	W.S/Irri	1975
2	Shiroro	6,050	105.0	Н.Р	1989
2	Suleja	48.5	27.8	w.s	?
2	Ussuman	100	45.0	w.s	1984
3	Balanga	63	41.0	Irri	1987
3	Dadin Kowa	1,770	42.0	Irri/H.P	1988
3	Kiri	325	37.0	Irri	1982
4	Doma	28.5	15.7	Irri	1988
6	Ikere Gorge	565	47.5	Irri/W.S/H.P	U.C
6	Oyan	254	30.4	Irri/W.S/H.P	1983
8	Erinle	92.5	27.0	w.s	1989
8	Gari	203	22.0	Irri	1980
8	Challawa	900	38.0	Irri/W.S	1992
8	Watari	92.7	19.8	Irri	1980
8	Tiga	1,845	47.2	Irri/W.S	1975
8	Kafin Zaki	2,500	40.0	Irri	U.C
8	Tomas	56.6	13.7	Irri	1976
8	Jakar	54.5	14.3	Irri	1976
8	Alau	106	9.5	Irri/W.S	1992

Irri: Irrigation

W.S: Water Supply

H.P: Hydropower U.C: Under Construction

TABLE 4.2.3 DISTRIBUTION OF EXISTING DAMS ON SHA BASIS (1/2)

			Irrigat	ion Dan				W	ater S	upply Da	ım	
SHA		Large	Sı	mall	Т	otal	L	arge	Sı	mall	Т	otal
	No	Ac	No	Ac	No	Ac	No	Ac	No	Ac	No	Ac
HA-1 101	1	121			1	121						-
102	1	170	5 A		1	170	·					-
103	; - I	403			-	=	·		1	25	i	25
104	1	933			1	403	1	3	· 1	5	2	8
103	1		1	20	2	953					<i>;</i>	
109				*.	-	-	1	5		i fing	1	5
110		***	1	1	1	1			3	6	3	6
111			1	2	1	2			1	0	1	0
113	;		1	0	1	Ó			1	0	1	0
114	2	75		1:	2	75	5.			470 . 4	-	1 -
Sub-total	6	1,702	4	23	10	1,725	2	8	7	36	9	44
HA-2 203	1. 1	200			1	200	1	16			1	16
204					- 1 <u>-</u>	-	1	34	1	0	2	34
205			1	0	1	0	1	19		4.11	1	19
200	-	1	. ` `	:	-		2	43	i	3	3	46
207	1	18		: : : : :	1	18		i i	* * * * * * * * * * * * * * * * * * * *		4	-
208	1	220	2	18	3	238			1	12	1	12
209			1	3	1	3		100			·	-
211			3	9	3	9	1	30	1	3	2	33
212		1			<u>-</u>	1, 7 1 <u>1</u>	1	59			1	59
214	.	1	1	21	1	21	2	27		Frank)	2	27
216			1	0	1	0	5	195	100		5	195
Sub-total	3	438	9	51	12	489	14	324	4	18	13	441
HA-3 303	2	335			2	335					-	
304	2	1,785			2	1,785	6	116	7	12	13	128
305	1	63	1	7	2	70		1 1		7944	· .	
310	)			5.4	-	-	1	4			1	4
Sub-total	5	2,183	1	7	6	2,190	7	120	7	12	14	132
HA-4 401			1	0	1	0	1	3	4	4	5	7
403	1	2			1	2				1 1	-	-
406	•		2	4	2	4				0		\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \
407	1	29		4.14	1	29			1	0	1	0
408	1				-				1	4	1	0
Sub-total	2	31	3	4	5	35	1	3	6		7	7
HA-5 501			1	0	1	0				0	•	
502	<b>:</b>		1	0	1	0			1	0	1	0
504			3	0	3	0			1	0	1	0
Sub-total	0	0	5	0	б	0	0	0	2	0	2	0

Remarks: AC Active reservoir capacity (Unit MCM)

TABLE 4.2.3 DISTRIBUTION OF EXISTING DAMS ON SHA BASIS

			Ir	rigati	on Dam				Wa	iter Su	pply Da	m	7 7 7
SH	A :	La	rge	Sn	iall	To	tal	La	rge	Sn	nall	To	tal
	1	No	Ac	No	Ac	No	Ac	No	Ac	No	Ac	No	Ac
HA-6	602	3	824	7	9	10	833			3	11	3	11
٠.	603	l		1	7	1	7			1		-	-
	604		- [		l l	-	-		. 1	ı	3	1	6
	605		l			<u>.</u>		2	123	6	20	8	143
	607				41.2		~-			1	9	1	. 9
	608					•		1	20	6	23	7	43
	610					-				1	1	: 1	1
Sub-to	tal	3	824	8	16	11	840	3	143	18	70	21	213
HA-7	702					-	-	1	2		1	1	2
	703	l l'		2	0	2	0			4	0	4	0
1	705		:	1	0	1	0		:			-	-
	706			2	0	2	0			1	0	1	0
Sub-to	tal	0	0	5	0	5	0	1	2	5	0	6	2
HA-8	802	1	203	2	111	3	314			1	7	1	. 7
	803	4	1,032	1	6	5	1,038	1	15	2	13	3	33
	804	4	1,883			4	1,883					-	-
	805	1	2,500	1	20	2	2,520					% <u>*</u>	-
	806		ugibi s	, 1,11		_	-		1	1	1	1	1
	807			2	24	2	24	1	25	٠,		1	25
	813			1	106	1	106					-	-
Sub-te	otal	10	5,618	7	267	17	5,885	2	40	4	26	6	66
Total		. 29	10,796	42	368	71	11,164	30	739	53	166	83	905

Remarks: AC Active reservoir capacity (Unit MCM)

TABLE 4.2.4 DISTRIBUTION OF EXISTING DAMS BY REGION AND OBJECTIVE

1. Large Dam  Irrigation  Water Supply  Hydropower  Total	North West HA-1		orth East	Centr	Central West	Cent	Central East	8 1 1 1	South West	8 1	South East	Ž	Total Nationwide
Ž. s			HA-8	4	HA-2	ı,	HA-3, 4	μű	HA-6	4	1,6-MI	3	
lly r		No	A.C	ON	A.C	No	A.C	No	A.C	No	A.C	No	A.C
>													
>	1,702	10	5,618	က	438	7	2,214	က	824	0	0	83	10,796
Hydropower 1 Total 9	<b>89</b>	63	40	14	423	∞	123	တ	143	ret	62	8	739
Total 9	11,500	0	0	87	7,050	က	49	0	0	0	0	φ	18,599
	13,210	12	5,658	19	7,911	8	2,386	φ	796	r-t	83	8	30,134
2. Small	1.1.			:				:					
Irrigation 4	23	7	267	on .	52	4	Ħ	80	16	2	0	42	368
Water supply 7	36	4	26	4	18	13	16	21.8	70	<b>~</b>	0	23	166
Hydro power 0	<b>o</b> ,	0	0	0	0	0	0	0	0	0	0	0	0
Total 11	62	11	293	13	69	17	27	56	86	17	0	95	534
3. Dam, Total													
Irrigation 10	1,725	17	5,885	12	489	Ħ	2,225	Ħ	840	10	0	Ę	11,164
Water supply 9	4	ဖ	99	18	441	ដ	139	21	213	òò	8	88	905
Hydro power 1	11,500	0	0	63	7,050	က	49	0	0	0	0	ဖ	18,599
Total 20	13,269	83	5,951	32	7,980	35	2,413	32	1,053	18	8	160	30668

Remark, Large dam > Height of 15m, Small dam < Height of 15m

A.C. Active reservoir capacity (Unit MCM)

COMPARISON OF RESERVOIR INFLOW AND ACTIVE RESERVOIR **TABLE 4.3.1** CAPACITY IN EXISTING LARGE IRRIGATION DAMS

					, <del></del> ,
на	Dam	Reservoir Inflow (MCM)	Active Capacity (MCM)	Rate of Inflow/ capacity	Remarks
HA-1	Jibiya	260	121	2.1	<b>O</b> :
HA-1	Zobe	240	170	1.4	0
HA-1	Bakolori	760	403	1.9	0
HA-1	Goronyo	660	933	0.7	×
HA-1	Kubli	160	62	2.5	0
HA-2	Kontagora	270	200	1.4	0
HA-2	Omi	260	220	1,2	0
HA-3	Balanga	86	63	1.4	0
HA-3	Waya	29	15	1.9	0
HA-4	Doma	21	28.5	0.7	×
HA-6	Ikere Gorge	1,020	565	1.8	0
HA-6	Oyan	2,250	254	8.8	0
HA-8	Gari	120	203	0.6	×
IIA-8	Challawa	420	900	0.5	×
HA-8	Guzuguzu	22	21.5	1.0	Δ
HA-8	Magaga	24	17.2	1.4	0
HA-8	Watari	65	92.7	0.7	×
HA-8	Tudun Wada	18	16.6	1.1	Δ
HA-8	Tiga	830	1,845	0.4	×
HA-8	Bagauda	41	20.9	2.0	0
IIA-8	Kafin Zaki	1,060	2,500	0.4	×
HA-8	Kafin Chiri	35	24.6	1.4	0
HA-8	Tomas	59	56.6	1.0	Δ
HA-8	Jakara	56	54.5	1.0	Δ
HA-8	Garara	46	20	2.3	0
на-8	Alau	164	106	1.5	0

Remarks

○ Abundant inflow△ Poor inflow

Moderate inflow

Insufficient inflow

TABLE 4.3.2 EVALUATION OF EVAPORATION LOSS FROM RESERVOIR AREA (1/2

	Т			Professional Francisco	4	ta ili di	JIN ANEA	(112)
на	Dam	Reservoir Inflow (MCM)	Active Capacity (MCM)	Reservoir Area (km²) ②	Pan-Evapo Value (mm)	Evapo-Loss in Dry Season (MCM) ①=②×③×0.4	Evapo-Loss Rate(%)	Remarks
HA-1	Jibiya	260	121	26	3,500	36.4	30	0
	Zobe	240	170	45	3,500	63	37	0
	Bakolori	760	403	80	3,000	96	24	
	Goronyo	• 660	933	200	3,500	280	42	0
. :	Kubli	160	62	9.4	2,500	9.4	15	
1	Bubo	41	25.1	11.5	2,800	12.9	51	0
	Mairuwa	24	5.3	1.6	2,800	1.8	34	0
HA-2	Kontagora (2)	270	200	39	2,500	39	20	
İ '	Kagara	40	38	5.8	2,800	7.3	19	
	Omi	260	220	25.7	2,500	25.7	12	
	Zaria	450	29.8	8	2,800	9	30	0
1.	Kangimi	1,800	59.3	6.6	2,500	6.6	11	
	Tagwai	33	26.5	5.5	2,500	5.5	21	
	Iku	42	36.2	4.3	2,500	4.3	12	
	Suleja	42	48.5	7.4	2,500	7.4	15	
	Usuma	260	100	8	2,500	8	8	
	Tungan Kawo	34	21	4	2,500	4	19	1
HA-3	Liberty	33	15	1.1	2,800	1.2	8	-
	Balanga	86	63	11	2,800	12.3	20	
	Y.Gowon	30	24	5	2,500	5	21	
	Dadin Kowa	3,100	1,770	300	2,800	336	19	
	Gubi	45	35	5.9	2,800	6.6	19	
	Waya	29	17	4.5	2,800	5	29	. 1.
1.	Kiri	4,480	325	110	2,500	110	34	0
	Lantang	31	3.5	0.6	2,500	0.6	17	
* .	Tenti	22	9.8	: <b>4</b> ], /	2,500	4 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	41	0
	Cham	20	6.5	1.8	2,500	1.8	28	
HA-4	Pankshin	42	2.8	0.5	2,500	0.5	18	
	Doma	* 21	30	2.2	2,500	2.2	9	
HA-6	Ikere Gorge	1,020	565	53	2,000	42.4	8	
	Oyan	2,250	254	40	2,000	32	13	
	Asejire	1,860	30.5	5.3	2,000	4.2	14	d.
	Erinte	360	75	16.4	2,000	13.1	17	
	Awon	88	8.4	2	2,000	1.6	19	
	Eleyele	17	5.5	1.6	2,000	1.3	24	· [

TABLE 4.3.2 EVALUATION OF EVAPORATION LOSS FROM RESERVOIR AREA (2/2)

		and the second of the second o			and the second second	and the second s	and the second second second	
на	Dam	Reservoir Inflow (MCM)	Active Capacity (MCM)	Reservoir Area (km²) ②	Pan-Evapo Value (mm) ③	Evapo-Loss in Dry Season (MCM) ①=2×③×0.4	Evapo-Loss Rate (%)	Remarks
HA-8	Tundan Wada	18	16.6	3.5	3,500	4.9	30	0
	Gari	• 120	203	33.2	3,500	46.5	39	0
	Challawa	* 420	900	100	3,500	140	33	0
	Guzuguzu	22	21.5	6.4	3,500	9	42	0
	Magara	24	17.2	3.7	3,500	5.2	30	· · O
	Watari:	65	92.7	19.6	3,500	27.4	30	0
	Tiga	* 830	1,845	178	3,500	249	- 30	0
	Kafin Zaki	• 1,060	2,500	235	3,500	329	31	0
	Kafin Chiri	35	24.6	8.4	3,500	11.8	48	. 0 :
	Tomas	59	56.6	15	3,500	21	37	0
	Jakara	56	54.5	16.6	3,500	23.2	43	0
	Galala	46	20	1,1	3,500	1.5	8	
	Alau	164	106	50	3,500	70	66	0

Remark (1) Evaporation loss in dry season is estimated by the following assumption

(a) Average reservoir area during operation; Reservoir area × 0.8

(b) Evaporation from large surface area;

Annual Pan-Evapo, Value X 0.7

(c) Evaporation during dry season

Annual Pan-Evapo. Value X 0.7 (Wet season, 0.3)

(d) Rate:  $0.8 \times 0.7 \times 0.7 = 0.4$ 

(e) Evaporation loss in dry season;

Area X Pan - Evapo, Value X 0.4

(2) Evapo-Loss rate is estimated as follows:

Evapo-Loss in dry season + Active capacity (Reservoir inflow)

In case, reservoir inflow value is smaller than active capacity value, reservoir inflow value is adopted for available reservoir capacity, because active capacity is not filled by reservir inflow.

(3) Mark O shows presenting high evaporation losses.

SUMMARY OF EVALUATION FOR AVAILABLE RESERVOIR WATER IN EXISTING DAMS TABLE 4.3.3

•	4 4 1				Hydrologica	Hydrological Area (HA)				Į v t v
ltems	C Pit	н	73	8	4	Ş	9	2	<b>60</b>	Teger
1. Reservoir Inflow (RI)	MCM	3,850	4,320	7,880	200	•	14,270	1	3,280	33,800
2. Active Capacity (A.C)	MCM	1,750	940	2,280	09	20	1,030	8	5,940	12,050
3. Evapo-Losses (E.L.)	MCM	620	140	490	0	0	06	0	1,060	2,400
4. Total Available Water (AW)	MCM	980	790	1,570	20	202	890	40	2,120	6,460
5. Downstream Water (D.W.)	MCM	270	8	260	0	0	20	0	730	1,390
6. Irrigation Water (IW)	MCM	580	380	1,140	50	10	490	20	1,120	3,760
7. Water supply Water (W.S.W.)	MCM	130	330	170	30	10	350	50	270	1,310
Rate of Reservoir Water Use		-								
(1) A.C/R.I 2÷1	88	45	22	53	30		7	•	181	36
(2) E.L/R.I 3+1	2%	16	က	ဖ	0		<b>F</b> 4		32	<i>-</i>
(3) E.L/A.C 3+2	82	35	15	ឌ	0	0	6	•	18	8
(4) A.W/R.I 4+1	88	\$3	81	8	25	•	Ç	ı	65	13
(5) A.W/A.C 4+2	89	99	22	69	83	100	98	130	36	\$
(6) D.W/A.C 5+2	88	15	6	11	0	•	\$	0	12	11

Remark: (1) Reservoir inflow is not available at many existing dams in HA 5 and 7.

(2) In case reservoir inflow is less than active reservoir capacity, the inflow value is used for evaluation of available reservoir water.

TABLE 4.3.4 EVALUATION OF AVAILABLE WATER IN EXISTING DAMS (UNIT MCM)

		Objec	clive	Reservoir Inflow	Active Capacity	Evapo- Loss	Ca C	rry Over apacity	Available Reservoir		Reservoir W tream Water		Dry Seaso	
HA	Dam		<del> </del>				Rate	Capacity	Reservoir Water ()=() -(2-3)				THE STREET STREET	
. 144		ŧ	W.S.	0	0	2	(%)	3=0x%	-O-D	Rate (%)	Water (5)=(1)×%	Total ©=①-③	Irrigation	Water Supply
HA-1		1								· · · · ·				
1010	Jiblya	0	0	260	121	36	15	18	67	20	13	54	37	17
1020	Zobe	Ó	O	265	170	76	.	'	104	+	22	82	48	34
1030	Tubo	o	0	41	25	13	15	4	8	0	0	- 8	3	5
1040	Gusau	0	0	410	3	0	0	0	40	0	- 0	40	10	30
1040	Bakalori	O		799	403	139			326	-	104	222	222	0
1040	Mairowa	o	0	24	5	0	0	0	3	0	0	3.	1	2
1050	Goronyo	0	l o	• 710	933	342	١.		344	-	128	216	203	13
1050	Wurno	o	-	750	ŹÓ	6	0	0	14	0	0 1	14	14	0
1090	Zuru	-	0	420	5	0	0	0	10	0	0	10	0	10
1100	4 Small Dams	-	0				-		5		- '	5	0	5
1110	Bin Yauri	o	Ĭ	_	2	- 0	0	0 0	5	0	0	5	0	5
1110	Nasko	ŏ	.0	13	2	0	0	0	3	0	0	3	2	1
1130	2 Small Dams	ŏ	ő	l	4	0	0.	0	5	Ö.	0 .	5	2	3
1140		ŏ		160	62	9	10	6	47	10	5 .	42	42	0
	Subtotal	~		3,852	1,755	621		28	981		272	709	584	125
HA-2	Outroit.	-					1-			<u> </u>		l · · · ·	<u> </u>	
2030	Kontagora (1)	_	0	36	16	4	10	2	10	- 10	1	9	0	ġ
2030	Kontagora (2)	0	Ţ	298	200	36	:_		189		l u	178	178	0
2040	Asa		o	184	34	7	10	3	24	0	0	24	Ò	24
2040	Offa		o	27		1 1			3	0	0	3	0	3
2050	Ero	0	ø	149	19	4	10	2	13	0	0	13	6	7
2050	Oshin	O						j .	5	Ġ	0	5	5	0
2060	Kagara		0	40	38	7	20	8	23	20	5	18	o	18
2060	Bagoma	lo	0	150	5		0	0	15	0	0	15	6	9
2060	Birmin Gwari	0	0	150	3	0	1 0		15	0	0	15	5	10
2070	Guzán	o	l ŏ		18	4	20	1	13	0	0	13	- 10	3
2080	Omi	o	Ιŏ	260	220	26	20	44	150	20	30	120	100	20
2080	lkole		o	22	12	2	20	2	8 :	. 0	0	8	0	8
2080	Kpada	0			14	3	20	3	8	. 0	0	8	8	0
2080	Apariko	o	lo.	8	4	0	0	0	4	0	0	4	3	
2090	Kerawa	o	~		3	0	0	0	3	0	0	3	3	ġ.
2110	Zaria	Ĭ	o	450	30	9	0	0	21	. 0	0	21	0	21
2110	Kargi	0	-	1 1 1	3	0	0	0	5	0	0	5	5	O
2110	lkara	l o	0	32	3	0	0	0	3	0	- 0	3	1	2
2110	Matari	0		220	8	2	0	0	6	0	0	6	6	Q
2110		0		1 .	3	. 0	0	.0	: 5	0	0	. 5	5	O
2120	The second secon	0	0	1,800	59	7	0	0	52	10	5	47	22	25
2140			0	33	27	6	20	5	16	10	2	14	0	14
2140	The second secon	0	-	37	21	4	1 .	1 12 3	18		1	17	17	0
2160			0	86	5	0	0	0	10	Ò	0	10	0	10
			0	42	36	4	20	7	25	10	3	22	0	22
	Suleja	.	0	• 42	49	7	20	10	32	10	3	29	.	29
2160		-	О	-	5	0	0	0	10	0	0	10		10
	Ussuman		0	260	100	8	C	0	100	20	20	89	0	80
	Subtotal			4,317	935	140	i	111	785		81	705	380	325

Remarks, Mark \* shows dams having less inflow than active reservoir capacity.

TABLE 4.3.4 EVALUATION OF AVAILABLE WATER IN EXISTING DAMS (UNIT MCM) (2/3)

	te :	Obje	ctive	Reservoir Inflow	Active Capacity	Evapo- Loss	C <sub>8</sub>	rry Over	Available		Reservoir W		<del></del>	
HA	Dam	<b></b> -	T		Curacity	17753		I	Water	Downs	lream Water	lerigati	on/Water S	upply
		,	W.S.	0	0	<b>@</b>	Rate (%)	Capacity D=0×%	Reservoir Water ()=() -2-3	Rate (%)	Water ⑤=④×%	Total	Irrigation	Water Supply
HA-3											7-1			
3030	Biu	0	0		10	3	20	2	5	0	0	5	4	ì
3030	Kiri .	O		4,480	325	110	10	33	182	0	0	182	182	0
3040	Liberty		0.	33	15	1	20	3	11	0	0	11	0	11
3040	Y. Gowon	-	0	30	24	5	20	3	16	10	2	14	0	14
3040	Gubi	-	0	45	35	7.	20	7	23	10	2	21		21
3040	Waya	0	0	29	17	- 5	10	2	10	0	l 6	10	8	2
3040	Dadin Kowa	0	0	3,100	1,770	336	10	177	1,257	20	251	1,006	900	106
3040	Tenti	-	Q	22	10	4	10	1	5	0	0	5	ő	5
3040	6 Small Dams	-	0		,	<del>.</del> .,			10	0	0	10	0	10
3050	Balanga	0		86	63	12	20	13	38	0	0	38	38	0
3050	Cham	0	Ó	20	7	2	0	٥	5	0	0	5	4	1
3100	Lablang	١.	0	31	4	0	0	Ö	3	Ö	Ö	3	0 -	3
	Subtotal	]	!	7,876	2,280	485		726	1,565	,	255	1,310		
HA-4									1,000		200	1,010	1,136	174
4010	Panksin		O	42	3	Ö	0	0	5	0	0	5		
4010	5 Small Dams		0	_	15	o	0	o	15	0			0	5
4030	Bokkos	o	ŏ	16	2	o o	0	Ö	3		0	15	_	15
4060	Naka	O	o	4	2	0	0	0		0	0	3	2	1
4060	Umogiđi	ŏ	ŏ	12	3	Ö.	0		4	0	0	4	3	1
4070	Doma	o	ŏ.	* 21	30	2	20	0	3	0	0	3	2	1
4070	Lafia	9	o	44			20	4	15	0	0	15	12	3
4080	Nasarawa		0	64				_	5	0	0	5	0	5
	Subtotal			203	1	0	0	0	5	0	0	5	0	5.
HA-5	Daviotai			203	56	- 2		4	55		. 0 .	65	19	36
5010	2 Small Dams	0	0						1.1		100			
5020	1 Small Dams	0	o	•	6	0	0	0	5	Ó	0	5	2	3
5040	4 Small Dams	0	0	-	2	0	0	0	3	0	0	3	2	. 1
0040	Subtotal				8	0	0	0	10	0	0	10	6	* 4
HA-6	Suotetai				16	0		0	18		0	18	10	. 8
6020	TALLET .	ما											14	
6020	Igbojatye	0	0.	38	5	0	0	0	5	0	0.	5	3	2
6020 6020	Ikere Gorge	0	0	1,020	565	42	10	57	466	10	46	420	300	120
	Oyan	0	0	2,250	254	32	0	0	222	0	0 -	222	170	52
6020 6020	Ofiki (1) Ofiki (2)	0	0	200	1	0	0	0	. 1	. 0	0	1	1	0
	Fawfaw		O	190	1	0	0	. 0	1	0	0	1,	1	0
6020			0	16	1	. 0	0	0	· 1	0	0.	1	0.	1
6020	Igbohr	0	ò	20	1	0	0	0	1	0	0	1	1	0
6029	Sepeteri (A)	0	0	57	2	0	0	0	2	0	. 0	2	2	0
	Sepeteri (B)	O	0	54	1	0	0	0	1	0	0	. 1	1	0
6020	Awon		0	88	8	0	.0	0	- 8	0	0	8	0	8
	Lekan Are	0	0	14	1	0	.0	. 0	1	.0.	0	1	1	0
	Opeki		0	180	2	0	0	0	. 2	0	C	2	0	2
	Mekoloki Div.	O	0	5,150	0	0	0	0	0	0	0	2	2	0
	Oke Odan	O	0	15	7	0	0	0	7	0	0	7	5	2
	Eleyele	• •	0	17	6	0	Ö	0	6	0	0	6	0	6
6050	Asejíre	-	0	1,860	31	0	0	0	31	0	0 -	31	0	31

Remarks, Mark \* shows dams having less inflow than active reservoir capacity.

TABLE 4.3.4 EVALUATION OF AVAILABLE WATER IN EXISTING DAMS (UNIT MCM)

. •	ADEC 4.3.4	- T	ALUP		LHAM	ICADLE							<del> </del>	·
				Reservoir	Active	Evapo-	Car	ry Over	Available	:	Reservoir W			
на	Dam	Objec	ctive	Inflow	Active Capacity	Loss			Receivair	Downst	ream Water	Irrigati	on/Water S	upply
1111	Dam	1	w.s.	0	0	0	Rate (%)	Capacity 3=0×%	Wa'er 0=0 2-0	Rate	Water ⑤=①×%	Total (5) = (3) -(5)	Irrigation	Water Supply
6050	Elinle (1)		O	360	75	8	0	0	67	Û	0	67	0	67
6050	Otin	-	0	310	. 5	0	0	0	5	0	0	5	0	5
6050	Eko-Ende	:	0	310	5	0	o	0	5	. 0	0	5	0	5
6050	Ayiba	-	0	14	2	0	9	0	2	0	0	2	0	2
6050	Elinle (2)	-	0	670	4	0	0	0	4	0	0	4	0	4
6050	Oba	<del>.</del>	0	45	4	0	0	. 0	4	0	0	4	0	4
6050	Esa-Odo	0	Ø	355	- '	-		7.	-	0	0	0	0	0
6070	Owena	О	0	48	9	2	0	0	i	0	0	3	2	5
6080	Egbe	-	0	670	20	2	Ó	0 '	18	. 0	0	18	0	18
6080	Owo	•	0	9		-	-		2	- 0	0	2	0	2
6080	Ado-Ekite	-	0	9	9	2	50	2	5	0	0	5	0	5
6080	Ojirami		0	10	14	1	0	0	3	0	. 0	3	0	3
6080	Ayede	1.	0	20		: •	· -		3	0	Ö	3	0	3
6080	lkare	-	0	9	8	2	20	2	4	0	Ò.	4	0	4
6100	Ikpoba		0	260			1	•	5	0	0	5	0	5
1	Subtotal			14,263	1,033	91	4	.61	891		46	845	489	356
HA-7			<del>                                     </del>		<b> </b>			:						
7020	Obudu	o ·	0	15	2	0	0	Ö	3	Ó	0	3	1	2
7030	6 Small Dams	0	O		18.	o	0	- / O	20	0	0	20	8	12
7050	Umopara	0		1 . ±	3	0	1.0	0	5	0	0	5	3	2
7060	3 Small Dams	o	0		9	1	0	- o	- 8	Ò	0	8	5	3
	Subtotal		i .	15	32	1		0	36	1	0	36	17	19
HA-8			1			<u> </u>								
8020	Gari	O	0	* 120	203	47	20	24	49	0	0	49	40	9
8020	Ibrahim Adamu	o	0	-	7	4	0	0	3	0	0	. 3	1	2
8020	Tomas	O		65	57	25	-		25	-	6	19	19	Ò
8020	1	0	_	65	55	26	-	-	24		3	21	21	0
8030			0	16	15	3	20	3	9	. 0	0,	9	-	9
8030	Challawa	0	0	460	900	130			330		130	203	130	70
8030	Guzuguzu	o		22	22	9	20	4	9	0	0.	9	9	0
8030	T	0	10	24	17	5	20	3	9	0	0	9	7	2
8030		o	lō	• 75	93	32			20	1	3	17.	17	0
8030		ŏ		9	6	3	20	1	2	0	0	2	2	0
8030	1	ŏ	0	13	11	5	20	2	4	1 0	0.	. 4	2	2
8030		Ĭŏ	o	9	8	1 4	20	2	2	0	0	2	1	1
8040		o		18	17	5	20	3	. 9	1 0	. 0	9	9	. 0
8040	1 1	0	0	• 910	1,845	270			700		270	430	280	150
8040		o	o	41	21	5	10	2	14	0	0	14	4	10
8050		l ŏ		1,170	2,500	400	] .		860		310	550	550	0
8050	1	o		46	20	2	10	2	16	10	2	. 14	14	0
8070		0	0	35	25	12	20		8	0	0	8	3	5
8070		o	0	21	10	5	10		4	0	0	4	3	1
8130	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	0	o	164	106	70	10		25	10	3	22	12	10
0130	Sublotal	1 .	~	3,283	5,938	1,062	"	63	2,122		727	1,395	1,124	271
2	Total	+	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	33,814	12,045		1.0	993	6,454		1,381	5,073	3,759	1,314
بنجساخ	1 Locat	<u> . ا ن ا ا</u>		00,014	1.0,020	1 2,100	<del></del> -							

Remarks: (1) Mark \* shows dams having less inflow than active reservoir capacity.

<sup>(2)</sup> Evaluation value of DAM, described by bold letter in the table is estimated by the reservoir operation study result of the JICA Team.

TABLE 4.3.5 EVALUATION OF DEAD RESERVOIR CAPACITY & SPILLWAY FLOOD CAPACITY BASED ON CATCHMENT AREA (1/2)

		Catchment	Reservoir	<b>A</b>	Dead R	leservoir Cap	acity	Spill	way Flood Capa	city
НА	Dam	Area (km²)	Area (km²)	Active Capacity (MCM)	Design (MCM)	Specific Yield (m <sup>9</sup> /km <sup>2</sup> )	Remark	Design (m³/sec)	Specific Yield (m³/sec/km²)	Remarl
HA-1	Jibiya	3,700	26	121	21	110		2,200	0.6	
: .	Zobe	2,309	45	170	7	60	•	1,087	0.5	
	Bakolori	4,857	80	403	47	190		3,750	0.8	
	Goronyo	21,445	200	933	9	10	2 0 1 0 0 1	1,540	0.1	•
	Zuru	3,170	23.5	5	1	10		432	0.1	•
	Kubli	791	9.4	62	13	330		522	0.7	
	Bubo	171	11.5	25,1	10.4	1,220	0	220	1.3	
	Mairuwa	120	1.6	5.3	0.2	30	•	43	0.4	
	Nasko	52	0.6	2.0	0.6	230		40	0.8	
HA-2	Kentagora (2)	2,000	39	200	140	1,450	0	240	0.1	0
	Asa	918		34,4	8.6	190			•	
	Ero	610	: -	18.5	1.5	50	•	*		
	Кадага	158	5.8	38	5	630	0	197	1.2	
	Omi	1,640	25.7	220	14	170		3,550	2.2	
	Zaria	3,200	8	29.8	16	100				
	Kangimi	14,946	6.6	59.3	14.8	20	1			
	Tagwai	110	5.5	26.5	1.8	330				
	Iku	144	4.3	36.2	6.5	900	0			
	Suleja	144	7.4	48.5	3.5	490	0	1		
	Ussuman	1,315	8,0	100	20	300				
	Birnin Gwari	594	2.0	3.2	0.8	30		56	0.1	•
	Apariko	21		4.0	0.7	670	0	134	6.4	0
	lkara	108	.	3.0	0.4	70		400	3.7	
	Matari	1,090	36	8.2	4.4	80	•	1,800	1.7	- F T :
;	Tungan Kawo	166	4	21.0	1.0	120		85	0.5	
HA-3	Liberty	113	1.1	15	5	890	0	96	0.8	
-	Balanga	385	11	63	10	520	0	2,500	6.5	0
	Y. Gowon	100	5	24	6	1,200	0	248	2.5	
1	Dadin Kowa	32,700	300	1,770	1,085	660	0	1,110	0.03	•
	Gubi	179	5.9	- 35	3	340		-		
	Waya	106	4.5	17	4	750	0			
1	Kiri	52,700	110	325	290	110		4,000	0.1	•
	Langtang	104	0.6	3.5	1.1	210		79	0.8	
	Tenti	90	4	9.8	4.2	930	0	171	1.9	
]	Cham	66	1,8	6.5	1.5	45		30	0.5	

TABLE 4.3.5 EVALUATION OF DEAD RESERVOIR CAPACITY & SPILLWAY FLOOD CAPACITY BASED ON CATCHMENT AREA (2/2)

		Catchment	Passausia.	Active	Dead F	teservoir Ca <sub>l</sub>	pacity	Spills	way Flood Capa	city
НА	Dam	Area (km²)	Area (km²)	Capacity (MCM)	Design (MCM)	Specific Yield (m³/km²)	Remark	Design (m³/sec)	Specific Yield (m³/sec/km²)	Remark
HA-4	Pankshin	156	0.5	2.8	1.2	150		165	1.1	• :
	Dp,a	60	2.2	30	7.5	2,500	0	63	1.1	
	Naka	11	1.5	2	0.5	910	0	6	0.5	
	Imogidi	40	1.0	1.8	0.7	350		: 100	2.5	
	Lafia	146	0.1	0.1	0.1	10	•	170	1.2	
HA-6	Ikere Gorge	4,620	53	565	125	541	0	6,850	1.5	
	Oyan	9,000	40	254	16	36	0	3,440	0.4	•
	Asejire	7,424	5.3	30.5	2.4	6	•	5,130	0.7	1
	Elinle	1,200	16.4	75	19	25	•	1,520	1.3	
	Egbe	2,389		20.3	2.7	23	•	453	0.2	0
	Awon	250	2.0	8.4	1.6	130		255	1.0	
	Eleyele	42	1.6	5.5	1.5	710	0	368	8.8	0
	Ekc-Ende	883	4.9	4.5	1.0	20	•	877	1.0	
HA-8	Gari	1,155	33.2	203	11	190		-	-	
7 157	Challawa	3,859	100	900	30	155		3,850	1.0	1
	Guzuguzu	106	6.4	21.5	3.1	585	0			;
*.	Magaga	119	3.7	17.2	2.5	420	0	-	<del>.</del>	
	Watari	653	19.6	92.7	11.8	361		-		
	Tiga	6,641	178	1,845	123	370	1	3,257		
	Kafin Zaki	5,300	235	2,500	200	755	0	1,460	0.3	•
	Kafin Chiri	225	8.4	24.6	6.5	578		ા હ	•	
. ;. *	Tomas	585	15	56.6	3.7	126		_	.:	
	Jakara	559	16.6	54.5	10.9	390				
Partie	Galala	462	1.1	20	3	130	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	-		:
	Alau	4,105	50	106	6.4	31	•	251	0.1	•

Remarks

(1) Specific Yield of Sediment transport = Design dead Capacity + (Catchment area × 50 years)

(2) Specific Yield of Spillway Flood = Design flood capacity + Catchment area

(3) Mark O; Large design capacity

(4) Mark : Small design capacity

TABLE 4.3.6 DEFECTIVE ITEMS IN THE EXISTING DAMS

Dam	Content
Zobe (HA-1)	(1) A little seepage through dam foundation and deteriorated concrete at outlet conduit.
Bakolori (HA-1)	(1) Large seepage through inlet chamber of minihydro power plant (suspension of plant operation).
Goronyo (HA-1)	(1) Large seepage through dam foundation (urgent need of remedial works from viewpoint of dam safety).
Kubli (HA-1)	<ul><li>(1) Scouring at the downstream of spillway.</li><li>(2) In complete gate installation at the outlet (sluice gate can't be closed).</li></ul>
Birnin Gwari (HA-2)	(1) Dam Collapse by flood over topping at dam due to small spillway flood capacity.
Kiri (HA-3)	(1) A little scepage through deteriorated concrete at outlet conduit.
Langtang (HA-3)	(1) No Reservoir storage due to leakage through reservoir area and dam foundation.
Doma (HA-4)	(1) Broken pumping equipment to lift up the reservoir water. Upgrading works of gravity system instead of lifting water by pump.
Ikere Gorge (HA-6)	(1) Scepage through concrete joint of intake tower due to poor concrete placing method.
Kafin Chiri (HA-8)	<ul> <li>(1) Scepage water through dam foundation.</li> <li>(2) Large vegetation and gully erosion on dam slope.</li> <li>(3) Large sediment deposit at approach canal of spillway.</li> </ul>
Birnin Kudu (HA-8)	<ol> <li>Seepage water through foundation.</li> <li>Large vegetation and gulley erosion on dam slope.</li> <li>Broken bridge connecting to intake tower.</li> <li>Deteriorated booster pump for water supply.</li> </ol>
Galala (HA-8)	<ul> <li>(1) Scouring at the downstream of spillway</li> <li>(2) Increasing of dam height due to insufficient reservoir capacity against much reservoir inflow.</li> </ul>
Bagauda	<ul> <li>(1) Treatment of meandering channel at the downstream of spillway to release flood smoothly.</li> <li>(2) Backfill at the borrow area located at the downstream toe of left dam embankment.</li> </ul>
Other Small Dams	<ol> <li>Many sediment deposit at the reservoir upstream and at approach channel of spillway.</li> <li>Some seepage water through foundation.</li> <li>Large vegetation and gully erosion on dam slope.</li> <li>Large aquatic weed covering reservoir surface water.</li> <li>Sweeping off of riprap material by reservoir surface water.</li> <li>Defect of gate and valve to be repaired.</li> </ol>

	TABLE 4.3.7 SUMMA	SUMMARY OF EVALUATION FOR DAM CONSTRUCTION COST	N FOR DAM CONS	TRUCTION COST		(103 US\$)
[tems	Challawa Gorge Dam	Kafin Zaki Dam	Kwali Dam	Rafin Jatau Dam	Kontagora Dam	Kagara Dam
1, Dam Construction Cost						
Dam Excavation	5,580	22,530	2,240	5,150	5,020	7,270
Grouting	1.970	29,440	4,540		780	5,470
Dam Embankment	22,860	156,030	21,740	15,970	22,080	12,300
Outlet/Spillway	13,370	18,320	14,560	4,410	10,590	630
Irrication Canal/Road			•	•	9,670	•
Others	5,570	19,550	069'9	2,330	4,480	9,030
Confinence	2.470	31,910	6,540		2,350	4,590
Total	51,820	277,780	56,310	27,860	54,970	39,290
· 其一行 分分析 等人 计特别的 有子 等						
2. Rate of Major Works	(%)					
Dam Excavation / Embankment	65	42	56	88	02	76
Grouting	۱Ġ	13	ᆏ	0	23	21
Outlet/Spillway	30	చు	33	17	28	က
3. Average Cost / Embankment						
Embank, Volume (103)	5,000	19,070	1,470	1,230	1,900	086
Cost/m3(\$/m3)	10.4	14.6	38.3	22.7	28.9	40.0

(1) Construction cost of Kawali dam / cu.m is a little high as 38.35/m³, because the dam is diversion dam which requires high cost for concrete and gate against overflow weir. Remark

(2) Construction cost for Kagara also show a little high cost of 403/m3 caused by high other cost which includes resettlement cost and restroration cost for temporary suspension.

TABLE 4.3.8 EVALUATION OF DAM CONSTRUCTION COST

		 		hailawa C	Challawa Gorge Dam			·	Kafin Zaki under Contract	under Co	ntract				Kawaji	Kawali Diversion Dam under Contract	a Dam um	der Contr	act	
	واسمين من نامد نسم ال	; ;		Const.	Convert	Cnit		Const. Cost	Cost	ဝီ	Convert to US\$	\$3			Const.	Cost	8	Convert to US\$		
	Cara theory of works	3	Q. ty	33	3		O tr	L'C N	P.C.	CC	ည်	Total	Kate Cost	S,	S.C	P.C D.M.C	) ()	ည်	Total	
				1037	703 103	25.5		103	103	103	103	103			103	103	103	103	103	3
ij	General & Preliminary Item	r L.S	•	24,000	1,600		L.S.	72,700	10,200	3,640	6,370	10,010		L.S	39,520	6,550	1,980	4,090	6,070	. •
23	Site Clearance	ha	3.4	170	10	3.0	152	2,080	1,000	100	620	720	4.7	22.6	310	150	30	06	110	4,870
က	Dam Excavation, Earth	103703	2,740	65,490	4,370	1.6	940	10,330	4,830	520	3,020	3,540	3.8	93	1,260	580	09	360	420	4.6
<b>-</b>	Dam Excavation, Rock	103m3	69	9,160	610	8.8	1.870	47,900	21,320	2,400	13,330	15,730	8.4	118	3,400	1,510	170	940	1,110	9,4
છ	Foundation Treatment	103m2	T.S	8,790	590		1,750	12,520	3,060	630	1,910	2,540	1.5	231	2,850	740	140	460	600	2,6
છ	Exploratory Drilling	103m		•	•		14.6	18,630	5,940	930	3,710	4,640	318	2.3	4,090	1,310	210	820	1,030	448
t-	Drilling for Grout Hole	103m	36.7	17,100	1,140	31.0	72.8	31,540	10,050	1,580	6,280	7,860	108	14.8.	3,500	1,130	180	710	890	09
భ	Grounting works	103no	1.2	12,390	830	069	36.0	23,400	8,350	1,170	5,220	6,390	175	0.6	089'6	3,430	480	2,140	2,620	291
ó	Slurry Trench	103m2		•	•		0.09	38,320	13,810	1,920	8,630	10,550	176	41.5	30,520	10,600	1,530	6,630	8,160	197
10.	Dam Embankment Earth	103m3	4,450	132,220	8,810	2.0	14.470	238,000	114,900	11,900	71,830	83,730	5.6	983	16,760	8,060	840	5,040	5,880	6
H	Dam Embankment Filter	103m3	***	* 110 100 100 100 100 100 100 100 100 100			3,290	73,220	28,770	3,660	17,940	21,600	6.6	262	6,600	2,530	330	1,580	1,910	6.5
12.	Dam Slope Riprap	103m3	545	181,220	12,080	22.2	1,310	159,730	68,330	7,990	42,710	50,700	39	196	18,220	7,810	910	4,880	5,790	30
13	Intake/Outlet, Concrete work	103213	63.1	157,420	10,490	991	17.5	22,670	8,990	1,130	5,620	6,750	385	23.5	26,030	006'6	1,300	061'9	7,490	318
14	Intake / Outlet, Mechanical Parts	L.S		43,200	2,880	•	L.S	38,410	23,080	1,920	14,430	16,350	•	r.s	16,710	9,970	840	6,230	7,070	
15.	15. Investigation Study	L.S	•	19,500	1,300	-	L.S	5,890	2,680	290	1,680	1,970		r.s	1,820	850	96	530	620	•
16.	Miscellaneous	S.I		69,580	4,540		S'T	008'6	3,680	490	2;300	2,790	•	•	* • · · · · · · · · · · · · · · · · · ·	•		* * * * * * * * * * * * * * * * * * *	•	•
Σ	Contingency	L.S.		37,010	2,470	: := 1	r.s	139,530	39,890	086'9	24,930	31,910		r.s.	30,630	8,010	1,530	5,010	6,540	•
	Total			777,250	51,820			944,740	368,880	47,250	230,530	277,780			211,900	73,130	10,610	45,700	56,310	
, ا	# 18 /s/	  -  :					:	44 ::												

Remarks: (1) Challawa Gorge dam construction cost is summarized based on the Final Bill of Quantity in Contract variation.

Conversion rate of Nira to US\$is 15Nira/8 taking into account average rate during construction period.

(2) Kafin Zaki and Kawali dam construction costs are summerized based on the Bill of Quantity just contracted in 1992

Conversion rate; US\$=20Nira and I.6DM.

(3) L.C. Local currency (N) F.C. Foreign Currency. (DM or USS).

TABLE 4.3.8 EVALUATION OF DAM CONSTRUCTION COST

			<u>.</u>	\$ 6 f 10 C															
	-		Rafin Jatau Dam	u Dam	-i			Kont	Kontagora Dam	ď					Kag	Kagara Dam			
				Convert	Chit		Const. Cost	Cost	& 	Convert to US\$	67			Const. Cost	ost	S	Convert to US\$	63	, ,
Description of works	c c	Š.	87.1 80.	了 3 3	2	Š.	1.0	ည္ည	2,1	5	Total	See E	ج ج	ນ <sub>່</sub> ຊ	S S S	ر ان	FC	Total	188
		T.	103N	103	\$SD.		 103 103	10 20 20 20 20 20 20 20 20 20 20 20 20 20	103	103	103	3		103	103	103	103	103	
1. General & Preliminary Item	r.s	r.s	45,450	2270		L.S.	3,960	1,140	200	1,140	1,340		L.S	24,410	6,620	1,220	4,140	5,360	•
2. Site Clearance	103ha	1.2	57,450	2870	2.4	0.2	1,680	450	80	450	530	2,650	0.65	2,780	1,090	140	089	820	13
	103m3	200	44,270	2210	4.4	359	006'9	1,990	350	1,990	2,340	6,5	1,500	8,820	3,560	440	2,230	2,670	1,8
Dam Excavation.	Rock 103m3	\$î	1,420	7.0	4.7	\$3	6,390	1,830	320	1,830	2,150	33	40.5	1,920	070	100	520	620	15.3
Foundation Treatmen	103m2					*				•			26.5	18,560	3,570	930	2,230	3,160	119
6 Delling for Grout Hole	10 <sup>3</sup> m					2.9	800	230	40	230	270	93	15.6	4,120	1,700	210	1,060	1,270	ಜ
7 Grounting works	10320			•		0.43	1,520	430	08	430	510	1,060	1.7	12,780	5,690	640	3,560	4,200	2,470
ي. ا	Earth 103m3	1,090	210,490	10,520	9.7	1,650	43,950	12,610	2,200	12,610	14,810	6	755	26,060	11,050	1,300	6,910	8,210	描
9. Dem Embankment,	103m3	3	22,310	1,120	28	451	7,300	2,100	370	2,100	2,470	16	9.89	4,340	1,860	220	1,160	1,380	20
Dam, Slope Riprap	103m3	82	86,540	4,330	43	95	14,240	4,090	710	060'7	4,800		155.7	10,820	3,470	540	2,170	2,710	17
11. Intake / Outlet, Concrete	103m3	6.5	80,210	4,010	617	20	30,850	8,850	1,540	8,850	10,390		0.74	2,360	810	120	510	630	850
12. Intake/Outlet, Mechanical Parts	I.S	r.s	7,910	400	• •	L.S	600	170		170	200	<u> </u>	•		•		•	•	•
13. River Diversion Works	r.s		•	•		L.S.	6,710	1,920	340	1,920	2,260		r.s	800	340	0,4	210	250	•
14. Canal/Road Construction	103m3	•			•	1,170	28,700	8,230	1,440	8,230	9,670		•	•				•	
15. Investigation Work	I.S		<del>-</del>		·		. •	•	•				L.S	1,900	096	82	009	30	
16. Miscellaneous	L.S	L.S	1,100	09	09	L.S	2,570	770	130	770	006		r.s	9,660	3,590	480	2,240	2,720	
17, Contingency	r.s	•	• :			I.S	6,970	2,000	350	2,000	2,350		L.S.	22,930	5,510	1,150	3,440	4,590	
Total		- 1 - 2 - 2	557,150	27,860			163,140	46,810	8,160	46,810	54,970			152,260	50,660	7,630	31,660	39,290	
74 Th. F		2000	İ	and hassed on Bill of Guan	a Rill of	Quantity,	tity of Tender Evaluation in 1993	Evaluation	n in 1993.										:.

Remarks: (1) Rafin Jatau dam construction cost is summerized based on Bill of Quantity of Tender Evaluation in 1993.

Conversion Rate; US\$=20N

(2) Kontagora dam and Kagara construction costs are summerized based on Bill of Quantity under construction Conversion Rate; US\$ = 20N and 1.6D.M

(3) L.C., Local currency (N) F.C., Foreign Currency (D.M or US\$)

REMARKABLE INCREASE OF CONSTRUCTION QUANTITY IN CONTRACT VARIATION **TABLE 4.3.9** 

			Kontagora Dam		ට්	Challawa Gorge Dam	tur.
Description	Unit	Original Quantity	Variation Quantity	Increase Rate	Original Quantity	Variation Quantity	Increase Rate
Dam Excavation, Earth	m3	276,700	359,000	1.3	1,579,000	2,740,000	1.7
Dam Excavation, Rock	Eg	2,100	65,000	31	6,400	000'69	10.8
Drilling for Grout Hold	В	2,400	2,900	1.2	0	36,700	•
Grouting	ou	400	480	1.2	0	1,200	•
Dam Embankment	Eat	1,657,000	1,899,000	<b>H</b>	4,973,000	5,000,000	•
Outlet/Spillway Concrete	m3	001'9	20,000	3.3	14,000	63,100	75.

(1) Increase of excavation volume in both dams is caused by unconsolidated and permeable dam foundation, which is not surveyed during detailed design stage. Remark:

(2) Drilling and grouting works at Challawa dam were estimated at N5,579,000by provisional sum in original contract but increase to N38,280,000 due to permeable foundation.

(3) Outlet & spillway was not designed properly in detailed design stage, so that its cost also increases remarkably.

(4) Construction cost of both dams is considerably increased by contract variation as follows;

V163,140,000 + US\$46,810,00	N163,140,000	N4,070,000+US\$29,900,000	Contagora
riation Contract	Var	Original Contract	Dam

TABLE 4.4.1 RESERVOIR OPERATIONS STUDY AT EXISTING DAMS

			,								
Dam	Tiga	Challawa	Kafin Zaki	Watari	Jakara	Tomas	Zobe	Goronyo	Bakalori	Konta- gora	Tungen- Kawa
1. River Basin	Chad	Chad	Chad	Chad	Chad	Chad	Rima	Rima	Sokoto	Niger	Niger
	178	100	235	20	17	15	34	200	80	39	7
	47	38		20	14	1.4	တ္ဆ	20	48	32	27
Voir C	1,845	900	2,500	92	55	57	170	933	403	195	ឌ
3. Reservoir Input (MCM p.a.)	62	36						•			
	831	422	1,058	99	59	39	245	656	757	277	8
ervoi	79	36		O	9	9	20	57	42	21	83
	910	458	1,174	75	65	65	265	713	799	298	37
4 Present Irrigation											
	22,000	40,000	125,000	1,700	2,000	1,100	8,200	000'69	23,000	11,200	800
Actually Serviced	14,000		0	1,350	820	200	100	800	4,200	0	400
8										•	
Potential Irrigation	26.000	:	50,000	1,500	2,000	1,800	5,000	17,000	23,000	11,200	1,200
5.1 Irrigation Demand (MCM)	278	132	:	16	21	19	48 8	203	222	178	17
and	146	<u>:</u>		1	1	I	8	13	1	1	1:
5.3 Release to Downstream (MCM)	276	<u>:</u>	312	က	က	9	22	128	104	11	Н
5.4 Reservoir Loss (MCM)	272	126	401	32	26	25	76	342	139	36	4
Total (5.1 to 5.	972	455	1,259	51	50	50	180	989	465	225	22
6. Water Balance (3 5.)	-62	8	-85	-24	15	15	85	27	334	73	15
7. Active Reservoir Capacity/	200	200	210	120	85	88	42	120	50	65	9
10/ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \											

TABLE 4.2. EXISTING DAM AND IRRIGATION AREA IN THE HADEJIA RIVER BASIN

Capacity (MCM) Dry Season	Inflow Capacity Coron Dry Season	Capacity (MCM) Dry Season	Inflow Capacity (MCM) Dry Season
(ALCIAL)	(MCM)	(TATOTAT)	"C (MCM)
900 130 330	900 130 330	900 130 330	0 460 900 130 330
1,845 270 700	1,845 270 700	0 910 1,845 270 700	0 910 1,845 270 700
92 30 20	92 30 20	- 75 92 30 20	- 75 92 30 20
18 16.6 5 9 360	16.6 5	16.6 5	18 16.6 5 9
41 20.9 5 14 610	20.9 5 14	20.9 5 14	0 41 20.9 5 14
9 5.8 3			
10.5			
24 17.2 5 9 600 6	17.2 5 9	17.2 5 9	17.2 5 9
3 9 9	24.7	24.7	24.7
21.5	27.5	27.5	27.5
21.5 9 9 35 12 8	21.5 35 12 8	21.5 35 12 8	21.5 35 12 8
35 35 8 8	35 12 8	21.5 9 9 35 12 8	21.5 35 12 8
900 130 330 12, 845 270 700 22, 92 1, 845 270 700 22, 16.6 5 9 1, 8 10.5 5 1, 4 17.2 5 9 9 1, 27.5 5 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9	900     130     330     12,500       1,845     270     700     22,000       92     30     20     1,900       16.6     5     9     360       20.9     5     14     610       5.8     3     2     -       10.5     5     4     -       17.2     5     9     600       14.7     3     9     -       21.5     9     530       35     12     8     660	0       460       900       130       330       12,500         0       910       1,845       270       700       22,000         -       75       92       30       20       1,900         -       41       20.9       5       14       610         -       9       5.8       3       2       -         0       13       10.5       5       4       -       -         0       24       17.2       5       9       600         0       16       14.7       3       9       -       -         0       35       35       35       8       660	0       460       900       130       330       12,500         0       910       1,845       270       700       22,000         -       75       92       30       20       1,900         -       41       20.9       5       14       610         -       9       5.8       3       2       -         0       13       10.5       5       4       -       -         0       24       17.2       5       9       600         0       16       14.7       3       9       -       -         0       35       35       35       8       660
900 130 330 1 1,845 270 700 2 92 30 20 16.6 5 9 20.9 5 14 5.8 3 2 10.5 5 9 14.7 3 9 21.5 9 9	900 130 330 1 1,845 270 700 2 92 30 20 16.6 5 9 20.9 5 14 5.8 3 2 10.5 5 4 17.2 5 9 14.7 3 9 21.5 9 9	0       460       900       130       330       1         0       910       1,845       270       700       20         -       75       92       30       20       20         -       41       20.9       5       14         -       9       5.8       3       2         0       13       10.5       5       4         0       24       17.2       5       9         0       16       14.7       3       9         0       16       14.7       3       9         0       35       35       12       8	0       460       900       130       330       1         0       910       1,845       270       700       20         -       75       92       30       20       20         -       41       20.9       5       14         -       9       5.8       3       2         0       13       10.5       5       4         0       24       17.2       5       9         0       16       14.7       3       9         0       16       14.7       3       9         0       35       35       12       8
900 130 1,845 270 92 30 16.6 5 20.9 5 5.8 3 10.5 5 14.7 3 21.5 9	900 130 1,845 270 92 30 16.6 5 20.9 5 5.8 3 10.5 5 14.7 3 21.5 9	0       460       900       130         0       910       1,845       270         -       75       92       30         -       18       16.6       5         0       41       20.9       5         0       13       10.5       5         0       16       14.7       5         0       16       14.7       3         0       16       14.7       3         0       16       14.7       3         0       16       14.7       3         0       16       19       35	0       460       900       130         0       910       1,845       270         -       75       92       30         -       18       16.6       5         0       41       20.9       5         0       13       10.5       5         0       16       14.7       5         0       16       14.7       3         0       16       14.7       3         0       16       14.7       3         0       16       14.7       3         0       16       19       35
900 1,845 92 16.6 20.9 5.8 10.5 17.2 14.7	900 1,845 92 16.6 20.9 5.8 10.5 17.2 14.7	0 460 900 0 1,845 - 75 92 - 18 16.6 0 41 20.9 - 9 5.8 0 13 10.5 0 24 17.2 0 16 14.7	0 460 900 0 1,845 - 75 92 - 18 16.6 0 41 20.9 - 9 5.8 0 13 10.5 0 24 17.2 0 16 14.7
	460 910 75 18 18 9 9 16 16	00 0 - 0 - 6 910 0 - 0 - 6 18 17 9 11 9 12 13	00 0 - 0 - 6 910 0 - 0 - 6 18 17 9 11 9 12 13
460 910 75 18 41 9 13 24 16	00 0 - 0 0 - 6 010 - 0 - 0 0 - 6 113 - 114 - 125	00110100016	00110100016
	00110100016		

TABLE 4.4.6 OUTLINE OF PRIORITY RIVER BASIN FOR THE PROPOSED MEDIUM AND SMALL DAM PACKAGE PROGRAM (1/2)

	HA.			HA-2	2			HA-3	63			HAA	4	
	whole Basin	Danzaki SFA 110	whole Basin	Awun SHA 204	Galma/ Karami SHA 211	Cboko SHA 214	whole Basin	Kilange SHA 301	Hawal SEA 303	Belwa SHA 307	whole Basin	Ankwe SELA 403	Katsina-Ala SHA 4052 ~53	Mada SHA 407
Le Basin Feature														
(1) Catchment Area (km²)	131,600	813	158,100	7,150	11,630	7,670	158,900	9,450	16,140	7,560	73,000	11,550	8,550	8,730
(2) Agricultural Land (303 ha)	6.952	٠.	6,702	413	989	653	4.628	368	552	161	3,929	447	869	272
	14.147	746	21,867	1.466	3,761	1,257	15,152	1,183	1,349	1,974	17,602	2,713	1,678	1,260
6	108	16	138	205	323	164	36	125	84	261	- 691	235	196	144
(5) Agricultural Area/Population (ha)	67.0	0.57	0.31	0.28	81.0	0.52	0.31	0.31	17:0	1.0	0.34	0.16	0.42	0.22
(6) Potential Surface Water (MCM)	22,400	1,100	32,600	1,070	2,930	1,680	49,200	1,430	2,150	2,120	27,400	1,390	12,800	1,010
Popu	1,580	1,480	1,490	730	780	1,340	3,250	1,210	1,590	1,070	2,360	510	7,630	800
2. Existing dam Project														
(I) Number (No)	14	4	28	63	. 2	2	21	0	0	. 0	8	0 .	0	. 2
(2) Active Capacity (MCM)	1,755	ż	935	34	1.75	8	2,280	0	0	0	56	0	0	30
(3) Available Reservoir Water (MCM)	186	22	786	27	07	34	1,565	0.	0	0	- 55	0	0	. 20
3. Proposed Dam Project		1 1 2 2 2			1 .			All the second			The Same and			
(1) Number (No)	64, 50)	77 73	304 (230)	29 (17)	23 (17)	24 (0)	140(110)	27 (18)	(7) 72	24 (18)	213(160)	32 (24)	37 (24)	41 (34)
(2) Active Capacity (MCM)	950(250)	204 (35)	4,090(1,150)	527 (85)	540 (85)	(0) 824	2,640(550)		315 (35)	447 (90)	2,060(800)	288(120)	351(120)	310(170)
(3) Available Water (MCM)	630(156)	134 (21)	3,060 (920)	380 (64)	378 (64)	(0) 069	1,900(440)	234 (67)	242 (27)	378 (67)	1,530(640)	213 (90)	240 (90)	231(130)
4. Total Available Water (MCM)	1,611	139	3,846	407	418	624	3,456	234	242	378	1,585	213	240	251
5. Irrigation Project.														
(1) Existing Area (103 ha)	09	pt	55	0	1	10	25	0	. 1	0	15		4	2
(2) Proposed area (103 ha)	52	7	234	30	. 29	43	125	16	16	. 25	125	17	19	22
(3) Total (103 ha)	115	12	289	30	30	53	150	16	17	25	140	30	23	20.
(4) Water Demand (MCM)	1,104	115	3,235	340	340	604	2,010	214	228	335	1,624	232	267	232
6. Water Supply Project														
(1) Water Supply Capacity in 1991 (MCM)	103	n	203	25.4	22.6	15.2	44	1.5	1.4	5.2	28	2.7	1,4	2.1
(Z) Water Demand in 2020 (MCM)	991	10	642	118	110	52	138	9	7	23	1.83	1.1	1.7	17
(3) Increase Rate of W.S (%)	1.6	3.3	3.2	4.6	4.9	3.4	3.1	3.7	5.3	4.4	6.5	4.1	12.1	8.1
7. Total Water Demande (MCM)														
(5(4) + 4(2))	1,270	125	3,937	458	450	656	2,148	220	235	358	1,807	243	284	249
8. Surface Water Withdrawal Rate (%)					-	:					:		-	

Remarks, ( ) in the above figure shows the value of small dams

TABLE 4.4.6 OUTLINE OF PRIORITY RIVER BASIN FOR THE PROPOSED MEDIUM AND SMALL DAM PACKAGE PROGRAM (2/2)

	HA.5	24.5		HA-6	و			HA.7	6-1	
		Memu	whole	Upper Ogun	Oshun	Osso	whole	Aloma	Aya	Abolne
	Basin	SHA 5041		-23	SHA 605	SHA 608	Basin	SEA TOL	201.1100	en vice
1. Basin Feature			2					The Assessment of the	The second secon	
(1) Catchment Area (km2)	53,900	4,270	300,500	20,140	10,130	13,730	59,800	7,490	8,660	9,490
(2) Agminultural Land (103 ha)	1.427	153	4,105	1,290	410	463	3,865	51.7	71.1	532
(2) Parallation in 2020 (103)	24,772	5.863	44,355	2,541	5,270	4,003	26,887	5,099	1,999	, 6,193
(4) Pomilation Density (No/km²)	460	1,370	441	126	520	292	450	280	231	653
(5) Aorientural Area ( Population (ha)	0.06	0.03	60.0	0.51	0.08	0.12	0.14	0.25	0.36	0.09
(6) Potential Surface Water (MCM)	20,000	4.270	35,400	009'9	4,000	4,800	65,700	4,180	4,820	3,240
Popu	810	730	800	2,600	160	1,200	2,440	1,990	2,410	1,490
2. Existing dam Project									1	
- i ~	6	4	31	13		. 5	12	0	7	7
DECITY	16	æ	1,033	844	126	21	104	0	7.7	18
(3) Available Reservoir Water (MCM)	81	10	1,145	908	152	17	- 87	o	2	20
3. Proposed Dam Project.				apara a sa	The state of the	and the second	The space from the			
(1) Number (No)	(32) 67	23 (17)	141(100)	36 (18)	(81) 12	22 (18)	144(115)	33 (22)	37 (26)	27 (22)
	515(175)	172 (85)	1,410(500)	(06) 009	279 (90)	196 (90)	1,205(575)	278(110)	345(130)	301(310)
	(091)065	161 (76)	1,280(450)	456 (81)	245 (81)	172 (81)	1,110(520)	267(102)	317(117)	280(102)
4. Total Available Wator (MCM)	508	171	3,207	1,361	397	189	1,197	267	371	38
5. Irrigation Project		1								
(1) Existing Area (103 ha)	20	87	45	36		0	10	0	2	٥
(2) Proposed area (103 ha)	25	00	09	19	11	G	65	16	16	16
(3) Total (103 ha)	4.5	41	105	55	12.	Ġ	75	16	18	76
(4) Water Demand (MCM)	621	138	1,533	803	175	131	930	211	238	211
6. Water Supply Project										
(1) Water Supply Capacity in 1991 (MCM)	91	9	277		26	13.9	76	0.6	1,6	9.0
(2) Water Demand in 2020 (MCM)	131	50	1,683	20	98	88	275	ó	TT.	2
	8.2	8.3	6.1	12.5	3.3	6.4	3.6	14.7	6.9	8.3
7. Total Water Demande (MCM)		1000								
{5(4) + 4(2)}	752	188	3,216	853	261	-220	1,265	220	249	216
8. Surface Water Withdrawal Rate (%)					I			The second		
(7 + 1 (6) )	7	7	6	13	. 9	9	2	H	10	77
						 	i			

Remarks, ( ) in the above figure shows the value of small dams

TABLE 4.4.7 EVALUATION OF HIGH PRIORITY RIVER BASIN

Items	Danzaki SHA 110	Awun SHA 204	Gbako SHA 2141	Kilange SHA 301	Mada SHA 407	Mamu SHA 504	Upper Ogun SHA 6021~23	Osse SHA 608		Aya SHA 702
1. Potential Surface Water	o O		1						0	五
2. Existing Project	ы О								O	(E <sub>t</sub>
3 Proposed Dam	O Ma								0	Ma
4. Population Density	ě O						-		0	ш
5. Per Capita farm Land	9	o Me	D D	O Me	s ⊲	ა <	) ()	ა ⊲	0	ᇔ
6 Proposed Irrigation Area	% O	-							0	Ϋ́
7 Increase Rate of W.S.	œ O								0	Ħ
S. Water Withdraw Rate	<b>%</b>	o W			٠.				0	Ψ
9. Manazement	Ne O	•							0	ඊ
10 Access to site	ы О		4						0	យ
11 Man Availability	0			0			0	◁	Ö	
12. Basin Area Size	× O	© Wo	© We	© Mo	© We	O Wo	Δ L	A L	0	Ϋ́

Mo; Moderate, F, Few, Ma, Many, H, High L, Large S, Small G, Good E, Easy, D, A little difficult 

Bost Condition 

A Poor Condition Remark

The above evaluation is made based on comparison of the condition between whole basin (HA) and the high priority basin.

TABLE 4.4.8 EVALUATION OF RESERVOIR OPERATION FOR KATSINA-ALA HYDROPOWER DAM

(unit MCM)

r				<del></del>	(and men)
Month	Reservoir Inflow ①	Reservoir Storage ②	Out flow from Inflow 3=1)-2	Out flow from storage ④	Total out flow \$=3+4
6	1,570	0	1,570	0	1,570
7	3,060	0	3,060	0	3,060
8	3,710	450	3,260	0	3,260
9	4,450	1,000	3,450	0	3,450
10	4,400	1,000	3,400	0	3,400
11	1,820	0	1,820	0	1,820
12	700	0	700	75	775
1	260	0	260	515	775
2	130	0	130	645	775
3	170	0	170	605	775
4	240	0	240	535	775
5 -	700	0	700	75	775
Total	21,210	2,450	3,260	2,450	21,210

Remarks;

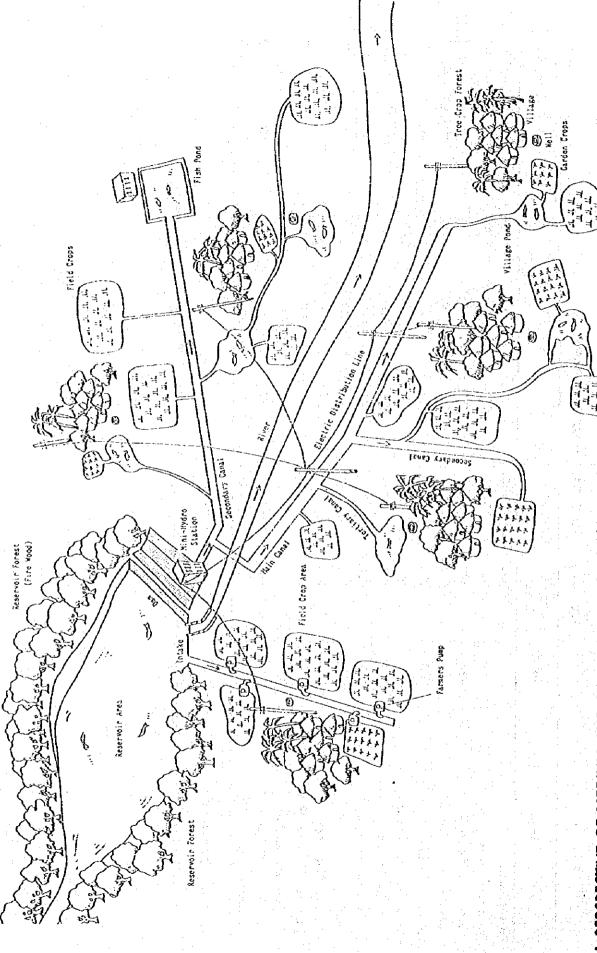
- (1) Average monthly runoff in 1980s is adopted for the reservoir inflow.
- (2) Reservoir with the capacity of 2,450MCM is filled with the rich inflow from August to October.
- (3) The reservoir water storaged in the above three months is used for the power outflow together with the reservoir inflow during dry season from December to May in order to increase the power outflow in dry season.
- (4) The power outflow in dry season decreases to 25 percent of that in wet season due to small reservoir capacity.

TABLE 4.9 POTENTIAL HYDROPOWER DAM

Items	Unit	Zungeru Dam	Katsina-Ala Dam	Suntai Dam	Karamti Dam	Mayo Yim Dam	Su Dam	Kam Dam
River Basin		Kaduna	Katsina-Ala	Donga	Taraba	Taraba	Taraba	Taraba
Catchment Area	km <sup>2</sup>	44,400	15,430	1,700	4,800	2,300	1,900	2,600
Runoffspecific Yield	日日日	230	1,500	800	800	200	700	100
Average Annual Runoff	МСМ	10,000	23,000	1,360	3,840	1,600	1,330	1,820
Reservoir Area	ķm <sup>2</sup>	974		30	170	120	09	100
Total Reservoir Capacity	MCM	29,500	2,040	1,700	4,500	2,000	1,800	2,500
Active Capacity	MCM	24,500	1,900	1,400		1,500	1,300	1,800
Dead Capacity	MCM	5,000	140	300	1,000	500	500	700
Dam Height	Я	113	43	09	02	09	50	50
Average Discharge	m3/sec	300	730	43	122	51	42	28
Effective Discharge (80%)	m3/sec	240	580	34	86	41	3%	46
Outflow for Peak Power	m3/sec	•		102	294	123	102	138
Power Plant Capacity	MW	950	440	55	190	65	45	09
Annual Production	GWH	000*9	1,900	160	550	190	130	175
						i		

Remarks; The above dimension of Zungeru and Katsina-Ala is based on NEPA data.

The dimensions of the other hydropower dam is studied preliminary on the map of 1 to 50,000 scale.



A PERSPECTIVE OF MEDIUM AND SMALL DAM PROJECT (M.S.D.P)

# APPENDIX 4-1. GUIDELINE FOR THE INTEGRATED WATER MANAGEMENT STUDY IN THE NORTH REGION

The integrated water management study is to be carried out with the following guideline:

## 1. Preparatory Work

)

- (1) Data to be Collected and Surveyed
  - (a) Map of 1 to 50,000 scale
  - (b) Aerial photograph in the river basin, if available
  - (c) Hydrological data in the basin on daily basis such as rainfall, evaporation, river runoff.
  - (d) Outline of the existing dams such as location, reservoir area, reservoir capacity, H-A and H-Q curve, dam dimension, spillway flood capacity.
  - (e) Outline of the existing irrigation projects such as location, planned area, actual irrigated area, cropping pattern, irrigation schedule, irrigation water demand.
  - (f) Outline of the existing water supply projects such as location, projected service population and water demand, supply capacity and dimension of the existing waterworks, actual water supply volume under the existing waterworks, etc.
  - (g) Outline of the wet land condition along the downstream river of the damsite such as the river slope and width, branched off tributaries, agricultural area, present water use, water right, etc.
    - If the above data are not available, the inventory survey including the site survey shall be carried out to collect data.
- (2) Review and Analysis based on the Collected data
  - (a) Runoff analysis by formulation of the runoff model at each damsite and the control points along the river.
  - (b) Review of the water demand for irrigation and water supply.

- (c) Analysis of the evaporation and seepage loss from the reservoir.
- (d) Analysis of the water loss along the downstream river.
- (e) Alternative water allocation to the downstream area.
- (f) Review of the outlet discharge capacity based on the reservoir water head and the opening degree of gate or valve at the outlet.

#### (3) Site Survey

- (a) Selection of location for the gaging stations to be installed at the upstream river of the reservoir for the inflow observation, in the reservoir to monitor the reservoir water level, and along the downstream river of the damsite to monitor the released water from the reservoir. In addition, selection of location along the river to install the monitoring well to monitor the water level for the subsurface ground water in Fadama Area.
- (b) Identification of the deteriorated and defective facility and structure to be rehabilitated in the existing dams.
- (c) Survey for the sediment accumulation condition at the upstream and surrounding area of the reservoir.
- (d) Survey for the downstream river condition at the river reaches which take place the large seepage loss and have many branched off channels.
- (e) Survey for the present water use condition for the irrigation and water supply in the service area.
- 2. Installation of Gaging Stations and Monitoring Wells and Hydrological Observation

#### (1) Installation

- (a) Provision of the hydrological equipment.
- (b) Design and construction of gaging stations and monitoring wells.
- (2) Discharge Measurement and Rating Curve
  - (a) Discharge measurement at gaging station.

(b) Review of the existing rating curve and preparation of new rating curve based on discharge measurement at the stations.

#### (3) Water Level Monitoring at Wells

Monitoring of the sub-surface water level at the wells in wet and dry season.

#### 3. Reservoir Operation Study

#### (1) Operation Study

The alternative reservoir operation study based on the different water demands.

# (2) Optimum Reservoir Operation Rule

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Formulation of the optimum reservoir operation rule based on the above operation study.

#### 4. Water Management Practice

#### (1) Reservoir Operation Practice

- (a) Reservoir inflow observation.
- (b) Outflow practice based on the proposed optimum reservoir operation rule taking into account the reservoir inflow.
- (c) Monitoring of the reservoir water level fluctuation based on the above outflow practice.
- (d) Data complication based on the above practice and monitoring result.

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## (2) Monitoring Practice for Released Water from Reservoir

- (a) Simultaneous monitoring of the water level fluctuation at the representative gaging station along the river based on the different released water from the reservoir such as 10 m³/sec, 30 m³/sec, 50 m³/sec, etc.
- (b) Simultaneous monitoring of the fluctuation of the sub-surface water level at the wells based on the above discharge.
- (c) Monitoring period of six months under the continuous outflow from the reservoir.

#### 5. Establishment of Water Management Plan

#### (1) Analysis of the Water Management Practice

- (a) The reservoir water level fluctuation comparing the reservoir operation rule previously set up.
- (b) Analysis of the water loss along the river based on the monitoring result at the gaging stations and wells.
- (c) Review of the reservoir operation rule taking into account the analyzed water loss along the river.

#### (2) Preparation of the Optimum Water Management Plan

- (a) Formulation of the optimum water allocation plan, taking into account the water loss.
- (b) Formulation of the standard outflow guideline in wet and dry season on 10 days basis.
- (c) Preparation of the reservoir operation rule curve.
- (d) Preparation of the guideline for the outflow in the abnormal dry season with less reservoir inflow.
- (e) Formulation of the rehabilitation and upgrading plan for the existing dams.
- (f) Formulation of the river improvement plan to minimize the water loss.
- (g) Preparation of the operation and maintenance manual for each dam.
- (h) Preparation of data compiling form for the water management.

# APPENDIX 4-2. GUIDELINE FOR PLANNING OF MEDIUM AND SMALL DAM PACKAGE PROGRAM

# 1. Master Plan or Pre-Feasibility Study

The master plan or the pre-feasibility study shall be performed at the model river basin with the priority at each RBDA.

## (1) Data to be Collected

- (a) Aerial Photograph and the map of 1 to 50,000 scale.
- (b) Hydrological data in the basin or the adjacent basin.
- (c) Socio-economic data at LGA in the basin.
- (d) General geological map in the basin.
- (e) Existing water resources projects including location and outline.
- (f) Other necessary data.

# (2) Preparatory Work for the Study in the Office

- (a) Division of the basin into the several tributary basins with the catchment area of 1,000 to 2,000 km<sup>2</sup> on the map of 1 to 50,000 scale and measurement of their catchment area.
- (b) Study for the basin salient feature such as the basin topography and geology, tributary system, land use, socio-economy, etc.
- (c) Study for hydrological condition such as aerial rainfall and evaporation, annual average runoff, sediment transport, peak flood discharge, etc. Preparation of the guideline for the specific yield of annual average runoff (mm), peak flood discharge (m³/see/km²) and sediment transport (m³/km²) in order to estimate approximate reservoir inflow and reservoir capacity.
- (d) Study for water demand in service area and reservoir water use under the existing dams.
- (e) Finding of the proposed dams and service area on the map of 1 to 50,000 map.
- (f) Measurement of catchment area, reservoir area, dam height and dam length at the finding damsite on the map of 1 to 50,000 scale.

- (g) Approximate estimation for the reservoir inflow based on the catchment area and the specific runoff yield.
- (h) Approximate estimation for the reservoir active capacity based on the reservoir area and effective reservoir depth. Preparation of the guideline for the effective reservoir depth taking into account the reservoir dead water depth, spillway overflow depth, and dam free board.
- (i) Classification of the finding dams by the reservoir and dam size.
- (j) Drawings the location, catchment area, reservoir area, damsite, etc. for the existing and proposed dams on the map of 1 to 50,000.

6

### (3) Site Survey Work

The site survey work is to be carried out with the expert of dam, hydrologist, geologist, soil mechanist, agriculture, irrigation, socio-economist and topo-surveyor under the team leader with capability of the water resources development plan.

- (a) Topographical and geological condition at damsite and reservoir area, especially rock outcrop, gully erosion, vegetation, etc.
- (b) Overburden depth at damsite by providing test pit or hand auger.
- (c) Water level and runoff fluctuation at rivers by observation and interview to local people.
- (d) Longitudinal survey along the proposed dam axis.
- (e) Longitudinal and cross section survey in the reservoir area to estimate the reservoir capacity. (Longitudinal length of 5km along the river and cross section interval of 250 to 300m)
- (f) Selection of gaging station site for the surface water observation.
- (g) Agricultural and socio-economic condition survey at the service area by questionnaire.
- (h) Beneficial intention survey for the proposed dams.
- (i) Environment survey in the reservoir and service area.

# (4) Analysis and Study in the Office Work based on Site Survey

(a) Hydrological analysis for monthly rainfall, evaporation and runoff and design flood capacity for spillway.

- (b) Study of the reservoir dimension and preparation of H-A and H-Q curve.
- (c) Water demand study in the service area.
- (d) Simple reservoir operation study and available reservoir water for the service area.
- (e) Water balance study in tributary basin based on the potential surface water and available reservoir water use.
- (f) Dam standard section and dam embankment volume.
- (g) Potential irrigation area and domestic water supply quantity by the proposed dam.
- (h) Environment impact at the project area.
- (i) Approximate dam construction cost based on the dam embankment volume.

#### (5) Selection of the Representative Dams for the Feasibility Study

In accordance with the above study, four to five representative dams will be selected for the feasibility study with the following conditions;

- Dams consisting of the medium and small dams with the representative size.
- Dams with priority from viewpoint of the project economy.
- Dams without environment problem.
- Dams with easy condition for the survey and study such as accessibility to damsite.

# 2. Feasibility Study

The feasibility study shall be performed for the above selected representative dams based on the international standard. The particular attention shall be paid for the following items.

# (1) Survey and Investigation Works

Other to view to the Mark the William Politics of the

(a) Preparation of aerial photo map of 1 to 10,000 scale covering the reservoir area, damsite and service area. (Average area of 50km²/ each representative dam)

- (b) Installation of bench marks at damsite and topo-survey at the damsite (Plain map scale of 1 to 2,000)
- (c) Installation of rainfall and water gage and continuous observation at stations.
- (d) Geological survey at the damsite by seismic prospecting, core drilling and test pits.
- (e) Soil sampling at the borrow area and its mechanical test in laboratory for construction materials.

## (2) Hydrological Analysis

- (a) Runoff analysis on daily basis by preparation of the runoff model.
- (b) Design flood capacity analysis for spillway based on the unit hydrograph.

#### (3) Reservoir Plan Study

- (a) Preparation of accurate reservoir H-A and H-Q curve.
- (b) Reservoir operation study on 10 days basis.
- (c) Formulation of the reservoir operation rule.
- (d) Estimation of available reservoir water in wet and dry season and also in wet and dry year.

# (4) Preliminary Design of Dam and Appurtenant Structures

#### (a) Dam Body

- Dam foundation treatment method based on the geological investigation result.
- Available embankment material at reservoir area and borrow area from viewpoint of quantity and quality.
- Analysis for flow net and equipotential line and dam stability based on the standard dam section.

## (b) Outlet and Spillway

- Hydraulic and structural analysis.
- Application of simple structure for intake such as drop inlet or inclined inlet.
- Application of outlet conduit by reinforced concrete without steel pipes, taking into account the low water head in the medium and small dam.
- Application of Ogee type weir without gate.

# (5) Construction Plan and Cost Estimate

- Construction method and schedule on the contractor basis or force account basis of RBDA.
- Cost estimation preparing the bill of quantity.

### (6) OM Plan

OM organization, equipment, facility and OM cost.

# (7) Environment Impact

- (a) Resettlement problem in the reservoir area.
- (b) Water pollution problem during construction.
- (c) Variation of the river flow at the downstream after completion of dam.
- (d) Other ecological problem by dam construction.

# (8) Project Evaluation

- (a) Water cost to be developed by dam.
- (b) Economical and financial analysis for the project cost.
- (c) Evaluation of farmer income generation and job creation.

## 3. Guideline of the Other Project Facility

The following project facility will be studied in the service area under the medium and small dam.

#### (1) Irrigation Canal System

- Main canal with the design discharge capacity of 0.5 to 3 m<sup>3</sup> / sec by concrete lining or stone pitching
- Secondary and tertiary canal with earth canal.

#### (2) Village Ponds

- Five to ten village ponds for fish culture and garden irrigation at each village in the service area.
- Ponds to be constructed with the surface area of 1,000m<sup>2</sup>, the depth of 2 to 3<sup>m</sup> and the capacity of 2,000 to 3,000m<sup>3</sup>.

## (3) L.G.A. Supporting Facility

- Hatchery center to feed fry for fish culture.
- Sapling center for reforestation.
- Agricultural extension service center.

#### (4) R.B.D.A. Construction Center

- Construction equipment workshop.
- Construction and OM office.

# APPENDIX 4-3. GUIDELINE OF OPERATION AND MAINTENANCE FOR

#### 1. Technical Materials to be Provided at OM office

(1) Report of feasibility study, detailed design, etc.

### (2) Map and Drawings:

- 1/50,000 map showing catchment area of dam.
- General plan map at damsite.
- Location map of gaging station for rainfall, evaporation. surface water, etc.
- Dam longitudinal section drawings along dam axis showing geological condition.
- Dam standard section and cross section drawings.
- Drawing showing location and structure dimension of dam instruments and relief wells.
- Structural drawings of intake, outlet and spillway.
- Structural drawings of gates, values, mini-hydropower plants.
- Drawings showing reservoir H-A and H-Q curve.
- Drawings showing outflow discharge based on reservoir water head and opening degree of gates and values.
- Drawing showing reservoir operation rule curve.

## (3) Basic Data:

- Hydrological data at damsite including rainfall, evaporation, reservoir inflow, peak flood discharge, water quality, etc.
- Reservoir dimension such as catchment area, reservoir surface area, reservoir capacity, water level, etc.
- Dam dimension such as dam height and length, spillway design flood capacity and overflow depth, outlet discharge capacity.
- Gate and valve dimension such as units, width, length, motor capacity, etc.

- Service area outline such as irrigation area and water requirement, population and water demand for domestic supply, water right of downstream area, etc.
- (4) Operation manual of gates and valves prepared by manufacturers.
- (5) List of O/M equipment and facility including operating condition.

#### 2. Reservoir Water Operation

### 2. 1 Hydrological Observation Management

#### (1) Installation of Gaging Station:

- Several rainfall gaging stations will be installed at the damsite with large catchment area taking into account proper distribution.
- Automatic water level gaging station is to be installed at the reservoir and the upstream river of reservoir in large dam with height of more than 30m or reservoir capacity of more than 50 MCM.
- Staff gage is to be installed at the river and reservoir of other dams.
- Particular attention shall be paid in selecting gaging station site, as cross section and river bed at the upstream river of reservoir changes by backwater of reservoir and sediment deposit.
- Original gaging station installed prior to dam construction shall be checked from the above viewpoint. The station will be replaced, if river condition is not suitable for observation.
- When two to three large rivers flow into the reservoir, gaging station at each river is to be required.

## (2) Observation and Data Compilation:

- Continuous observation shall be carried out at stations.
- Old rating curve for discharge estimation shall be checked and amended by discharge measurement.
- Accurate water level observation and discharge measurement in wet season from July to September shall be carried out, since potential

- reservoir inflow is defined by wet season runoff and reservoir operation rule is to be set up based on wet season runoff.
- Periodical measurement of water quality in the reservoir and sediment transport at the upstream river of reservoir shall be carried out.

## 2.3 Reservoir Operation Study and Rule Curve

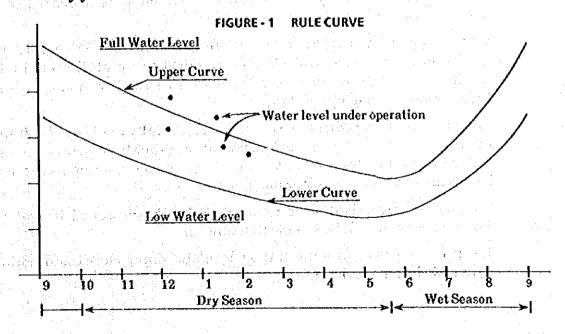
#### (1) Reservoir Operation Study

Reservoir operation study shall be carried out based on reservoir inflow, rainfall on reservoir area in wet season, evaporation from reservoir area, downstream water release, water demand in service area, etc. Computer program for reservoir operation study is prepared by the JICA Team and submitted to Data Bank of FMWRRD.

#### (2) Reservoir Operation Rule Curve

Reservoir operation rule curve as shown in Figure-1 is to be prepared based on the operation study result.

The rule curve is consisting of the upper curve for ordinary year and the lower curve for dry year.



# (3) Reservoir Water Level Monitoring and Evaluation:

- Fluctuation of reservoir water level during operation shall be measured at least on weekly basis and plotted on the reservoir rule curve.
- Remained storage capacity at the end of week, month and year shall be estimated based on the above water level and reservoir H-Q curve.
- When the plotted curve as mentioned above will be at the lower position than the reservoir rule curve, it is judged that the reservoir inflow is less and taking the dry year pattern or larger outflow than designed one.
- When reservoir water level is placed at the full water level or designed operation water level, the proposed service area could be sufficiently irrigated.
- When reservoir water level is not recovered at the end of wet season, the proposed service area at next season will be reduced based on the remained storage capacity in the reservoir and as a consequence, the reservoir operation is to be made by the lower rule curve.

#### 2.4 Outflow Control

- Outflow from reservoir is to be controlled by gate and valve operation in accordance with the water demand, water head of reservoir and opening degree of gate and valve. Outflow shall be measured on daily basis and summarized on monthly and its value is evaluated as compared with designed water demand.
- When supplement irrigation for wet season crops is carried out, the irrigation requirement will change by effective rainfall amount in service area. For example, service area will require none, or less irrigation water during rainfall.

Therefore, outflow control in wet season is carefully made by obtaining information for changing water demand by rainfall, otherwise excess water is released from reservoir and it is not only water losses but brings about water logging problem in service area.

The dry season irrigation is to be made by the designed irrigation demand, as there will be no effective rainfall.

It is necessary to control the outflow by maintaining close coordination with OM staff in service area.

#### 2.5 Flood Discharge Management at Spillway

- In Ogee type spillway (non controlled spillway), overflow depth during flood spilling period shall be measured and its flood discharge shall be estimated with hydraulic formula.  $(C \cdot L \cdot H^{3/2})$ 

In gate controlled spillway, flood discharge shall be estimated by gate opening degree and reservoir water head.

In gate spillway, particular attention shall be paid to gate control to prevent abnormal flood release to the downstream which may cause flood damage.

When large flood appears at spillway, flood alarm shall be given to the downstream area.

## 3. Monitoring work in Dam Maintenance

#### (1) Settlement of Dam Embankment

Settlement of dam embankment is measured by installing bench marks on dam crest and slope. In addition, appearance of crack on dam crest and slope, and surface slip on dam slope shall be checked.

# (2) Seepage Water

- Seepage water level and quantity shall be measured by dam instruments and relief wells. Fluctuation of seepage water line depending on reservoir water level is to be drawn up on dam cross section drawings.

Seepage water quantity is measured by weir installed at the end of drainage ditches to collect and guide seepage water.

When the reservoir water level draws down, the upstream slope condition such as phenomenon of surface slip, abnormal depression, crack, settlement, etc. shall be checked.

#### (3) Others

- After spilling flood through spillway, sediment and log accumulation at approach channel infront of spillway, and scouring at stilling basin and downstream river shall be checked.
- Scouring condition at the downstream of outlet shall be checked periodically.
- Sediment deposit condition at the reservoir upstream area and river reaches influenced by reservoir backwater shall be measured.
  - For this purpose, bench marks shall be installed at both banks of reservoir area and river. Measurement of sediment deposit is made by measuring the elevation of deposit with top-survey, when the reservoir water level reaches the low water level in the end of dry season.
- Seepage water through construction joint, souring and fissure of concrete surface, settlement of structure, etc. at concrete structure shall be checked at the outlet, and spillway.
- Mechanical parts such as gates and valves shall be periodically inspected and repaired. Periodical painting and oiling works for equipment shall be carried out.

## (4) Report and Data Analysis

The above monitoring result is summarized on report and drawings and submitted to agencies in charge of dams such as headquarter of RBDA, SID and SWA.

The following items to be analyzed, repaired and rehabilitated urgently shall be immediately reported to agencies concerned.

- High seepage water or rapid increased seepage line at dam instrument and seepage drainage ditch. Particular attention shall be paid for appearance of turbid seepage water.
- Phenomeon of boiling, spring and water level increasing at the dam down stream area and swamp area.
- Large erosion and gulley phenomenon on dam slope and abutment as well as reservoir area.
- Large scouring at the downstream of spillway and outlet.

- Large sediment deposit at intake mouth and approach channel of spillway.
- Defect of concrete structure to be urgently repaired.
- Large aquatic weed accumulation at intake mouth and spillway approach channel.
- Damage of mechanical parts.
- Others from viewpoint of dam safety.

#### 4. Rehabilitation Works in Dam Maintenance

# (1) Counter-measures for seepage water through Dam Foundation:

- Swamps and stagnant water area being found at the downstream area of dams shall be filled with soil materials, otherwise mosquito and pest will be generated in those area. Seepage water through dam foundation shall be guided by drainage ditches to the river.
- Large depression and excavated borrow area at the downstream of dam shall be backfilled with soil material, otherwise water head between reservoir water level and elevation of those area becomes larger and accelerate the piping phenomenon.
- When dams show relatively high seepage water which could endanger dam structure safety, counter weight berm installing filter drain will be required at the downstream dam toe.
- When dams present high seepage water with some piping pressure, grouting or slurry trench method will be applied for rehabilitation. Those works shall be made at the lower water level of reservoir in order to minimize the influence of reservoir water head to the hole and trench.

# (2) Protection Work on Dam Slope:

alor

Developed sheet and gulley erosion on dam slope shall be shaved and refilled with compacted earth material. In the easy eroded area by rainfall, catch drain of rain runoff shall be provided on dam crest and berm on dam slope. Catch drain is to be protected with riprap or dry stone pitching.

- Dense vegetation covering on dam crest and slope shall be removed.
- Where riprap material on the upstream slope of dam is sweeped away by wave of reservoir water, rehabilitation of the protection work with large size riprap material shall be carried out.

#### (3) Protection of Scouring Place:

Scouring place at the downstream of spillway and outlet is rehabilitated or upgraded with wire sausage, gabion, wet mansory or concrete structure taking into account souring energy of spilling flood and outlet discharge.

Stilling basin to dissipate scouring energy will be constructed, when scouring energy is large.

#### (4) Removal of Sediment Deposit:

If sediment deposit is found at the approach channel of spillway and downstream of outlet, the deposit shall be quickly removed, otherwise the flood and discharge could not be released smoothly to the downstream.

#### (5) Remedial works of concrete Structure:

Deteriorated, defected and broken concrete structure due to scouring energy, foundation settlement, etc. shall be reconstructed.

#### (6) Repair or replace of Mechanical Items:

Mechanical items, such as gate and valve with defect or broken part shall be repaired or replaced.

Emergency stand-by diesel generator shall be provided for smooth operation, as electric current often goes out during operation of gate, valve and pumps.

#### (7) Others:

- Access road, lighting system, control house, OM Office, etc. are in poor condition, so that remedial works for those facilities will be required.
  - OM equipment such as vehicles, motor cycle, survey equipment, hydrological equipment, construction equipment is not sufficiently provided in OM office. Those equipment shall be strengthened in order to carry out the smooth OM works.
  - Alarm system for flood to the downstream area will be required in large dam, especially in dam to control flood by gate.

#### 5. Improvement of O/M Organization for Dam:

OM organization for dam shall be improved taking into consideration of the following items:

- Strength of OM staff consisting of hydrologist, topo-surveyor, water controller, mechanics electrician, etc. in large dam.
- Strength of National Water Resource Institute in Kaduna in order to analyze, evaluate and respond to the rising problems of dam brought from OM Office.
- Strength of coordination between dam OM office and other agencies managing water demand in service area.
- Allocation of sufficient funds to OM work.

# APPENDIX 4-4 WATER RESOURCES MANAGEMENT PROGRAM IN THE UPPER HADEJIA

#### 1. Background

The Lake Chad Basin extends over the Sahel and Sudan Savannah zones and is broadly divided into two: (1) the Hadejia / Jama'are-Komadugu / Yobe sub-basin and (2) the Yedseram-Chad sub-basin, both discharging into the fresh water Lake Chad. There would be presently many problems and several constraints for adequate water resources management due to the recent downward trend of Basin rainfall under severe Sahelian drought and also to the alarming prospect of drying up of the Lake Chad. In particular, there are an extensive Hadejia-Yobe wetlands along the middle part of this Basin which are composed of swamp, grassland and woodland created by the passage of the Hadejia and Jama'are Rivers. The area floods annually to an extent determined by river discharge, and this has supported the diverse socioeconomic activities.

In view of the inherent geological structure and less rainfall availability prevailing in this Basin, the surface runoff yield in the upper Hadejia/Jama'are is rather high, and large-scale water abstraction schemes in the Upper Hadejia Basin that were based upon an international advice in the 1960s due to the need of expanding irrigated agriculture at rapid population growth rate have constructed 14 large and medium-scale dams with a total active reservoir capacity of  $3 \times 10^9$  cu.m which is about twice as much as the inflow. It appears that at present a large proportion of the surface water resources are trapped in these reservoirs, and many of them are always maintained at a full water level with the fear of less incoming flow resulting in a fact that the reservoir water to be released for downstream is evaporating. This evaporation from 14 reservoirs amounts to 500 × 106 cu.m that is equivalent to about 30 percent of the potential surface water resources. In particular, the construction of two large dams at Tiga and Challawa Gorge has restricted the extent of flooding and groundwater recharge for these wetlands, and this fact remains that the water management over these wetlands has become an increasingly complex, technical, economic and political issues. This unfavorable way at existing reservoirs would have been caused mainly by the lack of inflow monitorings and proper reservoir operations rules and partly by

less water demand for the underdevelopment in downstream public irrigation projects. In addition, less emphasis has been placed upon artificial flooding over the downstream wetlands. The IUCN-funded Hadejia-Nguru Wetland Conservation Project as initiated in 1985 is taking a central role in encouraging the open debate on the present and future water management issues that are critical to the effective conservation of existing biodiversity.

#### 2. Outline of the Proposed Program

During the course of the National Water Resources Master Plan Study conducted by the Japan International Cooperation Agency, the unfavorable performance of larger public irrigation schemes has been highlighted. Undoubtedly, the most successful would be the Kano River Irrigation Project, but while the farmers' skills have increased over time, newly perceivable problems of a low rate of return and an increasing level of operating costs due to the poor level of OM over the past ten years may ultimately render a significant portion of the scheme inoperable. With the implementation of Tiga and Challawa Gorge Dams, the total area flooded over their downstream wetlands has been greatly reduced, while the existence of these dams has led to an overall reduction in the variability in the extent of flooding, and this has significantly reduced the level of risk for the wetland farmers. However, this appears to have had serious impacts on the agricultural production and waterrelated environment as a whole because the large-scale flooding formerly distributed the available water over the large wetlands and thereby effected the vital shallow-groundwater recharge.

In accordance with the recent progress of appropriate, small-scale and local-level fadama development in a form of the private irrigation scheme as supported by the World Bank and the EC, a focus has been given to the rehabilitation and improvement of Hadejia-Yobe wetlands in view of the immediate local human needs and food security. At present, part of these countermeasures has been taken by the Hadejia-Jama'are RBDA under the political pressures to release the limited flow from upstream water storages during wheat-growing season. It is highly recommended that in view of a surplus reservoir capacity in the Upper Hadejia Basin, the flooding over the wetlands to be made possible by artificial flood releases from those dams in the wet season should be maintained to make possible the production of rice, dry

season agriculture, fish, grazing, wildlife as well as biodiversity and shallow-groundwater recharge in connection with the establishment of an appropriate surface water and shallow-groundwater monitoring system.

Because of a complex nature in estimating the water demand over the Hadejia-Yobe wetlands, a series of test releases from the upstream Tiga and Challawa Gorge storages in a trial-and-error manner may be one of the most feasible options to identify the volume and timing of artificial flooding through the examination of an interrelationship between surface and shallow-groundwater on the basis of simultaneous hydrological observation work throughout the area. To this end, a flow depletion and conveyance study would be made possible to quantify an appropriate level of the artificial flood releases from the dams. In view of the urgent importance of this matter, the National Water Resources Master Plan has called for appropriate implementation of the Study on the Water Resources Management Program in the Upper Hadejia to be carried out under a framework of the technical cooperation program from external multinational or bilateral agencies. It may be noted that this Study should be a model for the recovery of wetlands functions, a result of which could be applied to other cases including those of the Sokoto-Rima.

#### 3. Terms of Reference

#### 3. 1 Objectives

General objective for the Technical Assistance on the Water Resources Management Program in the Upper Hadejia Basin is to prepare a feasible plan on the artificial flood releases from Tiga and Challawa Gorge dams for the Hadejia-Yobe wetlands. More particularly, the following specific objectives may be involved:

- Test releases from upstream dams for simultaneous hydrological and hydrogeological observations after installation of the related gages and preparation of the topographical maps.
- Selection of the most feasible options to identify a scope of artificial flooding through a series of the related studies.

- Formulation of an integrated water management program including the rehabilitation and improvement for existing facilities and the river improvement proposal.
- Preparation of a feasibility report including possible implementation program.
- Transfer of the planning skills and technologies to Nigerian personnel in Nigeria and donor country.

## 3.2 Agencies

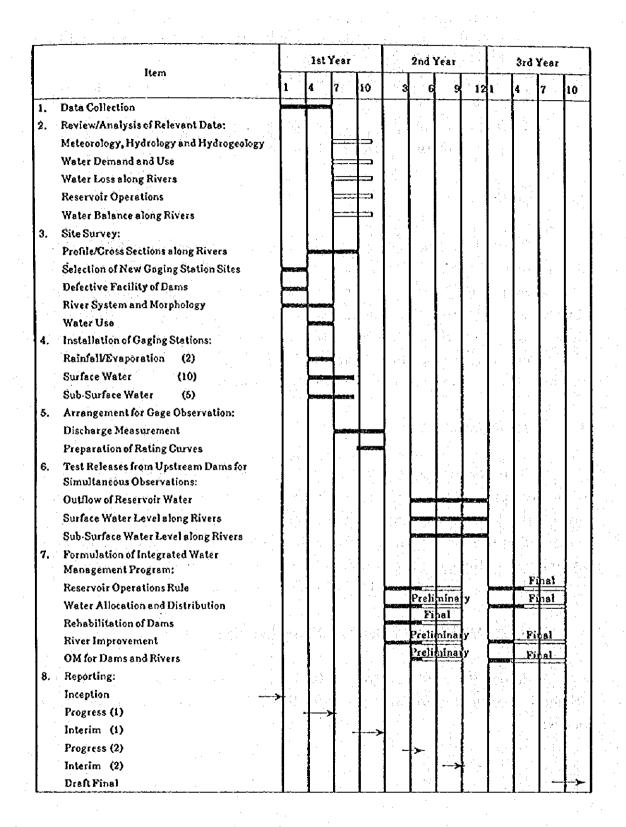
The executing agency will be the Hadejia-Jama'are River Basin Development Authority, Kano to be led by the Federal Ministry of Water Resources and Rural Development in cooperation with the National Water Resources Institute, the Federal Ministry of Agriculture and Natural Resources and the State Governments concerned. This Program will also be carried out in close consultation with the National Water Resources Manpower Training Program to be introduced in a separate manner.

#### 3.3 External Inputs

The external agency will dispatch a survey and study team of experts with the provision of equipment and materials to support proper implementation of the Technical Assistance Program. The team may be composed of the experts in the field of leadership, water resources planning, hydrology, hydrogeology, river, dam and reservoir, irrigation and drainage, socio-economy, agriculture and fisheries, and environment.

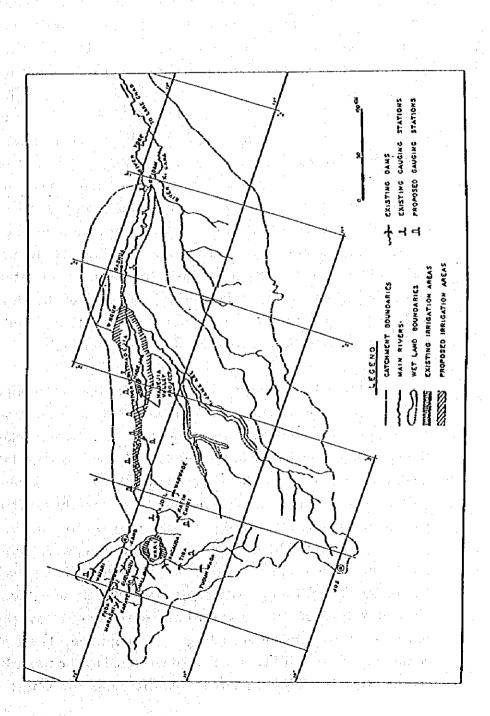
# 3.4 Scope of Work and Implementation Schedule Proposed

Major items to be involved in the Study and related implementing schedule during the proposed period of three years from 1996 are described below:



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# APPENDIX 4-5 PREPARATION OF MEDIUM / SMALL DAMS PACKAGE PROGRAM

## 1. Background

The JICA-assisted Study of the National Water Resources Master Plan (NWRMP) has reviewed that the Federal Government of Nigeria has placed the primary focus on large-scale water resources exploitation facilitated by the oil boom for food production; however, actual status is that the building of irrigation systems and related practices under large dams mostly in dry North has proceeded more slowly due to serious shortages of skilled and experienced manpower. With a view to mobilizing the water resources as called for by the National Perspective Plan, the NWRMP has been directed, apart from a comprehensive rehabilitation and improvement works for existing water storages, to implement a series of the proposed medium and small-scale multipurpose water resources development projects to meet water demand anticipated in various sectors concerned in such manner as (1) self-reliance, (2) human-centred approach and (3) focus on household production unit in informal sector on the basis of maintenance of environmental protection as well as the improvement in implementing capability of the Government agencies and the strengthening of institutions to facilitate more effective cooperation and greater participation of the local people concerned.

It may be noted that the history of irrigation in Nigeria has been that smaller-scale and decentralized schemes have been more successful from the cost-benefit and environmental standpoints than larger schemes. In line with the specific strategies involved, the NWRMP has been established to provide medium to small-size water storages at 1,084 sites located over the tributary basins in response to the demand predicted mainly for irrigation and water supply and partly for mini-hydro and aquaculture, on the premise that the priority area should be shifted to the Middle Zone with emphasis on irrigation and to the Southern Zone on water supply. These sites have been examined on the 1:50,000 FSN maps with short visits over some tributary basins with an area of 2 to  $5\times 10^3$  sq.km where a sufficient coverage of immediate downstream wetlands is seen for gravity irrigation. These multipurpose dams commonly of earth-fill type will be constructed by the River Basin Development Authorities (RBDAs) on force account basis for small and on domestic

contracting for medium, and relevant joint costs will be allocated among the responsible agencies concerned.

The NWRMP has given a preliminary guideline on the size of medium and small dams to cope with the above-mentioned functions:

	Medium Dam	Small Dam
Dam Height	15 to 25 m	below 15 m
Average Active Capacity	35 MCM	5 MCM
Maximum Active Capacity	50 MCM	7 MCM

While the proposed 264 medium dams with the catchment area of 50 to 500 sq.km have been confirmed on the 1:50,000 maps, it would be difficult to identify the small dams on these maps in view of the contour interval of 15 m; thus, only the number of small dams has been distributed in potential river basins on the basis of its catchment area (10 to 30 sq.km) as well as the location of farmland and village as are observed on the 1:50,000 maps. Basinwide dimensions of the medium and small dams as identified for implementation of the NWRMP are shown below:

			1.1	S 21 8 32	НΛ	, 5 t				Total
i Items		1	2	3	. 4	5	6	7	8	
1. Medium Dam				18 - 15 T						ago, di
(1) Number		14	74	39	53	14	41	29	0	264
(2) Active Capacity (	MCM)	700	2,940	2,090	1,250	340	910	630	0	8,860
(3) Available Water (	MCM)	480	2,140	1,460	890	. 330	830	590	. 0	6,720
For Irrigation (	MCM)	430	1,930	1,310	800	200	500	470	0	5,640
For Water Supply	(MCM)	50	210	150	90	130	330	120	0	1,080
2. Small Dam	********			*************					-, • • • • •	313 \$144
(1) Number	440.5	б0	230	110	160	35	100	115	20	820
	(MCM)	250	1,150	550	800	175	500	575	100	4,100
(3) Available Water		159	920	440	640	160	450	520	60	3,340
		135	830	395	575	140	405	465	55	3,000
For Water Supply		15	90	45	66	20	45	55	5	340
3. Total	*******		.,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,							
(1) Number	1.	64	304	149	213	49	141	144	20	1,084
(2) Active Capacity	(MCM)	950	4,090	2,640	2,050	515	1,410	1,205	100	12,960
(3) Available Water		630	3,060	1,900	1,530	490	1,280	1,110	60	10,060
	(MCM)	565	2,760	1,705	1,375	340	905	935	55	8,640
For Water Supply		65	300	195	155	150	375	175	5	1,420

Notes: (1) Rate of irrigation water for the total available water for the medium dams is given taking into account the large water demand for the water supply in the South region.

11A-1,2,3 and 4 · · · 90%, HA-5 and 6 · · · 60%, HA-7 · · · 80%.

(2) Active capacity and available water for the small dams are estimated:

Average active capacity...... 5 MCM/unit

Average available water ...... 3 MCM for HA-1 and 8

4 MCM for HA-2, 3, and 4 4.5 MCM for HA-5, 6, and 7

Irrigation water ...... 90% for the above available water

Prior to adequate implementation of the proposed small and mediumsized dam projects, a series of the sub-basin wide pre-feasibility and feasibility study should be a prerequisite to examine the possible site location and relevant water input-output for each dam and formulate a package program of the dams to be implemented with a high priority from the technical, social, economic and environmental point of view.

### 2. Outline of the Proposed Program

The NWRMP has identified 264 potential sites for medium-sized dams and 820 sites for small dams to be built along the tributaries throughout Nigeria during the NWRMP period through office work on the 1:50,000 FSN topographical maps along with short field visits to some potential sites; therefore, subsequent pre-feasibility study for site identification and full-scale feasibility study for preparation will be required for orderly implementation. When executing a series of these procedures, a sub-basinwide approach is a pre-requisite to prepare an appropriate medium and small dams package program in order to verify well-balanced allocation of the water resources in a particular basin with the priority sub-project rating.

The first step to be taken for this long-range implementation program during the NWRMP period is to carry out a pilot study and preparation of the medium and small dams package programs for some basins where the fundamental technology for selection and planning of potential sub-projects should be developed as model. To select these model basins, the following criteria have been provided:

- Potential water resources : High in amount

Existing water resources development : Few in number

- Potential medium and small dams proposed: Many in number

Population density : High
Per capita farmland : High

- Irrigable area proposed : High in scope

- Increase of demand for water supply

High rate at

2020/1991

- Surface water withdrawal rate in 2020

High

- Convenience for the management by RBDA and

the demonstration effect for other basins in

similar nature from locational point of view: Favorable

- Access to the site

Easy

- 1:50,000 FSN Maps

Available

- Basin area size for the Study

Moderate

First, 17 sub-basins were screened out, and with a principle of one basin per RBDA, the following nine model basins which are designated at "Priority Basins" have been identified:

				Basin Area	Population	Surface Water	No of Proposed	Proposed Irrigable Area	Water Supply Demand in 2020
No.	SHA	Basis	ROBA	(10 <sup>3</sup> sq.km)	2020(10 <sup>6</sup> )	(10 <sup>9</sup> cu.m)	Dams	(10 <sup>3</sup> ha)	(10 <sup>3</sup> cu.m)
P. 1	110	Danzoki	Sokoto-Rima	0.81	0.75	1.01	14	11	11
P. 2	214	Gbako	Upper Nigar	7.67	1.25	1.68	24	43	52
P. 3	204	Awua	Lower Niger	7.15	1,45	1.07	29	30	118
P. 4	901	Kilange	Upper Beage	9.45	1.18	1.43	37	16	6
	4052 to 3	Lower	Lower Beaue	8.55	1.68	12.80	37	19	17
• .	1000	Kalsina-Ala							•
P. 6	504	Mamu	Amambra Imo	4,27	5.88	0.73	23	8	50
P.7	6023 to \$	Upper Ogua	Ógu-Óshua	20.14	2.54	2.60	36	19	50
P. 8	608	Osse	Benin Owens	13.73	4.00	1.20	22	9	89
P 9	702	Aya	Cross	8.66	2.00	2.41	97	16	11

It is scheduled that all of the studies for nine Priority Basins will be carried out by the FMWRRD during the National Water Master Action Plan period by 2000 with the positive participation of State Water Agencies (SWAs) for domestic water supply, the National Electric Power Plc. (NEPA) and State Rural Electricity Boards for mini-hydro, the Federal Department of Fisheries and State agencies concerned for fisheries, and others. In principle, all of these activities will be supported under the proposed Water Resources Manpower Training Program to be assisted by the external agencies; however, in view of the work load under this Program, the study and preparation for three Priority Basins of P.3 (Awun), P.4 (Kilange) and P.9 (Aya) which are most representative from the locational point of view would be duly requested possibly to three external agencies for more elaboration. It is also

recommended that in view of the past neglect in the environmental factors, a special technical assistance program by an external agency that is called at "EIA Study and Environmental Monitoring Program for Dam Projects" is carried out in a separate way to support the preparation of medium and small dams package programs.

For more clear understanding on the scope of work to prepare a series of the medium and small dams package programs, the general guidelines and criteria have been presented being apart from the procedures for the EIA:

## Pre-Feasibility Study:

- Installation of hydrological gaging stations and their reliable observation over the basin.
- Study for potential surface water over the basin dividing into those for tributary basins with the area of 1,000 to 2,000 sq.km.
- Identification of potential sub-projects to be developed by proposed dams on the 1:50,000 maps and site reconnaissance survey.
- Preparation of approximate outline of identified sub-rpojects including potential surface water, number of sub-projects, dimension of dam and reservoir, irrigable area size, water demand, socio-economic condition, capital cost, etc.
- Water balance and allocation study at each sub-project and tributary basins as a whole based on potential surface water and water demand.
- Screening of most likely feasible sub-project and tributary basins for subsequent feasibility study.

# Feasibility Study:

- Preparation of topographical maps for tributary basin, damsite and service area.
- Geological investigation at damsite, soil survey over service area, socio-economic survey, EIA, etc. for the proposed sub-project areas.
- Feasibility study at international level including plan formulation, preliminary design for project facility, project cost estimates and evaluation, etc.

#### Preparation of Package Program for Implementation:

- Package program for two to four sub-projects to be developed by medium dams which will be implemented by domestic contractor.
- Package program for five to ten sub-projects to be developed by small dams which will be implemented on the RBDA force account basis.

In order to conduct the above-mentioned study smoothly and rationally, the FMWRRD will be directed to prepare the guidelines to be applied for the following items prior to the pre-feasibility study.

- Hydrological analysis.
- Reservoir planning.
- Geological and soil survey.
- Dam standard section and appurtenant structure.
- Irrigation water demand and per capita demand for water supply.
- Irrigation canal system.
- Cost estimate including the cost allocation procedures for multipurpose joint facilities.
- Environmental Impact Assessment (EIA).

In addition, particular attention will be paid to the feasibility study of service components under the proposed multipurpose medium and small water storages:

- Socio-economic and land tenure conditions in service area, especially problem and constraint to be solved for achievement of successful irrigated agriculture.
- Local people participation for project planning and implementation.
- Definitive area for irrigation based on available irrigation water in water storage and irrigation water demand in service area.
- Irrigation facilities to be managed easily and effectively by WUA.
- Multipurpose use of water for irrigated agriculture, domestic water supply, fish culture, mini-hydro, reforestation, etc.
- Adequate project implementation program including proper project cost estimate and implementation schedule.

- Project evaluation including economic evaluation.
- Institutional building of RBDA, State agencies, and beneficiaries to implement the project and manage irrigated agriculture and other activities.

#### 3. Terms of Reference

#### 3.1 Objective

General objective for the Technical Assistance Program on the Preparation of Medium / Small Dams Package Programs to be carried out by three Study Teams of the external agencies is to prepare the medium / small dams package programs for three Priority Basins of P.3 (AWUN), P.4 (Kilange) and P.9 (Aya) for immediate implementation. Major profiles on these basins as demarcated in the NWRMP are given below:

		T	HA-2	HA-3	HA-4
		1	Awun	Kilange	Aya
		. [	SHA 204	SHA 301	SHA 702
	Basin Features				
-	(1) Catchment Area	(km²)	7,150	9,450	8,660
	(2) Agriculture Land	(10 <sup>3</sup> ha)	413	368	711
	(3) Population in 2020	(10 <sup>3</sup> )	1,466	1,183	1,999
	(4) Population Density	(km²)	205	125	231
	(5) Agricultural Area / Population	(ha)	0.28	0.31	0.36
	(6) Potential Surface Water	(MCM)	1,070	1,430	4,820
Ž,	(7) Potential Surface Water/Population	(m <sup>3</sup> )	730	1,210	2,410
}	Existing Dam Projects				
	(1) Number		2	0	2
	(2) Active Capacity	(MCM)	34	0	74
	(3) Available Reservoir Water	(MCM)	27	0	54
١.	Proposed Dam Projects				
	(1) Number	1 de 18 (18)	29 (17)	27 (18)	37 (26)
: -	(2) Active Capacity	(MCM)	527 (85)	300 (90)	345 (130)
	(3) Available Water	(MCM)	380 (64)	234 (67)	317 (117)
<b>5</b> .	Total Available Water	(MCM)	407	234	371
5.	Irrigation Component			A facility to the parties	
	(1) Existing Area	(10 <sup>3</sup> ha)	0	0	2
	(2) Proposed Area	(10 <sup>3</sup> ha)	30	16	16
	(3) Total Area	(10 <sup>3</sup> ha)	30	16	18
	(4) Water Demand	(MCM)	340	214	238
6.	Water Supply Component				
	(1) Water Supply Capacity in 1991	(MCM)	25.4	1.5	1.6
1 7	(2) Water Demand in 2020	(MCM)	118	6	11
	(3) Increase Rate of Water Supply	(%)	4.6	3.7	6.9
1.	Total Water Demand	(MCM)			
	(5 (4) + 4 (2))		458	220	249
3.	Surface Water Withdrawal Rate	(%)			
	{7 + 1 (6)}	1.	43	16	10

Note: ( ) in the above figures shows the those of small dams

More particularly, the following specific objectives may be involved;

- Pre-feasibility study for identification of possible sites covering the Priority Basins of P.3 (Awun), P.4 (Kilange) and P.9 (Aya) as representatives.
- Feasibility study for five core multipurpose dam sub-projects and related service components including two for medium and three for small covering each of the Priority Basins.
- Preparation of a feasibility report for each Priority Basins compiling the guidelines and criteria for selection and planning in similarnatured projects and possible implementation program for sector financing.
- Transfer of the planning skills and technologies to Nigerian personnel in Nigeria and donor countries.

#### 3. 2 Agencies

The executing agencies will be each of the Lower Niger, Upper Benue and Cross River Basin Development Authorities to be coordinated by te Federal Ministry of Water Resources and Rural Development in cooperation with the National Water Resources Institute, Federal Ministry of Agriculture and Natural Resources, National Electric Power Plc., and State Governments concerned. This Study will also be carried out in close consultation with the National Water Resources Manpower Training Program and the EIA Study and Environmental Monitoring Program for Dam Projects both to be introduced under the technical assistance in a separate way.

# 3.3 External Inputs

Each of three external agencies will dispatch a survey and Study Team of required experts with the provision of equipment and materials to support proper implementation of the technical assistance program. Each team may be composed of the experts in the field of leadership, water resources planning, hydrology, dam and reservoir, engineering geology, soil mechanics, river, agriculture, irrigation and drainage, fisheries, mini-hydro, socio-economy, environment and project economy.

## 3. 4 Scope of Work and Implementation Schedule Proposed

## i) Pre-Feasibility Study

The pre-feasibility study shall be performed at the model river basin with the priority under each RBDA:

#### (a) Data to be Collected

- (i) Aerial Photograph and the 1 to 50,000 scale map.
- (ii) Hydrological data in the basin and adjacent basin.
- (iii) Socio-economic data at LGA level in the basin.
- (iv) General geological map in the basin.
- (v) Existing water resources projects including location and outline.
- (vi) Other necessary data.

## (2) Preparatory Work for the Study in the Office

- (i) Division of the basin into the several tributary basins with the catchment area of 1,000 to 2,000 km<sup>2</sup> on the 1 to 50,000 scale map of and measurement of their catchment area.
- (ii) Study for the basin salient features such as topography and geology, tributary system, land use, socio-economy, etc.
- (iii) Study for hydrological condition such as aerial rainfall and evaporation, annual average runoff, sediment transport, peak flood discharge, etc. Preparation of the guideline for the specific yield of annual average runoff (mm), peak flood discharge (m³/sec/km²) and sediment transport (m³/km²) in order to estimate approximate resevoir inflow and reservoir capacity.
- (iv) Study for water demand in service area and reservoir water use under the existing dams.
- (v) Location of the proposed dams and serice area on the 1 to 50,000 map.
- (vi) Measurement of catchment area, reservoir area, dam height and dam length at the proposed damsites on the 1 to 50,000 scale map.
- (vii) Approximate estimation for the reservoir inflow based on the catchment area and the specific runoff yield.

- (viii) Approximate estimation for the reservoir active capacity based on the reservoir area and effective reservoir depth.
  - Preparation of the guideline for the effective reservoir depth taking into account the reservoir dead water depth, spillway overflow depth, and dam free board.
  - (ix) Classification of the proposed dams by the reservoir and dam size.
  - (x) Drawings of the location, catchment area, reservoir area, damsite, etc. for the existing and proposed dams on the 1 to 50,000 map.

#### (c) Site Survey Works

3

- (i) Topographical and geological condition at damsite and reservoir area, especially rock outcrop, gully erosion, vegetation, etc.
- (ii) Overburden depth at damsite by providing test pit or hand auger.
- (iii) Water level and runoff fluctuation at rivers by observation and interview to local people.
- (iv) Longitudinal survey along the proposed dam axis.
- (v) Longitudinal and cross-section survey in the reservoir area to estimate the reservoir capacity. (Longitudinal length of 5 km along the river and cross section interval of 250 to 300 m)
- (vi) Selection of gaging station site for the surface water observation.
- (vii) Agricultural and socio-economic condition survey at the service area by questionnaire.
- (viii) Beneficial intention survey for the proposed dams.
  - (ix) Environment survey in the reservoir and service area.

# (d) Analysis and Study in the Office Work based on Site Survey

- (i) Hydrological analysis for monthly rainfall, evaporation and runoff and design flood capacity for spillway.
- (ii) Study of the reservoir dimension and preparation of H-A and H-Q curve.
- (iii) Water demand study in the service area.
- (iv) Simple reservoir operation study and available reservoir water for the service area.
- (v) Water balance study in tributary basin based on the potential surface water and available reservoir water use.

- (vi) Dam standard section and dam embankment volume.
- (viii) Potential irrigation area and domestic water supply quantity by the proposed dam.
- (viii) Environment impact at the project area.
  - (ix) Approximate dam construction cost based on the dam embankment volume.

## (e) Selection of the Representative Dam for the Feasibility Study

In accordance with the above study, five representative core dams will be selected for the feasibility study with the following conditions:

- Dams consisting of the medium and small dams with the representative size.
- Dams with priority from viewpoint of the project economy.
- Dams without environment problems.
- Dams with easy condition for the survey and study such as accessibility to damsite.

## ii) Feasibility Study

The feasibility study shall be performed for the above selected representative dams based on the international standard. The particular attention shall be paid for the following items:

## (a) Survey and Investigation Work

- (i) Preparation of aerial photo map of 1 to 10,000 scale covering the reservoir area, damsite and service area. (Average area of 50 km<sup>2</sup>/each representative dam)
- (ii) Installation of bench marks at damsite and topo-survey at the damsite (plain map scale of 1 to 2,000)
- (iii) Installation of rainfall and water gages and continuous observation at stations.
- (iv) Geological survey at the damsite by seismic prospecting, core drilling and test pits.

(v) Soil sampling at the borrow area and its mechanical test in laboratory for construction materials.

# (b) Hydrological Analysis

- (i) Runoff analysis on daily basis by preparation of the runoff model.
- (ii) Design flood capacity analysis for spillway based on the unit hydrography.

# (c) Reservoir Plan Study

- (i) Preparation of accurate reservoir H-A and H-Q curve.
- (ii) Reservoir operations study on 10 days basis.
- (iii) Formulation of the reservoir operation rule.
- (iv) Estimation of available reservoir water in wet and dry season and also in wet and dry year.

# (d) Preliminary Design of Dam and Appurtenant Structures

# (i) Dam Body

- Dam foundation treatment method based on the geological investigation.
- Available embankment material at reservoir area and borrow area from viewpoint of quantity and quality.
- Analysis for flow net and equipotential line and dam stability based on the standard dam section.

# (ii) Outlet and Spillway

- Hydraulic and structural analysis.
- Application of simple structure for intake such as drop inlet or inclined inlet.
- Application of outlet conduit by reinforced concrete without steel pipes, taking into account the low water head in the medium and small dam.

Application of Ogee type weir without gate.

### (e) Construction Plan and Cost Estimate

- Construction method and schedule on the contractor basis or force account basis of RBDA.
- Cost estimation preparing the bill of quantity.

### (f) OM Plan

OM organization, equipment, facility and OM cost.

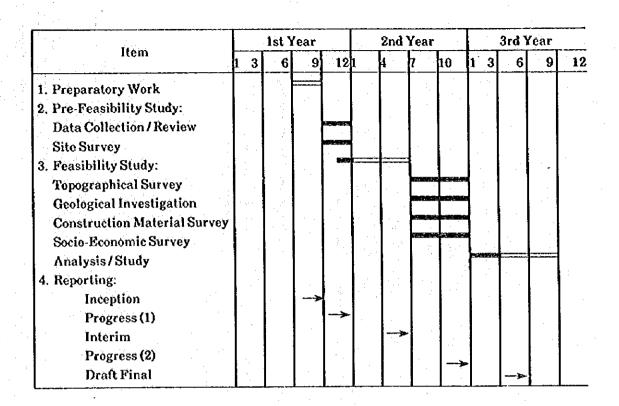
## (g) Environmental Impact

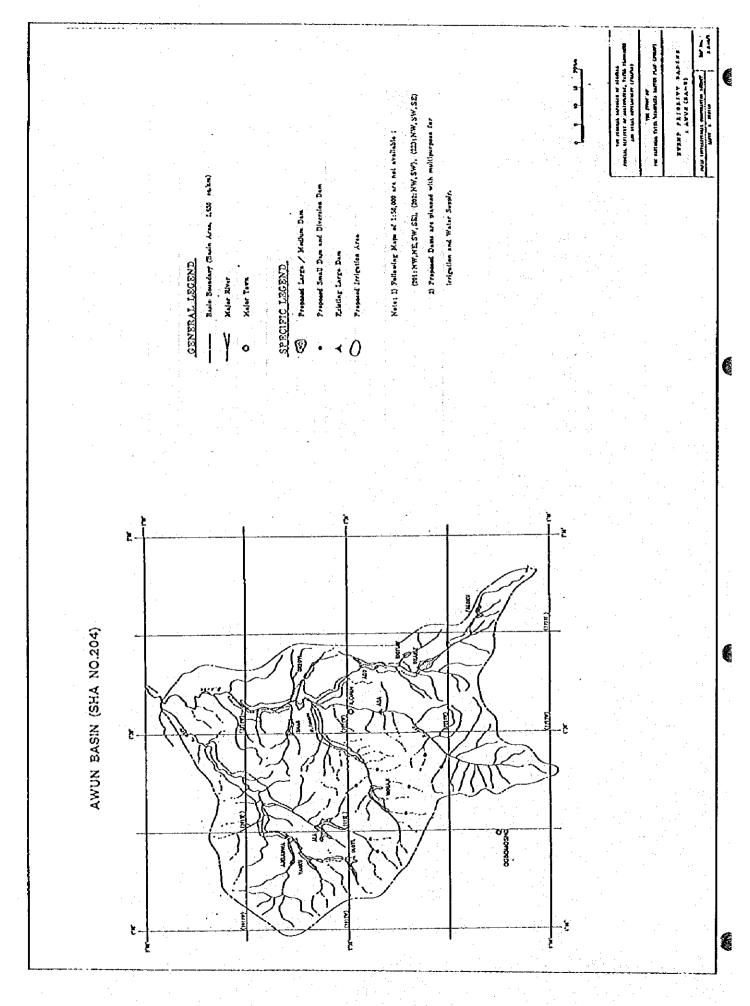
- (i) Resettlement problem in the reservoir area.
- (ii) Water pollution problem during construction.
- (iii) Variation of the river flow at the downstream after completion of dam.
- (iv) Other ecological problem by dam construction.
- (v) Preparation of the environmental impact statement.

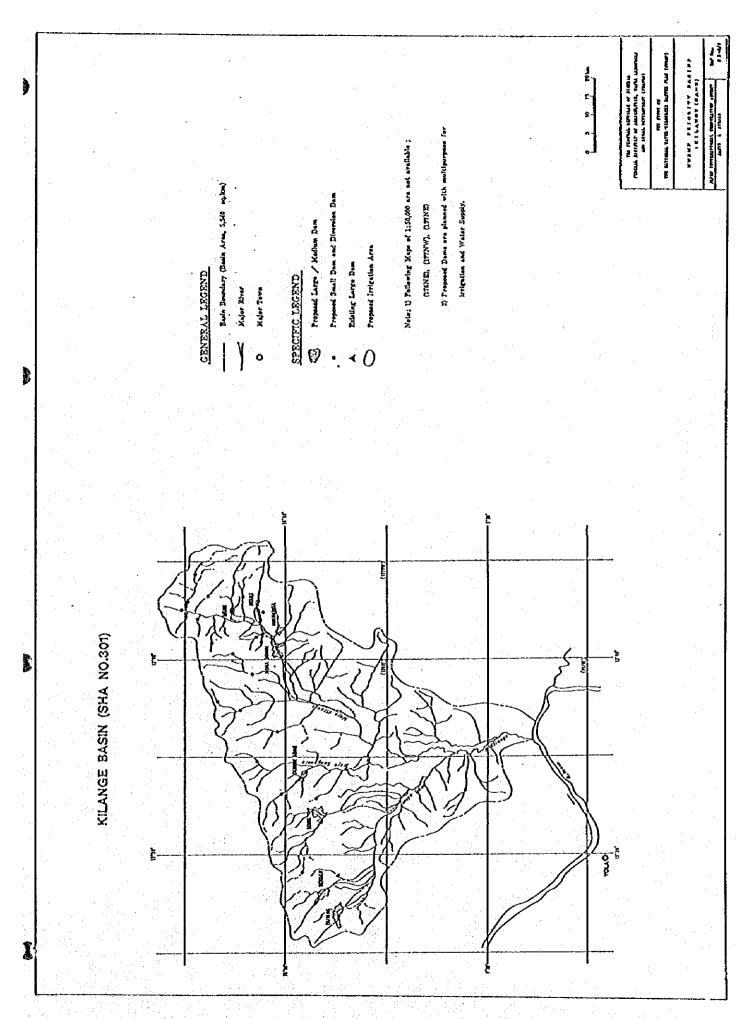
### (h) Project Evaluation

- (i) Water cost to be developed by dam.
- (ii) Economical and financial analysis for the project cost.
- (iii) Evaluation of farmer income generation and job creation.
- (i) Preparation of the Guideline and Criteria for Selection and Planning in Similar-Natured Projects
- (j) Preparation of Possible Implementing Program for Sector Financing.

The implementing schedule proposed for each of three priority subbasins is given below.







# CHAPTER 5. IRRIGATION AND DRAINAGE

# CHAPTER 5: IRRIGATION AND DRAINAGE

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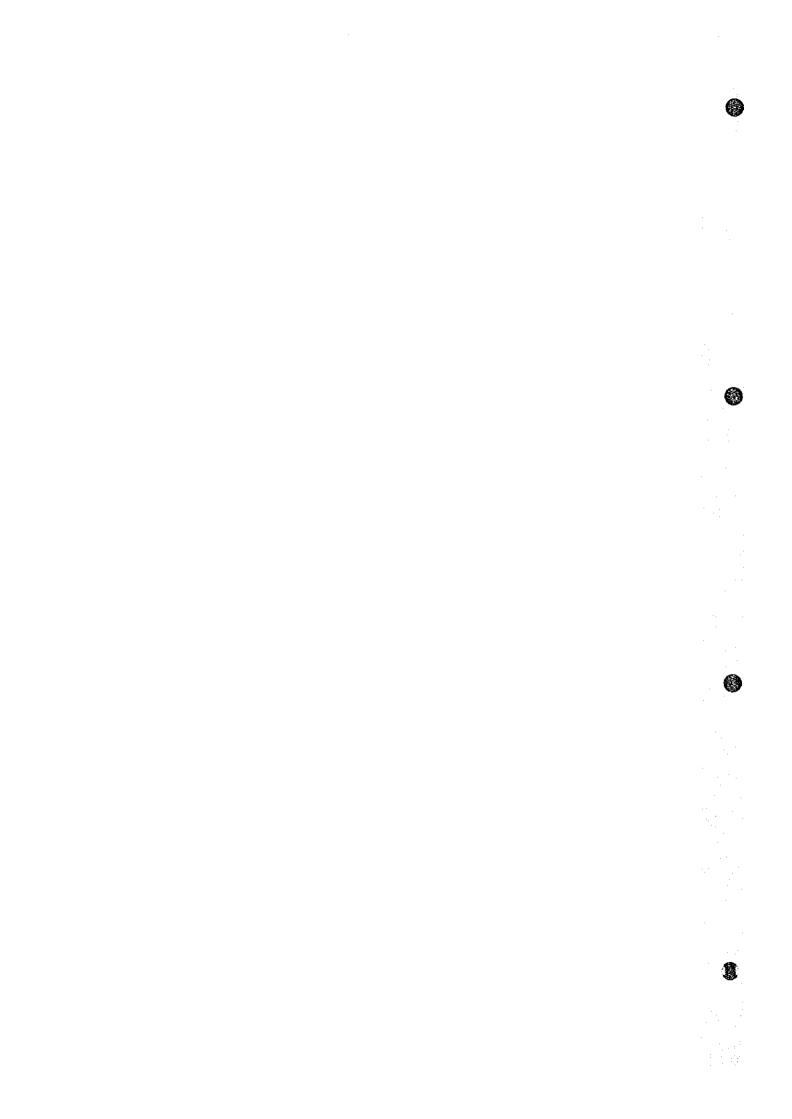
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# CHAPTER 5. IRRIGATION AND DRAINAGE

### 5. 1. GENERAL BACKGROUND

## 5. 1. 1 Historical Perspectives

1

1

Some simple forms of dry season irrigation on individual basis in the wetlands have probably been practiced in the North Region from time immemorial, particularly near large cities such as Kano and Sokoto. The farmers practiced lift irrigation through lifting devices such as shadouf and calabash to increase crop production. By the second decade of this century, the increasing population pressure and the knowledge that rainfall and river flow were trending downward under the frequent severe droughts had caused the then colonial administration to construct small dams and diversions in some areas of the North, especially to improve the Fadama water regime. Most of these works, however, were carried out in an ad hoc manner, and while some of the farmers have benefited, others have suffered from their lands being flooded for water storage. Moreover, wet season rainfall washed away many of the infrastructures. Under this situation, most of those activities had been abandoned by the middle of 1920's.

A fresh start was made in 1949 when an irrigation division was set up in the then Northern Provinces Agricultural Department and an irrigation training school was established in Sokoto. This new division initiated a village irrigation scheme, and by 1960 the area irrigated totaled 3,400 ha, of which 400 ha are in the arid zone and the remaining 3,000 ha in the Fadama areas of the Niger Valley. It is believed that this scheme failed more for social reasons than technical problems. In the South Region irrigation began much later. In the South-East Region the first irrigation engineer was appointed in 1960 and he began to promote rice growing mostly with wet season supplementary irrigation in the inland swamps, while in the South-West Region there was virtually no irrigation development except for the few attempts made to promote wet season supplementary irrigation in Fadama areas prior to the establishment of RBDAs in 1976.

Following the end of the civil war in 1970, various foreign-assisted studies and related pre-activities conducted in the 1960s had convinced FGN to

initiate more irrigation programs especially for rehabilitation of irrigation facilities and provision of more food after the war hazards ended and in association with rapid growth of the national economy resulting from rise in oil exports and more importantly, rise in the real price of oil. In the North Region, large-scale irrigation was initiated as a solution to the constraints caused by the poor response of basic crops (sorghum and millet) to modern inputs, increasing population pressure, and continued decline of rainfall. Between the years 1970 to 1974, the Kano State Government constructed the Tiga dam and the 17.6 km long main canal. In the humid South-East Region, population pressure was the major concern, and increase in rice production through irrigation was recognized as one of the ways where the problem of ensuring family food security from relatively small land holdings could be overcomed. In the 1970s, public sector irrigation investments rose sharply and remained very high until the end of the oil boom.

The balance of payments crisis in 1978/79 was met by government budget cutbacks and import restrictions. At the same time, the problems of rapid population growth in urban areas and the relative decline in rural areas became apparent; thus, the Fourth Plan (1981-85) tackled these problems to achieve the targets, in which the highest priority was given to "hastening the development of rural areas and agriculture", as compared with the relatively neglected budget for agriculture in the Third Plan. In mid-1986, FGN launched the Structural Adjustment Program (SAP) with the introduction of a marketdetermined exchange rate and elimination of import licensing system, which so far have resulted in significant increase in incentives for the agriculture sector. Under this situation, one of the successful initiatives of the World Bank-funded State ADPs, under the coordination of DOA, is the small-scale, low-cost, farmer-managed irrigation in wetlands, which is in contrast with the largescale irrigation schemes promoted by RBDAs. These large-scale schemes, involving high capital costs and long gestation period, have been completed with a few exceptions and the remaining are either reduced to pilot activities or frozen at various stages of implementation. 医二氏氏征 医野乳桂醇 计自由语言 医线线

# 5.1.2 Necessity of Irrigated Agriculture

Recently, increase in food production has become necessary to cope with the rapid population growth. Since increase in food production through

is the first physical factors are gardined as a sufficient set.