



SLAKING TENDENCY

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REPUBLIC OF THE PHILIPPINES
DEPARTMENT OF JUSTICE
OFFICE OF THE ATTORNEY GENERAL

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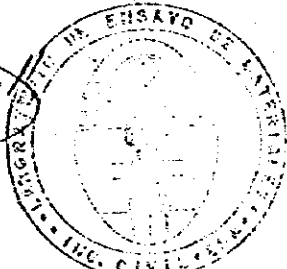


UNIVERSIDAD POLITÉCNICA NACIONAL
FACULTAD DE INGENIERÍA CIVIL
LABORATORIO DE MECÁNICA DE FLUIDOS

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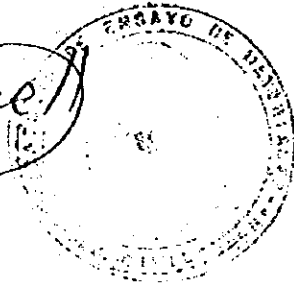
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SECRETARIA DE ECONOMIA Y FINANZAS
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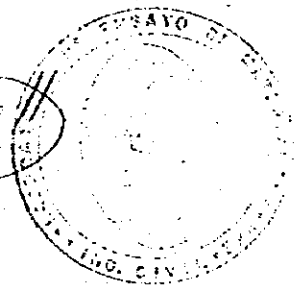


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SECRET
DEPARTMENT OF DEFENSE
OFFICE OF THE SECRETARY

CONFIDENTIAL

Announcement



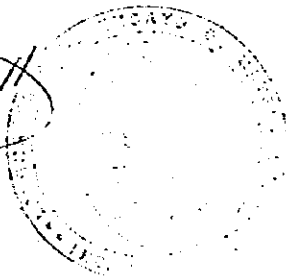
The seal is circular with the text "DEPARTMENT OF DEFENSE" around the top and "OFFICE OF THE SECRETARY" around the bottom. The center contains a smaller emblem.

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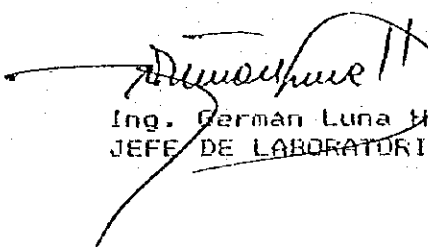
ESCUELA POLITECNICA NACIONAL
FACULTAD DE INGENIERIA CIVIL
LABORATORIO DE MECANICA DE ROCAS

PROYECTO "TRASVASES MANABI"

FECHA: ENERO 1994
ENVIO : 3
NORMA UTILIZADA: ISRM

ENSAYO DE DURABILIDAD SLAKE

SONDEO	SR-93-3
MUESTRA Nº	1
PROFUNDIDAD (m)	20.36-20.72
DESCRIPCION	ARENISCA GRAND FINO
PESO INICIAL SECO, A (gr)	521.0
PESO FINAL SECO DEL PRIMER CICLO, B (gr)	412.0
PESO FINAL SECO DEL SEGUNDO CICLO, C (gr)	280.6
INDICE DE DURABILIDAD SLAKE, Id2 (%)	79.1
FLUIDO EMPLEADO	AGUA POTABLE A 15 °C
APARIENCIA DEL MATERIAL QUE PASA LA MALLA	LIMOS Y ARCILLAS CON ARENA FINA
APARIENCIA DEL MATERIAL RETENIDO EN LA MALLA	REDONDEADAS/ONDULADAS


Ing. German Luna H.
JEFE DE LABORATORIO



REPUBLICAN PARTY OF CALIFORNIA
COUNTY OF SAN DIEGO
LOCAL COMMITTEE

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AMOS

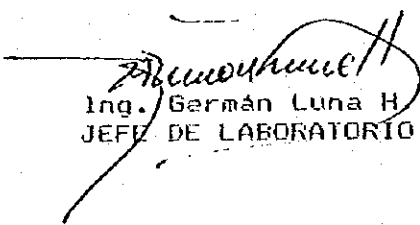
ESCUELA POLITÉCNICA NACIONAL
FACULTAD DE INGENIERÍA CIVIL
LABORATORIO DE MECÁNICA DE ROCAS

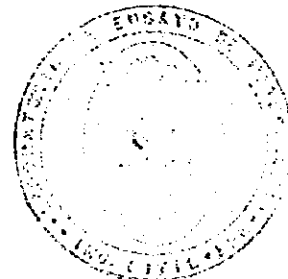
PROYECTO "TRASVASES MANABI"

FECHA: ENERO 1974
 ENVÍO : 3
 NORMA UTILIZADA: ISRM

ENSAYO DE DURABILIDAD SLAKE

SONDEO	SR-93-5
MUESTRA Nº	1
PROFUNDIDAD (m)	8.79-9.00
DESCRIPCIÓN	ARENISCA GRANO FINO
PESO INICIAL SECO, A (gr)	485.8
PESO FINAL SECO DEL PRIMER CICLO, B (gr)	443.7
PESO FINAL SECO DEL SEGUNDO CICLO, C (gr)	487.7
INDICE DE DURABILIDAD SLAKE, Id2 (%)	83.9
FLUIDO EMPLEADO	AGUA POTABLE A 15 °C
APARIENCIA DEL MATERIAL QUE PASA LA MALLA	ARENAS MEDIAS Y GRUESAS LIMOS
APARIENCIA DEL MATERIAL RETENIDO EN LA MALLA	REDONDEADOS CON FRAGMENTO PEQUEÑO REDONDEADO


 Ing. Germán Luna H.
 JEFE DE LABORATORIO



REPUBLICA DE COLOMBIA
SECRETARIA DE INGENIERIA Y CONSTRUCCION
DEPARTAMENTO DE INGENIERIA DE PUENTES

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ESTUDIO DE VIABILIDAD TECNICA Y ECONOMICA
DE LA OBRA DE CONSTRUCCION DE UN PUNTE

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SECRETARIA DE INGENIERIA Y CONSTRUCCION

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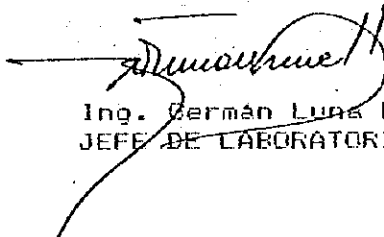
ESCUELA POLITECNICA NACIONAL
FACULTAD DE INGENIERIA CIVIL
LABORATORIO DE MECANICA DE ROCAS

PROYECTO "TRASVASES MANABI"

FECHA: ENERO 1994
ENVIO : 3
NORMA UTILIZADA: ISRM

ENSAYO DE DURABILIDAD SLAKE

SONDEO	MG-93-2
MUESTRA Nº	1
PROFUNDIDAD (m)	36.52-37.00
DESCRIPCION	ARENISCA GRANO FINO
PESO INICIAL SECO, A (gr)	446.5
PESO FINAL SECO DEL PRIMER CICLO, B (gr)	293.0
PESO FINAL SECO DEL SEGUNDO CICLO, C (gr)	56.1
INDICE DE DURABILIDAD SLAKE, Id2 (%)	12.6
FLUIDO EMPLEADO	AGUA POTABLE A 15 °C
APARIENCIA DEL MATERIAL QUE PASA LA MALLA	LIMOS Y ARENAS FINAS
APARIENCIA DEL MATERIAL RETENIDO EN LA MALLA	FRAGMENTOS REDONDEADOS TAMANO VARIABLE

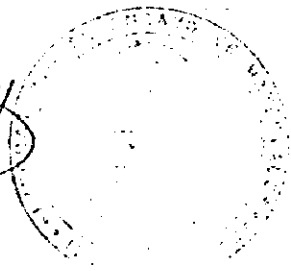

 Ing. Germán Luna H.
 JEFE DE LABORATORIO



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**JAPAN INTERNATIONAL
COOPERATION AGENCY
J I C A**

**THE DETAILED DESIGN STUDY ON THE WATER
TRANSBASIN SCHEMES FOR CHONE-PORTOVIEJO
RIVER BASINS**

**GEOLOGICAL - GEOTECHNICAL
INVESTIGATIONS REPORT**

VOLUME II

FEBRUARY 1994



JAPAN INTERNATIONAL COOPERATION AGENCY
J I C A

DETAILED DESIGN STUDY ON THE TRANSBASIN WATER
PROJECT FOR THE CHONE-PORTOVIEJO RIVER BASINS

REPORT ON GEOLOGICAL - GEOTECHNICAL RESEARCH

VOLUME II

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5.3	Results obtained
5.3.1	Open Channel
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7.	APPENDIXES
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JAPAN INTERNATIONAL COOPERATION AGENCY
J I C A

DETAILED DESIGN STUDY ON THE TRANSBASIN WATER
PROJECT FOR THE CHONE-PORTOVIEJO RIVER BASINS

REPORT ON GEOLOGICAL - GEOTECHNICAL RESEARCH

5. TEST PITTING AND MANUAL SOUNDING

5.1 PURPOSE AND SCOPE OF THE INVESTIGATIONS

The Geotechnical Investigation Campaign using test pitting and manual sounding with standard penetration tests, "SPT," is in order to investigate the geomechanical characteristics of the highly weathered soils and rock.

For this purpose, geotechnical logging was performed both for the test pitting as well as the sounding, and samples were taken for study, depending on the focus to be given the study whether based on the foundation work or the use of the material excavated for construction material. Thus, three main activities may be defined:

- Test Pits along the Open Channel

They were excavated for the purpose of learning the characteristics of the colluvial soil or weathered rock in order to use said soils as construction materials in the earth fill of the channel. Therefore the sampling used disturbed samples on two levels.

- Test Pits along the Transmission Line

Excavated for the purpose of learning the geomechanical characteristics of the soils or weathered rock for the foundations of towers or other types of structures. For this purpose, undisturbed samples were obtained of the soils wherever possible.



- Manual Sounding with standard penetration tests

Performed for the purpose of evaluating the soil and weathered rock resistance characteristics and on three different fronts:

- Along the Open Channel, especially on the intersections of the channel line and streams.

- Along the Transmission Line, basically at the sites for the foundations of the towers.

- At the entrance to the Poza Honda-Mancha Grande-Estero Guajabito Tunnel and the exit of the La Esperanza-Poza Honda Tunnel at the Pata de Pajaro River for the purpose of studying the building of a Retaining Wall for these sites for the construction of the underwater portals.

() 5.2 WORKS PERFORMED AND METHODS USED

5.2.1 Test Pits

Fifteen test pits were excavated, ten of which were along the section of the channel and five along the transmission line. The location of same is shown in Appendix No. 7.4

The depth and the samples taken are shown in the following table:



WORK: OPEN CHANNEL

TEST PIT №	COORDINATE		DEPTH ■	TYPE OF SAMPLE		ROCK LINE ■
	N	E		DISTURBED	INDISTURBED	
C - 10	9892630	607645	3,00	0,50-1,40 2,00-3,00	-	1,40 m
C - 11	9892440	607000	2,00	0,55-2,00	-	N.A. 1,60 m
C - 12	9891880	606500	4,00	0,60-1,60 2,00-3,60	-	3,60
C - 13	9891540	606387	4,00	0,80-2,00 2,00-3,50	-	2,60
C - 14	9891045	606183	4,00	0,90-1,90 1,90-3,50	-	3,60
C - 15	9890440	605330	3,00	0,40-1,00 1,20-1,60	-	1,60
C - 16	9890110	605090	4,00	0,85-1,80 2,00-3,50	-	3,20
C - 17	9889600	604670	3,00	0,40-1,60 2,00-3,00	-	1,60
C - 18	9889155	604290	3,00	0,30-1,60 2,00-3,50	-	0,90
C - 19	9888780	603850	4,00	1,00-1,70 1,20-3,60	-	3,60

WORK: TRANSMISSION LINE

TEST PIT №	COORDINATE		DEPTH ■	TYPE OF SAMPLE		ROCK LINE ■
	N	E		DISTURBED	INDISTURBED	
C - 20	9892665	607780	1,50	0,35-1,10	-	1,10
C - 21	9892400	613400	3,00	0,40-1,00 1,00-2,15	-	2,15
C - 22			3,50	1,00-2,35	0,60-0,90	2,35
C - 23	9895874	627354	3,50	2,00-2,80	1,20-1,50	2,80
C - 24	9897090	634780	4,00	2,50-4,00	1,50-1,80	no
TOTAL						
15	-	-	50,00	25	3	-



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The excavation of the test pits was performed manually, using conventional tools such as picks, shovels, buckers, etc. with dimensions of 2.0 x 2.0 m and at the maximum depth possible for excavation whether due to ground water as in C-11 or bedrock within which in some cases we excavated more than 1 meter due to completely weathered and soft mudstone.

In addition to the geological-geotechnical logs, we measured the resistance of undrained cuts every 20 cm at cohesive levels, through the use of the wind vane and the hand penetrometer although at most of the test pits the soil resistance was greater than the measuring capacity of the instruments. This information is included in the description of the consistency of the materials.

The levels and types of samples obtained were selected according to JICA. At each one of the test pits at the Open Channel two disturbed samples were taken of approximately 50 kg each of the materials considered most fitting for the purposes intended. At the test pits for the Transmission Line we could take only one undisturbed sample (0.3 x 0.3 x 0.3 m) from each test pit and one disturbed sample of the soils which could not be molded.

At test pits C-20 and C-21 we could not obtain undisturbed samples because of the characteristics of the materials.

We also obtained proper topographical documentation of each of the excavations as shown in Appendix No. 7.3.

For the geological-geotechnical classification of the soils, we used the criteria listed in Table No. 1 of Appendix No. 7.1 in which the character of the soils, color, consistency or compactness and organic material content were determined.

At the excavations where bedrock was found we evaluated geotechnical parameters such as weathering, fracturing, resistance, RQD as per the parameters listed in Appendix No. 7.1.

The geological and geotechnical logs for each of the test pits with the results of the laboratory tests are listed in Appendix No. 7.1.

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5.2.2 Manual Sounding with Standard Penetration Tests

Sixty (60) manual soundings were performed with continuation standard penetration tests (SPT) with 54 performed along the line of the Open Channel, three along the Transmission Line and three in the Poza Honda sector.

The number and depth of the sounds is shown in the table herebelow:



SITE: OPEN CHANNEL

SOUNDING NO	COORDINATE		DEPTH m	NUMBER OF MADE "SPT" TEST	ROCK LINE	N ₃₀ MAX	WATER LEVEL
	N	E					
S - 1	9892430	607520	4,05	9	2,95	44	-
S - 2	9892380	607370	3,60	8	1,95	47	-
S - 3	9892330	607370	3,60	8	1,60	40	-
S - 4	9892240	607100	2,20	4	1,00	57/25 R	-
S - 5	9892210	607010	3,15	7	1,40	50	-
S - 6	9892180	607030	7,50	6	1,50	58/25 R	-
S - 7	9892180	606840	4,95	11	3,95	21	-
S - 8	9892110	606865	4,95	11	4,25	31	-
S - 9	9891930	606620	3,60	8	1,60	37	2,25
S - 10	9891880	606650	3,15	7	1,30	42	-
S - 11	9891730	606440	4,50	10	3,00	47	-
S - 12	9891745	606410	4,05	9	2,80	29	-
S - 13	9891550	606440	3,15	7	2,20	35	-
S - 14	9891320	606485	4,05	9	3,40	55	1,50
S - 15	9891330	606460	2,00	5	1,60	30/10 R	-
S - 16	9891180	606320	2,40	6	1,70	30/10 R	-
S - 17	9891210	606310	2,50	6	1,60	30/10 R	-
S - 18	9891080	606140	4,95	11	3,80	29	-
S - 19	9890360	606130	3,50	8	3,00	30/10 R	-
S - 20	9890370	606090	3,60	8	2,60	31	-
S - 21	9890760	605950	3,90	9	2,60	30/15 R	-
S - 22	9890700	605900	2,90	7	1,60	30/10 R	-
S - 23	9890640	605830	1,20	3	0,25	30/10 R	-
S - 24	9890570	605770	0,80	3	0,30	30/10 R	-
S - 25	9890530	605730	3,10	7	2,00	54 R	-
S - 26	9890570	605830	3,15	7	1,60	49	-
S - 27	9890520	605630	3,15	7	1,20	63 R	-
S - 28	9890560	605420	2,70	6	1,20	51 R	-
S - 29	9890530	605435	3,60	8	2,50	43	-
S - 30	9890350	605280	4,30	10	2,80	30/10 R	-
S - 31	9890380	605240	2,10	5	1,60	30/10 R	-
S - 32	9890140	605240	1,60	4	1,50	30/10 R	1,00
S - 33	9890080	605090	3,00	7	2,50	30/15 R	-
S - 34	9889880	605000	2,80	7	1,70	30/10 R	-
S - 35	9889910	604960	1,50	4	0,70	30/ 5 R	-



SITE: OPEN CHANNEL

SOUNDING NO	COORDINATE		DEPTH ■	NUMBER OF MADE "SPT" TEST	ROCK LINE	N30 MAX	WATER LEVEL
	N	E					
S - 36	9889840	604790	2,70	6	1,20	34	-
S - 37	9889880	604760	3,15	7	1,20	40	-
S - 38	9889700	604670	3,15	7	1,20	48	-
S - 39	9889675	604660	1,00	3	0,90	30/10 R	-
S - 40	9889650	604630	2,70	6	1,40	44	-
S - 41	9889430	604520	1,10	3	1,00	30/10 R	-
S - 42	9889415	604540	0,95	3	0,90	30/ 5 R	-
S - 43	9889400	604560	3,15	7	1,80	38	-
S - 44	9889360	604365	4,95	11	3,20	31	-
S - 45	9889320	604370	4,20	10	3,40	30/15 R	-
S - 46	9889270	604280	4,50	10	3,80	27	-
S - 47	9889270	604240	4,95	11	3,40	35	-
S - 48	9889160	604260	2,25	5	0,90	48	-
S - 49	9889050	604190	2,25	5	1,00	36	-
S - 50	9889030	604090	1,20	3	1,10	30/10 R	-
S - 51	9889010	604010	2,70	6	1,20	42	-
S - 52	9888930	603975	4,05	9	3,00	47	-
S - 53	9888960	603950	3,20	7	2,90	30/10 R	-
S - 54	9888810	603810	4,05	9	3,00	51	-

SITE: TRANSMISSION LINE

SOUNDING NO	COORDINATE		DEPTH ■	NUMBER OF MADE "SPT" TEST	ROCK LINE	N30 MAX	WATER LEVEL
	N	E					
S - 55	9892320	608650	2,25	5	1,50	29	-
S - 56	9892380	611650	2,25	5	1,30	42	-
S - 57	9895885	627480	0,40	1	0,30	30/10 R	-
S - 57'	-	-	5,10	3	2,90	30/10 R	-

SITE: POZA HONDA

SOUNDING NO	COORDINATE		DEPTH ■	NUMBER OF MADE "SPT" TEST	ROCK LINE	N30 MAX	WATER LEVEL
	N	E					
S - 58	-	-	7,20	16	4,70	28	1,20
S - 59	-	-	4,00	8	3,00	17	2,20
S - 60	-	-	2,25	5	0,80	27	-



The manual soundings were performed with continuous standard penetration test, each 0,45 m.

Standard equipment was used for the execution of same, i.e.:

Weight of the mass:	140 lbs.
Height of the fall of the mass:	76 cm.
Diameter of sampler:	2"

At the sites where it was possible to install a headframe, when the slope was not strong, to expedite the work, a tripod or a cane tower.

Standard penetration tests were performed continuously every 45 cm using the standards for said test, i.e., the line is divided into 15 cm sections and the number of strokes necessary to drive each 15 cm section is counted. When more than 30 strokes are necessary to drive a 15 cm section or when the total strokes necessary to drive the last 30 cm reaches more than 50 strokes, then it is considered a reject.

For the interpretation of the test, we take the strokes necessary to drive the last 30 cm and obtaining the amount of N_{30} . The strokes from the top 15 cm were not used because the land could have been altered.

Samples obtained from each test with the split tube 2" in diameter have been logged with the geotechnical criteria listed in Table No. 1 of Appendix 7.1 for soils and with the geotechnical classification standards for the case of rocks. The samples have been preserved in double plastic bags, duly labeled in case it is necessary to perform laboratory tests or geotechnical logging later on.

As may be observed in the geotechnical logs of the soundings, they were performed on colluvial soils and completely weathered rock down to the depth in which rejection takes place or where it is normally possible to perform such soundings. It is important to point out that all the soundings performed detected the contact of the colluvial soils with altered or healthy rock, penetrating in certain cases more than 2 cm in completely weathered rock.



Also, we took photographic documentation of the way the soundings were performed as shown in Appendix No. 7.3.

5.3 RESULTS OBTAINED

We shall make a brief analysis of the results obtained from the field investigations for each of the sectors in the study.

5.3.1 Open Channel

Along the line of the Open Channel 10 test pits and 54 manual soundings were performed which when added to the investigations performed in the Feasibility Phase provide important documented information on the geotechnical characteristics of the soils and surface rock.

Location of the test pits and manual soundings are shown in Appendix No. 7.4.

Detailed logs of each of the test pits and each sounding are found in Appendix No. 7.1.

Excavations of the test pits were for the purpose of determining the properties of the colluvial soils for their use as building materials for the walls of the channel as well as to learn the strength of said soils for determining the exploitation volumes.

From the analysis of the bedding profiles detected we could determine the following bedding pattern:

- A black soil vegetation covering made up of black silty clays with indices of organic material (roots) with a consistency that is quite hard, in its dry state when excavations took place. Strength varies from 0.2 m to 0.50 m with a predominant strength of 0.35 m.
- Under the vegetable covering is a level of silty clays with gray or grayish brown coloring with approximately 20%-30% weathered



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debris with a consistency that is quite hard and hard, found at a depth from 0.20 m to 0.90 m with a median strength of 0.70 m.

Under the level of clays we found a stronger colluvial layer consisting completely of debris or highly weathered mudstone or siltstone, encased in a clayey-silty matrix with a brown-yellow coloration and a consistency which is quite hard. Within this layer we may observe the predominance of the clayey-silty matrix. The depth of the layer is from 0.90 m to 3.60 m with a strength varying from 1.00 m to 3.00 m.

The soils described constitute the colluvial layer which based on its characteristics are the product of on-site decay of the rock, whether mudstone or siltstone. Therefore, it is difficult to establish a clear contact of colluvial soils with the completely weathered rock since a transition occurs as a product of the degree of weathering of the rock.

Under the colluvial soils defined based on the criteria indicated, we find brown and yellow mudstone which is completely weathered and in many cases looks like weathered blocks or debris. But based on criteria for texture and bedding, we can define it as rock with a V quality index (very bad) starting from a V resistance index (no more than 20 kg/cm²), V fracturing grade, V weathering and RQD = 0.

The mudstone or siltstone detected in the excavations belong to the onzole formation.

We should indicate that the consistence of the colluvial soils or the resistance of the weathered rock changes quite a bit with water, such as the cases of test pit C-11 in which the water level is 1.60 m deep, decreasing the consistency of the debris layer to a clayey matrix which is quite hard to somewhat consistent (> 4.5 kg/cm² to 1 kg/cm²), determined with a penetrometer and hand torvane.

Manual soundings were for the purpose of determining the resistance of the soil for the purposes of foundations along the section of the Open Channel.

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All the soundings made contact with soil-rock and even some got more than 2 meters deep in the weathered rock.

Based on the analysis of same, we have determined the following: 7 soundings detected contact at a depth of less than 1 m; 27 soundings detected contact between 1.0 m and 2.0 m deep; 13 soundings determined it between 2.0 m and 5.0 m deep; 6 soundings determined it between 3.0 m and 4.0 deep; 1 sounding determined it at a depth of more than 4.0 m.

The depths reached by the soundings vary between 0.80 m and 4.95 m depending on the depth that detects the presence of rock and its characteristics since at the sites where the colluvial strength was higher, we managed to reach a greater depth since the strength of the completely weathered rock was greater. While at the sites, especially at the bottom of steams where the colluvial covering has been washed, their strength is less or does not exit. Likewise, the weathered rock has less strength or has not been washed paving the way for a rock with better geotechnical characteristics.

Regarding the bedding profiles detected, they were the same as those found in the test pits with the following sequence: One layer of vegetable soil with a strength between 0.20 m and 0.50 m, followed by a silty clay layer with debris which in some soundings is not detected and a layer of weathered debris with a clayey-silty matrix followed by completely weathered rock constituted by mudstone with a V quality index.

In some cases, where soundings were at the lower sections of streams with elevations of less than 100 meters about sea level, the sequence varies, passing from a small vegetable layer measuring a few centimeters to sandstone rock with a IV quality index.

The geotechnical logs for each sounding with its values of N_{30} are shown in Appendix No. 7.1.



5.3.2 Transmission Line

Five test pits and 3 manual soundings were performed along the Transmission Line. Said investigations were for the purpose of determining the properties of the surface soils or rocks for the purposes of the foundations of the Towers.

The location of the test pits and manual soundings are shown in Appendix No. 7.4.

Because of the distance between each test pit and sounding and the considerable differences in the elevations of the sites, we have obtained different results. Therefore we will analyze each cut obtained separately.

TEST PIT C - 20

From 0,00 to 0,35 m Hard brackish vegetal soil

From 0,35 to 1,10 m Weathered debris of mudstone in dark brown clayey matrix, hard.

From 1,10 to 1,50 m Completely weathered light gray mudstone with oxidated joints, soft rock.

We could not obtain an undisturbed sample from the debris level and, therefore, took a disturbed sample in order to perform the tests on the remolded samples.

TEST PIT C - 21

From 0,00 to 0,40 m Dark brown vegetal soil very stiff

From 0,40 to 1,10 m Weathered debris and blocks of siltstone and fine grained sandstone in yellowish brown silty sand matrix, very stiff.

From 1,10 to 2,20 m Completely weathered yellowish brown silty fine grained sandstone with oxidated joints, very soft rock, quality index V.



From 2,20 to 3,00 m Very weathered yellowish brown silty fine grained sandstone with a quality index IV.

As at test pit C-20 we could not obtain an undisturbed sample from the debris level, taking a sample of a disturbed sample for the tests on the remolded sample.

TEST PIT C - 22

From 0,00 to 0,30 m Blackish vegetal soil, hard.

From 0,30 to 0,95 m Grayish brown silty clay with some weathered debris, stiff.

From 0,95 to 2,35 m Weathered debris of siltstone and fine grained sandstone in brown silty matrix, stiff.

From 2,35 to 3,50 m Completely weathered brown mudstone with oxidated zone, quality index V (very soft rock).

Soundings S-55 and S-56 showed quite similar results with soil-rock contact at 1.50 and 1.30 m respectively and a lithological makeup constituted by hard and sandstone colluvial soils which are finely weathered.

Toward the site of test pits C-23 and C-24 and sounding S-57 the make up of the colluvial soils varies and are similar to one another. They are constituted by a vegetable soil level 0.35 m and 0.40 m thick.

There is a debris level in a clayey-silty matrix up to 0.80 m and a strong layer of grayish brown clay which is quite hard to 2.80 m at test pit C-23 and to 2.50 m at test pit C-24. This clay level takes a reddish coloration towards the base.

At test pit C-23, under the clays, we find sandstone with a very fine grain that is highly weathered with a V quality index while at C-24 from 2.50 m we find a grayish brown mud with some fine sand that is consistent. At this test pit we did not have soil-rock contact.



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The cut detected in sounding S-57' is equal to that at test pit C-23.

5.3.3 Poza Honda

Three manual soundings were performed with different results.

- Sounding S-58 which is located at Estero Guajabito, reached a depth of 7.20 m and detecting an alluvial layer made up of fine silty sand, alternating with levels of soft silt up to 4.70 m and later detecting a completely weathered level of sandstone up to 5.20 m deep (possible block) and from 5.20 m to 7.20 completely weathered brown and yellow mudstone.
- At Test Pit S-59, located on a slope at the exit of Estero Guajabito, we detected a colluvial layer with a hard consistency up to 3.0 m deep and under the colluvial level a completely weathered brown and yellow mudstone with a V quality index appears.
- Sounding S-60 was performed at the exit of the Pata de Pajaro River, on a slope and a colluvial layer up to 0.80 m deep was detected. Under the colluvial layer from 0.80 to 1.80 m completely weathered mudstone with a V index is detected and then a completely weathered sandstone with very fine silt with a quality V index.

6. LABORATORY SOIL TESTS

6.1 PURPOSE AND SCOPE OF THE WORK

The laboratory soil tests were focused on two fundamental objectives:

1. Determination of the basic geomechanical characteristics of the soils for their use as materials in the Channel earth fill.

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2. Determination of the geomechanical properties of the soils for the study of the foundation along the Transmission Line.

Based on these objectives, altered samples were taken of the soils and 10 test pits were excavated along the Open Channel and unaltered and altered samples from the test pits excavated along the Transmission Line.

6.2 TESTS PERFORMED

Nineteen altered tests of approximately 50 kg each were sent to the laboratory in order to study the materials for the Open Channel, and altered samples and 3 unaltered samples in a block for the study of the soils of the Transmission Line.

The number and type of tests performed is shown in the table below:



TIPO DE ENSAYO TYPE OF TEST	CANAL ABIERTO OPEN CHANNEL	LINEA DE TRANSMISION TRANSMISSION LINE	TOTAL
1. Contenido de Humedad Natural Natural Moisture content	19	9	28
2. Gravedad Especifica Specific Gravity	19	9	28
3. Granulometria (tamiz e hidrómetro) Grain size analysis (sieve and hydrometer)	19	9	28
4. Límites de Atterberg Atterberg Limits	19	9	28
5. Peso Unitario Unit Weight	10	5	15
6. Compresión Uniaxial Uniaxial Compression	10	5	15
7. Triaxial Triaxial Test	10	5	15
8. Consolidación Consolidation	10	5	15
9. Compactación Proctor Proctor Compaction	15	2	17
10. Hinchamiento Swelling	19	9	28
11. Encogimiento Shrinkage	19	9	28
12. Dispersividad Pin Hole Pin Hole Test for Dispersion	19	9	28



From the 19 samples from the Open Channel, 10 were selected for performing all the tests provided as the most promising layer for construction materials. Five were selected for performing tests as an alternating stratum including the Proctor test. On the four remaining ones only identification, swelling, shrinking and Pin Hole tests were performed.

For the Transmission Line, we managed to obtain only three unaltered samples from three different test pits. Therefore, we took altered samples from those test pits where it was impossible to obtain unaltered blocks for performing the tests provided on the remolded samples.

Therefore, from the 9 samples taken for the study of the Transmission Line, all the tests including 2 Proctor tests were performed and on the four remaining samples only the identification, swelling, shrinking and Pin Hole tests were performed.

All the Soil Mechanics Tests were performed at the HIDROSUELOS CIA. LTDA. laboratories except for the three block samples which were performed at the National Polytechnic School laboratories, complying with the standards set forth for each test.

6.3 RESULTS OBTAINED

A table summarizing the results obtained for each of the tests performed is shown herebelow. The complete tests are enclosed in ANNEX - VOLUME II.

SECTOR LOCATION	CALICATA N° TEST PIT N°	MUESTRA N° SAMPLE N°	PROFUNDIDAD DEPTH	HUMEDAD NATURAL NATURAL MOISTURE			GRAVEDAD ESPECIFICA SPECIFIC GRAVITY			GRANULOMETRIA (TAMIZ E. HIDROMETRO) GRAIN SIZE ANALYSIS (SIEVE AND HYDROMETER)			LIMITE DE ATTERBERG ATTERBERG LIMITS			CLASIFICACION CLASIFICATION	PESO UNITARIO UNIT WEIGHT			COMPRESION UNIAIXIAL UNIAIXIAL COMPRESSION	ENSAYO TRIAXIAL TRIAIXIAL TEST			CONSOLIDACION CONSOLIDATION			COMPACTACION PROCTOR PROCTOR COMPACTION			HINCHAMIENTO SWELLING			ENCOGIMIENTO SHRINKAGE			PIN - HOLE PIN - HOLE	OBSERVACIONES REMARKS
				W %	C-s g/cm3	#4 %	#200 %	2/4 %	WL %	WP %	IP %	SUGS USCS	HUM g/m3	SATUR m3	qu kg/cm2		C' kg/cm2	σ' kg	Pp kg/cm2		Cc	Cv mm2/min	γmax g/cm3	Wopt %	P kg/cm2	V %	Ws %	Ls %	Vc %								
CANAL ABIERTO OPEN CHANNEL	C-10	M-1	0.50-1.40	31.2	2.768	100	96.0	21	59.0	37.0	22.0	MH	1.54	1.683	1.82	0.90	20°	0.55	0.125	4.046	1.07	44.4	0.20	0.00		16.53	42.5	ND1	DISTURBADA / DISTURBED								
		M-2	2.00-3.00	34.3	2.727	100	98.7	18	55.0	37.0	18.0	MH	1.538	-	-	-	-	-	-	-	-	-	0.40	0.00		12.98	47.88	ND2									
	C-11	M-1	0.55-2.00	37.7	2.606	100	86.9	20	80	54	26	MH	1.558	1.659	1.70	0.70	16°	0.46	0.136	2.62	1.07	45.6	1.25	0.00		15.80	42.09	ND1	" "								
		M-2	2.00-3.60	41.6	2.663	100	89	16	71	48	23	MH	1.54	1.684	2.18	0.85	20°	1.05	0.126	4.752	1.09	42.60	1.60	0.00		25.19	35.15	ND2									
	C-12	M-1	0.60-1.60	32.2	2.679	100	90	14	61	43	18	MH	1.58	1.708	-	-	-	-	-	-	1.13	41.60	1.60	0.00		25.19	35.15	ND2	" "								
		M-2	2.00-3.60	41.6	2.663	100	89	16	71	48	23	MH	1.54	1.684	2.18	0.85	20°	1.05	0.126	4.752	1.09	42.60	1.60	0.00		21.53	41.19	ND2									
	C-13	M-1	0.80-2.00	35.7	2.743	100	97	53	92	47	45	MH	1.566	1.750	2.53	1.00	26°	0.95	0.16	1.92	1.18	36.30	1.35	0.00		21.12	57.33	ND1	" "								
		M-2	2.00-3.50	33.3	2.622	100	99	8.0	78	42	36	MH	-	-	-	-	-	-	-	-	-	-	-	-	-	21.99	40.11	ND1									
	C-14	M-1	0.90-1.90	31.9	2.598	100	76	19	78	43	35	MH	1.518	1.714	-	-	-	-	-	-	1.16	37.8	0.60	0.00		32.95	28.24	ND1	" "								
		M-2	1.90-3.50	39.7	2.565	100	94	18	78	44	34	MH	1.523	1.708	1.01	1.60	14°	0.72	0.145	5.494	1.16	40.2	0.10	0.00		31.07	31.17	ND1									
C-15	M-1	0.40-1.00	27.5	2.658	100	76	48	91	39	52	CH	1.399	1.661	1.09	0.70	22°	0.80	0.439	1.531	1.06	34.6	1.50	0.00		14.75	57.08	ND1	" "									
	M-2	1.20-1.60	32.4	2.698	100	98	18	90	44	46	MH	1.400	-	-	-	-	-	-	-	-	-	1.60	0.00		12.33	57.74	ND1										
C-16	M-1	0.85-1.80	30.1	2.636	100	78	28	62	36	26	MH	1.627	1.794	-	-	-	-	-	-	1.28	32.8	0.75	0.00		17.39	38.72	ND1	" "									
	M-2	2.00-3.50	32.7	2.456	100	95	30	63	34	29	MH	1.642	1.759	1.88	0.70	12°	0.75	0.107	13.91	1.28	31.8	1.65	0.00		15.40	41.82	ND1										
C-17	M-1	0.40-1.60	23.3	2.623	100	95	30	71	41	30	MH	1.498	1.718	1.66	1.50	11°	0.80	0.243	10.58	1.16	32.3	1.15	0.00		16.02	44.96	ND1	" "									
	M-2	2.00-3.00	28.6	2.680	100	99	43	73	44	29	MH	1.496	-	-	-	-	-	-	-	-	-	1.10	0.00		18.35	42.47	ND2										
C-18	M-1	0.30-1.60	21.7	2.726	100	95	24	65	35	30	MH	1.528	1.753	2.40	1.20	22°	0.70	0.335	6.224	1.19	34.6	1.30	0.00		18.28	46.14	ND1	" "									
	M-2	2.00-3.50	25.5	2.715	100	95	28	64	36	28	MH	1.527	1.707	-	-	-	-	-	-	1.12	41.6	1.50	0.00		11.55	53.59	ND2										
C-19	M-1	1.00-1.70	24.3	2.698	100	99	30	45	22	23	CL	1.423	1.831	-	-	-	-	-	-	1.32	29.6	0.30	0.00		13.83	39.02	ND1	" "									
	M-2	1.70-3.60	23.5	2.607	100	58	20	40	21	19	CL	1.423	1.857	2.56	1.65	10°	0.65	0.098	2.443	1.39	30.2	0.30	0.00		18.28	31.00	ND1										
LINEA DE TRANSMISION TRANSMISION LINE	C-20	M-1	0.35-1.10	25.7	2.649	100	80	39	49	31	18	ML	1.541	1.728	3.00	1.80	10°	0.75	0.106	7.81	1.17	37.5	0.15	0.00		19.36	40.73	ND1	" "								
	C-21	M-1	0.40-1.00	18.7	2.673	100	50	16	44	28	16	ML	1.668	1.826	0.83	0.40	20°	0.74	0.134	6.18	1.32	31.00	1.10	0.00		19.99	26.61	ND1	" "								
		M-2	1.00-2.15	26.7	2.579	100	88	8	40	22	18	CL	1.670	-	-	-	-	-	-	-	-	-	0.26	0.00		19.17	25.00	ND1									
	C-22	M-1	0.60-0.90	39.64	2.832	100	-	50	106	58	48	CH	1.673	1.750	13.18	1.10	26°	0.40	0.18	2.597	1.16	38.00	2.49	0.00		18.09	42.25	ND1	INDISTURBADA / UNDISTURBED DISTURBADA / DISTURBED								
		M-2	1.00-2.35	27.3	2.658	100	49	15	60	29	31	SC	1.660	-	-	-	-	-	-	-	-	-	1.25	0.00		19.89	39.94	ND1									
C-23	M-1	1.20-1.50	44.03	2.878	-	-	65	114	51	63	CH	1.719	1.757	8.38	2.60	14°	0.60	0.18	2.575	1.16	46.00	1.36	0.00		27.26	44.25	ND1	INDISTURBADA / UNDISTURBED DISTURBADA / DISTURBED									
	M-2	2.00-2.80	37.7	2.733	100	98	50	74	30	44	CH	1.67	-	-	-	-	-	-	-	-	-	0.75	0.00		20.27	49.19	ND1										
C-24	M-1	1.50-1.80	41.57	2.803	-	-	53	87	44	43	CH	1.655	1.843	1.97	0.80	14°	0.5	0.15	2.404	1.31	36.00	0.51	0.00		24.87	44.00	ND1	INDISTURBADA / UNDISTURBED DISTURBADA / DISTURBED									
	M-2	2.50-4.00	57.7	2.681	100	90	33	64	32	32	MH	1.66	-	-	-	-	-	-	-	-	-	0.76	0.00		16.36	44.57	ND2										

PESO UNITARIO TOMADO EN CONDICIONES DE COMPACTACION / UNIT WEIGHT TAKEN IN COMPACTION CONDITIONS
 Cv. TOMADO ENTRE 4.00 Y 5.00 Kg/cm² / Cv. TAKEN BETWEEN 4.00 AND 5.00 Kg/cm² CONSOLIDATION



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7. APPENDIXES

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7.1 LOGGING OF TEST PIT AND SOUNDING



RULES FOLLOWED FOR THE GEOTECHNICAL DESCRIPTION OF THE TEST PITS AND SOUNDING

The figure enclosed shown the distribution of the log of the geotechnical data obtained from test pits and sounding as per the type columns.

TEST PIT


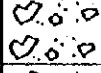

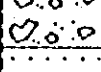
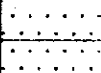
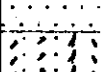
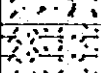
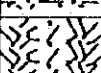
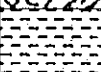

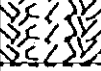

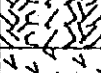
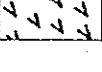

1. Project name
2. Test pit number
3. Execution date
4. Site and coordinate
5. Depth
6. Description of the type of soil or rock
The characterization of the soils was performed based on the laboratory tests as per the modified Casagrande Classification, standard ASTM-2487 and in based on the visual-manual description, as per ASTM-2488. The main criteria are summarized in table № 1.
The rock characterization was carried out according to the International Society of Rock Mechanics (Basic Geotechnical Description of Rock Masses)
7. Graphic representation of the type of land. Graph № 1
8. Result of the Laboratory Test
9. Type of sample and depth

SOUNDING

1. Project name
2. Sounding number
3. Coordinate
4. Execution date
5. Depth
6. Formation name
7. Soil and Rock type
8. Graphic representation of the type of land. Graph № 1
9. Description of the type of soil or rock
10. Soil consistency or rock grade (Bieniawsky)
11. Graphic representation of "SPT"

PROYECTO : DISEÑO TRASVASES CHONE-PORTOVIJEJO ① PROYECTO : THE DETAILING DESIGN ON TRANSBASIN CHONE-PORTOVIJEJO ①						SONDEO : ② SOUNDING No:				
COORDENADAS : N E ③ COORDINATE :						FECHA : ④ DATE :				
PROF. DEPTH	FORMA- CION FORMA- TION	TIPO DE SUELO o ROCA SOIL OR ROCK TYPE	PERFIL COLUMN SECTION	DESCRIPCION DESCRIPTION	CALIDAD DE SUELO o ROCA SOIL OR ROCK GRADE	SPT (N30)				
						0	10	20	30	40
1.0	⑤	⑥	⑦	⑧	⑩	⑪				
2.0										
3.0										
4.0										

CRITERION FOR SOIL DESCRIPTION

<p>COARSE GRAINED SOILS</p> <p>More than half of material is larger than No. 200 sieve size</p>	<p>GRAVELS</p> <p>More than half of coarse traction is larger than No. 4 sieve size</p>	<p>CLEAN GRAVELS</p> 	GW	Well graded gravels sand mixtures little or no fines	
		<p>FINES >5%</p> 	GP	Poorly graded gravels, gravel-sand mixtures, little or no fines	
		<p>GRAVELS WITH FINES</p> 	GM	Silty gravels, poorly graded gravel-sand-silt mixtures	
	<p>SANDS</p> <p>More than half of coarse traction is smaller than No. 4 sieve size</p>	<p>FINES >12%</p> 	GC	Clayey gravels, poorly graded gravel-sand-clay mixture	
		<p>CLEAN SANDS</p> 	SW	Well graded sands, gravelly sands, little or no fines	
		<p>FINES <5%</p> 	SP	Poorly graded sands, gravelly sands, little or no fines	
		<p>SANDS WITH FINES</p> <p>>12%</p> 	SM	Silty sands, poorly graded sand silt mixtures	
				SC	Clayey sands, poorly graded sand-clay-mixtures
					ML
	CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays			
				OL	Organic silty and organic silt-clays of low plasticity
	MH	Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts			
				CH	Inorganic clays or high plasticity, fat clays
	OH	Organic clays of medium to high plasticity			
			<p>HIGHLY ORGANIC SOILS</p> 	PT	Peat and other highly organic soils

PLASTICITY CHART

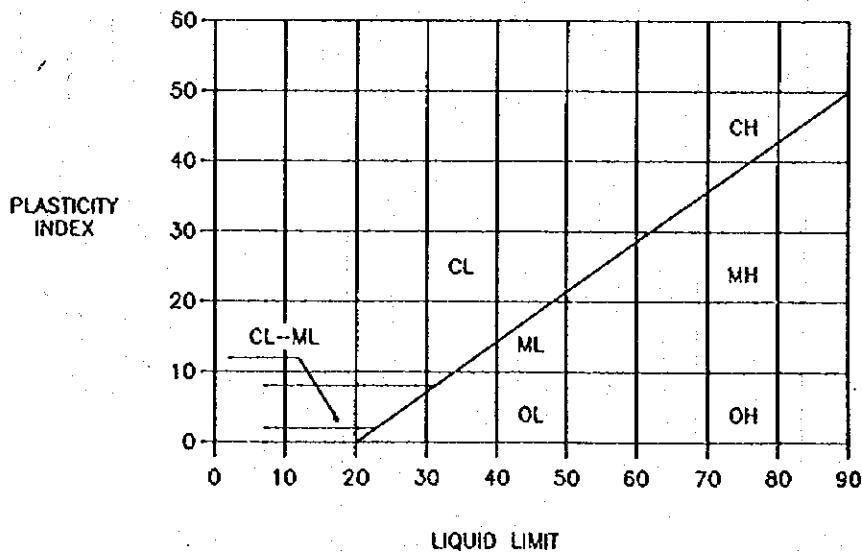


TABLE NO 1

DESCRIPTION FOR PARTICLE SIZE CONTENT

3/4"		No.4	No.10	No.40	No.200	2LL	SIEVE NUM ASTM
GRAVEL		SAND			FINES		
COARSE	FINE	COARSE	MEDIUM	FINE	SILTS	CLAYS	
100	20.0	4.7	2.0	0.5	0.74	0.002	SIEVE SIZE

RATE

DESCRIPTION TERM

5 - 10	TRACES
10 - 20	SOME
20 - 35	LARGE AMOUNT OF
35 - 50	CLAYEY, SILTY, ETC.

DESCRIPTION OF COMPRESSIBILITY TERMS FOR A GRANULAR SOIL

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DESCRIPTION TERM

0 TO 4	VERY LOOSE
5 TO 10	LOOSE
11 TO 30	COMPACT DENSE
31 TO 50	DENSE
>50	VERY DENSE

DESCRIPTION OF CONSISTENCY TERMS FOR A COHESIVE SOIL

<u>q_v (kPa)</u>	<u>N₃₀</u>	<u>DESCRIPTION TERM</u>	
<25	<2	VERY SOFT	
25 TO 50	2 to 4	SOFT	
50 TO 100	4 to 8	FIRM	1 kg/cm ² = 100 k Pa
100 TO 200	8 to 10	STIFF	q _v = UNIAXIAL COMPRESSION
200 TO 400	15 to 30	VERY STIFF	STRENGTH
>400	>30	HARD	



TABLE NO 2

ROCK'S WEATHERING

W1 Sound Rock

- a) Without visible meteorization
- b) The joints are slightly oxidated

W2 Slightly Weathered Rock

Weathering penetrates slightly into the rock thru main joints

W-3 Very Weathered Rock

- a) Weathering follows thru all the rock's body, but the rock is strong enough not to be broken by hand.
- b) Less than half of the rock's body is desintegrated and/or decomposed into soil. Some parts of the rock's body is healthy rock or slightly decolorized, as a whole, or as contiguous blocks.

W4 Highly Weathered Rock

- a) Weathering thru the whole rock's body. The rock can be easily desintegrated by hand.
- b) More than half of the rock's body is desintegrated and/or decomposed into soil. The rock's body could have healthy blocks or slightly decolorized blocks.

W5 Completely Weathered Rock

All the rock's body is desintegrated or decomposed into soil, being possible to recognize its original structure and rock type.



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TABLE Nº 3

VISUAL RESISTANCE GRADES OF ROCK'S MATRIX

S1 Very Stiff Rock

- Hard to break ($q_u > 2000 \text{ kg/cm}^2$)
- Very crisp sound when hit by hammer's head
- $q_u > 2000 \text{ kg/cm}^2$

S2 Stiff Rock

- Scratchable and breakable with hammer's head
- Crisp sound when hit by hammer's head
- $600 < q_u < 2000 \text{ kg/cm}^2$

S3 Medium Rock

- Easy to scratch with hammer's head
- Uncrisp sound when hit by hammer's head
- $200 < q_u < 600 \text{ kg/cm}^2$

S4 Soft Rock

- Easily to scratch with hammer
- Impossible to break by hand in blocks greater than 10 cm^2
- $60 < q_u < 200 \text{ kg/cm}^2$

S5 Very soft Rock

- Hammer's head corner hit leaves a mark on the rock
- Breakable by hand
- $20 < q_u < 60 \text{ kg/cm}^2$

Transition between rock and soil: $4 < q_u < 20 \text{ kg/cm}^2$

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TABLE NO 4

STRATA THICKNESS ACCORDING TO B.G.D.'S SPECIFICATIONS

INTERVALS cm	SYMBOL	DESCRIPTION
> 200	F 1	Very thick
60 - 200	F 2	Thick
20 - 60	F 3	Medium
6 - 20	F 4	Thin
< 6	F 5	Very thin

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Table 5 : The Rock Mass Rating System (after Bieniawski, 1979).

A. CLASSIFICATION PARAMETERS AND THEIR RATINGS

Parameter		Ranges of Values							
1	Strength of intact rock material	Point load strength index (NPa)	10	4-10	2-4	1-2	For this low range uniaxial compressive test is preferred		
	Uniaxial compressive strength (MPa)	>250	100-250	40-100	25-50	5-25	1-5	-1	
	Rating	15	12	7	4	2	1	0	
2	Drill core quality RQD %*	90-100	75-90	50-75	25-50	<25			
	Rating	20	17	13	8	3			
3	Spacing of discontinuities	>2 m	0.6-2 m	200-600 mm	60-200 mm	<60 mm			
	Rating	20	15	10	8	5			
4	Condition of discontinuities	Very rough surfaces Not continuous No separation Unweathered wall rock	Slightly rough surfaces Separation < 1 mm Slightly weathered walls	Slightly rough surfaces Separation < 1 mm Highly weathered walls	Slickensided surfaces or Gouge < 5 mm thick or Separation 1-5 mm Continuous	Soft gouge > 5 mm thick or Separation > 5 mm Continuous			
		Rating	30	25	20	10	0		
5	Groundwater	Inflow per 10 m tunnel length (L/min)	None	<10	10-25	25-125	>125		
		Joint water pressure Ratio Major principal stress	0	<0.1	0.1-0.2	0.2-0.5	>0.5		
	General conditions	Completely dry	Damp	Wet	Dripping	Flowing			
	Rating	15	10	7	4	0			

B. RATING ADJUSTMENT FOR DISCONTINUITY ORIENTATIONS

Strike and Dip Orientations of Discontinuities		Very Favorable	Favorable	Fair	Unfavorable	Very Unfavorable
Ratings	Tunnels and mines	0	-2	-5	-10	-12
	Foundations	0	-2	-7	-15	-25
	Slopes	0	-5	-25	-50	-60

C. ROCK MASS CLASSES DETERMINED FROM TOTAL RATINGS

Rating	100-81	80-61	60-41	40-21	<20
Class no.	I	II	III	IV	V
Description	Very good rock	Good rock	Fair rock	Poor rock	Very poor rock

D. MEANING OF ROCK MASS CLASSES

Class no.	I	II	III	IV	V
Average stand-up time	20 yr for 15 m span	1 yr for 10 m span	6 wk for 5 m span	10 h for 2.5 m span	30 min for 1 m span
Cohesion of the rock mass (NPa)	>400	300-400	200-300	100-200	<100
Friction angle of the rock mass (deg)	>45	35-45	25-35	15-25	<15

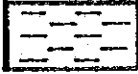
*After Bieniawski (1979).

2911

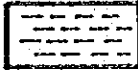
SYMBOLIC FOR EACH TYPE OF SOIL OR ROCK

GRAPHIC № 1

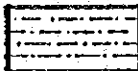
SOILS



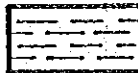
VEGETAL SOIL, ORGANIC SILT OR CLAY



SILT



SANDY SILT OR SILTY SAND

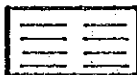


PLASTIC CLAYS

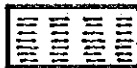


FINE GRAINED SAND WITH A SMALL CONTENT OF FINES

ROCKS



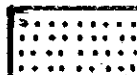
MUDSTONE



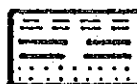
SILTSTONE



FINE GRAINED SANDSTONE



MEDIUM TO COARSE GRAINED SANDSTONE



ALTERNATING LAYERS OF SILTSTONE, MUDSTONE AND SANDSTONE



0

TEST PIT

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PROJECT : THE DETAILED DESIGN ON TRANSBASIN CHONE-PORTOVIEJO BASIN											TEST PIT N ^o C - 10					DATE : 20-12-93												
SITE: OPEN CHANNEL											COORDINATE : N 9892630					E 607645												
DEPTH m	DESCRIPTION	P R O F I L E	W %	S P E C I F I C G R A V I T Y	PARTICLE SIZE			LIMITES ATTERBERG			CLASIF UCCS	UNIT WEIGHT		TRIAxIAL		CONSOLIDATION			COMPACTION		SWELLING		SHRINKAGE			PIN HOLE	REMARKS	
					#4	#200	2/4	WL	WP	IP		MOIST	SATUR	C'	φ ^o	Pp	Cc	Cv	Y _{max}	OMC	P	V	Ws	Ls	Vc			
	BLACKISH VEGETAL SOIL, HARD DARK GRAY ORGANIC CLAY																									0.50		
1.0	WEATHERED DEBRIS OF SILTSTONE IN YELLOWISH BROWN SILT-CLAY MATRIX, HARD		31.2	2.768	100	96	21	59	37	22	MH	1.59	1.683	1.82	0.90	20	0.55	0.125	4.046	1.07	44.4	0.20	0.00		16.53	42.5	ND1	DISTURBED N 1
2.0	WEATHERED YELLOWISH BROWN MUDSTONE, VERY SOFT ROCK																									1.40		
3.0			34.3	2.727	100	98.7	18	55	37	18	MH	1.538	-	-	-	-	-	-	-	-	-	0.40	0.00	-	12.98	47.88	ND2	DISTURBED N 2
4.0	END : 3.0 m																									3.00		

287

PROJECT : THE DETAILED DESIGN ON TRANSBASIN CHONE- PORTOVIEJO BASIN										TEST PIT N ^o C - 11					DATE : 15-12-93													
SITE: OPEN CHANNEL										COORDINATE : N 9892440					E 607000													
DEPTH m	DESCRIPTION	PROFILE	W %	GRAVITY SPECIFIC	PARTICLE SIZE			LIMITES ATTERBERG			CLASIF UCCS	UNIT WEIGHT		MOCH V ₂₀ Kg/cm ²	TRIAIAL		CONSOLIDATION			COMPACTION		SWELLING		SHRINKAGE			PIN HOLE	REMARKS
					#4	#200	2/4	WL	WP	IP		MOIST	SATUR		C'	φ ^o	Pp	Cc	Cy	Y _{max}	OMC	P	V	Ws	La	Vc		
	BLACKISH VEGETAL SOIL, HARD																											
	GREYISH BROWN CLAYEY SILT WITH SOME OF WEATHERED CLASTICS, HARD																											
	WEATHERED CLASTICS OF SILTSTONE IN YELLOWISH BROWN CLAYEY-SILT MATRIX, HARD																										0.55	
1.0	CLAY WITH CLASTICS																											
	STIFF		37.7	2.606	100	86.9	20	80	54	26	MH	1.558	1.659	1.70	0.70	16	0.46	0.136	2.62	1.07	45.6	1.25	0.00		15.80	42.09	NDI	DISTURBED N° 1
2.0	END : 2.0 m																										2.00	
	END OF PIT BY WATER PRESENCE																											
3.0																												
4.0																												

280

PROJECT : THE DETAILED DESIGN ON TRANSBASIN CHONE-PORTOVIEJO BASIN										TEST PIT N ^o C - 12					DATE : 20-12-93													
SITE: OPEN CHANNEL										COORDINATE : N 9892440					E 606500													
DEPTH m	DESCRIPTION	ELECTROPHOR	W %	GRAVITY SPECIFIC	PARTICLE SIZE			LIMITES ATTERBERG			CLASIF UCCS	UNIT WEIGHT		F ₂₀₀ Kg/cm ²	TRIAxIAL		CONSOLIDATION			COMPACTION		SWELLING		SHRINKAGE			PIN HOLE	REMARKS
					#4	#200	2/4	WL	WP	IP		MOIST	SATUR		C [*] Kg/cm ²	φ ^o deg	P _p Kg/cm ²	C _c	C _v mm ² /min	Y _{max} gr/cm ²	OMC %	P Kg/cm ²	V %	W _s %	L _s %	V _c %		
	BLACKISH VEGETAL SOIL, HARD																											
	DARK GRAY CLAY WITH TRACES OF OXIDIZED DEBRIS AND ORGANIC MATERIAL, HARD																										0.60	
1.0	WEATHERED AND OXIDIZED DEBRIS OF SILT IN YELLOWISH BROWN SILTY-CLAY MATRIX, VERY STIFF		32.2	2.679	100	90	14	61	43	18	MH	1.58	1.708	-	-	-	-	-	-	1.13	41.60	1.60	0.00		25.19	35.15	ND2	DISTURBED N 1
																											1.60	
2.0	YELLOWISH BROWN CLAYEY SILT WITH NUMEROUS WEATHERED AND OXIDIZED DEBRIS, VERY STIFF																											2.00
3.0	STIFF		41.6	2.683	100	89	16	71	48	23	MH	1.54	2.18	0.85	20	1.05	0.126	4.752	1.09	42.60	1.60	0.00		21.53	41.19	ND2	DISTURBED N 2	
4.0	WEATHERED BLOCKS OF YELLOWISH BROWN SILTSTONE IN SILTY-CLAY MATRIX																											3.60

END : 4.0 m

289

PROJECT : THE DETAILED DESIGN ON TRANSBASIN CHONE- PORTOVIEJO BASIN										TEST PIT N ^o C - 13					DATE : 15-12-93															
SITE: OPEN CHANNEL										COORDINATE : N 9891540					E 606387															
DEPTH m	DESCRIPTION	PROFILE	W %	S P E C I F I C I T Y	PARTICLE SIZE			LIMITES ATTERBERG			CLASIF UCCS	UNIT WEIGHT		TRIAXIAL	CONSOLIDATION			COMPACTION		SWELLING		SHRINKAGE			PIN HOLE	REMARKS				
					#4	#200	2/1	WL	WP	IP		MOIST	SATUR		C'	σ ^o	Pp	Cc	Cv	Y _{max}	OMC	P	V	Ws			Ls	Vc		
	BLACKISH VEGETAL SOIL, HARD																													
	DARK GRAY CLAY WITH DEBRIS, HARD																													
	WEATHERED DEBRIS IN GRAYISH BROWN SILTY MATRIX, HARD																													
1.0	GRAYISH BROWN CLAY WITH TRACES OF OXIDIZED DEBRIS, HARD WITH SOME DEBRIS		35.7	2.743	100	97	53	92	47	45	MH	1.566	1.750	2.53	1.00	26	0.95	0.16	1.92	1.18	36.30	1.35	0.00				0.80			
2.0	YELLOWISH BROWN CLAY WITH NUMEROUS OXIDIZED DEBRIS, HARD																											2.00		
3.0	WEATHERED YELLOWISH BROWN VERY SOFT MUDSTONE		33.3	2.622	100	99	8	78	42	36	MH	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	21.99	40.11	NDI	
4.0																												3.50		

END : 4.0 m

300

PROJECT : THE DETAILED DESIGN ON TRANSBASIN CHONE- PORTOVIEJO BASIN										TEST PIT N ^o C-14					DATE : 20-12-93													
SITE: OPEN CHANNEL										COORDINATE : N 9891045 E 606183																		
DEPTH m	DESCRIPTION	PROFILE	W %	GRAVITY SPECIFIC	PARTICLE SIZE			LIMITES ATTERBERG			CLASIF UCCS	UNIT WEIGHT		TRIAXIAL	CONSOLIDATION			COMPACTION		SWELLING		SHRINKAGE			PIN HOLE	REMARKS		
					#4	#200	2/1	WL	WP	IP		MOIST	SATUR		C'	φ°	Pp	Cc	Cv	Y _{max}	OMC	P	V	Ws			Ls	Vc
	BLACKISH VEGETAL SOIL WITH OXIDIZED DEBRIS, HARD	Y-V																										
	BLACKISH GRAY CLAY WITH TRACES OF OXIDIZED DEBRIS AND ORGANIC MATERIAL, HARD	Y-V																										
1.0	YELLOWISH BROWN WEATHERED AND OXIDIZED DEBRIS IN DARK GRAY SILTY-CLAY MATRIX, HARD	Y-V	31.9	2.598	100	76	19	78	43	35	MH	1.518	1.714					1.16	37.8	0.60	0.00			32.95	28.24	NDI	DISTURBED N 1	
2.0	GRAYISH BROWN CLAY WITH NUMEROUS WEATHERED DEBRIS OF MUDSTONE, HARD	Y-V																										
3.0	BLOCKS OF SILTSTONE	Y-V																										
4.0	WEATHERED YELLOWISH BROWN MUDSTONE, VERY SOFT ROCK	Y-V																										

END : 4.0 m

PROJECT : THE DETAILED DESIGN ON TRANSBASIN CHONE- PORTOVIEJO BASIN										TEST PIT N ^o C-15					DATE : 20-12-93													
SITE: OPEN CHANNEL										COORDINATE : N 9890440 E 605090																		
DEPTH m	DESCRIPTION	PROFILE	W %	S P E C I F I C I T Y	G R A V I T Y	PARTICLE SIZE			LIMITES ATTERBERG			CLASIF UCCS	UNIT WEIGHT		TRIAxIAL	CONSOLIDATION			COMPACTION		SWELLING		SHRINKAGE			PIN HOLE	REMARKS	
						#4	#200	2/4	WL	WP	IP		MOIST	SATUR		C'	φ°	Pp	Cc	Cv	Y _{max}	OMC	P	V	Ws			Ls
	BLACKISH VEGETAL SOIL, VERY STIFF	Y																									0.40	
	YELLOWISH BROWN CLAY WITH SOME OF WEATHERED DEBRIS, VERY STIFF	Y																										
	LEVEL OF BLACKISH CLAY		27.5	2.658	100	76	48	91	39	52	CH	1399	1.661	1.09	0.70	22	0.80	0.439	1.531	1.06	34.6	1.50	0.00		14.75	57.08	NDI	1.00
1.0	LEVEL OF BLACKISH CLAY	Y																										
	WITH NUMEROUS DEBRIS, STIFF	Y																										
		Y	32.4	2.698	100	98	18	90	44	46	MH	1.400	-	-	-	-	-	-	-	-	-	1.60	0.00		12.33	57.74	NDI	1.20
		Y																										
2.0	WEATHERED YELLOWISH BROWN MUDSTONE, VERY SOFT ROCK	Y																									1.60	
3.0																												
	END : 3.0 m																											
4.0																												

PROJECT : THE DETAILED DESIGN ON TRANSBASIN CHONE-PORTOVIJEJO BASIN										TEST PIT N ^o C-16					DATE : 20-12-93													
SITE: OPEN CHANNEL										COORDINATE : N 9890110					E 605090													
DEPTH m	DESCRIPTION	POROSITY	W %	GRAVITY	PARTICLE SIZE			LIMITES ATTERBERG			CLASIF UCCS	UNIT WEIGHT		TRIAxIAL	CONSOLIDATION			COMPACTION		SWELLING		SHRINKAGE			PIN HOLE	REMARKS		
					#4	#200	2/4	WL	WP	IP		MOIST	SATUR		C'	σ ^o	Pp	Cc	Cv	Y _{max}	OMC	P	V	Ws			Ls	Vc
	BLACKISH VEGETAL SOIL, HARD																											
	DARK GRAY CLAY WITH SOME WEATHERED DEBRIS AND TRACES OF ORGANIC MATERIAL, HARD																											
1.0	WEATHERED DEBRIS OF SILTSTONE AND FINE GRAINED SANDSTONE IN YELLOWISH BROWN CLAY-SILTY MATRIX, LIGHT PLASTICITY VERY STIFF		30.1	2.636	100	78	28	62	36	26	MH	1.627	1.794	-	-	-	-	-	1.28	32.8	0.75	0.00		17.39	38.72	NDI	DISTURBED N 1	
2.0	GRAYISH BROWN CLAY WITH SOME OF WEATHERED DEBRIS, LIGHT PLASTICITY, VERY STIFF																											
3.0	WEATHERED YELLOWISH BROWN MUDSTONE, VERY SOFT ROCK		32.7	2.456	100	95	30	63	34	29	MH	1.642	1.759	1.88	0.70	12	0.75	0.107	13.91	1.28	31.8	1.65	0.00		15.40	41.82	NDI	DISTURBED N 2
4.0																												

END : 4.0 m

PROJECT : THE DETAILED DESIGN ON TRANSEASIN CHONE-PORTOVIJO BASIN										TEST PIT N ^o C-17					DATE : 20-12-93														
SITE: OPEN CHANNEL										COORDINATE :					N 9889600 E 604670														
DEPTH m	DESCRIPTION	PROFILE	W %	S P E C I F I C I T Y	PARTICLE SIZE			LIMITES ATTERBERG			CLASIF UCCS	UNIT WEIGHT		MOIST Kg/cmf	SATUR Kg/cmf	TRIAxIAL		CONSOLIDATION			COMPACTION		SWELLING		SHRINKAGE			PIN HOLE	REMARKS
					#4	#200	2/4	WL	WP	IP		MOIST	SATUR			C'	φ ^o deg	Pp Kg/cm2	Co	Cv mm2/min	Y _{max} gr/cmf	OMC %	P Kg/cm2	V %	Ws %	Ls %	Vc %		
	BLACKISH VEGETAL SOIL, HARD																												
	YELLOWISH BROWN CLAY WITH SOME DEBRIS, HARD																												0.40
1.0	WEATHERED DEBRIS OF MUDSTONE IN YELLOWISH BROWN SILTY-CLAY MATRIX, HARD		23.3	2.623	100	95	30	71	41	30	MH	1.498	1.718	1.66	1.50	11	0.80	0.243	10.58	1.16	32.3	1.15	0.00		16.02	44.96	ND1	DISTURBED N° 1	
2.0	WEATHERED YELLOWISH BROWN MUDSTONE, VERY SOFT ROCK																											1.60	
																												2.00	
3.0			28.6	2.680	100	99	43	73	44	29	MH	1.496										1.10	0.00		18.35	42.47	ND2	DISTURBED N° 2	
4.0	END : 3.0 m																											3.00	

30/1

PROJECT : THE DETAILED DESIGN ON TRANSBASIN CHONE--PORTOMEJO BASIN											TEST PIT N ^o C-18					DATE : 18-12-93												
SITE: OPEN CHANNEL											COORDINATE : N 9889155 E 604290																	
DEPTH m	DESCRIPTION	PROFILE	W %	GRAVITY SPECIFIC	PARTICLE SIZE			LIMITES ATTERBERG			CLASIF UCCS	UNIT WEIGHT		TRIAxIAL			CONSOLIDATION			COMPACTION		SWELLING		SHRINKAGE			PIN HOLE	REMARKS
					#4	#200	2/1	WL	WP	IP		MOIST	SATUR	C'	φ ^o	Pp	Cc	Cv	Y _{max}	OMC	P	V	Ws	Ls	Vc			
	DARK GRAY VEGETAL SOIL, HARD																											
	YELLOWISH BROWN SILTY CLAY WITH WEATHERED DEBRIS OF MUDSTONE, VERY STIFF																										0.30	
1.0	WEATHERED WITHISH BROWN MUDSTONE WITH NUMEROUS SHELLS, VERY SOFT ROCK		21.7	2.726	100	95	24	65	35	30	MH	1.528	1.753	2.40	1.20	22	0.70	0.335	6.224	1.19	34.6	1.30	0.00		18.28	46.14	N01	DISTURBED N 1
2.0	WEATHERED SOFT BROWN MUDSTONE BEDDING - HORIZONTAL																										1.60	
																											2.00	
3.0			25.5	2.715	100	95	28	64	36	28	MH	1.527	1.707							1.12	41.6	1.50	0.00		11.55	53.59	N02	DISTURBED N 2
	END : 3.0 m																										3.00	
4.0																												

PROJECT : THE DETAILED DESIGN ON TRANSBASIN CHONE-PORTOMEJO BASIN

TEST PIT N^o C-19

DATE : 15-12-93

SITE: CANAL ABIERTO OPEN CHANNEL

COORDINATE :

N 9888780

E 603850

DEPTH m	DESCRIPTION	PROFILE	W %	SPECIFIC GRAVITY	PARTICLE SIZE			LIMITS ATTERBERG			CLASIF UCCS	UNIT WEIGHT		TRIAXIAL	CONSOLIDATION			COMPACTION		SWELLING		SHRINKAGE			PIN HOLE	REMARKS		
					#4	#200	2/1	WL	WP	IP		MOIST	SATUR		C'	φ°	Pp	Cc	Cv	Y _{max}	OMC %	P	V	W _s %			L _s %	V _c %
	GRAY VEGETAL SOIL, HARD	Y-V																										
1.0	GRAY CLAY WITH SOME OF FINE GRAINED SANDSTONE AND NUMEROUS WEATHERED DEBRIS OF FINE GRAINED SANDSTONE, HARD	Y-V																										
	GRAY CLAY, HIGH PLASTICITY		24.3	2.698	100	99	30	45	22	23	CL	1.423	1.831	-	-	-	-	-	1.32	29.6	0.30	0.00		13.83	39.02	N D1	DISTURBED N 1	
2.0	GRAY CLAY WITH TRACES OF SAND AND LARGE AMOUNT OF WEATHERED DEBRIS OF FINE GRAINED SANDSTONE, HARD																											
	GRAY CLAY WITH TRACES OF SAND AND LARGE AMOUNT OF WEATHERED DEBRIS OF FINE GRAINED SANDSTONE, HARD		23.5	2.607	100	58	20	40	21	19	CL	1.423	1.857	2.56	1.65	10	0.65	0.098	2.443	1.39	30.2	0.30	0.00		18.28	31.00	N D1	DISTURBED N 2
3.0	STIFF AND VERY STIFF																											
4.0	WEATHERED YELLOWISH BROWN CLAY, VERY SOFT																											

END : 4.0 m

PROJECT : THE DETAILED DESIGN ON TRANSBASIN CHONE--PORTOMEJO BASIN										TEST PIT N ^o C-20					DATE : 15-12-93														
SITE: TRANSMISSION LINE										COORDINATE : N 9892665					E 607780														
DEPTH m	DESCRIPTION	PROFILE	W %	GRAVITY SPECIFIC	PARTICLE SIZE			LIMITES ATTERBERG			CLASIF UCCS	UNIT WEIGHT		TRIAXIAL	CONSOLIDATION			COMPACTION		SWELLING		SHRINKAGE			PIN HOLE	REMARKS			
					#4	#200	2/4	WL	WP	IP		MOIST	SATUR		C'	φ°	Pp	Co	Cv	Y _{max}	OMC	P	V	Ws			Ls	Vc	
	BLACKISH VEGETAL SOIL WITH SOME OF WEATHERED DEBRIS, HARD	Y																											
	WEATHERED DEBRIS OF MUDSTONE IN DARK BROWN CLAY MATRIX, HARD	Δ	25.7	2.649	100	80	39	49	31	18	ML	1.541	1.728	3.00	1.80	10	0.75	0.106	7.81	1.17	37.8	0.15	0.00			19.36	40.73	N01	0.35 DISTURBED N° 1
1.0	VERY WEATHERED LIGHT GRAY MUDSTONE WITH OXIDIZED JOINTS, SOFT ROCK	—																											1.10
	END : 1.50 m																												1.50
2.0																													
3.0																													
4.0																													

307

PROJECT : THE DETAILED DESIGN ON TRANSBASIN CHONE--PORTOMEJO BASIN										TEST PIT N ^o C-21					DATE : 23-12-93														
SITE: TRANSMISSION LINE										COORDINATE : N 9892400					E 613400														
DEPTH m	DESCRIPTION	PROFILE	W %	GRAVITY SPECIFIC	PARTICLE SIZE			LIMITES ATTERBERG			CLASIF UCCS	UNIT WEIGHT		TRIAXIAL	CONSOLIDATION			COMPACTION		SWELLING		SHRINKAGE			PIN HOLE	REMARKS			
					#4	#200	2/4	WL	WP	IP		MOIST	SATUR		C _c	γ _{max}	OMC %	P Kg/cm ²	V %	Ws %	Ln %	Vc %							
	DARK BROWN VEGETAL SOIL WITH SOME SAND AND ORGANIC MATERIAL, VERY STIFF																												
1.0	WEATHERED DEBRIS AND BLOCKS OF SILTSTONE AND FINE GRAINED SANDSTONE IN YELLOWISH BROWN SILTY-SANDY MATRIX, VERY STIFF		18.7	2.673	100	50	16	44	28	16	ML	1.668	1.826	0.83	0.40	20	0.74	0.134	6.18	1.32	31.00	1.10	0.00				0.40	DISTURBED N° 1	
2.0	VERY WEATHERED YELLOWISH BROWN SILTY FINE GRAINED SANDSTONE WITH OXIDIZED JOINTS, VERY SOFT ROCK		26.7	2.579	100	88	8	40	22	18	CL	1.670																1.00	DISTURBED N° 2
3.0	VERY WEATHERED YELLOWISH BROWN SILTY FINE GRAINED SANDSTONE WITH OXIDIZED JOINTS, SOFT ROCK																											2.20	
4.0	END : 3.0 m																											3.00	

PROJECT : THE DETAILED DESIGN ON TRANSBASIN CHONE-PORTOMEJO BASIN

TEST PIT N° C-22

DATE : 23-12-93

SITE: TRANSMISSION LINE ✓

COORDINATE :

DEPTH m	DESCRIPTION	PROF. FILE	W %	SPEC. GRAVITY	PARTICLE SIZE			LIMITES ATTERBERG			CLASIF. UCCS	UNIT WEIGHT		MOIST. %	SATUR. %	U _{max} Kg/cm ²	C' Kg/cm ²	φ° deg	CONSOLIDATION			COMPACTION		SWELLING		SHRINKAGE			PIN HOLE	REMARKS		
					#4	#200	2/1	WL	WP	IP		MOIST	SATUR						Pp Kg/cm ²	Cc	Cy	Y _{max}	OMC %	P Kg/cm ²	V %	Ws %	LS %	Vc %				
	BLACKISH VEGETAL SOIL WITH SOME OF FINE GRAINED SAND AND ORGANIC MATERIAL, VERY STIFF	✓																														
1.0	GRAYISH BROWN SILTY CLAY WITH SOME WEATHERED DEBRIS		39.64	2.832	100		50	106	58	48	CH	1.673	1.750	13.18	1.10	26	0.40	0.18	2.597	1.16	38.00	2.49	0.00				18.09			NDI	DISTURBED N° 1	
2.0	WEATHERED DEBRIS OF SILTSTONE AND FINE GRAINED SANDSTONE IN BROWN SILTY MATRIX		27.3	2.658	100	49	15	60	29	31	SC	1.660																				DISTURBED N° 2
3.0	WEATHERED BROWN MUDSTONE WITH OXIDIZED ZONE, VERY SOFT ROCK																															
4.0	END : 3.50 m																															

PROJECT : THE DETAILED DESIGN ON TRANSBASIN CHONE--PORTOMEJO BASIN										TEST PIT N° C-23					DATE : 27-12-93															
SITE: TRANSMISSION LINE										COORDINATE : 9895874					627354															
DEPTH m	DESCRIPTION	PROFILE	W %	GRAVITY SPECIFIC	PARTICLE SIZE			LIMITES ATTERBERG			CLASIF UCCS	UNIT WEIGHT		TRIAIAL		CONSOLIDATION			COMPACTION		SWELLING		SHRINKAGE			PIN HOLE	REMARKS			
					#4	#200	2/1	WL	WP	IP		MOIST	SATUR	C'	φ°	Pp	Cc	Cv	Ymax	OMC %	P	V	Ws %	Ls %	Vc %					
	BLACKISH CLAY SILT WITH SOME FINE GRAINED SAND, HARD																													
	GRAYISH BROWN SILTY CLAY, WITH NUMEROUS WEATHERED DEBRIS OF SILTSTONE, HIGH PLASTICITY, HARD	D D D D																												
1.0	GRAYISH BROWN SILTY CLAY, HIGH PLASTICITY, HARD		44.03	2.873	100		65	114	51	63	CH	1.719	1.757	8.38	2.60	14	0.60	0.18	2.575	1.16	46.00	1.36	0.00		27.26		NDI	1.20	DISTURBED N° 1	
2.0	REDISH BROWN SILTY CLAY, VERY STIFF																													
			37.7	2.733	100	98	50	74	30	44	CH	1.67	-	-	-	-	-	-	-	-	-	0.75	0.00		20.27	49.19	NDI	2.00	DISTURBED N° 2	
3.0	GRAYISH BROWN VERY WEATHERED SILTY FINE GRAINED SANDSTONE, SOFT ROCK																													
	END : 3.50 m																													
4.0																														

PROJECT : THE DETAILED DESIGN ON TRANSBASIN CHONE--PORTOVIEJO BASIN										TEST PIT N° C-24					DATE : 27-12-93																
SITE: TRANSMISSION LINE										COORDINATE : 9897090					634780																
DEPTH m	DESCRIPTION	PROFILE	W %	GRAVITY SPECIFIC	PARTICLE SIZE			LIMITES ATTERBERG			CLASIF UCCS	UNIT WEIGHT		TRAXIAL	CONSOLIDATION			COMPACTION		SWELLING		SHRINKAGE			PIN HOLE	REMARKS					
					#4	#200	2/1	WL	WP	IP		MOIST	SATUR		C'	φ°	Pp	Cc	Cv	Ymax	OMC %	P	V	Ws %			Ls %	Yc %			
	BLACKISH VEGETAL SOIL WITH SOME OF SAND, HARD																														
	WEATHERED DEBRIS OF SANDSTONE AND SILTSTONE IN GRAY SILTY-CLAY MATRIX, HARD																														
1.0	GRAYISH BROWN CLAY WITH REDISH ZONE (OXIDIZED) HIGH PLASTICITY, VERY STIFF																														
			41.57	2.803	100		53	87	44	43	CH	1.655	1.843	1.97	0.80	14	0.50	0.15	2.404	1.31	36.00	0.51	0.00		24.87		ND1	1.50	DISTURBED N° 1		
2.0																														1.80	DISTURBED N° 1
																														2.50	DISTURBED N° 1
3.0	GRAYISH BROWN SILT WITH SOME OF FINE GRAINED SAND, MEDIUM PLASTICITY, STIFF																														
			57.7	2.681	100	90	33	64	32	32	MH	1.66	-	-	-	-	-	-	-	-	-	0.76	0.00		16.36	44.57	ND2		DISTURBED N° 2		
4.0																															

END : 4.00 m



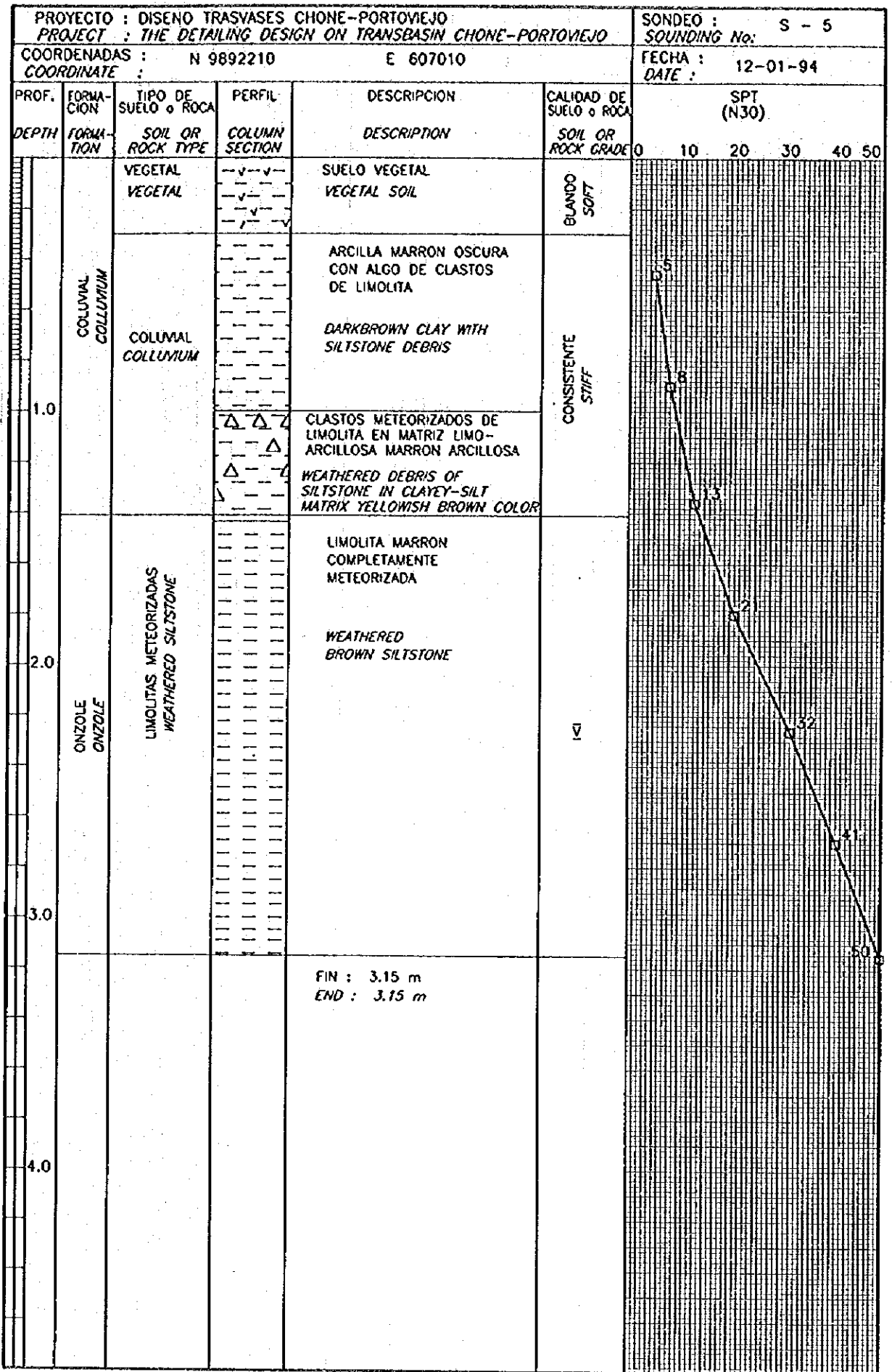
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PROYECTO : DISEÑO TRASVASES CHONE-PORTOVIEJO PROJECT : THE DETAILING DESIGN ON TRANSBASIN CHONE-PORTOVIEJO						SONDEO : S - 1 SOUNDING No:	
COORDENADAS : N 9892430 E 607520						FECHA : 11-01-94 DATE :	
PROF. DEPTH	FORMACION FORMATION	TIPO DE SUELO o ROCA SOIL OR ROCK TYPE	PERFIL COLUMN SECTION	DESCRIPCION DESCRIPTION	CALIDAD DE SUELO o ROCA SOIL OR ROCK GRADE	SPT (N30)	
						0	10 20 30 40 50
	VEGETAL VEGETAL	HUMUS VEGETAL SOIL	---v--- ---v--- ---v--- ---v---	SUELO VEGETAL VEGETAL SOIL	BLANDO SOFT		
1.0			----- ----- ----- -----	ARCILLA LIMOSA MARRON AMARILLENTO CON ALGO DE CLASTOS METEORIZADOS YELLOWISH BROWN SILTY CLAY WITH WEATHERED DEBRIS			9
2.0	COLLUVIAL COLLUVIUM	ARCILLAS Y CLASTOS CLAY AND DEBRIS	△△△ △△△ △△△ △△△ △△△ △△△ △△△ △△△ △△△ △△△	CLASTOS METEORIZADOS DE LIMOLITA EN MATRIZ LIMO-ARCILLOSO MARRON AMARILLENTO WEATHERED SILTSTONE DEBRIS IN YELLOWISH BROWN CLAYEY SILTY MATRIX	CONSISTENTE STIFF		10 12
3.0			----- ----- ----- -----	ARCILLA MARRON GRISACEA CON CLASTOS DE LIMOLITA MARRON OSCURA, MUY PLASTICA GRAYISH BROWN CLAY WITH DARK BROWN SILTSTONE DEBRIS, HIGH PLASTICITY			15
4.0	ONZOLE ONZOLE	LIMONITAS METEORIZADAS WEATHERED SILTSTONE	----- ----- ----- ----- ----- ----- ----- ----- ----- -----	LIMOLITA MARRON AMARILLENTO, COMPLETAMENTE METEORIZADA YELLOWISH BROWN SILTSTONE VERY WEATHERED	IV		26 34 44
				FIN : 4.05 m END : 4.05 m			

PROYECTO : DISEÑO TRASVASES CHONE-PORTOVIJEJO PROJECT : THE DETAILING DESIGN ON TRANSBASIN CHONE-PORTOVIJEJO						SONDEO : S - 2 SOUNDING No:					
COORDENADAS : N 9892380 E 607370 COORDINATE :				FECHA : 12-01-94 DATE :							
PROF.	FORMACION	TIPO DE SUELO o ROCA	PERFIL	DESCRIPCION	CALIDAD DE SUELO o ROCA	SPT (N30)					
DEPTH	FORMATION	SOIL OR ROCK TYPE	COLUMN SECTION	DESCRIPTION	SOIL OR ROCK GRADE	0	10	20	30	40	50
				COBERTURA VEGETAL VEGETAL SOIL							
				LIMO ARCILLOSO GRIS BLANQUESINO CON CLASTOS DE LIMOLITA LIGHTGRAY CLAYEY SILT WITH DEBRIS OF SILTSTONE	BLANDO SOFT						
1.0	COLLUVIAL COLLUVIUM	COLLUVIAL COLLUVIUM									
				CLASTOS METEORIZADOS DE LIMOLITA EN MATRIZ LIMO-ARCILLOSO MARRON AMARILLENTO WEATHERED DEBRIS OF SILTSTONE IN YELLOWISH BROWN CLAYEY SILTSTONE MATRIX	CONSISTENTE STIFF						
2.0											
	ONZOLE ONZOLE	LUTITA COMPLETAMENTE METEORIZADAS VERY WEATHERED MUDSTONE		LUTITA MARRON AMARILLENTO COMPLETAMENTE METEORIZADA VERY WEATHERED YELLOWISH BROWN MUDSTONE	1-1						
3.0											
4.0				FIN : 3.60 m END : 3.60 m							

PROYECTO : DISEÑO TRASVASES CHONE-PORTOVIJEJO PROJECT : THE DETAILING DESIGN ON TRANSBASIN CHONE-PORTOVIJEJO						SONDEO : S - 3 SOUNDING No:					
COORDENADAS : N 9892330 E 607370 COORDINATE :				FECHA : 12-01-94 DATE :							
PROF. DEPTH	FORMA- CION FORMA- TION	TIPO DE SUELO o ROCA SOIL OR ROCK TYPE	PERFIL COLUMN SECTION	DESCRIPCION DESCRIPTION	CALIDAD DE SUELO o ROCA SOIL OR ROCK GRADE	SPT (N30)					
		VEGETAL VEGETAL	--v--v-- --v-- --v--	SUELO VEGETAL VEGETAL SOIL		0	10	20	30	40	50
	COLLUVIAL COLLUVIUM	COLLUVIAL COLLUVIUM		ARCILLA GRIS AMARILLEN- TA CON ALGO DE CLASTOS METEORIZADOS YELLOWISH GRAY CLAY WITH WEATHERED DEBRIS	BLANDO SOFT						
1.0				CLASTOS METEORIZADOS DE LIMOLITA EN MATRIZ LIMO-ARCILLOSO MARRON AMARILLEN- TA WEATHERED DEBRIS OF SILTSTONE IN YELLOWISH BROWN CLAYEY SILT MATRIX	CONSISTENTE STIFF						
2.0	ONZOLE ONZOLE	LIMONITA COMPLETAMENTE METEORIZADAS VERY WEATHERED SILTSTONE		LIMOLITA MARRON AMARILLEN- TA, COMPLETAMENTE METEORIZADA CON PLANOS OXIDADOS VERY WEATHERED YELLOWISH BROWN SILTSTONE WITH OXIDIZED JOINTS	IV						
3.0											
4.0				FIN : 3.60 m END : 3.60 m							


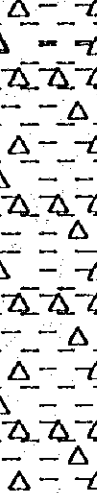
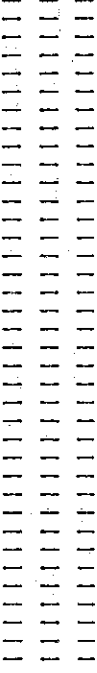
PROYECTO : DISEÑO TRASVASES CHONE-PORTOVIEJO PROJECT : THE DETAILING DESIGN ON TRANSBASIN CHONE-PORTOVIEJO					SONDEO : S - 4 SOUNDING No:	
COORDENADAS : N 9892240 E 607100 COORDINATE :					FECHA : 12-01-94 DATE :	
PROF. DEPTH	FORMACION FORMATION	TIPO DE SUELO o ROCA SOIL OR ROCK TYPE	PERFIL COLUMN SECTION	DESCRIPCION DESCRIPTION	CALIDAD DE SUELO o ROCA SOIL OR ROCK GRADE	SPT (N30)
						0 10 20 30 40 50
	COLUVAL COLLUVIUM	VEGETAL VEGETAL		SUELO VEGETAL VEGETAL SOIL	BLANDO SOFT	
		COLUVAL COLLUVIUM		CLASTOS METEORIZADOS DE ARENISCA FINA EN MATRIZ LIMO ARCILLOSA MARRON AMARILLA WEATHERED DEBRIS OF FINE GRAINED SANDSTONE IN CLAYEY SILT MATRIX IN YELLOWISH BROWN COLOR	CONSISTENTE STIFF	
1.0	ONZOLE ONZOLE	ARENISCA FINA METEORIZADA WEATHERED FINE GRAINED SANDSTONE		ARENISCA FINA LIMOSA AMARILLA, COMPLETAMENTE METEORIZADA VERY WEATHERED YELLOWISH SILTY FINE GRAINED SANDSTONE	V	
2.0				FIN : 2.20 m END : 2.20 m		
3.0						
4.0						

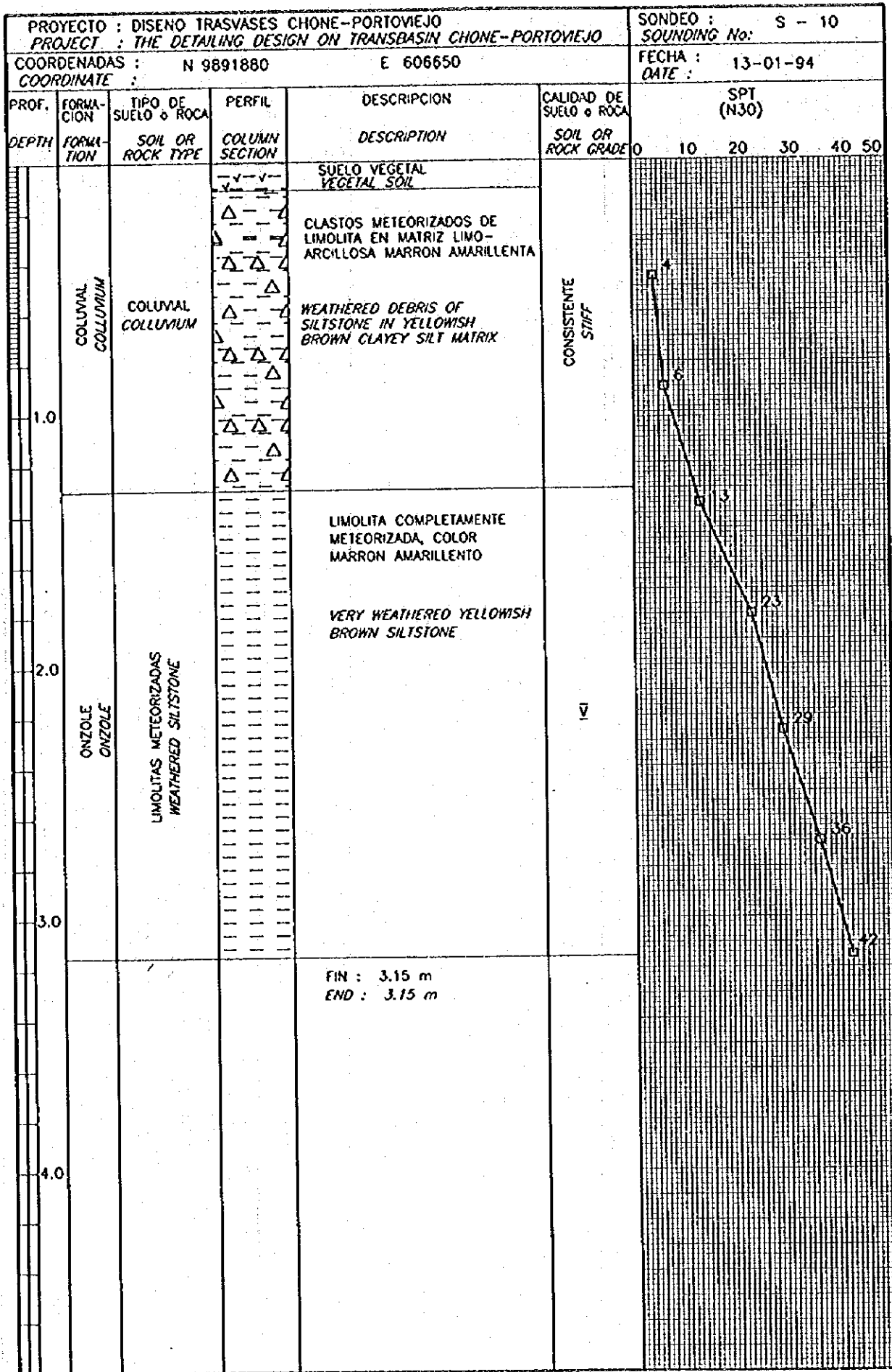


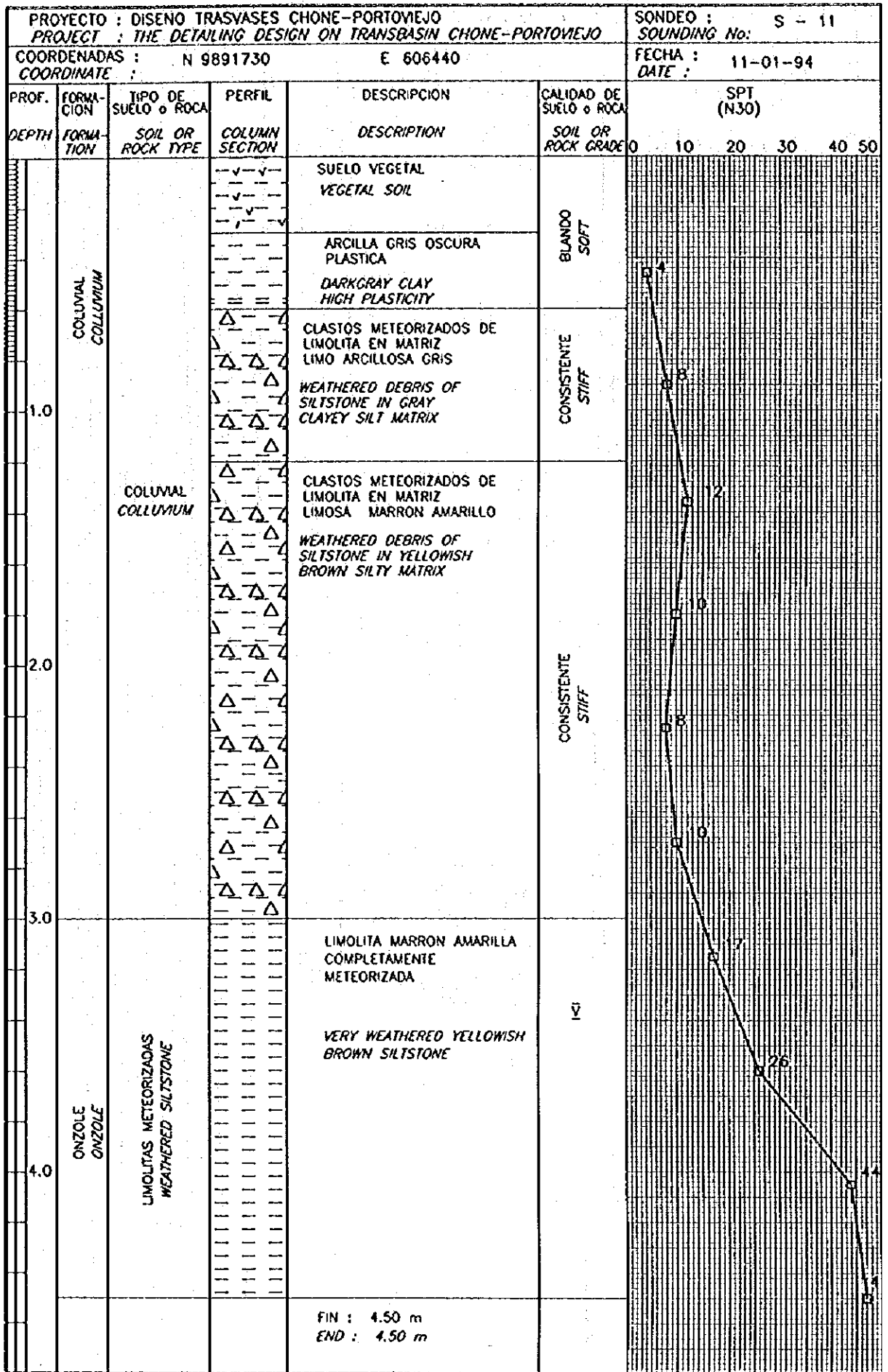
PROYECTO : DISEÑO TRASYASES CHONE-PORTOVIJEJO PROJECT : THE DETAILING DESIGN ON TRANSBASIN CHONE-PORTOVIJEJO						SONDEO : S - 6 SOUNDING No:					
COORDENADAS : N 9892180 E 607030 COORDINATE :				FECHA : 12-01-94 DATE :							
PROF. DEPTH	FORMA- TION	TIPO DE SUELO o ROCA SOIL OR ROCK TYPE	PERFIL COLUMN SECTION	DESCRIPCION DESCRIPTION	CALIDAD DE SUELO o ROCA SOIL OR ROCK GRADE	SPT (N30)					
						0	10	20	30	40	50
1.0	COLUVIAL COLLUVIUM	VEGETAL VEGETAL		SUELO VEGETAL VEGETAL SOIL	CONSISTENTE STIFF						
		ARCILLA MARRON OSCURA CON ALGO DE CLASTOS METEORIZADOS DARKBROWN CLAY WITH WEATHERED DEBRIS									
		CLASTOS METEORIZADOS DE LIMOLITA EN MATRIZ LIMOSA MARRON AMARILLENTO WEATHERED DEBRIS OF SILTSTONE IN YELLOWISH BROWN SILTY MATRIX									
2.0	ONZOLE ONZOLE	LIMOLITAS METEORIZADAS WEATHERED SILTSTONE		LIMOLITA MARRON AMARILLA COMPLETAMENTE METEORIZADA VERY WEATHERED YELLOWISH BROWN SILTSTONE MUY METEORIZADA VERY WEATHERED	IV IV						
3.0				FIN : 2.50 m END : 2.50 m							
4.0											

PROYECTO : DISEÑO TRASVASES CHONE-PORTOVIEJO PROJECT : THE DETAILING DESIGN ON TRANSBASIN CHONE-PORTOVIEJO						SONDEO : S - 7 SOUNDING No:	
COORDENADAS : N 9892180 E 606840						FECHA : 13-01-94 DATE :	
PROF. DEPTH	FORMA- CION FORMA- TION	TIPO DE SUELO o ROCA SOIL OR ROCK TYPE	PERFIL COLUMN SECTION	DESCRIPCION DESCRIPTION	CALIDAD DE SUELO o ROCA SOIL OR ROCK GRADE	SPT (N30)	
						0	10 20 30 40 50
				SUELO VEGETAL VEGETAL SOIL	BLANDO SOFT		
				CLASTOS METEORIZADOS DE LIMOLITA MARRON AMARILLENTO <i>WEATHERED DEBRIS OF SILTSTONE IN YELLOWISH BROWN SILTY MATRIX</i>	BLANDO SOFT	4	
1.0						10	
					CONSISTENTE STIFF	12	
2.0						10	
				ARCILLA GRIS OSCURA MUY PLASTICA <i>DARK GRAY CLAY WITH HIGH PLASTICITY</i>	CONSISTENTE STIFF	10	
3.0						10	
				CLASTOS METEORIZADOS DE LIMOLITA EN MATRIZ LIMOSA MARRON AMARILLENTO <i>WEATHERED DEBRIS OF SILTSTONE IN YELLOWISH BROWN SILTY MATRIX</i>	CONSISTENTE STIFF	8	
4.0						2	
	ONZOLE ONZOLE	LIMOLITA METEORIZADA WEATHERED SILTSTONE		LIMOLITA MARRON COMPLETAMENTE METEORIZADA <i>WEATHERED BROWN SILTSTONE</i>	V	1	
				FIN 4.95m. END 4.95m.		6	
						2	

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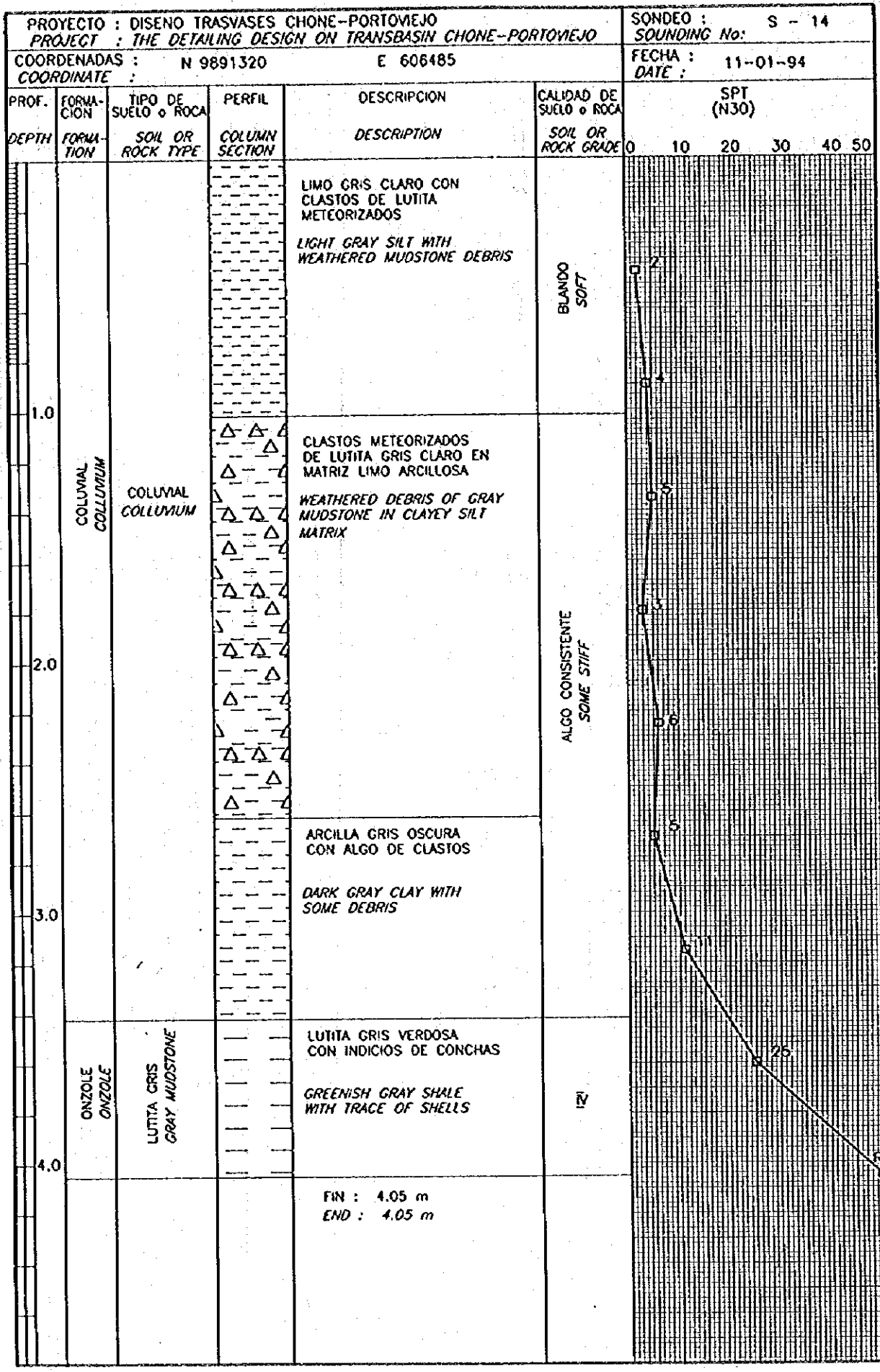
PROYECTO : DISEÑO TRASFASES CHONE-PORTOVIEJO PROJECT : THE DETAILING DESIGN ON TRANSBASIN CHONE-PORTOVIEJO						SONDEO : S - 9 SOUNDING No:	
COORDENADAS : N 9891930 E 606620				FECHA : 13-01-94 DATE :			
PROF. DEPTH	FORMACION FORMATION	TIPO DE SUELO o ROCA SOIL OR ROCK TYPE	PERFIL COLUMN SECTION	DESCRIPCION DESCRIPTION	CALIDAD DE SUELO o ROCA SOIL OR ROCK GRADE	SPT (N30)	
						0	10 20 30 40 50
	COLUVIAL COLLUVIUM	COLUVIAL COLLUVIUM		SUELO VEGETAL VEGETAL SOIL	BLANDO SOFT		
1.0				CLASTOS METEORIZADOS DE LIMOLITA EN MATRIZ LIMO-ARCILLOSA MARRON AMARILLENTO WEATHERED DEBRIS OF SILTSTONE IN YELLOWISH BROWN CLAYEY SILT MATRIX		3	
2.0	ONZOLE ONZOLE	LIMOLITAS METEORIZADAS WEATHERED SILTSTONE		LIMOLITA COMPLETAMENTE METEORIZADA, COLOR MARRON AMARILLENTO VERY WEATHERED YELLOWISH BROWN SILTSTONE	M	4	5
3.0				FIN : 3.60 m END : 3.60 m		6	7
4.0						8	9

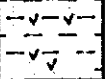
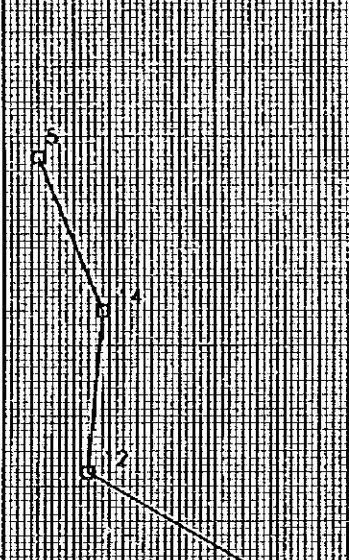

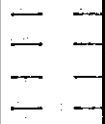




PROYECTO : DISEÑO TRASVASES CHONE-PORTOVIEJO PROJECT : THE DETAILING DESIGN ON TRANSBASIN CHONE-PORTOVIEJO						SONDEO : S - 12 SOUNDING No:	
COORDENADAS : N 9891745 E 606410						FECHA : 11-01-94 DATE :	
PROF.	FORMA- CION	TIPO DE SUELO o ROCA	PERFIL	DESCRIPCION	CALIDAD DE SUELO o ROCA	SPT (N30)	
DEPTH	FORMA- TION	SOIL OR ROCK TYPE	COLUMN SECTION	DESCRIPTION	SOIL OR ROCK GRADE	0	10 20 30 40 50
				SUELO VEGETAL VEGETAL SOIL	BLANDO SOFT		
				ARCILLA GRIS PLASTICA GRAY CLAY HIGHT PLASTICITY			
1.0	COLLUVIAL COLLUVIUM	COLLUVIAL COLLUVIUM		CLASTOS METEORIZADOS DE LIMOLITA EN MATRIZ LIMOSA GRIS WEATHERED DEBRIS OF SILTSTONE IN GRAY SILTY MATRIX	ALGO CONSISTENTE SOME STIFF	4	6
				MATRIZ COLOR MARRON AMARILLENTO YELLOWISH BROWN MATRIX		6	8
2.0						8	11
3.0	ONZOLE ONZOLE	LIMOLITAS METEORIZADAS WEATHERED SILTSTONE		LIMOLITA MARRON AMARILLA COMPLETAMENTE METEORIZADA VERY WEATHERED YELLOWISH BROWN SILTSTONE	17	16	25
4.0						29	
				FIN : 4.05 m END : 4.05 m			

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PROYECTO : DISEÑO TRASVASES CHONE-PORTOVIJEJO PROJECT : THE DETAILING DESIGN ON TRANSBASIN CHONE-PORTOVIJEJO						SONDEO : S - 15 SOUNDING No:	
COORDENADAS : N 9891330 E 606460 COORDINATE :						FECHA : 11-01-94 DATE :	
PROF. DEPTH	FORMACION FORMATION	TIPO DE SUELO o ROCA SOIL OR ROCK TYPE	PERFIL COLUMN SECTION	DESCRIPCION DESCRIPTION	CALIDAD DE SUELO o ROCA SOIL OR ROCK GRADE	SPT (N30)	
						0	10 20 30 40 50
1.0	COLLUVIAL COLLUVIUM	COLLUVIAL COLLUVIUM		LIMO ARCILLOSO NEGRUZCO BLACKISH CLAYEY SILT	CONSISTENTE STIFF		
				CLASTOS METEORIZADOS DE LIMOLITA GRIS CLARO CON ALGO DE ARCILLA GRIS OSCURA WEATHERED DEBRIS OF SILTSTONE WITH SOME DARK GRAY CLAY			
2.0	ONZOLE ONZOLE	LUTITA GRIS GRAY MUDSTONE		LUTITA GRIS VERDOSA CON INDICIOS DE CONCHAS GREENISH GRAY MUDSTONE WITH TRACE OF SHELLS	R		
3.0				FIN : 2.00 m END : 2.00 m			
4.0							

PROYECTO : DISEÑO TRASVASES CHONE-PORTOVIJEJO PROJECT : THE DETAILING DESIGN ON TRANSBASIN CHONE-PORTOVIJEJO					SONDEO : S - 16 SOUNDING No:	
COORDENADAS : N 9891180 E 606320 COORDINATE :					FECHA : 11-01-94 DATE :	
PROF. DEPTH	FORMACION FORMATION	TIPO DE SUELO o ROCA SOIL OR ROCK TYPE	PERFIL COLUMN SECTION	DESCRIPCION DESCRIPTION	CALIDAD DE SUELO o ROCA SOIL OR ROCK GRADE	SPT (N30)
						0 10 20 30 40 50
	COLUVIAL COLLUVIUM			SUELO VEGETAL NEGRUZCO BLACKISH VEGETAL SOIL	BLANDO SOFT	
				ARCILLA GRIS OSCURA CON ALGO DE CLASTOS DE LIMOLITA GRIS CLARA DARK GRAY CLAY WITH SOME DEBRIS OF LIGHT GRAY SILTSTONE		
1.0				CLASTOS Y BLOQUES DE LIMOLITA GRIS CLARO EN MATRIZ LIMO ARCILLOSA LIGHT GRAY DEBRIS AND BLOCKS OF SILTSTONE IN CLAYEY SILT MATRIX	CONSISTENTE STIFF	
2.0	ONZOLE ONZOLE	LIMOLITAS METEORIZADAS WEATHERED SILTSTONE		LIMOLITA MARRON AMARILLENTA, MUY METEORIZADA VERY WEATHERED YELLOWISH BROWN SILTSTONE	12	
3.0				FIN : 2.40 m END : 2.40 m		
4.0						

PROYECTO : DISEÑO TRASVASES CHONE-PORTOVIJO PROJECT : THE DETAILING DESIGN ON TRANSBASIN CHONE-PORTOVIJO						SONDEO : S - 17 SOUNDING No:	
COORDENADAS : N 9891210 E 606310 COORDINATE :				FECHA : 11-01-94 DATE :			
PROF. DEPTH	FORMA- TION	TIPO DE SUELO o ROCA SOIL OR ROCK TYPE	PERFIL COLUMN SECTION	DESCRIPCION DESCRIPTION	CALIDAD DE SUELO o ROCA SOIL OR ROCK GRADE	SPT (N30)	
						0	10 20 30 40 50
	COLUVAL COLLUVIUM			SUELO VEGETAL NEGRUZCO BLACKISH VEGETAL SOIL	BLANDO SOFT		
				ARCILLA LIMOSA GRIS OSCURA CON INDICIOS DE MATERIAL ORGANICO Y CLASTOS METEORIZADOS DARK GRAY SILTY CLAY WITH TRACES OF WEATHERED DEBRIS AND ORGANIC MATERIAL			
1.0				CLASTOS Y BLOQUES METEO- RIZADOS DE LIMOLITA EN MATRIZ LIMO ARCILLOSA MARRON AMARILLA WEATHERED SILTSTONE DEBRIS AND BLOCKS IN YELLOWISH BROWN CLAYEY SILT MATRIX	CONSISTENTE STIFF		
2.0	ONZOLE ONZOLE	LIMOLITAS METEORIZADAS WEATHERED SILTSTONE		LIMOLITA MARRON AMARILLENTO COMPLETAMENTE METEORIZADA Y OXIDADA VERY WEATHERED AND OXIDIZED YELLOWISH BROWN SILTSTONE	(R)		
3.0				FIN : 2.50 m END : 2.50 m			
4.0							

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PROYECTO : DISEÑO TRASFASES CHONE-PORTOVIJEJO PROJECT : THE DETAILING DESIGN ON TRANSBASIN CHONE-PORTOVIJEJO					SONDEO : S - 18 SOUNDING No:	
COORDENADAS : N 9891080 E 606140					FECHA : 11-01-94 DATE :	
PROF. DEPTH	FORMA- CION FORMA- TION	TIPO DE SUELO o ROCA SOIL OR ROCK TYPE	PERFIL COLUMN SECTION	DESCRIPCION DESCRIPTION	CALIDAD DE SUELO o ROCA SOIL OR ROCK GRADE	SPT (N30) 0 10 20 30 40 50
0.0			▽▽▽▽	SUELO VEGETAL VEGETAL SOIL	BLANDO SOFT	0
0.5			-----	ARCILLA NEGRUZCA CON INDICIOS DE CLASTOS OXIDADOS BLACKISH CLAY WITH TRACES OF OXIDIZED DEBRIS		5
1.0			▽▽▽▽	CLASTOS OXIDADOS DE LIMOLITA EN MATRIZ ARCILLOSA MARRON AMARILLENTO OXIDIZED DEBRIS OF SILTSTONE IN YELLOWISH BROWN CLAYEY MATRIX	CONSISTENTE STIFF	10
1.5			-----	ARCILLA MARRON GRISACEA CON BASTANTE CLASTOS METEORIZADOS GRAYISH BROWN CLAY WITH NUMEROUS WEATHERED DEBRIS		15
2.0			▽▽▽▽	CLASTOS METEORIZADOS Y OXIDADOS DE LIMOLITA EN MATRIZ LIMOSA MARRON AMARILLA WEATHERED AND OXIDIZED DEBRIS OF SILTSTONE IN YELLOWISH BROWN SILTY MATRIX		20
2.5			-----	LUTITA MARRON AMARILLENTO COMPLETAMENTE METEORIZADA WEATHERED YELLOWISH BROWN MUDSTONE	V	25
3.0			-----			30
3.5			-----			35
4.0			-----			40
4.5			-----			45
4.95			-----			49
4.95			-----			50

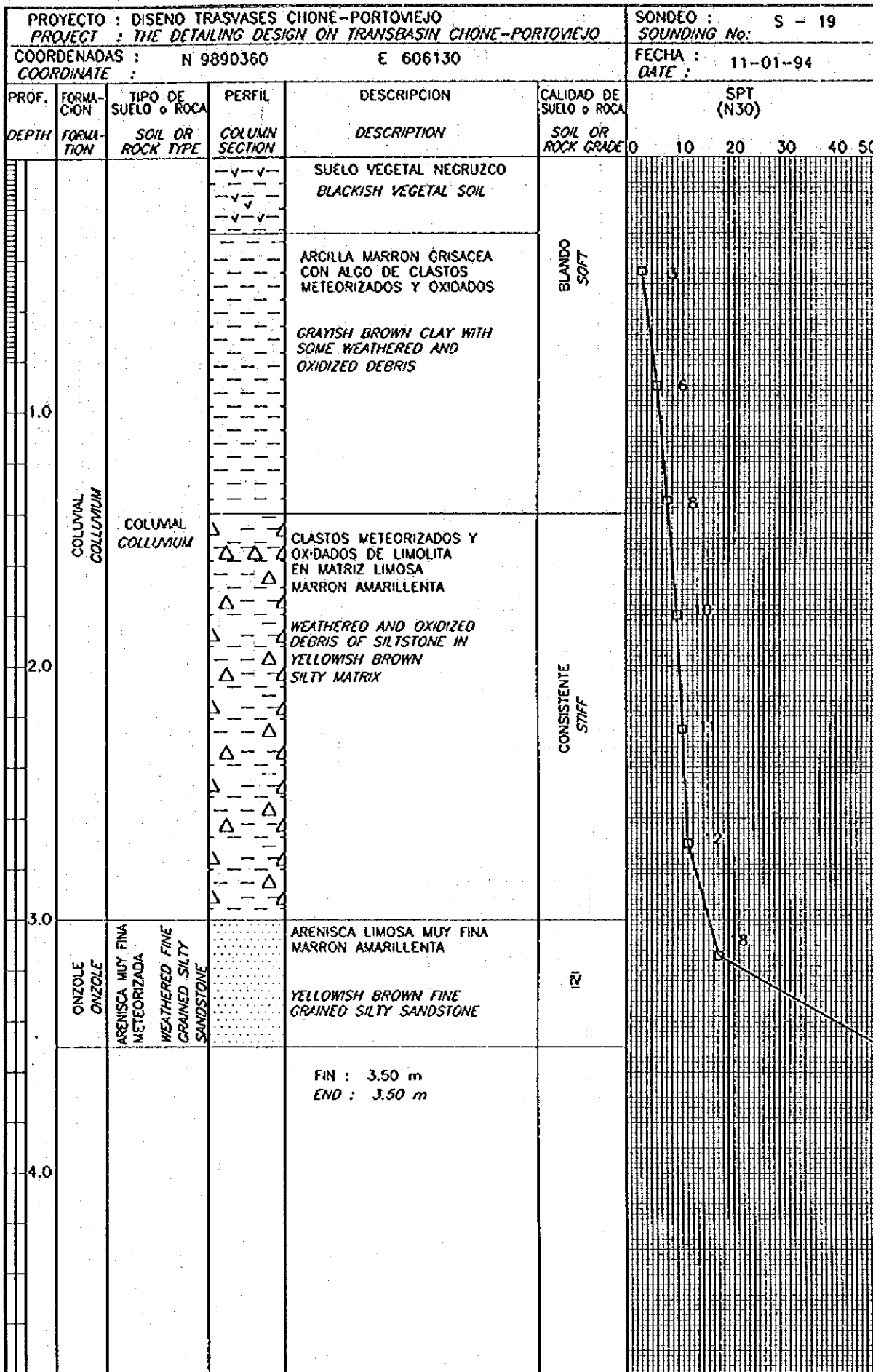
COLUVIAL
COLLUVIUM

COLUVIAL
COLLUVIUM

ONZOLE
ONZOLE

LUTITA METEORIZADA
WEATHERED MUDSTONE

FIN 4.95m.
END 4.95m.



PROYECTO : DISEÑO TRASFASES CHONE-PORTOVIJEJO PROJECT : THE DETAILING DESIGN ON TRANSBASIN CHONE-PORTOVIJEJO						SONDEO : S - 20 SOUNDING No:	
COORDENADAS : N 9890370 E 606090 COORDINATE :						FECHA : 12-01-94 DATE :	
PROF. DEPTH	FORMA- CION FORMATION	TIPO DE SUELO o ROCA SOIL OR ROCK TYPE	PERFIL COLUMN SECTION	DESCRIPCION DESCRIPTION	CALIDAD DE SUELO o ROCA SOIL OR ROCK GRADE	SPT (N30)	
						0	10 20 30 40 50
				SUELO VEGETAL NEGRUZCO BLACKISH VEGETAL SOIL			
				ARCILLA MARRON GRISACEA CON ALGO DE CLASTOS METEORIZADOS GRAYISH BROWN CLAY WITH SOME OF WEATHERED DEBRIS	BLANDO SOFT		
1.0				CLASTOS METEORIZADOS Y OXIDADOS DE LIMOLITA EN MATRIZ LIMOSA MARRON AMARILLENTO WEATHERED AND OXIDIZED DEBRIS OF SILTSTONE IN YELLOWISH BROWN SILTY MATRIX			
2.0					CONSISTENTE STIFF		
	ONZOLE ONZOLE	ARENISCA MUY FINA METEORIZADA WEATHERED FINE GRAINED SILTY SANDSTONE		ARENISCA GRANO FINO LIMOSA MARRON AMARILLENTO YELLOWISH BROWN FINE GRAINED SILTY SANDSTONE	R2		
3.0							
				FIN : 360 m END : 360 m			
4.0							

PROYECTO : DISEÑO TRASYASES CHONE-PORTOVIEJO PROJECT : THE DETAILING DESIGN ON TRANSBASIN CHONE-PORTOVIEJO						SONDEO : S - 21 SOUNDING No:					
COORDENADAS : N 9890760 COORDINATE :			E 605950			FECHA : 12-01-94 DATE :					
PROF. DEPTH	FORMA- TION	TIPO DE SUELO o ROCA SOIL OR ROCK TYPE	PERFIL COLUMN SECTION	DESCRIPCION DESCRIPTION	CALIDAD DE SUELO o ROCA SOIL OR ROCK GRADE	SPT (N30)					
						0	10	20	30	40	50
				ARCILLA LIMOSA GRIS OSCURA DARK GRAY SILTY CLAY							
1.0				ARCILLA MARRON GRISACEA CON ALGO DE CLASTOS METEORIZADOS GRAYISH BROWN CLAY WITH SOME OF WEATHERED DEBRIS	BLANDO SOFT						
				CLASTOS METEORIZADOS DE LUTITA EN MATRIZ ARCILLOSA MARRON AMARILLENTO WEATHERED DEBRIS OF MUDSTONE IN YELLOWISH BROWN CLAYEY MATRIX	CONSISTENTE STIFF						
2.0				LUTITA MARRON AMARILLENTO COMPLETAMENTE METEORIZADA WEATHERED YELLOWISH BROWN MUDSTONE	1<1						
3.0	ONZOLE ONZOLE	LUTITA METEORIZADA WEATHERED MUDSTONE		MUY METEORIZADA VERY WEATHERED	1<2						
4.0				FIN : 3.90 m END : 3.90 m							

PROYECTO : DISEÑO TRASVASES CHONE-PORTOVIJEJO PROJECT : THE DETAILING DESIGN ON TRANSBASIN CHONE-PORTOVIJEJO						SONDEO : S-22 SOUNDING No:	
COORDENADAS : N 9890700 E 605900 COORDINATE :						FECHA : 12-01-94 DATE :	
PROF. DEPTH	FORMA- CION FORMATION	TIPO DE SUELO o ROCA SOIL OR ROCK TYPE	PERFIL COLUMN SECTION	DESCRIPCION DESCRIPTION	CALIDAD DE SUELO o ROCA SOIL OR ROCK GRADE	SPT (N30)	
						0	10 20 30 40 50
				SUELO VEGETAL NEGRUSCO VEGETAL SOIL			
1.0	COLLUVIAL COLLUVIUM			LIMO ARCILLOSO MARRON OSCURO CON ALGO DE CLAS- TOS METEORIZADOS. DARK BROWN CLAYEY SILT WITH SOME WEATHERED DEBRIS MARRON AMARILLO YELLOWISH BROWN COLOR	BLANDO SOFT		
				CLASTOS DE ARENISCA MUY FINA COMPLETAMENTE METEORIZADOS EN MATRIZ LIMO-ARENOSA WEATHERED DEBRIS OF FINE GRAINED SANDSTONE IN SANDY SILT MATRIX	CONSISTENTE STIFF		
2.0	ONZOLE ONZOLE	ARENISCA MUY FINA METEORI- ZADA WEATHERED FINE GRAINED SILTY SANDSTONE		ARENISCA MUY FINA LIMOSA COMPLETAMENTE METEORIZADA MARRON AMARILLO VERY WEATHERED YELLOWISH BROWN FINE GRAINED SANDSTONE.	V		
3.0				FIN 2.95m. END 2.95m.			
4.0							

