JAPAN INTERNATIONAL COOPERATION AGENCY (JICA)

STATE SECRETARIAT OF PLANNING AND GENERAL COORDINATION, PARANÁ STATE, THE FEDERATIVE REPUBLIC OF BRAZIL

THE MASTER PLAN STUDY ON THE UTILIZATION OF WATER RESOURCES IN PARANÁ STATE IN

THE FEDERATIVE REPUBLIC OF BRAZIL

FINAL REPORT

SECTORAL REPORT VOLUME H FLOOD CONTROL



Yachiyo Engineering Co., Ltd. Tokyo, Japan

and

Nippon Koei Co., Ltd. Tokyo, Japan

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COMPOSITION OF FINAL REPORT

1. EXECUTIVE SUMMARY

2. MAIN REPORT

- I. Strategy for Paraná State
- II. Master Plan for Iguaçu River Basin
- III. Master Plan for Tibagi River Basin

3. SECTORAL REPORT

- A. Socio-economy
- B. Meteorology, Hydrology and Surface Water Resources
- C. Hydrogeology and Groundwater Resources
- D. Domestic and Industrial Water
- E. Agriculture
- F. Hydroelectric Power Generation
- G. Water Utilization Plan
- H. Flood Control
- I. Water Quality and Sewerage
- J. Soil Erosion and Forest
- K. Ecology
- L. Water Environment Management
- M. Institution
- N. Cost Estimate, and Economic and Financial Assessment

4. DATA BOOK

THE MASTER PLAN STUDY ON THE UTILIZATION OF WATER RESOURCES IN PARANÁ STATE IN THE FEDERATIVE REPUBLIC OF BRAZIL

Sectoral Report Vol. H Flood Control

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CHAPTER 1 INTRODUCTION

1.1 Purpose

Flood control is referred to as the management of water excess. The main purpose of flood control is to mitigate the damage caused by floods. It is divided into two concepts: 1) flood plain management and 2) urban storm water management. Flood plain management considers, on a comprehensive scale, the integrated view of all structural, non-structural, and administrative measures for minimizing the damage caused by floods. Urban storm water management considers the integrated view of urban sewage and storm drainage in addition to the framework of flood plain management. In this study, urban storm water management mainly deals with the increase of flood peak and waste discharge caused by the exponential growth of urban population and the expansion of urban areas.

1.2 Methodology

The flood control study in Phase II comprised the following components:

- 1) Analysis of flood runoff
- 2) Study of the design flood discharge
- 3) Flood inundation and damage analysis
- 4) Study of alternative flood control measures
- 5) Formulation of the Strategy for the long-term flood control

The analysis of flood runoff deals with objective flood and corresponding rainfall events. It is jointly conducted with the hydrology study. The study of design flood discharge determines the peak water level and flood discharge based on the criteria for improvement of water environment. The flood inundation and damage study made an analysis of the existing damage data and estimates the inundation-damage relationship for selected flood prone areas.

The study of alternative flood control measures includes prospective structural and non-structural measures for flood plain and urban storm water management. The structural measures include engineering works such as reservoirs, retarding basins, levees or dikes, flood walls, channel improvement and, diversion or floodways. Non-structural measures are referred to as modification of the damage susceptibility of flood prone areas such as flood proofing, relocation, flood warning and evacuation, and land use control. The long-term plan for flood control is formulated as a part of the Strategy taking into consideration other sectors' long-term plans.

CHAPTER 2 HISTORIC FLOOD RECORDS IN PARANÁ STATE

2.1 General

Record of monetary damage, detailed description of social, economic and structural damage, and maps of flood inundation areas are not available in Paraná State, except for some fraction of information kept in several municipal offices and states institutions. The National Department of Sanitation Works (DNOS) was in charge of the flood control management at national level. The DNOS had a function of flood investigation, study and keeping flood damage record. The historic record and investigation reports made by DNOS seems to be disappeared. After DNOS was abolished in 1990, the Civil Defense (CD) is the only institution that has officially kept historic record of floods in Paraná State. However, the record of CD is limited to number of persons, houses, buildings and bridges that suffered flood damage, since it takes charges of security of people and rescue activities, instead of flood control management.

The National Department of Water and Electric Energy (DNAEE) keeps a few secondary information about flood obtained from other organizations including photographs and newspaper articles. The Superintendency of Erosion Control and Environmental Sanitation (SUCEAM) is in charge of flood control in Paraná State. SUCEAM keeps some information on flood which are related to the Environmental Sanitation Program of Curitiba Metropolitan Region (PROSAM), but those information do not include monetary damage. The Coordination of the Metropolitan Area of Curitiba (COMEC) assesses flood control projects in terms of change in land value, since there is not available record of monetary damage caused by floods. The Institute of Investigation and Planning of Urban Curitiba (IPPUC) made a partial survey of the damage caused by major floods in Curitiba city.

2.2 Flood Records by Civil Defense

The historic record of flood damage kept by the Civil Defense covers number of people dislodged, injured and dead, number of houses damaged or destroyed, and damage to bridges and roads. Table-2.1 summarizes the historic distribution of damage caused by floods in Paraná State in terms of number of affected people (total number of people dislodged, injured, and dead) in the period from 1980 to 1993. Table 2.2 shows the number of dislodged people caused by the flood inundation of January and February 1995. The 1983 and 1992 floods are the two largest floods in Paraná State since 1932. It is difficult to make a precise assessment of the magnitude of floods that occurred in a year other than 1983, 1992 and 1995 because of the scarcity of damage data.

2.3 Flood Prone Areas in Parana State

The location distributions of municipalities which suffered the floods of 1983 and 1992, the two major floods in Paraná State during the last two decades, are shown in Figure-2.1 and Figure-2.2 respectively. The location distribution of municipalities affected by floods in a year other than 1983 and 1992 are shown in Figure-2.3. From Figures-2.1, 2.2 and 2.3 it can be seen that the majority of urban flood prone areas of Paraná State are located within the Iguaçu river basin.

Since flood inundation maps are not available in the institutions of Paraná State, JICA Study Team conducted a reconnaissance survey in the flood susceptible areas. The areas surveyed were scooped from the historic flood record kept by the Civil Defense and the information obtained from the counterparts. The field surveys were conducted during the periods from early July to early August, 1994 and from the beginning of February to mid March of 1995. The following listed areas were covered by the reconnaissance:

Iguaçu river basin

Region-1: Curitiba metropolitan area

Region-2: Porto Amazonas, São Mateus do Sul-

Region-3: Reboucas, Guarapuava, Irati (Tibagi river basin)

Region-4: União da Vitoria, Porto União (Santa Catarina state), Porto Vitoria

Region-5: Rio Negro, Mafra (Santa Catarina state)

Region-8: Capanema

Paraná river basin

Region-6: Foz do Iguaçu, Del Este city (Paraguay), Upstream of Itaipu dam

Coastal basin (Litoranea region)

Region-7: Morretes

The location map of these regions is shown in Figure-2.4.

Ipiranga and Ivai cities were investigated as a part of the Tibagi river basin in Phase III.

2.4 Description of Flood Inundation and Maps

2.4.1 General

The available records of flood in Paraná State indicate that flood inundation causing significant economic and social damage occurred generally in urban or semi-urban flood prone areas. Those

urban flood prone lands are of relatively small size. In many cases the flood prone areas in Paraná State are occupied by low income families. Most of the urban flood prone areas are located within the Iguaçu river basin. Interview with the authority from the Department of Agriculture confirmed that there is not significant flood inundation affecting large agricultural land area in Paraná State; There are only verbal reports of relatively small and scattered flood inundation damage to agriculture and livestock.

2.4.2 Region-1: Curitiba Metropolitan Area

The Region-1 comprises those flood prone areas located in the upper stretch of the Iguaçu river basin, principally the flood prone areas of Curitiba Metropolitan region. Floods in this region are caused by the runoff from the main streams of the Iguaçu river and its tributaries such as Atuba river, Irai river, Palmital river, Itaqui river, Pequeno river, Belem river, Ivo river and Barigui river. In Region-1 for the last fourteen years there are reports of flood inundation occurring in 1980, 1981, 1982, 1983, 1988, 1992, 1993 and 1995 respectively. Floods in this region are of relative short duration, lasting from few days to about one week.

The largest recorded flood occurred in 1983. The maximum inundation water level registered in 1983 at a block named Jardim Triangulo was 877.2 meters above sea level, the ground level at that point is between 876.3 meters to 876.6 meters (i.e.; the maximum inundation water depth was about 1 m). The maximum water level at Jardim city block, in Sao Jose dos Pinhais, was 872 above sea level, the ground level at that point is between 868 to 871 meters above sea level. The flood inundation of January 1995 covered a land area similar to that of 1983 flood. Figure-2.5 shows the recorded annual maximum gauge water level of the Iguaçu river at DNAEE gauge in Curitiba Metropolitan Region from 1977 to January 1995. Figure-2.6 shows the most significant areas inundated in Region-1 during the flood of 1983. In January 1995 the upper Iguaçu river basin, the upstream of the junction with the Belem river recorded the highest rainfall depth.

In the reports of 1983 COMEC and IPPUC suggested that the main cause of inundation within Region-1 seems to be insufficient capacity and interruption of the rivers and drainage systems (Refs: Relatorio sobre Enchentes Ocorridas na Região Metropolitana, Curitiba en 1983, COMEC; and Enchentes em Curitiba 1983, IPPUC-DOG-1983).

2.4.3 Region-2: Sao Mateus do Sul and Porto Amazonas

Region-2 includes the two urban flood prone areas of Porto Amazonas and Sao Mateus do Sul. This region is located in the upper stretch of the Iguaçu river basin. Both cities are located at the margin of the Iguaçu river. The significant flood inundation in these two cities are caused directly by the water of the main stream of the Iguaçu river. In Sao Mateus do Sul the two small tributaries, the Canoas river and the Taquara river contribute also to the flooding of the urban

area, in addition to the flooding water of the Iguaçu river. Significant floods occurred in Sao Mateus do Sul in 1983, 1987, 1989, 1992, 1993, and January 1995. The major flood inundation was in 1983 with the maximum recorded gauge water level of 7.5 meter (El. 762.9 m). The recorded maximum gauge water level during flood inundation of 1992 and 1995 were 6.69 meter and 6.07 meter respectively. The 1992 flood lasted for about three weeks, and the flood of January 1995 also lasted for three weeks.

The long-term water level record of DNAEE at Sao Mateus do Sul show in Figure-2.7 indicates that since 1931 the annual maximum gauge water level exceeded the 5 meter mark in 13 times: i.e., 2 times from 1931 to 1950 (20 years), 4 times from 1951 to 1979 (29 years), and 7 times from 1980 to 1995 (16 years).

Since 1931 the annual maximum gauge water level at Sao Mateus do Sul registered the level above the 6 meter mark only three times: 1983, 1992 and January 1995. Figure-2.7 shows the recorded annual maximum gauge water level of the Iguaçu river at DNAEE gauge in Sao Mateus do Sul. Figure-2.8 shows the most significant area inundated in Sao Mateus do Sul during the flood inundation of 1992.

The significant flood inundation in Porto Amazonas were in 1983, 1992, 1993 and January 1995. The maximum gauge water level during the major flood inundation of 1983 was 9.5 meter measured at DNAEE gauge. The 1992 and 1995 (January) flood water levels at the same gauge were 7.98 and 7.86 meters respectively. The average ground level at the flood prone area of Porto Amazonas is between 775 to 780 meter above sea level. The inundation at Porto Amazonas last for approximately one week.

The long-term water level record of DNAEE at Porto Amazonas shows that since 1936 the annual maximum gauge water level exceeded the 6 meter mark 10 times: i.e., 5 times from 1936 to 1979 (44 years), and 5 times from 1980 to 1995 (16 years).

Since 1936 the annual maximum gauge water level at Porto Amazonas registered the level above the 7 meter mark only three times: 1983, 1992 and January 1995. Figure-2.9 shows the recorded annual maximum gauge water level of the Iguaçu river at DNAEE gauge in Porto Amazonas from 1936 to January 1995. Figure-2.10 shows the area inundated in Porto Amazonas by the flood of 1983.

2.4.4 Region-3: Reboucas, Guarapuava, Irati

Region-3 is located about 180 km west of Curitiba, in the tributaries of the upper stretch of the Iguaçu river basin, southern part of the Tibagi river basin. The areas that suffered significant flood within Region-3 are Reboucas, Guarapuava and Irati. Irati belongs to Region-3 in the Tibagi river basin. The flood inundation in Reboucas are caused by the Potinga and Riozinho rivers, two tributaries of the Iguaçu river. In Reboucas during the last fourteen years significant

flood inundation occurred in 1983 and 1992. It occurred within the urban area of Reboucas. The 1992 flood in Reboucas lasted for about one week. The maximum level of flood water is estimated to be approximately 785 meters above the sea level. Figure-2.11 shows the major flood inundation area in Reboucas during the flood of 1992.

The significant flood reported in Guarapuava occurred in 1992. No detail is known about the characteristic of the flood that affected Guarapuava.

Floods in Irati are caused by the das Antas river, a tributary of the Tibagi river. The significant flood inundation in Irati city occurred in 1981, 1983 (two floods), 1986, 1987, and 1992. The major flood inundation was in 1983. The maximum flood water level raised about 3 meters above the normal low water level. The flood lasted only for one to two days. The significant urban flood prone area of Irati is shown in Figure-2.12.

2.4.5 Region-4: União da Vitoria, Porto União, Porto Vitoria

Region-4 is located in the middle reach of the Iguaçu river basin. This region comprises the cities most severely affected by flood inundation within the Study Area. The significant flood inundation of Region 4 occurred in the urban areas of the twin cities União da Vitoria and Porto União, and the city of Porto Vitoria. Porto União belongs to Santa Catarina State. Flood inundation in the urban areas of these cities are caused directly by the runoff of the main stream of the Iguaçu river.

The long-term water level record of DNAEE at União da Vitoria shows that since 1930 the annual maximum gauge water level exceeded the 6 meter mark in 14 times: i.e., 3 times from 1930 to 1950 (21 years), 4 times from 1951 to 1979 (29 years), and 7 times from 1980 to 1995 (16 years).

The recorded gauge water level at União da Vitoria shows that maximum floods inundation depth with gauge water level higher than 7 meter have occurred in 1935, 1957, 1983 (the recorded highest), 1992 and 1993. The maximum flood water level of 1983 reached 750.03 meters above sea level, and the inundation lasted for 62 days. In 1992 the maximum water level was 748.51 meters above the sea level, 1.5 meter lower than the flood of 1983, and the inundation lasted for 65 days. In 1993 the maximum water level was 747.86 meters above the sea level and the inundation lasted for 22 days.

The major flood inundation damage reported by the Civil Defense occurred in 1980, 1982 (two floods in 1982, June and November), 1983, 1984, 1987, 1992, 1993 and 1995. Figure-2.13 shows the recorded annual maximum gauge water level at DNAEE gauge in União da Vitoria. Figure-2.14 shows the flood inundation area in the urban perimeter of União da Vitoria and Porto União by the flood of 1983. Figure-2.15 shows the flood inundation area in urban perimeter of União da Vitoria by the flood of 1992.

Significant floods in Porto Vitoria were reported in 1983 and 1992 but no detail is known about the characteristics of floods in Porto Vitoria.

2.4.6 Region-5: Rio Negro and Mafra

Region-5 is located in the Negro river, a major tributary of the upper Iguaçu river basin. This region comprises one of the flood prone area that have suffered very severe degree of flood inundation within the Study Area. The areas are the twin cities of Rio Negro and Mafra. Mafra belongs to Santa Catarina state. The flood inundation in Rio Negro and Mafra cities is caused by the Negro river. In Mafra city the da Lanca river also causes significant flood inundation, in addition to the flood inundation of the Negro river. The high water level of the Negro river causes backwater in the da Lanca river.

The long-term water level record of DNAEE at Rio Negro shown in Figure 2.16 indicates that since 1931 the annual maximum gauge water level exceeded the 7 meter mark 24 times: i.e., 13 times from 1931 to 1979 (50 years) and 11 times from 1980 to 1995 (16 years).

Gauge water level higher than the 8 meter mark in Rio Negro were recorded in 1946,1953, 1954, 1957, 1970, 1973, 1980, 1983, 1984, 1989, 1992 and 1993. The maximum gauge water level registered in Rio Negro city was 14.63 meters in 1983. The peak gauge water level in 1992 was only 21 centimeter lower than that of 1983.

In Rio Negro city the inundation of 1983 lasted for about twelve days, while in 1992 the inundation lasted for less than one week. The flood inundation damage in 1983, 1984, 1989 and 1992 was reported to be significant by the Civil Defense. Figure-2.17 shows the flood inundation area in the urban perimeter of Rio Negro and Mafra cities by the flood of 1992.

2.4.7 Region-6: Foz do Iguaçu, Del Este City, Upstream of Itaipu Dam

Region-6 comprises the areas inundated by the main stream of the Paraná river in the downstream of the Itaipu dam, principally Foz do Iguaçu city and Del Este city. Del Este city belongs to Paraguay. The available data on significant flood inundation around Foz do Iguaçu city indicate that the peak flood occurred in 1992, when the water reached the level of 126.75 meters above sea level, some 4 centimeters higher than the flood level of 1983. Small flood inundation was also reported in Porto Meira in the lower end of the Iguaçu river, near of its confluence with the Paraná river. Table-2.3 shows the annual maximum water level recorded by Itaipu Binational since 1983. Figure-2.18 shows the location of some areas which suffered flood inundation in Region-6.

There are some other municipalities that have suffered from flood inundation along the riverine of the Paraná river upstream of the Itaipu dam. There are very little data about the characteristics of floods in those municipalities. Among those municipalities that suffered floods are: Marilena, Porto Rico, Sao Pedro do Paraná, Querencia do Norte, Icaraima, Altonia, and Guaira. Flood inundation in these municipalities are caused by the Paraná river in the many small islets (islands) that are within the Paraná river regime and in the communities located along the riverine. In the municipalities of Marilena, Porto Rico, Sao Pedro do Paraná, and Querencia do Norte there are about 38 islets of different sizes. Those islets are inhabited by some 3,000 people. The population per islet varies from 1,400 people in the largest one to few person in the smallest one (Ref: State Coordinator of Civil Defense, 1974).

A report of the Civil Defense about a major flood inundation of the Paraná river in 1974 indicates that water level in that area raised some 8 meters above the normal low water level. Almost all the inhabitants of those islets were affected by that flood inundation, causing damage to the agriculture, livestock and houses. Floods in that region have been also caused directly by the runoff of the tributaries of the Paraná and Paranapanema rivers, but there is no available data on the characteristics of flood caused by the tributaries rivers.

2.4.8 Region-7: Morretes

The significant flood in Region-7 occurred in the municipality of Morretes which is located in the coastal area of the Paraná state. The flood inundations in this municipality are caused by the Nhundiaquara river which originates in the mountain range named Serra do Mar. Morretes is located at an average altitude of 7 meters above the sea level. The high tide water at Paranagua bay causes back water of the Nhundiaquara river. Because of the low elevation where Morretes is located, the combination of increased river discharge with the back water causes the inundation of the city. In Morretes significant flood inundation have been reported in 1948, 1969, 1972, 1989 and February of 1995. The maximum flooding water level were 5.70, 5.87, 5.1, 4.70 and 5.10 meters above the sea level for the floods inundation of 1948, 1969, 1972, 1989 and 1995 respectively. The duration of flood inundation in 1989 and 1995 was only one day in both cases. Figure 2.19 shows the annual maximum water level recorded at DNAEE gauge in Morretes. Figure 2.20 shows the Flood Inundation Area in the Urban Perimeter of Morretes by the flood of February 1995.

2.4.9 Region-8: Capanema

The significant flood inundation in Region-8 occurred in the municipality of Capanema, located in the lower Iguaçu river basin. There is a verbal report on significant damage caused by flood inundation to the agricultural and livestock of this region, but quantification of damage and the specific areas that suffered inundation were not clarified by the data obtained during survey, except for the village of Porto Lupion. Porto Lupion is a small community located at the margin

of the Iguaçu river. Significant floods occurred in Porto Lupion in 1983 and 1992. Figure-2.21 shows the location of Porto Lupion and Capanema.

2,4.10 Flood Prone areas in Tibagi river basin

The flood inundation damage record of Civil Defense includes some areas within the Tibagi river basin where large number of people were dislodged due to the flood inundation of 1983. Ipiranga and Ivai cities were reported as having 3,000 and 3,200 affected people respectively by the flood inundation of 1983. This two cities were surveyed during in February of 1995 to clarify the flood prone area and the magnitude of damage. However the interview result to local authorities at each prefecture indicated that the flood of 1983 inundated only 10 houses in Ipiranga city and that no significant flood inundation have occurred in Ivai city. Figure 2.22 shows the area inundated in Ipiranga city by the flood of 1983.

2.5 Estimation of Flood Damage

2.5.1 Available flood damage data

The inundation-damage relationship is the basis for a sound evaluation and selection of alternative measures for flood management and reduction of flood damage. A comprehensive quantification of all significant damage, tangible and intangible damage, caused by flood inundation is required to estimate the inundation-damage relationship at each flood prone area. However, the available data on flood inundation damage in Paraná state are very scarce and incomplete. Comprehensive evaluation of tangible damage, specially monetary damage, has not been done yet in Paraná state. Some municipal offices have gathered partial data on monetary flood damage. Another limitation found for evaluating the flood inundation damage is that existing data are kept as cumulative total damage within a municipality. For example, the total number of bridges broken or the total length of road damaged in the past within the entire municipality. This make difficult to clarify which damage occurred at each specific flood prone area.

The Civil Defense has kept partial data on flood inundation damage, especially the number of people affected by flood inundation, number of houses affected, and partial quantification of damage to buildings and infrastructure. Table 2.1 presents the data kept by the Civil Defense on the number of people affected by flood inundation within the Study Area from 1980 to 1993, including the number of dislodged, injured, and dead people caused by flood inundation. Table 2.2 summarizes the dislodged people due to the flood inundation of January and February 1995 as reported by Civil Defense.

The available monetary damage reported by official institutions does not include all items of tangible flood inundation damage, therefore most of available monetary damage data shows values lower than the actual total damage. The most comprehensive evaluation of monetary flood

inundation damage is the one made in União da Vitoria and Porto União by officials of the respective municipality and CORPRERI. A part of monetary inundation damage of 1983 and 1993 in Curitiba city was given by IPPUC. A part of monetary inundation damage of 1983 and 1995 was given by COMEC. Tables from 2.4 to 2.8 present the available partial monetary flood inundation damage in the flood of 1983, 1993 and 1995 in Paraná State.

The most significant kinds of damages include: damaged houses of low income families and other type of buildings because of the long standing flooding water; damage to the families' furniture and various house items; damage to small and medium industries; damage to the commercial sector; damage to the road systems, mainly bridges and asphalt pavement; and expenses incurred for emergency relief, including dislodging and re-lodging the affected families, provision of food and medicine for the affected families, etc.

The increase in occurrence of some diseases due to flood inundation have being reported by the mass media and verbal reports, but no detailed official written report was found during the survey.

2.5.2 Estimation of flood damage

Probable flood damage was roughly estimated with reference to the data and information gathered during the field survey done during Phase II and III of the Flood Control Study. Probable direct flood damage rates were estimated as a function of the probable number of inundated houses and buildings, after making a relationship between the inundation water level and probable number of inundated houses. Indirect damage were assumed as fifty percent of total direct damage. The probable number of affected houses and buildings were interpolated between the minimum and maximum number of actually reported values at each flood prone area. The relationship of flood water level and probable number of affected houses and buildings was made for those flood prone areas where at least few data were available. Figures 2.23 to 2.29 show the relationship between flooding water level and number of inundated houses at each main flood prone area. (The relationship of flood damage and corresponding return period is not available at this stage.)

The results of estimated probable flood inundation damage are simply notional to get a general idea of the magnitude of probable flood inundation damage for different flood level at each selected flood prone area. Table 2.9 shows the assumed unit flood damage as a function of the number of houses flooded. The estimated probable range of monetary flood damage at price level of July 1994 are presented in Tables 2.10 to 2.16.

For Curitiba metropolitan region the estimated probable flood damage are as high as US \$ 20 million for a flood similar to that of 1993, and some US \$ 44 million for a large flood similar to that of January 1995. For União da Vitoria-Porto União area the estimated probable flood damage are in the range of US \$ 10 million for a flood inundation event similar to that of 1982 to

US \$ 78 million for a flood event similar to that of 1983. For Rio Negro-Mafra area the range of estimated probable flood damage is US \$ 3 million for a flood event similar to that of 1984 and US \$ 17 million for large flood event similar to that of 1983. For Sao Mateus do Sul the estimated range of probable flood damage is US \$ 0.1 million for a small flood event similar to that of 1993 and US \$ 9 million for large flood event similar to that of 1983. For Porto Amazonas area the estimated flood damage are in the range between US \$ 0.23 million for a relatively small flood similar to that of 1993, to some US \$ 2 million for a relatively large flood similar to that of 1983. For Foz do Iguaçu area the estimated probable flood damage are in the range between US \$ 0.02 million for relatively small flood of maximum water level reaching the 119 meter counter and some US \$ 3 million if relatively large flood occur, Up to the 130 meter level. For Morretes area the estimated probable flood damage are in the range between US \$ 5 million if relatively small flood occur, and some US \$ 10 million for a large flood similar to that of February 1995.

CHAPTER 3 DESIGN FLOOD DISCHARGE

3.1 Probable Peak Flood Discharge

The frequency analysis of the maximum daily mean discharge was made for the selected stream flow gauging stations and the 10, 50 and 100 year specific peak discharges are shown in Table 3.1. The 100 year peak flood discharges of the selected stations are plotted in Figure 3.1 with comparison to the Creager's curve. The corresponding specific discharges are plotted in Figure 3.2.

3.2 Design Flood Discharge

It is practically difficult to apply the recorded maximum flood as the design flood discharge in Paraná State. Practical flood control level is to be determined based on damage level, social significance, regional development policy, etc. The recurrence interval of the design flood for the urban areas in the major municipalities is assumed tentatively to be 100 years as a target for the future.

CHAPTER 4 PROBLEMS AND NEEDS IN FLOOD CONTROL MANAGEMENT

4.1 General

Flood control management deals with water excess that endangers human lives, causes economic damage and disrupt the normal socioeconomic human activities. Concept of flood control management is broadly divided into Flood Plain management and Urban Storm Water management. Flood plain management considers the integrated views of all structural and non-structural measures for minimizing the damage caused by floods on a comprehensive scale. Urban storm water management, besides the above framework of flood plain management, also considers the integrated view on urban sewage and storm drainage management. For the purpose of flood control study in Phase II, the flood prone areas of Paraná state were divided into eight regions as previously described. Flood plain management issues were identified in all the eight regions. At present urban storm water management issues are identified only in Region 1, specifically in Curitiba metropolitan area.

Curitiba metropolitan region has experimented an exponential growth of the urban population and expansion of urban areas. Because of topographic constraint and increasing occupancy of low income population in flood prone areas the existing urban drainage systems became not enough for handling the urban flood runoff of large magnitude. Beside this, some of the fast growing areas in the peripheries of the metropolitan region are not provided with the basic infrastructures required for management of urban storm.

In all eight regions, the most significant flood inundation damage occur in urban flood prone areas. In many cases the urban flood prone areas along rivers regime are occupied by low income families. In few cases, such as União da Vitoria-Porto União and Rio Negro-Mafra areas, the urban flood prone areas are occupied with relatively high cost infrastructures, important industries, commercial establishments, and high value houses.

4.2 Assessment of Problems and Needs

Problems and Needs in flood control were assessed by region, taking as main criteria the magnitude of damages caused by the past flood inundation. The major flood inundation events in Paraná state were the floods of 1983 and 1992. During the last fourteen years the largest damage caused by flood inundation occurred within the Iguaçu river basin. The regional assessment of flood inundation damage and related issues are summarized as described in Tables- 4.1 in terms of flood region and in Table- 4.2 in term of each river basin.

4.3 Institution in Charge of Flood Control

The Brazilian constitution defines that flood control is a mater under the responsibility of the federal government. The National Department of Sanitation Works (DNOS), within the organizational structure of the Ministry of Agriculture, used to be the institution in charge of flood control at national level. DNOS was abolished in 1990, and since then in Paraná state there is not any specific institution responsible for planning, designing, promoting, constructing, operating and maintaining projects, structures and activities for flood control and mitigation of flood damage. Some institutions have projects or activities related to flood control for some specific areas, such as PROSAM in Curitiba metropolitan region. The National Department of Water and Electric Energy (DNAEE) has responsibility on flood warning, while the Civil Defense has responsibility for rescue activities.

In Paraná state there is a need for establishing an institution in charge at state level of coordinating all the aspects related to flood control and mitigation of flood inundation damage, including plan, design, promotion and implementation of projects, operation and maintenance of structures, flood warning, rescue of people affected by flood, and keeping systematic record of flood inundation damage. SUCEAM has taken over the position after the re-organization in February, 1995, but its power and specific function is not clarified clearly yet.

CHAPTER 5 ALTERNATIVE FLOOD CONTROL MEASURES

5.1 General

The study of flood control considers both, structural and non-structural means as alternative to provide protection from flood inundation and to reduce the risk of flooding and the magnitude of damage caused by floods. The prospective structural flood control measures include flood control dams or gates, retarding basins, levees or dikes, flood walls, river channel improvement, and diversion or floodways. The prospective non-structural measures include flood proofing, flood forecasting, resettlement or relocation, flood warning and evacuation, and land use control.

5.2 Structural Measures

The prospective structural measures will be chosen among the alternatives set out below, according to the need for protection against flood inundation and mitigation of damage.

Dam: Construction of flood control dams or gates.

Dike: Construction of dikes systems including levee, concrete walls, and sluices

Channel improvement: Channel improvement including a short cut.

Floodway: Construction of floodways, bypasses, rain drainage channels separated from the sewerage systems.

Retarding basin: Construction of flood retarding basins and delineation of swampy areas as retarding basins or parks.

First flush: First flush control facilities consisting of retarding basins or regulation ponds, and treatment facilities for polluted water and solid wastes and sediments.

5.3 Non-Structural Measures

Prospective non-structural measures will be chosen among the alternatives set out below.

Zoning: Zoning for land use control, such as 1) restricted area including river regime and flood prone areas; 2) preservation park with functions of recreational areas; 3) retarding basins, and 4) control of waste disposal.

Resettlement: Resettlement or relocation of illegal residents occupying the river regime, and legal residents in the flood prone areas.

Forecasting: Improvement of flood forecasting and warning system.

Evacuation: Enhancing the flood warning, evacuation, and rescue activities.

Proofing: Flood proofing modifies the damage potential of individual structures susceptible to flood damage such as elevating structures, water proofing exterior walls and re-arrangement of structural working space.

Rules: Operation rule of reservoirs

CHAPTER 6 STRATEGY FOR WATER EXCESS MANAGEMENT

6.1 Strategy

6.1.1 Goal and Principle of Water Excess Management

The goal of the flood control (broadly water excess management) is to protect the people in the flood prone areas from the risk of death, injuries and property damages including infrastructures.

Non-structural measures with appropriate combination of land use control and flood forecasting, warning and evacuation systems are to be principal flood control measures in Paraná State because present population density is generally not significantly high in the flood prone areas and alternative land resources are expected to be available in Paraná State.

Structural measures are also to be provided in addition to the non-structural measures for the areas where existing land use is highly enhanced and property value in the flood prone areas is significantly high.

6.1.2 Flood Control Level and Design Standard

The flood control (or protection) level must be determined appropriately taking into account social significance of damage level and efficiency of benefit and cost with the Principle of Risk and Benefit (refer to the Sectoral Report Water Environment Management).

The same concept of flood control level is applied in several countries as shown in Table- 6.3.

6.1.3 Model Area

It is recommended to designate the following regions as the Model Area for Water Excess Management to which specific monitoring and / or financial arrangement and support are to be provided in long term:

Region-1: Curitiba Metropolitan area

Region-2: Porto Amazonas, São Mateus do Sul

Region-4: União da Vitoria

Region-5: Rio Negro

Region-6: Foz do Iguaçu, Upstream of Itaipu Dam

Region-7: Morretes

The location map is shown in Figure-2.4.

6.2 Long Term Plan for Water Excess Management

6.2.1 Flood Plain Management

Combination of structural and non-structural measures will be necessary for the municipalities of the Curitiba metropolitan area, São Mateus do Sul, União da Vitoria/Porto União and Morretes. Non-structural measures are to be primarily employed for the flood prone areas of the other municipalities.

6.2.2 Urban Storm Water Management

Integrated view of urban sewage, flood protection, storm drainage and environmental protection will be required for the Curitiba metropolitan area. Environmental protection includes waste disposal control, water quality control, protection of aqua ecosystem, and preservation of riverine landscape. View of urban storm water management might be evolved in other municipalities after the year 2005.

6.2.3 Non-structural Measures

Zoning for land use control is the most effective measure for all the flood prone areas in and around the urban areas in Paraná state (Region - 1 to 8). Zoning for land use control includes restricted area, river regime, natural preservation and recreational park, and retarding basin area. Zoning and resettlement are a tandem for implementation. Resettlement includes relocation of illegal residents occupying the river regime and legal residents in the flood prone areas. Zoning and resettlement have been widely applied in several municipalities in Paraná State such as the Curitiba metropolitan area, São Mateus do Sul, Porto Amazonas, Reboucas, Guarapuava, Irati, União da Vitoria, Rio Negro, etc.

Improvement of the existing flood forecasting and warning system will be necessary in the future together with enhancement of flood warning, evacuation and rescue activities which are mainly executed by the Civil Defense.

Flood proofing will be effective for some locally inundated areas.

Review of the operation rule of the existing and planned dams and reservoirs will be necessary taking flood control function into consideration for integrated and effective operation.

6.2.4 Structural Measures

(1) Curitiba metropolitan area

Continuation and extension of the flood control and drainage improvement projects of PROSAM is the first priority. Supplemental provision of dams, dikes, floodways, retarding basins, and channel improvement may be necessary together with the first flush treatment facilities after the year 2005 depending on the expansion of the urban area and the deterioration of the urban environment.

(2) São Mateus do Sul

A dike system on the right bank of the Iguaçu river may be effective for the flood prone area where demand of development of low cost housing for low income people are very high in spite of the city's zoning requirement. A detailed engineering study will be necessary for technical evaluation.

(3) Porto Amazonas

Excavation of a natural rock drop, which exists in the low water channel and interrupts the stream flow of the Iguaçu river, will improve flood discharge capacity, but it will not be financially viable.

(4) União da Vitoria and Porto União

There is no appropriate sites for flood control dams and retarding basins having sufficient regulation capacity in the upstream of União da Vitoria. It is recommended to conduct a feasibility study on provision of a set of structural measures which are composed of a dike system, channel improvement and sluice gates, because the property value and town function in the inundated areas affected during the 1983 and 1992 floods are significant. The channel improvement treats a series of natural rock drops along the main channel of the Iguaçu river in the upstream of the Foz do Areia dam just downstream of Porto Vitoria for improvement of channel discharge capacity. Flood protection for this area will not be materialized by the provision of non-structural measures only.

(5) Rio Negro and Mafra

Non-structural measures are most effective in particular by zoning in this area. Structural measures for the mainstream of the Negro river will not be financially viable due to topographic constraints in this area. However channel improvement of the Passa Tress in Rio Negro municipality side and the da Lanca river in Mafra municipality side may be effective in the future.

(6) Morretes

Channel improvement including a short cut and channel excavation may be effective for the flooding along the Nhundiaquara river. However, some detailed engineering study will be

necessary for technical evaluation because there is a back water effect by the high tide of the Paranagua bay.

6.2.5 Flood Forecasting and Warning Systems

The strategy for the flood forecasting and warning system (FFWS) in Paraná State aims to upgrade the existing system as a part of the integrated telemetric monitoring and operation system discussed in the Sectoral Report Water Environment management. It will also aims to reinforce a part of the nationwide flood forecasting and warning system under DNAEE which fulfills the following objectives in the future:

i) FFWS for Resident's Protection from Flood Incident

It aims to secure the life of people and to minimize flood damages in the flood prone area by enhancing prompt flood protection activities which necessitate sufficient and accurate information, through institutions concerned. It necessitates advanced forecast of extreme flood which may exceed the capacity of existing river facilities.

ii) FFWS for Flood Operation

It aims to execute promptly effective and safe operation of the flood control facilities such as dams, floodways and retarding basins by forecasting the magnitude of flood inflow into these facilities in advance. It also aims to avoid artificial flood disasters by disseminating in advance to the people information concerning flood release from those facilities.

iii) FFWS for Basin wide Flood Management

It aims to execute effective basin wide flood management and administration by integrated real time operation of all the flood control facilities. It necessitates real time access to the information on river and basin conditions.

6.2.6 Implementation Schedule

The implementation schedule for the water excess management is tentatively recommended in two stages as shown in Table - 6.1. for non-structural measures and in Table - 6.2 for structural measures.

CHAPTER 7 MASTER PLAN FOR WATER EXCESS MANAGEMENT

7.1 Master Plan for Iguaçu River Basin

7.1.1 Planning Criteria

Within the context of the goal and principle of water excess management of the Strategy the flood control model areas and design standard for the Master Plan are established as set out below.

(1) Model areas

Iguaçu River Basin

Region-1: Curitiba Metropolitan area

Region-2: Porto Amazonas, São Mateus do Sul

Region-4: União da Vitoria

Region-5: Rio Negro

Region-6: Foz do Iguaçu

Tibagi River Basin

No model area is designated in the Tibagi river basin, though some flood inundation is reported in the municipalities of Irati, Ivai and Ipiranga. The flood damage in these areas is minor.

(2) Flood Plain Management and Urban Storm Water Management

As a part of flood plain management, combination of structural and non-structural measures will be necessary for the Curitiba metropolitan area and municipalities of São Mateus do Sul and União da Vitoria/ Porto União. Non-structural measures are to be primarily employed for the flood prone areas in the other municipalities. Integrated view of urban sewage, flood protection, storm drainage and environmental protection is necessary for the Curitiba metropolitan area as urban storm water management. Environmental protection includes waste disposal control, water quality control, protection of aqua-ecosystem, and protection of riverine landscape.

(3) Design Standard

The flood control (or protection) level must be determined appropriately taking into consideration of social significance of damage level and efficiency of benefit and cost with the principle of risk and benefit. Appropriate combination of flood control level for zoning and structural measures must be also determined.

7.1.2 Non-structural Measures

Zoning for land use control is the most effective measures for all the flood prone areas in and around the urban areas in Paraná State (Region -1 to 8). Zoning for land use control includes restricted area, river regime, natural preservation and recreational park, and retarding basin area. Zoning and resettlement are a tandem for implementation and have been widely applied in the Curitiba metropolitan area (CMA), São Mateus do Sul, Porto Amazonas, Reboucas, Guarapuava, União da Vitoria, Rio Negro, etc. in the Iguaçu river basin. Resettlement includes relocation of illegal residents occupying the river regime and legal residents in the flood prone areas.

The existing flood forecasting and warning system (FFWS) is planned to be upgraded by the provision of the new lightening censoring and rainfall monitoring system under SIMEPAR. This upgraded system will provide basic warning information required for rescue activities for the time being. This system will be necessary to be upgraded for a basin wide real time flood management and operation to avoid both natural and artificial flood disaster in the future when number of water rescues and flood control facilities is increased significantly.

Flood proofing such as elevating ground level and structures is effective for some locally inundated areas in CMA and other flood prone areas. Review of the operation rule of the existing and planned dams and reservoirs will be necessary taking flood control function into consideration for the integrated and effective operation.

The proposed non-structural measures and their implementation schedule are listed in Table-7.1.

7.1.3 Structural Measures

The structural measures are proposed only for CMA, São Mateus do Sul and União da Vitoria - Porto União region.

The proposed structural measures and their implementation schedule are listed in Table-7.2.

(1) Curitiba Metropolitan Area

Non-structural measures are most effective in particular by zoning with resettlement and park in the Curitiba Metropolitan Area (CMA). Continuation and extension of the flood control and drainage improvement projects of PROSAM which is composed of structural and non-structural measures is the first priority. PROSAM is composed of the following components:

- i) 15 km long flood channel excavation of the main stream of the Iguaçu river parallel to the existing channel (about 1.3 million m³),
- ii) Landscape restoration and park development of river bank area,
- iii) Irai dam for flood control and to guarantee 1.8 m³/s to Curitiba water supply,

- iv) Relocation and resettlement of houses located in risky areas including occupying river flood plains, and
- v) Expropriation of 7,000 plots of land and rights needed for environmental protection along rivers and environmentally sensitive areas.

The total project cost was estimated to be 34.3 million US dollars in 1992 excluding the Irai dam. A part of the channel excavation (11 Km) shown in Figure-7.1 is now under construction, and the resettlement and park plans are in progress.

The extension program of PROSAM will be composed of the following components:

- i) Channel excavation and maintenance of the flood channel of the main stream of the Iguaçu river by Curitiba City Hall and COMEC, and
- ii) Piraquara II, Pequeno, Alto Miringuava dams for water supply (planned by SANEPAR) with flood control function.

In order to maintain the flood discharge capacity of the Iguaçu river a strong regulation to control extraction of sand and gravel in and around the water course of the Iguaçu rive is necessary. Modification and maintenance of the two existing and on-going parallel flood channels will also be continuously necessary.

The dams which are newly planned as water supply projects are to be reviewed as multipurpose having flood control function. The Piraquara II, Pequeno, Alto Miringuava dams may have a function to mitigate floods in the flood prone areas in the municipalities of Curitiba, Pinhais, Piraquara and São Jose dos Pinhais. The location map of the planned dams is shown in Figure 7.1.

Integrated view of urban sewage, flood protection (including floodways, retarding basins and channel improvement), storm drainage, and environmental protection is now in practice in CMA, and it will be more significant in the 21 st century depending on the expansion of urban area and the deterioration of urban environment.

(2) São Mateus do Sul

In the 1983 flood 5,800 people evacuated from houses and food of 22,858 tons was provided to the affected people in São Mateus do Sul. In the 1992 flood 970 people evacuated, 570 people lost houses and 1,200 houses were damaged.

Non-structural measures are most effective, and zoning with a combination of resettlement and park is first priority in São Mateus do Sul. However, a dike system on the right bank of the Iguaçu main stream may be effective in the future after the year 2006 for the flood prone area where demand of development of low cost housing for low income people are very high in spite of the city's zoning requirement. Channel improvement will not be financially feasible.

An idea of a prospective dike system with a sluice gate is preliminarily planned as follows.

- a) Design flood the recorded maximum flood (1983 flood, return period of around 100 years, peak discharge of 1,670 m³/s)
- b) Design flood water level 762.9 m
- c) Design crest elevation 763.9 m with freeboard of 1.0 m
- d) the dike system is to be aligned at elevation 760.4 m to 761.4 m
- e) Total length of dike about 2.5 km
- f) Total volume of dike about 110,000 m³
- g) Sluice with manual gate 1 site

A conceptual alignment of the dike system is illustrated in Figure-7.2. Feasibility study will be necessary for financial and technical evaluation.

(3) Porto Amazonas

Non-structural measures are most effective, and zoning with a combination of resettlement and park is first priority in Porto Amazonas. Excavation of a natural rock drop, which exists in the low water channel and interrupts the stream flow of the Iguaçu river, will improve flood discharge capacity, but it will not be financially viable.

(4) União da Vitoria and Porto União

União da Vitoria in Paraná State and Porto União in Santa Catarina State experienced significant flood inundation in 1983 (flood water level of 750.03 m; DNAEE gauge water level 10.42 m), 1992 (flood water level of 748.51 m), 1993 (flood water level of 746.86 m) and 1995 (flood water level of 746.36 m). In the 1983 flood about 30,000 people evacuated from their houses and food of 122,673 tons were provided to the affected people in União da Vitoria according to the report from the Civil Defense of Paraná State. In the 1992 flood 14,129 people evacuated from their houses, 3,736 people lost houses and 4,500 houses were damaged in União da Vitoria only. The flood inundation area is shown in Figure-2.14 for 1983 and Figure-2.15 for 1992.

Flood protection for this region will not be materialized by the provision of non-structural measures only. It will be practically and financially not acceptable for these municipalities to apply zoning by resettlement to the elevation of 750 m (1983 flood water level) because the property value in the flood prone area is extremely high and the town function in this area is significant. The land value is shown in Figure-2.15.

At present the municipality of União da Vitoria restricts the land use below the elevation of 744.5 m in the urban area and 745.0 m in the rural area as the dispossessed area coordinated with COPEL. In Porto União (Santa Catarina State) construction of public buildings (schools, hospitals, etc.) are prohibited in the area below 750.0 m by law. The flood water level exceeded

the elevation 744.5 m 36 times, 745.0 m 26 times in the period 1930 -1995 (66 years) as shown in Figure-2.13. If the elevation of restricted area is raised to 746.5 m the chance of exceeding this level will be reduced to 5 times during 66 years.

An alternative study including a channel improvement plan, a dike system plan and a combined plan of channel improvement and dike assessed that only dike system plan would be financially feasible with combination of zoning with resettlement.

The Study Team recommends the following zoning and structural measures

Zoning

a) Restricted Area

No private and public buildings and houses are allowed to exist below the ground elevation 746.5 m. The existing houses and buildings are to be resettled to the designated safe areas.

b) Conditional Area

Construction of new private and public buildings and houses is not allowed in the ground elevation between 746.5 m and 748.5 m before dike construction. Flood proofing such as elevating structures is to be enhanced as necessary.

Structural Measures

A dike system with sluice gates is to be provided to protect urban areas of both municipalities.

- a) Design flood the recorded maximum flood (1983 flood, return period of about 120 years, peak discharge of 4,980 m³/s)
- b) Design flood water level 750.0 m
- c) Design crest elevation 751.2 m with freeboard of 1.2 m
- d) the dike system is to be aligned at elevation 746.5 m to limit the maximum dike height less than 5 m.
- e) Total length of dike 17 km
- f) Total volume of dike 1.4 million m³
- g) Sluice with manual gate and drainage pump 8 sites

The channel improvement including excavation of a series of natural rock drops along the main stream of the Iguaçu river in the stretch upstream of the Foz do Areia dam just downstream of Porto Vitoria to União da Vitoria will not be financially feasible because of its extremely large excavation volume (over several million m³).

It is recommended to conduct a feasibility study on provision of the dike system and sluice gates. The total project cost of the dike system plan is approximately estimated to be about 86 million US dollars excluding cost for zoning. The conceptual alignment is illustrated in Figure-7.3.

(5) Rio Negro and Mafra

Rio Negro in Paraná State and Mafra in Santa Catarina State experienced severe flood inundation in 1983 and 1992. The recorded maximum gauge water level was 14.63 m at the DNAEE station in Rio Negro in 1983 which was higher than the normal water level by about 10 m. The peak flood water level of the 1992 flood was only 21 centimeters lower than that of 1983.

Non-structural measures by zoning are most effective in this region. Zoning with resettlement and park is in practice, but about 400 houses are still remained partly in the Negro river and partly in the Lança river the tributaries of the Negro river. A channel excavation of the Lança river in Mafra is in progress. Structural measures for the main stream of the Negro river will not be financially viable due to topographic constraints.

7.1.4 Alternative Flood Control Study for União da Vitoria

(1) Frequency analysis of peak flood discharges

COPEL conducted the frequency analysis of the peak flood discharge at DNAEE station in União da Vitoria using the historic record since 1891. The ploting position was estimated by the Weibull formula for censored data (Stedinger et al,1993). The used 68 data and result are shown in Table-7.3 and Figure-7.4 respectively. The return period of the 1983 and 1992 floods is estimated to be 123 years and 31 years respectively.

COPEL also conducted the frequency analysis of the annual maximum of 6 to 10 day rainfall by use of the rainfall records of 5 stations located upstream of União da Vitoria from 1941 to 1995 which were weighted by Thiessen method. The weighted maximum rainfall and Thiessen weight is shown in Table- 7.4 and the result is shown in Table- 7.5. The return period of the maximum 10 day rainfall in 1983 (383.8 mm) is estimated to be a little over 100 years. That of 1992 (369.9 mm) is estimated to be a little over 50 years.

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(2) Probable flood damage.

The relationship between probable flood damage and peak flood discharge is assumed for the União da Vitoria - Porto União area as shown in Figure-7.5 by use of the flood inundation damage estimated in Section 2.5.2. The annual average probable flood damage for the same area is also assumed as shown in Table-7.6. The corresponding damage in 1983 and 1992 are as follows:

	Probable Damage	Accumulated Annual Average
	(million US dollars)	Probable Damage (million US dollars)
1983 flood	78.1	9.8
100 year flood	70.7	9.6
50 year flood	56.8	9.0
1992 flood	54.6	8.3

(3) Alternative flood control measures

1) Possible alternatives

Possible combination of non-structural and structural measures are listed below.

	Non-structural	Structural
Case-1	Zoning with resettlement o	nly
Case-2	Zoning with resettlement	Channel excavation only
Case-3	Zoning with resettlement	Channel excavation and Dike system
Case-4	Zoning with resettlement	Dike system only
Case-5		Channel excavation only
Case-6		Dike system only
Case-7		Channel excavation and Dike system

Case-1 zoning with resettlement up to the elevation 750 m will be practically and politically not acceptable to this municipality. The total number of 2,298 families (population of 9,860) occupy the prospective restricted area, and there are 2,239 units of buildings consisting of 832 brick houses, 1,376 wooden houses and 85 apartment buildings even below El. 746.5 m. There are 103 commercial establishments and 126 sheds.

Case-2, Case-3, Case-5 and Case-7 will not be financially feasible due to very costly rock excavation. Channel excavation of about 2 million m³ (excavation-1) to 7 million m³ (excavation-2) is examined but reduction of flood water level is not significant (refer to Section 7.1.5 (2)).

Case-6 dike system only will be technically possible, but it may not be acceptable to the residents because the dike height becomes higher than 5 m (about 6.5 m for 1983 flood) and risk of dike failure becomes high. Accordingly Case-4 zoning with dike system is assessed to be most appropriate and chosen for further review.

2) Flood protection level

The following combination of flood protection level for the structural measures by a dike system is reviewed for Case-4 assuming that the restricted area will be extended up to the elevation 746.5 m.

	Design	Dike	Dike	Approximate
	water	height	volume	project cost
Design flood	level(m)	(m)	(million m3)	(US\$ million)
1983 (123 years)	750.0	4.7	1.4	86
100 year flood	749.5	4.2	1.0	61
50 year flood	748.6	3.3	0.7	40
1992 (31 years)	748.5	3.2	0.6	39

7.1.5 Non-uniform flow calculation

(1) Purpose

Non-uniform flow calculation was conducted to estimate the flood water level in the stretch from Foz do Areia to São Mateus do Sul in the Iguaçu river during the 1983 and 1992 floods, and evaluate an effect of natural river bed elevation and the reservoir water level of Foz do Areia on the flood water level in the stretch from União da Vitoria to São Mateus do Sul. It also estimates an effect of river bed excavation on the flood water level at União da Vitoria and São Mateus do Sul.

(2) Assumed calculation conditions

The design flood distribution of the stretch from Foz do Areia (at D1-78) to São Mateus do Sul (DNAEE station) for the 1983 and 1992 flood is assumed as shown in Figure-7.6 although there was a time lag among the assumed peak flood distribution in reality.

The roughness coefficient of Manning is assumed as follows:

Reservoir area (Section D1 to M1)	0.04
Low water channel in Section PV3 to B3	0.05
(the stretch of rock drops in Porto Vitoria)	
Low water channel in other sections	0.03
High water channel with trees and irregular vegetation	0.11

(Reference: Ven Te Chow," Open Channel Hydraulics", McGRAW-HILL, 1981, Section 5-7 to 5-10)

The reservoir water level at Foz do Areia (Section D1) is assumed as follows:

1983 flood	El. 744.0 m, 742.0 m, 739 m
1992 flood	El. 742.0 m, 739 m
1983 flood with riverbed excavation	El. 744.0 m, 742.0 m, 739 m

(3) River cross section data

Out of the following 119 river cross section survey data 117 sections are used for this calculation.

COPEL No. 1 Foz do Areia to União da Vitoria

41 cross sections, CN011 (D1-77) to CN381 (UV1-78) surveyed in the period 1973 - 1992

COPEL No.2 União da Vitoria to Fluviopolis

27 cross sections, CF011 (FINFL-27) to CF271 (FINFL-01) surveyed in 1995

JICA Site A Downstream to Upstream of União da Vitoria

21 cross sections, JT011 (01.GPF) to JT211 (21 GPF) surveyed in 1995

JICA Site B Fluviopolis to São Mateus do Sul

30 cross sections, JS011 (S-01.GPF) to JS301 (S-30GPF) surveyed in 1995

The location of FINFL-25 and 21.GPF and that of FINFL-01 and S-01.GPF are overlapped, and thus FINFL-25 and FINFL-01 are used for the calculation. The used cross section number and distance between sections are listed in Table-7.7. The total length of the river stretch is 242.6 km.

(4) Flood water level

The calculated flood water level is not definitive value but only notional due to insufficient topographic, hydrologic and hydraulic data and characteristics of the non-uniform flow model. However, the calculated results and their interpretation will indicate some tendency with some error range.

1) Flood water level with natural river bed

The result of flood water level of 1983 and 1992 with the existing natural river bed is shown in Table-7.7 and summarized in Table-7.8. The 1983 flood water level (with 744 m reservoir water level), 1992 flood water level (with 742 m reservoir water level), and average river bed elevation of the stretch are shown in Figure-7.7 and Table 7.7. The result indicates that the effect of the reservoir water level in Foz do Areia does not reach to Fluviopolis and São Mateus do Sul if the water level is kept at least lower than El. 744 m. The effect of reservoir water level to União da Vitoria seems to be a little if the reservoir water level is kept lower than El 744 m. It also seems to be that the key factor to cause increase of the flood water level at União da Vitoria is increase of flood discharge (size of flood) rather than the reservoir water level of Foz do Areia if the reservoir is operated within the calculated conditions. However the amount of contribution of the reservoir water level of Foz do Areia to the 1983 flood water level can not be quantified by this calculation model only.

It is guessed that the effect may be minimized if the reservoir water level is kept lower than El. 739 m. However the adopted non-uniform flow calculation model can not deal with the extent to which level the reservoir water level is to be lowered.

2) Flood water level with channel excavation

The 1983 flood water level was estimated for the conditions with two cases of channel excavation. Under excavation - 1 (with excavation) the river bed of the low water channel is excavated to El. 737.0 m at Porto Vitoria (BAT-1B) and 738.4 m at União da Vitoria (13.GPF) with average river bed gradient of about 1: 20,000. Under excavation - 2 (with large excavation) the river bed is excavated to El. 735.0 m at Porto Vitoria (BAT-1B) and 736.5 m at União da Vitoria (13.GPF) with average river bed gradient of about 1: 20,000. The cross sections BAT-1B and 13.GPF are shown in Figure - 7.8.

The used cross section number and the result for the case with reservoir water level 742 m are shown in Table-7.9. The summary of the results for the reservoir water level from 739 m to 744 m is shown in Table-7.8. It lindicates that the excavation of the low water channel in the stretch from Porto Vitoria (PV3-78) to União da Vitoria (17.GPF) does not contribute effectively to reduction of flood water level in União da Vitoria because flow areas of the low water channel is relatively small in comparison to those of the high water channel (flood plain) during a large flood. The effect of excavation does not reach to Fluviopolis and São Mateus do Sul due existence of many rock drops in the low water channel in the stretch. The calculated 1983 water levels (reservoir water level 744 m and 742 m) and the bottom excavation line are shown in Figure-7.9.

The foregoing calculation results infer that there are three key factors to cause large flood inundation in the stretch from Porto Vitoria to São Mateus do Sul:

- a) The flood discharge is extremely large comparing to the flow area of the channel of the Iguaçu river.
- b) The gradient of river bed in this stretch is very low (1: 10,000 to 20,000).
- c) There are many sections where their high river bed elevation interrupts discharge of flood in this stretch.

The reservoir operation of Foz do Areia will be required to be continued carefully taking into consideration of these characteristics in the future too.

7.2 Master Plan for Tibagi River Basin

7.2.1 Planning Criteria

Within the context of the goal and principle of water excess management of the Strategy the flood control model areas and design standard for the Master Plan are established as set out below.

(1) Model areas

Model areas are designated in the Iguaçu river basin as shown below, but no model area is designated in the Tibagi river basin, because the flood damage in these areas is minor though some flood inundation is reported in the municipalities of Irati, Ivai and Ipiranga.

Iguaçu River Basin

Region-1: Curitiba Metropolitan area

Region-2: Porto Amazonas, São Mateus do Sul

Region-4: União da Vitoria

Region-5: Rio Negro

Region-6: Foz do Iguaçu

(2) Flood Plain Management and Urban Storm Water Management

Non-structural measures are to be primarily employed for the flood prone areas in the municipalities in the Tibagi river. Integrated view of urban sewage, flood protection, storm drainage and environmental protection may be necessary for the urban area in the future after the year 2006. Environmental protection includes waste disposal control, water quality control, protection of aqua-ecosystem, and protection of riverine landscape.

(3) Design Standard

The flood control (or protection) level for zoning must be determined appropriately taking into consideration of social significance of damage level and efficiency of benefit and cost with the principle of risk and benefit for the municipalities in the Tibagi river basin.

7.2.2 Non-structural Measures

(1) General

Zoning for land use control is the most effective measures for all the flood prone areas in and around the urban areas in Paraná State (Region -1 to 8). Zoning and resettlement are a tandem for implementation and have been widely applied in the municipalities in the Iguaçu river basin, and Irati in the Tibagi river basin. Resettlement includes relocation of illegal residents occupying the river regime and legal residents in the flood prone areas.

The existing flood forecasting and warning system (FFWS) is planned to be upgraded by the provision of the new lightening censoring and rainfall monitoring system under SIMEPAR. This upgraded system will provide basic warning information required for rescue activities for the time being. This system will be necessary to be upgraded for a basin wide real time flood

management and operation to avoid artificial flood disaster in the future when number of water rescues and flood control facilities is increased significantly.

Flood proofing such as elevating ground level and structures is effective for some locally inundated areas.

Review of the operation rule of the existing and planned dams and reservoirs will be necessary taking flood control function into consideration for the integrated and effective operation in the Tibagi river in the future.

(2) Master Plan

There was a report of flood inundation in 1983 in the municipalities of Irati, Ipiranga and Ivai in the Tibagi river basin. However the damage of these municipalities was assessed to be rather light in accordance with the reconnaissance by the study team done in 1994 and 1995. Therefore, the Master Plan for the Tibagi river basin is limited to non-structural measures by zoning for land use control with resettlement and parks only as shown below.

•	Non-Structural	1st Stage	2nd Stage
Municipality	Measures	Present - 2005	2006 - 2015 onward
Irati	Zoning	Improvement of	Improvement of
	-	present method	Present method
	Evacuation	Extension of	Improvement of
		Present method	Present method
Ipiranga	Zoning	Improvement of	Improvement of
	•	present method	Present method
	Evacuation	Extension of	Improvement of
		Present method	Present method

CHAPTER 8 FUTHER STUDY REQUIREMENT

The following studies are recommended to be conducted for the municipalities where some flood control structural measures are expected to be necessary in the future:

- a) Curitiba Metropolitan Area; feasibility study on the prospective multipurpose dams such as Piraquara II, Pequeno and Alto Miringuava
- b) São Mateus do Sul; prefeasibility study and feasiblity study on non-structural and structural measures
- c) União da Vitoria; feasibility study on combination of zoning and structural measures
- d) Morretes; prefeasibility study and feasibility study on possible flood control measures

TABLES

Table 2.1 Number of People Affected by Historic Floods in Parana State

Municipality	1980	1981	1982	1983	1984	1986	1987	1988	1989	1990	1991	1992	1993
Uniao da Vitoria	1151		*3572	30003	140		815					14129	1378
São Mateus do Sul				5800	000		50		55 50			970 5001	70
Rio Negro	0000	0000	547	5502 5000	800			500	30			281	405
Curitiba	2000	2000	347	4000				300				60	403
Francisco Beltrao Ivai				3200								•••	
Porto Vitória				3150								130	
Itapejara do Oeste				3000								•	
Ipiranga				3000									
Irati		4	•	2892		160	90					700	
Coronel Vivida				2001								50	
São Jose dos Pinhais		1000	4000	2062									300
Piraquara	1000	1000	2038	2000									
Paula Freitas				1500					•			240	
Porto Amazonas				1301			80		20			352	130
Capanema				1200						•		400 150	
Tres Barras do Parana				1200								130	
Ortigueira				1002								18	
Biturona Palmas				700								10	
Vitorino				600									
Chopinzinho				600								197	
Sao Joao				600									
São Jorge do Oeste				502									
Mangueirinha				500								120	
General Cameiro				500								12	
Cruz Machado				500								1140	
Araucaria				500									
Candido de Abreu				400								450	
Reboucas				350								450 305	
Dois Vezinhos				340								228	
Quitandinha				300 250								220	
Alto Piquiri				252									
Imbituva Balsa Nova				241									
Veré				240								120	
Campo Tenente				200								240	
Castro				200									
Paulo Frontim				200								42	
Pien				200									
Mariópolis				102									
Mallet				5 0								278	
Colombo	400	400								2440			600
Santa Mariana										7660	0000		
Cambará											2000	620	
Foz do Iguacu												830	
Atonia		80											
Marilena		200									75		
Sao Pedro de Parana		230 350									7.5		
Porto Rico		330					100						
Laranjeiras do Sul Capitao L. Marques							.00			-		239	
Guarapuava												587	
Sulina					•							90	
Rio Azul												50	
Lapa												200	
N. Prata Iguacu												150	
Pinhais													400
Salto do Lontra												10	
Realeza												12	

Source: CIVIL DEFENSE,

Note: The data represent the total number of people affected by floods (total of dislodged, injured, and dead).

^{*} In two flood events, June and November 1982

Table 2.2 Number of People Distodged by Flood inundation of January-February 1995

Location	Number of People Dislodged
Curitiba	11,300
Sao Jose dos Pinhais	10,000
Pinhals	20,000
Piraquara	3,000
Campo Largo	20
Araucaria	150
Campo Grande do Sul	1,000
Sao Mateus do Sul	600
Porto Amazonas	400
Guarapuava	160
A. Tamandare	150
Jacarezinho	32
Castro	40
Uniao da Vitoria	2,410
Morretes	1,500

Table 2.3 Maximum Annual Water Level in the Parana River downstream of ITAIPU dam

Order of Date		Location of measuring point				
Magnitude		*Section R-11	Amistad Bridge			
2	13-Jul83	126.71	127,47			
9	11-Jan84	110.55	111,88			
11	17-Feb85	109.24	110.48			
12	22-May-86	107.54	108.52			
4	22-May-87	121.05	121.79			
. 7	25-May-88	110.99	111.59			
6	15-Sep89	116.27	116.92			
3	27-Jan90	121.49	122,79			
10	13-Apr91	110.51	111.84			
1	31-May-92	126.75	127,70			
5	04-Oct93	117.31	118.32			
8	23-Jun94	110.90	111.93			

Source: ITAIPU Binational

Note: *Located down stream of the confluence of Iguacu and Parana rivers

Table 2.4 Monetary Damage Caused by the Flood Inundation of 1983, 1992 and 1993 in Uniao da Vitoria and Porto Uniao

ITEMS	Sum of Losses of Uniao Vitoria and Porto Uniao				
THE PROPERTY OF THE PROPERTY O	1983	1992	1993		
Number of Houses and Building Flooded	7,537	5,266	2,502		
Emergency Relief					
Distodging and Re-todging	60,330	39,285	20,040		
Meals	1,456,767	1,097,337	92,096		
Medicine	1,181	1,185	618		
Repairing of Roads and Bridges	3,235,318	168,630	107,890		
Damages to Water Supply System	144,317	206,267	106,221		
Damages to Energy distribution System	945,020	400,694	31,116		
Repairing of Houses and Buildings	15,074,000	10,532,000	5,004,000		
Residential Furniture and other Goods	6,878,480	4,709,530	na		
Losses of Goods by the Commercial Sector	10,640,000	3,718,400	18,406		
Losses suffered by Industries	12,225,346	5,026,664	2,487,685		
Agriculture and Livestock	1,308,395	3,076,260	520,265		
Others	105,165	435,034	65,340		
TOTAL (US\$)	52,074,319	29,411,286	8,453,677		

Source: Municipal Comission of Civil Defense of respective municipalities and CORPRERI, 1994

Note: Monetary value are in USS at price level of July 1993

na: not available

Table 2.5 Partial Monetary Damage Caused by Flood Inundation of 1983 in Curitiba City

Place Affected	Houses Flooded	Partial 1	Estimate of Monetary	netary Losses		
		Repair of Houses	Movable Goods	Reconstruction of Public Infrastructures		
CURITIBA CITY Total	2065	US \$ 1,608,600	US \$ 1,032,500	US \$ 5,759,103		
Low Boqueron Sector	1335					
Cajuru Sector	171					
Barrio Alto Sector	132					
Santa Candida Sector	44					
Vila Diana Sector	65					
Barigui Sector	318					
Total Manetary Damage				US \$ 8,400,203		

Source: Institute of Investigation and Planning of Urban CURITIBA (IPPUC), 1983

Monetary value are in US\$ at price level of July 1993

Table 2.6 Partial Monetary Damage Caused by Flood of 1983 in Metropolitan Cities Except Curitiba

Municipality Affected	Number of Houses Flooded	Monetary Losses Including Repair of houses and Infrastructure
PIRAQUARA Municipality	1331	10,721,763
SAO JOSE DOS PINHAIS	2986	2,991,215
ARAUCARIA	195	316,323
BALSA NOVA	56	172,540
Tota Monetary Damage		14,201,841
Source: COMFC	Monetary value are in	US\$ at price level of June 1993

Table 2.7 Partial Monetary Damage Caused by Flood of 1993 in Curitiba City

Place Affected	Houses Flooded	Partial Estimate of Monetary Losses					
		Repair of Houses	Movable Goods	Reconstruction of Public Infrastructures			
CURITIBA CITY Total	1,220	US \$ 2,355,000	US \$ 650,450	US\$4,402,136			
Boa Vista Sector	310						
Cajuru Sector	300						
Portao Sector	550						
Santa Felicidade Sector	60			•			
Total Monetary Damage			÷	US \$ 7,407,586			

Source: Institute of Investigation and Planning of Urban CURITIBA (IPPUC)

Monetary value are in US\$ at price level of July 1993

Table 2.8 Partial Monetary Damage Caused by Flood Inundation of January 1995 in some Municipalities of the Metropolitan Area

Type of Damage	Monetary Damage
Emergency Relief	587,349
Rehabilitation of Road and Drainage in	frastructure 6,060632
Total	6,647,981
source: COMEC, 1995	Monetary value are in US \$ at level of February 1995

Note: Include only the municipalities of Sao Jose dos Pinhais, Pinhais, Piraquara and Campina Grande do Sul

Table-2.9 Estimated Unit Flood Damage Value as function of Number of Inundated Houses

						Unit: US \$/Ho	use Flooded
20 4 2 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	Uniao da	Rio Negro	Porto	Foz do	Cuntiba	Morretes	São Mateur
ITEMS	Vitoria		Amazonas	lguacu	MR		
Characteristics of Inundation and Flood Prone Area							
Relative Inundation Depth inside houses	Very High	Very High	High	Medium	Low	Low	Medium
Maximum Number of Inundation days	2 months	2 weeks	2 weeks	1 week	1 week	1 day	2 weeks
Relative economic activities and propieties value							
within the Flood Prone Area	High	High/Low	Medium/Low	Low	Medium/Low	Medium	Medium/Lov
Estimated Unit Damage Value	4.5						
Emergency Relief	350	150	150	100	100	50	150
Repairing of Roads and Bridges	350	200	200	100	200	160	200
Damage to Water Supply System	60	60	60	25	60	25	60
Damage to Energy distribution System	80	80	. 80	50	80	50	. 80
Repairing of Houses and Buildings	2500	2000	2000	1000	1000	1000	200
Residential Furniture and other Goods	1200	750	750	500	500	750	750
Losses of Goods by the Commercial Sector	1100	900	900	500	500	500	900
Losses suffered by Industries	1200	800	800	500	800	500	800
Damage to Agriculture and Livestock	350	350	350	100	100	100	350
Others Direct Monetary Damage	70	70	35	35	70	35	70
Indirect Damages as % of Direct Damages	50%	50%	50%	50%	50%	50%	50%

Source: Own estimation based on available data and field Survey

Table-2.10 Estimated Flood Inundation Damage for Curitiba Metropolitan Region

Items	•				
Flood Inundation Level similar to that of Year	Interpolated	Interpolated	1993	1983	1995
Flooding Water Level (m.a.s.l)	(no clarified)	(no clarified)	(no clarified)	(boilinel)	(no clarified)
Number of Houses and Buildings Affected	1750	3000	3950	6633	8800
Estimated Monetary Damage					
Emergency Relief	175,000	300,000	395,000	663,300	880,000
Repairing of Roads and Bridges	350,000	600,000	790,000	1,326,600	1,760,000
Damage to Water Supply System	105,000	180,000	237,000	397,980	528,000
Damage to Energy distribution System	140,000	240,000	316,000	530,640	704,000
Repairing of Houses and Buildings	1,750,000	3,000,000	3,950,000	6,633,000	8,800,000
Residential Furniture and other Goods	875,000	1,500,000	1,975,000	3,316,500	4,400,000
Losses of Goods by the Commercial Sector	875,000	1,500,000	1,975,000	3,316,500	4,400,000
Losses suffered by Industries	1,400,000	2,400,000	3,160,000	5,306,400	7,040,000
Damage to Agriculture and Livestock	175,000	300,000	395,000	663,300	880,000
Others Direct Monetary Damage	122,500	210,000	276,500	464,310	616,000
Subtotal	5,792,500	9,930,000	13,074,500	21,955,230	29,128,000
Indirect Damage (50% of Direct Damage)	2,895,250	4,965,000	6,537,250	10,977,615	14,564,000
Total Estimated Monetary Damage US \$	8,688,750	14,895,000	19,611,750	32,932,845	43,692,000

Table-2.11 Estimated Flood Inundation Damage for Uniao da Vitoria-Porto Uniao Area

Items					
Flood Inundation similar to that of Year	1982	1993	1992	1983	(Extrapolated)
Flooding Water Level (m.a.s.l)	746.06	746.86	748.51	750.03	
Number of Houses and Buildings Affected	1000	2502	5266	7537	8500
Estimated Monetary Damage					*
Emergency Relief	350,000	875,700	1,843,100	2,637,950	2,975,000
Repairing of Roads and Bridges	350,000	875,700	1,843,100	2,637,950	2,975,000
Damage to Water Supply System	60,000	150,120	315,960	452,220	\$10,000
Damage to Energy distribution System	80,000	200,160	421,280	602,960	680,000
Repairing of Houses and Buildings	2,500,000	6,255,000	13,165,000	18,842,500	21,250,000
Residential Furniture and other Goods	1,200,000	3,002,400	6,319,200	9,014,400	10,200,000
Losses of Goods by the Commercial Sector	1,100,000	2,752,200	5,792,600	8,290,700	9,350,000
Losses suffered by Industries	1,200,000	3,002,400	6,319,200	9,044,400	10,200,000
Damage to Agriculture and Livestock	350,000	875,700	1,843,100	2,637,950	2,975,000
Others Direct Monetary Damage	70,000	175,140	368,620	527,590	595,000
Subtotal	6,910,000	17,288,820	36,388,060	52,080,670	58,735,000
Indirect Damage (50% of Direct Damage)	3,455,000	8,614,410	18,194,030	26,040,335	29,367,500
Total Estimated Monetary Damage US \$	10,365,000	25,933,230	54,582,090	78,121,005	88,102,500

Table-2.12 Estimated Flood Inundation Damage for Rio Negro-Mafra Area

Items			
Flood Inundation Level similar to that of Year	1984	1992	1983
Flooding Water Level (m.a.s.l)	776.09	780.4	780.64
Number of Houses and Buildings Affected	400	2000	2200
Estimated Monetary Damage			
Emergency Relief	60,000	300,000	330,000
Repairing of Roads and Bridges	80,000	400,000	440,000
Damage to Water Supply System	24,000	120,000	132,000
Damage to Energy distribution System	32,000	160,000	176,000
Repairing of Houses and Buildings	800,000	4,000,000	4,400,000
Residential Furniture and other Goods	300,000	1,500,000	1,650,000
Losses of Goods by the Commercial Sector	360,000	1,800,000	1,980,000
Losses suffered by Industries	320,000	1,600,000	1,760,000
Damage to Agriculture and Livestock	140,000	700,000	770,000
Others Direct Monetary Damage	28,000	140,000	154,000
Subtotal	2,084,000	10,420,000	11,462,000
Indirect Damage (50% of Direct Damage)	1,042,000	5,210,000	5,731,000
Total Estimated Monetary Damage US \$	3,126,000	15,630,000	17,193,000

Table-2.13 Estimated Flood Inundation Damage for Sao Mateus do Sul

Items			
Flood Inundation similar to that of Year	1993	1992	1983
Flooding Water Level (m.a.s.l)	760.92	762.06	762.87
Number of Houses and Buildings Affected	. 15	194	1160
Estimated Monetary Damage			
Emergency Relief	2,250	29,100	174,000
Repairing of Roads and Bridges	3,000	38,800	232,000
Damage to Water Supply System	900	11,640	69,600
Damage to Energy distribution System	1,200	15,520	92,800
Repairing of Houses and Buildings	30,000	388,000	2,320,000
Residential Furniture and other Goods	11,250	145,500	870,000
Losses of Goods by the Commercial Sector	13,500	174,600	1,014,000
Losses suffered by Industries	12,000	155,200	928,000
Damage to Agriculture and Livestock	5,250	67,900	406,000
Others Direct Monetary Damage	1,050	13,580	81,200
Subtotal	78,150	1,010,740	6,043,600
Indirect Damage (50% of Direct Damage)	39,075	505,370	3,021,800
Total Estimated Monetary Damage US \$	117,225	1,516,110	9,065,400

Table 2.14 Estimated Flood Inundation Damage for Porto Amazonas

Items			
Flood Inundation Level similar to that of Year	1993	1992	1983
Flooding Water Level (m.a.s.l)	760.14	761.52	763.2
Number of Houses and Buildings Affected	30	70	260
Estimated Monetary Damage			
Emergency Relief	4,500	10,500	39,000
Repairing of Roads and Bridges	6,000	14,000	52,000
Damage to Water Supply System	1,800	4,200	15,600
Damage to Energy distribution System	2,400	5,600	20,800
Repairing of Houses and Buildings	60,000	140,000	520,000
Residential Furniture and other Goods	22,500	52,500	195,000
Losses of Goods by the Commercial Sector	27,000	63,000	234,000
Losses suffered by Industries	24,000	56,000	208,000
Damage to Agriculture and Livestock	10,500	24,500	91,000
Others Direct Monetary Damage	1,050	2,450	9,100
Subtotal	155,250	362,250	1,345,500
Indirect Damage (50% of Direct Damage)	77,625	181,125	672,750
Total Estimated Monetary Damage US \$	232,875	543,375	2,018,250

Table 2.15 Estimated Flood Inundation Damage for Foz do Iguacu Area

Items					
Flood Inundation similar to that of Year			1992		
Flooding Water Level (m.a.s.l)	119	120	126	129	130
Number of Houses and Buildings Affected	5	156	175	217	717
Estimated Monetary Damage		,	4	1	
Emergency Relief	500	15,600	17,500	21,700	71,700
Repairing of Roads and Bridges	500	15,600	17,500	21,700	71,700
Damage to Water Supply System	125	3,900	4,375	5,425	17,925
Damage to Energy distribution System	250	7,800	8,750	10,850	35,850
Repairing of Houses and Buildings	5,000	156,000	175,000	217,000	717,000
Residential Furniture and other Goods	2,500	78,000	87,500	108,500	358,500
Losses of Goods by the Commercial Sector	2,500	78,000	87,500	108,500	358,500
Losses suffered by Industries	2,500	78,000	87,500	108,500	358,500
Damage to Agriculture and Livestock	500	15,600	17,500	21,700	71,700
Others Direct Monetary Damage	175	5,460	6,125	7,595	25,095
Subtotal	14,050	438,360	491,750	609,770	2,014,770
Indirect Damage (50% of Direct Damage)	7,025	219,180	245,875	304,885	1,007,385
Total Estimated Monetary Damage US \$	21,075	657,540	737,625	914,655	3,022,155

Table 2.16 Estimated Flood Inundation Damage for Morretes

Items	-		
Flood Inundation similar to that of Year	1969	1995	Extrapolated
Flooding Water Level (m.a.s.l)	5.87	5.1	6.5
Number of Houses and Buildings Affected	1000	1925	2100
Estimated Monetary Damage	•		
Emergency Relief	50,000	96,250	105,000
Repairing of Roads and Bridges	100,000	192,500	210,000
Damage to Water Supply System	50,000	96,250	105,000
Damage to Energy distribution System	50,000	96,250	105,000
Repairing of Houses and Buildings	1,000,000	1,925,000	2,100,000
Residential Furniture and other Goods	750,000	1,443,750	1,575,000
Losses of Goods by the Commercial Sector	500,000	962,500	1,050,000
Losses suffered by Industries	500,000	962,500	1,050,000
Damage to Agriculture and Livestock	100,000	192,500	210,000
Others Direct Monetary Damage	35,000	67,375	73,500
Subtotal	3,085,000	5,938,625	6,478,500
Indirect Damage (50% of Direct Damage)	1,542,500	2,969,313	3,239,250
Total Estimated Monetary Damage US \$	4,627,500	8,907,938	9,717,750

Table-3.1 Summary of Probable Peak Discharge at Typical Discharge Reference Points Estimated by Gumbel's Method

Unit: m/s

	CHILD TO		professional and the second		*****	Mechanian St	Unit: m/s	
River	No.			Period	Catchment	F	Return Perio	<u> </u>
Basin					Area (lui)	10	50	100
itarare: Jaquadavia	1	64-242-000	Tamandua	1976-1993	1,622	646.0	998.6	1,147,7
Cinzas	2	64-360-000	Tomazina	1931-1993	2,015	477.3	692.6	783.9
	3	64-370-000	Andira	1931-1993	5,622	1,134.8	1,614.2	1,816.9
Tibagi	4	64-444-000	Uvala	1974-1993	4,450	614.4	864.8	970.7
	5	64-465-000	Tibagi	1931-1993	8,948	1,317.4	1,891.5	2,134.1
	6	64-491-000	*1) Barra Rib das Antas	1934-1993	15,600	2,313.1	3,268.9	3,673.0
	7	64-507-011	Jatalzinho (Extendido)	1931-1993	21,955	4,000.9	5,824.1	6,594.9
Pirapo	8	64-550-000	Vila Sliva Jardim	1967-1993	4,627	521.2	705.6	783.6
Ival	9	64-625-000	Tereza Cristina	1957-1992	3,572	1,753.0	2,431.0	2,717.6
	10	64-645-000	Porto Espanhol	1965-1993	8,600	3,010.2	4,144.2	4,623 .6
	11	64-675-002	Porto Bananeiras	1974-1993	24,200	5,428,1	7,072.2	7,767.2
	12	64-685-000	Porto Paralso do Norte	1953-1993	28,427	5,621.3	7,383.9	8,129.0
	13	64-693-000	Novo Porto Taquara	1974-1993	34,432	6,121.1	7,874.8	8,616.3
Piquiri	14	64-771-500	Porto Guarani	1976-1993	4,223	2,316.4	3,020.5	3,318.2
	15	64-795-000	°2) Porto do Piquiri	1970-1993	11,303	5,894.9	8,000.4	8,890.5
	16	64-820-000	Porto Formosa	1966-1993	17,500	5,403.3	7,456.4	8,324.4
	17	64-830-000	°2) Balsa do Santa Maria	1969-1993	20,982	5,305.5	7,036.2	7,767.9
lquacu:	18	65-010-000	Fazendinha	1955-1993	110	พบ	. ทบ	NU
lguacu	19	65-025-000	Guajuvita	1976-1993	2,304	491.4	746.7	854.7
	20	65-035-000	Porto Amazonas	1935-1993	3,662	558.7	793.2	892.4
	21	65-060-000	Sao Mateus do Sul	1930-1993	6,065	724.3	1.025.3	1,152.6
	22	65-310-000	Uniao da Vitoria	1930-1993	24,211	2,659.9	3,711.5	4,156.1
	23	65-895-002	Salto Osorio	1940-1993	45,824	10,438.5	15,829.2	18,108.1
	24	65-993-000	Salto Cateratas	1926-1993	67,317	18,088.6	26,676.0	30,590.9
Negro	25	65-175-000	Divisa	1964-1993	7,970	. NU	พบ	NU
Timbo	26	65-260-000	Foz do Cachoeira	1974-1993	693	283.1	422.5	481.6
Jordao	27	65-825-000	Santa Clara	1949-1993	3,913	1,408.3	2,103.6	2,397.9
Chopin	28	65-960-000	Aguas do Vere	1956-1993	6,696	2,971.8	4,354.6	4,939.2
Ribelra	29	81-200-000	Capela do Ribeira	1936-1993	7,252	1,216.2	1,707.4	1,915.1
Litoranea: Nhundiaquara	30	82-170-000	Morretes	1938-1993	217	180.4	240.8	268.4
Marumbl	31	82-195-002	Morretes	1975-1993	53	93.4	127.0	141.2

Note: *1) : The data period from 1947 to 1973 are not available.

*2) : These results are not accurate enough, because of the data source.

NU : Not used. SOURCE: COPEL

Table-4.1 Assessment of Flood Damage by Region in Paraná State

River Basin	Region	Degree of Flood Damage	Main Related Issues
Iguaçu river	From region 1 to region 5	4 for the entire basin	Resettlement, reservoirs operation
	Region I (Curitiba metropolitan area)	4	Resettlement
	Region 2 (São Mateus do Sul)	4	Resettlement
	Region 3 (Reboucas area)	2	Resettlement
	Region 4 (União da Vitoria)	5	Reservoir operation of Foz do Areia dam; Zoning and Resettlement
	Region 5 (Rio Negro area)	5	Rescutement
	Region 8 (Capanema)	2	Resettlement
Paraná river	Region 6 (Foz do Iguacu area)	3 for the entire basin	Resettlement, reservoirs operation related to ITAIPU dam, and those dams on Iguaçu river
	Upstream of ITAIPU dam	2	Resettlement
Costal basin	Region 7 (Morretes area)	3 .	
Ivai river	•	I for the entire basin	
Tibagi river		1 for the entire basin	
Others	Itarraré, Cinzas, Pirapó, Piguiri, Ribeira, Paranapanema	1 for all these basins	
i !	The degree of flood damage are Degree 5 is serious damage; De Degree 3 is medium level of da Degree 1 is negligible level; and	gree 4 is high level of mage; Degree 2 is low	damage; / level;

Table- 4.2 Assessment of Flood Damage by Basin in Parana State

River Basin	Degree of Flood Damage	Management Issues
IGUACU	High level of significance	1) Resettlement of resident in river regime and flood prone areas
		2) Zoning for restricted areas and preservation parks
		3) Operation of existing reservoirs
		4) Flood forecasting, warning, evacuation, and rescue activities
		5) Establishment of institution in charge of flood management
		6) Implementation of flood control projects
PARANA	Medium level of significance	Measures as 1), 2), 3), 4), and 5) above
IVAI	Negligible level of significance	Flood forecasting, warning, evacuation, and rescue activities
TIBAGI	Negligible level of significance	Flood forecasting, warning, evacuation, and rescue activities
Litoranea	Medium level of significance	Measures as 1), 2), 3), 4), and 5) above
Others	Negligible	Flood forecasting, warning, evacuation, and rescue activities

Table-6.1 Proposed Non-structural Flood Control Measures and Implementation Schedule for Paraná State

Region	Municipalities	Non-Structural Measures	1st Stage Present - 2005	2nd Stage 2006-2015 onward
i.	Curitiba Metropolitan Region	•Zoning	•	•
		•FFWS	Δ	0
		 Evacuation 	Δ	Δ
		 Proofing 	$\overline{\Delta}$. Δ
		 Operation Rule 	Δ	ö
2.	São Mateus do Sul	•Zoning	•	
		•FFWS	Δ	0
		 Evacuation 	•	Δ
		Proofing	Δ	Δ
	Porto Amazonas	•Zoning	•	•
	·	•FFWS	Δ	O.
	•	 Evacuation 	•	Δ
		Proofing	Δ	Δ
3.	Rebouças, Guarapuava	•Zoning		•
:	Irati, Ipiranga	•FFWS	Δ	Δ.
		 Evacuation 	•	Δ
4.	União da Vitória	•Zoning	Δ	Δ
		•FFWS	Δ	ō ·
		 Evacuation 	-	Δ
	•	Proofing	Δ	Δ
		Operation Rule	Δ	O
5.	Rio Negro	•Zoning	-	•
		•FFWS	Δ	0
		 Evacuation 	•	Δ
		•Proofing	Δ	Δ
6.	Foz do Iguaçu	•Zoning	Δ.	Δ
		•FFWS	- Δ ',	· ' O
		 Evacuation 	Δ -	Δ
		Proofing.	Δ.	- Δ
		 Operation Rule 	Δ	О
7.	Morretes	•Zoning	, · ∆	Δ
	• .	•FFWS	er en 🚅 🔭 e filosofi	The second of th
		 Evacuation 	•	Δ
		•Proofing	Δ	Δ
8.	Capanema	•Zoning	•	· •
		•FFWS	•	•
		•Evacuation	•	٨

Notes

⁽¹⁾ Zoning = zoning for land use control with resettlement and parks; FFWS = Flood Forecasting and Warning Systems; Evacuation = evacuation and rescue activities; Proofing = raising of ground level and buildings, etc.; Operation Rule = operation rule for reservoirs, flood control facilities, etc.

^{(2) - =} Extention of present method; Δ = Improvement of present method; Δ = Employment of new concept

Table-6.2 Proposed Structural Measures and Implementation Schedule for Flood Control

Region	Municipality	Structural Measures	Project Cost (USS 10 ⁶)	Implementat	Implementation Schedule
				1st Stage Present - 2005	2006 - 2015 convent
≓	Curitiba Metropolitan Area	Continuation of PROSAM - 15 km long channel excavation (about 1.3 million m³); - landscape restoration and park development of river bank area; - Irai dam for flood control and to guarantee 1.8 m³/s to Curitiba water supply; - relocation and resettlement of 1,400 houses located in risky areas including occupying river flood plains; - expropriation of 7,000 plots of land and rights needed for environment protection along rivers and environmentally sensitive areas;	Total 34.3 excluding Irai dan (1992 price)	0	
		Extension of PROSAM - channel excavation by Curitiba municipality - Piraquara II, Pequeno, Alto Miringuava dams for water supply with flood control function	¥Z	0 4	0
4	Sao Mateus do Sul	Dike system with a sluice	11.11	•	0
4,	Unixo da Vitoria	Dike system with sluices	85.9	0	
۲.	Morretes	channel improvement and dike	NA A	٥	0

NA: Not available in this study phase

O: Full operation

Note: A: Partial Operation

Table-6.3 Example of Rational Methods of Flood Control Planning and Measures (1/2)

Nation	Law Concerning Flood Control	Structural Measures & Assessment	Non-Structural Measures & Assessment	Basin Development & Flood Control Level
Austria	Federal law concerning water use right (1959) Federal law concerning assistance to construction of water use facilities	 Priority on river improvement of the rivers experiencing large flood damage Flood control plan varies with adjustment of demand of people 	No permission of development of flood prone areas Disaster fund automatic water level gauging at major sites	Human tife and properties with high cultural and economic value (1/100) Transportation facilities depends importance and damage valve Agricultural land depends risk level
Russia	Basic laws for water	 Multipurpose reservoir, dike Comprehensive federal plan for water use and flood control 	Identification of dangerous inundation areas (National insurance)	Minimization of total amount of investment (1/5~1/100)
France	River law Domestic navigation law	Optimum improvement with traditional and modern technologies and calculations Multipurpose dam	Atomization of flood warning system Rainfall radar	Benefit shall be larger than investment
Greece	No information	Dike, dam, reservoir, drainage	No information	Urban (1/1,000) Industrial land (1/100) Agricultural land (1/50)

Source: United Nations, "Rational Method of Flood Control Planning in River Basin Development," 1976.

Table-6.3 Example of Rational Methods of Flood Control Planning and Measures (2/2)

Nation	Law Concerning Flood Control	Structural Measures & Assessment	Non-Structural Measures & Assessment	Basin Development & Flood Control Level
USA	1936 law (Flood control of navigable rivers, flood control against hurricane, flood insurance, official announcement of flood inundation areas)	Continuous investment since 1936	Flood inundation area map Flood forecasting and warning system	Implementation based on the effectiveness of flood control facilities (flood damage reduction)
Rumania	• Water law (1974)	Dike, multipurpose dam, afforestation and watershed management	 Identification of flood inundation area (by special law) Flood forecasting and warning system is effective for very large rivers 	Determination by cost-benefit analysis depending on importance of society and economic facilities
Japan	 River law Manual for river works 	Comprehensive flood control plan (dike, multipurpose dam, retarding basin, flood way)	Comprehensive flood control plan with structural measures	 Flood control level is determined based on social significance and necessity of the region (generally 1/100~1/200)

Source: United Nations, "Rational Method of Flood Control Planning in River Basin Development," 1976.

Table-7.1 Proposed Non-structural Flood Control Measures and Implementation Schedule for Iguaçu River Basin

Region	Municipalities	Non-Structural Measures	1st Stage Present - 2005	2nd Stage 2006-2015 onward
1.	Curitiba Metropolitan Region	•Zoning	2.005	2000-2013 Oliward
	•	FFWS	Δ	ó
		*Evacuation	Δ	Δ
		•Proofing	Δ	Δ Δ
		Operation Rule	Δ	Ö
		operation read	ч	. •
2.	São Mateus do Sul	•Zoning		: <u> </u>
		FFWS	Δ .	ó
		•Evacuation		
		•Proofing	Δ	Δ
	$(1, \dots, 1, \dots, k)$	Tioomig	Δ	Δ
* .*	Porto Amazonas	•Zoning		
	· Ow · · · · · · · · · · · · · · · · · ·	•FFWS	Ā	
		Evacuation	Δ .	. 0
			-	Δ
		Proofing	-Δ	[Δ
3.	Rebouças, Guarapuava	•Zoning		
3.	Irati, Ipiranga	•FFWS	•	•
	ituo, ipa anga	Evacuation	Δ	Δ
	•	Bvacuation	•	Δ
4.	União da Vitória	•Zoning	Δ	•
- •		•FFWS		Δ
	•	•Evacuation	Δ	O
	A. Carrier and A. Car		•	Δ
		•Proofing	Δ	Δ
		Operation Rule	۸	0
5.	Rio Negro	7		
٥.	KIO REGIO	•Zoning	•	•
		·FFWS	Δ	0
		•Evacuation	-	Δ
		•Proofing	Δ	Δ
6.	Foz do Iguaçu			
0.	r os oo igaaya	· Zoning	Δ	Δ
		FFWS	Δ	O
		Evacuation	Δ	, Δ
		Proofing	Δ	<u> </u>
		 Operation Rule 	Δ ,	O
8.	Cananama	a.		•
0.	Capanema	•Zoning		
		•FFWS	•	•
· · · · · · · · · · · · · · · · · · ·		Evacuation	•	Δ

Notes

⁽¹⁾ Zoning = zoning for land use control with resettlement and parks;

FFWS = Flood Forecasting and Warning Systems; Evacuation = evacuation and rescue activities;

Proofing = raising of ground level and buildings, etc.; Operation Rule = operation rule for reservoirs,

flood control facilities, etc.

^{(2) - =} Extention of present method; Δ = Improvement of present method; O = Employment of new concept

Table-7.2 Proposed Structural Measures and Implementation Schedule for Iguaçu River

Implementation Schedule 1st Stage 2nd Stage 1996 ~ 2000 2001 ~ 2005 2006 ~ 2010 2011 ~ 2015				
Project Cost (USS 10°)	Total 34.3 (1992 price)	lity Not Available	11.1	85.9
Structural Measures	Continuation of PROSAM	Extension of PROSAM - channel excavation by Curitiba municipality - Piraquara II, Pequeno, Alto Miringuava dams with flood control function	Dike system with a sluice	Dike system with sluices
Municipality	Curitiba Metropolitan Area		São Mateus do Sul	União da Vitoria
Region	ri		તં	4

Table-7.3 Historic Peak Flood Discharges at União da Vitoria 1891-1995

				-			
	year	Peak Flood	R¢turn		year	Peak Flood	Return
		Discharge	Period			Discharge	Period
		m3/s	years			m3/s	years
1	1983	4,980	122.59	39	1966	1,400	1.81
2	1891	4,030	61.30	40	1994	1,300	1.77
3	1905	3,900	40.86	41	1969	1,290	1.72
4	1992	3,810	30.65	42	1941	1,270	1.68
5	1911	3,440	24.52	43	1936	1,240	1.64
6	1935	3,270	20.43	44	1956	1,230	1.60
7	1957	2,680	13.88	45	1945	1,220	1.56
8	1993	2,640	11.49	46	1967	1,210	1.52
9	1971	2,430	9.80	47	1986	1,210	1.49
10	1954	2,400	8.55	48	1942	1,200	1.46
11	1938	2,380	7.58	49	1951	1,190	1.43
12	1995	2,340	6.81	50	1991	1,100	1.40
13	1946	2,330	6.18	51	1952	1,090	1.37
14	1982	2,160	5.66	52	1960	1,090	1.34
15	1990	2,110	5.21	53	1963	1,050	1.31
16	1987	2,080	4.84	54	1977	1,050	1.29
17	1955	1,980	4.51	55	1958	1,030	1.27
18	1984	1,850	4.22	56	1974	1,020	1.24
19	1932	1,830	3.97	57	1962	1,000	1.22
20	1972	1,830	3.75	58	1944	956	1.20
21	1981	1,800	3.55	59	1964	914	1.18
22	1989	1,800	3.37	60	1943	902	1.16
23	1947	1,780	3.21	61	1933	836	1.14
24	1961	1,780	3.06	62	1934	820	1.12
25	1973	1,760	2.93	63	1978	816	1.10
26	1939	1,720	2.81	64	1959	804	1.08
27	1975	1,710	2.69	65	1949	800	1.06
28	1988	1,680	2.59	66	1968	745	1.05
29	1980	1,640	2.49	67	1985	734	1.03
30	1950	1,610	2.40	68	1940	568	1.02
31	1979	1,560	2.32				
32	1953	1,550	2.24			* .	
33	1970	1,500	2.17				
34	1965	1,470	2.10			· ·	
35	1931	1,450	2.04				
36	1948	1,440	1.98	1		•	
37	1976	1,420	1.92		•		
38	1937	1,400	1.87	.ء ا			

Table-7.4 6-10 Day Rainfall by Thiessen Method for Upstream of União da Vitoria

¢ dana	7 days	8 days	9 days	10 days			
6 days		131.6	143.8	171.6			
1941 118.7	126,6	116.7	130	152.8			
102.5	109			113.2	•		
81.4	83	105.9	111.4				
93	95.2	105.9	106	106.1			
115.6	124.8	127.3	133.1	134.9			
163	174.3	185	193.6	200.6			
137.8	145.3	167.9	178.1	183.5			
102.4	114.9	124.5	134.3	142.2			
111.4	118	120,5	120.7	121.6			
130.8	132.1	134.7	139.2	144			
152.6	176.3	182.4	184.5	185.8	·		
114.3	114.3	114.3	123.7	136.1		•	
95.3	108.5	110	110.6	123.1			
191.6	204.7	219.2	238.9	252	2		
141	142.2	147.1	166.5	171	Conton	This same Waight	
106.7	120.1	128.5	135.2	144.2	Station	Thiessen Weight	-
168.3	171.6	174	178.4	182.4	 Porto Amazonas 	0.08	
. 79	82.4	94	106.6	127.4	2) Rio da Varzea	•	
78.6	90.5	94.9	95	95	dos Limas	0.14	
124.5	127.2	127.3	127.3	127.4	3) Curitiba	0.10	
127.2	141.4	153.7	153.9	154.4	4) Sao Mateus		
99.5	100.8	101.7	104.5	109.9	do Sul	0.25	
122	129.2	132.7	134.3	145.3	5) Rio Negro	0.23	
113.1	119.8	120.4	123.6	127.7		0.23	
115.5	118.6	128.9	138.6	146.2	6) Uniao da	0.00	
94.1	103.7	111.7	118.9	122.4	Vitoria	0.20	-
84.1	90.9	94	105.4	109.2		1.00	
113.8	131.7	142.6	149.9	154.1			
114.6	114.6	115.5	118	131.7			
172	173.5	178.4	202.1	216.2	Source: COPEL		
100.6	105	107.7	107.8	108.1	000100. COLLE		
			172.4	179.3			
131.9	132.7	154.9		135,.			
110.3	111.4	120.1	133.5				
101.6	104.9	106.8	106.9	106.9			
134.1	135.1	135.8	135.9	135.9			
98.7	108.5	112.7	114.7	123			
110.4	118.5	125.1	133.2	137.3			
79.8	94.3	101.9		122.5			
176	189.1	194.5	200.5	203.6		•	
144.4	150.4	151.7	161.3	177.9			
80.5	122.2	127.1	127.7	145.9			
155.9	162.2	171.5	172.7	179.2	•		
319	354.6	380.2	383.7	383.8		·	
163.5	176.2	176.4		176.9			
91.6	94.1	103.5	112.6	116.4			
108.2	108.3	116.4	137	140.9			
149.6	159.1	164.8	199	204.2			
106.4	130.9	131.5	131.6	131.7			
161.8	169.5	175.7	177.3	177.4		•	
178.2	179.6	179.6	190,7	194			
127.7	128.8	134.8	139	142.3			
321.3	323.3	323.6	364	369.9	XMED 130.2 138.4	144.8 152.8 159.3	
179.6	183.1	184.2	203.8	222.6	S 47.9 50.1	51.4 54.9 55.2	
93.6	93.8	93.9	96.8	105.3	C.VAR3681 .3619	.3552 .3596 .3464	
1995 171.8	192.5	197.6	203.4	214	C.ASS 2.338 2.470	2.622 2.504 2.355	
'							

Table-7.5 Frequency Analysis of 6-10 day
Rainfall Upstream of Uniao da Victoria

123.1 169.0	129.0	1260	
169.0		136.0	142.4
	176.1	186.3	193.0
203.7	211.8	224.4	231.3
238.4	247.4	262.5	269.6
249.6	258.9	274.7	281.9
258.7	268.3	284.8	292.0
284.3	294.5	312.8	320.2
319.0	330.2	350.9	358.4
399.6	413.0	439.3	447.3
434.4	448.6	477.4	485.6
549.7	567.0	603.9	612.7
	249.6 258.7 284.3 319.0 399.6	249.6 258.9 258.7 268.3 284.3 294.5 319.0 330.2 399.6 413.0 434.4 448.6	249.6 258.9 274.7 258.7 268.3 284.8 284.3 294.5 312.8 319.0 330.2 350.9 399.6 413.0 439.3 434.4 448.6 477.4

SOURCE: COPEL

AJUSTE DA	A DISTRIBUICAC	DEXPONDEN	CIAL AOS DA	DOS	
PARAMET	ROS ESTIMADO	8 PELO METO	DO DOS MO	MENTOS	
	6 days	7 days	8 days	9 days	10 days
XMED	130.1982	138.2436	144.7873	152.8145	159.3836
\$	47.92067	50.09318	51.42868	54.94634	55.21326
ВЕГА	47.92067	50.09318	51.42868	54.94634	55.21320
ALFA	82.27751	88.33045	93.35859	97.86821	104.1704

Table-7.6 Annual Average Probable Flood Damage

Return	Peak Flood	Annual Mean	Peak Flood Annual Mean Annual Mean	Probable Flood	Probable Flood Average Probable Average Probable	Average Probable	Accumulated
Period	Discharge	Prability of	Occurrence	Damage	Flood Damage	Flood Damage	Average Probable
		Exceedence	Probability	at a Event	of Two Event		Flood Damage
year	m3/s			×10 3 USS	x103 USS	x10 3 USS	x10 3 USS
1.05	745	0.9524		0			
	1436	0.5000	0.4524	6,254	3,127	1,415	1,415
· ·	2079	0.2000	0.3000	12,076	9,165	2,750	4,164
10	722	0.1000	0.1000	23,894	17,985	1,799	5,963
8	3117	0.0500	0.0500	37,622	30,758	1,538	7,501
30.65(1992)	3810	0.0326	0.0174	54,582	46,102	801	8,302
20	3921	0.0200	0.0126	56,807	55,694	703	9,005
100	4610	0.0100	0.0100	70,679	63,743	637	9,642
122.59(1983)	7086	0.0082	0.0018	78,121	74,400	137	9.779
200	5382	0.0050	0.0032	86.209	82,165	259	10,039

Table-7.7 River Cross Section Number and Calculated Flood Water Level in 1983 and 1992 (1/2)

No. Continue Con	ed 1992 Level n) 742,000 742,001 742,000 742,000 741,996 742,003 741,959 742,049 742,052 741,947 742,039 743,256 743,735 745,307
name (m) (m) Elevation (m) (m) (m) D1-77 CN011 9230.000 0.000 664.129 744.000 D2-77 CN021 9230.000 9230.000 676.636 744.001 D3-77 CN031 12300.000 18460.000 681.506 744.001 D4-77 CN041 10000.000 30760.000 692.566 744.004 D5-77 CN051 11540.000 40760.000 701.539 744.003 PV8-78 CN061 2160.000 52300.000 715.789 744.012 PV7-78 CN071 3840.000 54460.000 722.196 743.956 PV6-78 CN081 400.000 58300.000 723.849 744.061 PV5-78 CN091 1400.000 58700.000 727.227 744.065 PV5-78 CN101 1420.000 60100.000 726.772 743.939 PV4-78 CN111 100.000 61520.000 727.191 744.032 M1-73	742.000 742.000 742.000 742.000 742.000 741.996 742.003 741.959 742.049 742.052 741.947 742.039 741.793 743.256 743.735
D1-77 CN011 9230.000 0.000 664.129 744.000 D2-77 CN021 9230.000 9230.000 676.636 744.001 D3-77 CN031 12300.000 18460.000 681.506 744.001 D4-77 CN041 10000.000 30760.000 692.566 744.004 D5-77 CN051 11540.000 40760.000 701.539 744.003 PV8-78 CN061 2160.000 52300.000 715.789 744.012 PV7-78 CN071 3840.000 54460.000 722.196 743.956 PV6-78 CN081 400.000 58700.000 723.849 744.061 PV5A-78 CN091 1400.000 58700.000 727.227 744.065 PV5-78 CN101 1420.000 60100.000 726.772 743.939 PV4-78 CN111 100.000 61520.000 727.191 744.032 M1-73 CN121 3040.000 61620.000 735.357 745.014 PV2-	742.000 742.001 742.000 742.000 741.996 742.003 741.959 742.049 742.052 741.947 742.039 741.793 743.256 743.735
D2-77 CN021 9230.000 9230.000 676.636 744.001 D3-77 CN031 12300.000 18460.000 681.506 744.001 D4-77 CN041 10000.000 30760.000 692.566 744.004 D5-77 CN051 11540.000 40760.000 701.539 744.003 PV8-78 CN061 2160.000 52300.000 715.789 744.012 PV7-78 CN071 3840.000 54460.000 722.196 743.956 PV6-78 CN081 400.000 58300.000 723.849 744.061 PV5A-78 CN091 1400.000 58700.000 727.227 744.065 PV5-78 CN101 1420.000 60100.000 726.772 743.939 PV4-78 CN111 100.000 61520.000 727.191 744.032 M1-73 CN121 3040.000 61620.000 735.357 745.014 PV2-78 CN141 500.000 65300.000 737.397 745.375	742.001 742.000 742.000 741.996 742.003 741.959 742.049 742.052 741.947 742.039 741.793 743.256 743.735
D3-77	742.000 742.000 741.996 742.003 741.959 742.049 742.052 741.947 742.039 741.793 743.256 743.735
D4-77	742,000 741,996 742,003 741,959 742,049 742,052 741,947 742,039 741,793 743,256 743,735
D5-77	741.996 742.003 741.959 742.049 742.052 741.947 742.039 741.793 743.256 743.735
D5-77	742.003 741.959 742.049 742.052 741.947 742.039 741.793 743.256 743.735
PV8-78 CN061 2160.000 52300.000 715.789 744.012 PV7-78 CN071 3840.000 54460.000 722.196 743.956 PV6-78 CN081 400.000 58300.000 723.849 744.061 PV5A-78 CN091 1400.000 58700.000 727.227 744.065 PV5-78 CN101 1420.000 60100.000 726.772 743.939 PV4-78 CN111 100.000 61520.000 727.191 744.032 M1-73 CN121 3040.000 61620.000 731.129 743.805 PV3-78 CN131 640.000 64660.000 735.357 745.014 PV2-78 CN141 500.000 65300.000 737.397 745.375 PV1-78 CN151 70.000 65800.000 740.164 746.637 BAT-1A CN152 70.000 65870.000 739.703 746.622 BAT-1B CN153 60.000 66000.000 738.286 746.716	741.959 742.049 742.052 741.947 742.039 741.793 743.256 743.735
PV7-78 CN071 3840,000 54460,000 722,196 743,956 PV6-78 CN081 400,000 58300,000 723,849 744,061 PV5A-78 CN091 1400,000 58700,000 727,227 744,065 PV5-78 CN101 1420,000 60100,000 726,772 743,939 PV4-78 CN111 100,000 61520,000 727,191 744,032 M1-73 CN121 3040,000 61620,000 731,129 743,805 PV3-78 CN131 640,000 64660,000 735,357 745,014 PV2-78 CN141 500,000 65300,000 737,397 745,375 PV1-78 CN151 70,000 65800,000 740,164 746,637 BAT-1A CN152 70,000 65870,000 739,703 746,622 BAT-1B CN153 60,000 65940,000 738,286 746,716	742.049 742.052 741.947 742.039 741.793 743.256 743.735
PV6-78 CN081 400.000 58300.000 723.849 744.061 PV5A-78 CN091 1400.000 58700.000 727.227 744.065 PV5-78 CN101 1420.000 60100.000 726.772 743.939 PV4-78 CN111 100.000 61520.000 727.191 744.032 M1-73 CN121 3040.000 61620.000 731.129 743.805 PV3-78 CN131 640.000 64660.000 735.357 745.014 PV2-78 CN141 500.000 65300.000 737.397 745.375 PV1-78 CN151 70.000 65800.000 740.164 746.637 BAT-1A CN152 70.000 65870.000 739.703 746.723 BAT-1B CN153 60.000 66000.000 738.286 746.716	742.052 741.947 742.039 741.793 743.256 743.735
PV5A-78 CN091 1400.000 58700.000 727.227 744.065 PV5-78 CN101 1420.000 60100.000 726.772 743.939 PV4-78 CN111 100.000 61520.000 727.191 744.032 M1-73 CN121 3040.000 61620.000 731.129 743.805 PV3-78 CN131 640.000 64660.000 735.357 745.014 PV2-78 CN141 500.000 65300.000 737.397 745.375 PV1-78 CN151 70.000 65800.000 740.164 746.637 BAT-1A CN152 70.000 65870.000 740.563 746.622 BAT-1B CN153 60.000 65940.000 738.286 746.743 BAT-2 CN154 50.000 66000.000 738.286 746.716	741.947 742.039 741.793 743.256 743.735
PV5-78 CN101 1420.000 60100.000 726.772 743.939 PV4-78 CN111 100.000 61520.000 727.191 744.032 M1-73 CN121 3040.000 61620.000 731.129 743.805 PV3-78 CN131 640.000 64660.000 735.357 745.014 PV2-78 CN141 500.000 65300.000 737.397 745.375 PV1-78 CN151 70.000 65800.000 740.164 746.637 BAT-1A CN152 70.000 65870.000 740.563 746.622 BAT-1B CN153 60.000 65940.000 739.703 746.743 BAT-2 CN154 50.000 66000.000 738.286 746.716	742.039 741.793 743.256 743.735
PV4-78 CN111 100.000 61520.000 727.191 744.032 M1-73 CN121 3040.000 61620.000 731.129 743.805 PV3-78 CN131 640.000 64660.000 735.357 745.014 PV2-78 CN141 500.000 65300.000 737.397 745.375 PV1-78 CN151 70.000 65800.000 740.164 746.637 BAT-1A CN152 70.000 65870.000 740.563 746.622 BAT-1B CN153 60.000 65940.000 739.703 746.743 BAT-2 CN154 50.000 66000.000 738.286 746.716	741.793 743.256 743.735
M1-73 CN121 3040.000 61620.000 731.129 743.805 PV3-78 CN131 640.000 64660.000 735.357 745.014 PV2-78 CN141 500.000 65300.000 737.397 745.375 PV1-78 CN151 70.000 65800.000 740.164 746.637 BAT-1A CN152 70.000 65870.000 740.563 746.622 BAT-1B CN153 60.000 65940.000 739.703 746.743 BAT-2 CN154 50.000 66000.000 738.286 746.716	743.256 743.735
PV3-78 CN131 640.000 64660.000 735.357 745.014 PV2-78 CN141 500.000 65300.000 737.397 745.375 PV1-78 CN151 70.000 65800.000 740.164 746.637 BAT-1A CN152 70.000 65870.000 740.563 746.622 BAT-1B CN153 60.000 65940.000 739.703 746.743 BAT-2 CN154 50.000 66000.000 738.286 746.716	743.256 743.735
PV2-78 CN141 500.000 65300.000 737.397 745.375 PV1-78 CN151 70.000 65800.000 740.164 746.637 BAT-1A CN152 70.000 65870.000 740.563 746.622 BAT-1B CN153 60.000 65940.000 739.703 746.743 BAT-2 CN154 50.000 66000.000 738.286 746.716	
PV1-78 CN151 70.000 65800.000 740.164 746.637 BAT-1A CN152 70.000 65870.000 740.563 746.622 BAT-1B CN153 60.000 65940.000 739.703 746.743 BAT-2 CN154 50.000 66000.000 738.286 746.716	
BAT-1A CN152 70.000 65870.000 740.563 746.622 BAT-1B CN153 60.000 65940.000 739.703 746.743 BAT-2 CN154 50.000 66000.000 738.286 746.716	
BAT-1B CN153 60.000 65940.000 739.703 746.743 BAT-2 CN154 50.000 66000.000 738.286 746.716	745.312
BAT-2 CN154 50.000 66000.000 738.286 746.716	745,447
	745.439
B3-84 CN161 260.000 66050.000 738.367 746.730	745.458
R5-94 CN171 1200.000 66310.000 737.910 746.759	745.497
R4-94 CN171 1200.000 67510.000 743.174 746.939	745.646
M2-73 CN191 690.000 67810.000 741.312 747.011	745.707
UV8-78 CN201 1400,000 68500,000 740,570 747,117	745.835
UV7-78 CN211 1400.000 69900.000 742.092 747.321	746.017
UV6-78 CN21 1600.000 71300.000 743.596 747.505	746,171
M3-73 CN231 1400.000 72900.000 740.348 747.619	746.337
UV5A-78 CN241 300.000 74300.000 743.269 747.634	746.402
R3-94 CN251 300.000 74500.000 742.154 748.296	. 747.026
UV5-78 CN261 2170.000 74900.000 742.690 748.162	746.910
R2-94 CN271 230.000 77070.000 743.089 748.647	747.330
UV4-78 CN281 1200.000 77300.000 743.125 748.665	747.348
M4-73 CN291 1400.000 78500.000 742.782 748.761	747,445
UV3-78 CN301 1368.559 79900.000 743.316 748.952	747.624
	747.778
	747.866
	747.868
	747.866
	747.935
	747.966
04.GPF JT041 725.542 84174.458 741.043 749.295 M5-73 CN331 308.594 84900.000 744.169 749.374	748.021
	748.021
	748.062
	748.171 748.184
	748.203
	748.244
	748.181
	748.181
	748.355
	748.435
	748.562
13.GPF JT131 758.182 92941.818 743.317 750.033	748.638
UV-92 CN371 214.332 93700.000 746.141 750.110	748.702
14.GPF JT141 185.668 93914.332 742.433 750.072	748.700
UV1-78 CN381 804.008 94100.000 746.548 750.107	748.710
15.GPF JT151 295.992 94904.008 742.873 750.222	748,824
FINFL-27.ENT CF011 712.720 95200.000 744.477 750.290	
16.GPF JT161 990.062 95912.720 742.282 750.274	748.884
17.GPF JT171 1005.718 96902.782 743.076 750.323	

Table-7	1,7 River Cr	oss Section Nun	iber and Calcul		Level in 1983 and 1	992 (2/2)
18.GPF	JT181	291.500	97908.500	742.648	750.463	749.073
FINFL-26.ENT	CF021	699.237	98200,000	744.311	750.551	749.148
19.GPF	JT191	998.171	98899.237	742.981	750.510	749.126
20.GPF	JT201	1002.592	99897.408	743.280	750.645	749.251
FINFL-25.ENT	CF031	3300.000	100900.000	744.780	750.722	749.329
FINFL-24.ENT	CF041	3500.000	104200.000	745.096	750.904	749.532
FINFL-23.ENT	CF051	9950.000	107700.000	745.533	751.186	749.803
FINFL-22.ENT	CF061	4650.000	117650.000	748.157	751.839	750.505
FINFL-21.ENT	CF071	3900.000	122300.000	747.286	752.180	750.886
FINFL-20.ENT	CF081	5000.000	126200.000	748.311	752.492	751.220
FINFL-19.ENT	CF091	2000.000	131200.000	747.692	752.797	751.559
FINFL-18.ENT	CF101	2000.000	133200.000	747.528	752.976	751.750
FINFL-17.ENT	CFIII	5750.000	135200.000	749.973	753.390	752.092
FINFL-16.ENT	CF121	3250.000	140950.000	746.560	754.008	752.715
FINFL-15.ENT	CF131	3500.000	144200.000	748.492	754.361	753.055
FINFL-14.ENT	CF141	2250.000	147700.000	749.700	754.766	753.470
FINFL-13.ENT	CF151	3250.000	149950.000	749.288	755.005	753.706
FINFL-12.ENT	CF161	5050.000	153200.000	750.727	755.186	753.905
FINFL-11.ENT	CF171	3050.000	158250.000	750.188	755.587	754.317
FINFL-10.ENT	CF181	2950.000	161300.000	751.385	755.928	754.639
FINFL-09.ENT	CF191	4550.000	164250.000	751.790	756.099	754.822
FINFL-08.ENT	CF201	4400.000	168800.000	751.703	756.458	755.196
FINFL-07.ENT	CF211	4850.000	173200.000	752.721	756.880	755.627
FINFL-06.ENT	CF221	1700.000	178050.000	751.088	757.383	756.123
FINFL-05.ENT	CF231	3950.000	179750.000	753.563	757.525	756.261
FINFL-04.ENT	CF241	2225.000	183700.000	753.115	757.675	756.421
FINFL-03.ENT	CF251	4775.000	185925.000	750.880	757.901	756.639
FINFL-02.ENT	CF261	7000.004	190700.000	751.505	758.204	756.938
FINFL-01.ENT	CF271	1805.800	197700.004	754.577	758.979	757.671
\$-02.GPF	JS021	1552.490	199505.804	752.546	759.146	757.837
\$-03.GPF	JS031	1762.610	201058.294	752.867	759.259	757.953
S-04.GPF	JS041	2644.140	202820.904	753.358	759.402	758.101
S-05.GPF	JS051	2191,590	205465.044	752.866	759.594	758.306
S-06.GPF	JS061	2388.090	207656.634	752.581	759.847	758.533
S-07.GPF	JS071	3521.930	210044.724	752.193	759.941	758.632
S-08.GPF	JS081	1817.230	213566.654	753.634	760.111	758.823
S-09.GPF	JS091	2028.390	215383.884	755.011	760.243	758.990
S-10.GPF	JS101	2469.990	217412.274	755.567	760.557	759.399
S-11.GPF	JS111	1076.570	219882.264	755.280	761.053	760.009
S-12.GPF	JS121	2390.910	220958.834	756.625	760.995	760.012
S-13.GPF	JS131	2077.590	223349.744	756.995	762.640	761.628
S-14.GPF	JS141	2319.950	225427.334	757.179	763.006	761.987
S-15.GPF	JS151	1916.490	227747.284	756.478	763.248	762.227
S-16.GPF	JS161	2866.920	229663.774	757.104	763.458	762.426
S-17.GPF	JS171	1134.620	232530.694	756.811	763.650	762.615
S-18.GPF	JS181	1295.530	233665.314	757.203	763.705	762.670
S-19.GPF	JS191	737.220	234960.844	758.317	763.772	762.747
S-20.GPF	JS201	998.120	235698.064	757.558	763.924	762.890
S-21.GPF	JS211	391.560	236696.184	757.216	764.096	763.050
S-22.GPF	JS221	1191.210	237087.744	757.659	764.117	763.072
S-23.GPF	J\$231	487.750	238278.954	758.231	764.195	763.156
S-24.GPF	JS241	557.770	238766.704	757.782	764.268	763.227
S-25.GPF	JS251	436.940	239324.474	758.438	764.334	763.285
S-26.GPF	JS261	516.210	239761.414	757.891	764,441	763.379
S-27.GPP	JS271	512,410	240277.624	757.985	764.447	763.385
S-28.GPP	JS281	919.310	240790.034	759.013	764.440	763.382
S-29.GPF	JS291	875.150	241709.344	757.964	764.479	763.425
S-30.GPF	JS301	0.000	242584.494	758.218	764.482	763.432
						· · · · · · · · · · · · · · · · · · ·

Table-7.8 Summany of Calculated Flood Water Level

without Excavation					Unit: water level in meters	
Case	Flood	Foz do Arcia (D1-77) CN011	Porto Vitoria (BAT-1B) CN153	Uniao da Vitoria (14.GPF) JT141	Fluviopolis (FINFL-01) CF271	Sao Mateus do Sul (S-25.GPF) JS251
A1	1983	744.0	746.7	750.1	759.0	764.3
A2	1983	742.0	746.3	750.0	759.0	764.3
A3	1983	739.0	746.1	749.9	759.0	764.3
A4	1992	742.0	745.4	748.7	757.7	763.3
A5	1992	739.0	745.3	748.7	757.7	763.3

83 Flo	od Water Le	Unit: water level in meters			
Case	Foz do Arcia (D1-77) CN011	Porto Vitoria (BAT-1B) SC153	Uniao da Vitoria (14,GPF) JS141	Fluviopolis (FINFL-01)	Sao Mateus do Sul (S-25.GPF)
rcavali	on -1 (with exc	ravation)			
,,caran	744.0	746.1	749.8	759.0	764.3
	742.0	745.4	749.7	759.0	764.3
	739.0	745.3	749.7	759.0	764.3
xcavati	on - 2 (with lar	ge excavation)			
	744.0	745.7	749.5	759.0	764.3
	742.0	744.7	749.4	759.0	764.3
	739.0	744.2	749.3	759.0	764.3

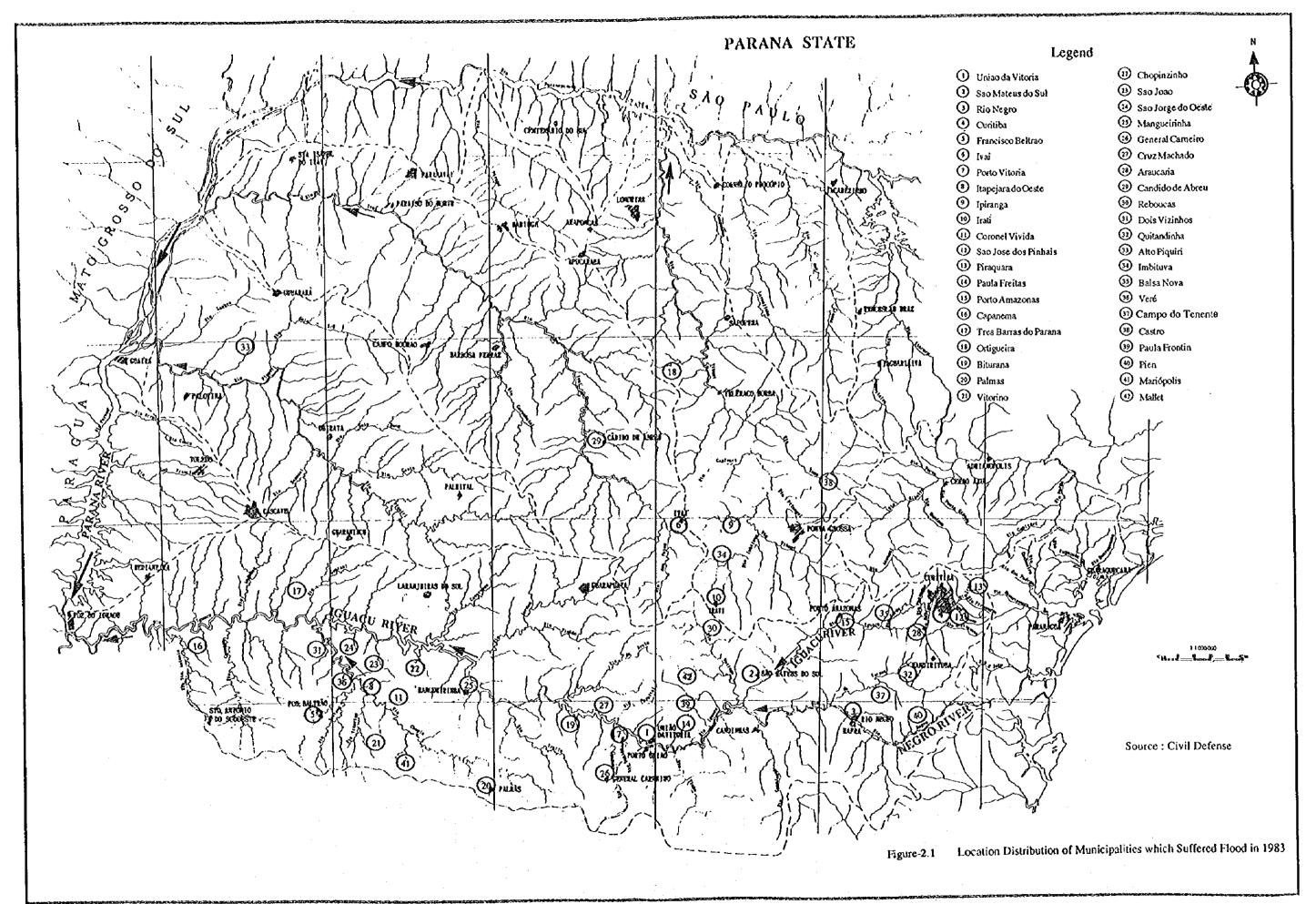
Table-7.9 Calculated Flood Water Level in 1983 with Excavations (1/2)

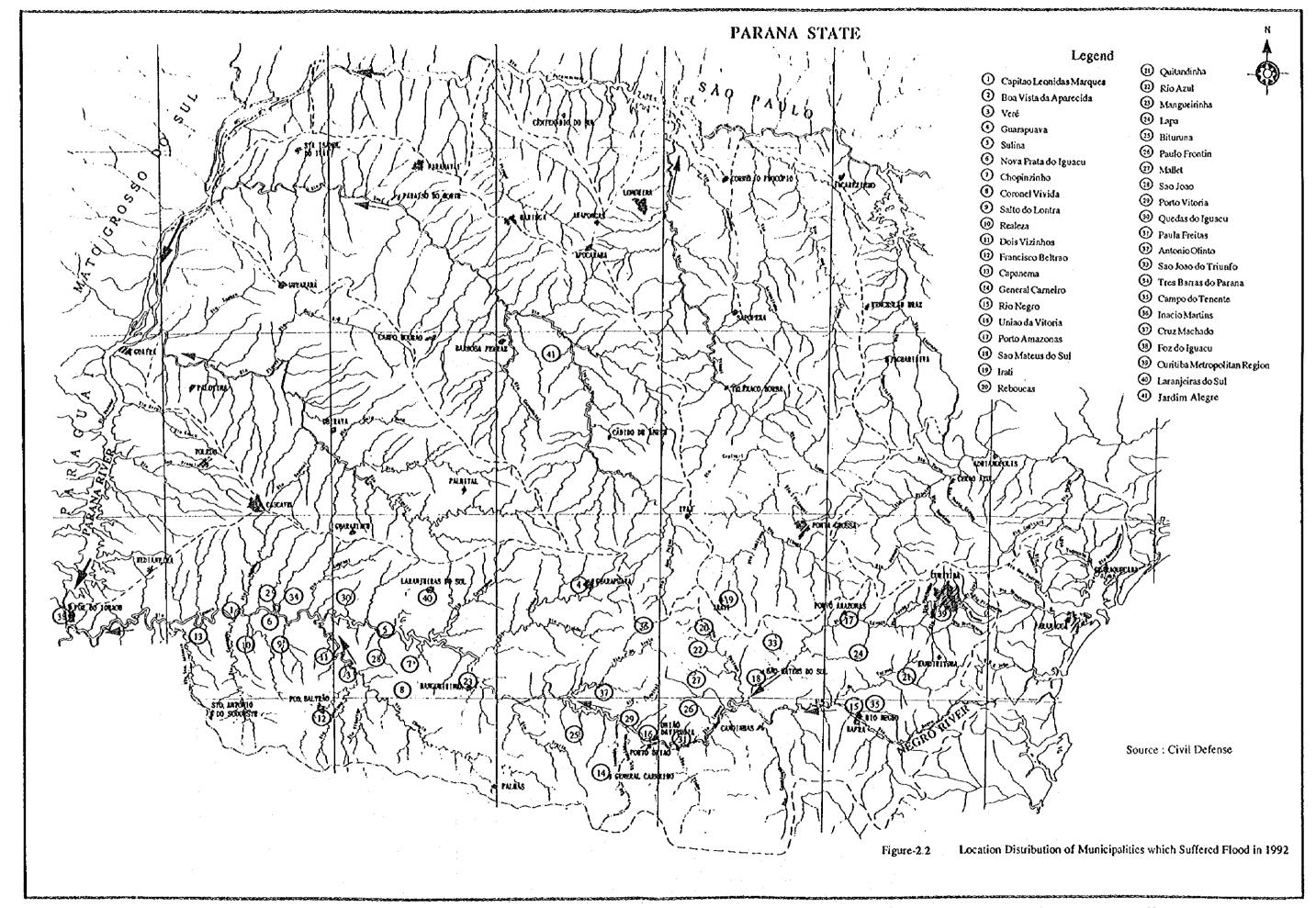
	Table-7	and the second s	and the second s	evet in 1983 with E	CATHANINE THE PROPERTY OF THE PARTY OF THE P	and the second s
Original	Section	Distance	Accumulated	Average Original	with excavation	with large exc.
Section	No.		Distance	River Bed	Water Level	Water Level
name		(m)	(m)	Elevation (m)	(m)	(m)
D1-77	CN011	9230,000	0.000	664.129	742,000	742.000
D2-77	CN021	9230.000		676.636	742.001	742.001
D3-77	CN031	12300.000	18460.000	681.506	742,001	742.001
D4-77	CN041	10000.000	30760.000	692.566	742.004	742.004
D5-77	CN051	11540.000	40760.000	701.539	742.003	742,003
PV8-78	CN061	2160.000	52300.000	715.789	742.015	742.015
PV7-78	CN071	3840.000	54460.000	722.196	741.951	741.951
PV6-78	CN081	400.000	58300.000	723.849	742.080	742,080
PV5A-78	CN091	1400.000	58700.000	727.227	742,085	742.085
PV5-78	CN101	1420.000	60100.000	726.772	741.931	741.931
PV4-78	CNIII	100.000	61520.000	727.191	742.063	742.063
M1-73	CN121	3040.000	61620.000	731.129	741.696	741.696
PV3-78	CN131	640.000		735.357	743.318	743,256
PV2-78	CN141	500.000	65300.000	737.397	743.713	743.600
PV1-78	CN151	70.000	65800.000	740.164	745.405	744.682
BAT-1A	CN152	70.000	65870.000	740.563	745.374	744.648
BAT-1B	CN153	60,000	65940.000	739.703	745.435	744.699
BAT-2	CN154	50.000		738.286	745.348	744.611
B3-84	CN161	260.000	66050.000	738.367	745.335	744.574
R5-94	CN171	1200.000	66310.000	737.910	745.383	744.594
R4-94	CN181	300.000		741.174	745.646	744,919
M2-73	CN191	690.000		741.312	745.766	745.020
UV8-78	CN201	1400.000		740.570	745.866	745.003
UV7-78	CN211	1400.000		742.092	746.177	745.383
UV6-78	CN221	1600.000		743.596	746.432	745.640
M3-73	CN231	1400.000		740.348	746.669	746.032
UV5A-78	CN241	300,000		743.269	746.746	746.057
R3-94	CN251	300.000		742.154	747.687	747.315
UV5-78	CN261	2170.000		742.690	747.522	747.123
R2-94	CN271	230.000			748.114	747.794
UV4-78	CN281	1200.000		743.125	748.137	747.818
M4-73	CN291	1400.000			748.255	747.953
UV3-78	CN301	1368.559		743.316	748.483	748.206
01.GPF	J1011	999.116			748.667	748.409
02.GPF	JT021	232.325		741.194	748.772	748.489
UV2-78	CN311	762.845		743.346	748.777	748.489
03.GPF	JT031	737.155			748.757	748.478
	CN321	174.458			748.845	748.539
R1-94		725.542			748.876	748.544
04,GPF	JT041	308.594				748.632
M5-73 05.GPF	CN331 JT051	999.700			748.937	748.625
05.GPF 06.GPF	JT051 JT061	999.700			748.990	748.661
M6-73	CN341	736.650			749.156	748.836
07.GPF	JT071	263.350			749.154	748.836
M7-73	CN351	735.024			749.180	748.853
	JT081	988.412			749.228	748.906
08.GPF 09.GPF	J1081 JT091	986.009			749.115	748.783
		890.555			749.301	748.989
10.GPF	JT101	95.809			749.371	749.059
M8-73	CN361	990.253			749.456	749.185
11.GPF	JTIII	955.756			749.604	749.302
12,GPF	JT121	758.182			749.709	749.401
13.GPF	JT131				749.750	749,414
UV-92	CN371	214.332			749.719	749.377
14.GPF	JT141	185,668			749.751	749,407
UV1-78	CN381	804,008			749.882	749.534
15.GPF)Ti51	295.992			749.957	749.604
FINFL-27.ENT	CF011	712.720			749.937	749.620
16.GPF	JT161	990.062				749.642
17.GPF	JT171	1005.718	1 (6/2/0/1/19/1			7.7(1) 6.7(1)

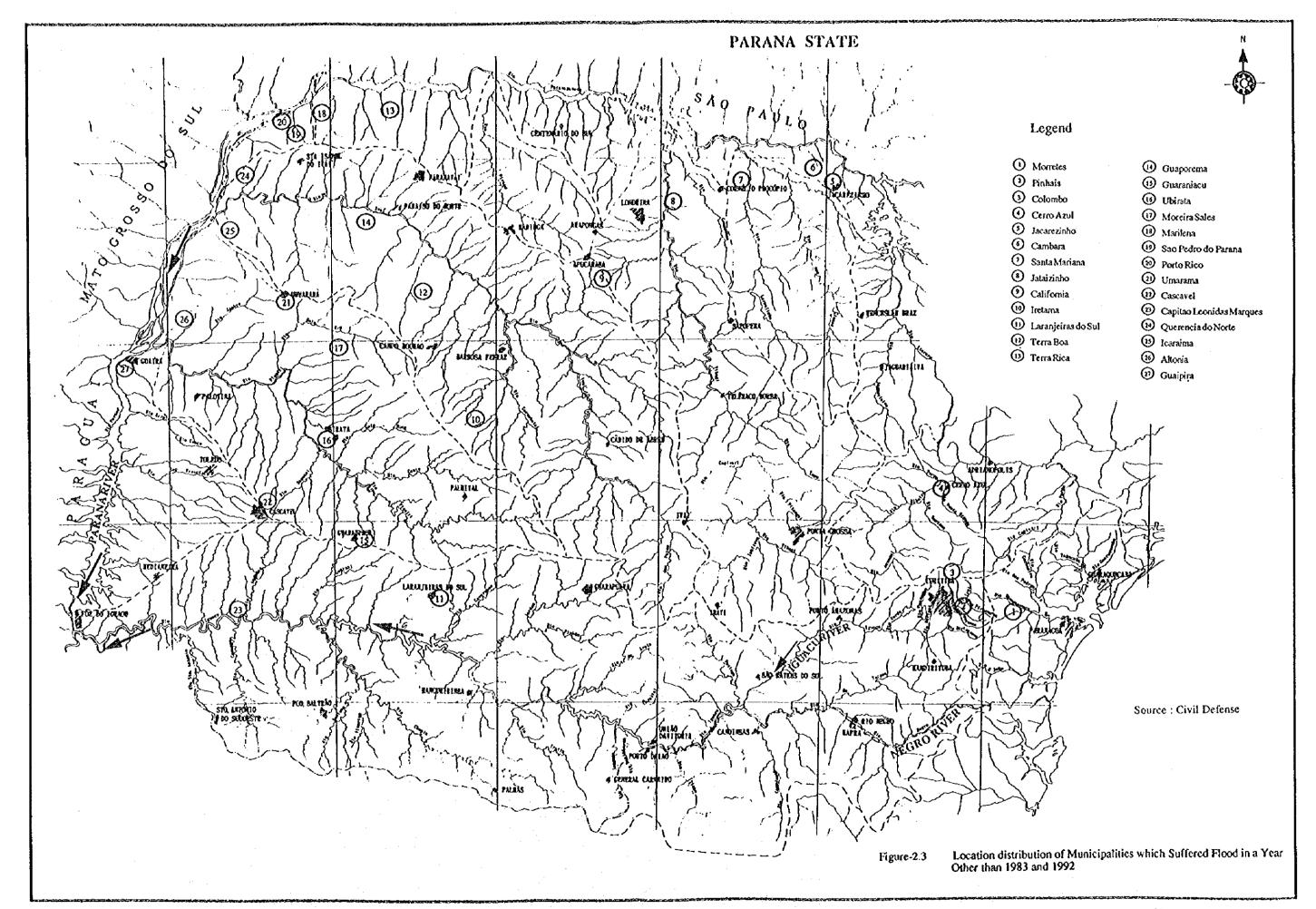
Table 7.9	Calculated Flood Water	Level in 198	83 with Excavations	(2/2)

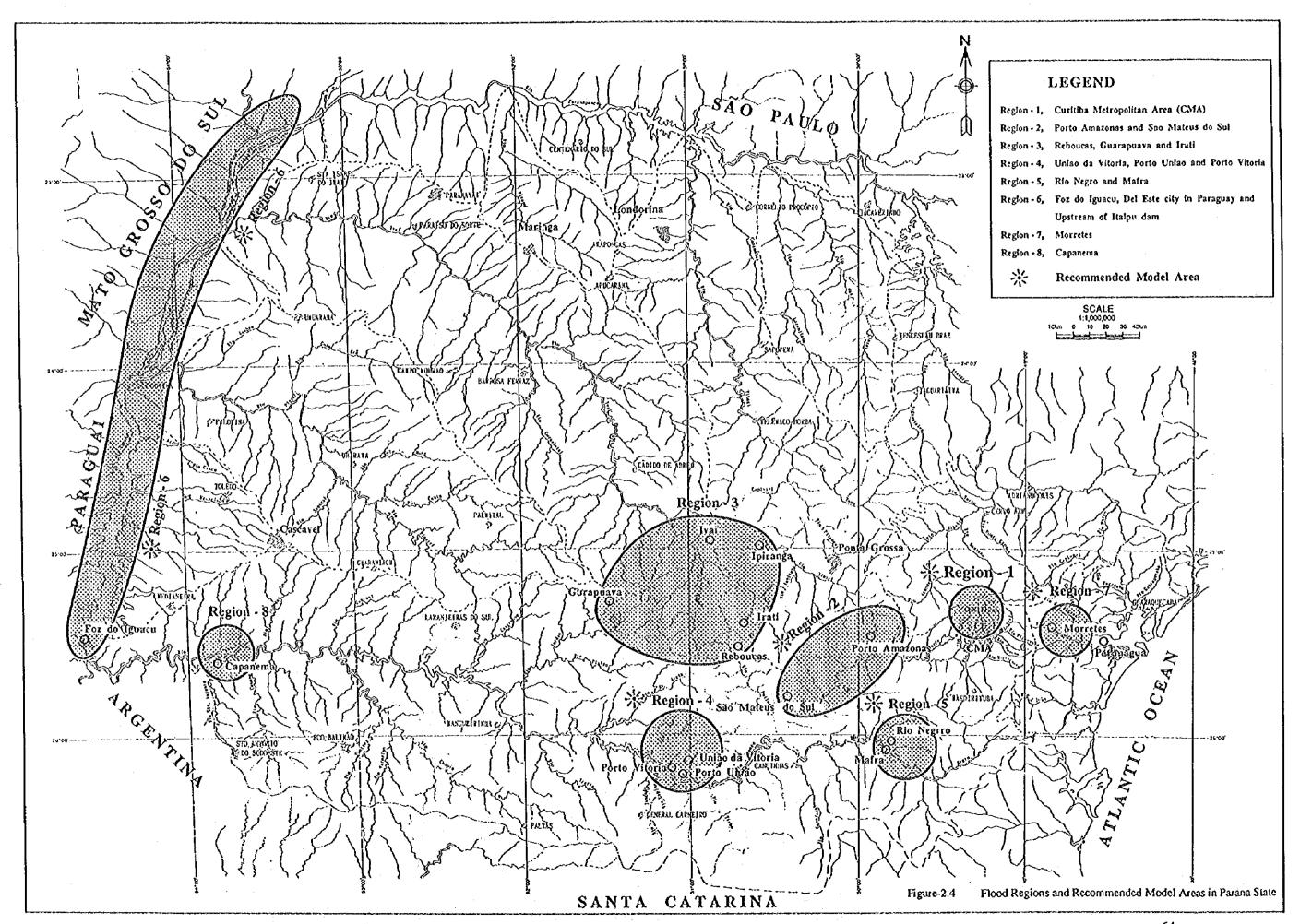
	i anic.			react in 1802 with 1		
18.GPF	JT18t	291.500				
FINFL-26.ENT	CF021	699.237	98200.000	744.311	750.223	749.827
19.GPF	JT191	998.171	98899.237	742.981	750.181	749.787
20.GPF	JT201	1002.592	99897.408	743.280	750.330	749.958
FINFL-25.ENT	CF031	3300.000	100900.000	744.780	750.414	750.057
FINFL-24.ENT	CF041	3500.000	104200,000	745.096	750.621	
FINFL-23,ENT	CF051	9950.000	107700.000			
FINFL-22.ENT	CF061	4650.000	117650.000			
FINFL-21,ENT	CF071	3900.000	122300.000			
FINFL-20.ENT	CF081	5000.000	126200.000	748.311	752.388	
FINFL-19.ENT	CF091	2000.000				
FINFL-18.ENT	CF101	2000.000	133200.000			
FINFL-17.ENT	CFIII	5750.000	135200.000	749.973		
FINFL-16.ENT	CF121	3250.000	140950.000			
FINFL-15.ENT	CF131	3500.000	144200.000	748.492		
FINFL-14,ENT	CF141	2250.000	147700.000			
FINFL-13.ENT	CF151	3250.000	149950.000	749.288		
FINFL-12.ENT	CF161	5050.000	153200.000	750.727		755.122
FINFL-11.ENT	CF171	3050.000	158250.000	750.188	755.573	
FINFL-10.ENT	CF181	2950.000	161300.000	751.385		
FINFL-09.ENT	CF191	4550.000	164250.000	751.790		
FINFL-08.ENT	CF201	4400.000	168800.000	751.703		
FINFL-07.ENT	CF211	4850.000	173200.000	752.721	756.872	
FINFL-06.ENT	CF221	1700.000	178050.000	751.088		
FINFL-05.ENT	CF231	3950.000	179750.000	753.563		
FINFL-04.ENT	CF241	2225.000	183700.000	753.363		
FINFL-03.ENT	CF251	4775.000	185925.000	750.880		
FINFL-02.ENT	CF261	7000.004	190700.000	751.505		758.187
FINFL-01.ENT	CF271	1805,800	197700.004	754.577		
S-02,GPF	JS021	1552.490	199505.804	752.546		
S-03.GPF	JS031	1762.610	201058.294	752.867		
S-04.GPF	JS041	2644.140	202820.904	753.358		
S-05.GPF	JS051	2191.590	205465.044	752.866		
S-06.GPF	JS061	2388.090	207656.634	752.581	759.842	
S-07.GPF	JS071	3521.930	210044.724	752.193		
S-08.GPF	JS081	1817.230	213566.654	753.634		
S-09.GPF	JS091	2028.390	215383.884	755.011	760.241	760.234
S-10.GPF	JS101	2469.990	217412.274	755.567		
S-11.GPF	JS111	1076.570	219882.264	755.280		
S-12.GPF	JS121	2390.910	220958.834	756.625		
S-13.GPF	JS131	2077.590	223349,744	756.995		
S-14,GPF	JS141	2319.950	225427.334	757.179		
S-15.GPF	JS151	1916.490	227747.284	756.478		
S-16.GPF	JS161	2866.920	229663.774	757.104		
S-17.GPP	JS171	1134.620	232530.694	756.811		
S-18.GPF	JS181	1295.530	233665.314	757,203		
S-19.GPF	JS191	737.220	234960.844	758.317		
S-20.GPF	JS201	998.120	235698.064	757.558		
S-21.GPF	JS211	391.560	236696.184	757.216		
S-22,GPF	JS221	1191,210	237087.744	757.659		
S-23.GPF	JS231	487.750	238278.954	758.231	764.195	
S-24.GPF	JS241	557.770	238766.704	757.782		
S-25.GPF	JS251	436.940	239324,474	758.438		
S-26.GPF	1\$261	516.210	239761.414	757.891	764.441	
S-27.GPF	3S271	512.410	240277.624	757.985		764.447
S-28.GPF	JS281	919.310	240790.034	759.013		
S-29.GPF	JS291	875.150	241709.344	757.964		
S-30.GPP	JS301	0.000	242584.494	758.218		
L		0.000		,55,216	,07,702	107,702

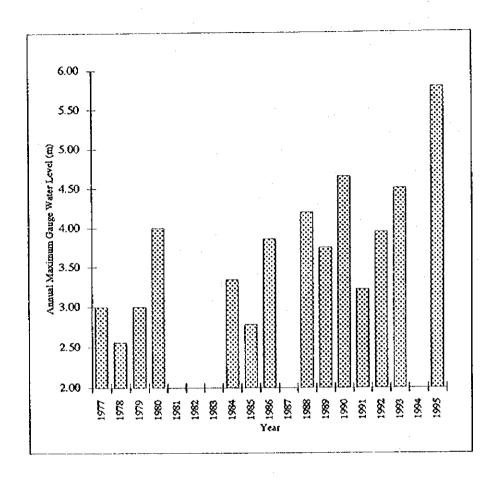
FIGURES









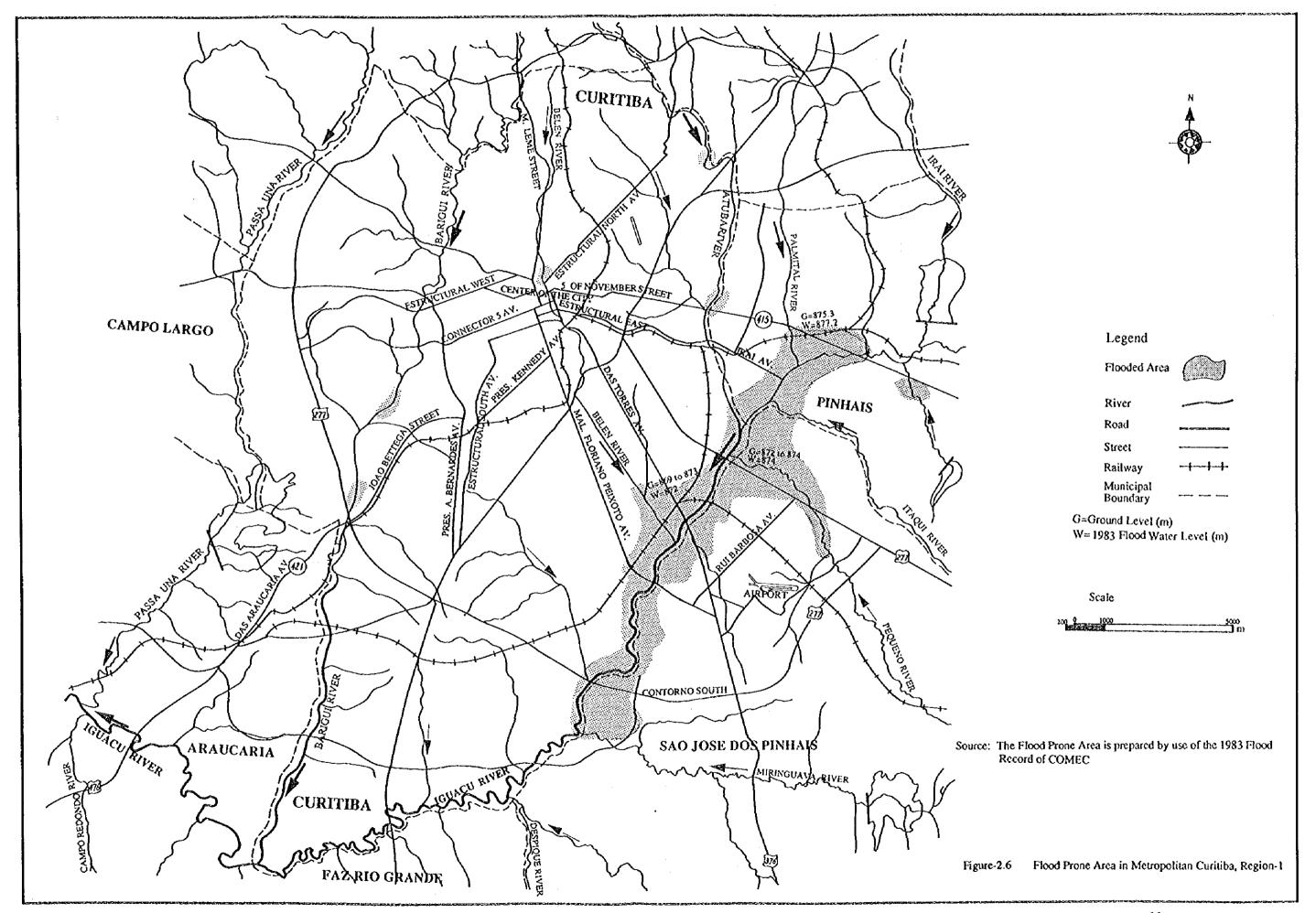


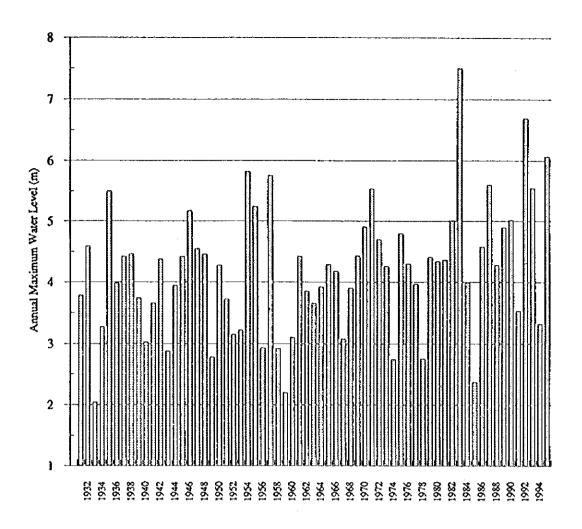
Year	Annual Maximum	Year	Annual Maximum
	Gauge Water Level		Gauge Water Level
1977	3.00	1987	n.a
1978	2.56	1988	4.20
1979	3.00	1989	3.75
1980	4.00	1990	4.65
1981	n.a	1991	3.22
1982	n.a	1992	3.95
1983	n.a	1993	4.50
1984	3.34	1994	n.a
1985	2.78	1995	5.78
1986	3.86		•
Source: DN	/AFE		n a: no available

Gauge Station of Iguacu river at Bridge on Road BR-277

1

Figure 2.5 Annual Maximum Gauge Water Level at Iguacu River in Curitiba

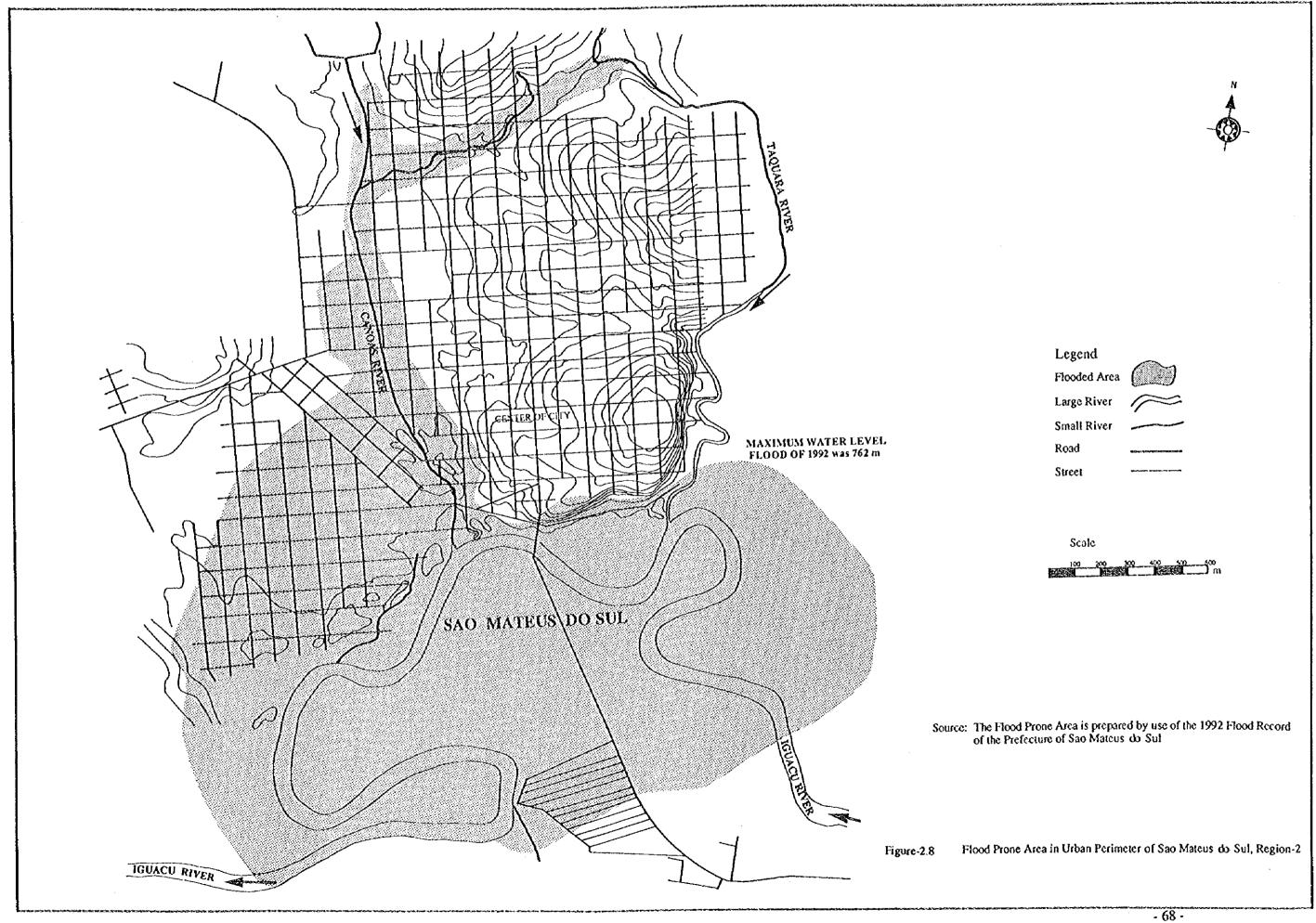


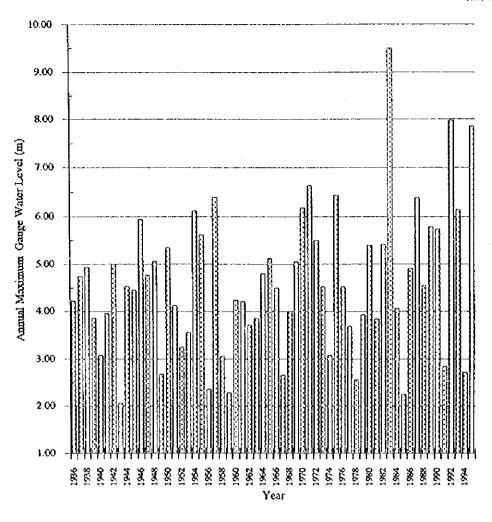


	Annual Maximum		Annual Maximum		Annual Maximum	
Year	Gauge Water Level	Year	Gauge Water Level	Year	Gauge Water Level	_Elevation (m)
1931	3.78	1953	3.22	1975	4.80	
1932	4.59	1954	5.82	1976	4.30	
1933	2.04	1955	5.24	1977	3.97	
1934	3.27	1956	2.93	1978	2.75	
1935	5.50	1957	5.75	1979	4.41	
1936	3.98	1958	2.92	1980 *	4.34	
1937	4.42	1959	2.20	1981	4.36	
1938	4.46	1960	3.10	1982	5.01	
1939	3.74	1961	4.42	1983	7.50	762.9
1940	3.03	1962	3.85	1984 *	3.99	
1941	3.66	1963	3.66	1985	2.37	
1942	4.38	1964	3.92	1986	4.58	
1943	2.88	1965	4.29	1987	5.60	
1944	3.94	1966	4.18	1988	4.28	
1945	4.42	1967	3.08	1989	4.90	
1946	5.17	1968	3.90	1990	5.02	
1947	4.55	1969	4.43	1991	3.53	
1948	4.46	1970	4.91	1992 *	6.69	762.09
1949	2.78	1971	5.54	1993	5.55	
1950	4.28	1972	4.70	1994 *	3.32	
1951	3.72	1973	4.26	1995	6.07	
1952	3.15	1974	2.74			
	Source: DNAEE					

The values with * are estimated by COPEL.

Figure 2.7 Annual Maximum Gauge Water Level at Sao Mateus do Sul





1

	Annual Maximum		Annual Maximum		Annual Maximum
Year	Gauge Water Level	Year	Gauge Water Level	Year	Gauge Water Level
1936	4.22	1956	2.36	1976	4.52
1937	4.74	1957	6.40	1977	3.69
1938	4.91	1958	3.06	1978	2.56
1939	3.86	1959	2.28	1979	3.93
1940	3.08	1960	4.24	1980	5.40
1941	3.96	1961	4.21	1981	3.84
1912	5.00	1962	3.72	1982	5.42
1943	2.06	1963	3.86	1983	9.50
1944	4.53	1964	4.80	1984	4.06
1945	4.45	1965	5.12	1985	2.25
1946	5.94	1966	4.50	1986	4.91
1947	4.77	1967	2.66	1987	6.40
1948	5.06	1968	4.00	1988	4.55
1949	2.68	1969	5.05	1989	5.79
1950	5.35	1970	6.18	1990	5.74
1951	4.12	1971	6.64	1991	2.84
1952	3.25	1972	5.50	1992	7.98
1953	3.56	1973	4.52	1993	6.14
1951	6.12	1974	3.08	1994	2.72
1955	5.62	1975	6.44	1995	7.85
	Source: DNAEE		unit meter		

Figure 2.9 Annual Maximum Gauge Water Level at Porto Amazonas

