

SECTOR VI

URBAN DRAINAGE PLAN

DATA BOOK  
URBAN DRAINAGE PLAN

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## I. HYDRAULIC FEATURES OF EXISTING MAJOR CHANNELS

1. SIRINGIN RIVER (TABLE I-1, FIG. I-1)
2. TENGGANG RIVER (TABLE I-2, FIG. I-2)
3. SEMARANG RIVER (TABLE I-3, FIG. I-3)
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7. TM-1, TM-2 AND TM-3 CHANNELS (TABLE I-7, FIGS. I-9 TO I-11)

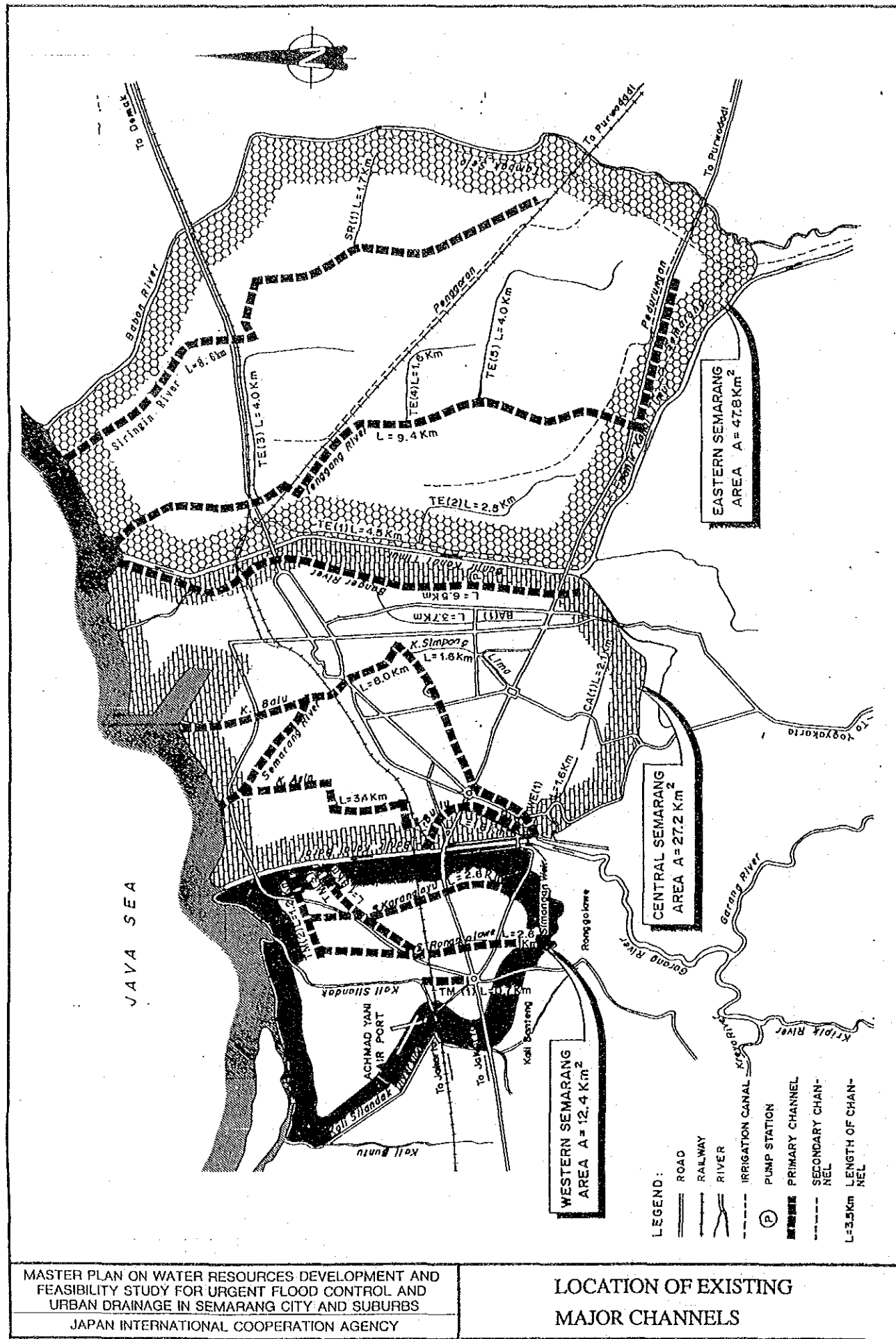
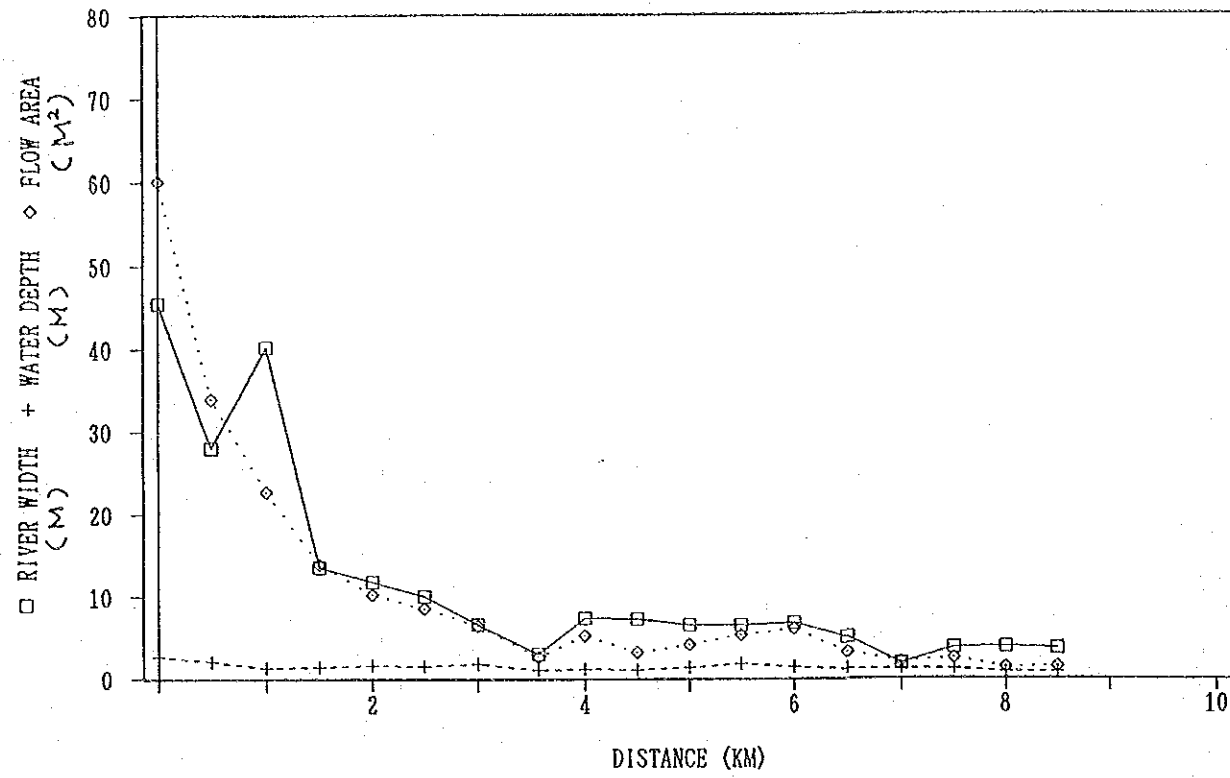


Table I-1 HYDRAULIC FEATURES OF EXISTING MAJOR CHANNELS  
(SIRINGIN RIVER)

RIVER	Station (km)	Channel Width B (m)	Water Depth H (m)	Flow Area A (m <sup>2</sup> )	Wetted Perimeter P (m)	Hydraulic Radius R (m)	Roughness Coefficient n	Hydraulic Gradient i	Velocity V (m/s)	Discharge Q (m <sup>3</sup> /s)
	0.00	45.5	2.7	60.13	39.2	1.53	0.031	0.000350	0.81	48.7
	0.50	28.0	2.0	33.90	26.6	1.27	0.031	0.000350	0.72	24.4
	1.00	40.2	1.3	22.65	39.8	0.57	0.031	0.000350	0.42	9.5
	1.50	13.5	1.4	13.92	14.0	0.99	0.031	0.000350	0.61	8.5
	2.00	11.8	1.6	10.19	10.5	0.97	0.031	0.000350	0.60	6.1
	2.50	10.0	1.5	8.48	9.6	0.88	0.031	0.000350	0.56	4.7
	3.00	6.6	1.8	6.36	8.7	0.73	0.031	0.000350	0.50	3.2
	3.567	3.0	1.0	2.58	4.5	0.57	0.024	0.000513	0.65	1.7
	4.00	7.3	1.1	5.15	7.3	0.71	0.031	0.000513	0.58	3.0
	4.50	7.2	1.0	3.14	6.5	0.48	0.031	0.000513	0.45	1.4
	5.00	6.5	1.3	4.09	6.3	0.65	0.031	0.000513	0.55	2.2
	5.50	6.5	1.8	5.31	7.8	0.68	0.031	0.000513	0.56	3.0
	6.00	6.8	1.4	6.05	7.3	0.83	0.031	0.001110	0.95	5.7
	6.50	5.0	1.1	3.17	5.3	0.60	0.024	0.001110	0.99	3.1
	7.00	1.8	1.1	1.93	3.9	0.49	0.024	0.001110	0.86	1.7
	7.50	3.7	1.1	2.42	4.2	0.58	0.031	0.001110	0.75	1.8
	8.00	3.8	0.7	1.30	3.1	0.42	0.031	0.001110	0.60	0.8
	8.50	3.6	0.7	1.40	3.7	0.38	0.031	0.001110	0.56	0.8
	AVERAGE	10.5	1.2	10.11	10.4	0.67	0.027	0.000587	0.62	6.9

(SRINGIN RIVER)



(SRINGIN RIVER)

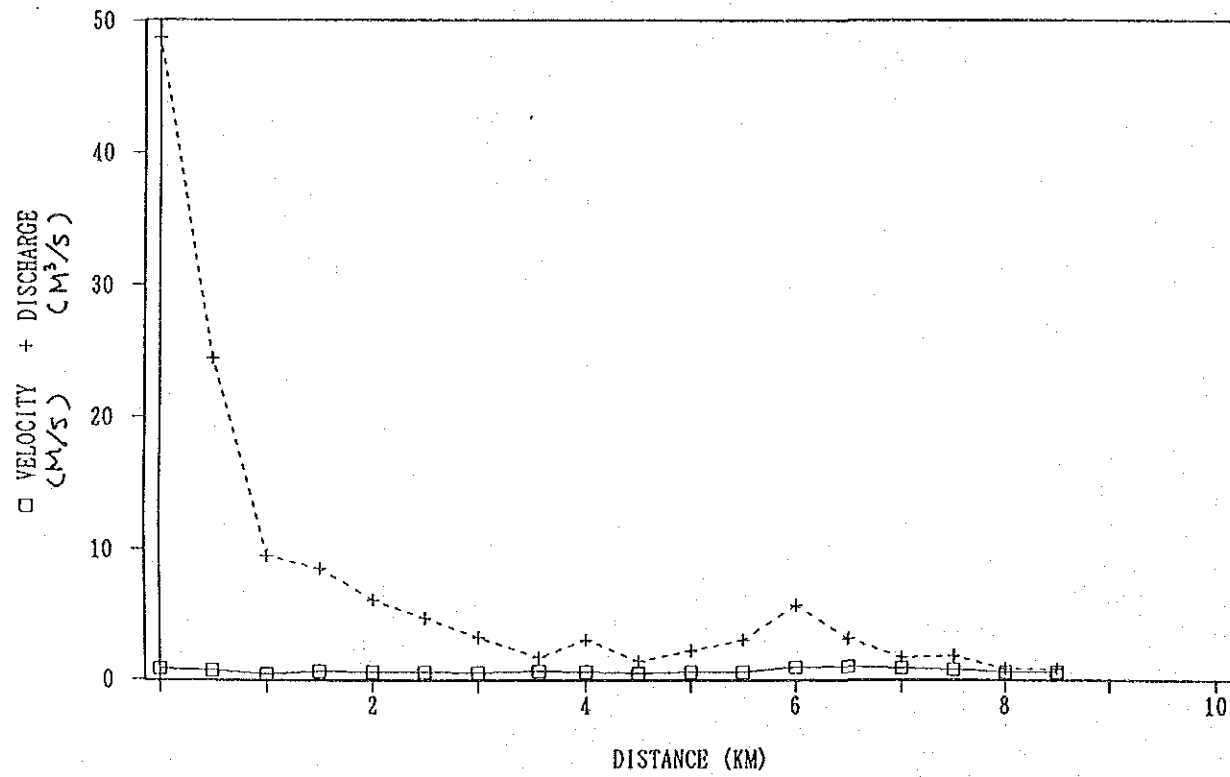


FIG. I-1 HYDRAULIC FEATURES OF EXISTING MAJOR CHANNELS  
(SRINGIN RIVER)

Table I-2 HYDRAULIC FEATURES OF EXISTING MAJOR CHANNELS  
(TENGGANG RIVER)

RIVER	Station (km)	Channel Width B(m)	Water Depth H(m)	Flow Area A(m²)	Wetted Perimeter P(m)	Hydraulic Radius R(m)	Roughness Coefficient n	Hydraulic Gradient i	Velocity V(m/s)	Discharge Q(m³/s)
	0.00	39.1	1.8	44.61	39.7	1.12	0.031	0.000140	0.41	18.3
	0.50	49.5	1.9	42.54	41.9	1.02	0.031	0.000140	0.39	16.6
	1.00	20.1	1.8	23.22	20.4	1.14	0.031	0.000140	0.42	9.8
	1.50	39.2	1.7	23.47	39.6	0.59	0.031	0.000140	0.27	6.3
	2.00	17.7	1.9	17.63	17.7	1.00	0.031	0.000140	0.38	6.7
	2.50	12.9	2.4	18.65	13.6	1.37	0.031	0.000500	0.89	16.6
	3.00	9.0	1.9	10.19	9.9	1.03	0.031	0.000500	0.74	7.5
	3.50	10.6	1.6	7.61	10.4	0.73	0.031	0.000500	0.58	4.4
	4.00	8.4	1.5	6.28	8.5	0.74	0.031	0.000500	0.59	3.7
	4.50	9.0	2.2	10.62	10.2	1.04	0.031	0.001330	1.21	12.9
	4.99	5.9	1.9	11.03	9.3	1.19	0.024	0.001330	1.71	18.9
	5.50	12.3	1.7	11.96	10.8	1.11	0.024	0.001330	1.63	19.5
	6.00	11.5	1.4	11.17	11.0	1.02	0.024	0.001570	1.67	18.7
	6.50	3.5	1.1	1.41	4.2	0.34	0.031	0.001570	0.62	0.9
	7.00	5.8	1.0	2.88	5.8	0.50	0.031	0.001570	0.81	2.3
	7.50	5.7	1.2	3.14	5.8	0.54	0.031	0.001570	0.85	2.7
	8.00	6.8	1.2	3.91	6.2	0.63	0.031	0.001570	0.94	3.7
	8.50	6.2	1.0	2.80	5.6	0.50	0.031	0.001570	0.81	2.3
	9.00	6.1	1.2	2.86	5.9	0.48	0.031	0.001570	0.78	2.2
	9.50	7.2	0.8	3.35	7.0	0.48	0.024	0.001570	1.01	3.4
	AVERAGE	14.3	1.5	12.97	14.2	0.83	0.030	0.000963	0.84	8.9

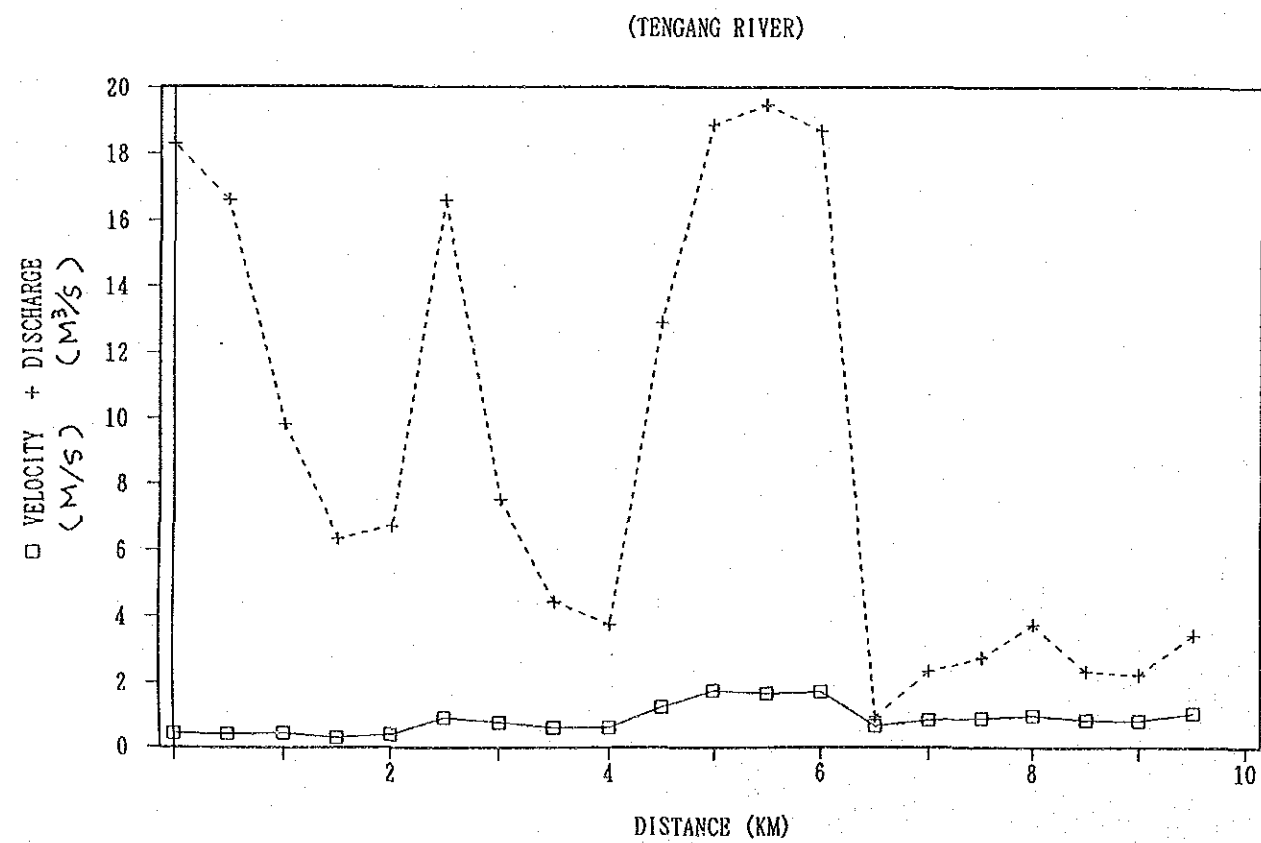
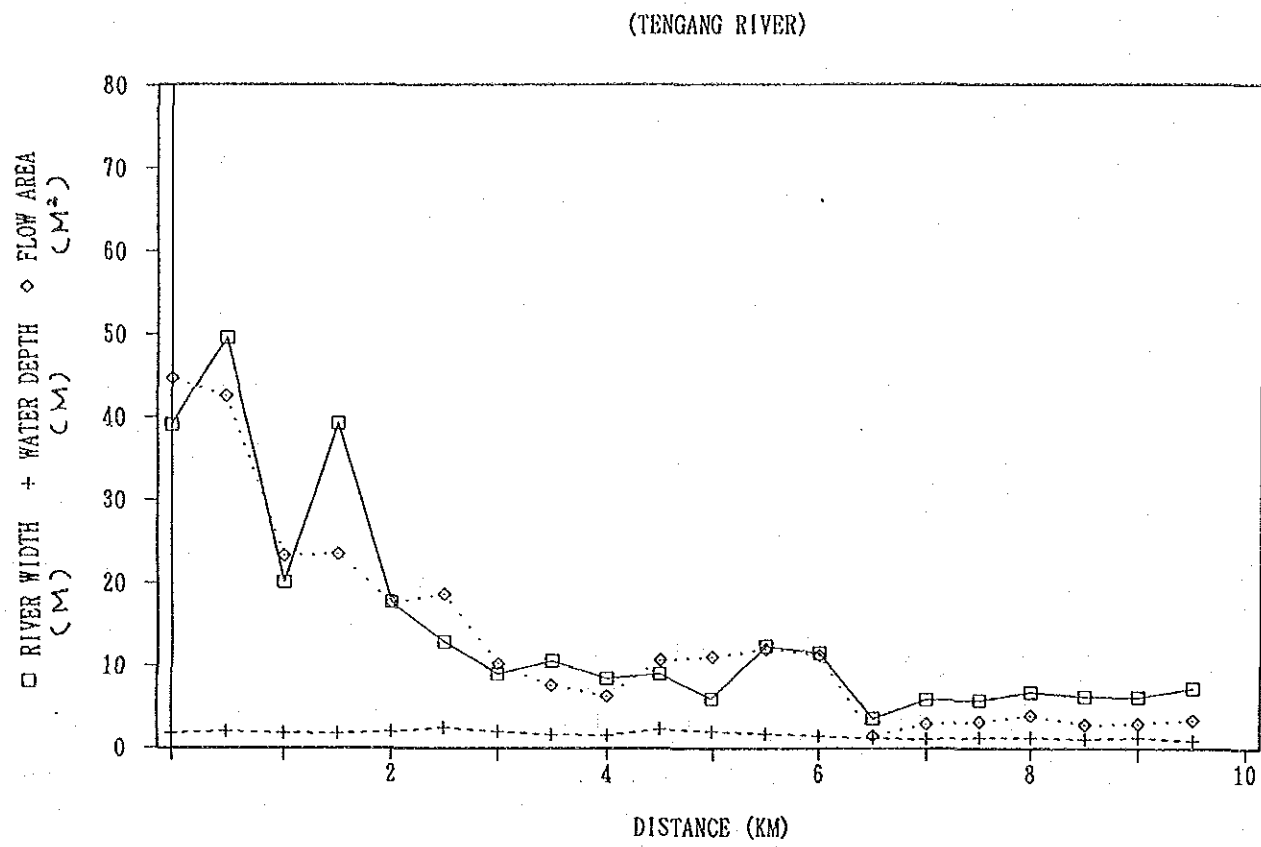


FIG. I-2 HYDRAULIC FEATURES OF EXISTING MAJOR CHANNELS (TENGGANG RIVER)

Table I-3 HYDRAULIC FEATURES OF EXISTING MAJOR CHANNELS (SEMARANG RIVER)

RIVER	Station (km)	Channel Width B (m)	Water Depth H (m)	Flow Area A (m <sup>2</sup> )	Wetted Perimeter P (m)	Hydraulic Radius R (m)	Roughness Coefficient n	Hydraulic Gradient i	Velocity V (m/s)	Discharge Q (m <sup>3</sup> /s)
	0.00	52.0	1.9	44.86	46.3	0.97	0.031	0.000209	0.46	20.6
	0.31	66.0	2.1	63.10	56.7	1.11	0.031	0.000209	0.50	31.6
	0.50	48.0	1.6	47.79	40.7	1.17	0.031	0.000209	0.52	24.9
	1.00	37.5	2.1	44.57	33.9	1.31	0.031	0.000209	0.56	25.0
	1.26	20.0	1.8	28.48	21.7	1.31	0.024	0.000209	0.72	20.5
	1.50	34.5	2.2	47.14	30.6	1.54	0.031	0.000209	0.62	23.2
	1.74	19.0	1.9	27.65	21.2	1.30	0.024	0.000209	0.72	19.9
	2.00	34.0	1.9	40.53	29.9	1.36	0.031	0.000209	0.57	23.1
	2.36	19.0	1.6	12.03	11.8	1.02	0.024	0.000209	0.61	7.3
	2.50	25.5	2.1	39.73	27.9	1.42	0.024	0.000209	0.76	30.2
	2.85	24.0	1.5	29.20	36.1	0.81	0.024	0.000643	0.92	25.9
	3.11	22.6	1.5	21.51	24.7	0.87	0.024	0.000643	0.96	20.6
	3.45	19.7	0.8	10.66	20.3	0.53	0.024	0.000643	0.69	7.4
	3.93	18.0	1.6	22.90	20.5	1.12	0.024	0.000643	1.14	26.1
	4.34	18.1	1.6	25.12	20.6	1.22	0.024	0.000643	1.21	30.4
	4.47	10.8	1.8	16.99	13.5	1.26	0.024	0.000761	1.34	22.8
	5.00	12.7	1.7	19.86	15.4	1.29	0.024	0.000761	1.36	27.0
	5.51	9.0	1.7	14.55	12.0	1.21	0.024	0.000761	1.31	19.1
	5.97	4.3	1.4	4.33	9.4	0.51	0.020	0.000761	0.88	4.3
	6.39	7.9	0.7	4.91	8.7	0.56	0.024	0.000761	0.78	3.8
	6.87	2.2	1.6	3.37	5.2	0.65	0.024	0.000761	0.86	2.9
	6.87	5.7	1.6	7.87	8.2	0.96	0.024	0.000761	1.12	8.8
	AVERAGE	23.2	1.7	26.26	23.4	1.12	0.026	0.000433	0.85	19.7

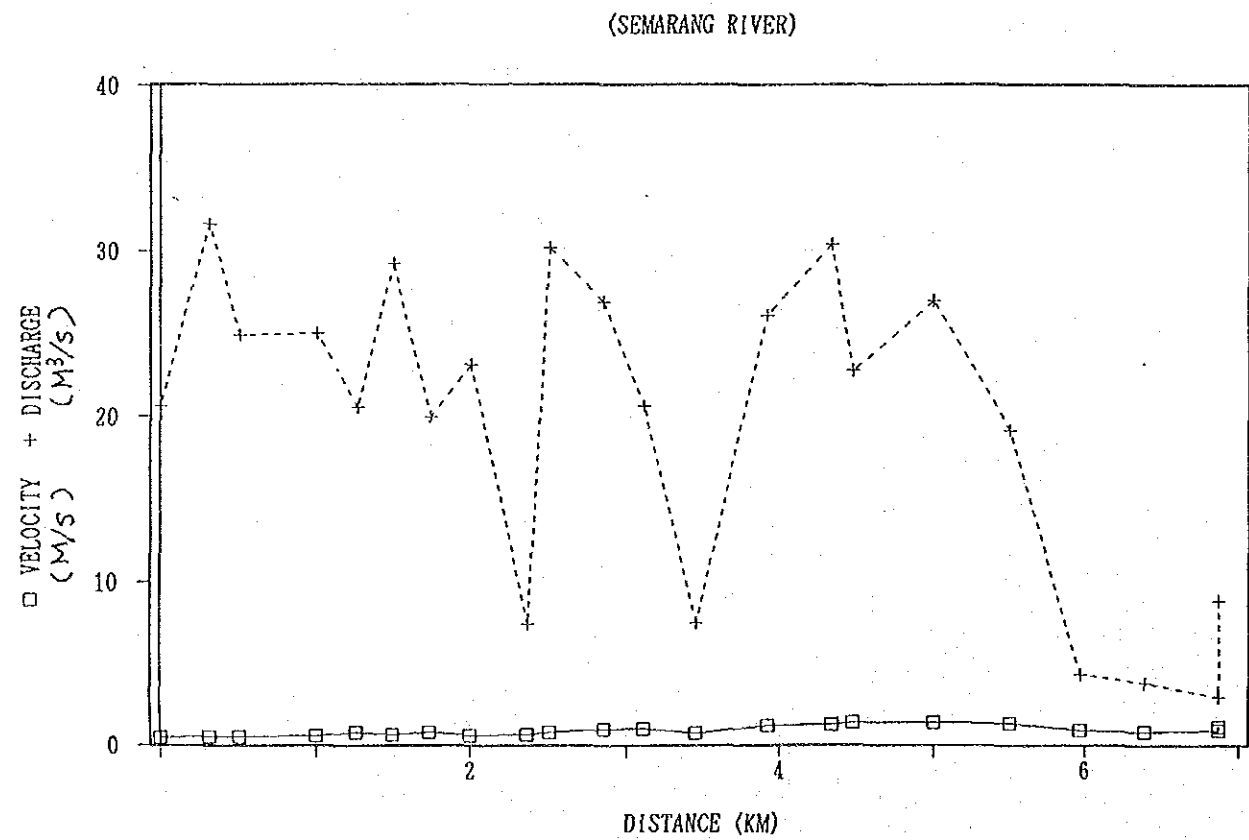
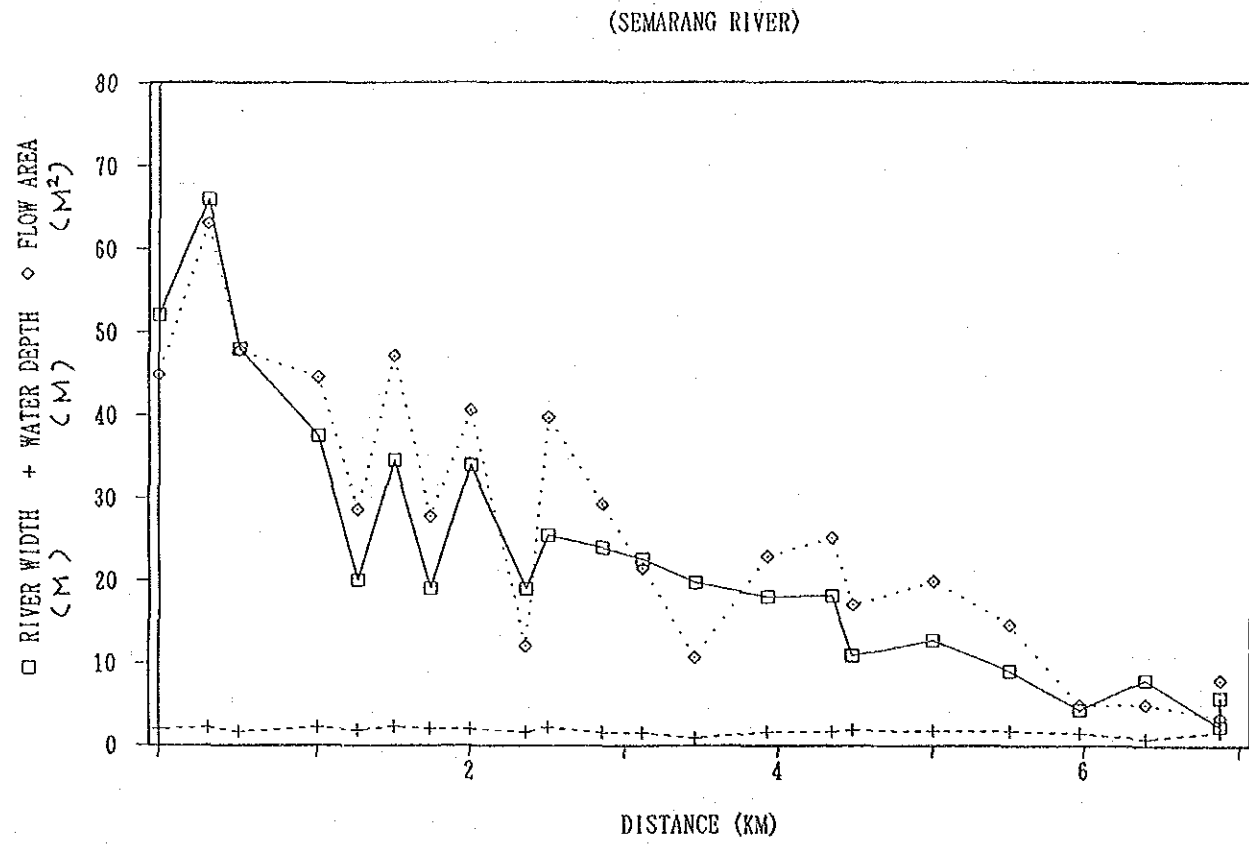


FIG. I-3 HYDRAULIC FEATURES OF EXISTING MAJOR CHANNELS (SEMARANG RIVER)

Table I-4 HYDRAULIC FEATURES OF EXISTING MAJOR CHANNELS (BANGER RIVER)

RIVER	Station (km)	Channel Width B(m)	Water Depth H(m)	Flow Area A(m²)	Wetted Perimeter P(m)	Hydraulic Radius R(m)	Roughness Coefficient n	Hydraulic Gradient i	Velocity V(m/s)	Discharge Q(m³/s)
	0.00	53.5	1.1	40.71	53.0	0.77	0.031	0.000174	0.36	14.7
	0.50	43.5	1.1	26.79	40.3	0.66	0.031	0.000174	0.32	8.5
	1.00	43.5	1.5	41.41	40.9	1.01	0.031	0.000174	0.43	17.8
	1.49	43.8	1.6	35.12	33.4	1.05	0.031	0.000174	0.44	15.5
	2.00	11.7	1.0	6.62	11.7	0.57	0.031	0.000322	0.40	2.6
	2.50	8.8	1.2	7.58	10.3	0.74	0.024	0.000322	0.61	4.5
	2.99	7.2	1.2	7.21	8.5	0.85	0.024	0.000322	0.67	4.8
	3.49	7.7	1.0	5.12	8.7	0.59	0.024	0.000510	0.66	3.4
	3.90	7.7	1.0	5.75	8.4	0.68	0.024	0.000510	0.73	4.2
	4.46	7.1	1.0	6.42	8.7	0.74	0.024	0.000510	0.77	4.9
	4.99	3.5	0.8	2.78	5.1	0.55	0.024	0.000510	0.63	1.8
	5.60	2.7	1.0	2.32	4.3	0.54	0.024	0.000634	0.70	1.5
	6.00	7.9	1.5	9.43	9.1	1.04	0.024	0.000634	1.08	10.2
	6.57	5.2	1.5	4.10	5.5	0.75	0.024	0.000634	0.87	3.6
	AVERAGE	14.1	1.0	11.84	14.6	0.81	0.022	0.000330	0.54	6.1

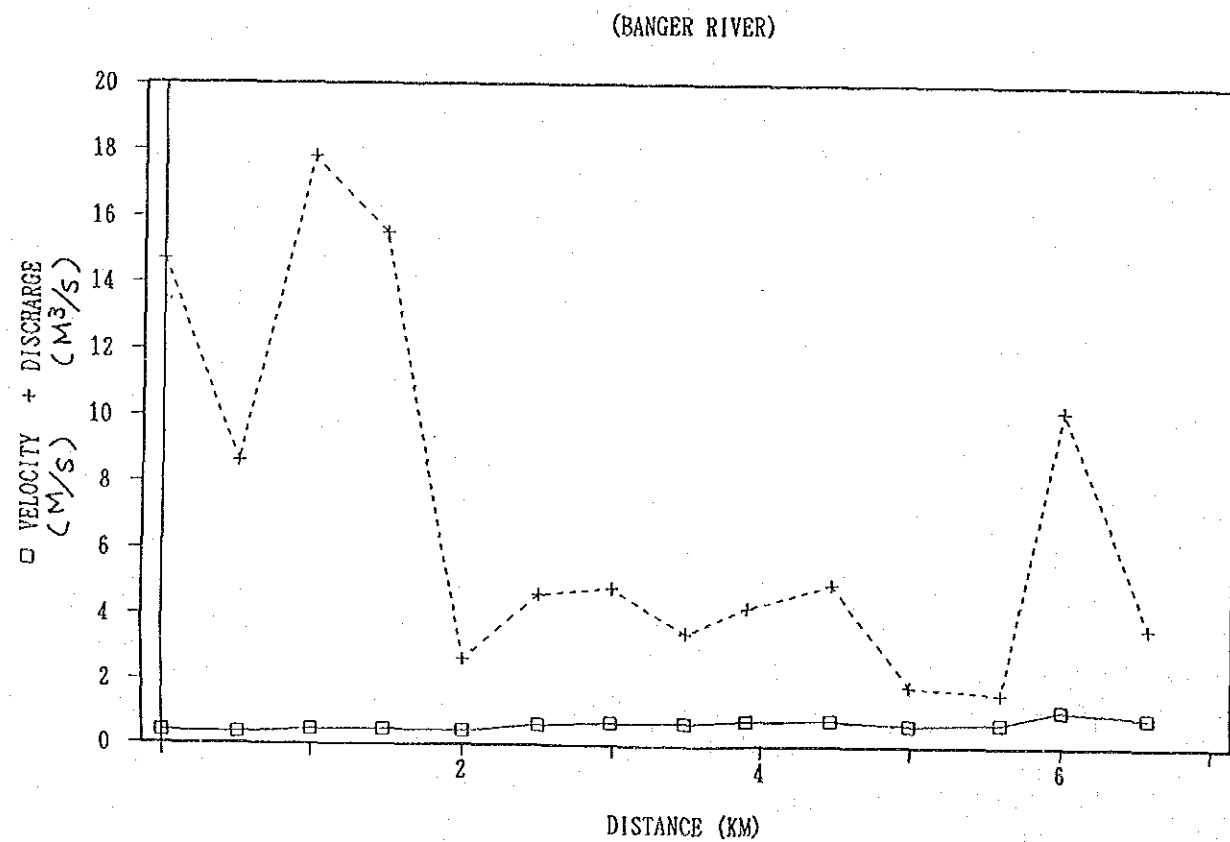
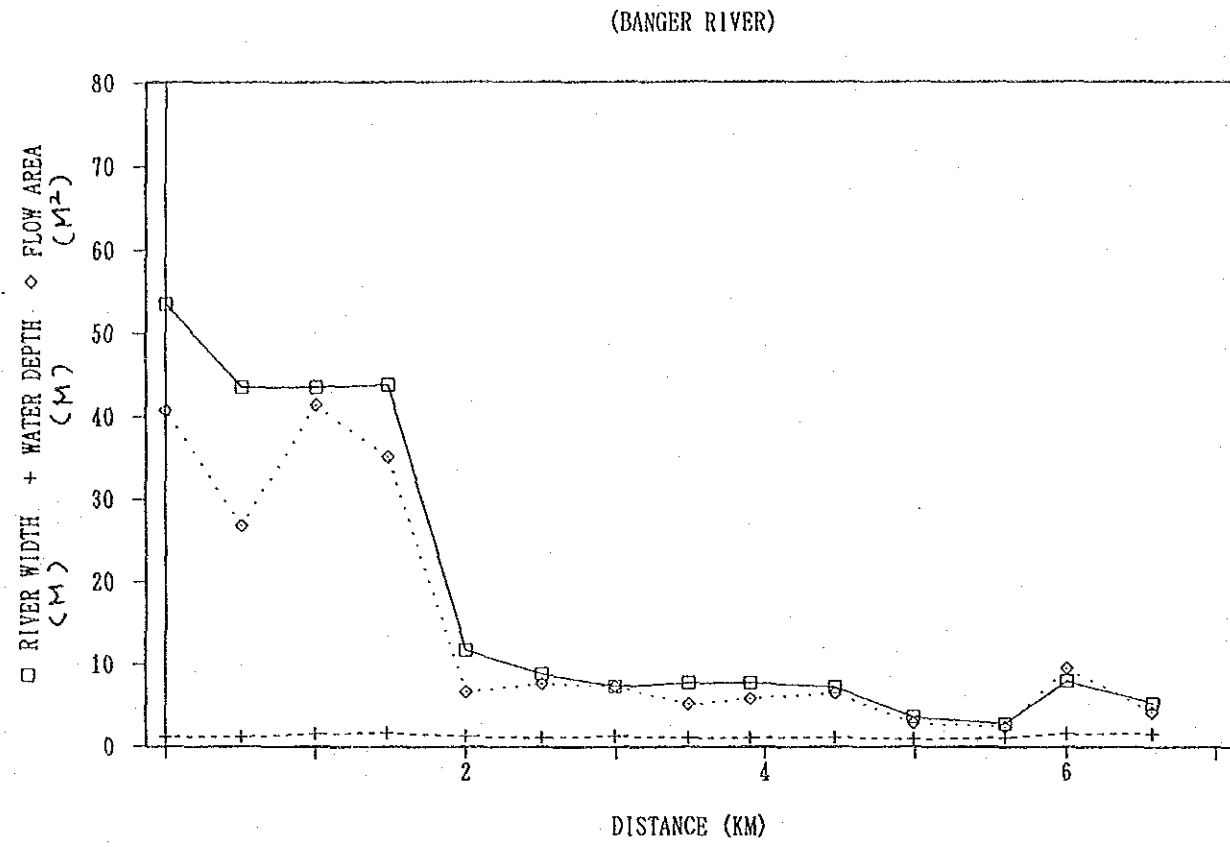


FIG. I-4. HYDRAULIC FEATURES OF EXISTING MAJOR CHANNELS (BANGER RIVER)

Table I-5 HYDRAULIC FEATURES OF EXISTING MAJOR CHANNELS (ASIN RIVER AND BULU RIVER)

RIVER	Station (km)	Channel Width B (m)	Water Depth H (m)	Flow Area A (m²)	Wetted Perimeter P (m)	Hydraulic Radius R (m)	Roughness Coefficient n	Hydraulic Gradient i	Velocity V (m/s)	Discharge Q (m³/s)
	0.00	35.0	1.6	36.79	32.1	1.15	0.024	0.000550	1.07	39.4
	0.50	14.1	2.1	19.07	15.6	1.22	0.024	0.000550	1.12	21.4
	1.00	21.5	1.9	27.61	22.0	1.26	0.024	0.000550	1.14	31.5
	1.30	3.9	0.9	2.76	4.9	0.56	0.024	0.001250	1.00	2.8
	1.50	4.5	1.0	2.57	5.6	0.46	0.031	0.001250	0.68	1.7
	2.00	1.4	0.7	0.84	2.5	0.34	0.024	0.001250	0.72	0.6
	2.50	1.3	0.9	1.05	3.0	0.35	0.240	0.001250	0.07	0.1
	3.00	2.3	0.8	1.00	3.2	0.31	0.240	0.001250	0.07	0.1
	3.09	2.9	1.3	2.73	4.7	0.58	0.240	0.001250	0.10	0.3
	3.30	2.2	1.0	1.98	3.8	0.52	0.240	0.001250	0.10	0.2
	3.50	2.3	0.7	1.27	3.4	0.37	0.240	0.001250	0.08	0.1
	AVERAGE	8.3	1.2	8.88	9.2	0.65	0.123	0.001059	0.56	5.0
	0.00	3.5	1.9	4.52	6.6	0.68	0.024	0.003800	1.99	9.0
	0.00	2.0	1.3	2.39	6.8	0.35	0.020	0.003800	1.53	3.7
	0.34	4.7	1.8	6.58	7.0	0.94	0.024	0.003800	2.46	16.2
	0.34	2.8	0.7	1.59	7.4	0.21	0.020	0.003800	1.09	1.7
	0.50	2.8	1.1	2.49	4.2	0.59	0.024	0.003800	1.81	4.5
	0.54	2.9	0.9	2.23	4.4	0.51	0.024	0.003800	1.64	3.7
	0.81	2.0	0.9	1.65	3.6	0.46	0.024	0.003800	1.53	2.5
	1.00	2.5	0.6	1.10	3.3	0.33	0.024	0.000830	0.57	0.6
	1.50	1.8	0.6	1.00	2.8	0.36	0.024	0.000830	0.61	0.6
	1.73	4.1	2.1	7.64	11.4	0.67	0.024	0.000830	0.92	7.0
	AVERAGE	3.4	1.2	3.64	6.1	0.52	0.032	0.002741	1.34	4.9



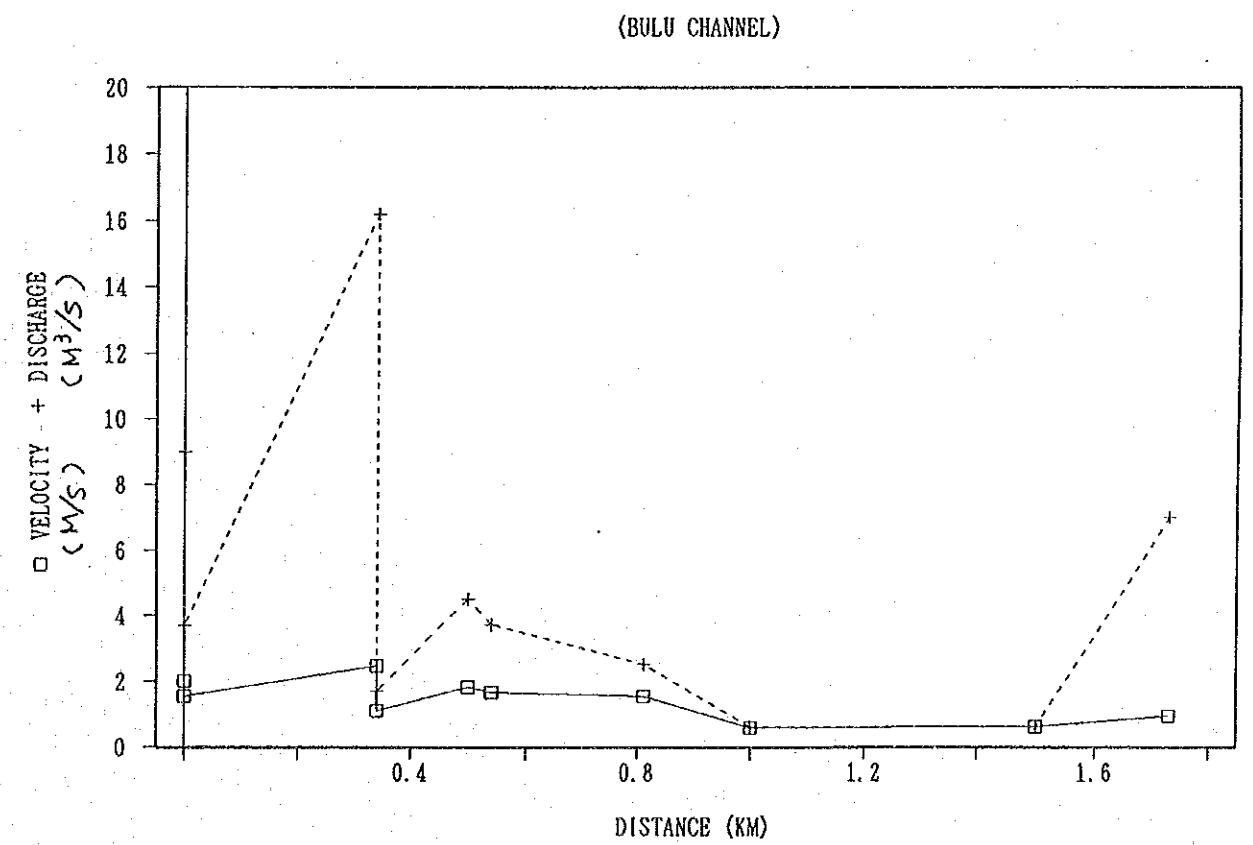
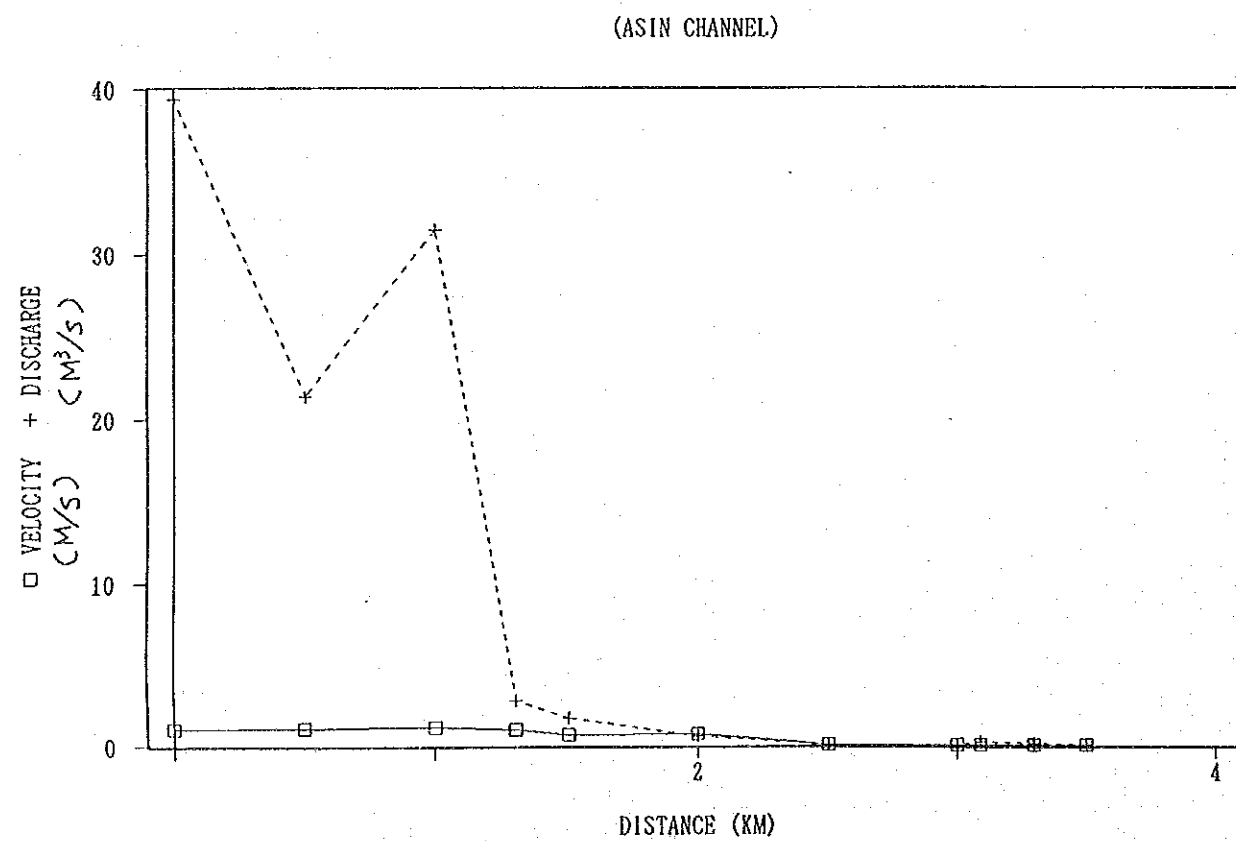
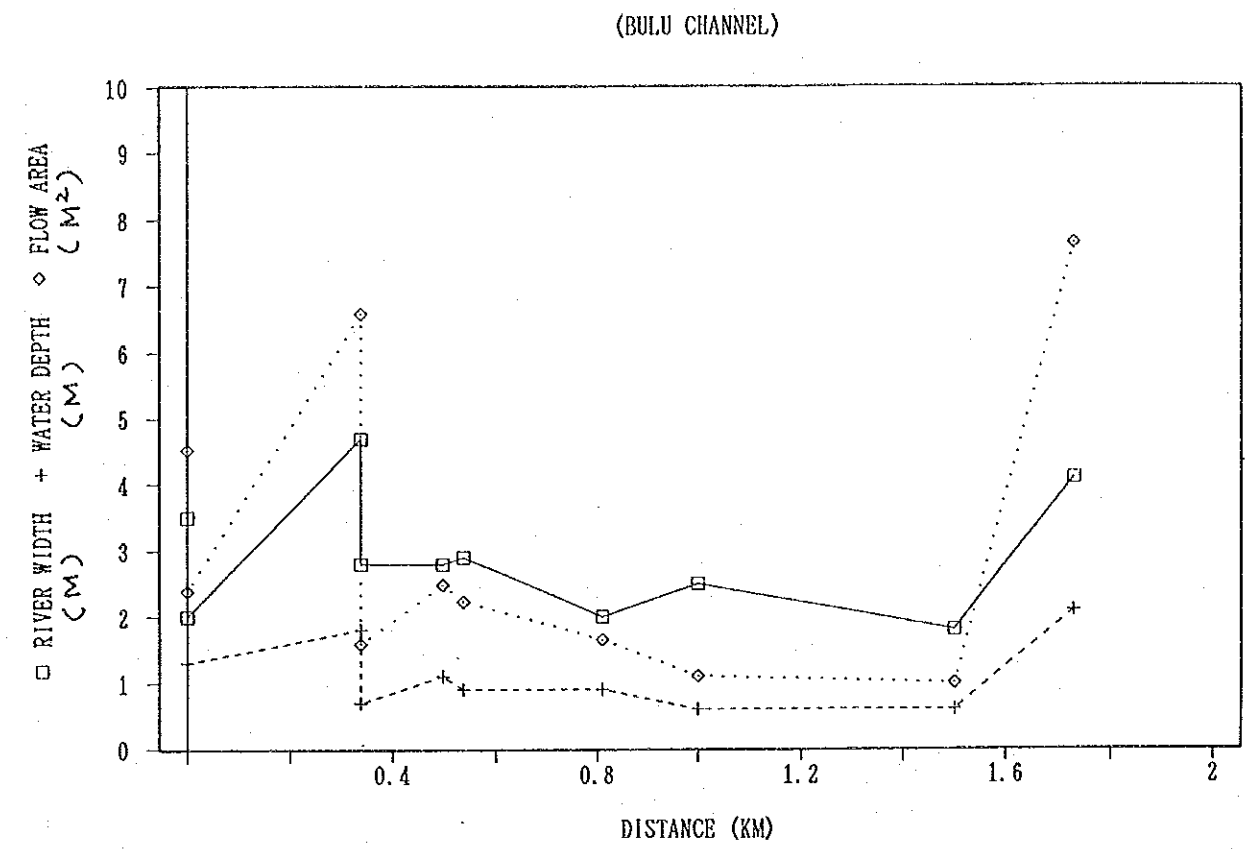
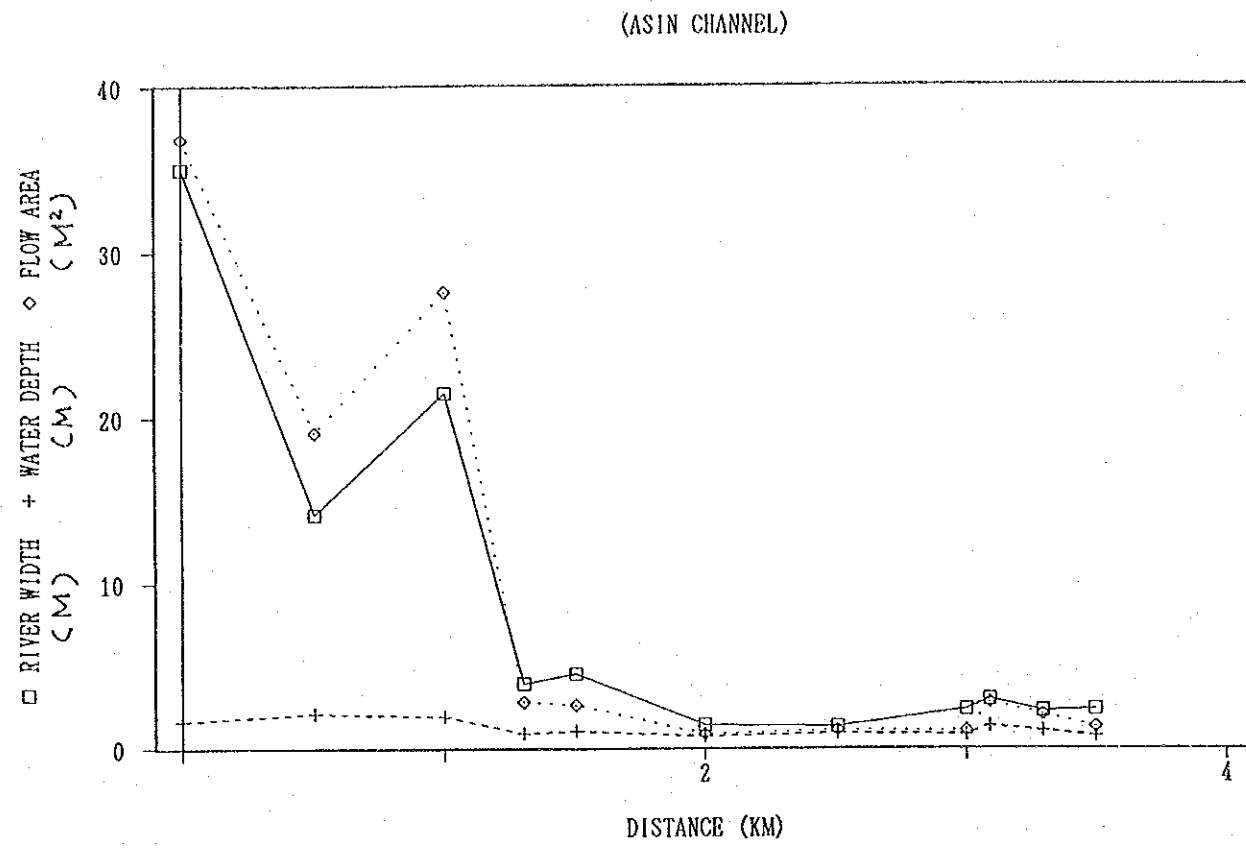


FIG. I-5 HYDRAULIC FEATURES OF EXISTING MAJOR CHANNELS (ASIN RIVER)

FIG. I-6 HYDRAULIC FEATURES OF EXISTING MAJOR CHANNELS (BULU RIVER)

Table I-6 HYDRAULIC FEATURES OF EXISTING MAJOR CHANNELS  
(KALANGAYU RIVER AND RONGGOLAWA RIVER)

RIVER	Station (km)	Channel Width B(m)	Water Depth H(m)	Flow Area A(m <sup>2</sup> )	Wetted Perimeter P(m)	Hydraulic Radius R(m)	Roughness Coefficient n	Hydraulic Gradient i	Velocity V(m/s)	Discharge Q(m <sup>3</sup> /s)
	0.00	4.0	1.4	4.88	6.1	0.80	0.024	0.000500	0.80	3.9
	0.50	3.8	0.9	2.91	5.2	0.56	0.024	0.000500	0.63	1.8
	0.69	9.0	0.9	4.62	12.2	0.38	0.024	0.000500	0.49	2.3
	0.75	9.8	1.7	14.26	12.0	1.19	0.031	0.000500	0.81	11.6
	1.00	5.2	1.0	4.33	6.4	0.68	0.024	0.000500	0.72	3.1
	1.30	5.0	0.7	2.31	13.7	0.17	0.020	0.000500	0.34	0.8
	1.50	7.9	1.5	7.07	9.1	0.78	0.031	0.000500	0.61	4.3
	2.00	4.9	1.7	6.40	6.9	0.93	0.024	0.001440	1.51	9.7
	2.17	3.4	1.1	3.50	5.5	0.64	0.020	0.001440	1.41	4.9
	2.50	4.8	1.2	5.16	6.6	0.78	0.024	0.001440	1.34	6.9
	2.86	12.2	2.2	11.39	10.9	1.04	0.031	0.001440	1.26	14.4
	AVERAGE	6.4	1.3	6.1	8.6	0.72	0.025	0.000842	0.90	5.5
	0.00	4.8	1.5	5.48	6.5	0.84	0.024	0.000270	0.61	3.3
	0.54	4.8	1.3	5.44	6.6	0.82	0.024	0.000270	0.60	3.3
	1.00	3.3	1.0	3.06	4.9	0.62	0.024	0.000270	0.50	1.5
	1.50	7.5	1.6	8.00	8.4	0.95	0.024	0.000270	0.66	5.3
	1.78	5.1	1.2	4.75	11.6	0.41	0.020	0.000270	0.45	2.1
	2.00	7.0	1.6	8.70	8.8	0.99	0.024	0.000270	0.68	5.9
	2.26	11.5	2.1	20.10	18.8	1.07	0.024	0.000270	0.72	14.5
	2.50	4.7	1.8	6.60	6.8	0.97	0.024	0.004600	2.77	18.3
	2.75	4.9	2.6	11.50	9.1	1.26	0.024	0.004600	3.30	38.0
	AVERAGE	6.6	1.7	8.3	9.2	0.88	0.024	0.001216	1.13	9.4

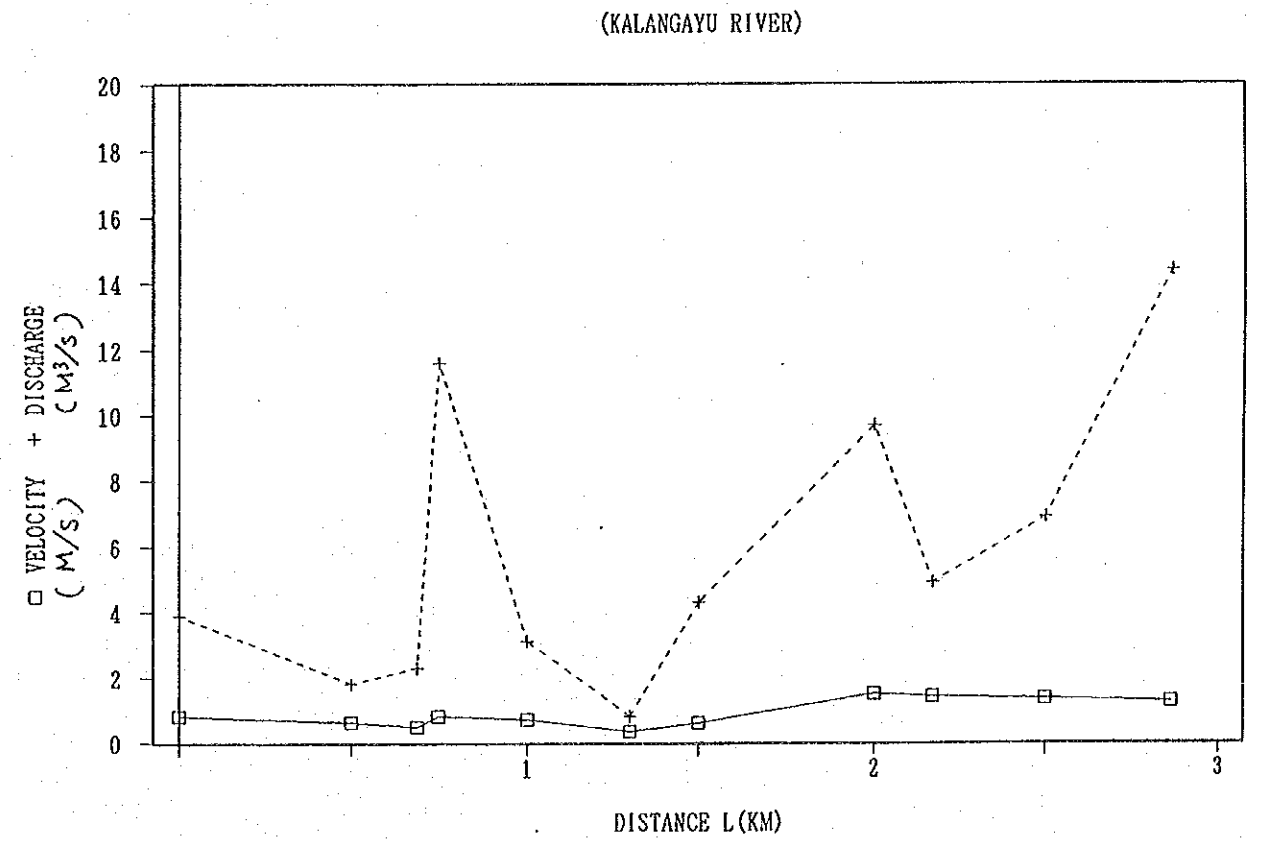
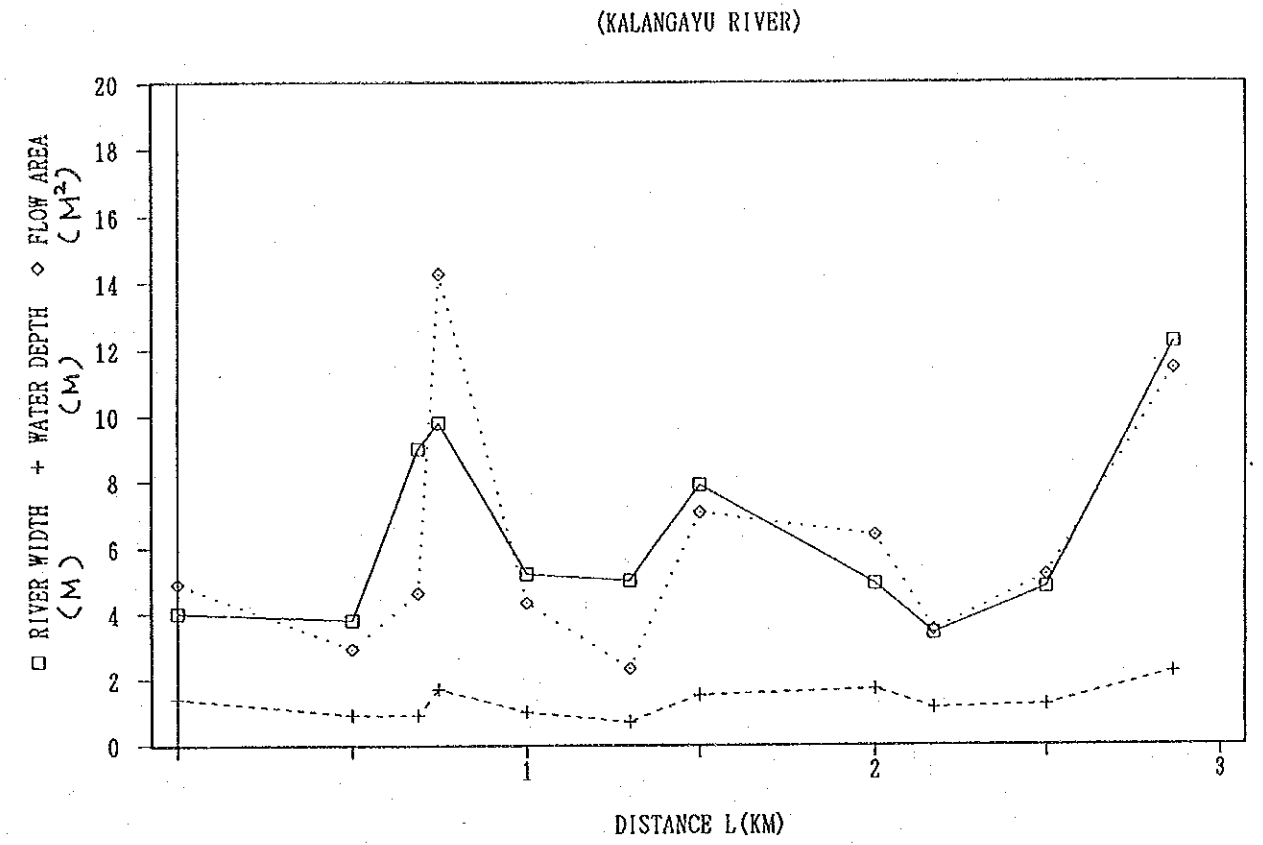


FIG. I-7 HYDRAULIC FEATURES OF EXISTING MAJOR CHANNELS  
(KALANGAYU RIVER)

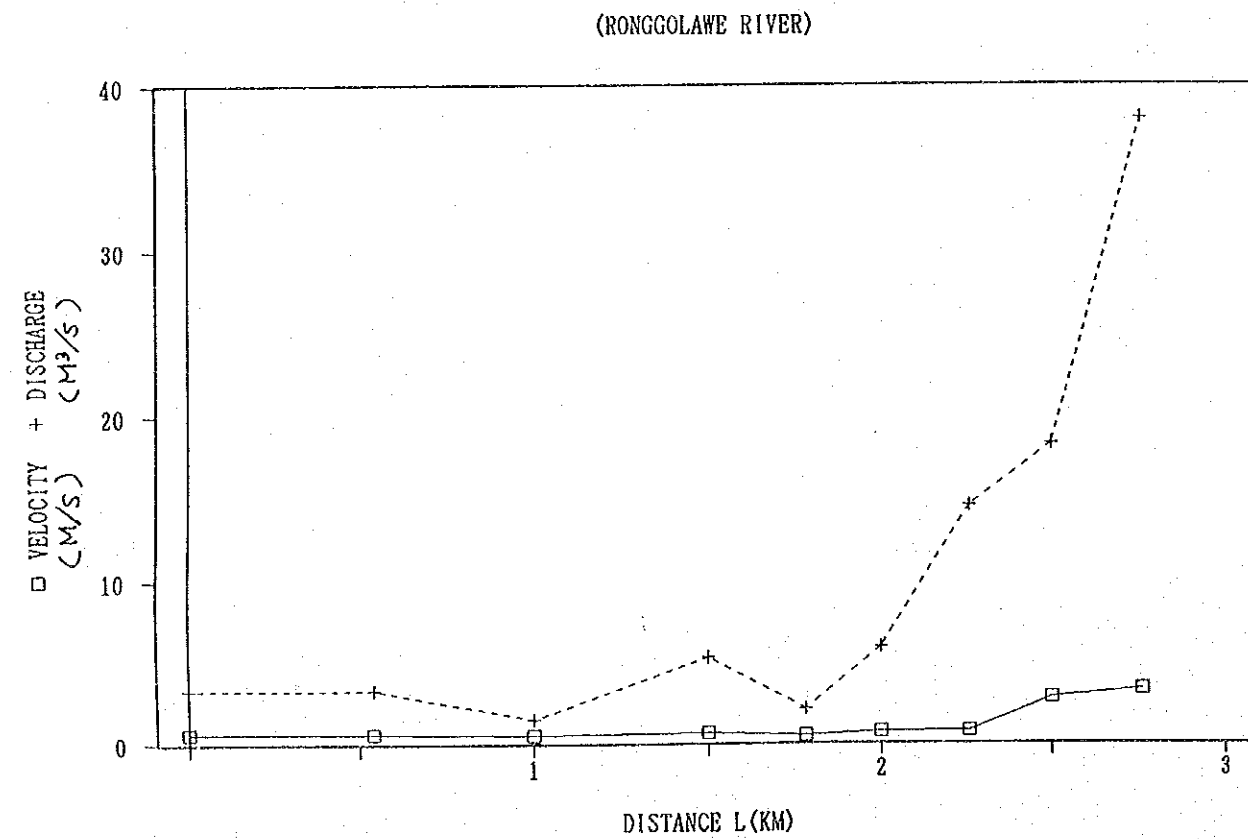
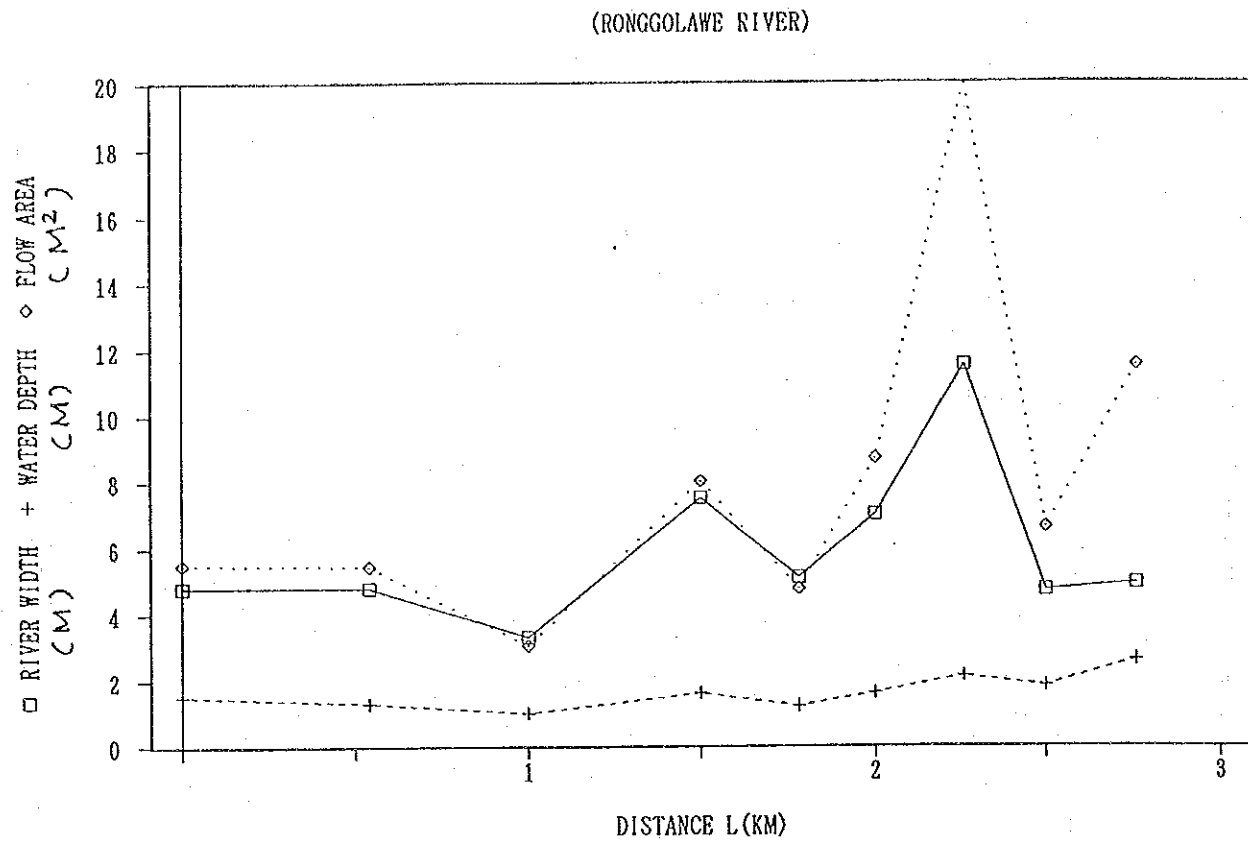
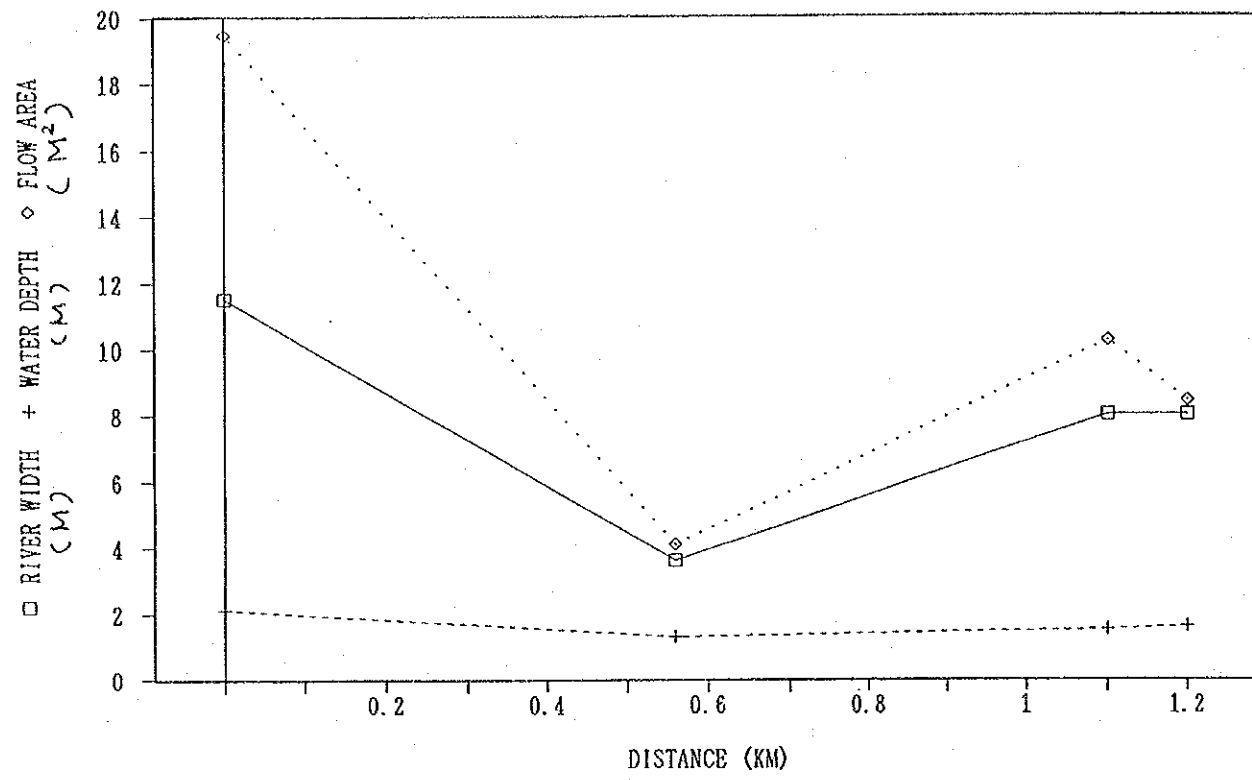


FIG. I-8 HYDRAULIC FEATURES OF EXISTING MAJOR CHANNELS (RONGGOLAWE RIVER)

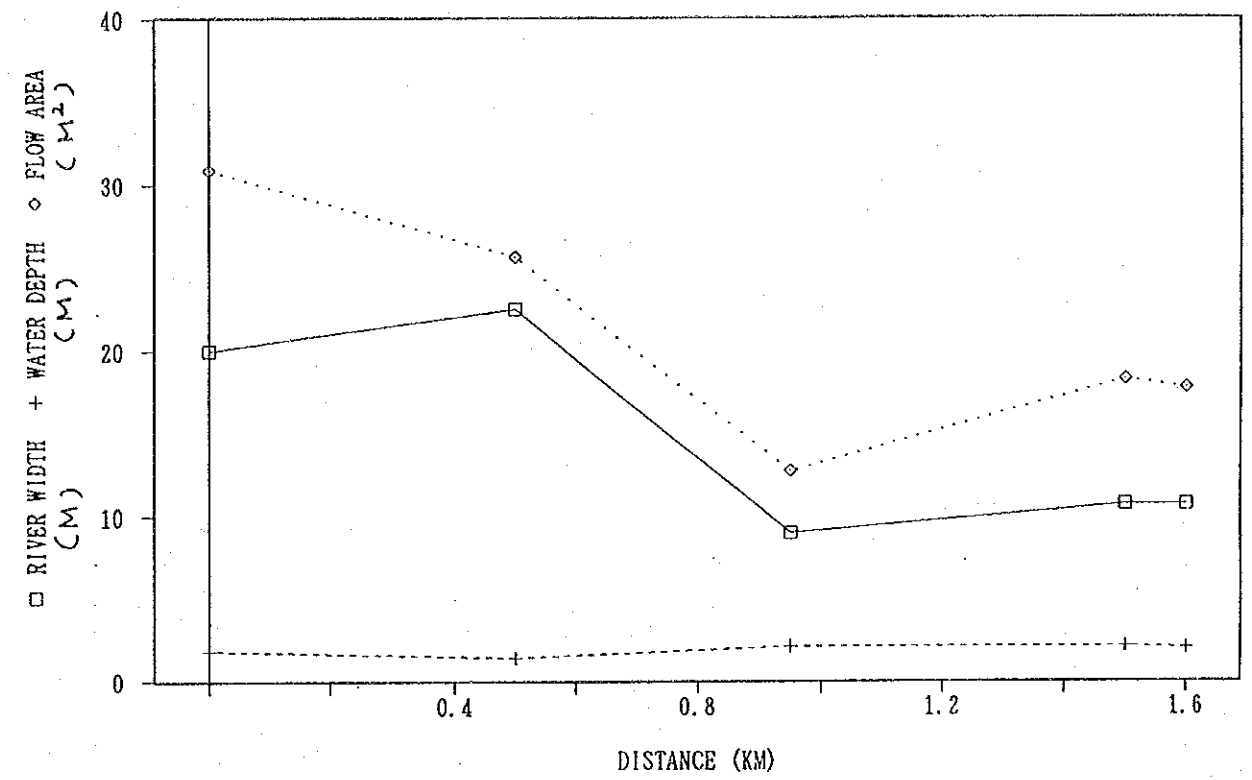
Table I-7 HYDRAULIC FEATURES OF EXISTING MAJOR CHANNELS (TM-1, TM-2 AND TM-3 CHANNELS)

RIVER	Station (km)	Channel Width B(m)	Water Depth H(m)	Flow Area A(m <sup>2</sup> )	Wetted Perimeter P(m)	Hydraulic Radius R(m)	Roughness Coefficient n	Hydraulic Gradient i	Velocity V(m/s)	Discharge Q(m <sup>3</sup> /s)
	0.00	11.5	2.1	19.46	14.3	1.36	0.024	0.000670	1.32	25.7
	0.56	3.6	1.3	4.09	8.0	0.51	0.024	0.000670	0.69	2.8
	1.10	8.0	1.5	10.22	9.9	1.03	0.024	0.000670	1.10	11.2
	1.20	8.0	1.6	8.42	8.6	0.98	0.024	0.000670	1.06	8.9
	AVERAGE	7.8	1.6	10.55	10.2	0.97	0.024	0.000670	1.04	11.0
	0.00	20.0	1.8	30.89	22.5	1.37	0.024	0.000310	0.90	27.8
	0.50	22.5	1.4	25.67	21.9	1.17	0.024	0.000310	0.81	20.8
	0.95	9.0	2.1	12.74	11.0	1.16	0.031	0.000310	0.63	8.0
	1.50	10.7	2.1	18.23	12.6	1.45	0.024	0.000310	0.94	17.1
	1.60	10.7	2.0	17.71	13.2	1.34	0.024	0.000310	0.89	15.8
	AVERAGE	13.2	1.9	18.59	14.7	1.28	0.026	0.000310	0.82	15.2
	0.00	7.2	1.9	6.15	7.4	0.83	0.031	0.002080	1.30	8.0
	0.50	5.5	1.4	3.96	5.5	0.72	0.031	0.002080	1.18	4.7
	0.77	6.5	0.9	5.68	8.1	0.70	0.024	0.002080	1.50	8.5
	0.77	5.9	0.9	5.17	10.8	0.48	0.024	0.002080	1.16	6.0
	AVERAGE	6.3	1.3	5.24	8.0	0.68	0.028	0.002080	1.29	6.7

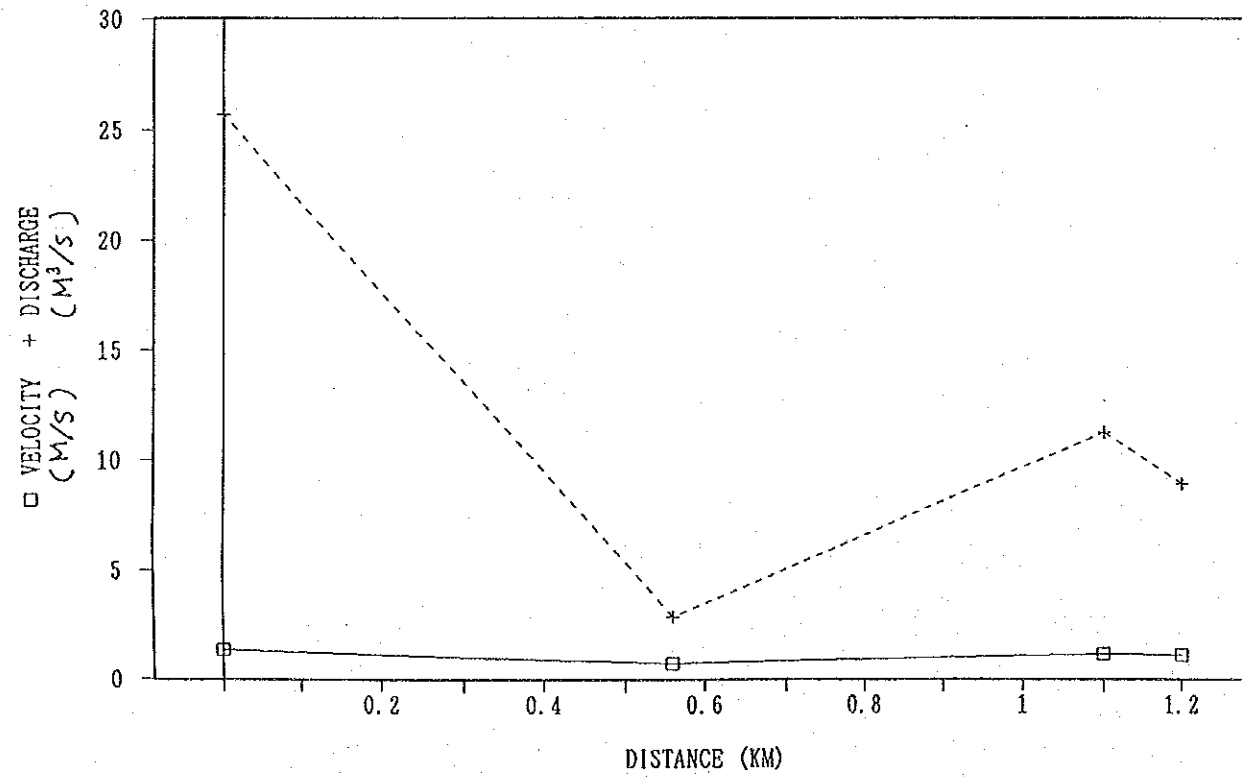
(TM 1 RIVER)



(TM 2 RIVER)



(TM 1 RIVER)



(TM 2 RIVER)

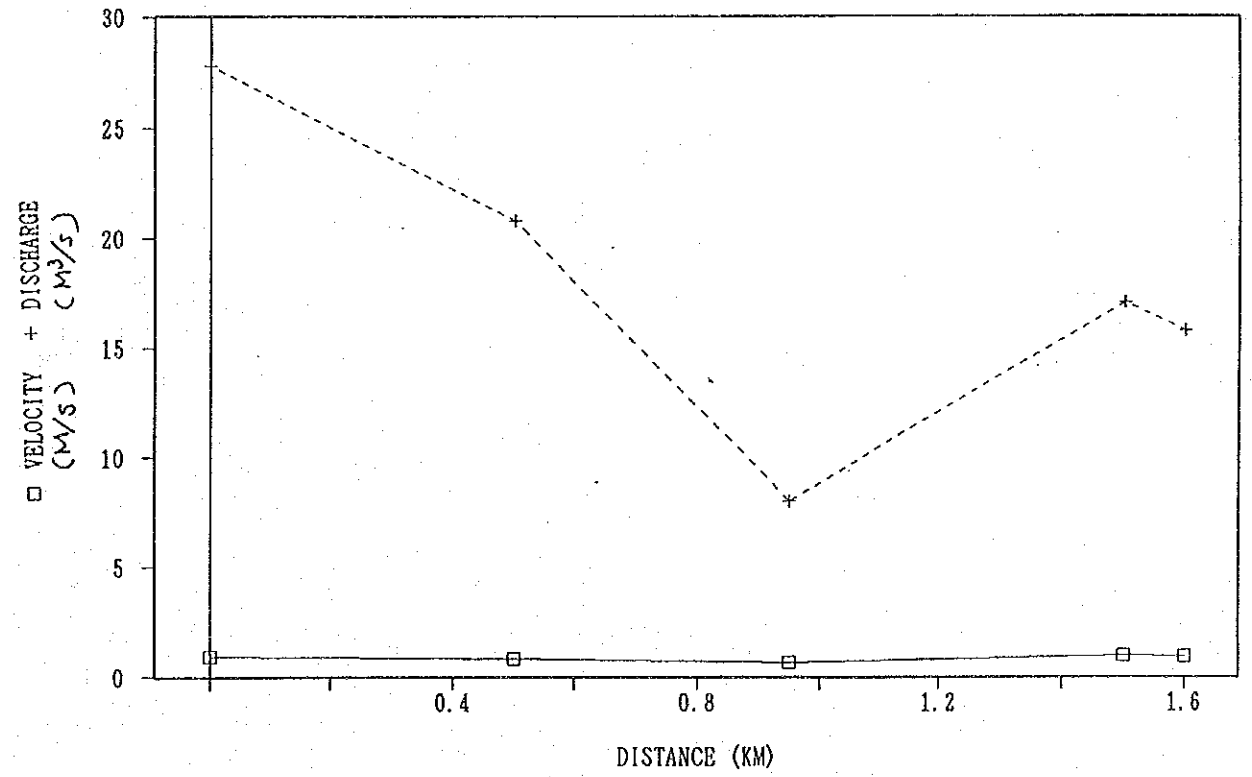
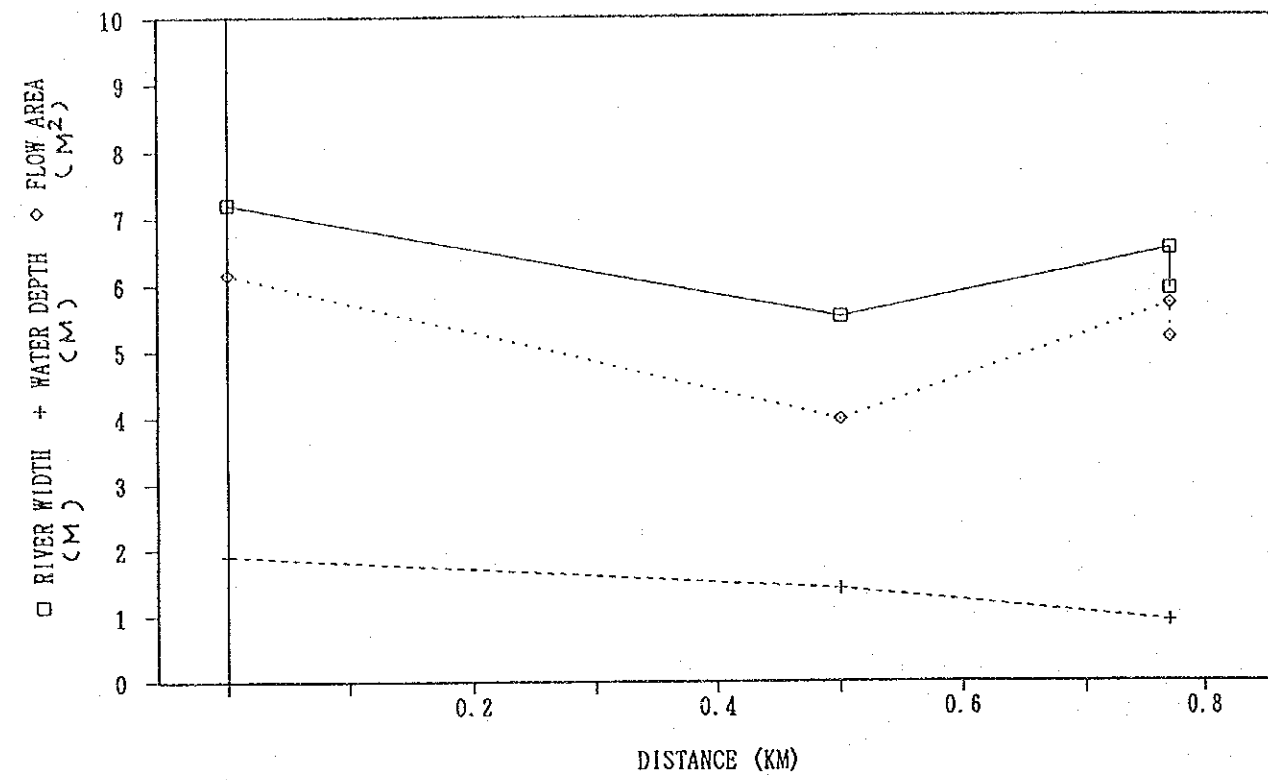


FIG. I-9 HYDRAULIC FEATURES OF EXISTING MAJOR CHANNELS ( TM-1 CHANNEL)

FIG. I-10 HYDRAULIC FEATURES OF EXISTING MAJOR CHANNELS ( TM-2 CHANNEL)

(TM 3 RIVER)



(TM 3 RIVER)

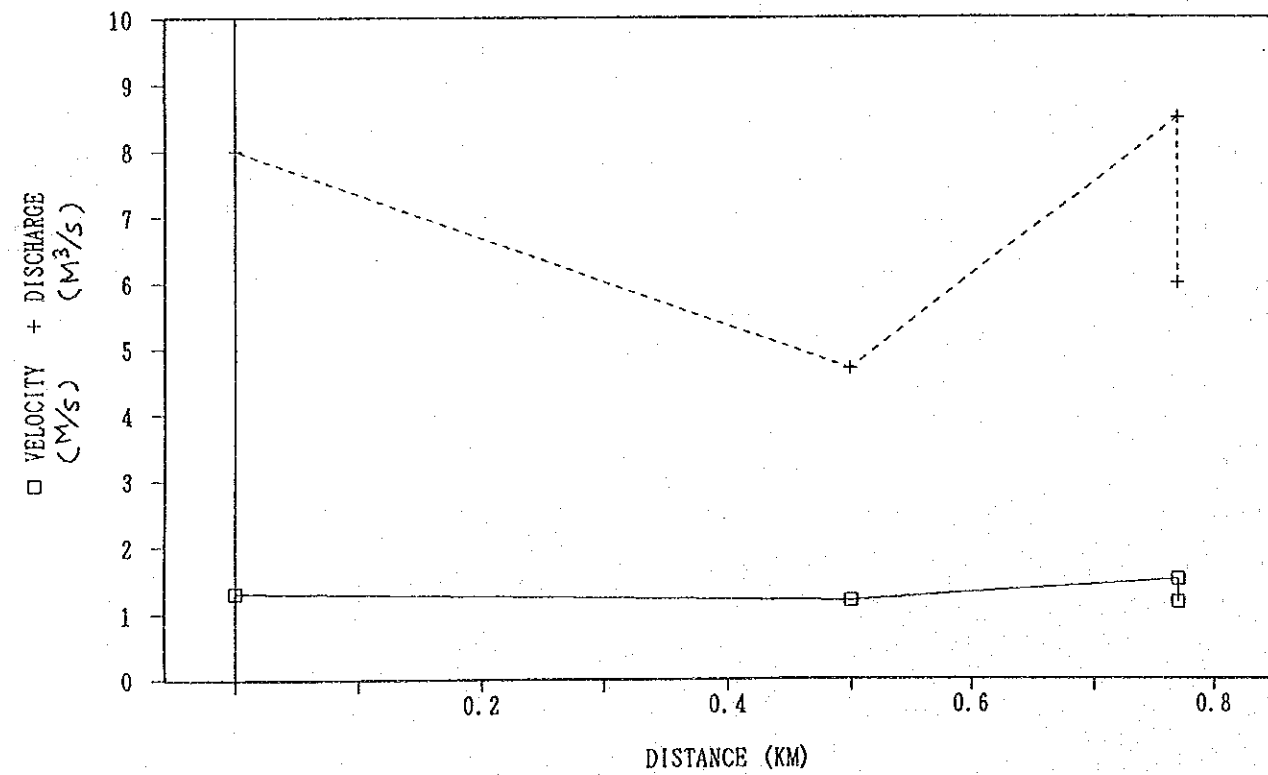


FIG. I-11 HYDRAULIC FEATURES OF EXISTING MAJOR CHANNELS  
(TM-3 CHANNEL)

1. Siringin River

- (1) Drainage System
- (2) Run-off Model
- (3) Run-off Calculation
- (4) Distribution of Design Discharge

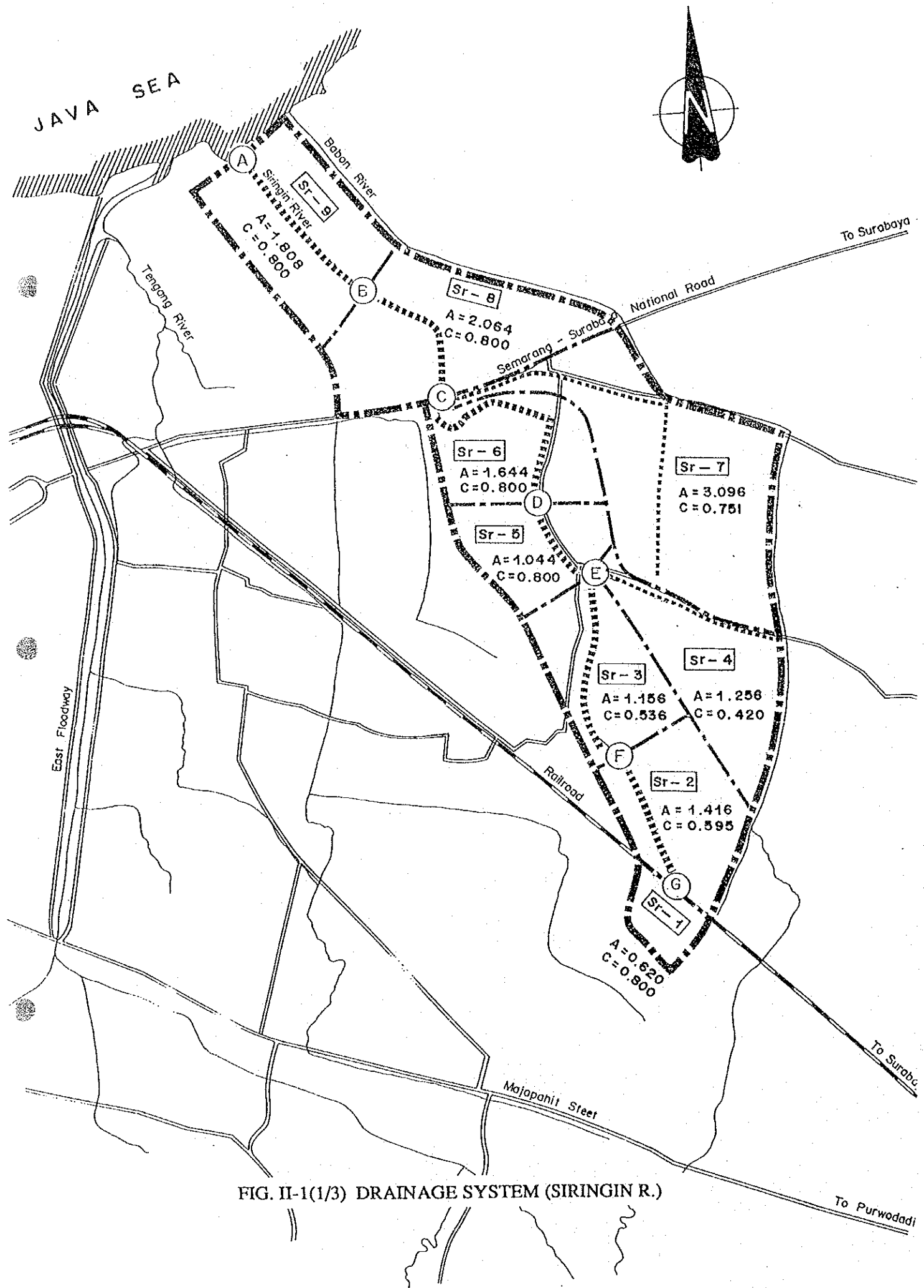
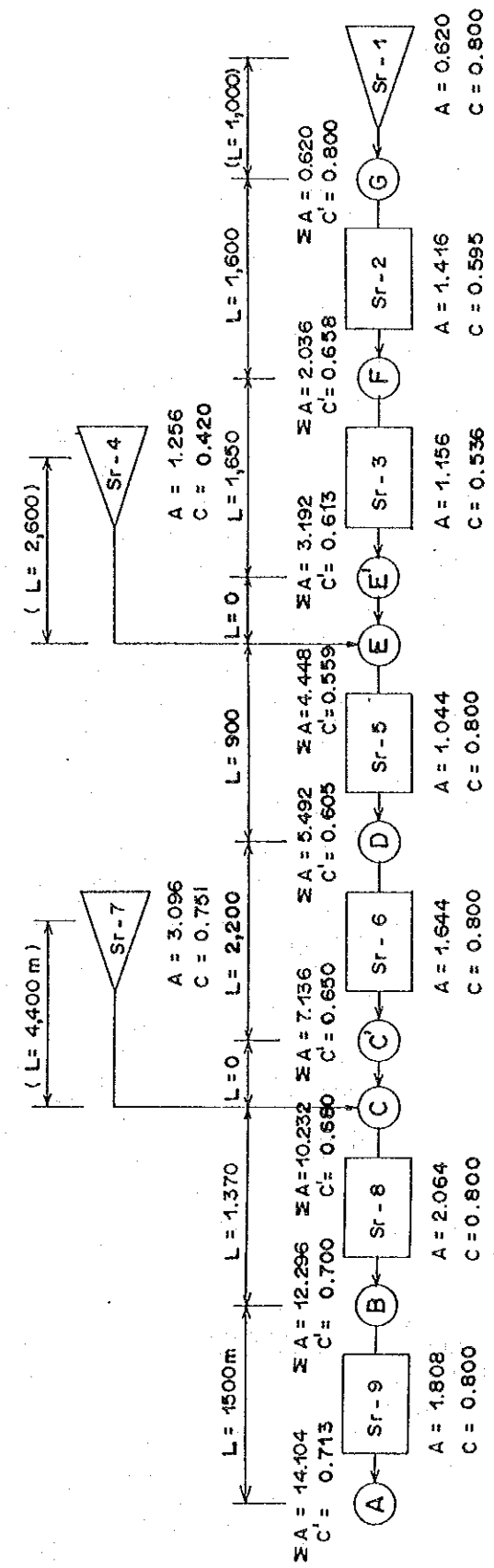


FIG. II-1(1/3) DRAINAGE SYSTEM (SIRINGIN R.)

FIG. II-1(2/3) RUNN-OFF MODEL (SIRINGIN R.)



Land Use	Run-off C		T<2hr		T>2hr	
	0.8	0.5	2-year	10-year	2-year	10-year
Business Area	0.8	0.5	1400	1000	1313	2050
Neighborhood B.A.	0.8	0.5	10.19	4.49	-5.4	2.95
Residential Area	0.5	0.3	0.73	0.6	0.74	0.76
Industrial & Harbor	0.8	0.3				
Green Zone & Others	0.3	0.3				

A. Eastern Semarang Area  
1. Sringin River

Proposed Land Use	km2								
	SI-1	SI-2	SI-3	SI-4	SI-5	SI-6	SI-7	SI-8	SI-9
Business Area	0.620	0.539	-	-	-	-	-	-	-
Neighborhood B.A.	-	0.742	1.018	0.609	-	-	-	-	-
Residential Area	-	-	0.138	0.057	1.044	1.644	2.616	2.064	1.808
Industrial & Harbor	-	0.135	-	0.590	-	-	-	-	-
Green Zone & Others	-	1.416	1.156	1.256	1.044	1.644	3.096	2.064	1.808
Total Area	0.620	1.416	1.156	1.256	1.044	1.644	3.096	2.064	1.808
Run-off Coefficient	0.800	0.595	0.536	0.420	0.800	0.800	0.751	0.800	0.800
Length of River Cannal(m)	1,000	1,600	1,650	2,600	900	2,200	4,400	1,370	1,500

Calculation of Peak Discharge

Calculation Point	G	F	E'	E''	E	D	C'	C''	C	B	A
Business Area	0.620	1.159	1.159	-	1.159	1.159	1.159	-	1.159	1.159	1.159
Neighborhood B.A.	-	0.000	0.000	-	0.000	0.000	0.000	-	0.000	0.000	0.000
Residential Area	-	0.742	1.760	0.609	2.370	2.370	2.370	0.440	2.809	2.809	2.809
Industrial & Harbor	-	0.000	0.138	0.057	0.194	1.238	2.882	2.616	5.498	7.562	9.370
Green Zone & Others	-	0.135	0.135	0.590	0.725	0.725	0.725	0.040	0.765	0.765	0.765
Total Area	0.620	2.036	3.192	1.256	4.448	5.492	7.136	3.096	10.232	12.296	14.104
Run-off Coefficient	0.800	0.658	0.613	0.420	0.559	0.605	0.650	0.751	0.680	0.700	0.713
Length of River Cannal(m)	1,000	1,600	1,650	2,600	900	2,200	4,400	1,370	1,500	1,370	1,500
Accumulated Length (m)	1,000	2,600	4,250	2,600	4,250	5,150	7,350	4,400	7,350	8,720	10,220
Proposed River Slope(%)	S	0.111%	0.111%	S	0.050%	0.050%	S	S	0.036%	0.036%	0.036%
Proposed Velocity (m/s)	1.250	1.000	1.000	1.250	-	0.750	0.750	1.000	-	0.600	0.600
Flowing Time (min)	13.33	26.67	27.50	34.67	-	20.00	48.89	73.33	-	38.06	41.67
Duration Time (min)	28.33	55.00	82.50	49.67	82.50	102.50	151.39	88.33	151.39	189.44	231.11
Rainfall Intensity(mm/hr)	123.11	86.17	68.60	91.16	68.60	60.59	44.52	65.98	44.52	37.65	32.44
Reduction Factor	0.995	0.983	0.974	0.989	0.965	0.958	0.948	0.975	0.931	0.922	0.915
Peak Discharge(m3/s)	13.499	25.197	29.079	10.561	36.564	42.845	43.484	33.229	64.105	66.422	66.312
Specific Q (m3/s/km2)	21.772	12.376	9.110	8.408	8.220	7.801	6.094	10.733	6.265	5.402	4.702

TABLE II-1(1/2) RUN-OFF CALCULATION (5-YEAR FERQUENCY FLOOD)  
(SIRINGIN R.)

Land Use	Run-off C		T<2hr		T>2hr	
	0.8	0.5	2-year	10-year	2-year	10-year
Business Area	0.8	0.5	1400	967	1313	2600
Neighborhood B.A.	0.8	0.5	10.19	2.93	-5.4	7.2
Residential Area	0.5	0.3	0.73	0.57	0.74	0.78
Industrial & Harbor	0.8	0.3				
Green Zone & Others	0.3	0.3				

A. Eastern Semarang Area  
1. Sringin River

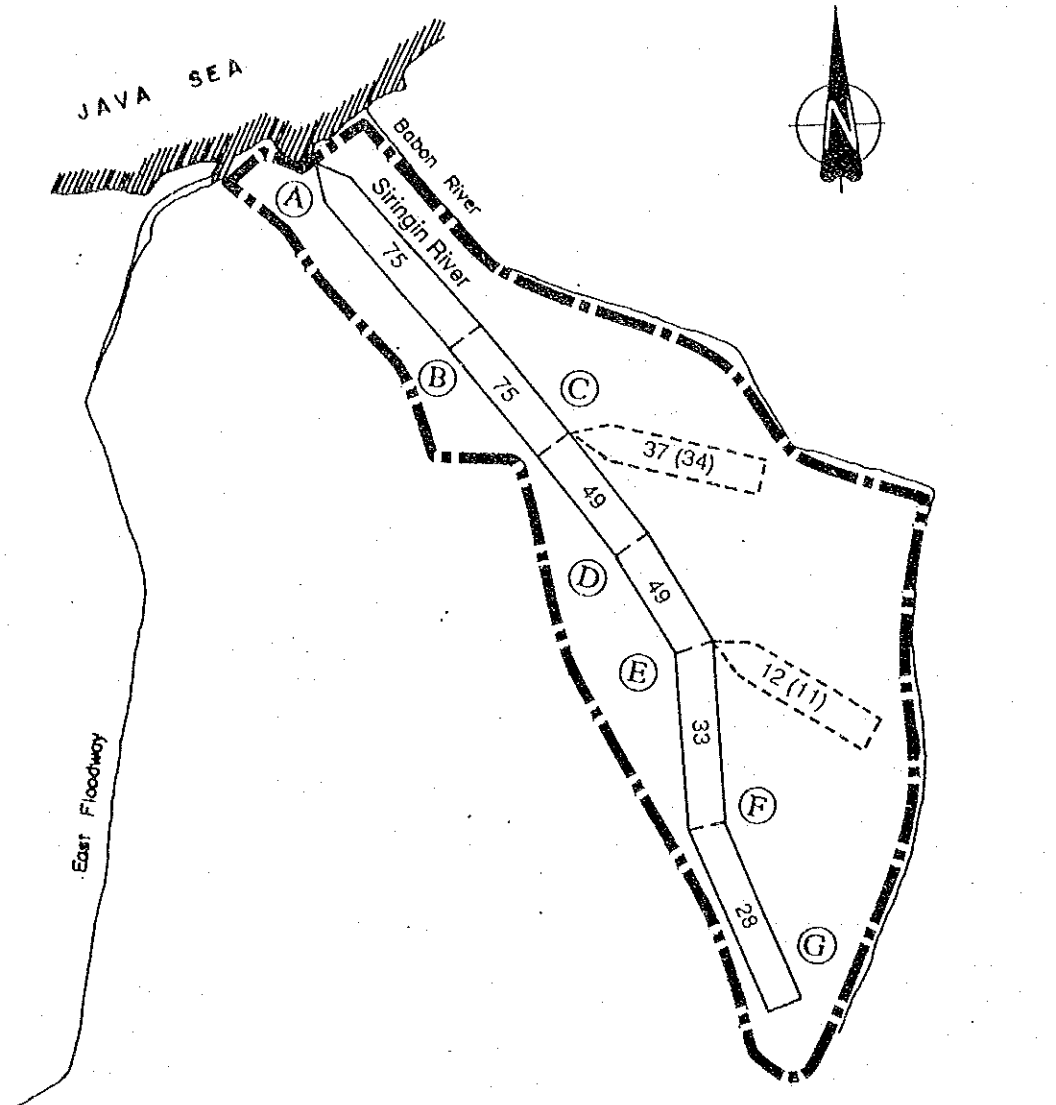
Proposed Land Use	km2								
	SI-1	SI-2	SI-3	SI-4	SI-5	SI-6	SI-7	SI-8	SI-9
Business Area	0.620	0.539	-	-	-	-	-	-	-
Neighborhood B.A.	-	0.742	1.018	0.609	-	-	-	-	-
Residential Area	-	-	0.138	0.057	1.044	1.644	2.616	2.064	1.808
Industrial & Harbor	-	0.135	-	0.590	-	-	-	-	-
Green Zone & Others	-	1.416	1.156	1.256	1.044	1.644	3.096	2.064	1.808
Total Area	0.620	1.416	1.156	1.256	1.044	1.644	3.096	2.064	1.808
Run-off Coefficient	0.800	0.595	0.536	0.420	0.800	0.800	0.751	0.800	0.800
Length of River Cannal(m)	1,000	1,600	1,650	2,600	900	2,200	4,400	1,370	1,500

Calculation of Peak Discharge

Calculation Point	G	F	E'	E''	E	D	C'	C''	C	B	A
Business Area	0.620	1.159	1.159	-	1.159	1.159	1.159	-	1.159	1.159	1.159
Neighborhood B.A.	-	0.000	0.000	-	0.000	0.000	0.000	-	0.000	0.000	0.000
Residential Area	-	0.742	1.760	0.609	2.370	2.370	2.370	0.440	2.809	2.809	2.809
Industrial & Harbor	-	0.000	0.138	0.057	0.194	1.238	2.882	2.616	5.498	7.562	9.370
Green Zone & Others	-	0.135	0.135	0.590	0.725	0.725	0.725	0.040	0.765	0.765	0.765
Total Area	0.620	2.036	3.192	1.256	4.448	5.492	7.136	3.096	10.232	12.296	14.104
Run-off Coefficient	0.800	0.658	0.613	0.420	0.559	0.605	0.650	0.751	0.680	0.700	0.713
Length of River Cannal(m)	1,000	1,600	1,650	2,600	900	2,200	4,400	1,370	1,500	1,370	1,500
Accumulated Length (m)	1,000	2,600	4,250	2,600	4,250	5,150	7,350	4,400	7,350	8,720	10,220
Proposed River Slope(%)	S	0.111%	0.111%	S	0.050%	0.050%	S	S	0.036%	0.036%	0.036%
Proposed Velocity (m/s)	1.250	1.000	1.000	1.250	-	0.750	0.750	1.000	-	0.600	0.600
Flowing Time (min)	13.33	26.67	27.50	34.67	-	20.00	48.89	73.33	-	38.06	41.67
Duration Time (min)	28.33	55.00	82.50	49.67	82.50	102.50	151.39	88.33	151.39	189.44	231.11
Rainfall Intensity(mm/hr)	135.91	95.63	76.63	101.04	76.63	67.97	49.98	73.80	49.98	42.26	36.37
Reduction Factor	0.995	0.983	0.974	0.989	0.965	0.958	0.948	0.975	0.931	0.922	0.915
Peak Discharge(m3/s)	14.903	27.963	32.485	11.705	40.845	48.066	48.817	37.168	71.968	74.550	74.358
Specific Q (m3/s/km2)	24.037	13.734	10.177	9.319	9.183	8.752	6.841	12.005	7.034	6.063	5.272

TABLE II-1(2/2) RUN-OFF CALCULATION (10-YEAR FERQUENCY FLOOD)  
(SIRINGIN R.)





- NOTE : 1. Unit of the figures is m<sup>3</sup>/s.  
 2. The figures with and without parenthesis show the design discharge of a 5-year and 10-year frequency flood respectively.

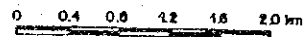


FIG. II-1(3/3) DISTRIBUTION OF DESIGN DISCHARGE(SIRINGIN R.)

2. Tenggang River

2-1 : (Alternative 1-A)

- (1) Drainage System
- (2) Run-off Model
- (3) Run-off Calculation
- (4) Distribution of Design Discharge

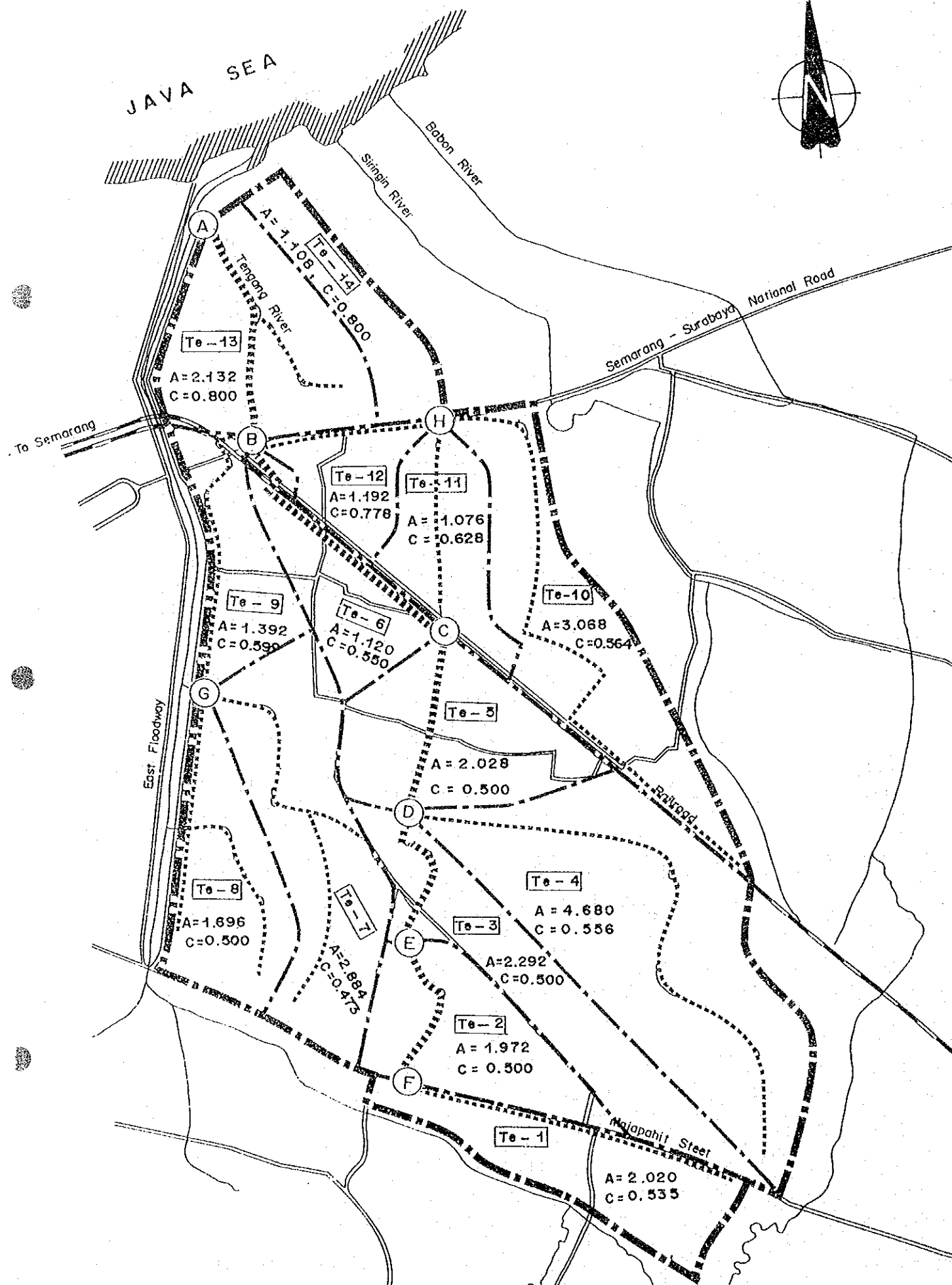
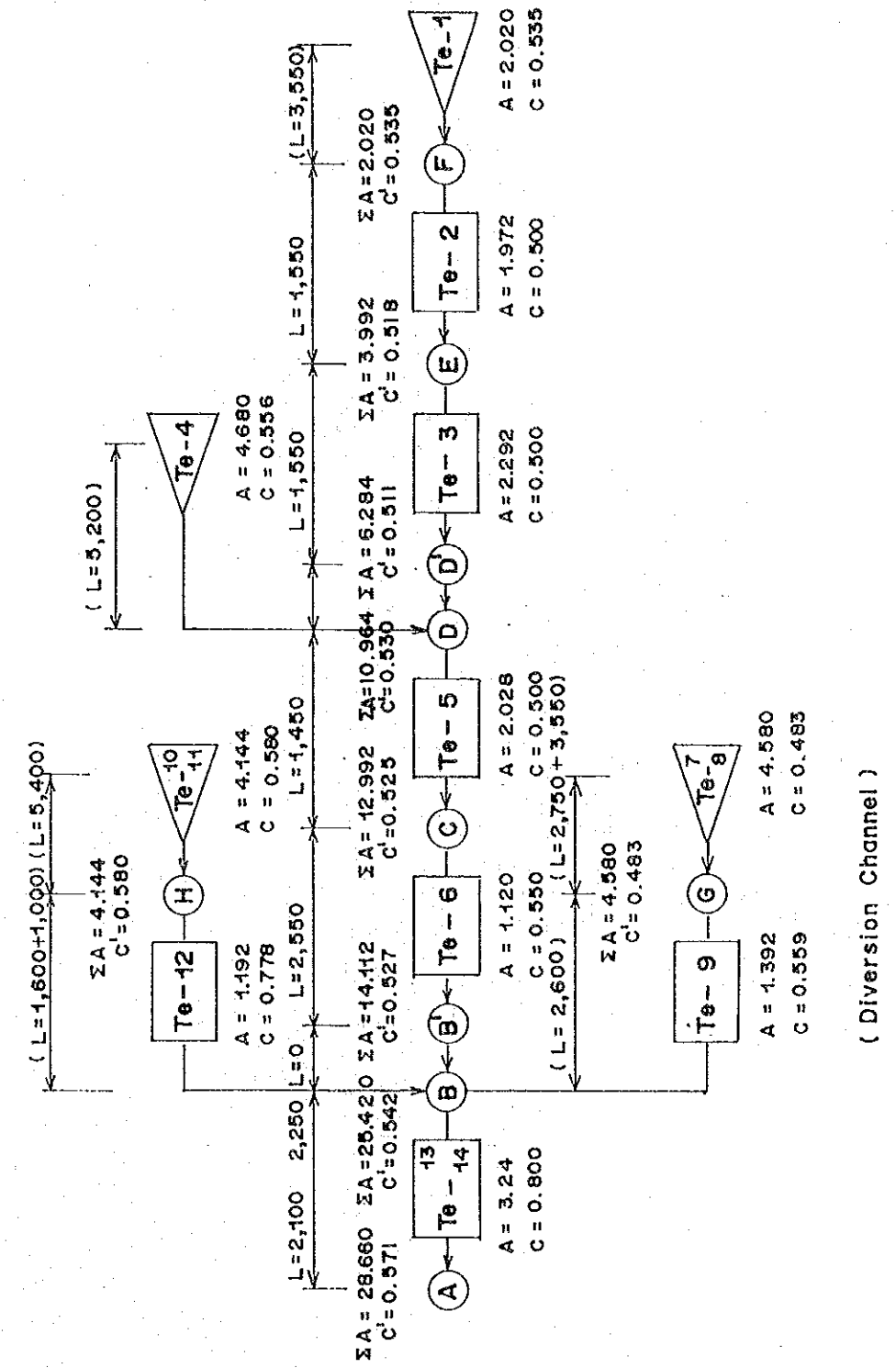


FIG. II-2(1/3) DRAINAGE SYSTEM (TENGANG R., ALTER. 1-A)

FIG. II-2(2/3) RUN-OFF MODEL (TENGANG R., ALTER. 1-A)



Land Use Run-off C  
 Business Area 0.8  
 Neighborhood B.A. 0.8  
 Residential Area 0.5  
 Industrial & Harbor 0.8  
 Green Zone & Others 0.3

T<2hr T>2hr  
 2-year 5-year 10-year 2-year 5-year 10-year  
 1400 1000 1000 1313 2050 2050  
 10.19 4.49 4.49 -5.4 2.95 2.95  
 0.73 0.6 0.6 0.74 0.76 0.76

A. Eastern Semarang Area 2. Tenggang River

Name of Basin	km2													
	Te-1	Te-2	Te-3	Te-4	Te-5	Te-6	Te-7	Te-8	Te-9	Te-10	Te-11	Te-12	Te-13	Te-14
Business Area	-	-	-	0.866	-	-	-	-	-	-	-	-	-	-
Neighborhood B.A.	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Residential Area	1.782	1.972	2.290	3.814	2.028	0.933	2.498	1.696	1.119	2.415	0.618	0.089	-	-
Industrial & Harbor	0.238	-	-	-	-	0.187	-	-	0.273	0.653	0.458	1.103	2.132	1.108
Green Zone & Others	-	-	0.002	-	-	-	0.386	-	-	-	-	-	-	-
Total Area	2.020	1.972	2.292	4.680	2.028	1.120	2.884	1.696	1.392	3.068	1.076	1.192	2.132	1.108
Run-off Coefficient	0.535	0.500	0.500	0.556	0.500	0.550	0.473	0.500	0.559	0.564	0.628	0.778	0.800	0.800
Length of River Cannal	3.550	1.550	1.550	5.200	1.450	2.550	3.550	2.750	2.500	5.400	2.250	1.600	2.100	2.250

Calculation of Peak Discharge 2. Tenggang River Alternative 1-A

Calculation Point	Alternative 1-A													
	F	E	D'	D''	D	C	B'	B''	G	B''	H	B''	A	H-C
Business Area	-	0.000	0.000	0.866	0.866	0.866	0.866	0.000	0.000	0.000	0.000	0.000	0.866	0.866
Neighborhood B.A.	-	0.000	0.000	-	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
Residential Area	1.782	3.754	6.043	3.814	9.858	11.886	12.819	4.194	5.313	3.032	3.122	21.253	21.253	0.618
Industrial & Harbor	0.238	0.238	0.238	-	0.238	0.238	0.425	0.000	0.273	1.112	2.214	2.913	6.153	0.458
Green Zone & Others	-	0.000	0.002	-	0.002	0.002	0.002	0.386	0.386	0.000	0.000	0.389	0.389	-
Total Area	2.020	3.992	6.284	4.680	10.964	12.992	14.112	4.580	5.972	4.144	5.336	25.420	28.660	1.076
Run-off Coefficient	0.535	0.518	0.511	0.556	0.530	0.525	0.527	0.483	0.501	0.580	0.628	0.542	0.571	0.628
Length of River Cannal(m)	3.550	1.550	1.550	5.200	-	1.450	2.550	3.550	2.600	5.400	1.600	-	2.100	2.250
Accumulated Length (m)	3.550	5.100	6.650	5.200	6.650	8.100	10.650	3.550	6.150	5.400	7.000	10.650	12.750	2.250
Proposed Reiver Slope (%)	S	0.154%	0.154%	S	-	0.133%	0.050%	S	S	S	S	-	0.029%	0.012%
Proposed Velocity (m/s)	1.250	1.250	1.250	1.250	-	1.000	0.750	1.250	1.250	1.000	1.000	-	0.600	1.000
Flowing Time (min)	47.33	20.67	20.67	69.33	-	24.17	56.67	47.33	34.67	90.00	26.67	-	58.33	37.50
Duration Time (min)	62.33	83.00	103.67	84.33	103.67	127.83	184.50	62.33	97.00	105.00	131.67	184.50	242.83	52.50
Rainfall Intensity(mm/hr)	80.36	88.36	60.20	67.75	60.20	50.49	38.40	80.36	62.54	59.76	52.43	38.40	31.26	88.41
Reduction Factor	0.983	0.968	0.953	0.964	0.928	0.919	0.914	0.964	0.955	0.957	0.959	0.874	0.864	0.931
Peak Discharge(m3/s)	18.984	30.409	40.977	37.716	72.138	70.395	58.091	38.104	39.701	30.894	37.249	102.652	98.176	13.152
Specific Q (m3/s/km2)	9.398	7.617	6.521	8.059	6.580	5.418	4.116	8.320	6.648	7.455	6.981	4.038	3.426	12.223

TABLE II-2(1/2) RUN-OFF CALCULATION (5-YEAR FERQUENCY FLOOD)  
 (TENGGANG R.,ALTER.1-A)

Land Use Run-off C  
 Business Area 0.8  
 Neighborhood B.A. 0.8  
 Residential Area 0.5  
 Industrial & Harbor 0.8  
 Green Zone & Others 0.3

T<2hr T>2hr  
 2-year 5-year 10-year 2-year 5-year 10-year  
 1400 967 2600 1313 2600 2600  
 10.19 2.93 2.93 -5.4 7.2 7.2  
 0.73 0.57 0.57 0.74 0.78 0.78

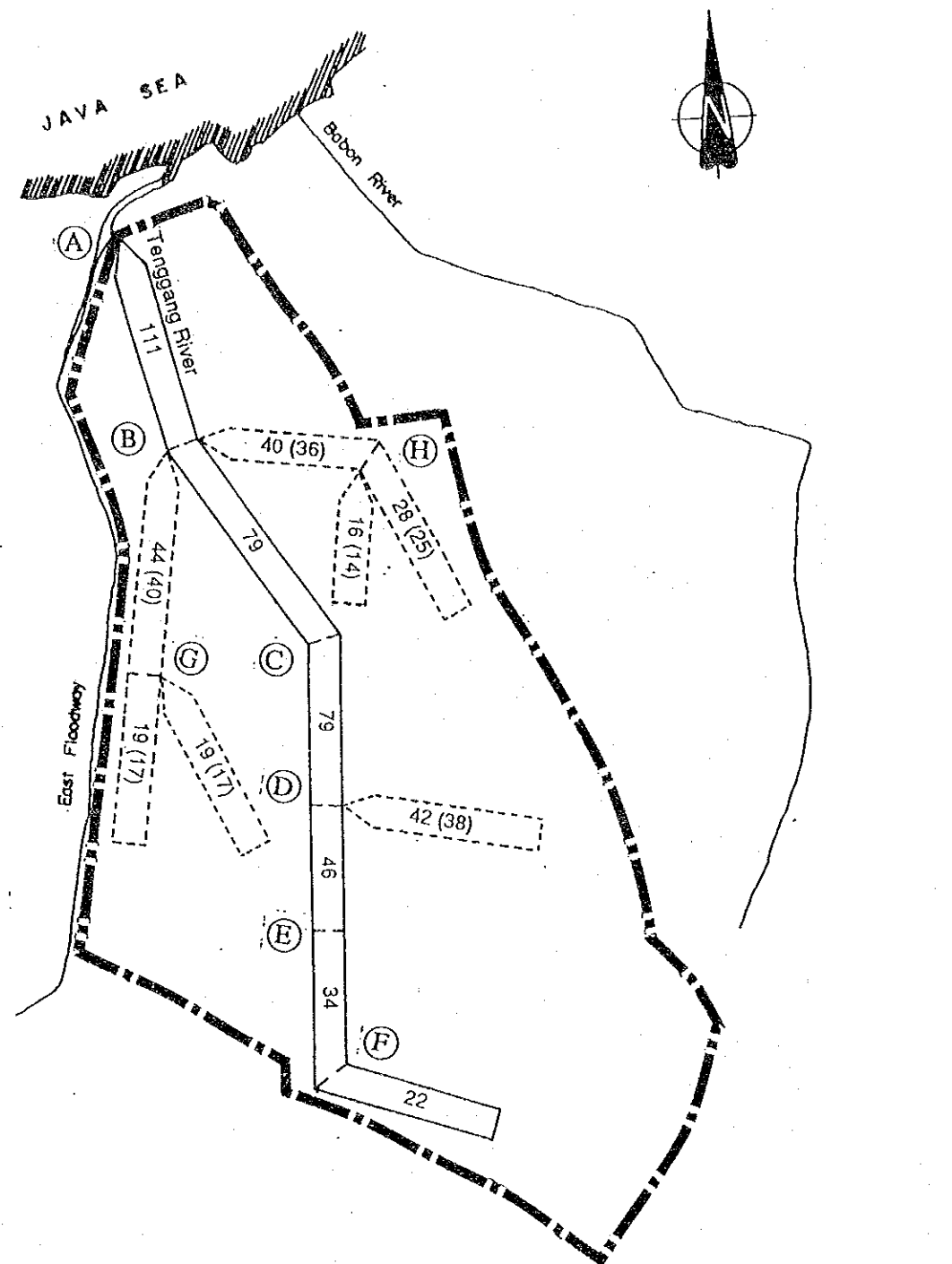
A. Eastern Semarang Area 2. Tenggang River

Name of Basin	km2													
	Te-1	Te-2	Te-3	Te-4	Te-5	Te-6	Te-7	Te-8	Te-9	Te-10	Te-11	Te-12	Te-13	Te-14
Business Area	-	-	-	0.866	-	-	-	-	-	-	-	-	-	-
Neighborhood B.A.	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Residential Area	1.782	1.972	2.290	3.814	2.028	0.933	2.498	1.696	1.119	2.415	0.618	0.089	-	-
Industrial & Harbor	0.238	-	-	-	-	0.187	-	-	0.273	0.653	0.458	1.103	2.132	1.108
Green Zone & Others	-	-	0.002	-	-	-	0.386	-	-	-	-	-	-	-
Total Area	2.020	1.972	2.292	4.680	2.028	1.120	2.884	1.696	1.392	3.068	1.076	1.192	2.132	1.108
Run-off Coefficient	0.535	0.500	0.500	0.556	0.500	0.550	0.473	0.500	0.559	0.564	0.628	0.778	0.800	0.800
Length of River Cannal	3.550	1.550	1.550	5.200	1.450	2.550	3.550	2.750	2.600	5.400	2.250	1.600	2.100	2.250

Calculation of Peak Discharge 2. Tenggang River Alternative 1-A

Calculation Point	Alternative 1-A													
	F	E	D'	D''	D	C	B'	B''	G	B''	H	B''	A	H-C
Business Area	-	0.000	0.000	0.866	0.866	0.866	0.866	0.000	0.000	0.000	0.000	0.000	0.866	0.866
Neighborhood B.A.	-	0.000	0.000	-	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
Residential Area	1.782	3.754	6.043	3.814	9.858	11.886	12.819	4.194	5.313	3.032	3.122	21.253	21.253	0.618
Industrial & Harbor	0.238	0.238	0.238	-	0.238	0.238	0.425	0.000	0.273	1.112	2.214	2.913	6.153	0.458
Green Zone & Others	-	0.000	0.002	-	0.002	0.002	0.002	0.386	0.386	0.000	0.000	0.389	0.389	-
Total Area	2.020	3.992	6.284	4.680	10.964	12.992	14.112	4.580	5.972	4.144	5.336	25.420	28.660	1.076
Run-off Coefficient	0.535	0.518	0.511	0.556	0.530	0.525	0.527	0.483	0.501	0.580	0.628	0.542	0.571	0.628
Length of River Cannal(m)	3.550	1.550	1.550	5.200	-	1.450	2.550	3.550	2.600	5.400	1.600	-	2.100	2.250
Accumulated Length (m)	3.550	5.100	6.650	5.200	6.650	8.100	10.650	3.550	6.150	5.400	7.000	10.650	12.750	2.250
Proposed Reiver Slope (%)	S	0.154%	0.154%	S	-	0.133%	0.050%	S	S	S	S	-	0.029%	0.012%
Proposed Velocity (m/s)	1.250	1.250	1.250	1.250	-	1.000	0.750	1.250	1.250	1.000	1.000	-	0.600	1.000
Flowing Time (min)	47.33	20.67	20.67	69.33	-	24.17	56.67	47.33	34.67	90.00	26.67	-	58.33	37.50
Duration Time (min)	62.33	83.00	103.67	84.33	103.67	127.83	184.50	62.33	97.00	105.00	131.67	184.50	242.83	52.50
Rainfall Intensity(mm/hr)	80.34	76.38	67.55	75.71	67.55	56.65	43.11	89.34	70.08	67.07	59.14	43.11	35.04	98.06
Reduction Factor	0.983	0.968	0.953	0.964	0.928	0.919	0.914	0.964	0.955	0.957	0.959	0.874	0.864	0.931
Peak Discharge(m3/s)	21.106	33.973	45.981	42.149	80.849	78.995	65.204	42.364	44.490	34.876	42.016	115.222	110.054	14.587
Specific Q (m3/s/km2)	10.448	8.510	7.317	9.006	7.383	6.080	4.620	9.250	7.450	8.368	7.874	4.533	3.840	13.556

TABLE II-2(2/2) RUN-OFF CALCULATION (10-YEAR FERQUENCY FLOOD)  
 (TENGGANG R.,ALTER.1-A)



NOTE : 1. Unit of the figures is m<sup>3</sup>/s.  
 2. The figures with and without parenthesis show the design discharge of a 5-year and 10-year frequency flood respectively.

FIG. II-2(3/3) DISTRIBUTION OF DESIGN DISCHARGE(TENGGANG R.,ALTER.1-A)

2. Tenggang River

2-2 : (Alternative 1-B)

- (1) Drainage System
- (2) Run-off Model
- (3) Run-off Calculation
- (4) Distribution of Design Discharge

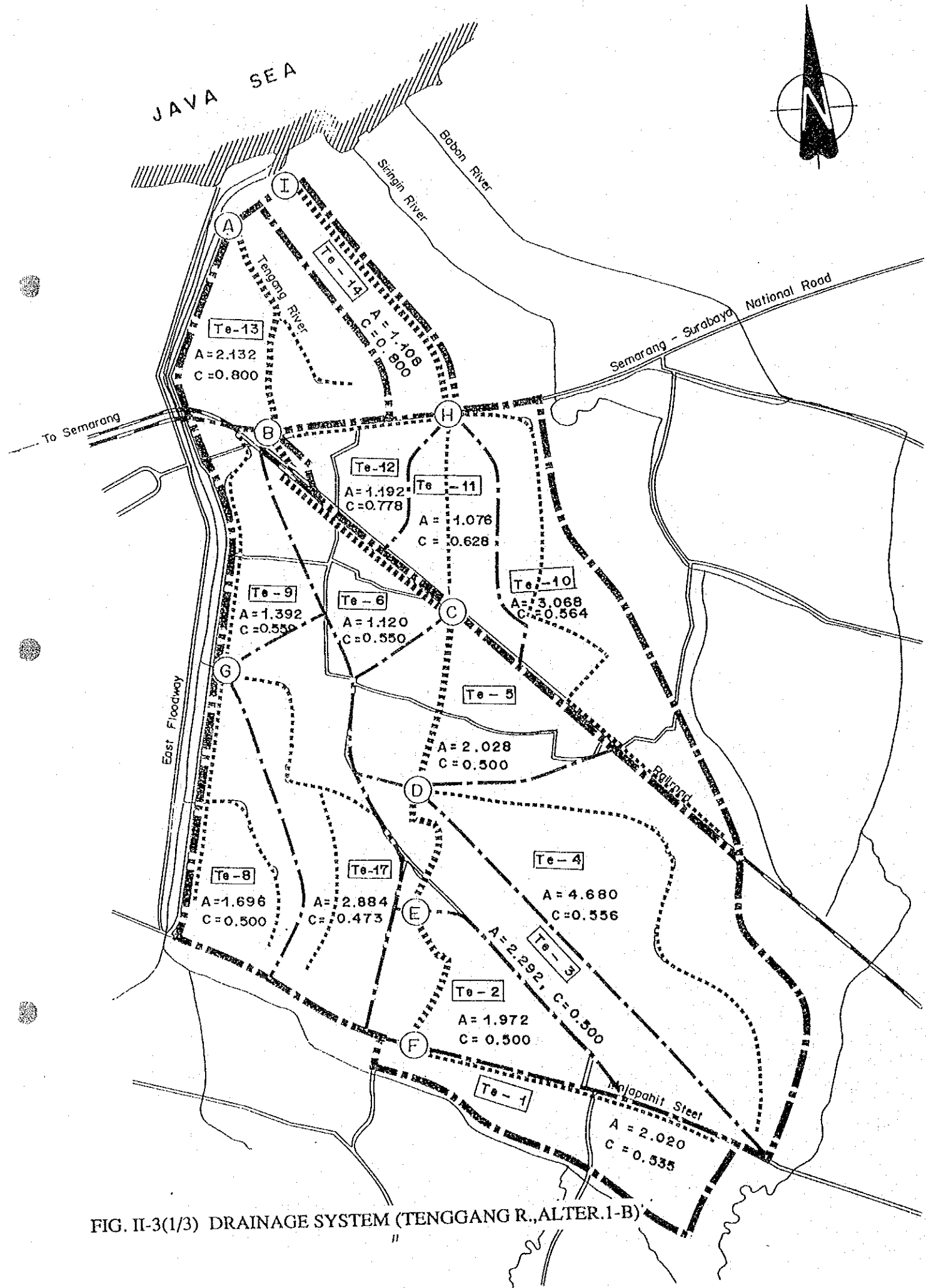
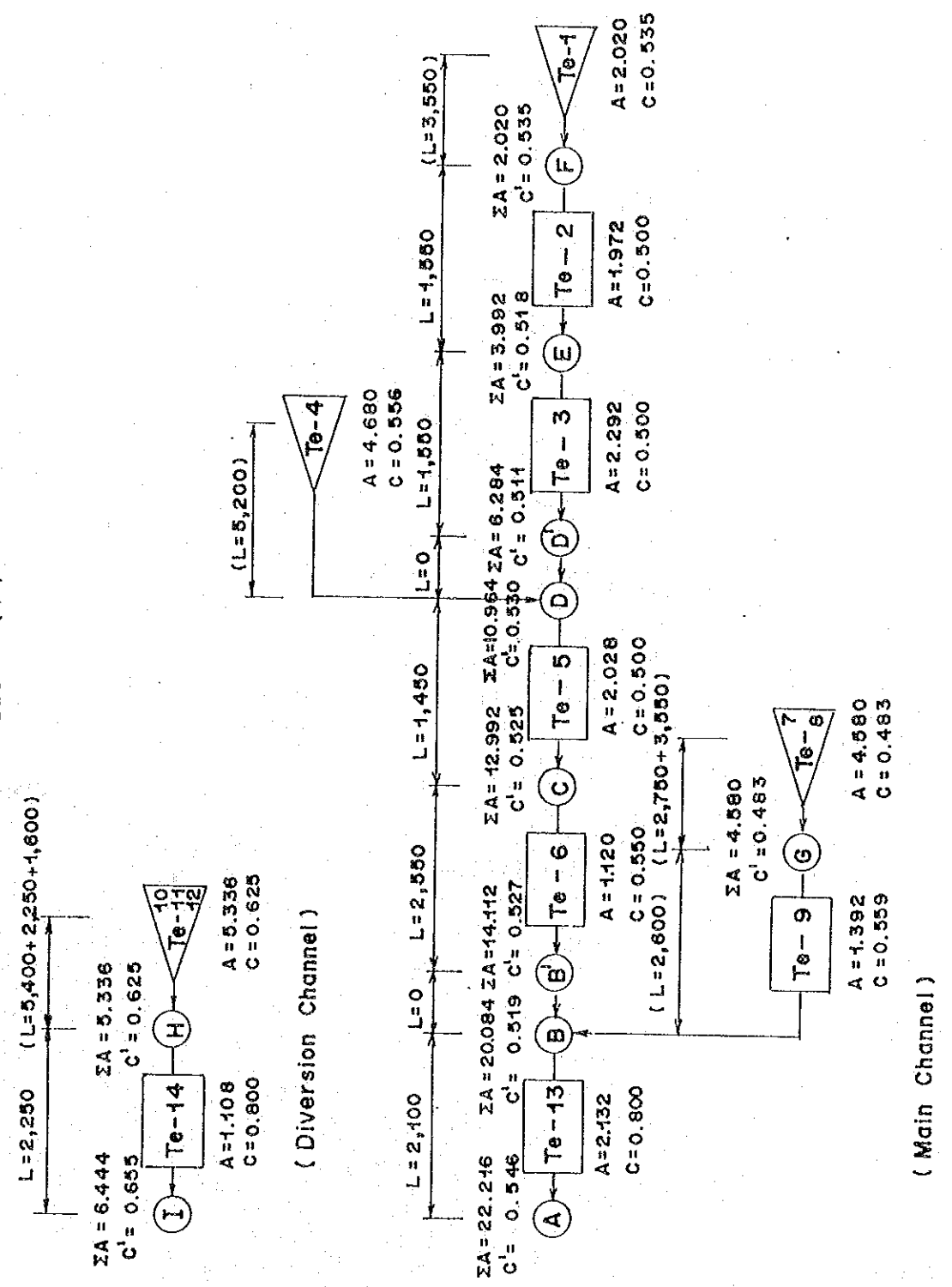


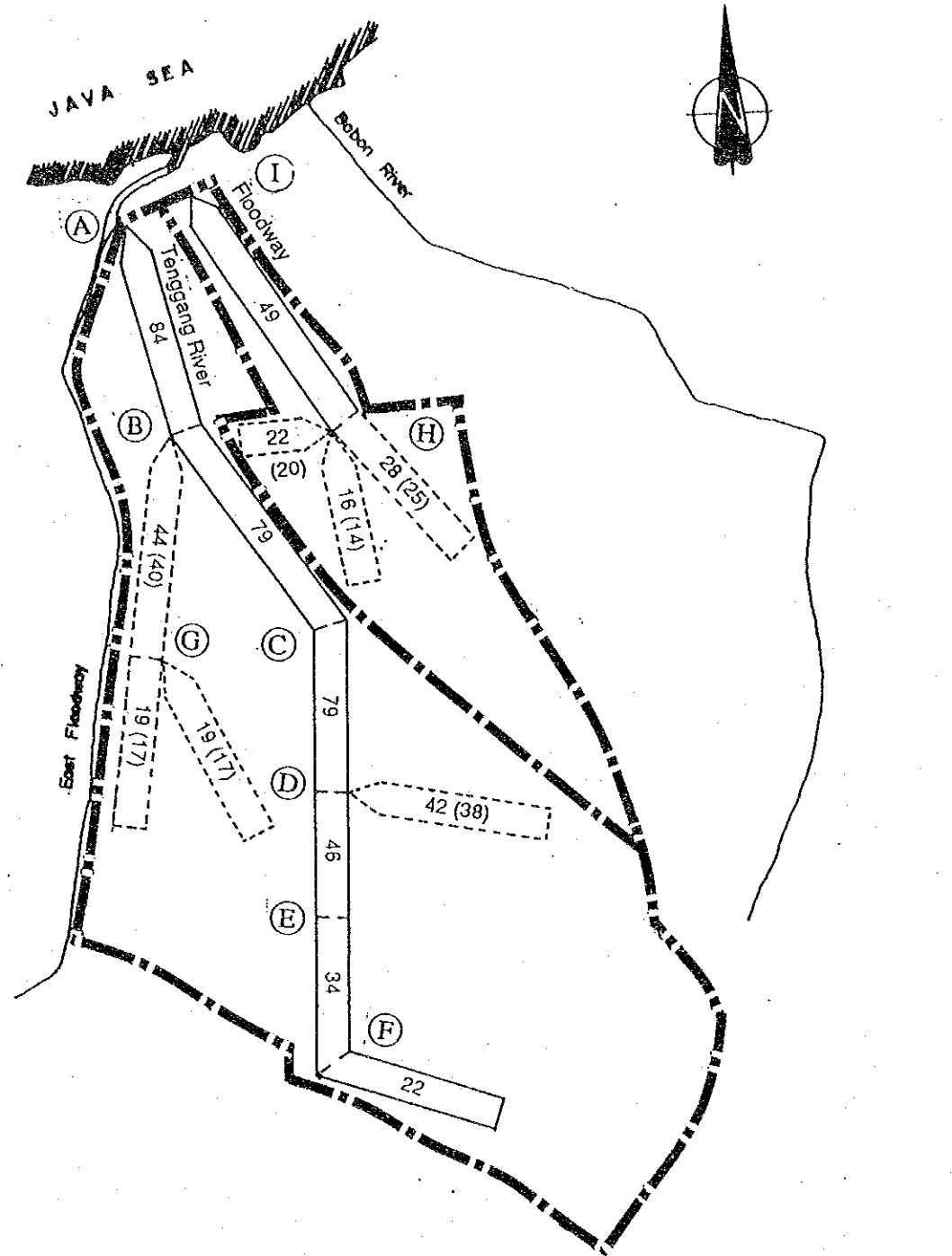
FIG. II-3(1/3) DRAINAGE SYSTEM (TENGGANG R., ALTER. 1-B)

FIG. II-3(2/3) RUNN-OFF MODEL (TENGGANG R., 1-B)









- NOTE : 1. Unit of the figures is m<sup>3</sup>/s.  
 2. The figures with and without parenthesis show the design discharge of a 5-year and 10-year frequency flood respectively.

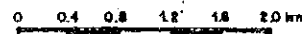


FIG. II-3(3/3) DISTRIBUTION OF DESIGN DISCHARGE(TENGGANG R.,1-B)

2. Tenggang River

2-3 : (Alternative 1-C)

- (1) Drainage System
- (2) Run-off Model
- (3) Run-off Calculation
- (4) Distribution of Design Discharge

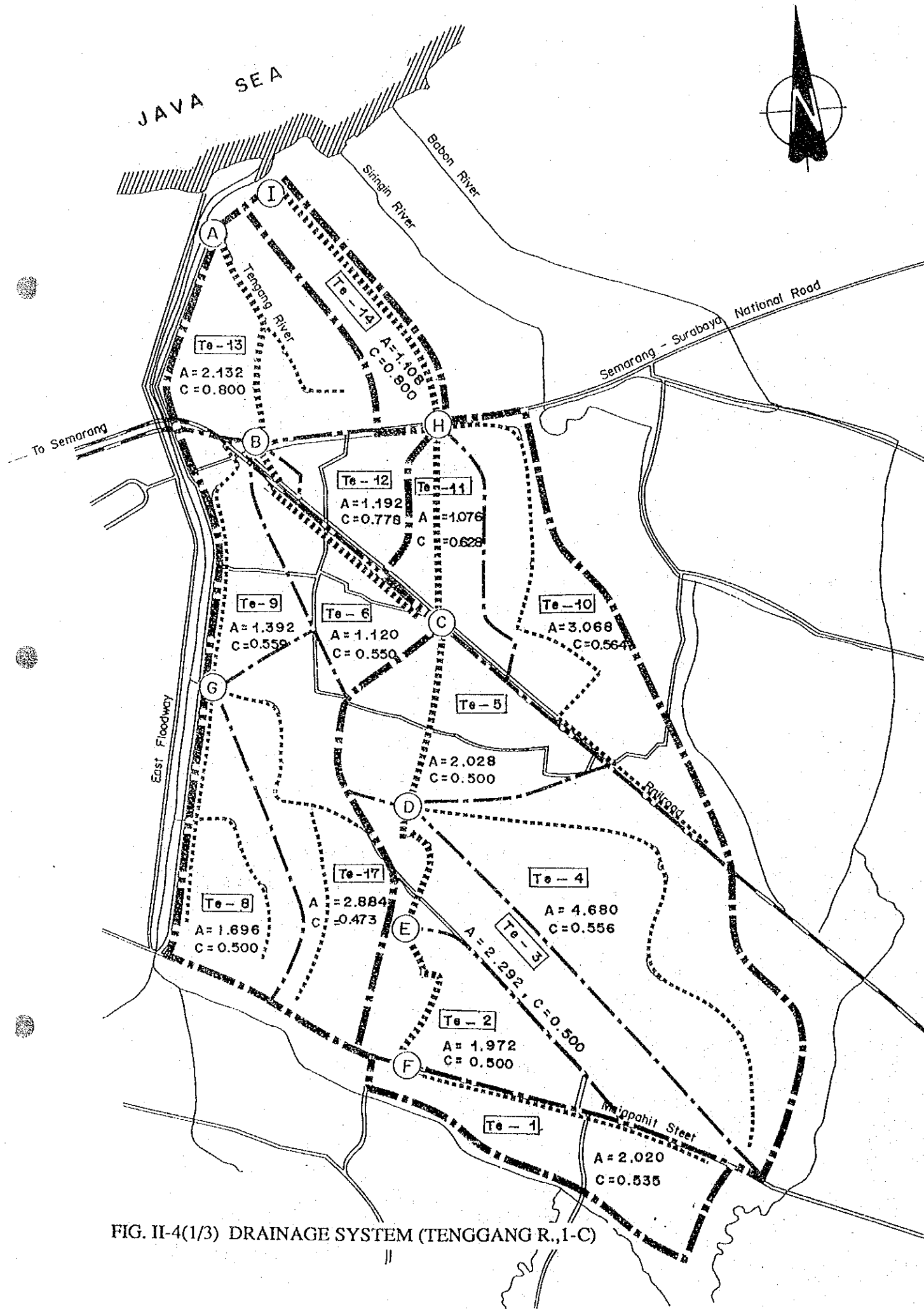


FIG. II-4(1/3) DRAINAGE SYSTEM (TENGANG R., I-C)

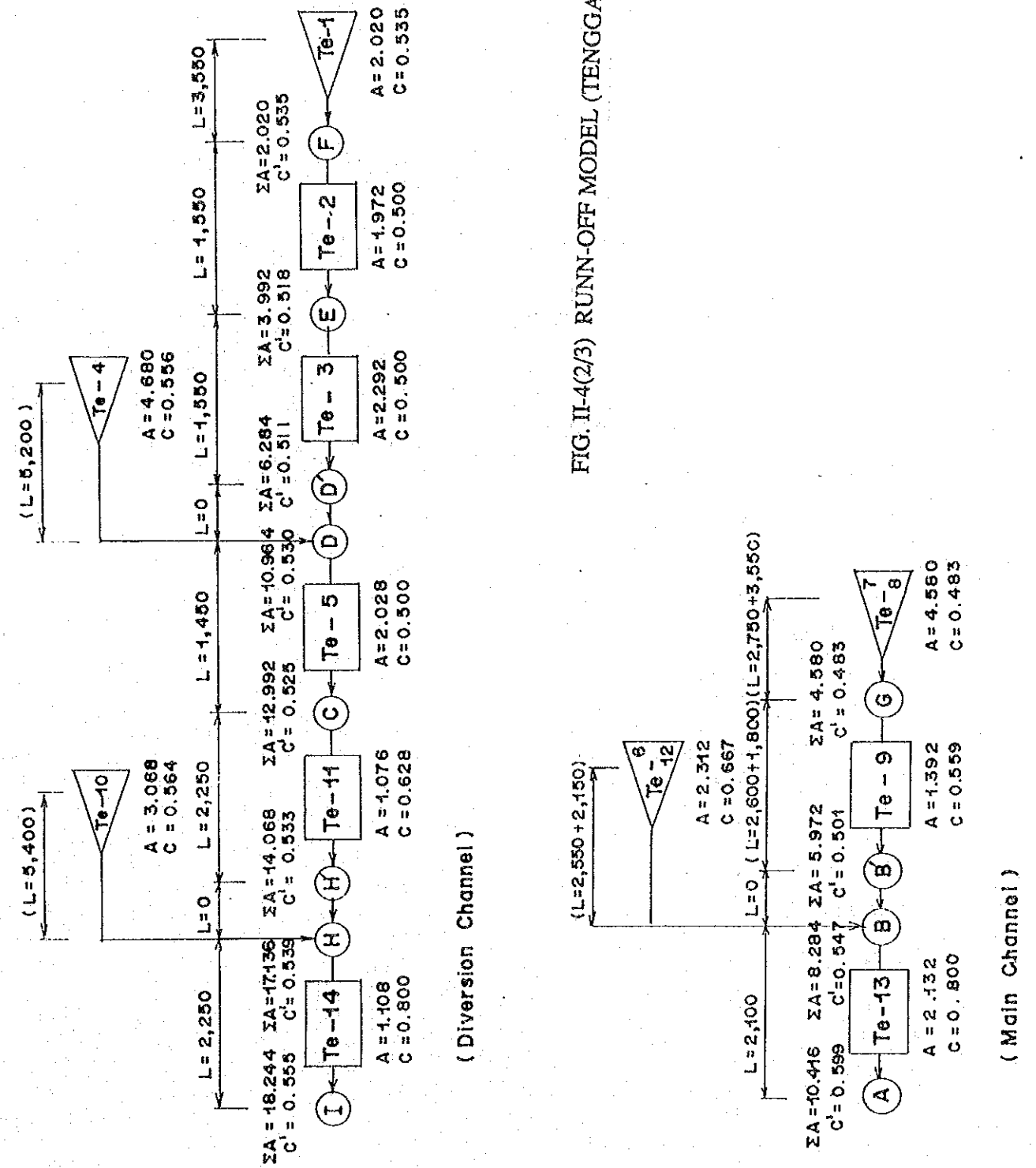
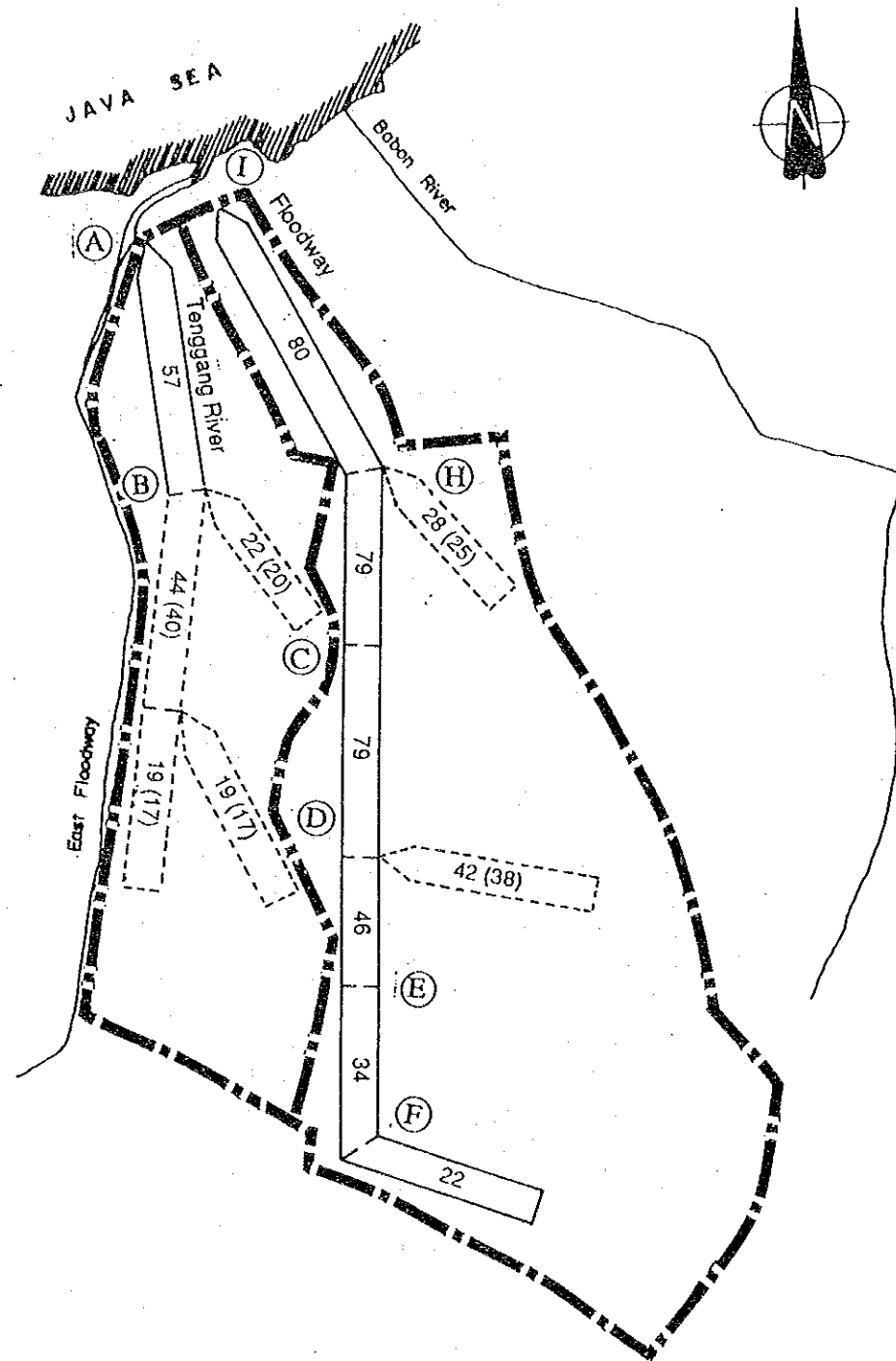


FIG. II-4(2/3) RUNN-OFF MODEL (TENGANG R., I-C)





- NOTE : 1. Unit of the figures is m<sup>3</sup>/s.  
 2. The figures with and without parenthesis show the design discharge of a 5-year and 10-year frequency flood respectively.



FIG. II-4(3/3) DISTRIBUTION OF DESIGN DISCHARGE(TENGGANG R.,1-C)

3. Semarang River

3-1 : (Alternative 2-A)

- (1) Drainage System
- (2) Run-off Model
- (3) Run-off Calculation
- (4) Distribution of Design Discharge

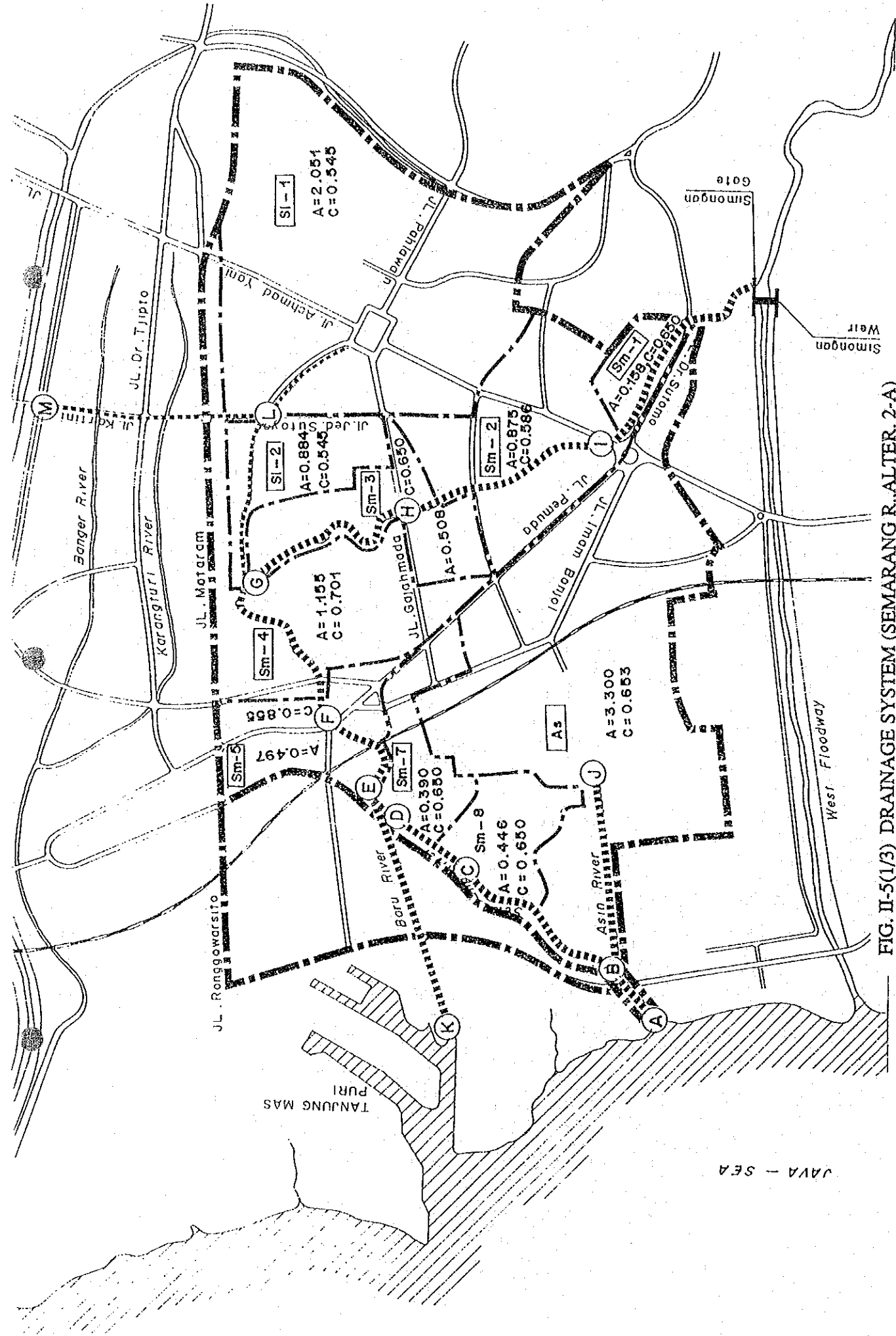


FIG. II-5(1/3) DRAINAGE SYSTEM (SEMARANG R., ALTER. 2-A)

FIG. II-5(2/3) RUNN-OFF MODEL (SEMARANG R., ALTER. 2-A)

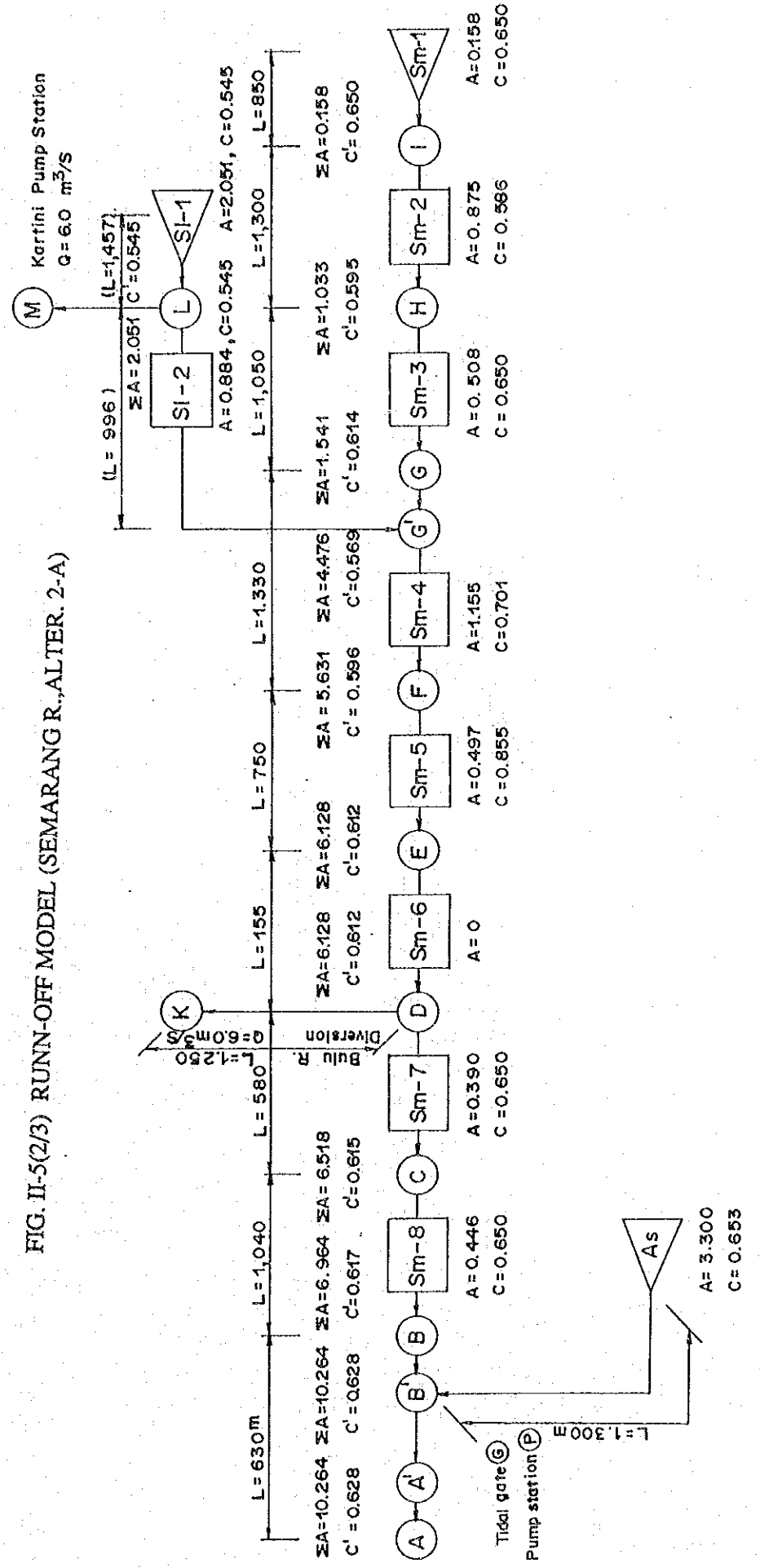
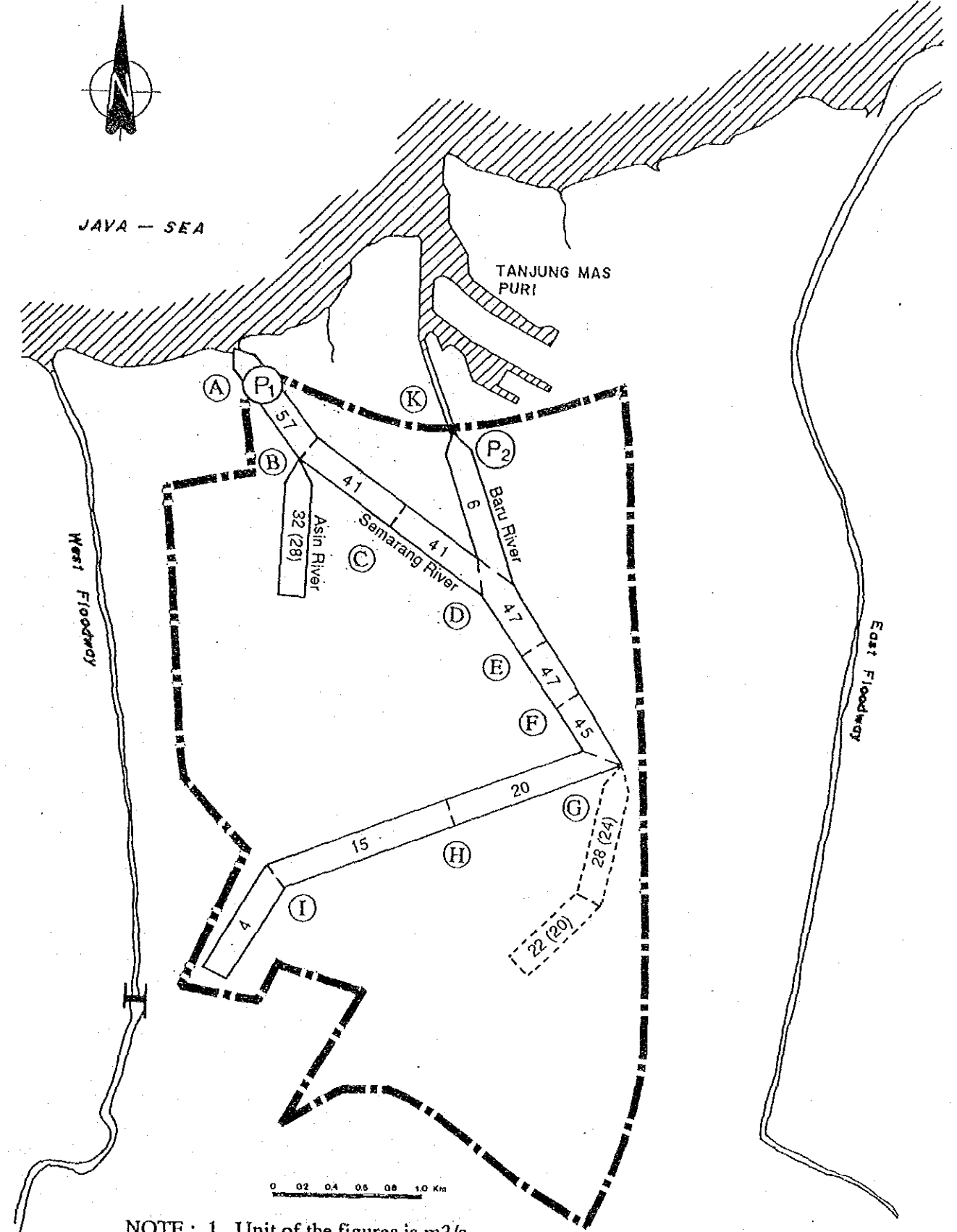


TABLE II-5(1/2) RUN-OFF CALCULATION (5-YEAR FERQUENCY FLOOD)  
(SEMARANG R.,ALTER. 2-A)

B. Central Semarang Area		1. Semarang River Alternative 2-A												
Calculation of Peak Discharge		I	H	G	L	G'	F	E	D	C	B	B'	A'	A
Catchment Area (km <sup>2</sup> )	0.158	1.033	1.541	2.051	2.935	4.476	5.631	6.128	6.128	6.518	6.964	3.300	10.264	10.264
Run-off Coefficient	0.650	0.595	0.614	0.545	0.545	0.569	0.596	0.612	0.612	0.615	0.617	0.653	0.628	0.628
Length of River Canal (m)	850	1.300	1.050	1.457	996	-	1.330	750	155	580	1.040	1.300	630	-
Accumulated Length (m)	850	2.150	3.200	1.457	2.453	3.200	4.530	5.280	5.435	6.015	7.055	1.300	7.055	7.685
Proposed River Slope (%)	0.115%	0.067%	0.067%	S	0.067%	-	0.059%	0.059%	0.020%	0.020%	0.055%	-	0.020%	-
Proposed Velocity (m/s)	1.250	1.000	1.000	1.000	1.000	-	0.750	0.750	0.500	0.500	0.500	0.500	0.500	0.500
Flowing Time (min)	11.33	21.67	17.50	24.28	16.60	-	29.56	16.67	5.17	19.33	34.67	43.33	-	21.00
Duration Time (min)	25.40	44.20	60.90	39.28	55.88	60.90	105.00	118.90	122.20	136.00	160.70	58.33	160.70	174.70
Rainfall Intensity (mm/hr)	130.22	97.17	81.41	103.58	85.41	81.41	59.78	55.62	52.20	48.21	42.58	83.39	42.58	40.00
Peak Discharge (m <sup>3</sup> /s)	2.972	13.273	17.119	25.731	30.351	45.081	44.569	46.358	43.511	42.933	40.658	39.937	60.992	57.304
Specific Q (m <sup>3</sup> /s/km <sup>2</sup> )	18.811	12.849	11.109	12.546	10.345	10.295	7.915	7.365	7.100	6.580	5.838	12.102	5.942	5.583
Diversion Drainage (m <sup>3</sup> /s)	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Peak Discharge (m <sup>3</sup> /s)	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Specific Q (m <sup>3</sup> /s/km <sup>2</sup> )	-	-	-	-	-	-	-	-	-	-	-	-	-	-

TABLE II-5(1/2) RUN-OFF CALCULATION (10-YEAR FERQUENCY FLOOD)  
(SEMARANG R.,ALTER. 2-A)

B. Central Semarang Area		1. Semarang River Alternative 2-A												
Calculation of Peak Discharge		I	H	G	L	G'	F	E	D	C	B	B'	A'	A
Catchment Area (km <sup>2</sup> )	0.158	1.033	1.541	2.051	2.935	4.476	5.631	6.128	6.128	6.518	6.964	3.300	10.264	10.264
Run-off Coefficient	0.650	0.595	0.614	0.545	0.545	0.569	0.596	0.612	0.612	0.615	0.617	0.653	0.628	0.628
Length of River Canal (m)	850	1.300	1.050	1.457	996	-	1.330	750	155	580	1.040	1.300	630	-
Accumulated Length (m)	850	2.150	3.200	1.457	2.453	3.200	4.530	5.280	5.435	6.015	7.055	1.300	7.055	7.685
Proposed River Slope (%)	0.115%	0.067%	0.067%	S	0.067%	-	0.059%	0.059%	0.020%	0.020%	0.055%	-	0.020%	-
Proposed Velocity (m/s)	1.250	1.000	1.000	1.000	1.000	-	0.750	0.750	0.500	0.500	0.500	0.500	0.500	0.500
Flowing Time (min)	11.33	21.67	17.50	24.28	16.60	-	29.56	16.67	5.17	19.33	34.67	43.33	-	21.00
Duration Time (min)	25.40	44.20	60.90	39.28	55.88	60.90	105.00	118.90	122.20	136.00	160.70	58.33	160.70	174.70
Rainfall Intensity (mm/hr)	143.76	107.56	90.48	114.53	94.80	80.48	67.07	62.60	58.57	54.12	47.80	92.62	47.80	44.91
Peak Discharge (m <sup>3</sup> /s)	3.281	14.692	19.026	28.481	33.702	51.214	50.025	52.172	48.815	48.211	45.646	44.358	56.475	64.323
Specific Q (m <sup>3</sup> /s/km <sup>2</sup> )	20.787	14.223	12.347	13.872	11.483	11.442	8.884	8.514	7.986	7.397	6.555	13.442	6.671	6.267
Diversion Drainage (m <sup>3</sup> /s)	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Peak Discharge (m <sup>3</sup> /s)	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Specific Q (m <sup>3</sup> /s/km <sup>2</sup> )	-	-	-	-	-	-	-	-	-	-	-	-	-	-



NOTE : 1. Unit of the figures is m<sup>3</sup>/s.  
2. The figures with and without parenthesis show the design discharge of a 5-year and 10-year frequency flood respectively.

FIG. II-5(3/3) DISTRIBUTION OF DESIGN DISCHARGE (SEMARANG R.,ALTER. 2-A)  
VI - 29



3. Semarang River

3-2 : (Alternative 2-B)

- (1) Drainage System
- (2) Run-off Model
- (3) Run-off Calculation
- (4) Distribution of Design Discharge

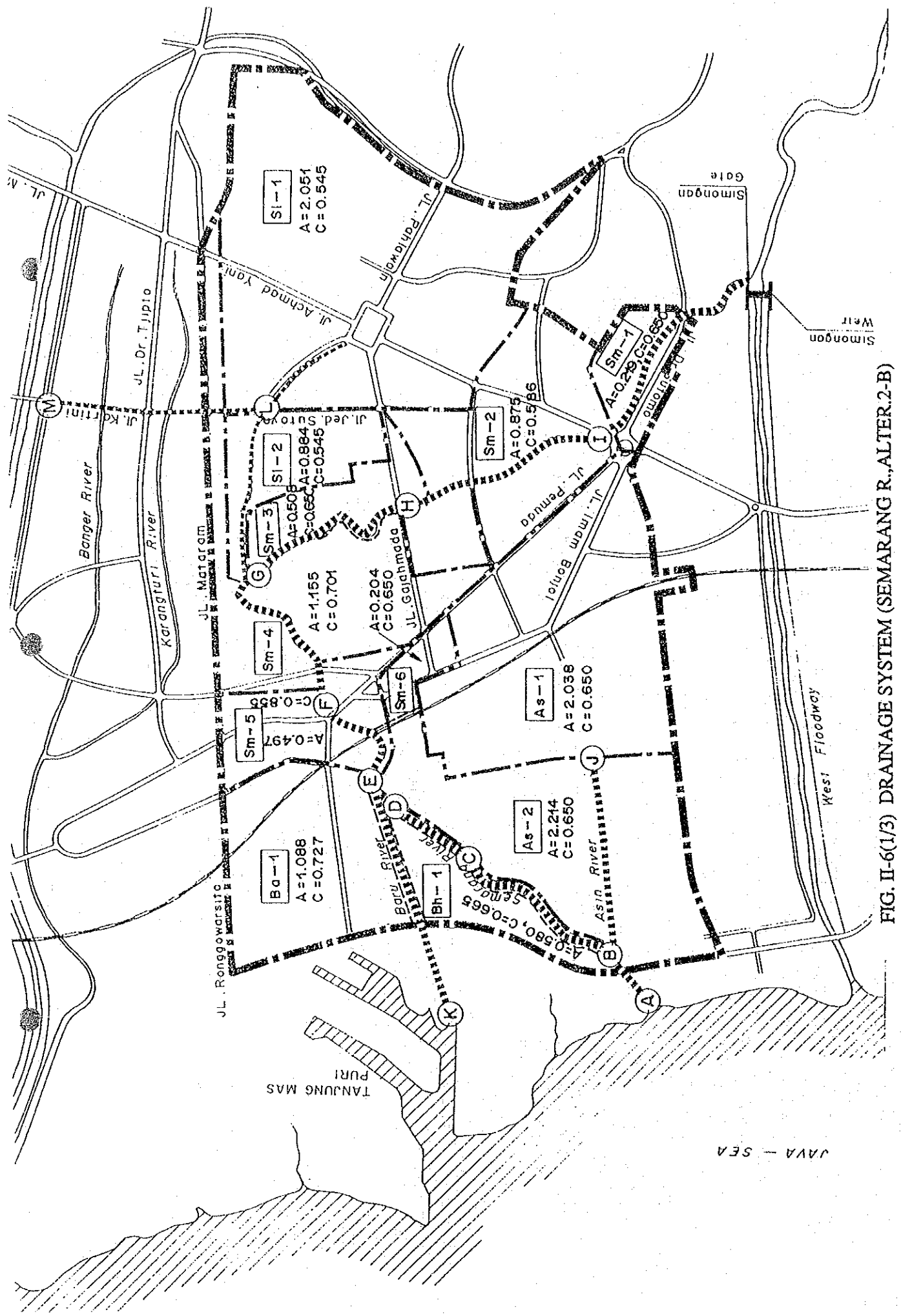


FIG. II-6(1/3) DRAINAGE SYSTEM (SEMARANG R., ALTER. 2-B)

FIG. II-6(2/3) RUNN-OFF MODEL (SEMARANG R., ALTER. 2-B)

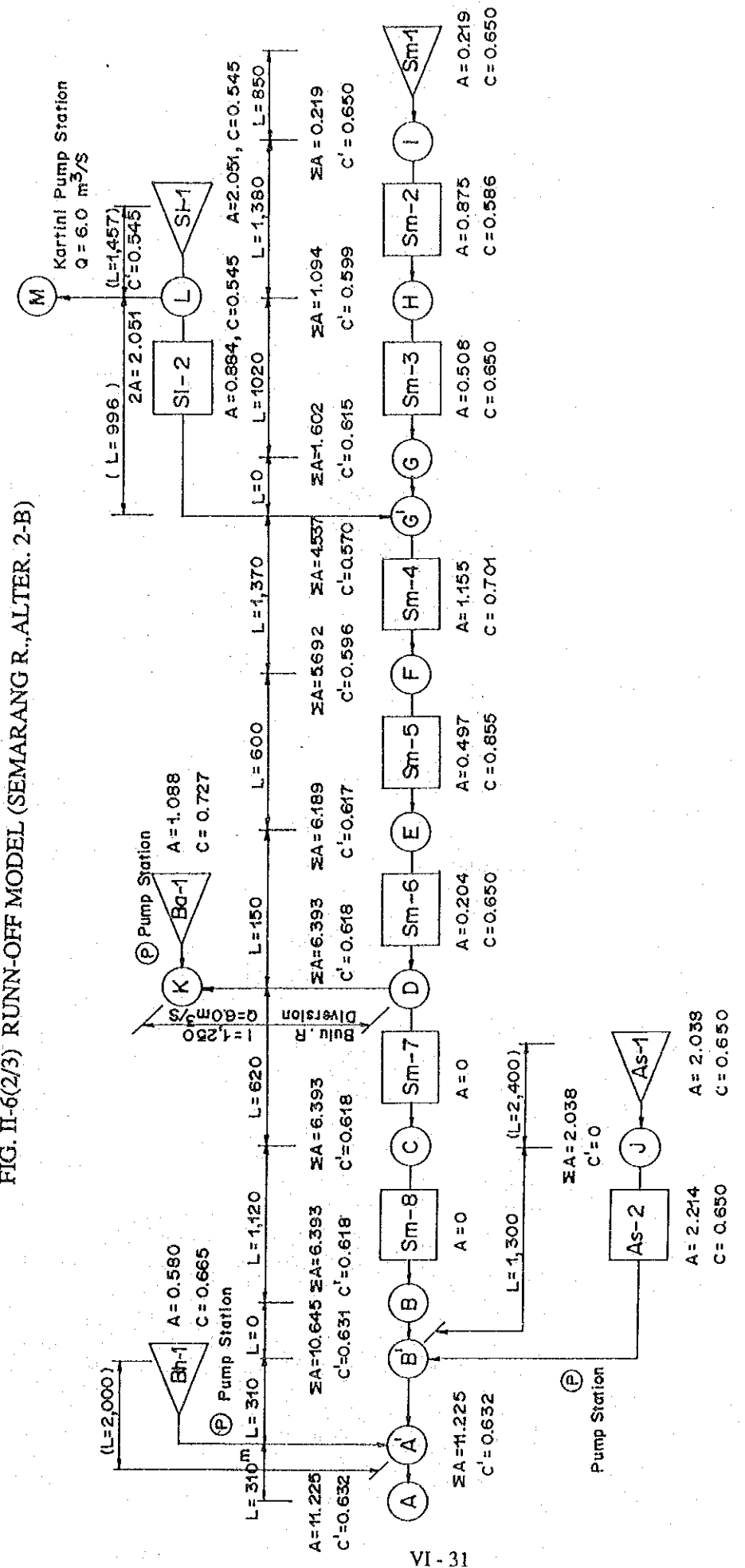
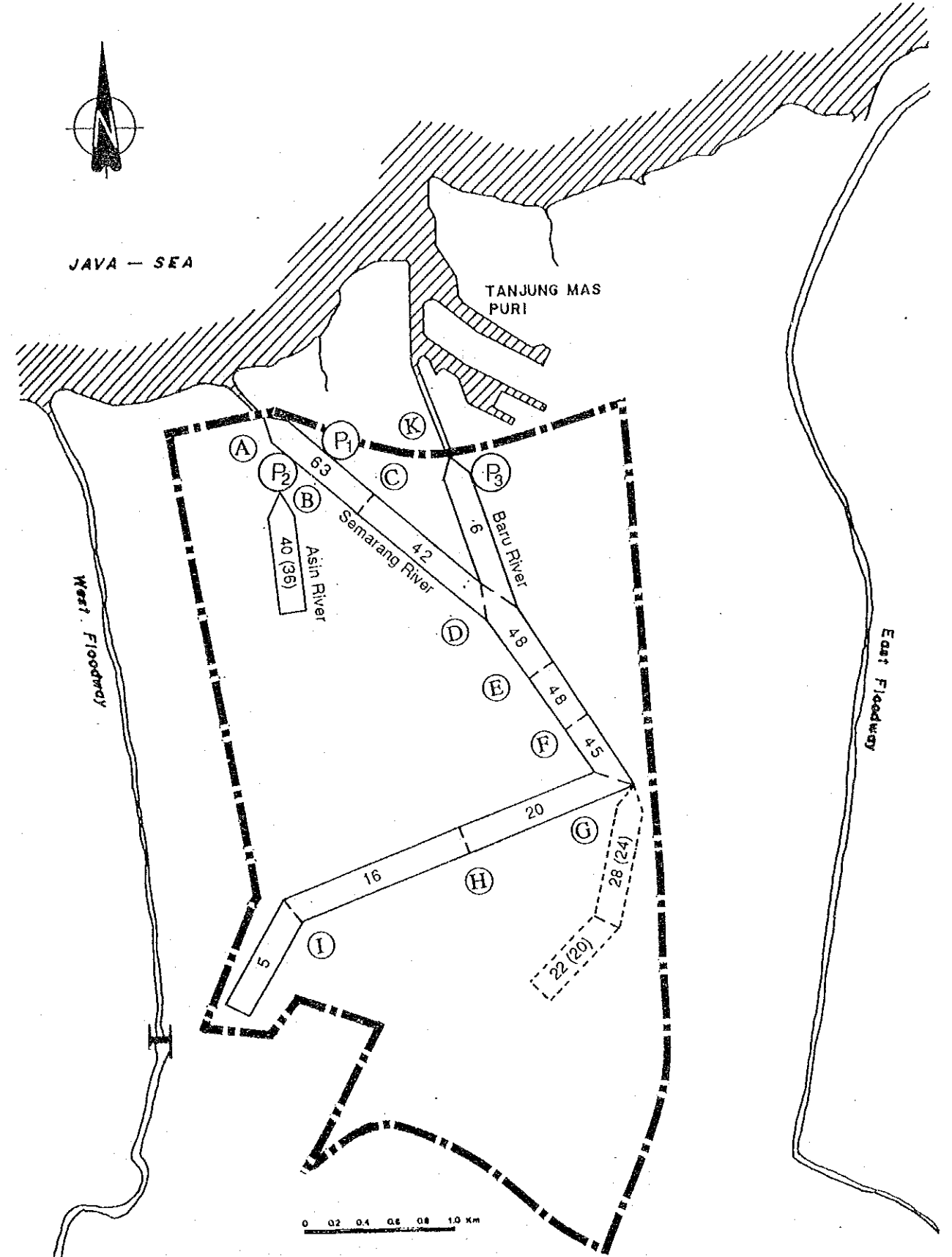


TABLE II-6(1/2) RUN-OFF CALCULATION (5-YEAR FREQUENCY FLOOD)  
(SEMARANG R., ALTER. 2-B)

B. Central Semarang Area		1. Semarang River														
Calculation of Peak Discharge		Alternative 2-B														
Calculation Point	I	H	G	L	G'	F	E	D	C	B	J	B''	B'	A''	A'	A
Catchment Area (km <sup>2</sup> )	0.219	1.094	1.602	2.051	2.935	4.537	5.692	6.189	6.393	6.393	2.038	4.252	10.645	0.580	11.225	11.225
Run-off Coefficient	0.650	0.589	0.615	0.545	0.545	0.570	0.586	0.617	0.618	0.618	0.650	0.650	0.650	0.655	0.632	0.632
Length of River Canal (m)	850	1.380	1.020	1.457	996	-	1.370	600	150	620	2.400	1.300	-	2.000	310	310
Accumulated Length (m)	850	2.230	3.250	1.457	2.453	3.250	4.620	5.220	5.370	5.990	2.400	3.700	7.110	2.000	7.420	7.730
Proposed River Slope (%)	S	0.067%	0.067%	S	0.067%	-	0.059%	0.059%	0.020%	0.020%	S	0.055%	-	S	0.020%	0.020%
Proposed Velocity (m/s)	1.250	1.000	1.000	1.000	1.000	-	0.750	0.750	0.500	0.500	0.500	0.500	0.500	0.500	0.500	0.500
Floving Time (min)	11.33	23.00	17.00	24.28	16.60	-	30.44	13.33	5.00	20.57	37.33	80.00	43.33	-	66.67	10.33
Duration Time (min)	25.40	44.00	60.90	39.28	55.88	60.90	105.00	118.90	122.20	136.00	85.00	138.33	160.70	81.67	164.37	174.70
Rainfall Intensity (mm/hr)	130.22	97.41	81.41	103.58	85.41	81.41	59.76	56.62	52.20	48.21	47.61	42.58	42.58	69.00	41.87	40.00
Peak Discharge (m <sup>3</sup> /s)	4.120	14.185	17.826	25.731	30.361	46.791	45.051	47.202	45.838	42.335	18.633	29.242	63.559	5.914	66.006	63.068
Specific Q (m <sup>3</sup> /s/km <sup>2</sup> )	18.811	12.968	11.127	12.546	10.345	10.313	7.915	7.627	7.170	6.822	8.848	9.143	6.877	5.971	10.197	5.880
Diversion Drainage (m <sup>3</sup> /s)	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Peak Discharge (m <sup>3</sup> /s)	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Specific Q (m <sup>3</sup> /s/km <sup>2</sup> )	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

TABLE II-6(1/2) RUN-OFF CALCULATION (10-YEAR FREQUENCY FLOOD)  
(SEMARANG R., ALTER. 2-B)

B. Central Semarang Area		1. Semarang River														
Calculation of Peak Discharge		Alternative 2-B														
Calculation Point	I	H	G	L	G'	F	E	D	C	B	J	B''	B'	A''	A'	A
Catchment Area (km <sup>2</sup> )	0.219	1.094	1.602	2.051	2.935	4.537	5.692	6.189	6.393	6.393	2.038	4.252	10.645	0.580	11.225	11.225
Run-off Coefficient	0.650	0.589	0.615	0.545	0.545	0.570	0.586	0.617	0.618	0.618	0.650	0.650	0.650	0.655	0.632	0.632
Length of River Canal (m)	850	1.380	1.020	1.457	996	-	1.370	600	150	620	2.400	1.300	-	2.000	310	310
Accumulated Length (m)	850	2.230	3.250	1.457	2.453	3.250	4.620	5.220	5.370	5.990	2.400	3.700	7.110	2.000	7.420	7.730
Proposed River Slope (%)	S	0.067%	0.067%	S	0.067%	-	0.059%	0.059%	0.020%	0.020%	S	0.055%	-	S	0.020%	0.020%
Proposed Velocity (m/s)	1.250	1.000	1.000	1.000	1.000	-	0.750	0.750	0.500	0.500	0.500	0.500	0.500	0.500	0.500	0.500
Floving Time (min)	11.33	23.00	17.00	24.28	16.60	-	30.44	13.33	5.00	20.57	37.33	80.00	43.33	-	66.67	10.33
Duration Time (min)	25.40	44.00	60.90	39.28	55.88	60.90	105.00	118.90	122.20	136.00	85.00	138.33	160.70	81.67	164.37	174.70
Rainfall Intensity (mm/hr)	143.76	107.82	90.48	114.53	94.80	90.48	67.07	62.60	58.57	54.12	47.80	70.89	53.44	47.80	77.06	44.91
Peak Discharge (m <sup>3</sup> /s)	4.548	15.702	19.812	28.451	33.702	52.003	50.567	53.122	51.426	47.517	41.971	20.871	32.824	71.356	6.606	74.103
Specific Q (m <sup>3</sup> /s/km <sup>2</sup> )	20.767	14.353	12.367	13.872	11.483	11.462	8.884	8.583	8.044	7.433	6.565	10.241	7.720	6.703	11.389	6.602
Diversion Drainage (m <sup>3</sup> /s)	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Peak Discharge (m <sup>3</sup> /s)	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Specific Q (m <sup>3</sup> /s/km <sup>2</sup> )	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



NOTE : 1. Unit of the figures is m<sup>3</sup>/s.  
2. The figures with and without parenthesis show the design discharge of a 5-year and 10-year frequency flood respectively.

FIG. II-6(3/3) DISTRIBUTION OF DESIGN DISCHARGE (SEMARANG R., ALTER. 2-B)

4. Banger River

4-1 : (Alternative 3-A)

- (1) Drainage System
- (2) Run-off Model
- (3) Run-off Calculation
- (4) Distribution of Design Discharge

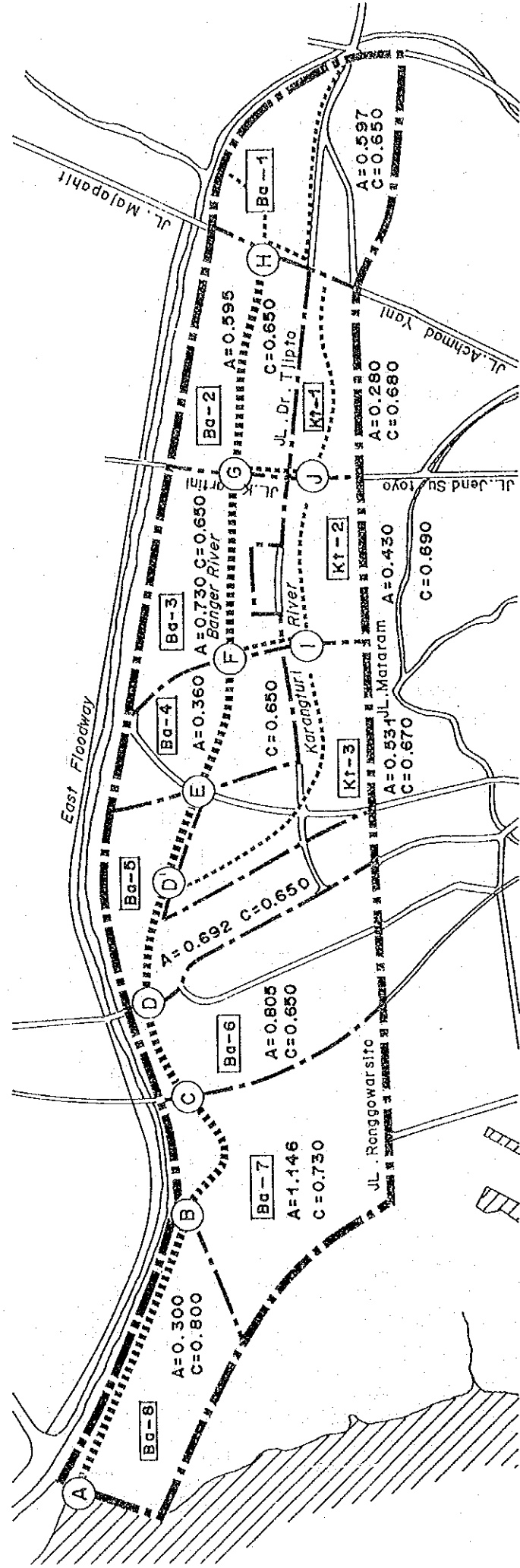


FIG. II-7(1/3) DRAINAGE SYSTEM (BANGER R., ALTER.3-A)

FIG. II-7(2/3) RUNN-OFF MODEL (BANGER R., ALTER.3-A)

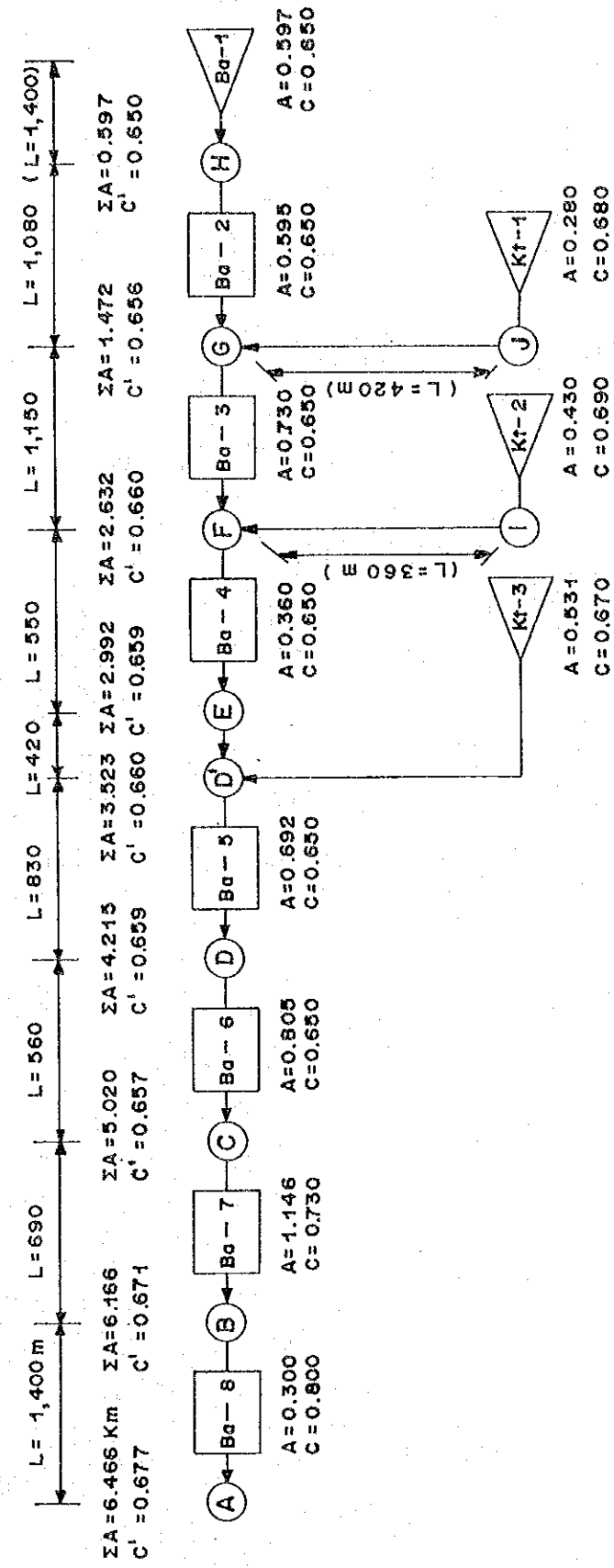


TABLE II-7(1/2) RUN-OFF CALCULATION (5-YEAR FERQUENCY FLOOD)  
(BANGER R.,ALTER.3-A)

B. Central Semarang Area		2. Banger River												
Calculation of Peak Discharge		Alternative 3-A												
Calculation Point	H	G'	J	G	F'	I	F	E	D''	D'	D	C	B	A
Catchment Area(km <sup>2</sup> )	0.597	1.192	0.280	1.472	2.202	0.430	2.632	2.992	0.531	3.523	4.215	5.020	6.166	6.466
Run-off Coefficient	0.650	0.650	0.660	0.656	0.654	0.690	0.660	0.659	0.670	0.660	0.659	0.657	0.645	0.652
Length of River Cannal(m)	1.400	1.080	1.000	-	1.160	1.030	-	550	1.550	420	830	560	690	1.100
Accumulated Length (m)	1.400	2.480	1.000	2.480	3.630	1.030	3.630	4.180	1.550	4.600	5.430	5.990	6.680	8.080
Proposed River Slope (%)	S	0.063%	S	-	0.050%	S	-	0.050%	S	0.050%	0.027%	0.027%	0.027%	0.018%
Proposed Velocity (m/s)	1.000	1.000	1.000	-	0.750	0.750	-	0.750	0.750	0.750	0.600	0.600	0.600	0.500
Floving Time (min)	23.33	18.00	16.67	-	25.56	22.89	-	12.22	34.44	9.33	23.06	15.56	19.17	46.67
Duration Time (min)	38.33	52.10	31.67	52.10	69.20	37.89	69.20	77.20	49.44	86.53	100.00	124.50	137.30	178.10
Rainfall Intensity(mm/hr)	104.95	88.79	116.17	88.79	75.78	105.61	75.78	71.24	91.39	66.76	61.45	51.49	47.87	39.43
Peak Discharge(m <sup>3</sup> /s)	9.051	15.289	4.771	19.054	24.254	6.964	29.255	31.215	7.226	34.497	37.937	37.738	42.285	36.929
Specific Q (m <sup>3</sup> /s/km <sup>2</sup> )	15.161	12.826	17.039	12.944	11.014	16.195	11.115	10.433	13.607	9.792	9.000	7.518	6.358	5.711
Pumping Drainage (m <sup>3</sup> /s)														
Peak Discharge(m <sup>3</sup> /s)														
Specific Q (m <sup>3</sup> /s/km <sup>2</sup> )														

TABLE II-7(1/2) RUN-OFF CALCULATION (10-YEAR FERQUENCY FLOOD)  
(BANGER R.,ALTER.3-A)

B. Central Semarang Area		2. Banger River												
Calculation of Peak Discharge		Alternative 3-A												
Calculation Point	H	G'	J	G	F'	I	F	E	D''	D'	D	C	B	A
Catchment Area(km <sup>2</sup> )	0.597	1.192	0.280	1.472	2.202	0.430	2.632	2.992	0.531	3.523	4.215	5.020	6.166	6.466
Run-off Coefficient	0.650	0.650	0.660	0.656	0.654	0.690	0.660	0.659	0.670	0.660	0.659	0.657	0.645	0.652
Length of River Cannal(m)	1.400	1.080	1.000	-	1.150	1.030	-	550	1.550	420	830	560	690	1.400
Accumulated Length (m)	1.400	2.480	1.000	2.480	3.630	1.030	3.630	4.180	1.550	4.600	5.430	5.990	6.680	8.080
Proposed River Slope (%)	S	0.063%	S	-	0.050%	S	-	0.050%	S	0.050%	0.027%	0.027%	0.027%	0.018%
Proposed Velocity (m/s)	1.000	1.000	1.000	-	0.750	0.750	-	0.750	0.750	0.750	0.600	0.600	0.600	0.500
Floving Time (min)	23.33	18.00	16.67	-	25.56	22.89	-	12.22	34.44	9.33	23.06	15.56	19.17	46.67
Duration Time (min)	38.33	52.10	31.67	52.10	69.20	37.89	69.20	77.20	49.44	86.53	100.00	124.50	137.30	178.10
Rainfall Intensity(mm/hr)	115.03	98.47	128.29	98.47	84.39	116.74	84.39	79.48	101.28	74.64	68.91	57.77	53.74	44.26
Peak Discharge(m <sup>3</sup> /s)	10.006	16.955	5.269	21.131	27.010	7.698	32.580	34.828	8.008	38.572	42.538	42.344	47.463	41.455
Specific Q (m <sup>3</sup> /s/km <sup>2</sup> )	16.761	14.224	18.817	14.355	12.266	17.902	12.378	11.640	15.081	10.949	10.982	8.435	7.688	6.411
Pumping Drainage (m <sup>3</sup> /s)														
Peak Discharge(m <sup>3</sup> /s)														
Specific Q (m <sup>3</sup> /s/km <sup>2</sup> )														

- NOTE : 1. Unit of the figures is m<sup>3</sup>/s.  
2. The figures with and without parenthesis show the design discharge of a 5-year and 10-year frequency flood respectively.

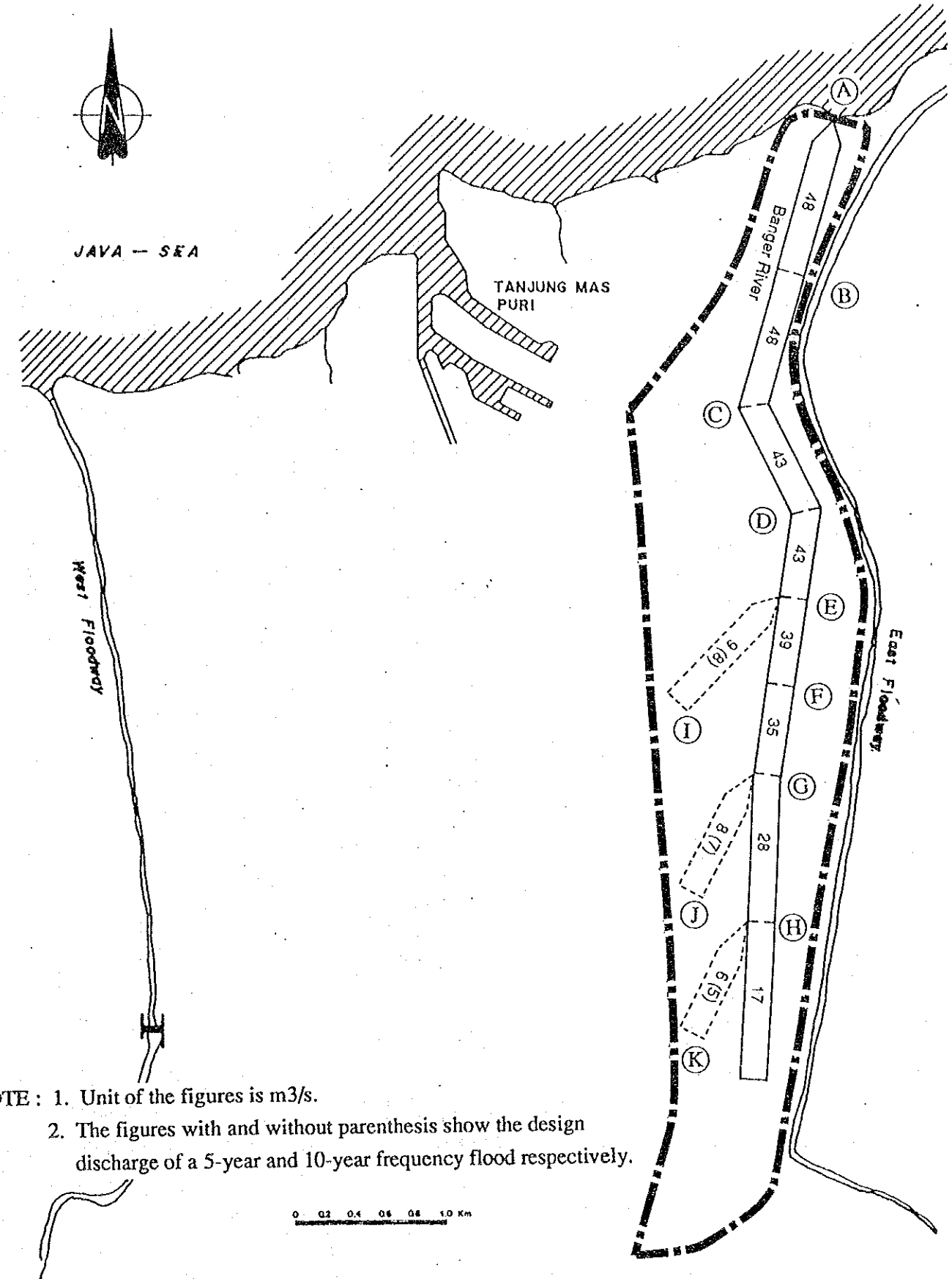


FIG. II-7(3/3) DISTRIBUTION OF DESIGN DISCHARGE(BANGER R.,ALTER3-A)

4. Banger River

4-2 : (Alternative 3-B)

- (1) Drainage System
- (2) Run-off Model
- (3) Run-off Calculation
- (4) Distribution of Design Discharge

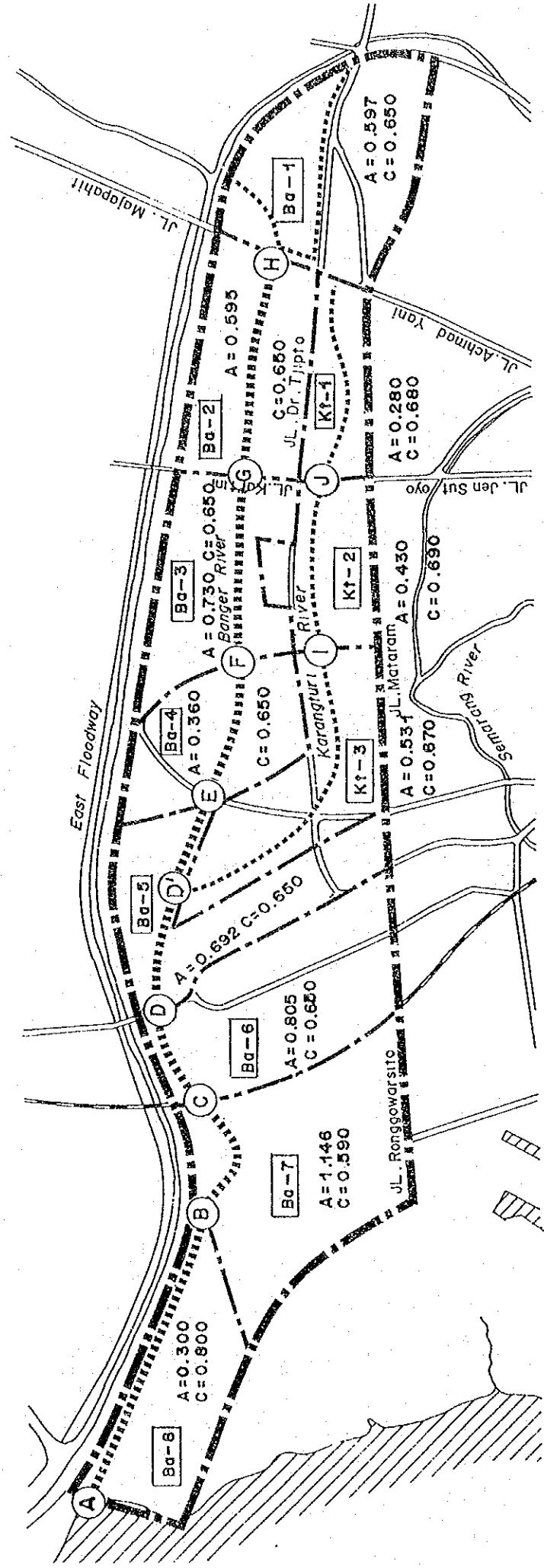


FIG. II-8(1/3) DRAINAGE SYSTEM (BANGER R., ALTER.3-B)

LEGEND

- Ba-1 = Sub-drainage Area
- Ba-2 = River Channel
- (A) = Calculation Point
- A = Drainage Area ( Km<sup>2</sup> )
- ΣA = Accumulated Drainage Area
- C = Run off Coefficient
- L = Length of River Channel ( m )

FIG. II-8(2/3) RUNN-OFF MODEL (BANGER R., ALTER.3-B)

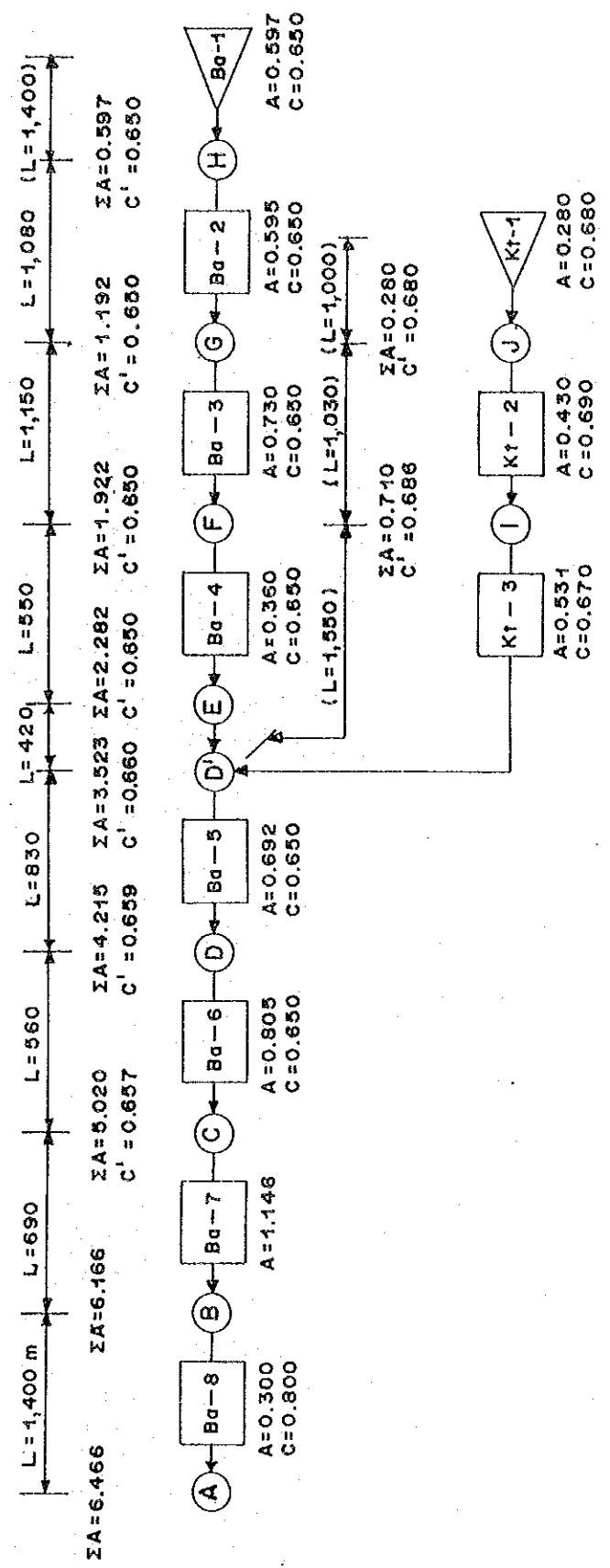




TABLE II-8(1/2) RUN-OFF CALCULATION (5-YEAR FERQUENCY FLOOD)  
(BANGER R.,ALTER.3-B)

B. Central Semarang Area		2. Banger River Alternative 3-B												
Calculation of Peak Discharge		H	G	F	E	J	I	D'	D	C	B	A		
Calculation Point		0.597	1.192	1.922	2.282	0.280	0.710	1.241	3.523	4.215	5.020	6.166	6.466	
Catchment Area(km <sup>2</sup> )		0.597	1.192	1.922	2.282	0.280	0.710	1.241	3.523	4.215	5.020	6.166	6.466	
Run-off Coefficient		0.650	0.650	0.650	0.650	0.686	0.686	0.679	0.660	0.659	0.657	0.645	0.652	
Length of River Cannal(m)		1.400	1.080	1.150	550	1.000	1.030	1.550	420	830	560	690	1.400	
Accumulated Length (m)		1.400	2.480	3.630	4.180	1.000	2.030	3.580	4.600	5.430	5.990	6.680	8.080	
Proposed River Slope (%)	S	0.083%	0.050%	0.050%	0.050%	S	S	0.050%	0.027%	0.027%	0.027%	0.027%	0.018%	
Proposed Velocity (m/s)		1.000	1.000	0.750	0.750	1.000	0.750	0.750	0.750	0.600	0.600	0.600	0.500	
Flowing Time (min)		23.33	18.00	25.56	12.22	16.67	22.89	34.44	9.33	23.06	15.56	19.17	46.67	
Duration Time (min)		38.33	52.10	69.20	77.20	31.67	54.56	89.00	86.53	100.00	124.50	137.70	178.10	
Rainfall Intensity(mm/hr)		104.95	88.79	75.78	71.24	116.17	86.55	55.70	56.76	61.45	51.49	47.77	39.43	
Peak Discharge(m <sup>3</sup> /s)		9.051	15.289	21.040	23.483	4.916	9.369	12.306	34.497	37.937	37.739	42.194	36.929	
Specific Q (m <sup>3</sup> /s/km <sup>2</sup> )		15.161	12.826	10.947	10.280	17.556	13.196	9.916	9.792	9.000	7.518	6.843	5.711	
Pumping Drainage (m <sup>3</sup> /s)														
Peak Discharge(m <sup>3</sup> /s)														
Specific Q (m <sup>3</sup> /s/km <sup>2</sup> )														

TABLE II-8(1/2) RUN-OFF CALCULATION (10-YEAR FERQUENCY FLOOD)  
(BANGER R.,ALTER.3-B)

B. Central Semarang Area		2. Banger River Alternative 3-B												
Calculation of Peak Discharge		H	G	F	E	J	I	D'	D	C	B	A		
Calculation Point		0.597	1.192	1.922	2.282	0.280	0.710	1.241	3.523	4.215	5.020	6.166	6.466	
Catchment Area(km <sup>2</sup> )		0.597	1.192	1.922	2.282	0.280	0.710	1.241	3.523	4.215	5.020	6.166	6.466	
Run-off Coefficient		0.650	0.650	0.650	0.650	0.686	0.686	0.679	0.660	0.659	0.657	0.645	0.652	
Length of River Cannal(m)		1.400	1.080	1.150	550	1.000	1.030	1.550	420	830	560	690	1.400	
Accumulated Length (m)		1.400	2.480	3.630	4.180	1.000	2.030	3.580	4.600	5.430	5.990	6.680	8.080	
Proposed River Slope (%)	S	0.083%	0.050%	0.050%	0.050%	S	S	0.050%	0.027%	0.027%	0.027%	0.027%	0.018%	
Proposed Velocity (m/s)		1.000	1.000	0.750	0.750	1.000	0.750	0.750	0.750	0.600	0.600	0.600	0.500	
Flowing Time (min)		23.33	18.00	25.56	12.22	16.67	22.89	34.44	9.33	23.06	15.56	19.17	46.67	
Duration Time (min)		38.33	52.10	69.20	77.20	31.67	54.56	89.00	86.53	100.00	124.50	137.70	178.10	
Rainfall Intensity(mm/hr)		116.03	98.47	84.39	79.48	128.29	96.05	73.49	74.64	68.91	57.77	53.62	44.26	
Peak Discharge(m <sup>3</sup> /s)		10.006	16.935	23.431	26.201	5.428	10.396	13.766	38.572	42.539	42.344	47.361	41.455	
Specific Q (m <sup>3</sup> /s/km <sup>2</sup> )		16.761	14.224	12.191	11.482	19.387	14.643	11.093	10.949	10.092	8.435	7.681	6.411	
Pumping Drainage (m <sup>3</sup> /s)														
Peak Discharge(m <sup>3</sup> /s)														
Specific Q (m <sup>3</sup> /s/km <sup>2</sup> )														

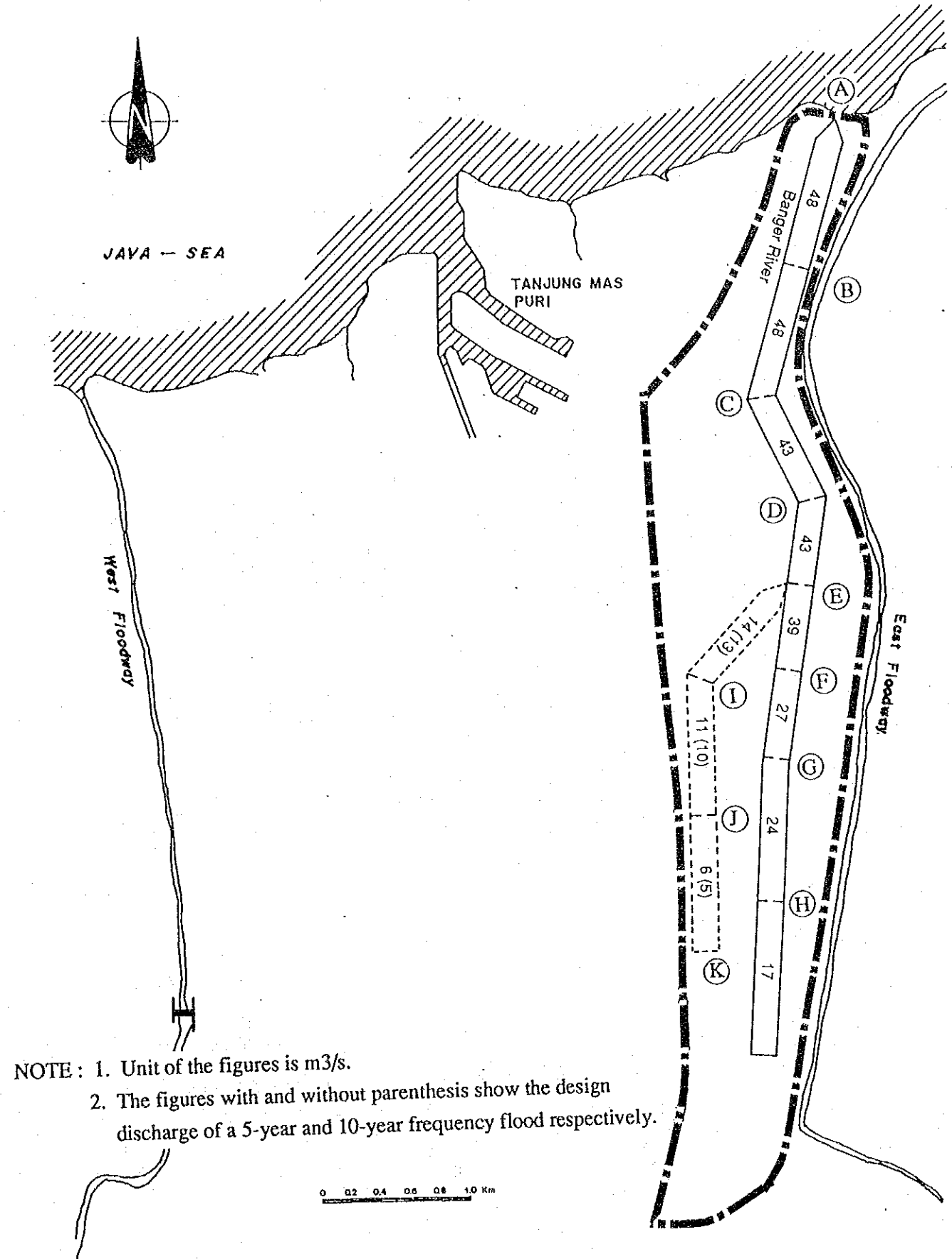
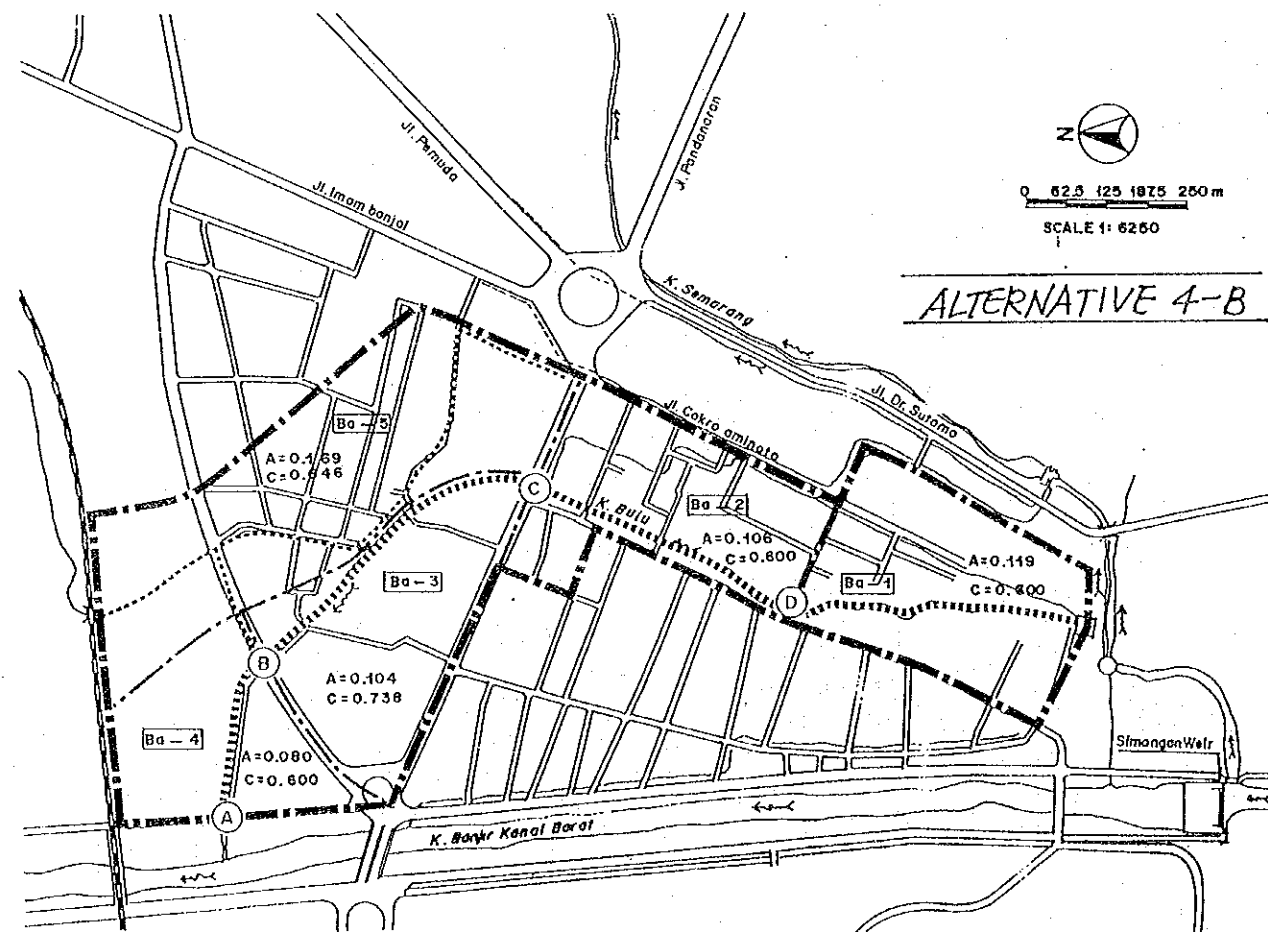
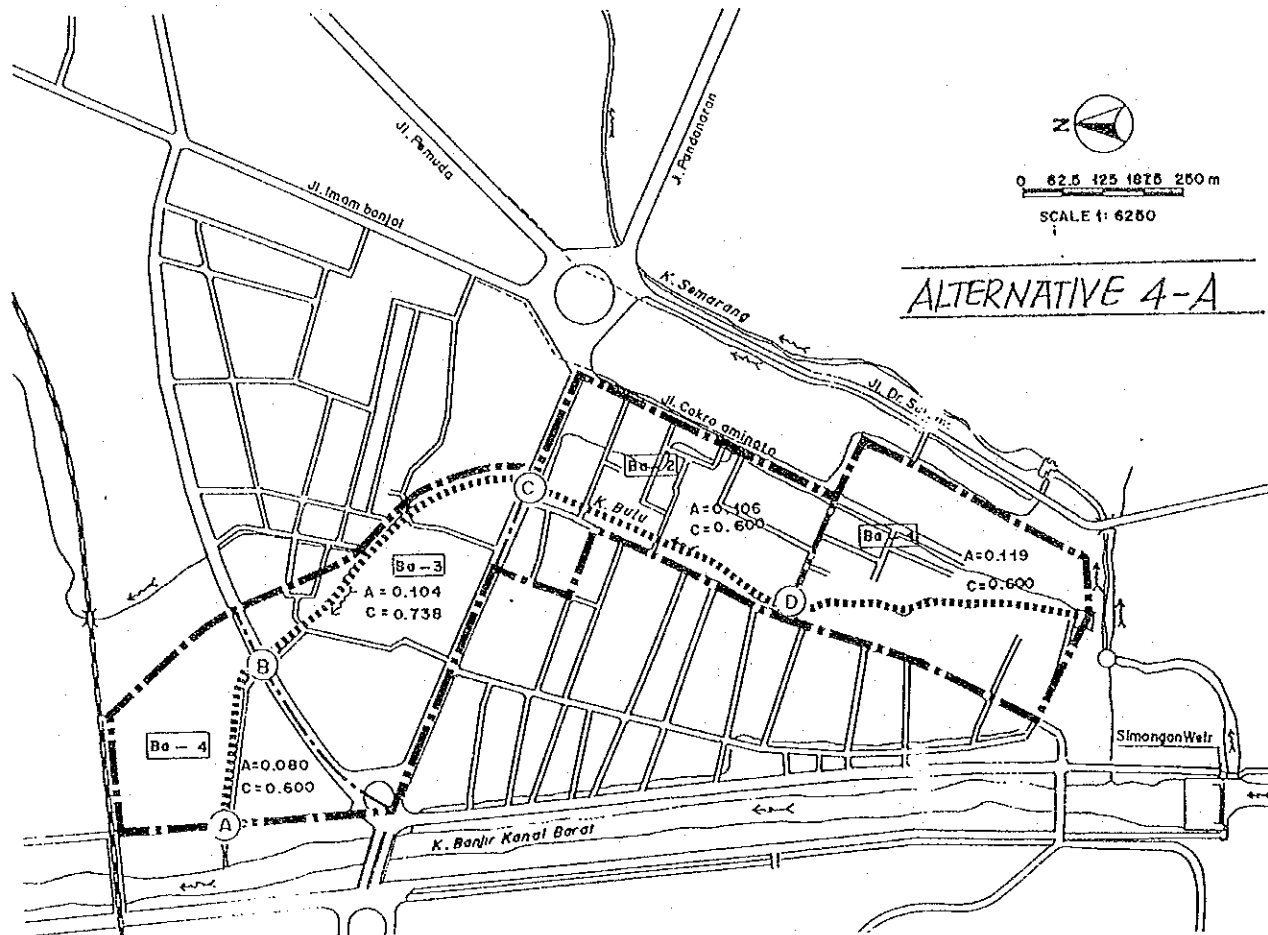


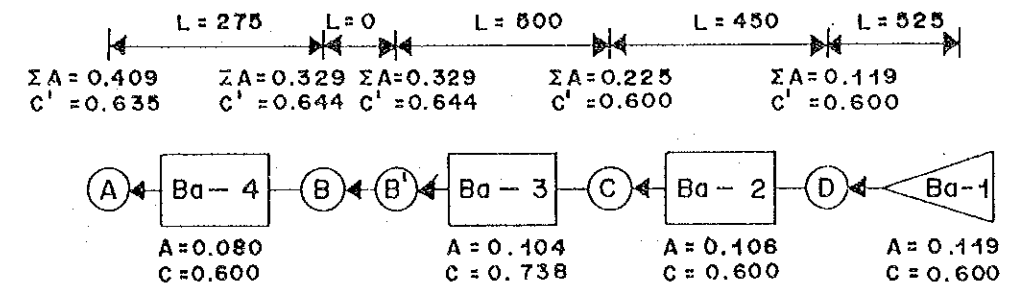
FIG. II-8(3/3) DISTRIBUTION OF DESIGN DISCHARGE(BANGER R.,ALTER.3-B)

5. Bulu River  
(Alternative D-1, D-2)

- (1) Drainage System
- (2) Run-off Model
- (3) Run-off Calculation
- (4) Distribution of Design Discharge
- (5)



**PRESENT BULU DRAINAGE SYSTEM  
(ALTERNATIVE 4-A)**



**PLANNING FOR BULU DRAINAGE SYSTEM  
(ALTERNATIVE 4-B)**

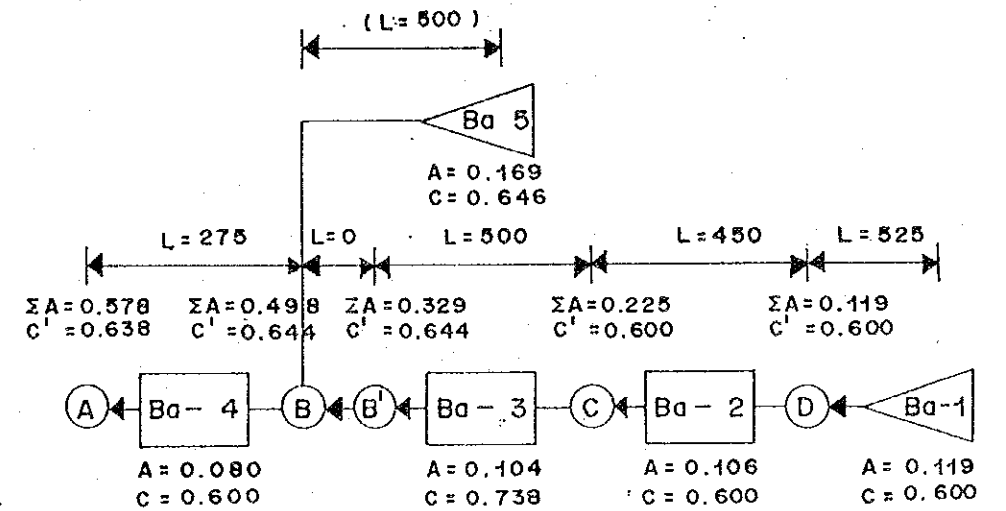


FIG. II-9(1/3) DRAINAGE SYSTEM (BULU R.)

FIG. II-9(2/3) RUNN-OFF MODEL (BULU R.)

Land Use	Run-off C	T<2hr		T>2hr	
		2-year	10-year	5-year	2-year
Business Area	0.8	1400	1000	967	1313
Neighborhood B.A.	0.8	10.19	4.49	2.93	-5.4
Residential Area	0.6	0.73	0.6	0.57	0.74
Industrial & Harbor	0.8				
Green Zone & Others	0.3				

B.Sentral Semarang Area  
4.Bulu River for 1/5  
Alternative 4-A

Proposed Land Use	km2	Alternative 4-A				
		Ba-1	Ba-2	Ba-3	Ba-4	Ba-5
Name of Basin						
Business Area	-	-	0.072	-	-	-
Neighborhood B.A.	-	-	-	-	-	-
Residential Area	0.119	0.106	0.032	0.080	-	-
Industrial & Harbor	-	-	-	-	-	-
Green Zone & Others	-	-	-	-	-	-
Total Area	0.119	0.106	0.104	0.080	-	-
Run-off Coefficient	0.600	0.600	0.738	0.600	-	-
Length of River Cannal	525	450	500	275	-	-

Calculation of Peak Discharge

Calculation Point	D	C	B',B	A
Business Area	-	0.000	0.072	0.072
Neighborhood B.A.	-	0.000	0.000	0.000
Residential Area	0.119	0.225	0.257	0.337
Industrial & Harbor	-	0.000	0.000	0.000
Green Zone & Others	-	0.000	0.000	0.000
Total Area	0.119	0.225	0.329	0.409
Run-off Coefficient	0.600	0.600	0.644	0.635
Length of River Cannal(m)	525	450	500	275
Accumulated Length (m)	525	975	1,475	1,750
Proposed River Slope (%)	0.222%	0.222%	0.222%	0.222%
Proposed Velocity (m/s)	1.250	1.250	1.250	1.250
Flowing Time (min)	7.00	6.00	6.67	3.67
Duration Time (min)	22.00	28.00	34.67	38.33
Rainfall Intensity(mm/hr)	140.01	123.87	110.74	104.95
Reduction Factor	0.999	0.998	0.997	0.997
Peak Discharge(m3/s)	2.219	3.709	5.198	6.039
Specific Q (m3/s/km2)	18.650	16.485	15.799	14.764

TABLE II-9(1/2) RUN-OFF CALCULATION (5-YEAR FERQUENCY FLOOD)  
(BULU R.,ALTER.4-A)

Land Use	Run-off C	T<2hr		T>2hr	
		2-year	10-year	5-year	2-year
Business Area	0.8	1400	1000	967	1313
Neighborhood B.A.	0.8	10.19	4.49	2.93	-5.4
Residential Area	0.6	0.73	0.6	0.57	0.74
Industrial & Harbor	0.8				
Green Zone & Others	0.3				

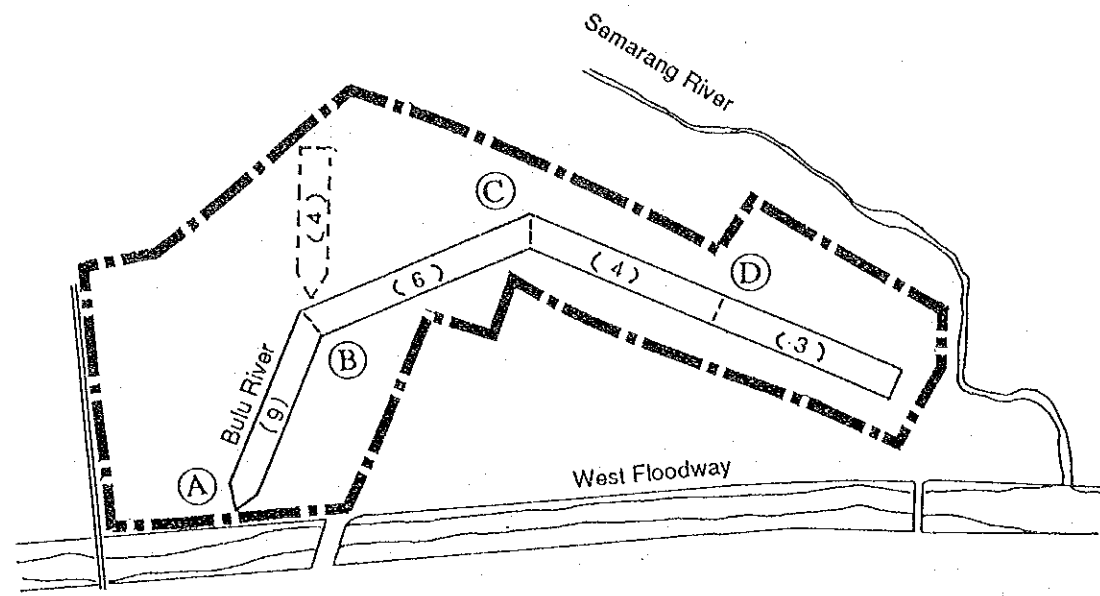
B.Sentral Semarang Area  
4.Bulu River for 1/5  
Alternative 4-B

Proposed Land Use	km2	Alternative 4-B				
		Ba-1	Ba-2	Ba-3	Ba-4	Ba-5
Name of Basin						
Business Area	-	-	0.072	-	-	-
Neighborhood B.A.	-	-	-	-	0.039	-
Residential Area	0.119	0.106	0.032	0.080	0.130	-
Industrial & Harbor	-	-	-	-	-	-
Green Zone & Others	-	-	-	-	-	-
Total Area	0.119	0.106	0.104	0.080	0.169	-
Run-off Coefficient	0.600	0.600	0.738	0.600	0.646	-
Length of River Cannal	525	450	500	275	800	-

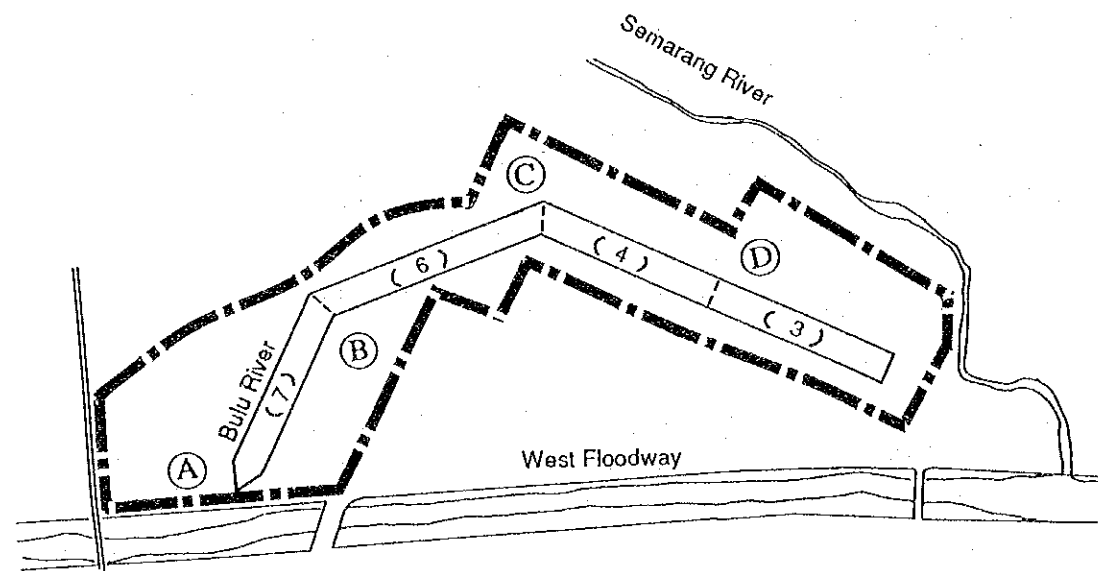
Calculation of Peak Discharge

Calculation Point	D	C	B'	B''	A
Business Area	-	0.000	0.072	0.039	0.111
Neighborhood B.A.	-	0.000	0.000	-	0.000
Residential Area	0.119	0.225	0.257	0.130	0.467
Industrial & Harbor	-	0.000	0.000	-	0.000
Green Zone & Others	-	0.000	0.000	-	0.000
Total Area	0.119	0.225	0.329	0.169	0.578
Run-off Coefficient	0.600	0.600	0.644	0.646	0.638
Length of River Cannal(m)	525	450	500	800	275
Accumulated Length (m)	525	975	1,475	800	1,750
Proposed River Slope (%)	0.222%	0.222%	0.222%	S	0.222%
Proposed Velocity (m/s)	1.250	1.250	1.250	1.500	1.250
Flowing Time (min)	7.00	6.00	6.67	8.89	3.67
Duration Time (min)	22.00	28.00	34.67	23.89	38.33
Rainfall Intensity(mm/hr)	140.01	123.87	110.74	134.34	104.95
Reduction Factor	0.999	0.998	0.997	0.999	0.996
Peak Discharge(m3/s)	2.219	3.709	5.198	3.254	7.865
Specific Q (m3/s/km2)	18.650	16.485	15.799	19.253	14.815

TABLE II-9(2/2) RUN-OFF CALCULATION (5-YEAR FERQUENCY FLOOD)  
(BULU R.,ALTER.4-B)



BULU RIVER (ALT. 4-B)



BULU RIVER (ALT. 4-A)

FIG. II-9(3/3) DISTRIBUTION OF DESIGN DISCHARGE(BULU R.)

6. Ronggolawe-Karangayu River,

Tawany River, and

Silandak Chemnel

- (1) Drainage System
- (2) Run-off Model
- (3) Run-off Calculation
- (4) Distribution of Design Discharge
- (5)

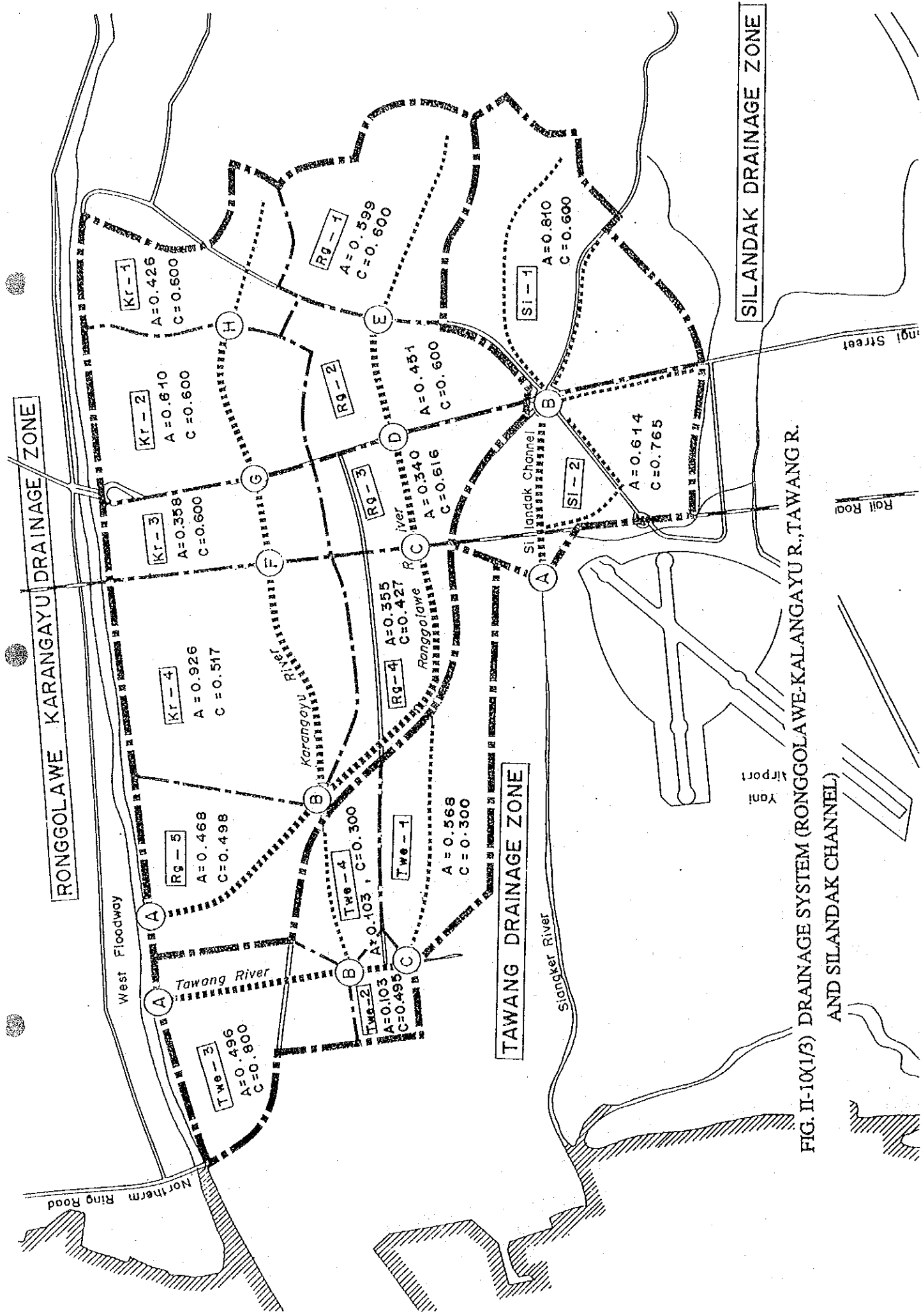
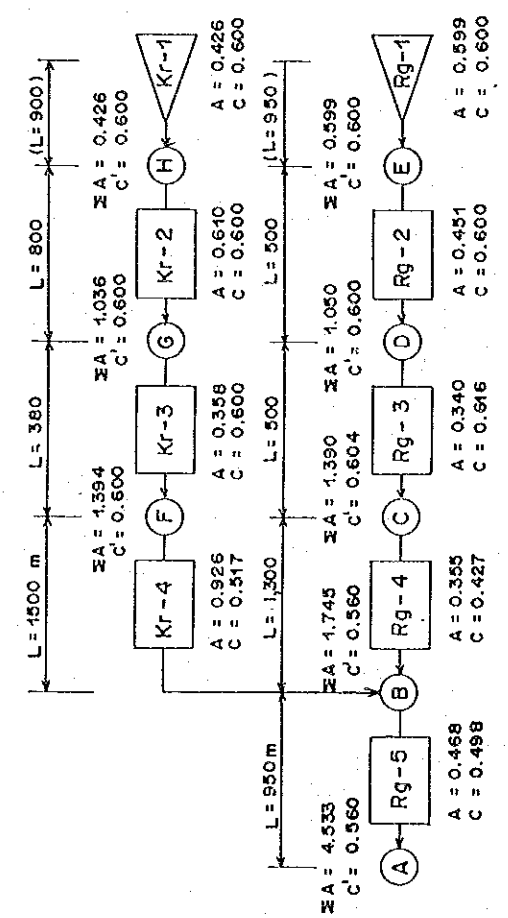
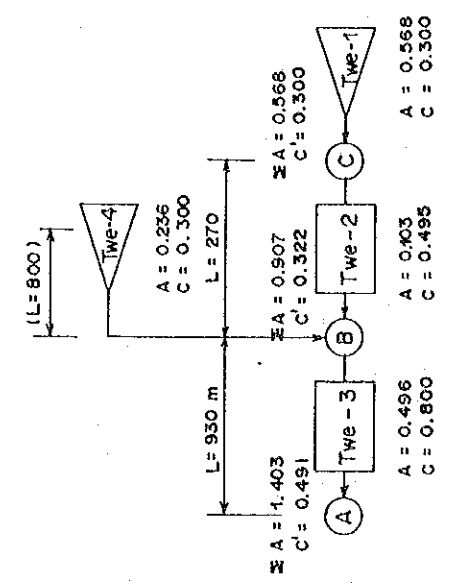


FIG. II-10(1/3) DRAINAGE SYSTEM (RONGGOLAWE-KALANGAYU R., TAWANG R. AND SILANDAK CHANNEL)

RONGGOLAWE - KARANGAYU DRAINAGE SYSTEM



TAWANG DRAINAGE SYSTEM



SILANDAK DRAINAGE SYSTEM

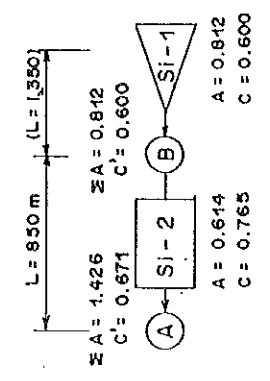


FIG. II-10(2/3) RUNN-OFF MODEL (RONGGOLAWE -KALANGAYU R., TAWANG R. AND SILANDAK CHANNEL)

Land Use	Run-off C
Business Area	0.8
Neighborhood B.A.	0.8
Residential Area	0.6
Industrial & Harbor	0.8
Green Zone & Others	0.3

T<2hr	5-year	10-year	2-year	5-year
2-year	1000	967	1313	2050
10.19	4.49	2.93	-5.4	2.95
0.73	0.6	0.57	0.74	0.76

C. Western Semarang Area  
1. Ronggolawe, 2. Karangayu

Proposed Land Use	km2	Rg-1	Rg-2	Rg-3	Rg-4	Rg-5	Kf-1	Kf-2	Kf-3	Kf-4
Business Area	-	-	-	-	-	-	-	-	-	-
Neighborhood B.A.	-	-	-	-	-	-	-	-	-	-
Residential Area	0.599	0.451	0.313	0.150	0.426	0.610	0.358	0.669		
Industrial & Harbor	-	-	0.027	-	-	-	-	-	-	-
Green Zone & Others	-	-	-	0.205	-	-	-	-	-	-
Total Area	0.599	0.451	0.340	0.355	0.468	0.610	0.358	0.926		
Run-off Coefficient	0.600	0.600	0.616	0.427	0.498	0.600	0.600	0.517		
Length of River Cannal	950	500	500	1,300	950	800	380	1,500		

Calculation of Peak Discharge

Calculation Point	E	D	C	B'	H	G	F	B"	B	A
Business Area	-	0.000	0.000	0.000	-	0.000	0.000	0.000	0.000	0.000
Neighborhood B.A.	-	0.000	0.000	0.000	-	0.000	0.000	0.000	0.000	0.000
Residential Area	0.599	1.050	1.363	1.513	0.426	1.036	1.394	2.063	3.576	3.779
Industrial & Harbor	-	0.000	0.027	0.027	-	0.000	0.000	0.000	0.027	0.090
Green Zone & Others	-	0.000	0.000	0.205	-	0.000	0.000	0.257	0.462	0.664
Total Area	0.599	1.050	1.390	1.745	0.426	1.036	1.394	2.320	4.065	4.533
Run-off Coefficient	0.600	0.600	0.604	0.568	0.600	0.600	0.600	0.567	0.567	0.560
Length of River Cannal (m)	950	500	500	1,300	900	800	380	1,500	-	950
Accumulated Length (m)	S	1,450	1,950	3,250	900	1,700	2,080	3,580	3,580	4,530
Proposed River Slope (%)	1.500	0.455%	0.455%	0.027%	S	0.167%	0.167%	0.020%	-	0.027%
Proposed Velocity (m/s)	10.56	1.500	1.500	0.600	1.500	1.250	1.250	0.500	-	0.600
Flowing Time (min)	25.56	5.56	36.67	72.78	10.00	10.67	5.07	50.00	-	26.39
Duration Time (min)	129.82	117.25	107.48	73.65	131.28	109.08	40.73	90.73	117.12	117.12
Rainfall Intensity (mm/hr)	0.995	0.991	0.988	0.985	0.996	0.991	0.988	0.981	0.968	0.965
Reduction Factor	10.316	16.270	19.815	15.981	7.430	14.936	18.657	18.617	32.216	30.531
Peak Discharge (m3/s)	17.222	15.495	14.255	9.158	17.442	14.417	13.384	8.025	7.925	6.735
Specific Q (m3/s/km2)										

TABLE II-10(1/2) RUN-OFF CALCULATION (5-YEAR FERQUENCY FLOOD)  
(RRONGGOLawe-KALANGAYU R., TAWANG R. AND SILANDAK CHANNEL)

Land Use	Run-off C
Business Area	0.8
Neighborhood B.A.	0.8
Residential Area	0.6
Industrial & Harbor	0.8
Green Zone & Others	0.3

T<2hr	5-year	10-year	2-year	5-year
2-year	1400	1000	967	1313
10.19	4.49	2.93	-5.4	2.95
0.73	0.6	0.57	0.74	0.76

C. Western Semarang Area  
3. Tawang

Proposed Land Use	km2	Twe-1	Twe-2	Twe-3	Twe-4	Si-1	Si-2
Business Area	-	-	-	-	-	-	-
Neighborhood B.A.	-	-	-	-	-	-	-
Residential Area	-	-	-	-	-	0.810	0.107
Industrial & Harbor	-	0.040	0.496	-	-	-	0.507
Green Zone & Others	0.568	0.063	-	0.236	-	-	-
Total Area	0.568	0.103	0.496	0.236	-	0.810	0.614
Run-off Coefficient	0.300	0.495	0.800	0.300	0.600	0.600	0.765
Length of River Cannal	1,000	270	930	800	800	1,350	850

4. Silandak

Calculation of Peak Discharge

Calculation Point	C	B'	B"	B	A	B	A
Business Area	-	0.000	-	0.000	0.000	-	0.000
Neighborhood B.A.	-	0.000	-	0.000	0.000	-	0.000
Residential Area	-	0.000	-	0.000	0.000	0.810	0.917
Industrial & Harbor	-	0.040	-	0.040	0.536	-	0.507
Green Zone & Others	0.568	0.631	0.236	0.867	0.867	-	0.000
Total Area	0.568	0.671	0.236	0.907	1.403	0.810	1.424
Run-off Coefficient	0.300	0.330	0.300	0.322	0.491	0.600	0.671
Length of River Cannal (m)	1,000	270	800	-	930	1,350	850
Accumulated Length (m)	S	1,270	800	1,270	2,200	1,350	2,200
Proposed River Slope (%)	1.500	0.067%	S	0.067%	0.067%	S	0.100%
Proposed Velocity (m/s)	11.11	1.000	1.500	-	1.000	1.500	1.250
Flowing Time (min)	26.11	4.50	8.89	-	15.50	15.00	11.33
Duration Time (min)	128.40	30.61	23.99	30.61	46.11	30.00	41.33
Rainfall Intensity (mm/hr)	0.995	0.994	0.988	0.992	0.988	0.993	0.988
Reduction Factor	4.839	5.785	2.110	7.619	14.366	12.819	21.148
Peak Discharge (m3/s)	8.519	8.621	8.939	8.401	10.239	15.825	14.851
Specific Q (m3/s/km2)							

TABLE II-10(2/2) RUN-OFF CALCULATION (5-YEAR FERQUENCY FLOOD)  
(RRONGGOLawe-KALANGAYU R., TAWANG R. AND SILANDAK CHANNEL)



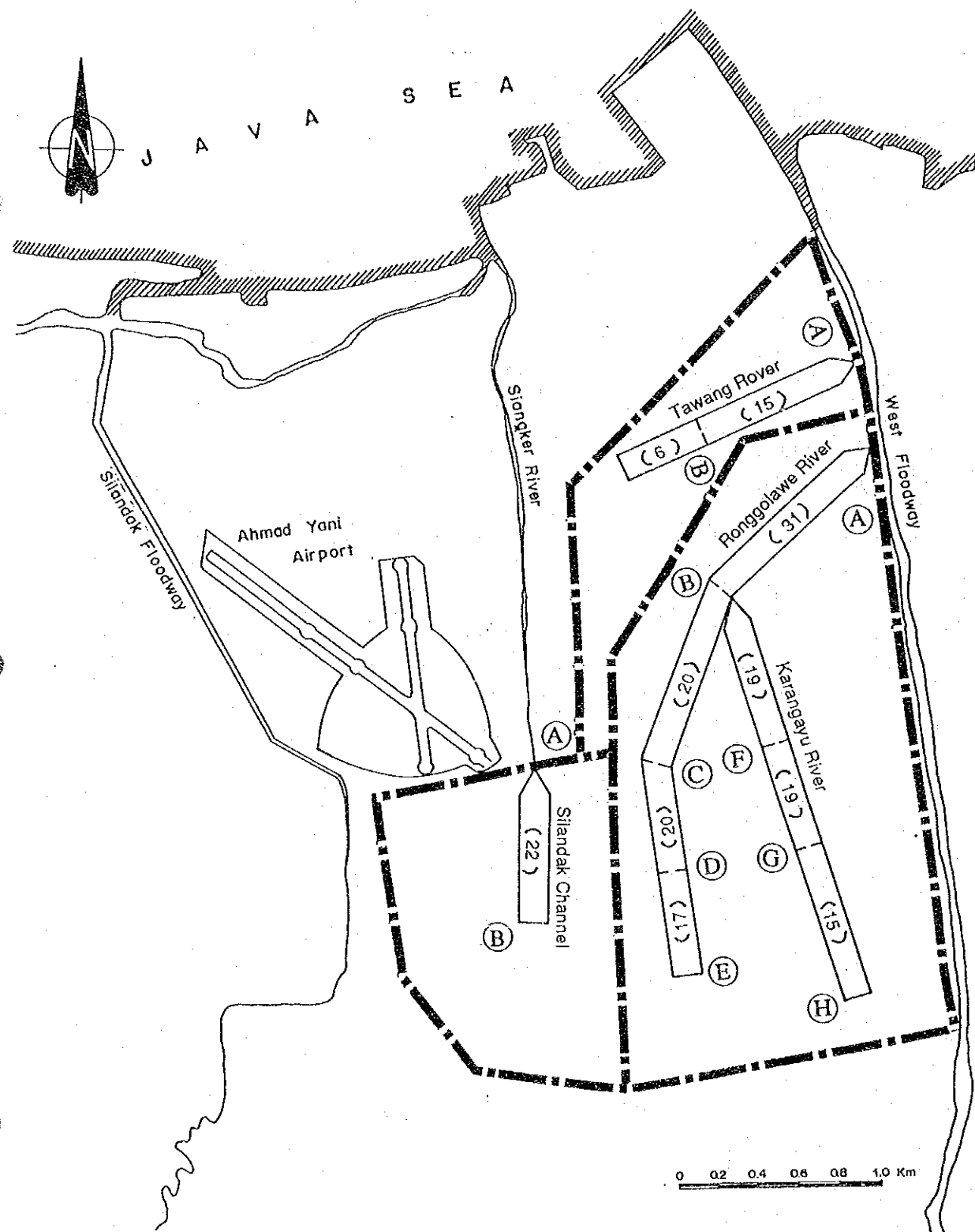


FIG. II-10(3/3) DISTRIBUTION OF DESIGN DISCHARGE(RONGGOLAWE-KALANGAYU R.,TAWANG R. AND SILANDAK CHANNEL)

## 7. 9 Channels in Kec. Tugu Area

(Tambakhario Channel, Buntu River Jumbleng River,  
Tugurejo River, Tapak River, Boom Karanganyar  
River, Randugarut River, Mangkang Wetan River  
and Mangkang Kulon River)

- (1) Drainage System
- (2) Run-off Model
- (3) Run-off Calculation
- (4) Distribution of Design Discharge

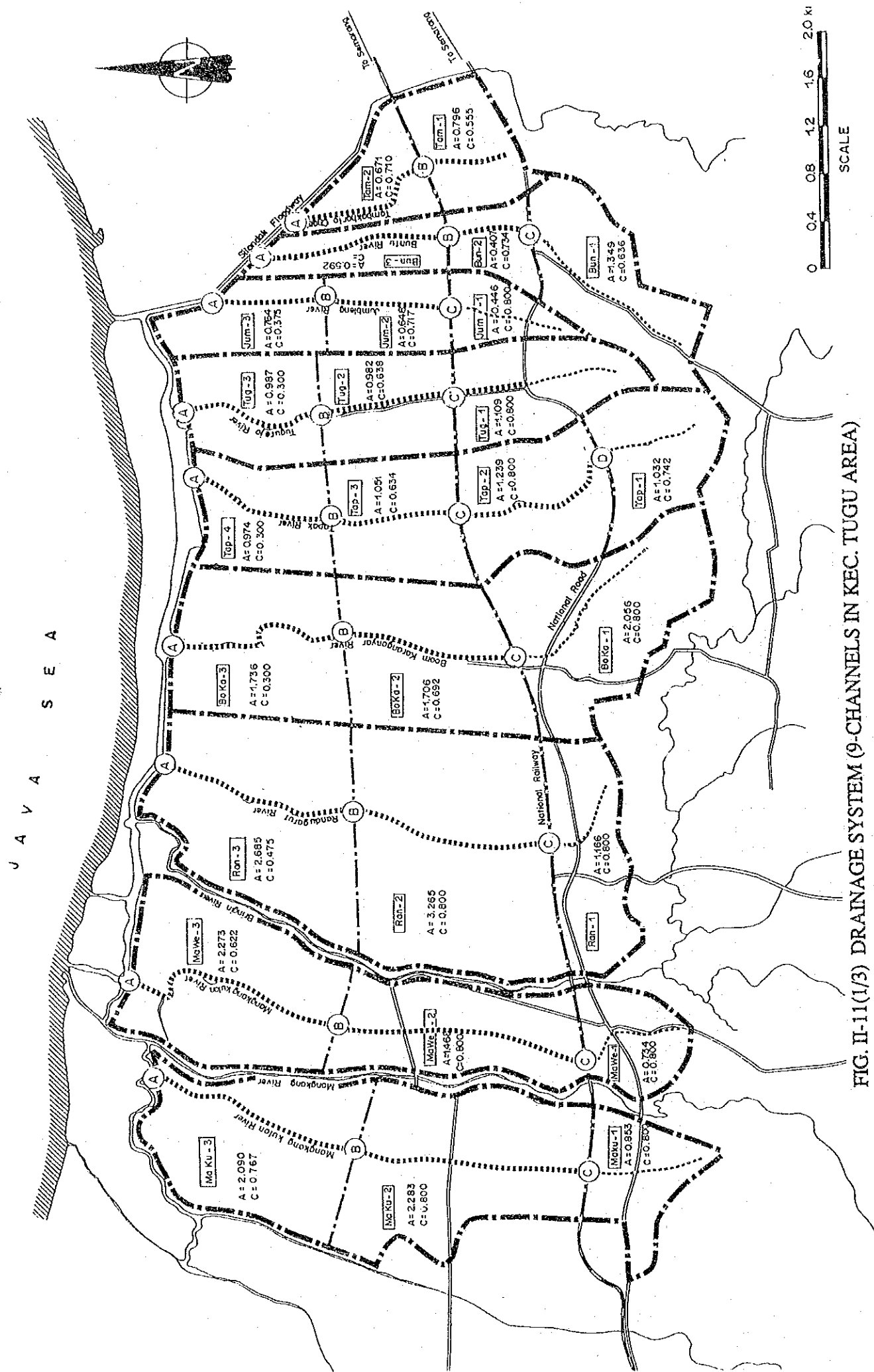
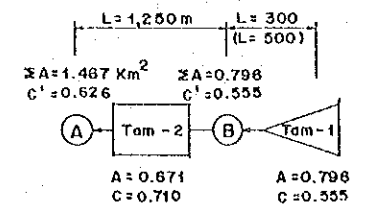
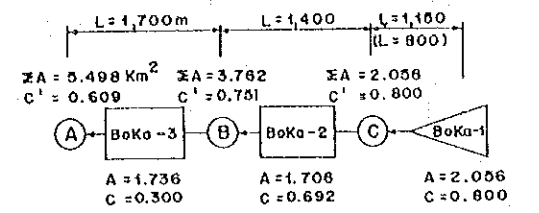


FIG. II-11(1/3) DRAINAGE SYSTEM (9-CHANNELS IN KEC. TUGU AREA)

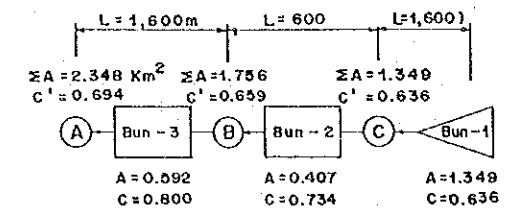
TAMBAKHARJO DRAINAGE SYSTEM



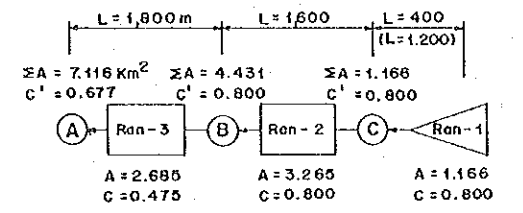
BOOMKARANGANYAR DRAINAGE SYSTEM



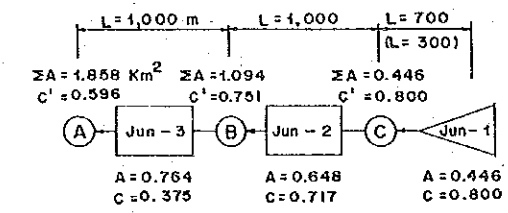
BUNTU DRAINAGE SYSTEM



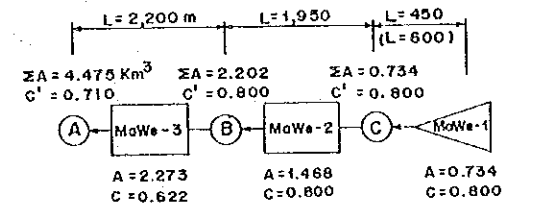
RANDUGARUT DRAINAGE SYSTEM



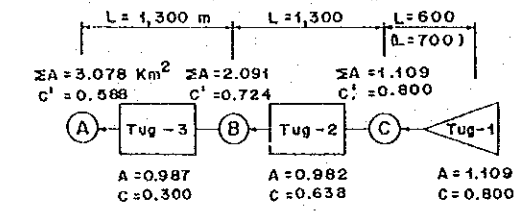
JUMBLENG DRAINAGE SYSTEM



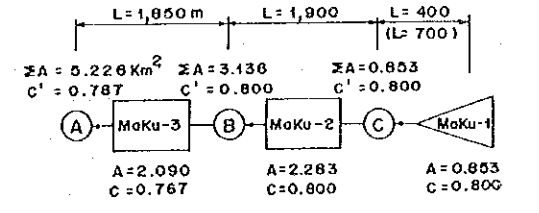
MANGKANGWETAN DRAINAGE SYSTEM



TUGUREJO DRAINAGE SYSTEM



MANGKANG KULON DRAINAGE SYSTEM



TAPAK DRAINAGE SYSTEM

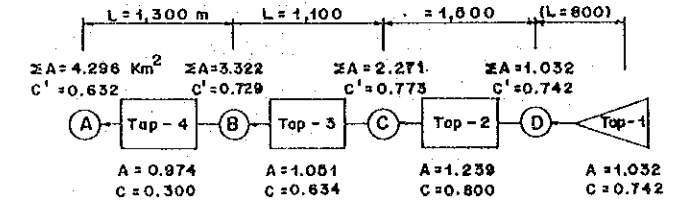


FIG. II11(2/3) RUNN-OFF MODEL (9-CHANNELS IN KEC. TUGU AREA)

Land Use	Run-off C
Business Area	0.8
Neighborhood B.A.	0.8
Residential Area	0.5
Industrial & Harbor	0.8
Green Zone & Others	0.3

T<2hr	5-year	10-year	2-year	5-year
2-year	1400	967	1313	2050
10.19	4.49	2.93	-5.4	2.95
0.73	0.6	0.57	0.74	0.76

1. Tambakharjo

3. Jumbaleng

2. Buntu

Proposed Land Use	Tam-1	Tam-2	Bun-1	Bun-2	Bun-3	Jun-1	Jun-2	Jun-3
Name of Basin	-	-	0.534	-	-	-	-	-
Business Area	-	-	-	-	-	-	-	-
Neighborhood B.A.	-	-	-	-	-	-	-	-
Residential Area	0.651	0.201	0.738	0.089	-	-	-	-
Industrial & Harbor	0.145	0.470	0.077	0.318	0.592	0.372	0.540	0.115
Green Zone & Others	-	-	-	-	-	-	0.108	0.649
Total Area	0.796	0.671	1.349	0.407	0.592	0.446	0.648	0.764
Run-off Coefficient	0.555	0.710	0.636	0.734	0.800	0.800	0.717	0.375
Length of River Cannal	300	1,250	1,500	600	1,600	700	1,000	1,000

Calculation of Peak Discharge

Calculation Point	B	A	C	B	A	C	B	A
Business Area	-	0.000	0.534	0.534	0.534	0.074	0.074	0.074
Neighborhood B.A.	-	0.000	-	0.000	0.000	-	0.000	0.000
Residential Area	0.651	0.852	0.738	0.827	0.827	-	0.000	0.000
Industrial & Harbor	0.145	0.615	0.077	0.395	0.987	0.372	0.911	1.026
Green Zone & Others	-	0.000	-	0.000	0.000	-	0.108	0.758
Total Area	0.796	1.467	1.349	1.756	2.348	0.446	1.094	1.858
Run-off Coefficient	0.555	0.626	0.636	0.659	0.694	0.800	0.751	0.596
Length of River Cannal(m)	300	1,250	1,600	600	1,600	700	1,000	1,000
Accumulated Length (m)	300	1,750	1,600	2,200	3,800	700	1,700	2,700
Proposed River Slope (%)	0.020%	0.020%	S	0.020%	0.020%	S	0.020%	0.020%
Proposed Velocity (m/s)	0.500	0.500	1.500	0.500	0.500	1.500	0.500	0.500
Flowing Time (min)	10.00	41.67	17.78	20.00	53.33	7.78	33.33	33.33
Duration Time (min)	25.00	66.67	32.78	52.78	106.11	22.78	56.11	89.44
Rainfall Intensity(mm/hr)	131.28	77.39	114.08	88.16	59.39	137.60	85.21	65.51
Reduction Factor	0.993	0.988	0.989	0.985	0.980	0.996	0.991	0.984
Peak Discharge(m3/s)	12.793	15.589	21.499	22.323	21.097	10.869	15.405	15.871
Specific Q (m3/s/km2)	16.071	10.627	15.937	12.713	8.985	24.371	14.082	8.542

TABLE II-11(1/5) RUN-OFF CALCULATION (5-YEAR FERQUENCY FLOOD)  
(9-CHANNELS IN KEC. TUGU AREA)

Land Use	Run-off C
Business Area	0.8
Neighborhood B.A.	0.8
Residential Area	0.5
Industrial & Harbor	0.8
Green Zone & Others	0.3

T<2hr	5-year	10-year	2-year	5-year
2-year	1400	967	1313	2050
10.19	4.49	2.93	-5.4	2.95
0.73	0.6	0.57	0.74	0.76

4. Tugurejo

5. Tapak

Proposed Land Use	Tug-1	Tug-2	Tug-3	Tap-1	Tap-2	Tap-3	Tap-4
Name of Basin	-	-	-	0.101	0.228	-	-
Business Area	0.608	-	-	-	-	-	-
Neighborhood B.A.	-	-	-	-	-	-	-
Residential Area	0.501	0.664	-	0.201	-	-	-
Industrial & Harbor	-	0.318	-	0.730	1.011	0.701	-
Green Zone & Others	-	0.982	0.987	-	-	0.350	0.974
Total Area	1.109	0.982	0.987	1.032	1.239	1.051	0.974
Run-off Coefficient	0.800	0.638	0.300	0.742	0.800	0.634	0.300
Length of River Cannal	600	1,300	1300	800	1,500	1100	1,300

Calculation of Peak Discharge

Calculation Point	C	B	A	D	C	B	A
Business Area	0.608	0.608	0.608	0.101	0.329	0.329	0.329
Neighborhood B.A.	-	0.000	0.000	0.000	0.000	0.000	0.000
Residential Area	-	0.000	0.000	0.201	0.201	0.201	0.201
Industrial & Harbor	0.501	1.165	1.165	0.730	1.741	2.442	2.442
Green Zone & Others	-	0.318	1.305	0.000	0.000	0.350	1.324
Total Area	1.109	2.091	3.078	1.032	2.271	3.322	4.296
Run-off Coefficient	0.800	0.724	0.588	0.742	0.773	0.729	0.632
Length of River Cannal(m)	600	1,300	1,300	800	1,500	1,100	1,300
Accumulated Length (m)	600	1,900	3,200	800	2,300	3,400	4,700
Proposed River Slope (%)	0.020%	0.020%	0.020%	S	0.020%	0.020%	0.020%
Proposed Velocity (m/s)	0.500	0.500	0.500	1.500	0.500	0.500	0.500
Flowing Time (min)	20.00	43.33	43.33	8.89	50.00	36.67	43.33
Duration Time (min)	35.00	78.33	121.67	23.89	73.89	110.56	153.89
Rainfall Intensity(mm/hr)	110.18	70.65	54.89	134.34	73.03	58.01	43.98
Reduction Factor	0.991	0.982	0.975	0.991	0.981	0.973	0.966
Peak Discharge(m3/s)	21.520	23.347	21.522	22.646	27.963	30.386	25.628
Specific Q (m3/s/km2)	19.405	11.165	6.992	21.944	12.313	9.147	5.966

TABLE II-11(2/5) RUN-OFF CALCULATION (5-YEAR FERQUENCY FLOOD)  
(9-CHANNELS IN KEC. TUGU AREA)

Land Use	Run-off C
Business Area	0.8
Neighborhood B.A.	0.8
Residential Area	0.5
Industrial & Harbor	0.8
Green Zone & Others	0.3

T<2hr		T>2hr	
2-year	5-year	10-year	2-year
1400	1000	967	1313
10.19	4.49	2.93	-5.4
0.73	0.6	0.57	0.74
			0.76

6. Boomkaranganyar 7. Randugarut

Proposed Land Use	Boka-1	Boka-2	Boka-3	Ran-1	Ran-2	Ran-3
Name of Basin	0.074	-	-	-	-	-
Business Area	-	-	-	-	-	-
Neighborhood B.A.	-	-	-	-	-	-
Residential Area	1.982	1.338	-	1.166	3.262	0.940
Industrial & Harbor	-	0.368	1.736	-	0.003	1.745
Green Zone & Others	2.056	1.706	1.736	1.166	3.265	2.685
Total Area	0.800	0.692	0.300	0.800	0.800	0.475
Run-off Coefficient	1,150	1,400	1700	400	1,600	1800
Length of River Cannal						

Calculation of Peak Discharge

Calculation Point	C	B	A	C	B	A
Business Area	0.074	0.074	0.074	0.000	0.000	0.000
Neighborhood B.A.	-	0.000	0.000	0.000	0.000	0.000
Residential Area	-	0.000	0.000	0.000	0.000	0.000
Industrial & Harbor	1.982	3.319	3.319	1.166	4.428	5.367
Green Zone & Others	-	0.368	2.104	0.000	0.003	1.749
Total Area	2.056	3.762	5.498	1.166	4.431	7.116
Run-off Coefficient	0.800	0.751	0.609	0.800	0.800	0.677
Length of River Cannal(m)	1,150	1,400	1,700	400	1,600	1,800
Accumulated Length (m)	1,150	2,550	4,250	400	2,000	3,800
Proposed River Slope (%)	S	0.020%	0.020%	S	0.020%	0.020%
Proposed Velocity (m/s)	1.500	0.500	0.500	1.500	0.500	0.500
Flowing Time (min)	12.78	46.67	56.67	4.44	53.33	60.00
Duration Time (min)	27.78	74.44	131.11	19.44	72.78	132.78
Rainfall Intensity(mm/hr)	137.31	81.08	55.60	164.46	82.09	55.09
Reduction Factor	0.983	0.970	0.958	0.990	0.965	0.948
Peak Discharge(m <sup>3</sup> /s)	49.317	49.374	39.624	33.756	62.395	55.941
Specific Q (m <sup>3</sup> /s/km <sup>2</sup> )	23.987	13.124	7.207	28.950	14.081	7.861

TABLE II-11(3/5) RUN-OFF CALCULATION (5-YEAR FERQUENCY FLOOD)  
(9-CHANNELS IN KEC. TUGU AREA)

Land Use	Run-off C
Business Area	0.8
Neighborhood B.A.	0.8
Residential Area	0.5
Industrial & Harbor	0.8
Green Zone & Others	0.3

T<2hr		T>2hr	
2-year	5-year	10-year	2-year
1400	1000	1000	1313
10.19	4.49	4.49	-5.4
0.73	0.6	0.6	0.74
			0.76

8. Mangkang Wetan 9. Mangkang Kulon

Proposed Land Use	MaWe-1	MaWe-2	MaWe-3	MaKu-1	MaKu-2	MaKu-3
Name of Basin	-	-	-	-	-	-
Business Area	-	-	-	-	-	-
Neighborhood B.A.	-	-	-	-	-	-
Residential Area	0.734	1.468	1.464	0.853	2.283	1.952
Industrial & Harbor	-	-	0.809	-	-	0.138
Green Zone & Others	-	-	2.273	0.853	2.283	2.090
Total Area	0.734	1.468	0.622	0.800	0.800	0.767
Run-off Coefficient	0.800	0.800	0.800	0.800	0.800	0.800
Length of River Cannal	450	1,950	2200	400	1,900	1,850

Calculation of Peak Discharge

Calculation Point	C	B	A	C	B	A
Business Area	-	0.000	0.000	0.000	0.000	0.000
Neighborhood B.A.	-	0.000	0.000	0.000	0.000	0.000
Residential Area	-	0.000	0.000	0.000	0.000	0.000
Industrial & Harbor	0.734	2.202	3.666	0.853	3.136	5.088
Green Zone & Others	-	0.000	0.809	0.000	0.000	0.138
Total Area	0.734	2.202	4.475	0.853	3.136	5.226
Run-off Coefficient	0.800	0.800	0.710	0.800	0.800	0.787
Length of River Cannal(m)	450	1,950	2,200	400	1,900	1,850
Accumulated Length (m)	450	2,400	4,600	400	2,300	4,150
Proposed River Slope (%)	S	0.020%	0.020%	S	0.020%	0.020%
Proposed Velocity (m/s)	1.500	0.500	0.500	1.500	0.500	0.500
Flowing Time (min)	5.00	65.00	73.33	4.44	63.33	61.67
Duration Time (min)	20.00	85.00	158.33	19.44	82.78	144.44
Rainfall Intensity(mm/hr)	146.76	67.44	43.05	148.79	68.47	46.10
Peak Discharge(m <sup>3</sup> /s)	19.152	26.404	30.381	22.566	38.175	42.127
Specific Q (m <sup>3</sup> /s/km <sup>2</sup> )	26.093	11.991	6.789	26.454	12.173	8.061

TABLE II-11(4/5) RUN-OFF CALCULATION (5-YEAR FERQUENCY FLOOD)  
(9-CHANNELS IN KEC. TUGU AREA)

Land Use	Run-off C
Business Area	0.8
Neighborhood B.A.	0.8
Residential Area	0.5
Industrial & Harbor	0.8
Green Zone & Others	0.3

	T<2hr		T>2hr	
	2-year	5-year	2-year	5-year
1400	967	967	1313	2600
10.19	2.93	2.93	-5.4	7.2
0.73	0.57	0.57	0.74	0.78

8. Mangkang Wetan

9. Mangkang Kulon

Proposed Land Use	MaWe-1	MaWe-2	MaWe-3	MaKu-1	MaKu-2	MaKu-3
Business Area	-	-	-	-	-	-
Neighborhood B.A.	-	-	-	-	-	-
Residential Area	0.734	1.468	1.464	0.853	2.283	1.952
Industrial & Harbor	-	-	0.809	-	-	0.138
Green Zone & Others	-	-	2.273	0.853	2.283	2.090
Total Area	0.734	1.468	2.273	0.800	0.800	0.767
Run-off Coefficient	0.800	0.800	0.622	0.800	0.800	0.767
Length of River Cannal	450	1,950	2200	400	1,900	1,850

Calculation of Peak Discharge

Calculation Point	A	B	C	A	B	C
Business Area	0.000	0.000	0.000	0.000	0.000	0.000
Neighborhood B.A.	0.000	0.000	0.000	0.000	0.000	0.000
Residential Area	0.734	2.202	3.666	0.853	3.136	5.088
Industrial & Harbor	0.000	0.809	0.000	0.000	0.000	0.138
Green Zone & Others	0.734	2.202	4.475	0.853	3.136	5.226
Total Area	0.800	0.800	0.710	0.800	0.800	0.787
Run-off Coefficient	0.800	0.800	0.710	0.800	0.800	0.787
Length of River Cannal(m)	450	1,950	2,200	400	1,900	1,850
Accumulated Length (m)	450	2,400	4,600	400	2,300	4,150
Proposed River Slope (%)	S	0.020%	0.020%	S	0.020%	0.020%
Proposed Velocity (m/s)	1.500	0.500	0.500	1.500	0.500	0.500
Flowing Time (min)	5.00	65.00	73.33	4.44	63.33	61.67
Duration Time (min)	20.00	85.00	158.33	19.44	82.78	144.44
Rainfall Intensity(mm/hr)	162.18	75.38	48.33	164.46	76.49	51.75
Peak Discharge(m <sup>3</sup> /s)	21.164	29.512	34.109	24.942	42.647	47.292
Specific Q (m <sup>3</sup> /s/km <sup>2</sup> )	28.834	13.402	7.522	29.240	13.599	9.049

TABLE II-11(5/5) RUN-OFF CALCULATION (10-YEAR FREQUENCY FLOOD)  
(9-CHANNELS IN KEC. TUGU AREA)

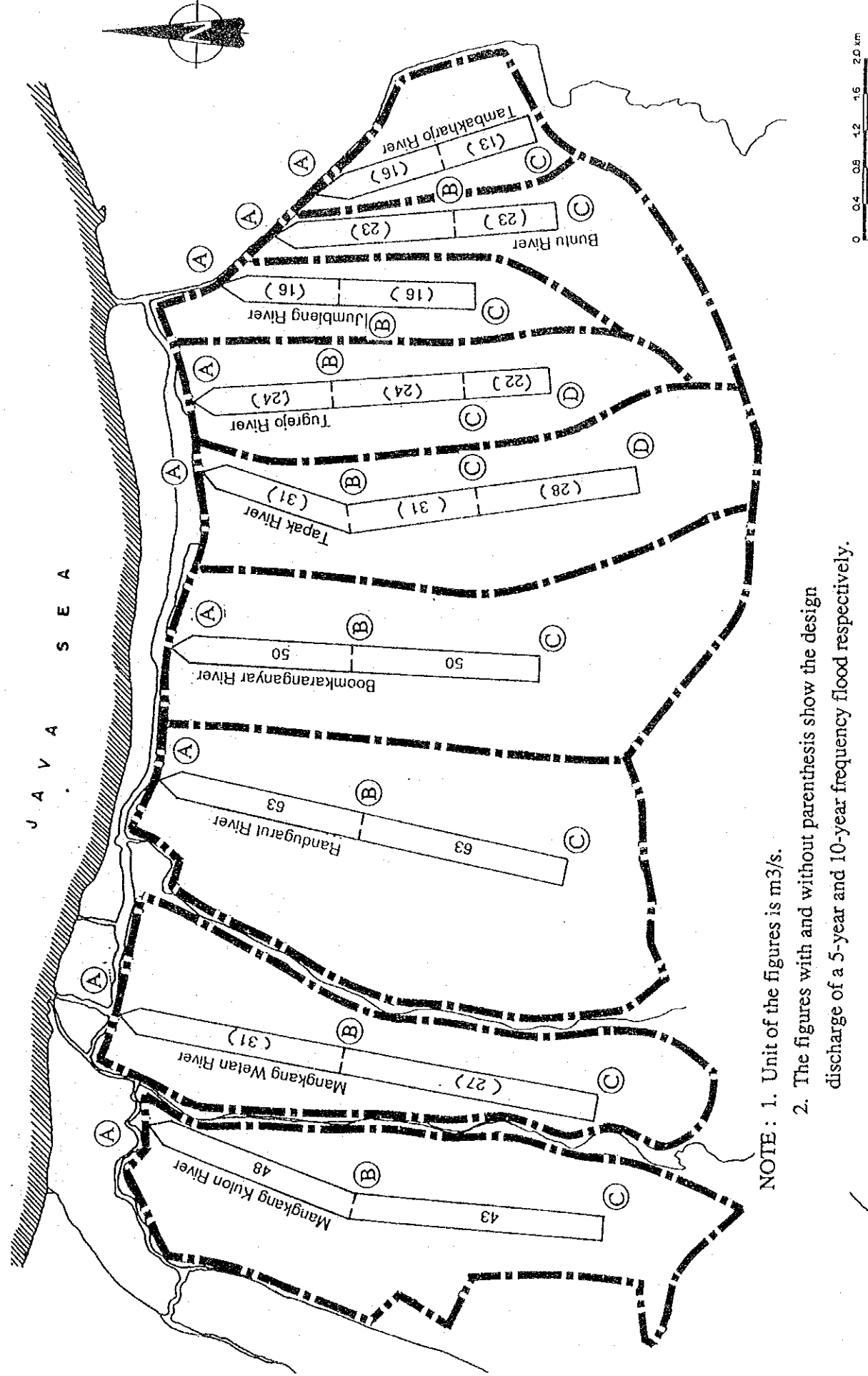


FIG. III1(3/3) DISTRIBUTION OF DESIGN DISCHARGE(9-CHANNELS IN KEC. TUGU AREA)

NOTE : 1. Unit of the figures is m<sup>3</sup>/s.

2. The figures with and without parenthesis show the design discharge of a 5-year and 10-year frequency flood respectively.

SECTOR X

RIPARIAN STRUCTURE DESIGN

Table 1 WORK QUANTITY OF PUMPING STATION

Item	Unit	Bandarharjo West	Asin River Basin	Bandarharjo East	Remarks
		P1 (Q=0.78m <sup>3</sup> /s)	P2 (Q=5.70m <sup>3</sup> /s)	P3 (Q=2.00m <sup>3</sup> /s)	
<b>I. Civil Work</b>					
1. Preparatory Works	L.S.	1	1	1	
2. Excavation	m <sup>3</sup>	972	2,690	1,400	
3. Embankment	m <sup>3</sup>	55	542	559	
4. Backfilling	m <sup>3</sup>	522	1,173	623	
5. Foundation Pile (ø0.5m)	m	512	990	776	
6. Conc. Sheet Pile (L=4.0m)	m <sup>2</sup>	23	126	108	
7. Reinforced Concrete	m <sup>3</sup>	414	1,027	680	
8. Leveling Concrete	m <sup>3</sup>	20	57	36	
9. Concrete Block (t=0.5m)	m <sup>2</sup>	-	160	100	
10. Building Works	L.S.	1	1	1	
11. Miscellaneous	L.S.	1	1	1	
<b>II. Mechanical &amp; Electrical Works</b>					
	L.S.	1	1	1	

Table 2 WORK QUANTITY OF GATE STRUCTURE

Item	Unit	Bandarharjo	Asin River	Bandarharjo	Baru	Remarks
		West	Basin	East	River	
<b>I. Civil Work</b>						
1. Preparatory Works	L.S.	1	1	1	1	
2. Excavation	m <sup>3</sup>	389	776	544	212	
3. Backfilling	m <sup>3</sup>	188	138	261	88	
4. Foundation Pile (ø0.5m)	m	488	748	360	288	
5. Conc. Sheet Pile (L=4.0m)	m <sup>2</sup>	52	189	66	66	
6. Reinforced Concrete	m <sup>3</sup>	104	367	122	108	
7. Leveling Concrete	m <sup>3</sup>	15	43	18	15	
8. Concrete Block (t=0.5m)	m <sup>2</sup>	100	400	100	50	
9. Miscellaneous	L.S.	1	1	1	1	
<b>II. Mechanical &amp; Electrical Works</b>						
	L.S.	1	1	1	1	

Table 3 WORK QUANTITY OF RETARDING POND

Item	Unit	Bandarharjo	Asin River	Bandarharjo	Remarks
		West	Basin	East	
1. Preparatory Works	L.S.	1	1	1	
2. Excavation	m <sup>3</sup>	20,160	75,660	15,200	
3. Drain Ditch	m	488	857	480	Table 9
4. Overflow Section	L.S.	1	1	1	Table 6
5. Revetment (Type-F)	m <sup>2</sup>	1,374	3,342	-	Table 8
6. Retaining Wall	m	-	-	500	Table 10
7. Miscellaneous	L.S.	1	1	1	

Table 4 WORK QUANTITY OF ROAD BRIDGE

Item	Unit	Quantity	Remarks
(1) Excavation (common 2)	m <sup>3</sup>	2440	
(2) Back Filling	m <sup>3</sup>	2110	
(3) Reinforced Concrete	m <sup>3</sup>	297	
(4) PC Girder (l=20.0m)	p.c.	5	
(5) PC Pile (ø0.5m)	m	480	
(6) Pavement	m <sup>2</sup>	120	
(7) Handrail	m	40	

Table 5 WORK QUANTITY OF OVERFLOW SECTION

Item	Unit	Bandarharjo	Asin River	Bandarharjo	Remarks
		West	Basin	East	
1. Excavation	m <sup>3</sup>	237	505	135	
2. Revetment (Type-E)	m <sup>2</sup>	75	568	-	Table 8
3. Reinforced Concrete	m <sup>3</sup>	41	185	135	
4. Base Concrete	m <sup>3</sup>	7	31	66	
5. Gabion Mattress	m <sup>3</sup>	196	320	-	
6. Bar Screen	m	17	77	41	
7. Operation Bridge	m <sup>2</sup>	26	116	-	Table 11
8. RC Slab Bridge	m <sup>2</sup>	-	-	328	Table 12



Table 6 WORK QUANTITY OF OPEN CHANNEL

Item	Unit	(for 10m)				
		Bandarharjo West	Asin River Basin	Bandarharjo East	Semarang River	Baru River
<b>1. Open Channel (Type-A)</b>						
(1) Excavation	m3	-	-	-	326	-
(2) Embankment	m3	-	-	-	27	-
(3) Sodding	m2	-	-	-	28	-
<b>2. Open Channel (Type-D)</b>						
(1) Excavation	m3	198	300	210	483	227
(2) Back Filling	m3	134	206	143	294	143
(3) Stone Masonry	m3	63	94	67	113	92
(4) Log Pile	p.c.	40	40	40	40	40
(5) Base Concrete	m3	6	-	6	-	-
<b>3. Open Channel (Type-F)</b>						
(1) Excavation	m3	-	-	-	84	-
<b>4. Open Channel (Type-G)</b>						
(1) Excavation	m3	-	-	-	-	226
(2) Back Filling	m3	-	-	-	-	130
(3) Stone Masonry	m3	-	-	-	-	50
(4) Reinforced Concrete	m3	-	-	-	-	34
(5) Base Concrete	m3	-	-	-	-	16
(6) Concrete Sheet Pile	m2	-	-	-	-	40
(7) Log Pile	p.c.	-	-	-	-	40

Table 7 WORK QUANTITY OF REVETMENT

Revetment (Type D)			
(for 100m2)			
Item	Unit	Quantity	Remarks
(1) Excavation (common 2)	m3	68	
(2) Back Filling	m3	20	
(3) Reinforced Concrete	m3	6	
(4) Log Pile (ø0.2m)	p.c.	8	
(5) Cobble Stone Pitching	m3	20	
(6) Gravel	m3	21	
Revetment (Type E)			
(for 100m2)			
Item	Unit	Quantity	Remarks
(1) Excavation (common 2)	m3	68	
(2) Back Filling	m3	20	
(3) Reinforced Concrete	m3	6	
(4) Conc. Sheet Pile (t=0.2m)	m2	50	
(5) Cobble Stone Pitching	m3	20	
(6) Gravel	m3	21	
Revetment (Type F)			
(for 100m2)			
Item	Unit	Quantity	Remarks
(1) Excavation (common 2)	m3	86	
(2) Back Filling	m3	13	
(3) Reinforced Concrete	m3	5	
(4) Conc. Sheet Pile (t=0.2m)	m2	73	
(5) Cobble Stone Pitching	m3	30	
(6) Gravel	m3	38	

Table 8 WORK QUANTITY OF DRAIN DITCH

(for 100m)			
Item	Unit	Quantity	Remarks
(1) Excavation (common 2)	m3	161	
(2) Back Filling	m3	63	
(3) Stone Masonry	m3	44	
(4) Leveling Concrete	m3	7	
(5) Gravel	m3	7	

Table 9 WORK QUANTITY OF RETAINING WALL

(for 10m)			
Item	Unit	Quantity	Remarks
(1) Excavation (common 2)	m3	140	
(2) Back Filling	m3	86	
(3) Stone Masonry	m3	54	
(4) Conc. Sheet Pile (t=0.2m)	m2	40	
(5) Log Pile ( $\phi$ 0.2m, l=4.0m)	p.c.	20	

Table 10 WORK QUANTITY OF OPERATION BRIDGE

Item	Unit	Bandarharjo	Asin River	Bandarharjo	Remarks
		West	Basin	East	
1. Steel Girder (h=0.25m)	m	34	154	-	
2. Checked Steel Plate	m2	26	116	-	
3. Handrail (h=1.0m)	m	34	154	-	
4. Reinforced Concrete	m3	21	84	-	
5. Log Pile ( $\phi$ 0.2m)	p.c.	16	64	-	
6. Miscellaneous	L.S.	1	1	-	
7. Installation	L.S.	1	1	-	

Table 11 WORK QUANTITY OF RC SLAB BRIDGE

Item	Unit	Bandarharjo	Asin River	Bandarharjo	Remarks
		West	Basin	East	
1. Reinforced Concrete	m3	-	-	205	
2. Log Pile ( $\phi$ 0.2m)	p.c.	-	-	216	
3. Handrail (h=1.0m)	m	-	-	82	

Table 12 (1/2) DREDGING VOLUME OF CHANNEL IMPROVEMENT

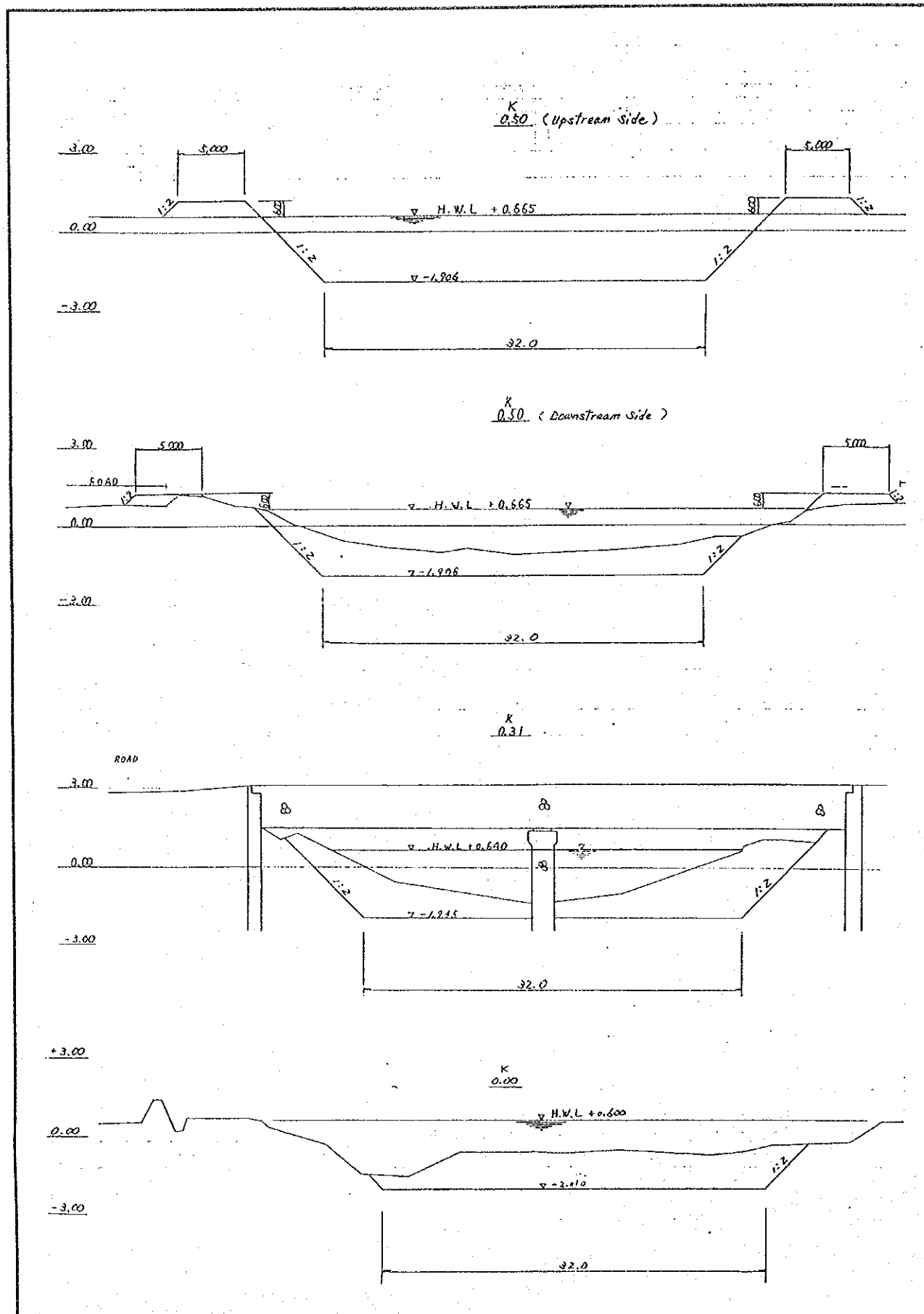
## Semarang River

Station No.	Distance (m)	Excavation		Filling		Sodding								
		Area(m <sup>2</sup> )	Ave. (m <sup>2</sup> )	Volume(m <sup>3</sup> )	Area(m <sup>2</sup> )	Avg(m <sup>2</sup> )	Volume(m <sup>3</sup> )	Slope(m)	Avg(m)	Area(m <sup>2</sup> )				
-k0.10		36.4												
k0.00	100.0	36.4	36.4	3640.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
k0.31	310.0	45.0	40.7	12617.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
k0.50(D)	190.0	33.7	39.3	7467.0		4.0	2.0	380.0		3.2	1.6	304.0		
k0.50(U)	0.0	87.7	60.7	0.0		7.4	5.7	0.0		5.7	4.4	0.0		
k1.00(D)	500.0	59.5	73.6	38800.0		7.4	7.4	3700.0		6.0	5.8	2900.0		
k1.00(U)	0.0	14.4	24.0	0.0		6.1	5.1	0.0		6.1	4.7	0.0		
k1.26	260.0	17.1	15.8	4108.0		0.0	3.1	806.0		0.0	3.1	806.0		
k1.50	240.0	7.4	12.2	2928.0		4.2	2.1	504.0		4.3	2.2	528.0		
k1.74	240.0	13.0	10.2	2448.0		0.0	2.1	504.0		0.0	2.2	528.0		
k2.00	260.0	19.8	16.4	4264.0		1.4	0.7	182.0		5.2	2.6	676.0		
k2.36	360.0	13.0	16.4	5904.0		0.0	0.7	252.0		0.0	2.6	936.0		
k2.50	140.0	10.1	11.5	1610.0		8.3	4.2	588.0		2.8	1.4	196.0		
k2.85	350.0	13.7	11.9	4165.0		0.0	4.2	1470.0		0.0	1.4	490.0		
k3.11	260.0	11.7	12.7	3302.0		0.0	0.0	0.0		0.0	0.0	0.0		
k3.45	340.0	17.8	14.8	5032.0		0.0	0.0	0.0		0.0	0.0	0.0		
k3.93	480.0	14.8	16.3	7824.0		0.0	0.0	0.0		0.0	0.0	0.0		
k4.34	410.0	9.2	12.0	4920.0		0.0	0.0	0.0		0.0	0.0	0.0		
k4.47	130.0	6.6	7.9	1027.0		0.0	0.0	0.0		0.0	0.0	0.0		
k5.00	530.0	8.0	7.3	3869.0		0.0	0.0	0.0		0.0	0.0	0.0		
k5.51	510.0	0.3	4.1	2091.0		0.0	0.0	0.0		0.0	0.0	0.0		
k5.97	460.0	1.0	0.6	276.0		0.0	0.0	0.0		0.0	0.0	0.0		
k6.39	420.0	8.6	4.8	2016.0		0.0	0.0	0.0		0.0	0.0	0.0		
k6.88	490.0	5.7	7.2	3528.0		0.0	0.0	0.0		0.0	0.0	0.0		
Total	6880.0			119836.0				8386.0						7364.0

Table 12 (2/2) DREDGING VOLUME OF CHANNEL IMPROVEMENT

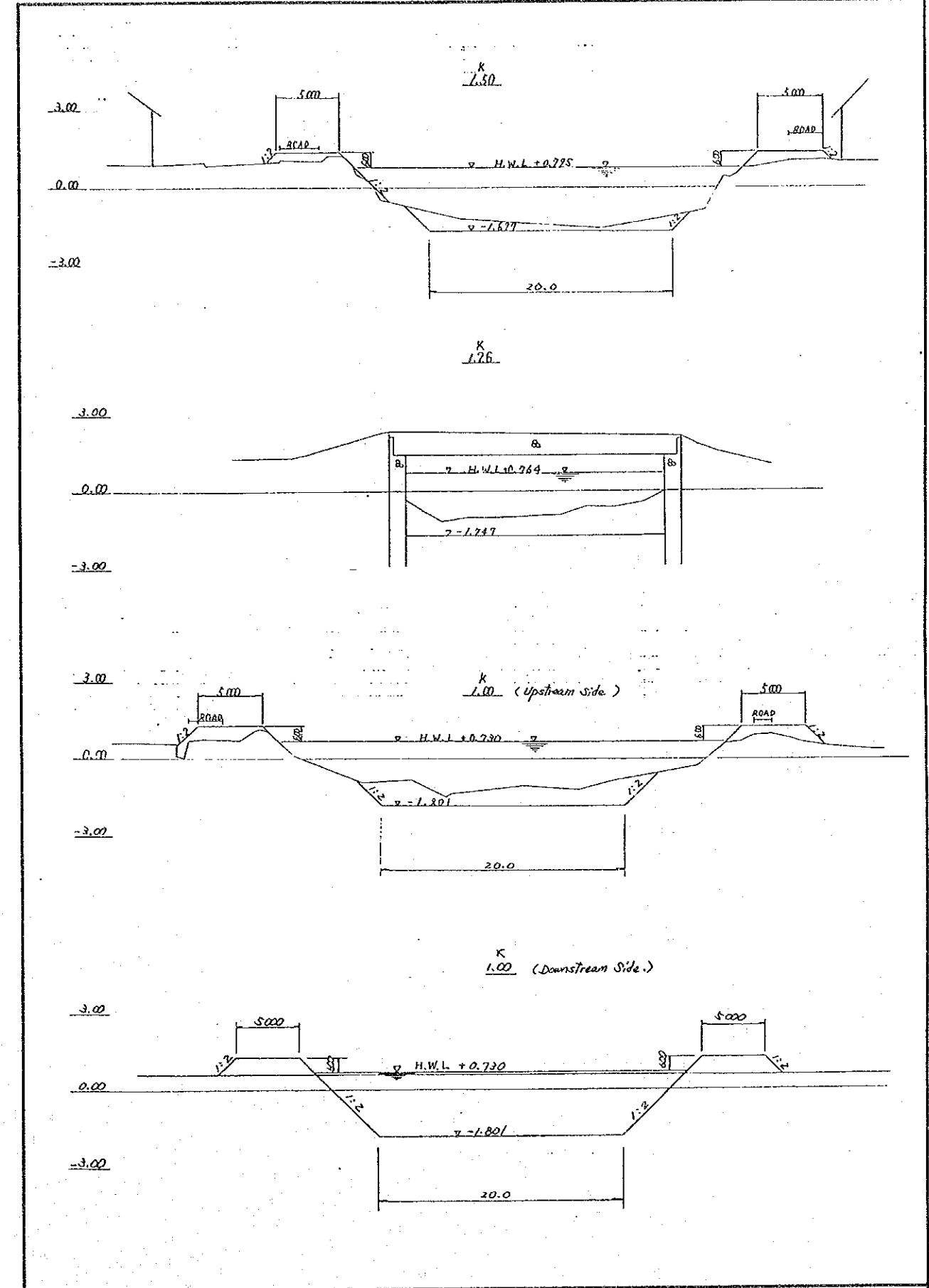
## Asin River

Station No.	Distance (m)	Excavation		Filling		Sodding								
		Area(m <sup>2</sup> )	Ave. (m <sup>2</sup> )	Volume(m <sup>3</sup> )	Area(m <sup>2</sup> )	Avg(m <sup>2</sup> )	Volume(m <sup>3</sup> )	Slope(m)	Avg(m)	Area(m <sup>2</sup> )				
k0.00		32.4				0.0								
k0.50	500.0	32.4	32.4	16200.0		0.0	0.0	0.0		0.0	0.0	0.0		0.0
k1.00	500.0	20.1	26.3	13150.0		0.0	0.0	0.0		0.0	0.0	0.0		0.0
k1.20	200.0	44.0	32.1	6420.0		0.0	0.0	0.0		0.0	0.0	0.0		0.0
k1.30	100.0	44.0	44.0	4400.0		0.0	0.0	0.0		0.0	0.0	0.0		0.0
Total	1300.0			40170.0				0.0						0.0



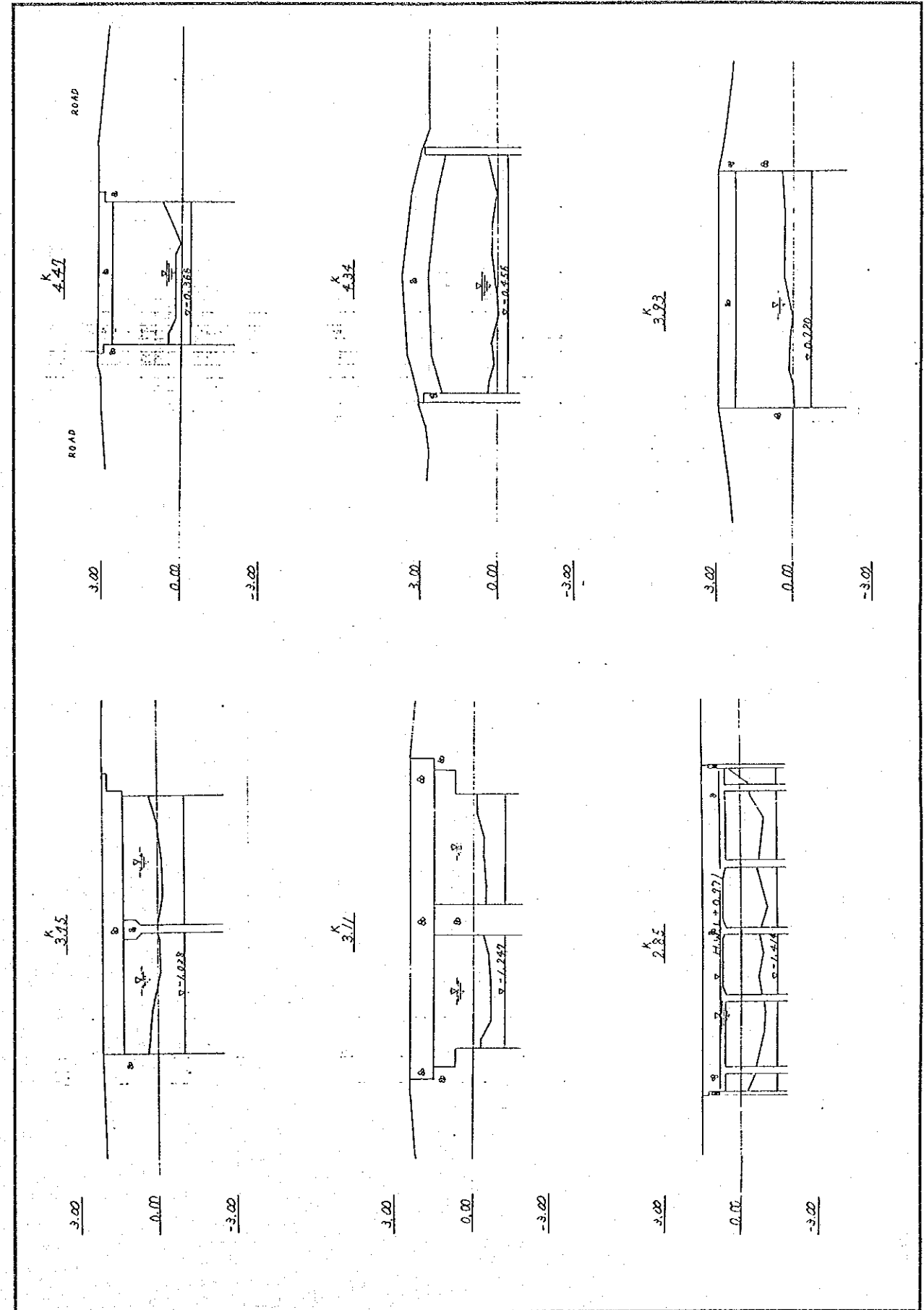
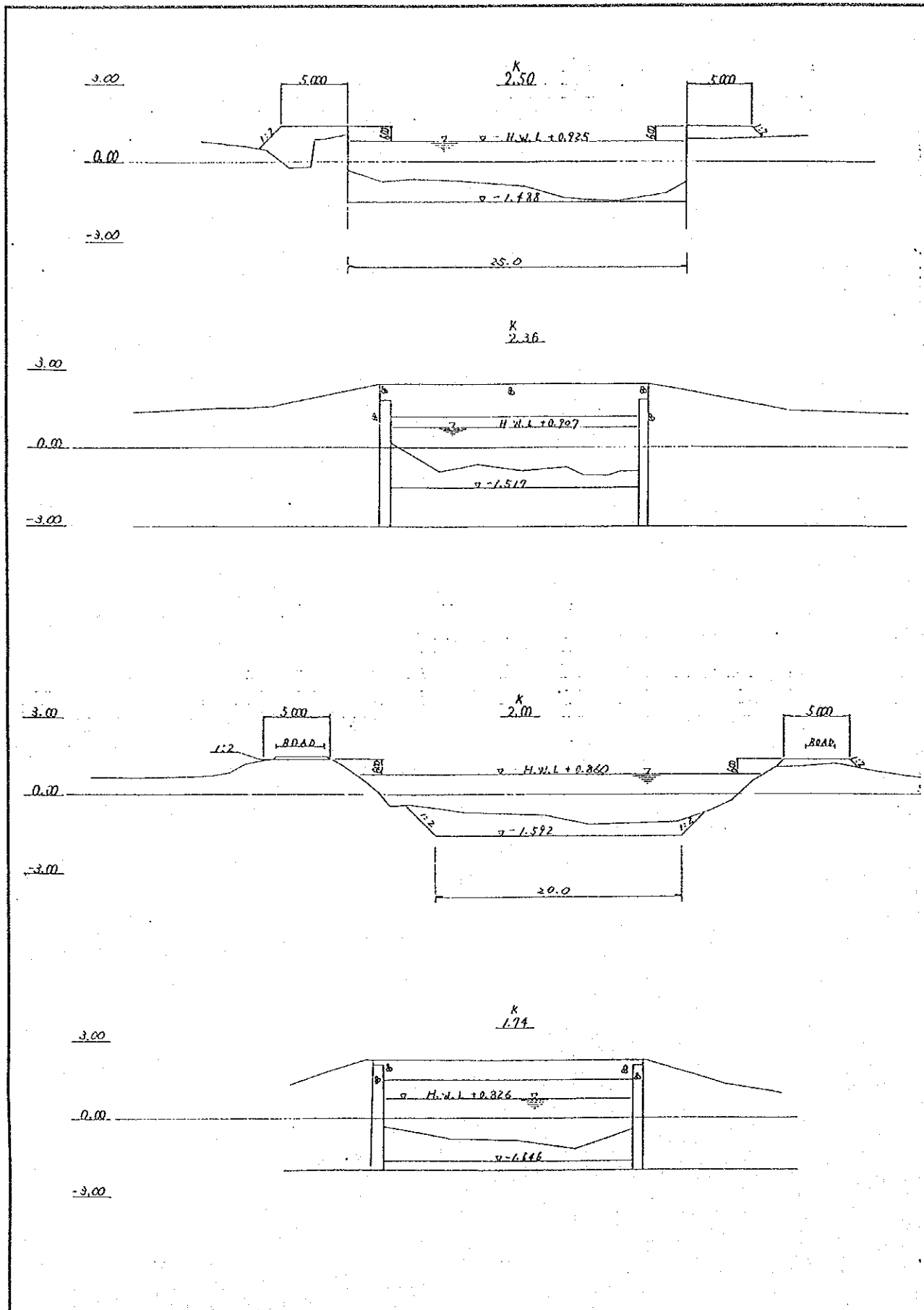
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 FEASIBILITY STUDY FOR URGENT FLOOD CONTROL AND  
 URBAN DRAINAGE IN SEMARANG CITY AND SUBURBS  
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Fig.1 (1/5)  
 CROSS SECTIONS OF SEMARANG RIVER



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Fig.1 (2/5)  
 CROSS SECTIONS OF SEMARANG RIVER

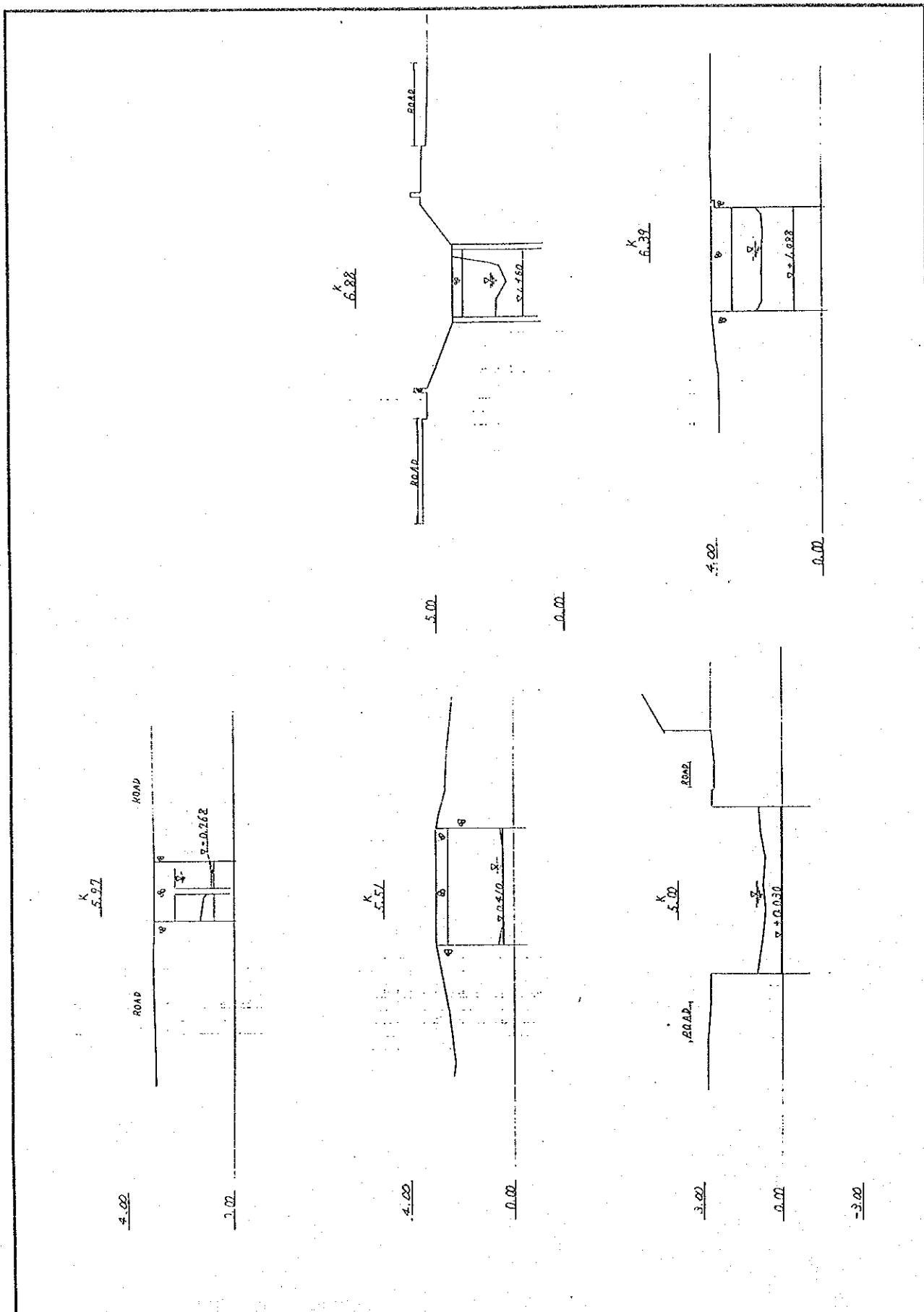


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Fig.1(3/5)  
CROSS SECTIONS OF SEMARANG RIVER

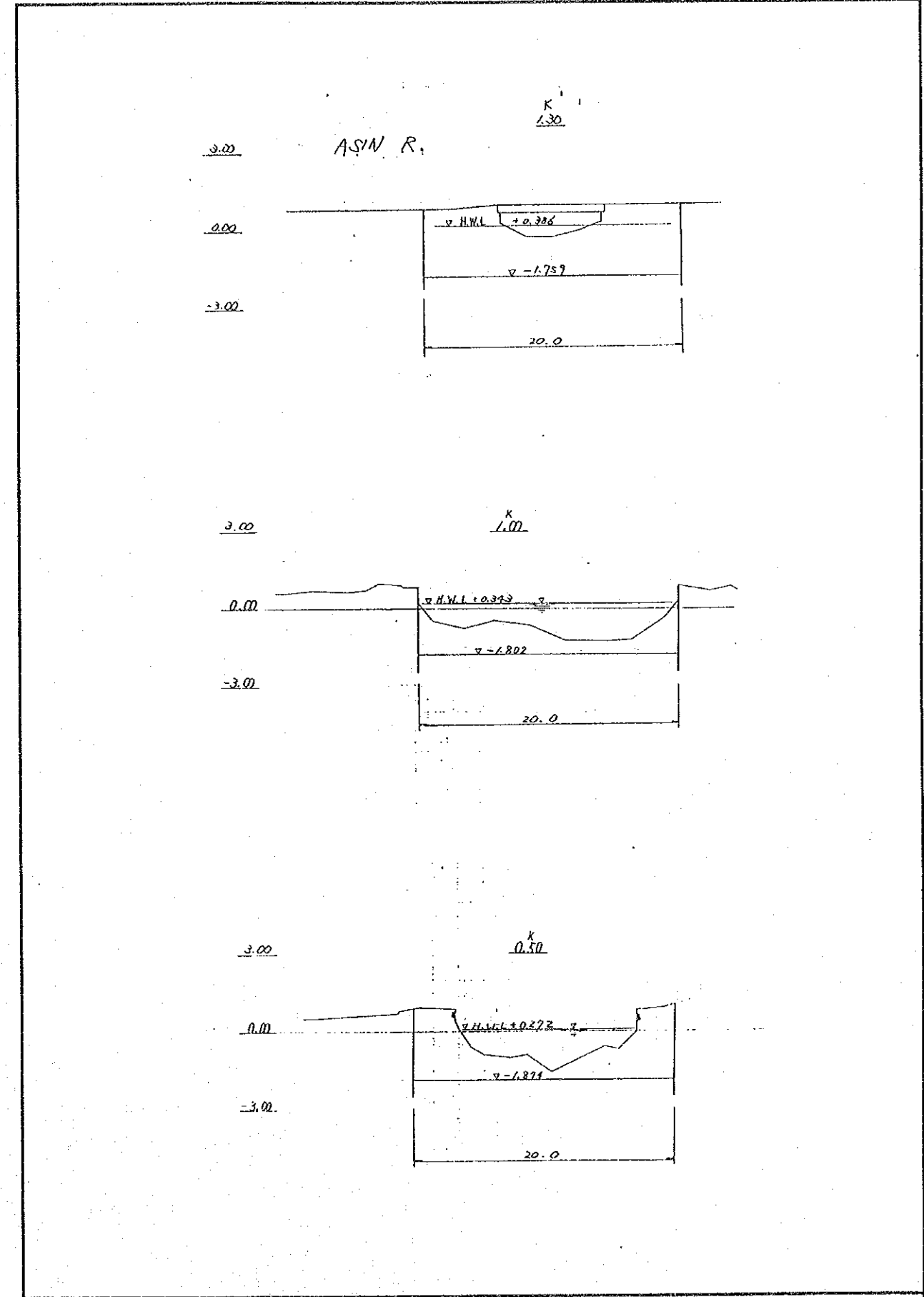
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Fig.1(4/5)  
CROSS SECTIONS OF SEMARANG RIVER



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Fig.1 (5/5)  
CROSS SECTIONS OF SEMARANG RIVER



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Fig.2  
CROSS SECTIONS OF ASIN RIVER