

5.4 Water Demand Projection by Sector and by Region

5.4.1 Demand Distribution of Municipalities Located in the Basin Boundary

(1) Urban Areas (Domestic and Industrial Water)

There are some urban areas which straddle boundaries of several basins such as Cascavel and others. Water demand for these urban areas are to belong to Iguaçu river basin.

(2) Rural Areas (Domestic and Agricultural Water)

Water demand for rural areas which straddle boundaries of several basins are distributed from municipality unit to Iguaçu river basin in accordance with the following equation:

$$D_P = D_M \times \frac{A_P}{A_M}$$

where:

- D_P ; rural water demand of Iguaçu river basin
- D_M ; rural water demand of municipality
- A_P ; area of municipality in Iguaçu river basin
- A_M ; area of municipality

5.4.2 Water Demand Projection in Iguaçu River Basin

The water demand projection by sector and by region in Iguaçu river basin was calculated for both base and alternative cases as shown in Table-5.13.

5.5 Environmental Sanitation Program for Curitiba Metropolitan Region (PROSAM)

PROSAM is an environmental sanitation project for Curitiba Metropolitan region including water supply, flood control and urban drainage, sewerage system, environment conservation etc. It includes construction of Irai dam for water supply of 2.0 m³/sec, drainage channel excavation with a length of 15 km along left bank of the Iguaçu River, development river park, sewer pipe line with a length of 1,300 km, 8 sewerage treatment plants, solid waste disposal, bridges, and environmental education, etc. The total project cost amounts to US\$ 233 x 10⁶, 52% of which is to be financed by the World Bank. The project is scheduled to be implemented from 1992 to 1997. However, the progress rate as of June 1995 is estimated less than 20%.

ANEXO 1.1 - DEMANDA DE SETOR E POR REGIÃO DO BACIA DO RIBEIRÃO IGUAÇU [m³/dia]

MRH	No.	Name	Type	Zone	1993						2003						2015					
					Urban			Rural			Urban			Rural			Urban			Rural		
					Domestic	Industrial	Agricultural	Domestic	Agricultural	Domestic	Industrial	Agricultural	Domestic	Industrial	Agricultural	Domestic	Industrial	Agricultural	Domestic	Industrial	Agricultural	
MRH 268	1	Alm. Tamandare	A		6.560	1.640	180	110	15.850	2.550	180	120	29.700	3.520	180	140						
MRH 268	2	Aracaria	A		5.930	68.000	540	410	13.300	62.800	530	500	24.270	66.610	540	620						
MRH 268	3	Balsa Nova	A		260	3.090	370	160	570	4.750	370	210	1.020	6.510	330	250						
MRH 268	5	Campina Grande do Sul	A		1.520	660	70	80	5.210	1.600	60	100	10.750	2.440	60	130						
MRH 268	6	Campo Largo	A		5.580	6.530	310	150	9.370	12.140	280	180	14.980	15.380	250	210						
MRH 268	7	Colombo	A		12.080	4.400	350	90	27.120	7.270	320	100	49.570	9.860	310	120						
MRH 268	8	Contenda	A		490	8	290	170	780	130	290	200	1.210	190	300	230						
MRH 268	9	Curitiba	A		193.990	141.020	0	30	278.370	231.160	0	40	377.770	303.270	0	50						
MRH 268	10	Fazenda Rio Grande	A		2.650	260	240	220	9.920	440	280	270	20.850	640	260	330						
MRH 268	12	Nandirituba	A		470	90	660	890	870	150	710	1.140	1.470	210	640	1.360						
MRH 268	13	Pinhais	A		7.500	3.870	270	20	13.330	9.250	340	20	22.000	13.210	300	30						
MRH 268	14	Piraquara	A		2.050	230	900	40	3.330	560	1.010	40	5.190	800	920	50						
MRH 268	15	Quatro Barras	A		910	1.670	70	90	2.160	2.020	60	110	4.520	2.510	30	130						
MRH 268	17	Sao Jose dos Pinhais	A		12.260	17.550	860	370	27.340	27.550	820	440	49.800	38.510	750	510						
MRH 268	208	Citibá Metropolitan Area	A		252.250	244.090	5.110	2.830	407.720	362.370	5.250	3.470	613.060	463.660	4.890	4.160						
MRH 268	274	Casevel	A		20.430	8.700	550	1.010	40.040	8.230	380	1.250	60.660	8.600	260	1.450						
MRH 288	282	Foz do Iguaçu	A		28.610	1.190	170	200	63.710	1.510	50	230	103.070	1.580	20	270						
MRH 290	352	Guarapuava	A		12.330	5.870	1.550	2.530	20.070	8.810	1.790	3.110	28.790	12.930	1.890	3.700						
MRH 289	324	Dois Vizinhos	B		2.570	740	460	910	5.090	1.120	290	1.100	7.220	1.170	190	1.270						
MRH 289	327	Francisco Beltrão	B		2.150	2.680	720	1.470	4.210	5.920	420	1.830	6.610	8.300	240	2.170						
MRH 291	367	Palmas	B		5.040	4.230	1.030	1.980	10.630	6.830	310	2.450	17.590	9.810	520	2.920						
MRH 271	30	União da Vitória	B		2.730	2.060	690	2.580	8.190	1.980	550	1.500	11.820	2.840	410	1.780						
MRH 272	32	Campo do Tenente	C		4.200	2.960	260	470	5.350	4.540	250	580	6.630	4.230	300	3.800						
MRH 271	30	Quatanduba	C		210	30	860	530	310	50	970	670	410	70	1.030	810						
MRH 272	35	Porto Amazonas	C		240	20	230	170	510	10	260	210	850	10	280	250						
MRH 272	36	Rio Negro	C		260	50	60	70	370	120	80	90	500	180	80	110						
MRH 275	47	Sao Joao do Triunfo	C		2.170	6.420	430	350	3.140	9.160	460	440	4.340	12.610	470	520						
MRH 275	48	Sao Mateus do Sul	C		270	30	670	100	530	10	720	120	800	19	710	140						
MRH 276	51	Mallet	C		1.730	1.740	1.230	370	2.750	2.310	1.350	440	4.000	3.360	1.370	520						
MRH 276	53	Rebouças	C		620	250	430	510	1.180	470	390	640	1.860	690	330	750						
MRH 288	273	Capitao Leonidas Marques	C		590	70	530	220	910	100	570	270	1.310	110	580	320						
MRH 289	318	Boa Esperanca do Iguaçu	C		-490	30	420	570	790	40	170	710	950	40	80	840						
MRH 289	319	Bom Sucesso do Sul	C		50	0	220	310	110	0	150	380	170	0	90	450						
MRH 289	328	Itaperara do Oeste	C		100	0	200	590	160	10	190	720	230	10	160	870						
MRH 289	330	Jarumeleiro	C		400	120	340	850	550	190	250	1.060	730	280	150	1.250						
MRH 289	338	Kenelza	C		620	80	650	550	1.180	210	700	670	1.840	300	680	770						
MRH 289	342	Santo Inezel do Oeste	C		920	90	510	390	1.080	200	310	470	1.240	290	190	540						
MRH 289	346	Saudade do Iguaçu	C		460	20	520	510	540	40	340	630	620	50	160	730						
MRH 289	347	Sulino	C		190	70	190	360	270	110	170	450	370	160	130	540						
MRH 290	357	Quebras do Iguaçu	C		90	270	280	350	100	450	170	420	100	650	110	500						
MRH 290	358	Rio Bomito Iguaçu	C		1.720	1.450	1.040	540	2.500	2.250	910	650	3.180	3.300	830	750						
MRH 290	360	Vimond	C		70	460	320	90	90	520	280	100	100	760	250	110						
MRH 291	361	Bituruna	C		60	40	200	90	80	60	170	100	90	90	160	110						
MRH 291	363	Paula Freitas	C		620	650	500	3.810	1.110	1.190	450	4.700	1.700	1.180	390	3.700						
MRH 291	363	Paula Freitas	C		170	20	220	200	420	20	180	260	720	20	140	300						

Table-S.13 (2) Water Demand by Sector and by Region in Iguaçu River Basin [m³/day]

MNH	No.	Name	Type	Zone	1993						2005						2015					
					Urban		Rural		Urban		Rural		Urban		Rural		Urban		Rural			
					Domestic	Industrial	Domestic	Agricultural	Domestic	Industrial	Domestic	Agricultural	Domestic	Industrial	Domestic	Industrial	Domestic	Agricultural				
NRH 270	370	Porto Victoria	C	a	200	20	130	80	100	100	270	100	100	100	350	60	120	110				
NRH 271	28	Aquidões do Sul	C	b	60	10	380	360	70	20	70	20	440	440	70	30	470	530				
NRH 271	29	Plan	C	b	130	230	450	320	370	330	370	310	390	500	460	550	460	460				
NRH 276	54	Rio Azul	C	b	340	120	660	540	480	190	480	190	760	650	650	280	800	790				
NRH 283	269	Boa Vista da Aparecida	C	b	270	20	490	250	390	20	290	20	470	290	470	20	420	340				
NRH 283	275	Caranduvá	C	b	430	20	400	420	870	30	280	30	280	500	1.260	30	200	590				
NRH 283	285	Itarna	C	b	340	250	160	280	790	300	340	340	1.220	310	140	140	410	410				
NRH 283	305	Santa Lucia	C	b	160	10	190	260	250	30	460	660	670	30	310	310	760	760				
NRH 283	313	Terra Barras Paraná	C	b	340	30	700	550	550	30	600	600	320	1.490	1.020	860	190	1.090				
NRH 289	316	Amperre	C	b	600	390	470	1.180	800	600	1.170	300	550	930	980	166	166	1.090				
NRH 289	320	Caponeira	C	b	780	60	730	750	880	110	450	450	930	1.340	1.540	430	300	2.170				
NRH 289	321	Chopinazinho	C	b	840	130	780	1.480	1.170	300	300	300	550	1.340	2.210	820	430	1.160				
NRH 289	322	Coronel Vivida	C	b	1.250	250	860	820	1.710	570	660	660	990	2.210	820	430	1.160	1.160				
NRH 289	323	Cruzeiro do Iguaçu	C	b	200	200	220	800	410	650	140	320	450	240	210	210	500	500				
NRH 289	325	Encas Marques	C	b	290	60	240	280	390	120	210	210	330	510	180	180	390	390				
NRH 289	331	Nova Esperanca do Sudoeste	C	b	70	0	330	490	100	0	330	390	130	10	300	600	600	600				
NRH 289	334	Perola do Oeste	C	b	320	20	390	320	390	40	200	100	0	200	100	0	60	240				
NRH 289	335	Pinhal São Bento	C	b	50	0	150	160	80	0	110	200	100	0	60	200	640	640				
NRH 289	336	Planalto	C	b	410	20	720	470	500	40	470	560	610	60	60	200	640	640				
NRH 289	337	Trembitá	C	b	270	20	390	490	450	70	240	240	600	630	100	90	710	1.050				
NRH 289	339	Renascença	C	b	210	110	370	720	240	220	360	360	880	320	310	310	1.050	1.050				
NRH 289	340	Salgado Filho	C	b	180	10	650	630	260	30	530	770	350	40	410	380	380	380				
NRH 289	341	Saúdo do Lomira	C	b	480	20	600	1.380	760	30	380	380	1.070	30	130	130	2.370	2.370				
NRH 289	343	Santo Antonio Sudoeste	C	b	860	80	590	940	1.170	160	380	380	1.160	1.330	220	160	1.390	1.390				
NRH 289	344	São João	C	b	490	30	580	1.230	710	50	340	340	1.540	960	70	170	1.820	1.820				
NRH 289	345	São Jorge do Oeste	C	b	370	30	430	580	400	60	280	730	420	80	130	130	870	870				
NRH 289	348	Veré	C	b	280	20	500	710	450	50	360	380	640	70	200	1.050	1.050					
NRH 289	349	Vitório	C	b	260	30	260	620	370	60	210	770	490	90	150	150	910	910				
NRH 290	351	Cantagalo	C	b	710	110	590	210	2.330	200	700	700	240	6.300	290	740	290	290				
NRH 290	356	Pinhão	C	b	1.060	730	1.690	780	1.010	710	2.010	910	900	1.050	2.130	1.060	1.060	1.060				
NRH 291	362	Clevelândia	C	b	1.390	870	330	1.650	1.750	1.030	330	2.020	2.140	660	310	2.430	2.430	2.430				
NRH 291	363	Cruz Machado	C	b	270	200	1.000	980	410	840	1.090	1.180	380	900	1.110	1.360	1.360	1.360				
NRH 291	364	General Carneiro	C	b	700	580	360	320	1.530	940	380	2.570	920	270	450	450	450	450				
NRH 291	365	Monótono Serto	C	b	80	20	500	120	200	10	570	130	340	10	610	160	160	160				
NRH 291	366	Nanucrinha	C	b	500	1.290	970	360	1.180	1.210	1.080	420	2.030	1.260	1.120	490	490	490				
NRH 291	369	Paulo Frontin	C	b	170	30	350	540	310	40	410	660	470	40	440	440	440	440				
NRH 271	51	Tijuca do Sul	C	e	110	120	410	180	130	220	490	220	200	200	340	520	270	270				
NRH 272	33	Lapa	C	e	2.110	840	1.460	680	3.230	870	1.490	830	1.640	1.050	1.440	970	970	970				
NRH 275	46	Antonio Olinto	C	e	70	20	510	150	910	30	550	180	30	560	200	200	200	200				
NRH 283	276	Cru. Azul	C	e	490	870	200	460	910	1.310	200	550	1.260	1.260	40	240	830	830				
NRH 283	284	Guramaçu	C	e	680	30	430	630	1.330	40	320	760	1.330	40	240	830	830	830				
NRH 283	290	Lindóeste	C	e	70	10	380	480	70	10	240	570	50	10	160	660	660	660				
NRH 283	293	N. Interlândia	C	e	730	130	310	760	1.340	210	150	830	1.820	220	80	770	770	770				
NRH 283	306	Santa Tereza do Oeste	C	e	290	20	100	210	550	30	60	250	760	40	40	290	290	290				
NRH 283	307	Santa Terezinha Unipau	C	e	1.040	90	90	140	2.300	80	60	160	3.480	90	40	180	180	180				
NRH 283	309	São Niquel do Iguaçu	C	e	880	350	300	480	1.800	340	150	570	2.610	360	60	660	660	660				
NRH 289	317	Ibarrucano	C	e	480	20	640	320	890	40	400	380	1.360	50	160	450	450	450				
NRH 289	326	Flores da Serra do Sul	C	e	30	0	340	1.120	40	10	330	1.380	60	10	290	1.650	1.650	1.650				

Table-S.13 (3) Water Demand by Sector and by Region in Iguape River Basin [m³/day]

NRH	No.	Name	Type	Zone	1993						2003						2013					
					Urban		Rural		Urban		Rural		Urban		Rural		Urban		Rural			
					Domestic	Industrial	Domestic	Agricultural	Domestic	Industrial	Domestic	Agricultural	Domestic	Industrial	Domestic	Industrial	Domestic	Industrial	Domestic	Agricultural		
NRH 290	350	Candói	C	c	190	210	1,220	700	300	240	1,500	820	400	360	1,580	930						
NRH 290	353	Inacio Martins	C	c	270	330	810	720	370	380	1,080	890	450	560	1,140	1,060						
NRH 290	354	Laranjeiras do Sul	C	c	2,100	310	710	470	2,520	400	520	330	2,720	590	410	670						
NRH 290	355	Nova Laranjeiras	C	c	30	50	350	210	40	70	330	250	40	100	300	280						
NRH 272	34	Palmeira			0	0	190	140	0	0	210	130	0	0	220	180						
NRH 276	50	Iraí			0	0	520	200	0	0	440	230	0	0	330	280						
		Total			373,410	294,780	49,970	58,010	628,920	434,920	45,210	71,050	946,980	556,340	39,090	83,890						

CHAPTER 6 WATER RESOURCES DEVELOPMENT

6.1 Surface Water Potential

6.1.1 Zoning for Surface Water Potential

In order to compare required water supply with surface water potential, Iguaçu river basin was divided into 22 blocks as shown in Figure-6.1.

Discharge reference point was determined downstream of each block. Each reference point is the same as the point of water quality study. Surface water potential was calculated at each discharge reference point.

6.1.2 Surface Water Potential

Surface water potential was calculated by deducting maintenance discharge ($50\%Q_{10,7}$) from the low water flow ($Q_{10,7}$) at each reference point. Low water flow was applied as follows:

- (1) Curitiba metropolitan area ----- HG64 (CEHPAR,1990)
- (2) catchment area $< 5,000\text{km}^2$ ----- HG52 (CEHPAR,1982)
- (3) catchment area $\geq 5,000\text{km}^2$ ----- MINIMUM DISCHARGE VALUES FOR
THE STATIONS STUDIED BY JICA IN
PARANÁ STATE (COPEL,1995)

The result are shown in Table-6.1.

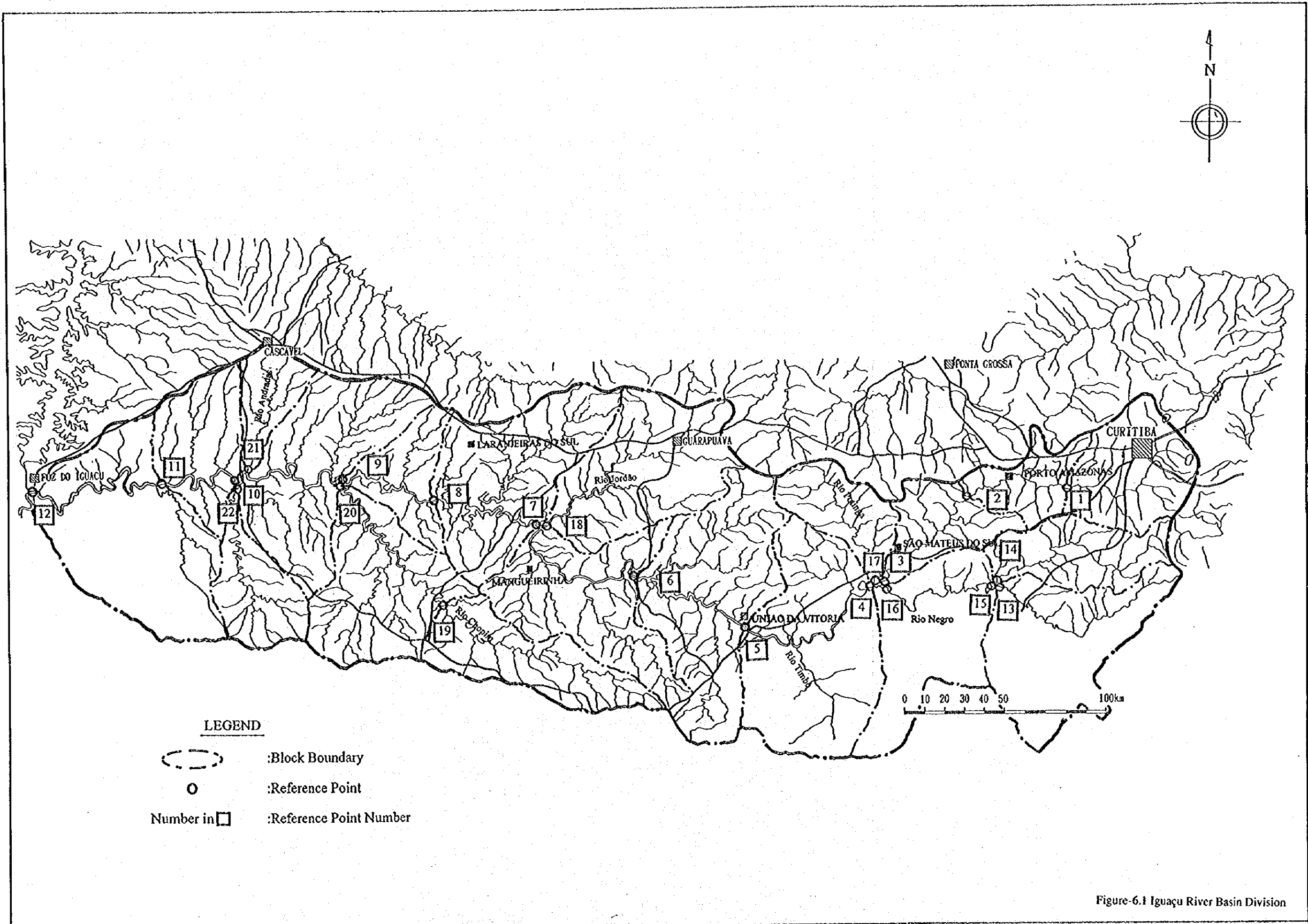


Figure-6.1 Iguazu River Basin Division

Table-6.1 Surface Water Potential and Quality

Reference Point	River Name	Location	Catchment Area (km ²)	Surface Water Quality in 1993 BOD(mg/l)	Surface Water Potential (m ³ /sec)	Required Water Supply (m ³ /sec) *		
						1993	2005	2015
						Urban	Urban	Urban
1	Rio Iguacu	Guajuvira (downstream Curitiba metropolitan Area)	2,830	13.7	3.27	8.04	11.12	15.01
						-4.77	-7.85	-11.74
						0.41	0.29	0.22
2	Rio Iguacu	south of Porto Amazonas	4,219	1.37	4.18	8.31	11.53	15.56
						-4.13	-7.35	-11.38
						0.50	0.36	0.27
3	Rio Iguacu	upstream confluence of Rio Negro	5,992	0.39	6.88	8.40	11.65	15.72
						-1.52	-4.77	-8.84
						0.82	0.59	0.44
4	Rio Iguacu	downstream confluence of Rio Potinga	18,465	0.30	20.57	8.78	12.12	16.33
						11.79	8.45	4.24
						2.34	1.70	1.26
5	Rio Iguacu	upstream Uniao da Vitoria	24,414	0.06	33.31	8.94	12.32	16.54
						24.37	20.99	16.77
						3.73	2.70	2.01
6	Rio Iguacu	Foz do Areia Dam	30,031	0.00	42.72	9.13	12.56	16.84
						33.59	30.16	25.88
						4.68	3.40	2.54
7	Rio Iguacu	downstream confluence of Rio Jordao	38,722	0.00	57.69	9.61	13.20	17.66
						48.08	44.49	40.03
						6.00	4.37	3.27
8	Rio Iguacu	Salto Santiago Dam	43,421	0.00	65.75	9.82	13.44	17.99
						55.93	52.29	47.76
						6.70	4.89	3.65
9	Rio Iguacu	downstream confluence of Rio Chopim	52,244	0.00	80.29	10.84	14.80	19.74
						69.45	65.49	60.55
						7.41	5.43	4.07
10	Rio Iguacu	downstream confluence of Rio Capanema	60,245	0.18	92.20	11.77	16.04	21.32
						80.43	76.16	70.88
						7.83	5.75	4.32
11	Rio Iguacu	route 163	62,656	0.05	95.46	11.87	16.16	21.46
						83.59	79.30	74.00
						8.04	5.91	4.45
12	Rio Iguacu	river mouth (Foz do Iguacu)	68,700	0.10	102.76	12.71	17.58	23.48
						90.05	85.18	79.28
						8.08	5.85	4.38
13	Rio Negro	upstream confluence of Rio da Varzea (include Rio Negro)	4,472	0.18	5.63	0.17	0.22	0.29
						5.46	5.41	5.35
						33.12	25.59	19.41
14	Rio da Varzea	river mouth	2,289	0.85	2.88	0.13	0.15	0.19
						2.75	2.73	2.69
						22.15	19.20	15.16
15	Rio Negro	downstream confluence of Rio da Varzea	6,761	0.41	8.50	0.30	0.37	0.48
						8.20	8.13	8.02
						28.33	22.97	17.71
16	Rio Negro	river mouth	10,639	0.07	12.65	0.31	0.38	0.49
						12.34	12.27	12.16
						40.81	33.29	25.82
17	Rio Potinga	river mouth	1,834	2.61	1.05	0.08	0.10	0.12
						0.97	0.95	0.93
						13.13	10.50	8.75
18	Rio Jordao	river mouth (include Guarapuava)	4,446	0.00	6.10	0.44	0.58	0.76
						5.66	5.52	5.34
						13.86	10.52	8.03
19	Rio Chopim	Sao Luiz (include Palmas)	3,005	0.87	4.35	0.19	0.24	0.29
						4.16	4.11	4.06
						22.89	18.13	15.00
20	Rio Chopim	river mouth (include Pato Branco, Francisco Beltrao and Dois Vizinhos)	7,076	0.00	8.16	0.93	1.25	1.62
						7.23	6.91	6.54
						8.77	6.53	5.04
21	Rio Andrade	river mouth (include Cascavel)	1,267	11.4	1.21	0.56	0.81	1.10
						0.65	0.40	0.11
						2.16	1.49	1.10
22	Rio Capanema	river mouth	1,974	2.38	2.21	0.11	0.13	0.14
						2.10	2.08	2.07
						20.09	17.00	15.79

*Remark

first line
Required Water Supply (calculated in section 6.3)
second line
Surface Water Potential - Required Water Supply
third line
Possible Development Water / Required Water Supply

6.2 Groundwater Potential

6.2.1 Definition of Boundary of Area for Groundwater Study

The major municipal urban areas located in the Iguacu river basin straddles over the boundary of other river basins. Therefore, the Iguacu river basin for the study of the groundwater resources is composed of such areas as Iguacu river basin, Karst basin on the right bank of Ribeira river, a part of the left bank of Piquiri river, and a part of Paraná III river basin including a part of the neighboring groundwater basins related to the major urban demand centers

6.2.2 Assessment of Groundwater Potential in Iguacu River Basin

The Iguacu River Basin is composed of the following aquifers in order of older age:

Karst, Crystalline Rocks (including Granitic Rocks), Furnas Formation, Upper-Middle Paleozoic, Upper Paleozoic, Botucatu Formation, Serra Geral Formation north, Serra Geral Formation south, Guabirotuba Formation.

The specific mean discharge which is defined as the specific mean of the annual minimum of average discharge of continuous 7 days (mQ_7) is used for the key data for the assessment of groundwater potential in this study. The result of the assessment of aquifers in the Iguacu River Basin is shown in Table-6.2. Assessment of Furnas Formation and Guabirotuba Formation are not able to be done by limitation of the data conditions.

Table-6.2 Spatial Groundwater Potential of Iguacu River Basin Estimated by Water Circulation

(1) Aquifer	(2) Location in River Basin	(3) Study Area km ²	(4) Spatial mQ_7 $\times 10^{-3}$ m ³ /km ²	(5) Permissible Yield		(6) Required Recharge km ² /s/m ³	(7) Total Permissible Yield m ³ /s	(8) Productivity of Borehole $\times 10^{-3}$ m ³ /s
				%	$\times 10^{-3}$ m ³ /s/km ²			
Karst	mainly Ribeira nad Upper Iguacu	3,500	8.29	30	2.49	400	8.750	44.40
Cristalline Rocks	Upper Iguacu	4,500	6.37	10	0.64	1600	2.880	5.56
Furnas Formation	Upper Iguacu	350	-	15	-	-	-	11.10
Middle-Upper Paleozoic	Upper Iguacu	3,900	4.69	10	0.47	2,100	1.830	2.78
Upper Paleozoic	Upper to Middle Iguacu	3,100	4.90	10	0.49	2,000	1.520	2.78
Botucatu Formation	Middle to Lower Iguacu	32,000	-	-	-	-	-	124.00
Serra Geral Formation north	Lower Iguacu	1,900	5.32	20	1.10	610	3.120	19.20
Serra Geral Formation south	Middle to Lower Iguacu	32,000	5.26	15	0.79	1,300	11.900	3.33
Guabirotuba Formation	Upper Iguacu	920	3.53	20	0.76	1,300	0.699	3.33

Note

[4]: Spatial and specific mQ_7

[6]: Required Rechargeing Area by 1m³/s of groundwater yield

[7]: Total Permissible Yield of Aquifer in Study Area

The characteristics of each aquifer are described as follows:

(1) Karst

Groundwater potential of Karst is evaluated to be high. The Karst area included in the Iguaçú River basin has a drainage area of about 3,500 km², and about 8.75m³/s can be developed within the permissible yield. This groundwater resource is appropriate for large scale development since its borehole productivity (borehole yield) is extremely large as 44 l/s/borehole.

The water quality of this aquifer is adequate for drinking water such as mineral water for Curitiba City, but not adequate for hydro-thermal and steam resources of industrial water resources because of its chemical characteristics.

(2) Botucatu Formation

The permissible yield of Botucatu Formation can not be estimated in this study, and it is difficult to apply the concept of permissible yield to this formation at present. Its permissible yield can be technically estimated by use of the drawdown data of groundwater table, but the drawdown data are not available. The specific mean discharge also cannot be applied to this formation because of the nature of its geologic structure.

However, the amount of its groundwater is assessed to be very large based on its extraordinarily large borehole productivity (124 l/s; in average of 9 boreholes) and storage volume. Its storage volume is assessed to be more than 20 times larger than that of Karst and a little less than 10 times of that of Serra Geral Formation.

This aquifer forms layering, and its water temperature becomes 40-70 °C at the depth of deeper than 800 m. This groundwater resource, therefore, is assessed to have high potential of industrial water use with appropriate control of pH and Na by mixing with other fresh water resources in consideration of confined water pressure, pH level and content of sodium.

(3) Serra Geral Formation north

This aquifer is broadly distributed from near Cascavel to the north, but the study area within the Iguaçú River basin is limited to the area of 1,900 km² near Cascavel. Though the spatial permissive yield and mean productivity of borehole of this formation is less than a half of those of Karst, its potential is relatively large and is assessed to be an adequate groundwater resource for medium scale development.

The water quality of this aquifer is appropriate for both domestic and industrial water supply.

(4) Serra Geral Formation south

The aquifer of Serra Geral Formation is broadly distributed with a basin area of 32,000 km² in the area middle reach to downstream of the Iguaçú River Basin. The groundwater resource of this aquifer is assessed to be appropriate for small to medium scale development based on its spatial permissive yield and productivity.

(5) Guabirota Formation

This aquifer is distributed in Curitiba metropolitan area (CMA) with a basin area of 900 km², and its groundwater resource is widely used for the domestic and industrial water in CMA. Monitoring of groundwater of this formation is required with high maneuverability because it is distributed in the urban area. It will be required to measure promptly chemical contents in response to necessity not limiting to the standard observation items for drinking water because there is a possibility of contamination of groundwater.

The total permissive yield of the whole aquifer is estimated to be about 0.7 m³/s (average of CMA). Various kind of adverse effects on the use of wells will be expected in the central urban area of Curitiba city in the near future because present groundwater use for industries is estimated to be very high in this area.

(6) Furnas Formation

The aquifer of Furnas Formation is assessed to be appropriate for small scale development based on its productivity of borehole.

(7) Other Aquifers

Groundwater development of other aquifers not aforementioned is assessed to be infeasible except for the rural areas facing shortage or lack of other fresh water sources because of its low permissive yield and productivity.

6.3 Required Water Supply Amount

6.3.1 Water Demand and Sources

Water demands are estimated for urban domestic water, rural domestic water, industrial water and agricultural water. Water source which is appropriate for each water demand seems to be basically as shown in Table-6.3, from the view point of developed amount, technology, realization, etc.

Table-6.3 Water Demand and Source

Water Demands	Region	Main Water Sources	Sub Water Sources
Domestic	Urban	Surface Water	Groundwater
	Rural	Groundwater	Surface Water
Industrial	Urban	Surface Water	Groundwater
Agricultural	Rural	Surface Water	Groundwater

Water source of urban domestic water and industrial water will be established by considering the characteristics of the region, surface water potential, groundwater potential, etc.

6.3.2 Water Losses

Required water supply amount is calculated by adding various losses to each water demand. Percentage of total water loss which includes losses for intake, conveyance, treatment, distribution of water, etc., is assumed as shown in Table-6.4 taking into consideration present loss percentage, future improvement and type of water development.

Table-6.4 Percentage of Water Losses

Purpose of Water Use	Region	1993(%)	2005(%)	2015(%)
Domestic	Urban	40	30	25
	Rural	15	10	10
Industry	Urban	15	10	10
Agriculture	Rural	20	20	20

6.3.3 Classification and Zoning of Region

The urban areas were classified into the following categories by considering characteristics of each area:

(1) Type-A: Large urban areas

The large urban areas were defined that their population will be more than approximately 100,000 in 2015.

(2) Type-B: Medium urban areas

The medium urban areas were defined that their population will be more than approximately 50,000 in 2015.

(3) Type-C: Other urban areas

The other urban areas were classified into the following zoning by considering topographic conditions:

1) Zone-a: Urban areas located nearby main streams

These areas located nearby main stream or downstream of tributaries, therefore problems of the shortage of intake rate and water quality are few.

2) Zone-b: Urban areas located upstream of second or third tributaries

Although there are problems of possible water development volume and intake method, water quality problems are quite few.

3) Zone-c: Urban areas located at top or ridge of mountains

These areas require to intake the water from the downstream of urban town, and water volume, water quality and intake method have many problems.

A hundred and one municipalities belong to Iguazu river basin, out of which 17 urban areas were classified into Type-A and other 6 urban areas were classified into Type-B. 76 belong to Type-C and 2 municipalities belong to only rural areas.

Type-C urban areas were classified into 3 zones by considering topographic conditions. Urban classification and zoning of Type-C urban areas are shown in Table-6.5.

6.3.4 Required Water Supply

Assuming water loss percentage as shown in Table-6.4, based on water demand estimated in the Section-5.4, required water supply by sector and by region was calculated for both base and alternative cases as shown in Table-6.6.

Table-6.5 Classification and Zoning of Urban Areas in Iguazu River Basin

No.	Municipality Name	Type	Zone	MRH
1-14	Curitiba Metropolitan Area	14 municipalities of	MRH 268	
1	-Alm. Imandare	A	MRH 268	
2	-Araucaria	A	MRH 268	
3	-Balsa Nova	A	MRH 268	
4	-Campina Grande do Sul	A	MRH 268	
5	-Campo Largo	A	MRH 268	
6	-Colombo	A	MRH 268	
7	-Contenda	A	MRH 268	
8	-Curitiba	A	MRH 268	
9	-Fazenda Rio Grande	A	MRH 268	
10	-Mandirituba	A	MRH 268	
11	-Pinhais	A	MRH 268	
12	-Piraquara	A	MRH 268	
13	-Quatro Barras	A	MRH 268	
14	-Sao Jose dos Pinhais	A	MRH 268	
15	-Cascavel	A	MRH 288	
16	Foz do Iguacu	A	MRH 288	
17	Guapiruvã	A	MRH 290	
18	Medianeira	B	MRH 288	
19	Dois Vizinhos	B	MRH 289	
20	Francisco Beltrão	B	MRH 289	
21	Pato Branco	B	MRH 289	
22	Palmas	B	MRH 291	
23	União da Vitória	B	MRH 291	
24	Quitandinha	C	MRH 271	
25	Campo do Tenente	C	MRH 272	
26	Porto Amazonas	C	MRH 272	
27	Rio Negro	C	MRH 272	
28	Sao Jose do Trunfo	C	MRH 275	
29	Sao Mateus do Sul	C	MRH 275	
30	Mallet	C	MRH 276	
31	Rebouças	C	MRH 276	
32	Capitao Leonidas Marques	C	MRH 288	
33	Boa Esperanca do Iguacu	C	MRH 289	
34	Boa Sucesso do Sul	C	MRH 289	
35	Irapuara do Oeste	C	MRH 289	
36	Marmeleiro	C	MRH 289	
37	Realza	C	MRH 289	
38	Santa Izabel do Oeste	C	MRH 289	
39	Saudade do Iguacu	C	MRH 289	
40	Sulina	C	MRH 289	
41	Quebras do Iguacu	C	MRH 290	
42	Rio Bonito Iguacu	C	MRH 290	
43	Virmond	C	MRH 290	
44	Bituruna	C	MRH 291	
45	Paula Freitas	C	MRH 291	
46	Porto Vitória	C	MRH 291	
47	Aguaios do Sul	C	MRH 271	
48	Piçarr	C	MRH 271	
49	Rio Azul	C	MRH 276	
50	Boa Vista da Aparecida	C	MRH 288	
51	Canaduias	C	MRH 288	
52	Ituma	C	MRH 288	
53	Santa Lucia	C	MRH 288	
54	Tres Barras Parana	C	MRH 288	
55	Ampere	C	MRH 289	
56	Capatzena	C	MRH 289	
57	Chopinzinho	C	MRH 289	
58	Coronel Vivida	C	MRH 289	
59	Cruzeiro do Iguacu	C	MRH 289	
60	Eneas Marques	C	MRH 289	
61	Mariópolis	C	MRH 289	
62	Nova Esperanca do Sudoeste	C	MRH 289	
63	Perola do Oeste	C	MRH 289	
64	Pinhal Sao Bento	C	MRH 289	
65	Planalto	C	MRH 289	
66	Pranchita	C	MRH 289	
67	Renascenta	C	MRH 289	
68	Salgado Filho	C	MRH 289	
69	Saito do Lontra	C	MRH 289	
70	Santo Antonio Sudoeste	C	MRH 289	
71	Sao Jose	C	MRH 289	
72	Sao Jorge do Oeste	C	MRH 289	
73	Vere	C	MRH 289	
74	Vionno	C	MRH 289	
75	Canizalo	C	MRH 290	
76	Pinhao	C	MRH 290	
77	Clelandia	C	MRH 291	
78	Cruz Machado	C	MRH 291	
79	General Carneiro	C	MRH 291	
80	Honorio Serpa	C	MRH 291	
81	Manguerinha	C	MRH 291	
82	Paulo Frontin	C	MRH 291	
83	Jucas do Sul	C	MRH 271	
84	Lapa	C	MRH 272	
85	Antonio Olindo	C	MRH 275	
86	Ceu Azul	C	MRH 288	
87	Guaraniau	C	MRH 288	
88	Lindoeste	C	MRH 288	
89	Marlandia	C	MRH 288	
90	Santa Tereza do Oeste	C	MRH 288	
91	Santa Tereza Itapu	C	MRH 288	
92	Sao Miguel do Iguacu	C	MRH 288	
93	Barracao	C	MRH 289	
94	Fior da Serra do Sul	C	MRH 289	
95	Nova Praia do Iguacu	C	MRH 289	
96	Candoi	C	MRH 290	
97	Inacio Martins	C	MRH 290	
98	Laranjeiras do Sul	C	MRH 290	
99	Nova Laranjeiras	C	MRH 290	
100	Palmeira	*	MRH 272	
101	Irati	*	MRH 276	

*: Urban area is not included in Iguazu river basin. Only rural area spreaded in the basin.

6.4 Water Development in Curitiba Metropolitan Area

The large urban areas included in Curitiba Metropolitan Area are as shown below:

- 1) Curitiba
- 2) Almirante Tamandare
- 3) Colombo
- 4) Piraquara
- 5) Sao Jose dos Pinhais
- 6) Araucaria
- 7) Campo Largo
- 8) Pinhais
- 9) Fazenda Rio Grande
- 10) Quatro Barras
- 11) Campina Grande do Sul
- 12) Balsa Nova
- 13) Contenda
- 14) Mandirituba

6.4.1 Water Requirement

Water requirement for urban area is mainly composed of urban domestic water and industrial water. Required water supply in Curitiba Metropolitan Area is shown in Table-6.7.

Table-6.7 Required Water Supply in Curitiba Metropolitan Area (m³/s)

Case	Required Water	Year		
		1993	2005	2015
Base Case	Required Water Supply	8.190	11.401	15.425
	Water to be newly developed		3.211	7.235

6.4.2 Process of Water Resources Development Study

As studied in the Section-6.1, there is no room for direct intake from river due to shortage of natural discharge in the upstream of Iguaçu river until confluence of Negro river. Therefore, development of new water resources has to depend on the combination of surface water by dam-reservoir and groundwater.

Process of water resources development in Curitiba Metropolitan Area was as shown below:

- (1) Water supply in Curitiba Metropolitan Areas was studied for surface water development by dams. Water development in proposed 10 dams, planned by SANEPAR around Curitiba at the upstream of Iguaçu river was studied.
- (2) Water supply in Curitiba Metropolitan Area was studied for groundwater development by wells.
- (3) The Combination of dams and wells was optimized for the water supply.

6.4.3 Surface Water Development by Dam

The water development calculation was made based on the following conditions.

- a) Assuming the daily discharge at proposed dam sites are inflow to the reservoir, daily water balance in the reservoir is simulated for 20 years.
- b) Maintenance discharge from the reservoir and downstream of intake point is assumed to be 50% of $Q_{10,7}$ and the daily discharge is to be more than the maintenance discharge.
- c) i) When inflow is less than the sum of proposed development water and maintenance discharge, deference is supplied from reservoir water.
ii) When inflow is more than the sum of proposed development water and maintenance discharge, excess of inflow is recharged to the reservoir. If the reservoir is full at that time, excess water is discharge to the downstream of dam.
- d) Evaporation from reservoir is also counted by applying average monthly evaporation data for 20 years at Piraquara observation station.
- e) Seepage or infiltration from reservoir is neglected.
- f) The maximum period of recovery is about 5 years.

The proposed 10 dams planned by SANEPAR are shown in Table-6.8 and Figure-6.2.

Table-6.8 Proposed 10 dams planned by SANEPAR

Name of Dam (River)	Dam Site				Intake Point				Treat-ment Station	Supply Reser-voir
	Catch-ment Area (km ²)	$q_{10,7}$ [m ³ /s/100km ²]	$Q_{10,7}$ 50% [m ³ /s]	Correc-tion Coeffi-cient	Catch-ment Area (km ²)	$q_{10,7}$ [m ³ /s/100km ²]	$Q_{10,7}$ 50% [m ³ /s]	Correc-tion Coeffi-cient		
1 Irai	112.6	0.355	0.200	0.781	226.6	0.408	0.460	1.807	ETA Irai	Cajura
2 Piraquara 2	58.0	0.397	0.115	0.450						
3 pequeno	62.3	0.465	0.145	0.566	110.0	0.465	0.255	1.000	ETA Iguacu	
4 Alto Miringuava	71.9	0.417	0.150	0.586	97.1	0.402	0.195	0.768	ETA BARRO PRETO	Xaxim
5 Cotia Despique	154.7	0.271	0.210	0.820	same as dam site				ETA COCHOENE	
6 Alto Mauricio	36.0	0.277	0.050	0.195	ditto					
7 Das Onças (Mandirituba)	29.0	0.276	0.040	0.156	ditto					Ceasa
8 Faxinal	63.3	0.269	0.085	0.333	ditto					
9 Das Onças (Coutenda)	75.6	0.265	0.100	0.392	ditto					Araucaria
10 Pianduva	25.4	0.276	0.035	0.137	ditto					

[Note]: daily discharge at each dam site is calculated by multiplying daily discharge at Fazendinha station by correction coefficient.

Correction Coefficient : $\alpha = C.A./110.0 \times q_{10,7}/0.465$

110.0 : C.A. of Fazendinha

$q_{10,7}$ value was calculated by HG64

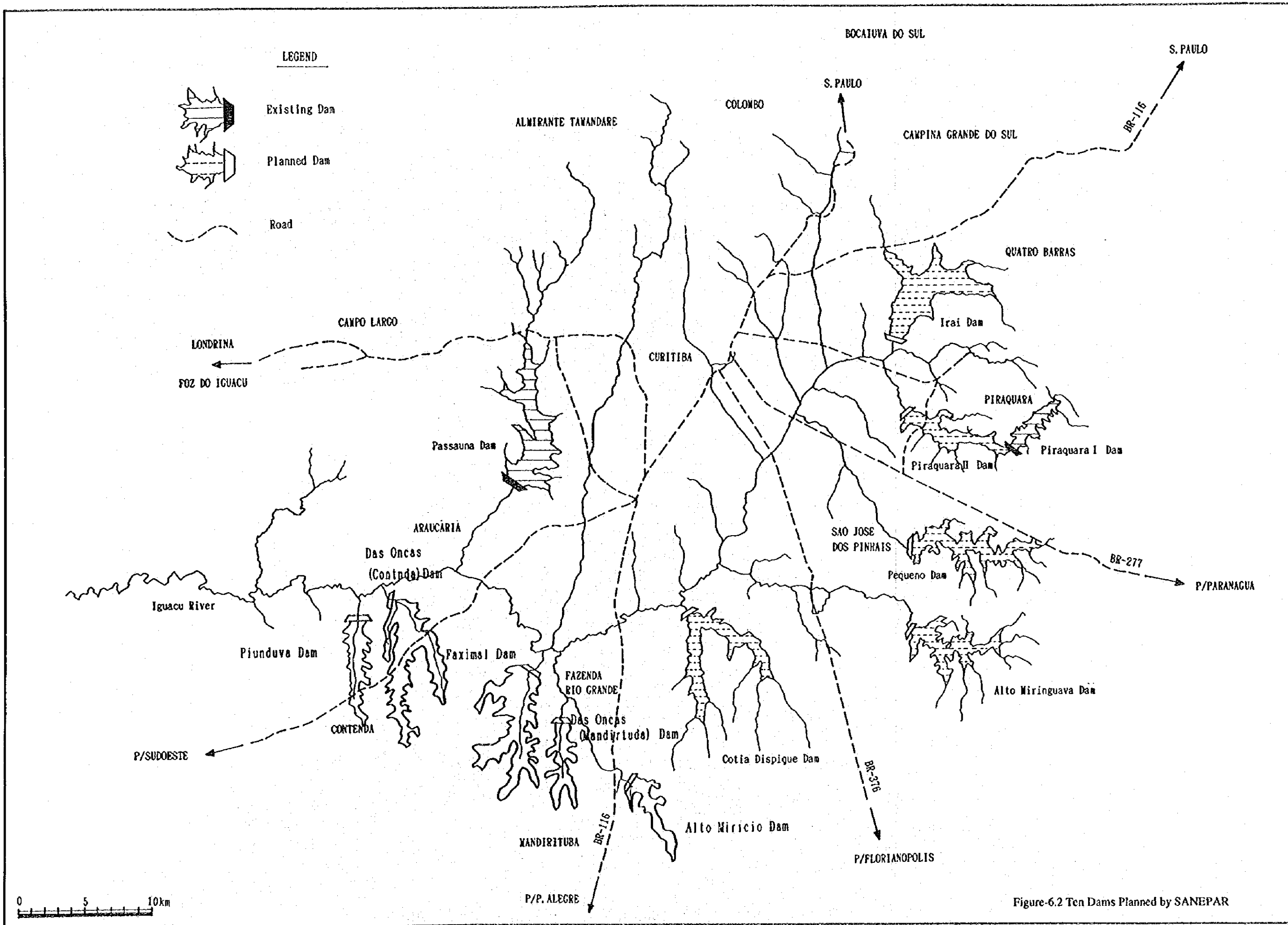


Figure-6.2 Ten Dams Planned by SANEPAR

Several cases of proposed water development volume range from 0.10 m³/sec to 1.40 m³/sec were assumed at each proposed dam. Simulation of daily water balance in reservoir was carried out for 20 years. The result of simulation are shown in Table-6.9.

For example, judging from required recovery period of reservoir capacity, an appropriate water development volume by Pequeno dam(No.3) seems to be 0.8m³/sec.

The water development volume by dams around Curitiba is as shown in Table-6.9.

Table-6.9 Developed Water and Required Reservoir Capacity by Planned Dam

Name of Dam	Development Water (m ³ /sec)	Reservoir Capacity (x 10 ⁶ m ³)	Period of Recovery (Year)
1	Irai	1.30	47.9
		1.40	52.4
		1.50	57.9
		1.60	63.6
2	Piraquara II	0.70	15.3
		0.75	21.9
		0.80	36.7
		0.85	47.7
3	Pequeno	0.60	15.5
		0.70	20.1
		0.80	25.9
		0.90	33.0
4	Alto Miringuava	1.00	48.2
		0.40	5.2
		0.50	14.4
		0.60	19.1
5	Cotia Despique	0.70	27.6
		0.80	46.4
		1.00	27.6
		1.10	32.9
6	Alto Mauricio	1.20	48.2
		1.30	47.2
		1.40	-
		0.15	3.0
7	Das Onças (Mandirituba)	0.20	5.3
		0.25	7.8
		0.30	11.6
		0.36	-
8	Faxinal	0.10	1.7
		0.15	3.5
		0.20	5.8
		0.25	9.7
9	Das Onças (Contenda)	0.30	5.2
		0.40	9.8
		0.50	14.9
		0.60	25.0
10	Pianduva	0.40	7.2
		0.50	11.8
		0.60	16.8
		0.70	25.3
Total		0.80	-
		0.10	1.8
		0.15	4.0
		0.20	6.7
Total		6.50	210.0

[Note] "-": It means that capacity is not recovery.

Several cases of proposed water development volume range from 0.10 m³/sec to 1.40 m³/sec were assumed at each proposed dam. Simulation of daily water balance in reservoir was carried out for 20 years. The result of simulation are shown in Table-6.9.

For example, judging from required recovery period of reservoir capacity, an appropriate water development volume by Pequeno dam(No.3) seems to be 0.8m³/sec.

The water development volume by dams around Curitiba is as shown in Table-6.9.

Table-6.9 Developed Water and Required Reservoir Capacity by Planned Dam

Name of Dam	Development Water (m ³ /sec)	Reservoir Capacity (x 10 ⁶ m ³)	Period of Recovery (Year)	
1	Irai	1.30	47.9	2.0
		1.40	52.4	5.5
		1.50	57.9	9.0
		1.60	63.6	-
2	Piraquara II	0.70	15.3	4.5
		0.75	21.9	5.5
		0.80	36.7	10.0
		0.85	47.7	-
3	Pequeno	0.60	15.5	2.0
		0.70	20.1	4.0
		0.80	25.3	5.5
		0.90	33.0	-
4	Alto Miringuava	1.00	48.2	-
		0.40	5.2	2.0
		0.50	14.4	3.0
		0.60	19.4	5.5
5	Cotia Despique	0.70	27.6	-
		0.80	46.4	-
		1.00	27.6	3.0
		1.10	32.9	4.5
6	Alto Mauricio	1.20	38.9	5.5
		1.30	47.2	-
		1.40	-	-
		0.15	3.0	1.5
7	Alto Mauricio	0.20	5.3	2.0
		0.25	7.8	5.0
		0.30	11.6	-
		0.36	-	-
8	Das Onças (Mandirituba)	0.10	1.7	1.5
		0.15	3.5	2.0
		0.20	5.8	4.5
		0.25	9.7	-
9	Das Onças (Contenda)	0.30	5.2	1.5
		0.40	9.8	2.0
		0.50	14.9	5.5
		0.60	25.0	-
10	Piundava	0.70	7.2	2.0
		0.80	11.8	2.5
		0.60	16.8	5.0
		0.70	25.3	-
Total		0.40	7.2	2.0
		0.15	4.0	2.0
		0.20	6.7	5.5
		0.25	-	-
	0.30	-	-	
	6.50	210.0		

[Note] "-": It means that capacity is not recovery.

6.4.4 Groundwater Development by Wells

The aquifers in and around the Curitiba Metropolitan Area are Crystalline Rock, Guabirota Formation and Karst. The aquifer being targeted for groundwater development is the Karst aquifer, which is the most productive of the three.

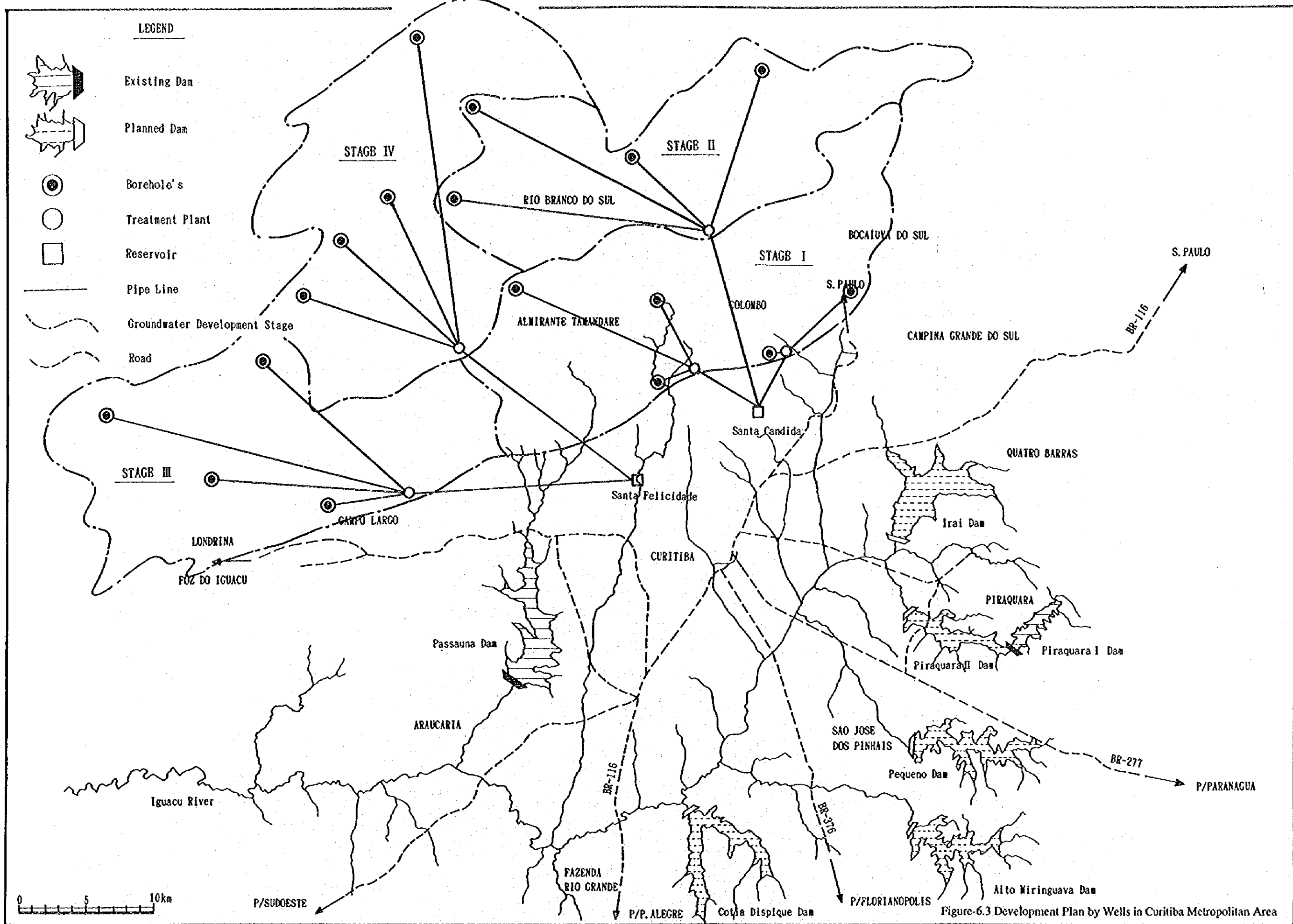
The Karst aquifer is located some 10 to 50 km to the north of Curitiba, its influence area is 400 km²/m³/s and its permissive yield per well is 0.044 m³/s/well.

The survey for the groundwater development of the Karst aquifer was carried out over four stages using the development of approximately 1 m³/s as one unit. The four stages have been numbered 1 to 4 in order starting from the easiest one to develop. Table-6.10 and Figure-6.3 provide production data and indicate the locations of the wells in the aquifer respectively.

Table-6.10 Productivity of Karst aquifer

No.	Number of Productive Boreholes	Productivity (m ³ /s)	Stage	Number of Productive Boreholes	Productivity (m ³ /s)	Influence Area (km ²)
1	9	0.40	1	29	1.20	480
2	5	0.20				
3	5	0.20				
4	3	0.10				
5	7	0.30				
6	6	0.25	2	24	1.00	400
7	6	0.25				
8	6	0.25				
9	6	0.25				
10	6	0.25	3	24	1.00	400
11	6	0.25				
12	6	0.25				
13	6	0.25				
14	6	0.25	4	24	1.00	400
15	6	0.25				
16	6	0.25				
17	6	0.25				
Total				101	4.20	1680

{Note} borehole depth is 60 m
permissive yield is 0.044m³/s
influence area is 400 km²/m³/s



6.4.5 Optimization of the Water Supply System

As was mentioned earlier, whereas the required water supply in the Curitiba Metropolitan Area is 7.235 m³/s, the maximum available amount that can be developed from the ten dams is just 6.50 m³/s, meaning that it will be difficult to meet the demand through surface water alone. Moreover, as the amounts of water developed from the groundwater of the Karst aquifer increase, the feeding of water over long distances will lead to an increase in the unit water costs (rates) of development. For these reasons, it is necessary to ensure the water supply in the Curitiba Metropolitan Area through a combination of both surface water development by dams and groundwater development by the Karst aquifer.

The features of surface water development by dam in contrast to groundwater development are as described in Table 6.11.

Table-6.11 Comparison of Surface Water Development by Dam and Groundwater Development

Item	Surface Water Development by Dam	Groundwater Development
① Stable water intake (certainty of available water)	As the development plan has been formulated based on the results of statistically processing materials relating to water, which have been collected over a long period, and performing simulations using actual daily flow rates, the degree of certainty regarding the design water intake is high.	The monitoring of groundwater has been implemented in recent times, however, compared to the data on surface water, that relating to groundwater is lacking in terms of the length of period and size of area. Moreover, it cannot be said that a full understanding has been gained of the potentially available groundwater quantities and of the effects of development on surrounding ground and surface water quantities. It is therefore less certain that the design water intake can be secured, compared to the case of surface water development.
② Water quality	In order to preserve the quality of the water in the dam reservoirs, development and improvement of the sewerage systems in upstream towns and the taking of measures to counter eutrophication in the reservoirs will be necessary.	As the Karst aquifer contains hard water, careful thought will need to be given to its specific uses in the case where it is used as industrial water.
③ Construction period	As large-scale works will be necessary, the construction period will be relatively long.	The boring of wells will not take such a long time, however, the laying of pipe lines will be slightly more time consuming than the construction of dams.
④ Environmental impact	As reservoirs will be constructed artificially, it will be necessary to formulate a detailed plan upon first understanding the effects on ecological systems, the surrounding residents and water quality, etc.	Little direct effect on the surface environment can be expected, however, planning will have to take into consideration ground subsidence and the effects on other wells. As groundwater also acts as a source of surface water, it will also be necessary to amply consider the effects the development will have on the flows of downstream rivers.
⑤ Development cost	In the case of dam development, the cost will vary depending on the topographical conditions of the dam sites and the flow conditions of the rivers. Similarly, in the case of groundwater development, cost will vary depending on the distances between the development sites and supply areas, and also on the topographical conditions in the surrounding areas. It is therefore difficult to make sweeping statements about which form of development is the cheaper.	

Regarding the optimal combination of surface water development by dams and groundwater development in order to fulfill the water requirement of Curitiba Metropolitan Area, setting will be done upon conducting the following examinations based on the characteristics described in Table 6.11.

- (1) Development costs will be calculated for each stage of both the dam development and the groundwater development, and the cheapest combination in the case where both are combined will be adopted.
- (2) The order of priority of the development will be decided in consideration of the required construction periods and the exploitable water quantities, etc.

The costs involved in the development of each dam and wells in each stage are as indicated in Table-6.11. The relationship between development costs when dam development and groundwater development are combined in order to develop the required water supply of 7.235 m³/s is as shown in Figure-6.4.

Table-6.12 Development Cost

Water Source	Supply Reservoir	Name of Dam	Development Volume (m ³ /s)	Cost (10 ⁶ US\$)	Unit Cost (10 ⁶ US\$/m ³ /s)	
	Well Field Zone	Number of Wells				
Surface Water	Cajura	Irai	1.400	49.3	35.2	
		Piraquara 2	0.750	22.0	29.3	
		Pequeno	0.800	28.6	35.8	
	Xaxim	Alto Mirinquava	0.600	35.3	58.8	
		Cotia Despique	1.200	43.8	36.5	
		Alto Mauricio	0.250	20.0	80.0	
	Ceasa	Das Oncas (Mandirituba)	0.200	25.4	127.0	
		Faxinal	0.500	25.5	51.0	
	Araucaria	Dos Oncas (Contenda)	0.600	23.1	38.5	
		Piunoluva	0.200	17.4	87.0	
	Groundwater	Stage 1	29	1.290	40.3	31.2
		Stage 2	24	1.066	51.3	48.1
Stage 3		24	1.066	53.7	50.4	
Stage 4		24	1.066	54.9	51.5	

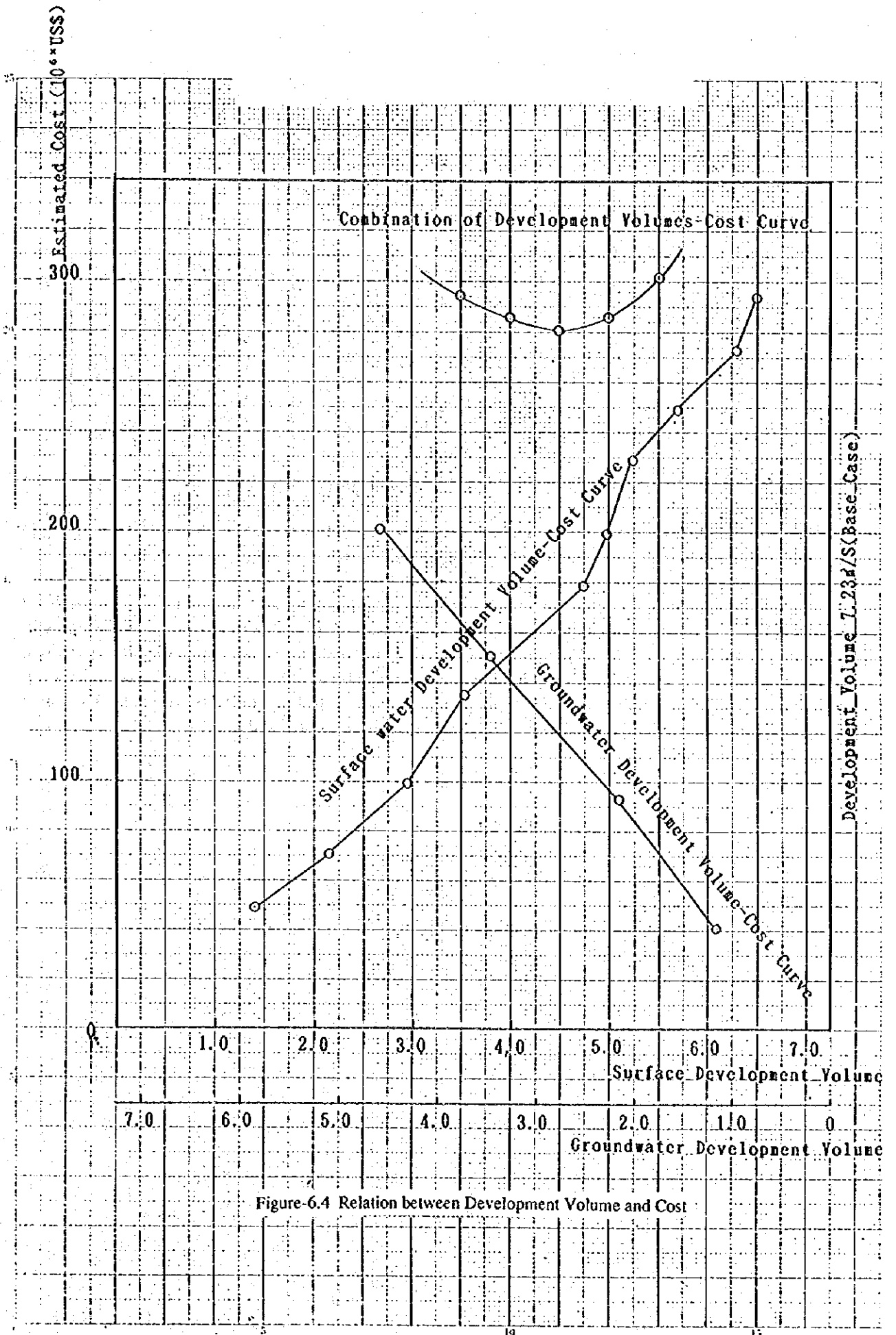


Figure-6.4 Relation between Development Volume and Cost

Based on Table-6.12 and Figure-6.4, the optimum use of dams and groundwater in terms of development cost is shown in Table-6.13. The water supply system in Curitiba metropolitan area is shown in Figure-6.5.

Table-6.13 Optimization of Water Supply

Name of Water Resource	Constructions	Development Volume (m ³ /s)	Cost (10 ⁶ US\$)
Irai Dam	dam, pipeline (Ø 1,200 x 15,000 m)	1.400	49.3
Piraquara II Dam	dam, pipeline (do.)	0.750	22.0
Pequeno Dam	dam, pipelin (Ø 800 x 8,000 m)	0.800	28.6
Alto Miringuava Dam	dam, pipeline (Ø 900 x 23,500 m)	0.600	35.3
Cotio Despique	dam, pipeline (Ø 900 x 17,000 m)	1.200	43.8
wells (stage 1)	29 wells, pipeline	1.290	40.3
wells (stage 2)	27 wells, pipeline	1.195	57.5
Total		7.235	276.8

6.4.6 Implementation Schedule of Water Development

The development schedule in the case of the combination of dam development and groundwater development shown in Table-6.13 is as illustrated below.

Dam construction will take a relatively long time until completion, however, once completed it will be possible to obtain large amounts of water. The development of groundwater will take less time compared to dam. As 5 dams will be constructed over 20 years, one dam will be built every 4 years. Groundwater development will be implemented in the period during dam construction in order to supplement the water supply. Figure-6.6 gives a detailed representation of the implementation schedule of the development.

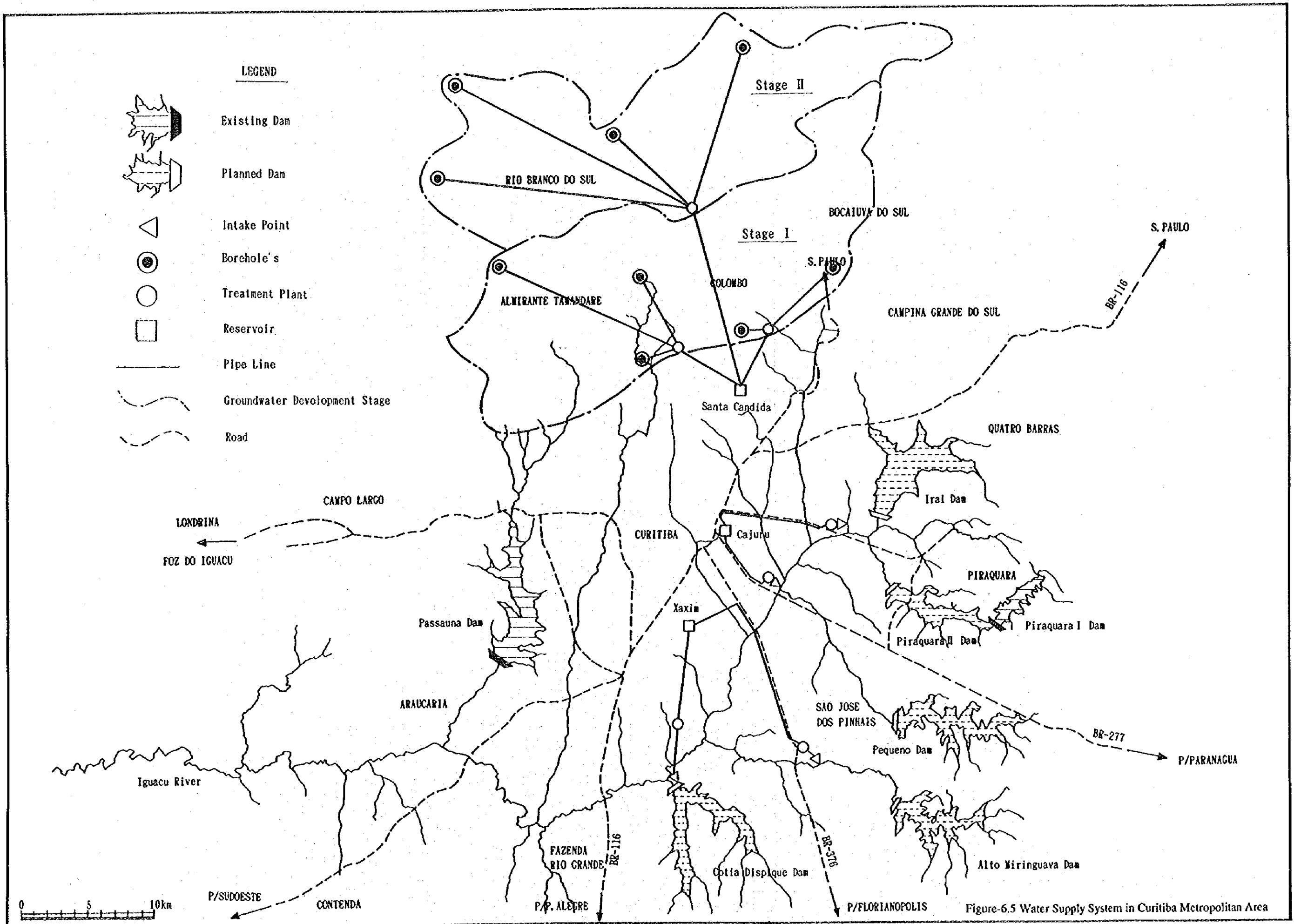
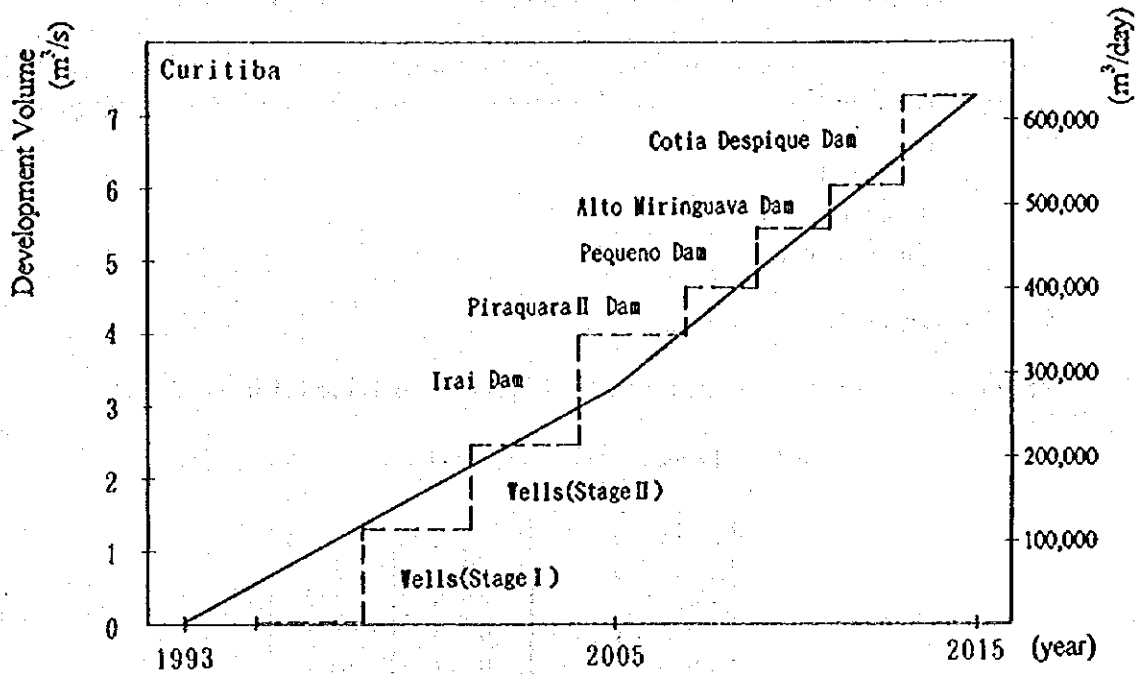


Figure-6.5 Water Supply System in Curitiba Metropolitan Area



Wells (Stage I)	110.6			
Wells (Stage II)	157.9			
Irai Dam		135.4		
Piraquara II Dam			60.4	
Pequeno Dam				78.5
Alto Miringuava Dam				96.9
Cotia Despique Dam				120.3
Total : 760.0 million us\$	215.9 [28%]	218.2 [29%]	211.4 [28%]	114.5 [15%]

Figure-6.6 Implementation Schedule of Curitiba Metropolitan Area

6.5 Water Development in Large Urban Areas (Type-A)

The urban areas were defined that their population will be more than approximately 100,000 in 2015. The following urban areas belong to Type-A as large urban areas in Iguazu river basin except for Curitiba Metropolitan Area.

- Cascavel
- Foz do Iguazu
- Guarapuava

6.5.1 Water Requirement

Required water supply in large urban areas is shown in Table-6.14.

Table-6.14 Required Water Supply in Large Urban Areas (m³/s)

Municipality	Year	
	2005	2015
Cascavel	0.268	0.542
Foz do Iguazu	0.504	1.043
Guarapuava	0.127	0.292

[Note] Water requirement for urban area is mainly composed of urban domestic water and industrial water.

6.5.2 Process of Water Resources Development Study

The process of water resources development in large urban areas is as shown below.

- (1) In cities where main rivers are nearby and direct intake is easy, water supply shall be secured through surface water development.
- (2) In cases where development by means of direct intake is difficult, careful consideration shall be given to the ease of development to the development capacity and the development cost, etc. for both surface water and groundwater.
- (3) Regarding the development of surface water, more detailed examination shall be made on the promising alternatives stated in the Strategy (Main Report I) upon consideration of the local survey results and the state of existing facilities.
- (4) Examination shall be given to the case where the whole water supply is provided by groundwater development.
- (5) Based upon the examination results of (3) and (4), the optimal development plan shall be formulated upon first giving careful consideration to the conditions stated in (2).

6.5.3 Water Resources Development Policies

Based upon consideration of the topographical conditions in Type-A cities and the surface water and groundwater conditions in the target areas, the water resources development policies as shown in Table-6.15 were decided upon.

Table-6.15 Water Resources Development Policies for Large Urban Areas

City	Topographical Condition	State of Water Resources		Water Resource Development
		Surface Water	Groundwater	
Cascavel	Cascavel is situated in the mountains within the basins of the Iguaçu, Piquiri and Paraná.	As the water resources are the rivers that flow down from the mountains, the water intake points must be placed downstream in order to expand their catchment area, meaning that the pipe lines will become very long. As the catchment area is small, the construction of a dam will be necessary.	The town is surrounded by the Serra Geral Formation north aquifer and below that the Botucatu Formation aquifer, and the productivity levels in both of these are high.	As the city has a large water requirement and the development of surface water is not easy, the development plan shall be formulated upon first examining the potential of both surface water development and groundwater development.
Foz de Iguacu	This city is situated at the mouth of the Iguaçu river next to the reservoir of the Itaipu Dam.	The city currently obtains its water from the reservoir of Itaipu Dam. Compared to the water quantity of Paraná river, the necessary water requirement is very small.	Same as above (however, change north to south)	Development will involve improving the intake facilities from Itaipu Dam and the pipe line facilities for taking water from Paraná river.
Guarapuava	This city is situated in the upper reaches of the Jardao river, which is a right tributary of the Iguaçu river. It is situated 20-30 km from the mountain tops.	A river with a catchment area of 700 km ² runs nearby the city and direct intake development is feasible.	The Serra Geral Formation south aquifer is located around the town, however, the productivity of the existing wells is low.	In view of the fact that direct intake development is easy and the groundwater productivity is low, the development will be performed on the surface water resources.

6.5.4 Water Supply System in Large Urban Areas

The water supply systems that should be promoted in large urban areas are as shown in Table-6.16 below.

Table6-.16 Water Supply System Recommended in Large Urban Area

City	Water Supply System	Constructions	Catchment Area or Well Number	Development Volume (m ³ /s)	Cost (10 ⁶ US\$)
Cascavel	direct intake from Sao Jose river	pumps, pipeline (Ø 300 x 13,000 m x 2)	145.0km ²	0.300	14.2
	wells (Serra Geral F. aquifer) and (Botucatu F. aquifer)	wells, pipeline (Ø 400 x 11,000 m) wells, pipeline (Ø 300 x 8,000 m)	9 boreholes 1 boreholes	0.180 0.120	24.7
Foz do Iguaçu	direct intake from Paraná river	pumps, pipeline (Ø 500 x 1,900 m x 3)	-	1.043	11.1
Guarapuava	direct intake from Bananas river	pumps, pipeline (Ø 300 x 4,800 m x 2)	704.0km ²	0.292	9.1

The intake points and pipe lines for each city are as illustrated in Figure-6.7 - 6.9.

6.5.5 Implementation Schedule of Water Development

The implementation schedules for each city are as shown in Figure-6.10.

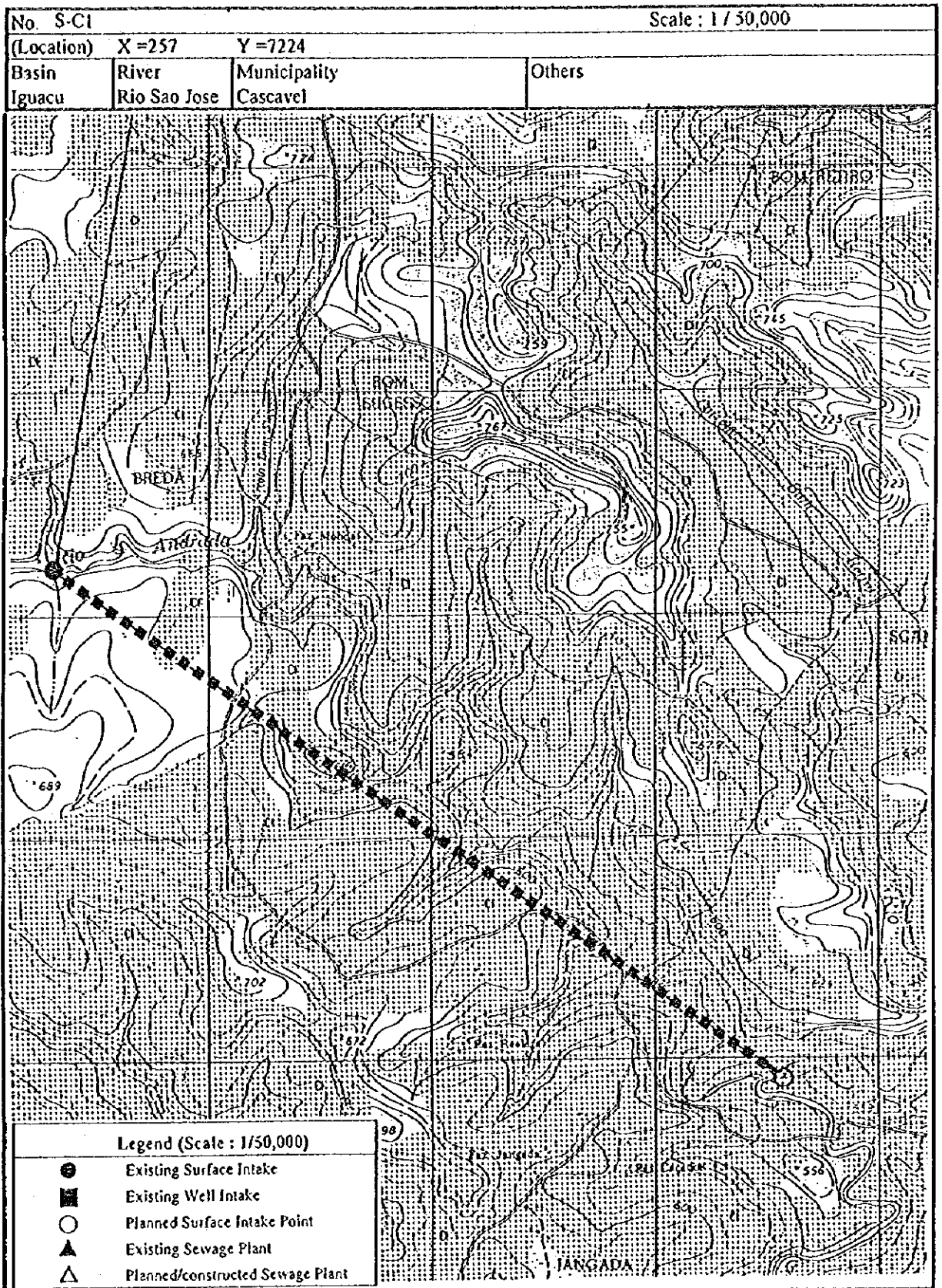


Figure-6.7 (1) Water Supply System by Surface Water in Cascavel

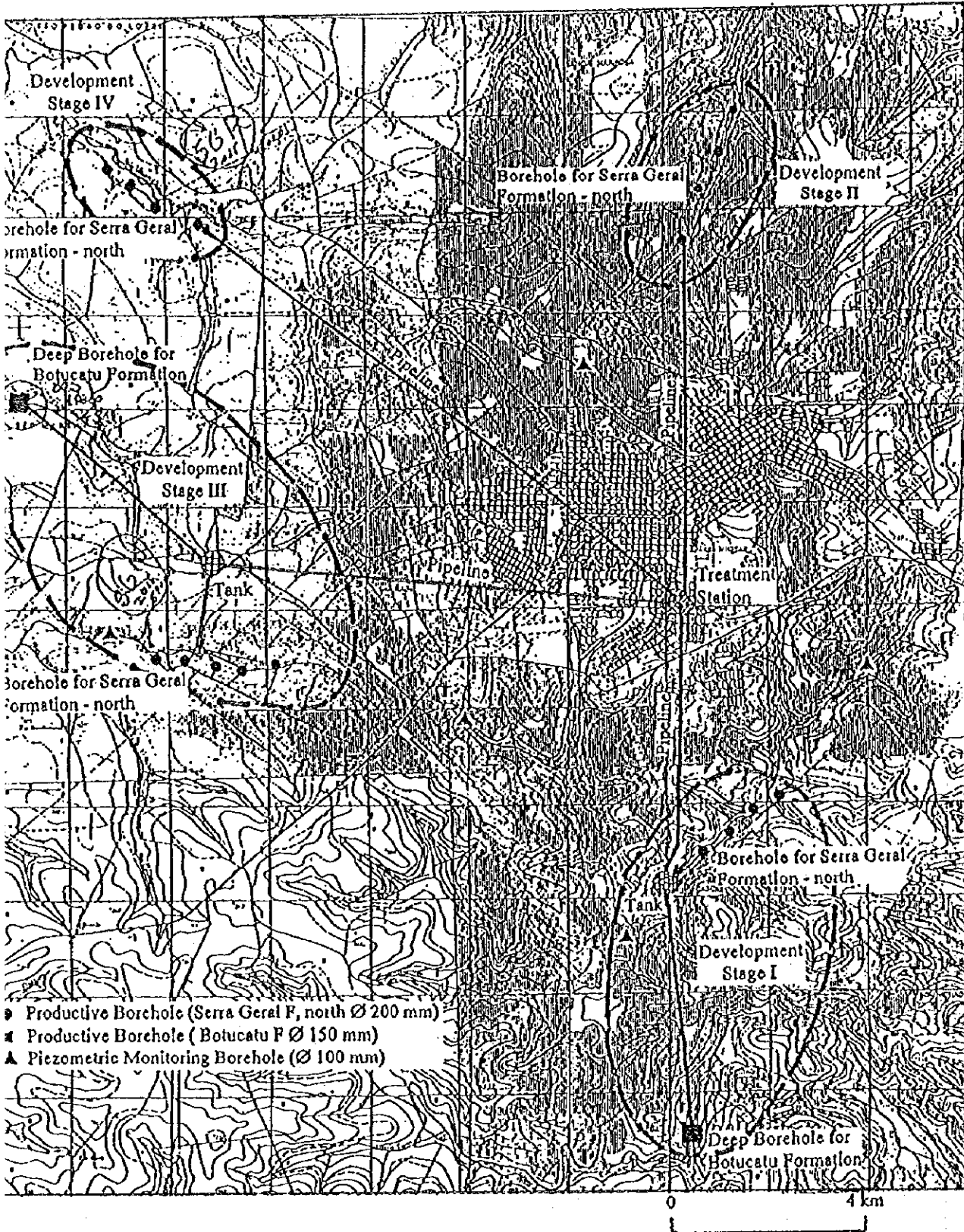


Figure-6.7 (2) Water Supply System by Groundwater in Cascavel (using stage I and II)

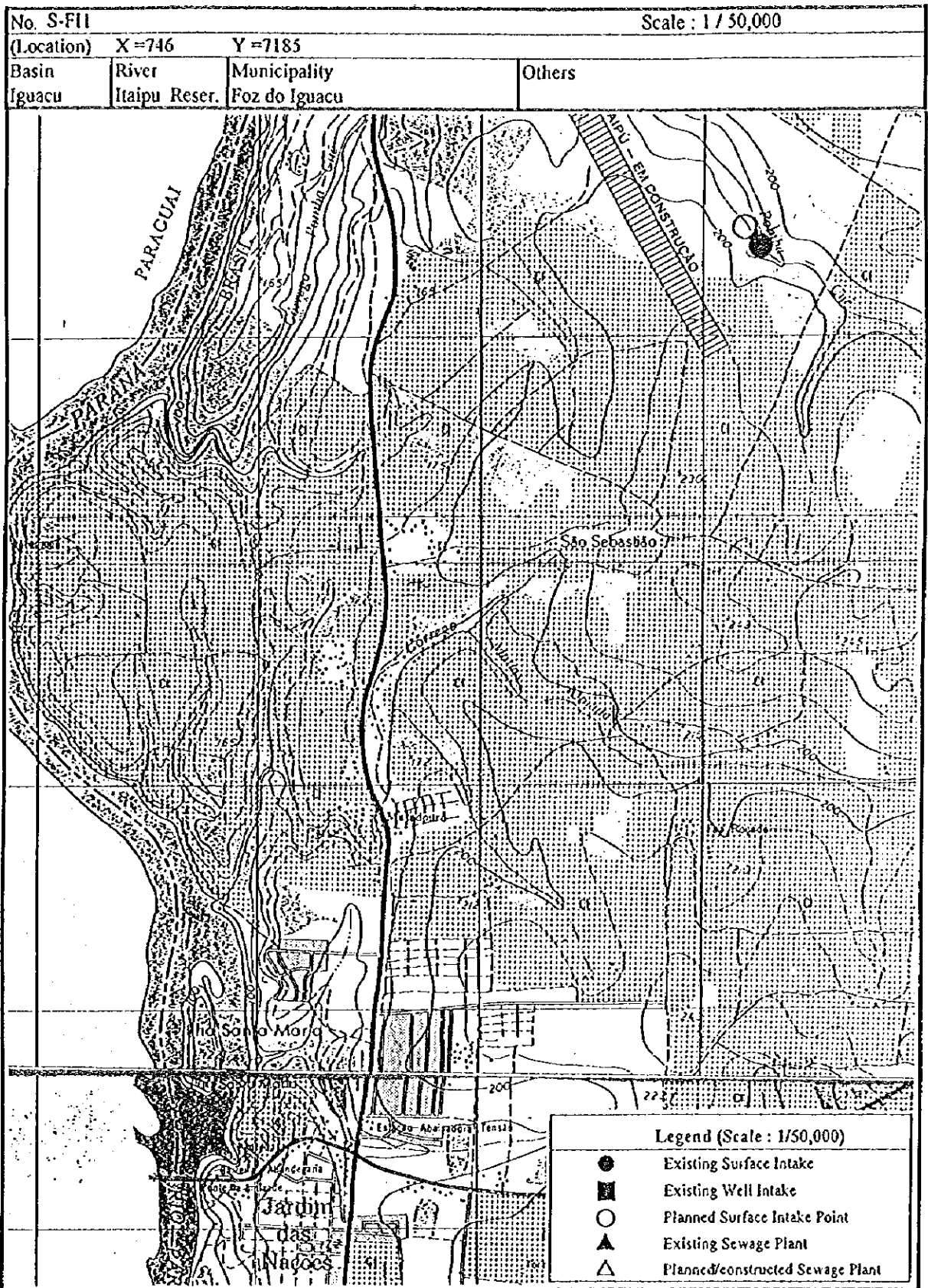


Figure-6.8 Water Supply System in Foz do Iguacu

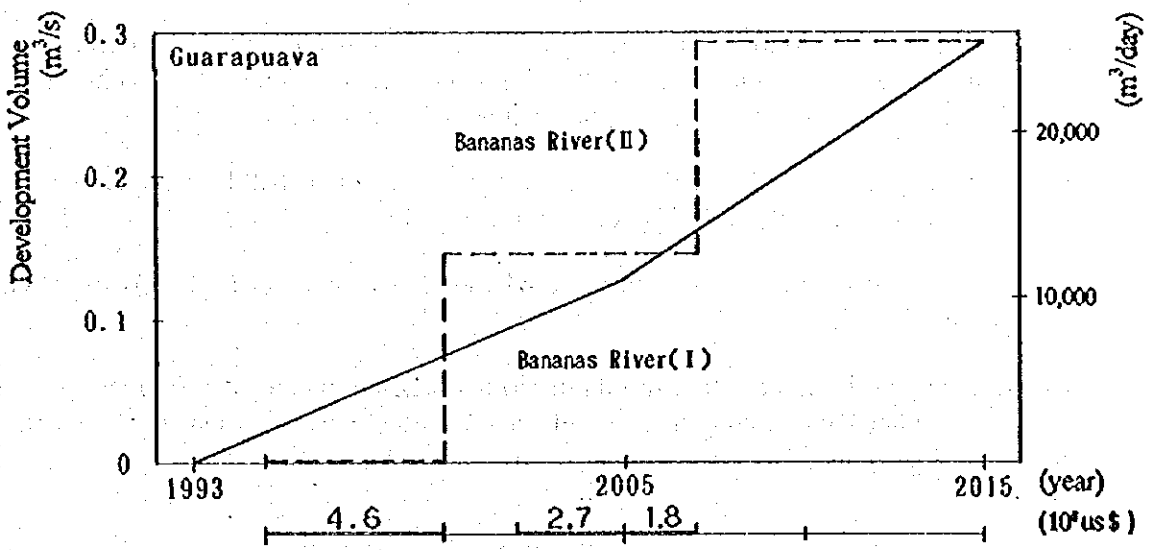
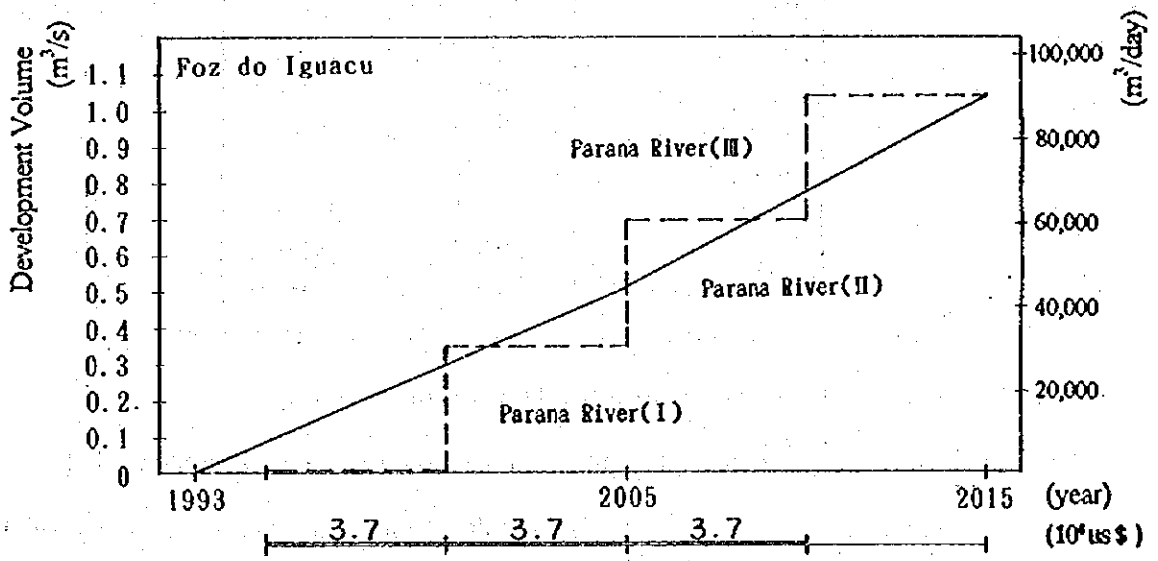
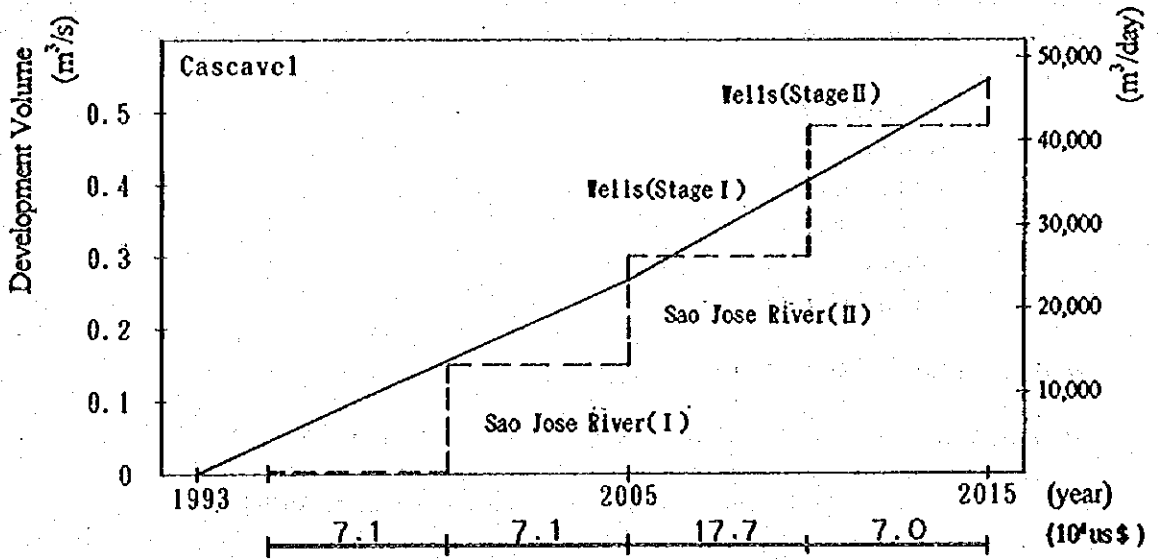


Figure-6.10 Implementation Schedule of Large Urban Area

6.6 Water Development in Medium Urban Areas (Type-B)

The urban areas were defined that their population will be more than approximately 50,000 in 2015. The following urban areas belong to Type-B as medium urban areas in Iguaçú river basin.

- Francisco Beltrao
- Pato Branco
- Medianeira
- Dois Vizinhos
- Palmas
- Uniao da Vitoria

6.6.1 Water Requirement

Required water supply in medium urban areas was shown in Table-6.17.

Table-6.17 Required Water Supply in Medium Urban Areas (m³/s)

Municipality	Year	
	2005	2015
Francisco Bertrao	0.098	0.231
Pato Brauco	0.053	0.112
Mediuneira	0.038	0.066
Dois Vizinhos	0.061	0.164
Palmas	0.028	0.065
Uniao da Vitoria	0.025	0.035

[Note] Water requirement for urban area is mainly composed of urban domestic water and industrial water.

6.6.2 Process of Water Resources Development Study

The process of water resources development in medium urban areas is as shown below.

- (1) In cities where main rivers are nearby and direct intake is easy, water supply shall be secured through surface water development.
- (2) In cases where development by means of direct intake is difficult, careful consideration shall be given to the ease of development, the development capacity and the development cost, etc. for both surface water and groundwater.
- (3) Regarding the development of surface water, examination shall be made based on the topographical conditions and water resource materials.
- (4) Examination shall be given to the case where the whole water supply is provided by groundwater development.
- (5) Based upon the examination results of (3) and (4), the optimal development plan shall be formulated upon first giving careful consideration to the conditions stated in (2).

6.6.3 Water Resources Development Policies

Based upon consideration of the topographical conditions in Type-B cities and the surface water and groundwater conditions in the target area, the water resources development policies as shown in Table-6.18 were decided upon.

Table-6.18 Water Resources Development Policies for Medium Urban Areas

City	Topographical Condition	State of Water Resources		Water Resources Development
		Surface Water	Groundwater	
Francisco Beltrao	This city is situated in the mid-stream of a tributary to the Iguacu river and 30-40 km downstream from the mountain tops.	A river with a catchment area of 400 km ² runs nearby the city and direct intake development is feasible.	The Serra Geral Formation south aquifer is located around the town, however, the productivity of the existing wells is low.	In view of the fact that direct intake development is easy, the development will be performed on the surface water resources.
Pato Branco	This city is situated in the upper reaches (near the mountain tops) of a tributary to the Iguacu river.	Development through the direct intake of water from the river running nearby the town is feasible.	The Serra Geral Formation south aquifer is located around the town, and the productivity of the existing wells is high.	As the development of both surface water and groundwater is easy, the development plan will be formulated upon first examining both possibilities.
Medianeira	This city is situated on the ridge that separates the Iguacu river and the Parana river.	As the city is situated on a ridge, it is difficult to obtain large amounts of water from just one intake point, so intake will need to be performed at a number of locations.	Same as above	The water supply plan that combines both surface water development and groundwater development will be formulated.
Dois Vizinhos	This city is situated in the mid-stream of a tributary to the Iguacu river. The catchment area of the nearby river is small at less than 100 km ² .	The water demand cannot be satisfied solely through the intake of water from the city's surrounding small rivers. If water was taken from Chopim river, the demand for water would be satisfied, however, the pipe line length would be 210 km.	Same as above	If taking water from Chopim river is effective, the water supply can be secured through development of surface water alone, however, if this turns out to be unrealistic a supply plan that combines both surface water development and groundwater development
Palmas	This city is situated in the upper reaches (near the mountain tops) of a tributary to the Iguacu river.	Development through the direct intake of water from the river running nearby the city is feasible.	Same as above	As the direct intake development of surface water is easy, the river running nearby the city will be developed as the water supply source.
Uniao da Vitoria	This city is situated in the mid-stream of the mainstream Iguacu river.	The direct intake of water from the Iguacu river mainstream is possible.	The Upper Paleozoic aquifer is located around the town, however, the productivity of existing wells is low and the permissive yield is small.	As the direct intake of surface water is easy and the nearby aquifer is not suited to groundwater development, surface water will be developed as the water supply source.

6.6.4 Water Supply System in Medium Urban Areas

Water supply systems proposed for medium urban areas are shown in Table-6.19.

Table-6.19 Proposed Water Supply System for Medium Urban Areas

City	Water Supply System	Constructions	Catchment Area or Well Number	Development Volume (m ³ /s)	Cost (10 ⁶ US\$)
Francisco Beltrao	direct intake from Marrecas river	pumps, pipeline (Ø 300 x 700 m x 2)	437.0km ²	0.231	4.7
Pato Branco	direct intake from Chopim river	pump, pipeline (Ø 300 x 12,500 m)	2817.0km ²	0.112	9.1
	(Alternative) Wells (Botucatu F.aquifer)	wells, pipeline (Ø 300 x 6,000 m)	1 borehole	0.124	(8.1)
Medianeira	Wells(Botucatu F.aquifer)	well, pipeline (Ø 300 x 4,000 m)	1 borehole	0.124	4.3
Dois Vizinhos	direct intake from Chopim river	pump, pipeline (Ø 300 x 7,500 m)	4050.0km ²	0.134	9.1
	(Alternative) Wells (Serra Geral F.aquifer) and (Botucatu F.aquifer)	wells, pipeline (Ø 300 x 6,000 m)	3 boreholes 1 boreholes	0.012 0.124	(10.3)
Palmas	direct intake from Caldeiras river	pump, pipeline (Ø 200 x 3,400 m)	83.7km ²	0.065	4.9
Uniao da Vitoria	direct intake from Iguacu river	pump, pipeline (Ø 200 x 200 m)	24.414km ²	0.035	3.7

6.6.5 Implementation Schedule of Water Development

Implementation schedule of water development is shown in Figure-6.11 (1)-(2).

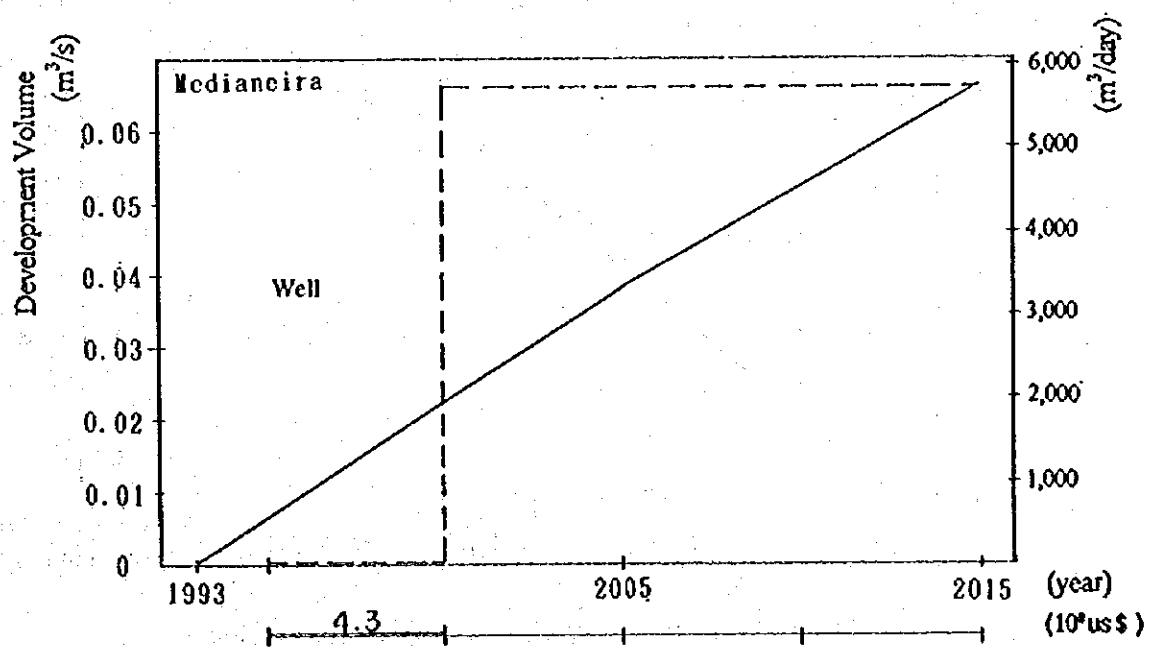
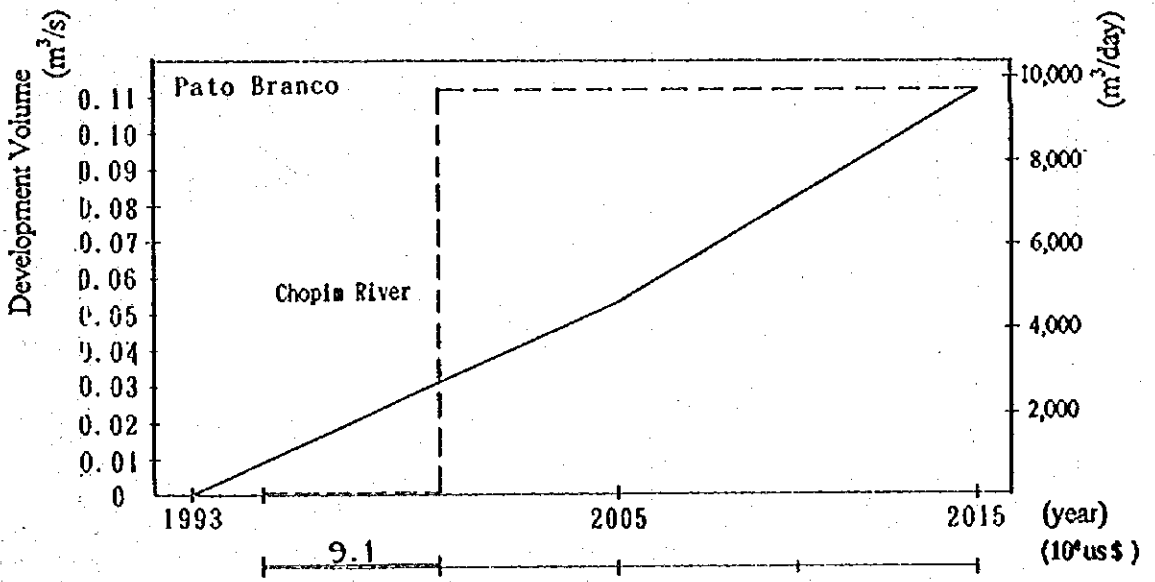
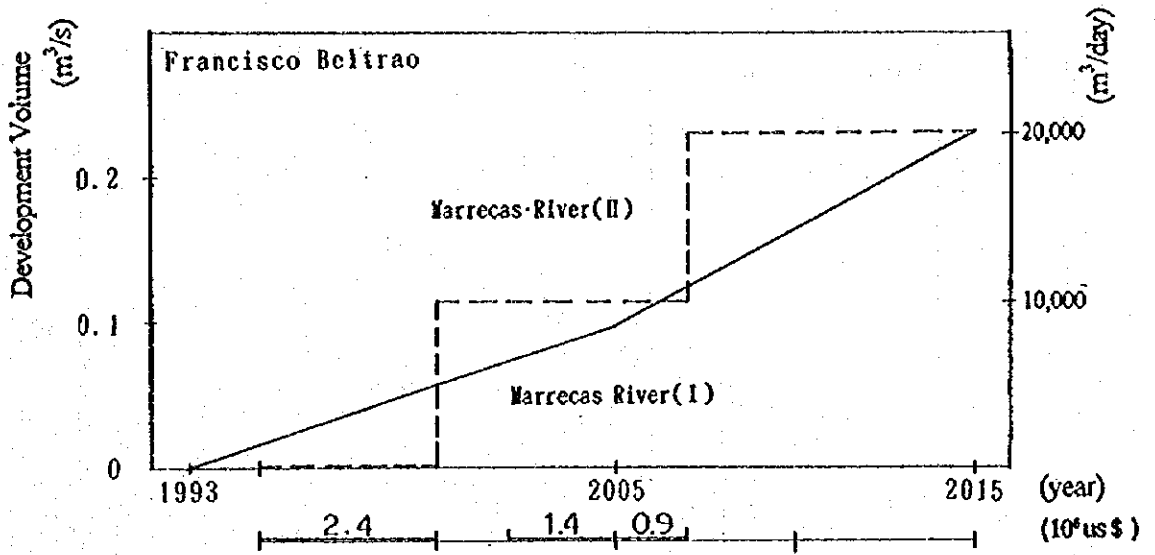


Figure-6.11 (1) Implementation Schedule of Medium Urban Areas

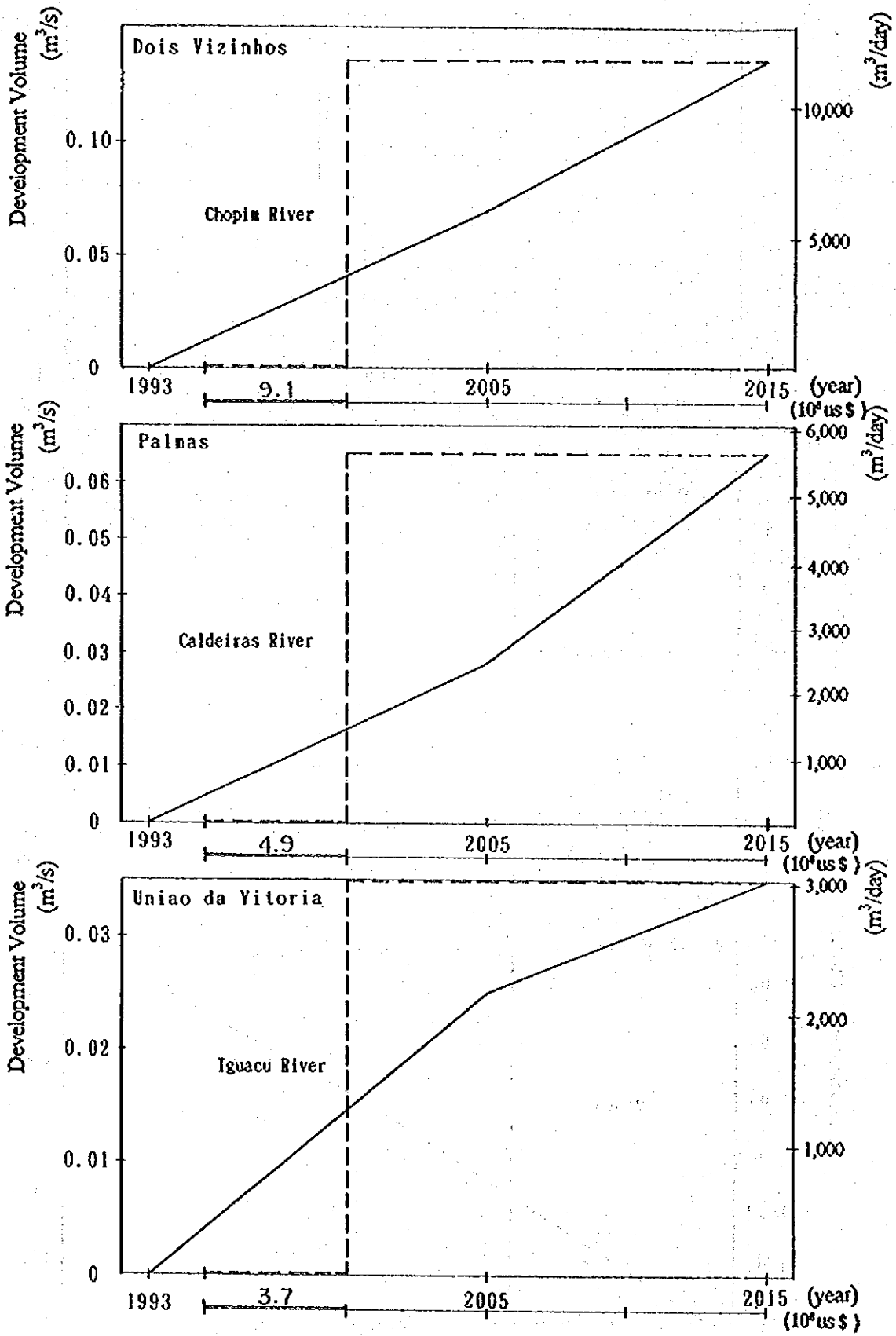


Figure-6.11 (2) Implementation Schedule of Medium Urban Areas

6.7 Water Development in Other Urban Areas (Type-C)

Water development study of other urban areas was done for zone-a, zone-b and zone-c.

6.7.1 Water Requirement

Required water supply in other urban areas was shown in Table-6.20.

Table-6.20 Required Water Supply in Other Urban Areas (m³/s)

Zone	Year	
	2005	2015
Zone-a	0.143	0.322
Zone-b	0.142	0.326
Zone-c	0.091	0.180

[Note] Water requirement for urban area is mainly composed of urban domestic water and industrial water.

6.7.2 Process of Water Resources Development Study

Process of water resources development in other urban areas was as shown below:

- (1) Determination of water resource for each zone evaluating surface water potential and groundwater potential.
- (2) Identification of the relationship between the water requirement and its development cost based on the cost estimation of several municipalities selected from each zone.
- (3) Cost estimation of all municipalities applying the above relationship to the water requirement of each municipalities.

6.7.3 Water Resources Development Policies

The water resources development policies for Type-C cities, based upon consideration of the topographical conditions and surface water and groundwater conditions in each zone, are as indicated in Table-6.21.

Table-6.21 Water Resources Development Policies for Other Urban Areas

City	Topographical Condition	State of Water Resources		Water Resources Development Policies
		Surface Water	Groundwater	
Zone-a	These areas are situated nearby mainstream or downstream of tributaries.	As these areas are located nearby rivers with ample catchment areas, direct intake development is easy to achieve.	Of the aquifers located within the Iquacu River basin, those suited to groundwater development are the Karst, Farnas Formation, Guabirota Formation, Botucatu Formation and the Serra Geral Formation north and south aquifers. Of these, the first three are located in the Curitiba metropolitan area. Regarding the supply of groundwater to Type-C cities, the Botucatu Formation and Serra Geral Formation aquifers are situated in usable locations. The former of these possesses greater productivity potential, however, deep drilling would be necessary.	As the direct intake development of surface water is easy, the nearby rivers will be developed as water supply sources.
Zone-b	These areas are situated near the second or third tributaries	The catchment areas of the nearby rivers are too small for performing the direct intake of water. The development of surface water would be possible if the intake points are placed further downstream, although the pipe line lengths would become long.	As the Type-C cities do not have such a high water requirement, development of the latter (Serra Geral Formation) aquifer is more appropriate.	For those cities, which are located on the Serra Geral Formation south aquifer and where the required water supply can be met by one well (0.003 m ³ /s or less), groundwater will be developed in order to provide the water supply. For those cities requiring a bigger water supply or which are not located on the said aquifer, direct intake development of surface water will be implemented.
Zone-c	These areas are situated on top of ridges of mountains.	Surface water resources are not sufficient to provide the required water in those cities with a large water demand.		For those cities which are located on the Serra Geral Formation south aquifer and where the required water supply can be met by three wells (0.010 m ³ /s or less), groundwater will be developed in order to provide the water supply. For those cities requiring a bigger water supply, or which are not located on the said aquifer, direct intake development of surface water will be implemented to meet the supply requirement.

6.7.4 Water Supply System in Other Urban Areas

Water supply system in other urban areas are shown in Table-6.22 by each zone.

Table-6.22 Water supply System in Other Urban Areas

Zone	Number of Municipalities	Water Supply System	Development Volume		Cost (10 ⁶ US\$)
			Surface Water (m ³ /s)	Groundwater (m ³ /s)	
C-a	23	direct intake from river	0.322	-	9.7
C-b	22	direct intake from river	0.298	-	42.5
	16	wells	-	0.028	14.8
C-c	9	direct intake from river	0.157	-	28.4
	8	wells	-	0.023	7.5
Total			0.777	0.051	102.9

6.7.5 Implementation Schedule of Water Development

Implementation schedule of water development is shown in Figure-6.12.

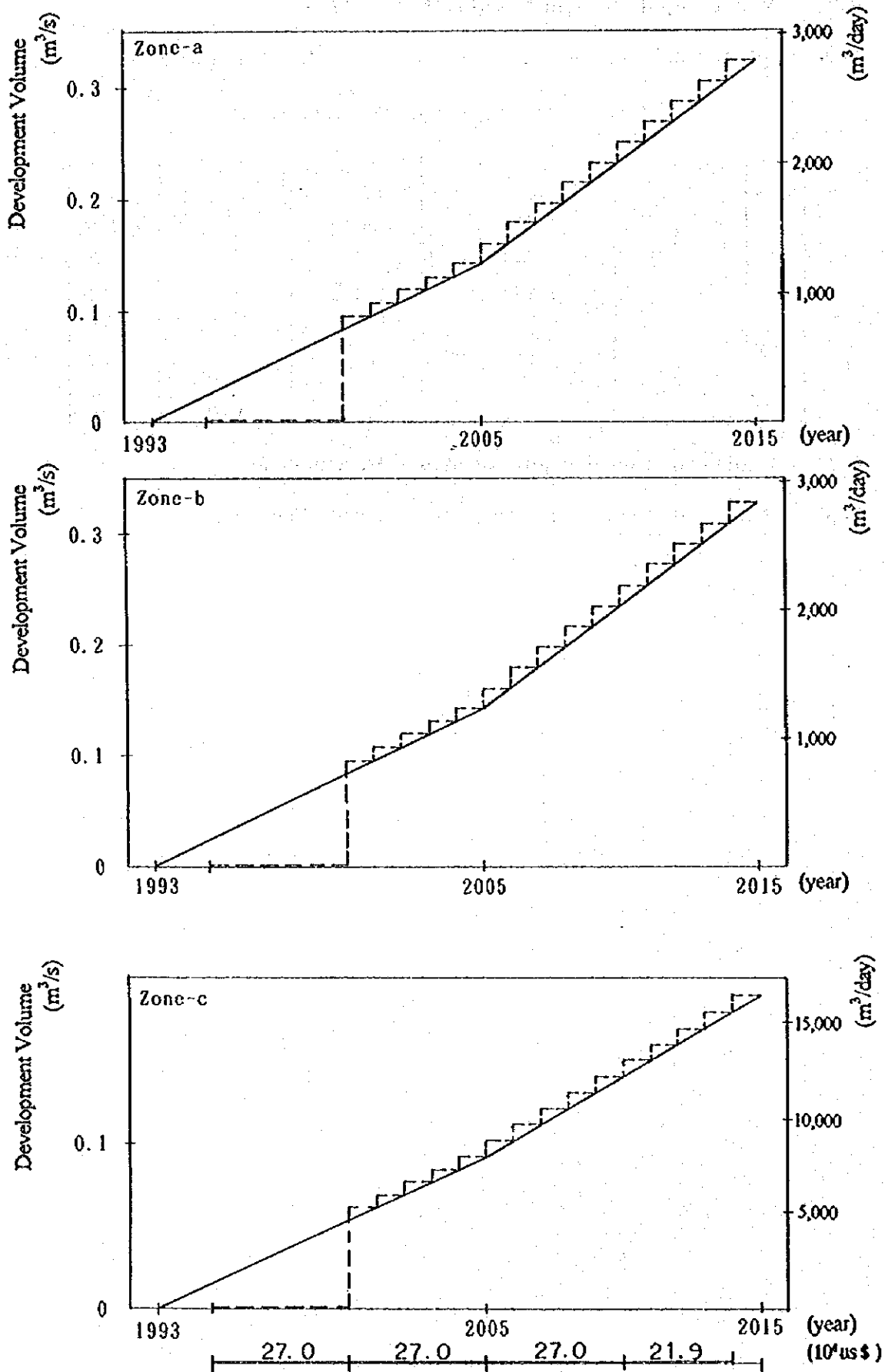


Figure-6.12 Implementation Schedule of Other Urban Area

6.8 Water Development for Rural Domestic Water

In rural areas, it is difficult to supply the water requirement by surface water systematically, because demand of domestic water is scattered due to topographic condition. Therefore, supply for domestic water will be done by groundwater development.

The demand of domestic water in rural areas tends to decrease from the point of view of the whole Iguaçú river basin. Although there is an increase in some municipalities if the demand is examined with municipality wise, the volume of demand is very little. The maximum volume to be newly development is about 0.004m³/s.

As a result, the development of rural domestic water will not be necessary and only improvement or maintenance of existing wells is enough to satisfy the future water demand.

6.9 Water Development for Agricultural Water

Supply method of agricultural water at rural areas is generally a pipeline method with a direct intake using a pipeline and headworks.

According to hearing and field reconnaissance, an average of intake volume was less than 0.001 m³/s, and average length of pipeline was 3 km.

The total water requirement for agricultural sector is 0.381 m³/s. The total cost of its development was estimated applying the cost of unit water development determined during the cost estimation for large and medium urban areas and thus the total cost is US\$4.6 million.

6.10 Total Cost for Water Development

The total cost for water development covering from intake to water-service installation was summarized in Table-6.23.

Table-6.23 Total Cost for Water Development

Development Volume (m ³ /s)		Cost (10 ⁶ us\$)
(1) Domestic and Industrial Water Development (Urban Area)		
1) Curitiba Metropolitan Area	7.235 (2.638)	760.0
2) Large Urban Areas	1.877 (0.090)	59.1
3) Medium Urban Areas	0.643 (0.192)	35.8
4) Other Urban Areas	0.828 (0.243)	102.9
Sub-total	10.583 (3.163)	957.8
(2) Agricultural Water Development (Rural Area)		
	0.381	4.6
Total	10.964 (3.163)	962.4

Note: () shows industrial water

The implementation schedule of water supply project is shown in Table-6.24.

Table-6.24 Implementation Schedule of Water Supply Project for Iguacu River Basin

Area	Project	Water Resource	Development Volume (m ³ /s)	Project Cost (million us\$)	Construction Schedule																		
					96	97	98	99	00	01	02	03	04	05	06	07	08	09	10	11	12	13	14
<Curitiba Metropolitan Area>																							
		Wells (Stage I)	111,000	110.6																			
		Wells (Stage II)	103,000	157.9																			
		Irai Dam	121,000	135.4																			
		Piraquara II Dam	65,000	60.4																			
		Pequeno Dam	69,000	78.5																			
		Aho Miringuava Dam	52,000	96.9																			
		Cofa Despique Dam	104,000	120.3																			
(5 year Progress Rate)			625,000	760.0	215.9				207.2				222.4				114.5						
<Large Urban Area>																							
Cascavel		Sao Jose River (I)	13,000	7.1																			
		Sao Jose River (II)	13,000	7.1																			
		Wells (Stage I)	16,000	17.7																			
		Wells (Stage II)	10,000	7.0																			
Foz do Iguacu		Parana River (I)	30,000	3.7																			
		Parana River (II)	30,000	3.7																			
		Parana River (III)	30,000	3.7																			
Guarapuava		Bananas River (I)	13,000	4.6																			
		Bananas River (II)	12,000	4.5																			
(5 year Progress Rate)			167,000	59.1	15.4				13.5				23.2				7.0						
<Medium Urban Areas>																							
Francisco Beltrao		Marrecas River (I)	10,000	2.4																			
		Marrecas River (II)	10,000	2.3																			
Palo Branco		Chopin River	10,000	9.1																			
Medianeira		Wells (Stage II)	11,000	4.3																			
Dois Vizinhos		Chopin River	12,000	9.1																			
Palmas		Caldeiras River	6,000	4.9																			
Uniao da Vitoria		Iguacu River	3,000	3.7																			
(5 year Progress Rate)			62,000	35.8	33.5				1.4				0.9				0						
<Other Urban Area>																							
		Surface water & Wells																					
(5 year Progress Rate)			72,000	102.9	27.0				27.0				27.0				21.9						
<Agricultural Water>																							
		Surface water																					
(5 year Progress Rate)			33,000	4.6	1.1				1.2				1.1				1.2						
Total			959,000	962.4	292.9				231.3				274.6				141.6						

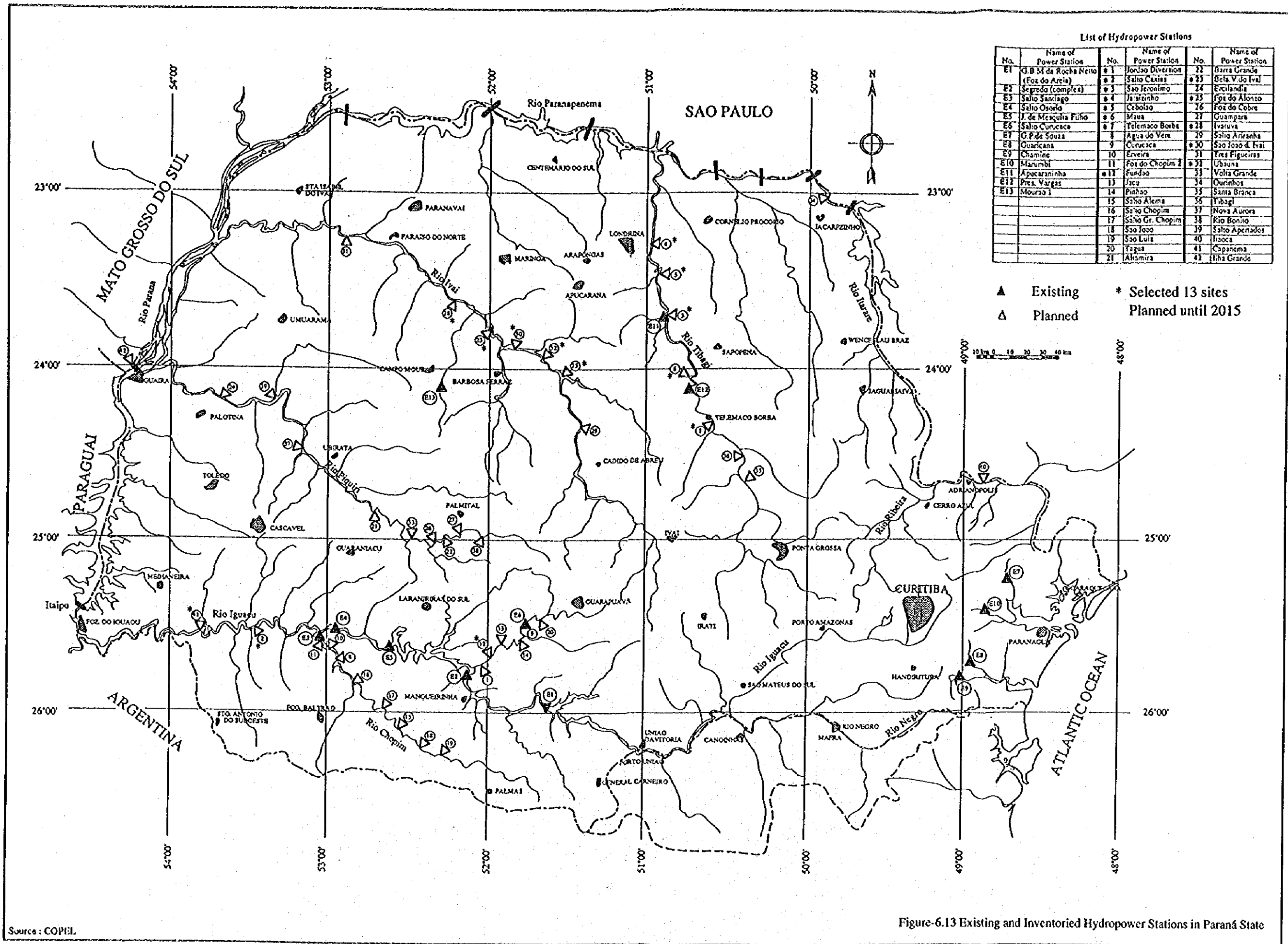
6.11 Hydropower Development

The hydropower development in the Iguacu river basin is planned as shown in Table-6.25 and in Figure-6.13.

Table-6.25 Planned Hydropower Stations in Iguacu River Basin

No.	Name of Power Station	Basin	River System	Intalled Capacity MW	Firm Energy Gwh	Planned Start-up Year
1	Jordao Diversion	Iguacu	Jordao	6.5	526*	Mar. 96
2	Salto Caxias	Iguacu	Iguacu	1,240	4,853	Dec. 98
Total (up to 2005)				1,247	5,379	
12	Fundao	Iguacu	Jordao	154	640	2005-09
Total (2005 to 2015)				154	640	
Grand Total		Iguacu		1,400	6,019	

Remarks: * denotes increment of energy in the existing Segredo plant and Jordao small plant.



List of Hydropower Stations

No.	Name of Power Station	No.	Name of Power Station	No.	Name of Power Station
E1	G. B. M. de Rocha Netto (Foz de Areia)	#1	Jordão Diversion	22	Barra Grande
E2	Segredo (complex)	#2	São Casias	23	São João d. Itaipu
E3	São Santiago	#3	São Jerônimo	24	Ercilândia
E4	São Osório	#4	Jataizinho	25	Foz do Alonzo
E5	J. de Mesquita Filho	#5	Cebolas	26	Foz do Cobre
E6	São Curucaca	#6	Mauá	27	Guampara
E7	G. P. de Souza	#7	Telemaco Borba	28	Ivaíruva
E8	Guaricana	8	Água do Vere	29	São Ariranhá
E9	Chamine	9	Curucaca	30	São João d. Itaipu
E10	Marumbi	10	Erveira	31	Treze Figueiras
E11	Apucarantina	11	Foz do Chopim 1	32	Ubaituba
E12	Pres. Vargas	12	Fundo	33	Volta Grande
E13	Mourão I	13	Jacú	34	Ourobranco
		14	Pinhão	35	Santa Branca
		15	São Alema	36	Tibagi
		16	São Chopim	37	Nova Aurora
		17	São Gr. Chopim	38	Rio Bonito
		18	São João	39	São Apertados
		19	São Luís	40	Itaoca
		20	Tagua	41	Caparema
		21	Altamira	42	Itaipu Grande

▲ Existing * Selected 13 sites
 △ Planned Planned until 2015

Source: COPEL.

Figure-6.13 Existing and Inventoried Hydropower Stations in Paraná State

