Appendix 3.8 Results of Geological Surveys

INTRODUCTION

In May 1994, Japan International Cooperation Agency (JICA) authorized Asian Engineering Consultants Co., Ltd (AEC) to carry out a site investigation for their project entitled Road Disaster Prevention Plan in Thailand (RDPP). The scope of investigation was limited to the following tasks:

- Drill exploratory boreholes within sliding mass at the specified locations.
- Perform standard penetration test (SPT) within soil mass and, collect soil and rock samples for laboratory testing.
- Conduct classification tests on soil samples.

II LOCATION

A total of 9 boreholes designated as BH 1 through BH 9 as summarized below were drilled. The location of each borehole is shown in Fig. 1(a) through Fig. 1(i). In the field, the exact location of each borehole was selected and marked by the JICA engineer prior to drilling at respective location.

BH 1	Route 1095	km 161.35
BH 2	Route 109	km 24.65
BH 3	Route 1149	km 5.80
BH 4	Route 1256	km 25.35
BH 5	Route 1256	km 40.15
вн 6	Route 1256	km 45.60
BH 7	Route 4	km 44.70
BH 8	Route 410	km 76.42
BH 9	Route 410	km 97.80

III FIELD INVESTIGATION

Soil Boring

Soil boring was performed to different depth at all locations except borehole BH 6 and BH 9 where rock coring only was performed. Drilling was carried out by rotary boring through the bottom of the borehole. Two rotary boring machines were employed to drill all boreholes. The hole was first advanced by a casing 1 m long, then by fish tail bit followed by jetting water pumped through the hollow drill rods and bit. Soil debris was then carried upwards by the raising water between drill rods and the borehole wall.

Standard Penetration Test

Standard penetration test (SPT) conforming to ASTM D 1586-84 was performed in all boreholes drilled through soil material. The test was performed at an interval of 1 m. The test was carried out by driving a split spoon barrel of 50.8 mm outside diameter and 34.9 mm internal diameter into the soil by a 63.5 kg free falling weight through

a distance of 760 mm. Number of blows were recorded for every 150 mm penetration until either 450 mm have been penetrated or 50 blows applied. The sum of blows for second and third 150 mm penetration is termed as the standard penetration resistance (SPT-N).

Disturbed soil samples from standard penetration tests were collected for subsequent laboratory testing and kept in polythene bags to retain natural water content.

Rock Coring

Rock coring was performed exclusively for two boreholes (BH 6 and BH 9) located along highway route 1256 in the North and route 410 in the South. In another two locations namely, BH 1 and BH 2 rock coring followed the soil boring.

Rock coring was accomplished using triple tube core barrel (NW) and diamond bit to permit maximum recovery of all core. The bit used was NMLC (76 mm outside diameter) and core size obtained was 54 mm diameter.

Rock cores thus obtained were placed in wooden core boxes for preservation until transportation to the laboratory.

Following the instruction of JICA, the actual amount of drilling carried out by AEC compared to the amount of drilling as stipulated in contract is summarized below:

Bore Route Hole No No		km	Contr drilling		Actual dri	lling	Remarks
	······································		Soil	Rock	Soil	Rock	
вн 1	Route 1095	km 161.35	5	5	10.50	6.50	Vertical
BH 2	Route 109	km 24.65	5	5	8.52	4.28	Vertical
BH 3	Route 1149	km 5.80	10	- '	10.45	_	Vertical
BH 4	Route 1256	km 25.35	5	5	13.95	1.50	Vertical
BH 5	Route 1256	km 40.15	5	5	21.58	•	Vertical
BH 6	Route 1256	km 45.60	5	5	•	15.00	80° from vert.
BH·7	Route 4	km 44.70	10	-	15.45		Vertical
BH 8	Route 410	km 76.42	5	5	10.05	•	Vertical
BH 9	Route 410	km 97.80	5	5	7	5.00	75° from vert.
Total	drill meterage		55	35	90.50	32.28	

Installation of Open Stand Pipe

A total of 7 open stand pipes (38 mm diameter) were installed at the bottom of the each borehole except for borehole BH 6 and BH 9 which were drilled at an inclination of 80 degree and 75 degree respectively from the vertical. At borehole BH 1, galvanized steel pipe with perforation at bottom 1 m was installed whereas the remaining 6 stand pipes were of PVC type with bottom 1 m perforation.

IV LABORATORY INVESTIGATION

Laboratory investigation was carried out on the representative samples of disturbed soil obtained from boreholes. The number and type of test was decided by JICA. Following type of laboratory tests were performed on selected samples:

	Natural water content	100	ASTM D 2216-80
•	Particle size analysis		ASTM D 422-63
•	Atterberg limits		ASTM D 4318-84

The results of laboratory investigation are summarized in Table 1.

TABLE 1
RESULTS OF LABORATORY TESTS

Location		km	161.3	5		Rout	e: 1	095		Borer	ole :	BH 1
Sample	Depth	W,	Atter	berg	Limits			Sieve	Anal	ysis		USCS
No.	(m)	(%)		(%))	% J	Passin	g (US	stan	dard s	ieves)	Group
			LL	PL	PI	3/8 *	# 4	# 10	# 40	# 100	# 200	
SS - 2 3	.00 - 3.45	14.0	33.0	20.8	12.2	97	88	81	72	57	51	CL
SS - 4 5	.00 - 5.45	14.3	34.0	20.2	13.8	76	63	55	48	41	36	SC
	.00 - 9.45	6.9	Insufi	icient	sample	47	31	22	17	14	12	GC?

Locatio	n :	km ₂	4.65			Rout	e : 1	09		Boreh	ole :	BH 2
Sample No.	The Street of Transportation of the Company	₩. (%)		berg (%)	Limits	200000000000000000000000000000000000000			Anal stan	ysis dard si	eves)	USCS Group
140.	(M)		LL	PL	PΙ	3/8 *				# 100		
SS - 3	3.00 - 3.45	22.4	Insuft	icient	sample	100	96	77	50	40	37	SC
SS - 4	4.00 - 4.45	27.7	50.2	34.4	15.8	100	98	81	45	31	• 27	SC
SS - 5	6.33 - 6.75	21.7	49.1	38.2	10.9	100	, 98.	85	54	41	36	SC
SS - 6	7.00 - 7.45	19.7	46.5	37.1	9.4	100	, 99	85	54	42	36	SC
SS - 7	8.00 - 8.45	18.0	44.9	31.8	13.1	100	98	81	43	28	23	SC

Locatio	n : kπ	n 5.8	0	Ten.		Rout	e : 1	149		Boreh	ole :	внз
Sample No.		W. (%)	J. 1000 1977 198	berg (%	Limits		a laboration of the second	1.00	Anal stan	ysis dard s	ieves)	USCS Group
.,	, v		LL	PL	⊗PI ∘	3/8	# 4	# 10	# 40	# 100	# 200	
SS - 3	3.00 - 3.45	29.5	36.7	27.4	9.3	100	96	83	64	40	30	SC
SS - 4	4.00 - 4.45	35.8	42.4	30.4	12.0	100	95	93	75	59	52	CL
SS - 6	6.00 - 6.45	21.8	30.4	26.3	4.1		100	84	44	29	23	SM-SC
SS - 7	7.00 - 7.45	16.2	35.4	31.5	3.9	100	100	90	63	46	39	SM-SC
SS - 8	8.00 - 8.45	24.8	35.9	27.1	8.8		100	88	63	45	37	SC

TABLE 1 (Continued)

RESULTS OF LABORATORY TESTS

Locatio	on :	km	25.35	5		Rout	te : 1	1256		Boreh	ole :	BH 4
Sample	Depth	W	Atter	berg	Limits			Sieve	Anal	ysis		USCS
No.												Group
			LL	PL	PI	3/8 "	#4	# 10	# 40	# 100	# 200	
SS - 2	2.00 - 2.45	30.0	37.6	27.4	10.2	100	98	95	. 86	50	40	SC
SS - 3	3.00 - 3.45	31.6	44.7	30.6	14.1			100	99	88	72	CL
SS - 6	6.00 - 6.45	27.5	38.2	30.4	7.8	100	99	96	89	83	.76	CL-ML

Location	n : k					Rout		1256		Boret	role :	BH 5
Sample	Depth	W	Atte	berg	Limits	6 W. 1941		Sieve	Anal	ysis		USCS
No.	(m)	(%)		(%))	%	Passin	ig (US	stan	dard s	ieves)	Group
			LL	PL	PI	3/8 "	# 4	# 10	# 40	# 100	# 200	
SS - 2	2.00 - 2.45	31.1	40.6	32	8.6			100	88	77	73	CL
SS - 6	6.00 - 6.45	26.7	40.5	24.3	16.2	100	99	99	94	85	79	CL
SS - 9_	9.00 - 9.45	25.3	38.7	29.6	9.1	100	88	76	. 57	50	46	SC
SS - 13	13.00 - 13.45	24.4	39.1	28.7	10.4			100	89	73	73	CL

Location	n : k	m 44.	70	<u> </u>	<u> </u>	Rout	e: 4	4		Boreh	iole :	BH 7
Sample	Depth	W.	Atter	berg	Limits			Sieve	Anal	ysis		USCS
No.	(m)	(%)		(%))	%	Passin	g (U	stan	dard s	ieves)	Group
			LL	PL	PI	3/8 *	#4	# 10	# 40	# 100	# 200	
SS - 2	2.00 - 2.45	14.2	46.5	32.8	13.7	99	76	71	60	57	57	CL
SS - 3	3.00 - 3.45	16.1	44.8	33.4	11.4	100	94	91	82	73	72	CL
SS - 4	4.00 - 4.45	10.7	44	33.2	10.8	92	71	62	48	43	41	SC
SS - 5	5.00 - 5.45	19.3	45.7	33.5	12.2	100	96	95	89	82	80	CL
SS - 9	9.00 - 9.45	12.1	44.9	30.6	14.3	100	99	Sept.	90	84	82	CL

Location	n :	km	76.	42	441		Rout	te:	410		Boreh	ole :	BH 8
Sample	Dep	th	W.	Atter	berg	Limits			Sieve	Anal	ysis		USCS
No.	(m)	(%)		(%)	%	Passin	g (US	stan	dard si	eves)	Group
											# 100		
SS - 4	4.00 -	4.45	15.3	37.8	24.0	13.8	71	57	41	25	21	19	SC
SS - 7	7.00 - 1	7.45	10.2	40.9	26.3	14.6	72	54	44	29	24	22	SC

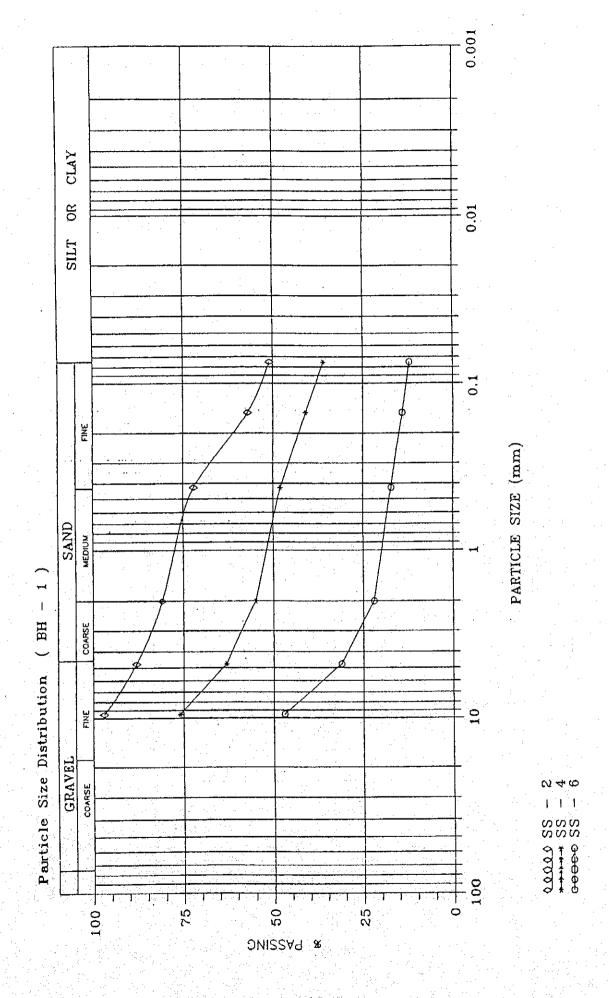
Source: Geo-Technology Consultants Co., Ltd.

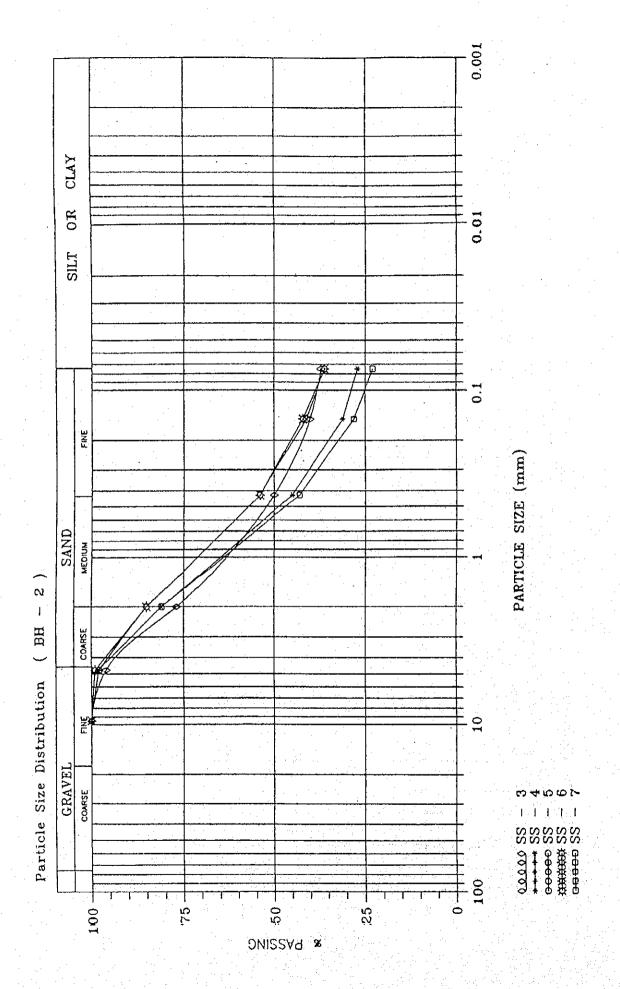
Notes:

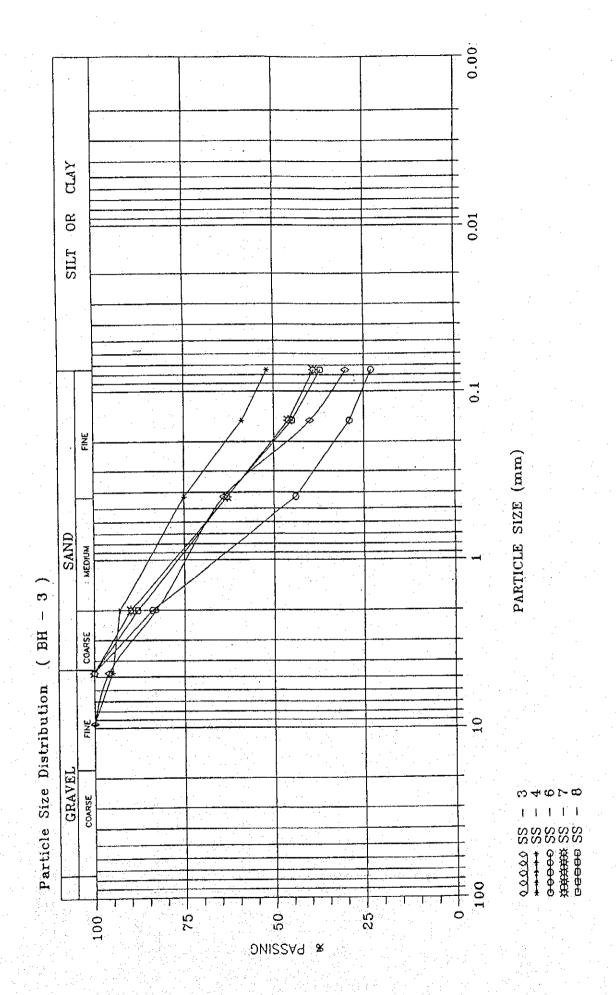
Wa = Natural water content; LL = Liqui

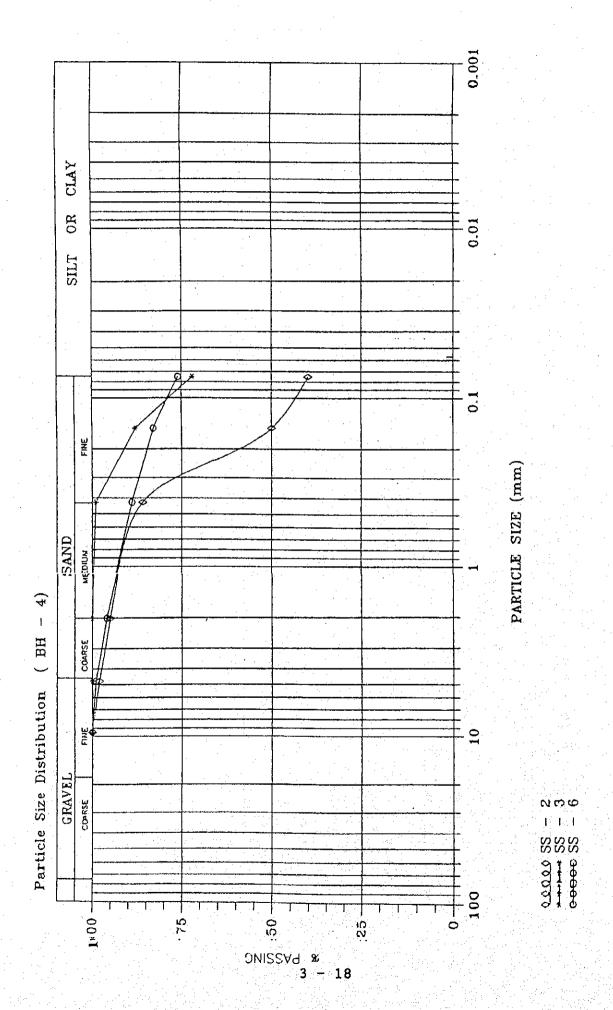
Natural water content; LL = Liquid limit; PL = Plastic limit; PI = Plasticity index

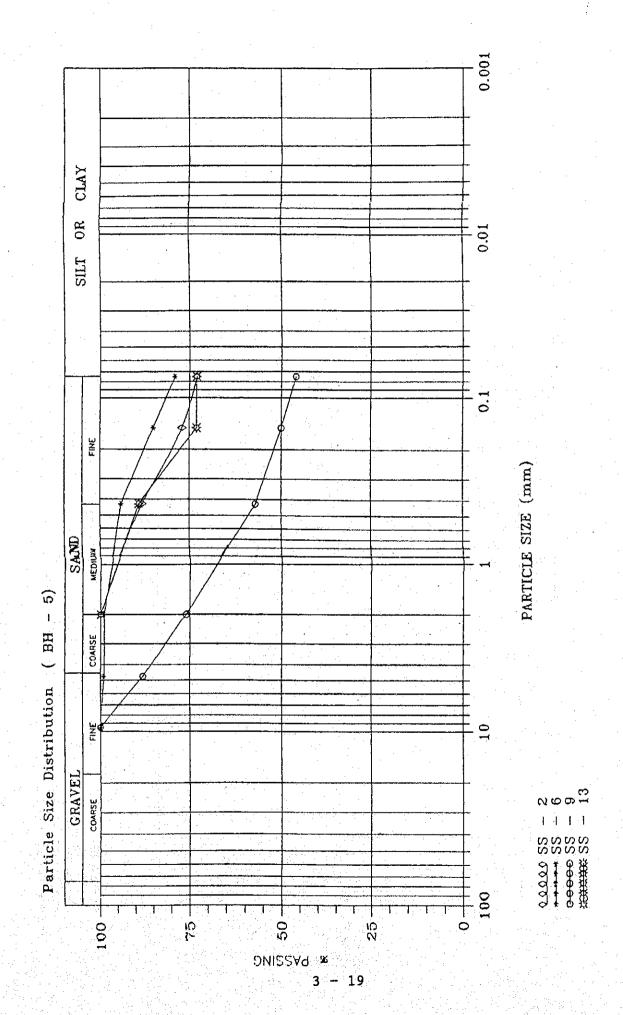
= Unified Soil Classification System SS = Split Spoon Sampl

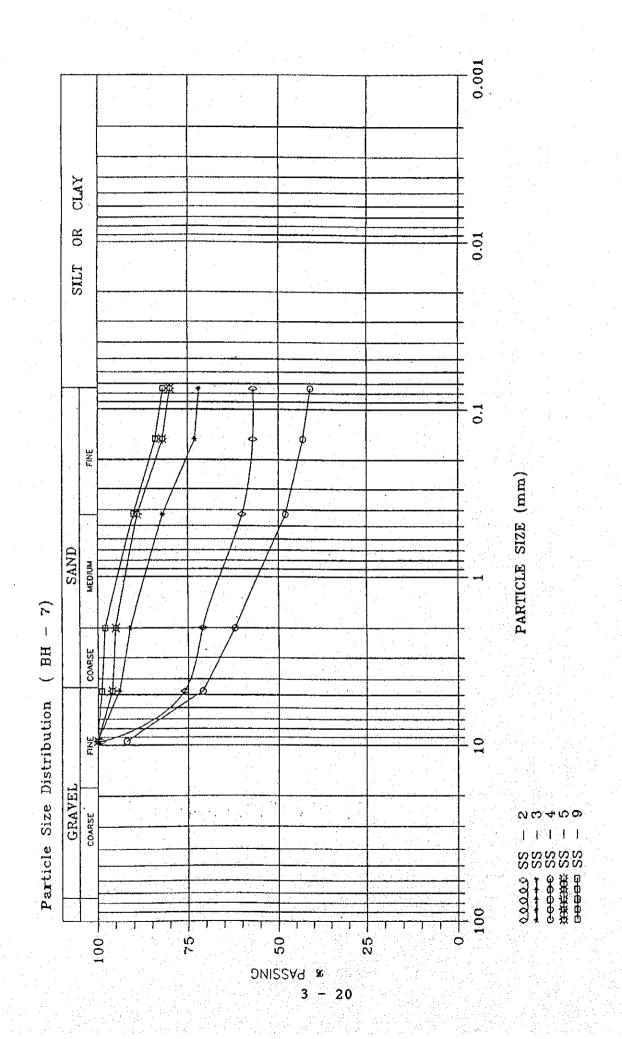


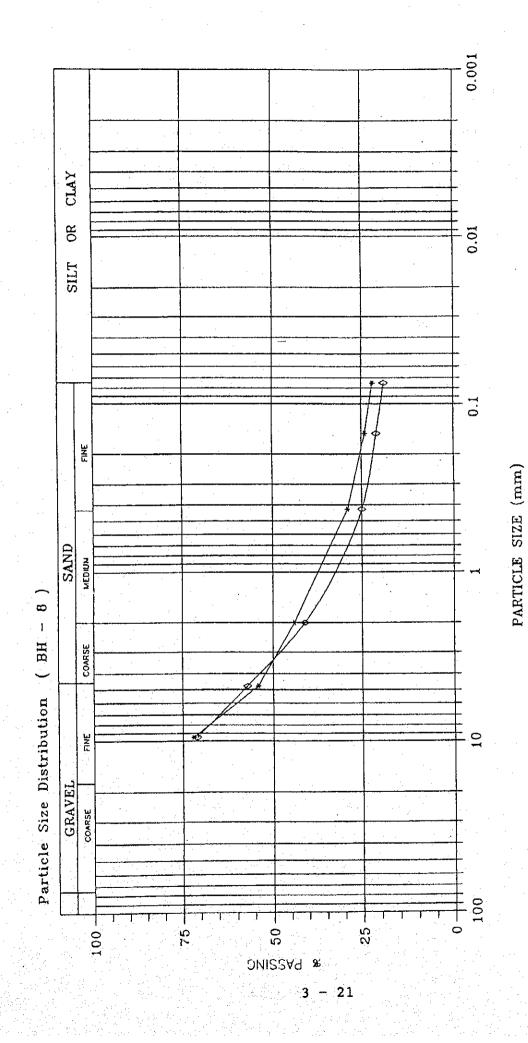












00000 SS - 4

All information on the soil and rock as to their physical property were recorded for every test depth in each borehole. Position of water table as measured 24 hours after boring is also recorded. Material properties as obtained from field and laboratory investigation were plotted against depth for each borehole. Borehole logs with relevant material properties incorporated in them are presented in Appendix - A.

The photographic records of important site activities and photographs of rock core samples are presented in Appendix - B.

Borehole:

BH-1

Project number: 61.35 Left Vertical scale:

4017

Site location:

Rt 1095 km 161.35 Left

1:50

Co-ords (x,y):

Elevation [m]:

121.50

G.W. Table [m]: Date finished:

10.70 23-05-94 Date started: 16-05-94 Total depth [m]: 17.00

S У m Rec-SPT - N Lab tests ь Description Depth overy Ö [m] w (1.PL 1961 0.00 Stiff sandy clay, reddish brown, low plasticity, some completelyweathered sandstone fragments-(Colluvium) 1.00 X|ss-1 2.00 SS-2 3.45 Medium clayey sand greyish brown, low plasticity, some 4.00 completely weathered sandstone fragments (Colluvium) sandstone boulder encountered at 6.45 mdepth 5.00 X∣ss-4 SS-5 6.45 Hard sandstone boulders, moderately to highly weathered, 7.00 light grey Dense clayey gravel, brownish

Borehole BH-1 Page 2 /Cont. y m Rec-SPT - N Lab tests Depth b Description overy [m]0 0 (%100 grey, some plastic clay X∣ss-6 10.50 Shale, moderately weathered, grey to dark grey, 2 joint sets-45 and 60 degree form vertical Hard sandstone, moderately tohighly weathered, light grey, 4 SS-7 12.00 joint sets 10,10,40 and 80 degree, form vertical irregular micro fracture, fe-oxide and clay staining on joint planes 13.23 Hard sandy clay, brownish grey 14.87 Medium to hard sandstone, moderately to highly weathered, grey to dark grey, 3 sets 30,60 and 80, Fe-oxide and clay 16.00 SS-8 staining joint planes End of Borehole at 17.00 m Atterberg limits

Borehole:

BH-2

Rt 109 km 24.65 Left

Co-ords (x,y): G.W. Table (m):

Site location:

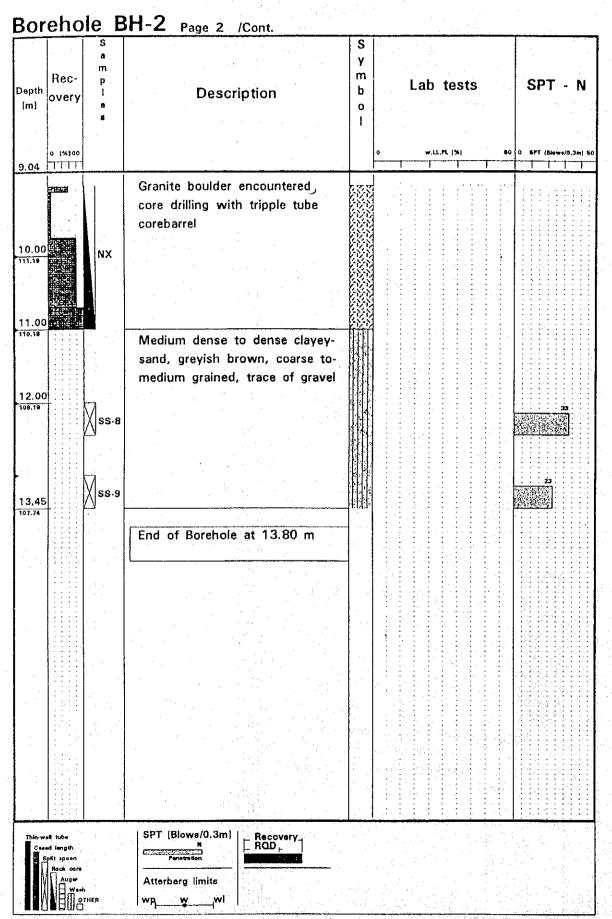
3.30 04-06-94 Project number:

4017 Vertical scale: 1:50

121.19 Elevation [m]:

Date started: Total depth [m]: 25-05-94 13.80

Date finished: Y m m SPT - N Lab tests Recb Description Depth overy (m) 0. [%] 00 0.00 | 1111 Soft silty clay, reddish brown,medium plasticity, trace of finegravel X|ss-1 1.45 Medium dense clayey sand, browish grey to reddish brown, 2.00 coarse to medium grained, trace SS-2 of gravel 3.00 4.15 ⊠ss-4 Granite boulder encountered core drilling with tripple tube core barrel 5.00 NX 6.33 Dense to very dense clayey SS-5 sand, greyish brown, coarse tomedium grained, trace of gravel 7.00 8.90



Borehole: Site location:

Rt 1149 km 5+800

G.W. Table [m]: Date finished:

Co-ords (x,y):

2.60 09-06-94

Project number:

4017

Vertical scale:

1:50

Elevation [m]: Date started:

113.50

05-06-94

Total depth [m]: 10.45

Depth [m]	Rec- overy	S a m p I e s	Description	S y m b o	Lab tests	SPT - N
0.00	0 1541,00				0 W,LL.PL (%) 5	0 0 SPT [Blows/0.3m] 50
113.50		 ss-1	Medium gravelly clay, reddish brown, medium plasticity			<u></u>
2.00		SS-2	Stiff to very stiff sandy clay, clayey sand greyish light brown, low to medium plasticity			-11
3.00		Ss-3				11
4,45 100,06 5,00 100,50			Medium dense to dense silty sand, medium grained, greyish-brown			14.
6.00		Ss-6			• 1	18
7.00		SS-7			•	27
8.00 106.60 9.00		ss-e		4.14.11.11.11.11.11.11.11.11.11.11.11.11		34

Borehole BH-3 Page 2 /Cont. y m Rec-SPT - N Depth overy Lab tests Description (m) 9.04 e-aa SS-10 10.45 End of Borehole at 10.45 m

ROAD DISASTER PREVENTION PLAN (RDPP)

Borehole: Site location: BH-3

Rt 1149 km 5+800

Project number: Vertical scale: 4017 1:50 113.50

Co-ords (x,y): G.W. Table [m]:

2.60

Elevation [m]: Date started:

05-06-94

Date finished: 09-06-94

Total depth [m]: 10.45

	Rec-	S a m		S y m	Lah	tests		SPT - N
Depth (m)	overy	1	Description	b o	Lau	16313	·	
		5		1				:
	0 [563.00				o w,L	L,PL [%]	50	0 SPT [Blows/0.3m] 60
0.00			Medium gravelly clay, reddish					
			brown, medium plasticity					
		SS-1						8
1.45		M 33-1	Stiff to very stiff sandy clay,					
2.00			clayey sand greyish light brown,					144
111.50		SS-2	low to medium plasticity					
		7.7	racktriangle					
3.00							: :	<u>iu</u> : : : : : : : : : : : : : : : : : : :
110.60		SS-3				141		
 							<u>.</u>	18
4.45		SS-4	24					
			Medium dense to dense silty sand, medium grained, greyish-	1				
5.00	 	SS-5	brown					14
		M.33.3		-			: :	
6.00								18
107.50	T	ss-e		¥.		• 1		
		ובץ						
7,00	_							37
, ,,,,,,		Ss-7				H		12450457
				1				
8,00 105.60	+	\square				•		34
		∭ss-€						
9.00								

Borehole BH-3 Page 2 /Cont. S у m m Rec-SPT - N Lab tests Depth Description b overy [m] 0 w,LL,PL (%) 0 (%)00 9.04 THIT 184.60 X ss-9 SS-10 10.45 End of Borehole at 10.45 m

Borehole:

BH-4

Project number:

4017

Site location:

Rt 1256 km 25.35 Left

1:50

Co-ords (x,y):

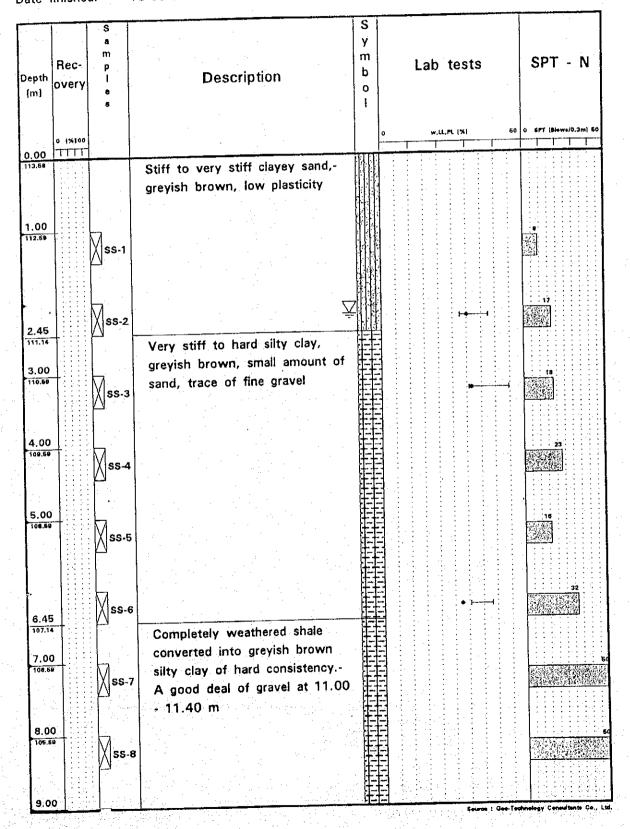
Vertical scale: Elevation [m]:

113.59

G.W. Table [m]: Date finished: 2.20 13-06-94 Date started:

10-06-94

Total depth [m]: 15.45



Borehole BH-4 Page 2 /Cont. S γ m m Rec-SPT - N Lab tests Depth þ Description overy (m) 0 1%500 9.04 104.59 X|ss-9 10.00 X|ss-1¢ 11,00 102.58 12.00 13.00 SS-13 14.00 |X||ss-1 S5-15 15.45 End of Borehole at 15.45 m SPT [Blows/0.3m] Recovery_ ROD Atterberg limits

ROAD DISASTER PREVENTION PLAN (RDPP)

Borehole: Site location: **BH-5**

Co-ords (x,y):

G.W. Table [m]: Date finished:

Rt 1256 km 40.15 Left

12.03 06-06-94 Project number: Vertical scale:

4017

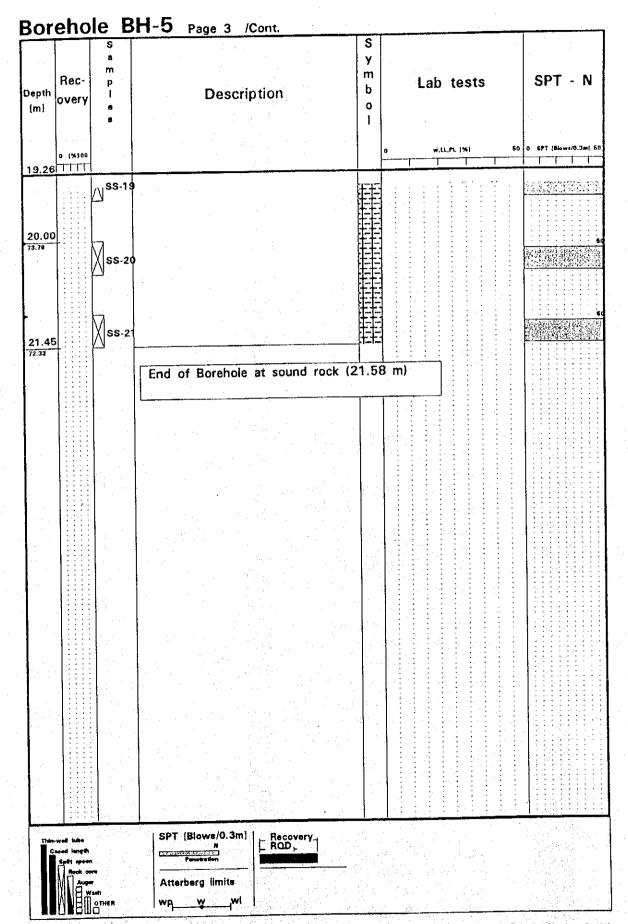
Elevation (m): Date started:

1:50 93.78 31-05-94

21.58 Total depth [m]:

pth m]	Rec- overy	S m p l e s	Description	S y m b o	Lab	tests		SPT - N
	0 (%)00				0 w.L	L,PL (%)	60	0 SFT (Blows/0.3m) 6
	1111			6000	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \			
72		ļ	Fill material, filled with slide					
]	debris of completely weathered-					
			dark brown siltstone,					
00 70		L,	uncompacted.					4
./=		SS-1				:	: :	
		K)						
						: : :		
00								a ::::::::::::::::::::::::::::::::::::
.71		SS-2				*	⊣ :	
		M						
		1 1			8 : : : :			
.00								
.79	†	M_{-}		***		: : :	: :	<u> </u>
•	1::::	SS-3			8		: :	
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.00				****				
2.78	+ : : : :	\square				: : :		5 (i)
		SS-4						
				***		: : :	: :	
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8.78	+:::::	ka i		***			1 .:	5
		SS-5						
1.	1::::						: :	
7.78	4 : : :	ka		***	8	: : :		13
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7.00 4.78	4	<u> </u>		₩	X : : : :			•
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3,00								
6.78		∭ss-8	Hard Control of the Control					
		M.22.0					: :	MAN
				8		: :: : ;		
9.00				**	\otimes			hnology Censultants Ce.

Donah	Rec- overy	S a m p l e	H-5 Page 2 /Cont. Description	S y m b o	Lab tests	SPT
10.00 83.78 11.00 82.78 12.00 81.78			Completely weathered siltstone,-	HINTER CONTRACTOR CONT		21
16.00 77.78 17.00 76.78 18.00 76.76		SS-16				



ROAD DISASTER PREVENTION PLAN (RDPP)

Source : Gov-Technology Consultants Co., Lts

Borehole:

BH-6

09-05-94

Site location:

Rt 1256 km 45.6

Co-ords (x,y): G.W. Table [m]:

Date finished:

Project number:

Vertical scale:

4017 1:50

Elevation [m]:

97.74

Date started:

07-05-94

Total depth [m]:

15.00

		S		S			
į		m	·	У			
	Rec-	Р	· ·	m	Lab tes	ts	SPT - I
epth	overy	1 .	Description	b	Lab (63		011 - 1
[m]				0			
		•		11	:		
0.00	0 (%)00				0 w,tk,PL (%)	60	0 GPT (Blows/0.3m
7.74	ENGINE.	- 	Bd. Ji pandatana hisblu	TT		: : :	
			Medium sandstone, highly				
	運搬		weathered, reddish brown, 4	100			
			joint sets 70,30,45 and 10				
1.00			degree from core axis, fe-oxide-	11			.
6.74			and some clay stain on joint				
			planes				
2.00						: : :	
6.74			Hard sandstone, moderately				
			weathered, reddish brown,			i. i i	
			coarse to medium grained, 3			: : :	
			joint sets 10,30 and 70 degree				
	墨		from core axis, fe-oxide and				
3.42			some clay staining on joint	يير			
			planes /	/			
3.85							
.5.55			Medium hard shale, highly	歴			
			weathered, reddish brown, 2				
	4		joint sets 30 and 70 degree			: : :	
į			from core axis, some parts				
5.00			decomposed to clay		} : : : : : : :		
92.74			\		} : : : : : :		
	4		Medium hard siltstone,				
	E 1	1	moderately weathered, reddish				
6,00			brown, 3 joint sets 10,10 and-				
91.74			70 degree from core axis, some				
		1	shale fragments embeded,	F			
			irregular cavity				
			Hiegulai Cavity				
						. : : :	
		NX	The second secon	莊			
7.80				#			
89.94			Medium hard sandstone, highly-	1			
			I to the state of				
			weathered, reddish brown, 3				
			joint sets 10,10 and 40 degree				
100			from core axis, coarse to	11	r in the below		Liniation

Borehole BH-6 Page 2 /Cont. m SPT - N Rec-Lab tests b Depth Description overy (m) 9.04 medium grained, fe-oxide and some clay staining on joint 9.55 planes Medium hard sandstone, moderately to highly weathered, reddish brown, 4 joint sets 10-,40,60 and 80 degree 11.00 12.00 13.00 Medium hard sandstone, highlyweathered, reddish brown, 3 ioint sets 10,40 and 70 degree 13.85 from core axis Medium hard siltstone, moderately to highly weathered, reddish brown, 2 joint sets 45and 70 degree from core axis Soft shale, highly weathered, reddish brown, 1 joint set 70 degree from core axis End of Borehole at 15.00 m SPT [Blows/0.3m] Recovery Note: BH-6 was drilled 80-Atterberg limits degree from vertical

ROAD DISASTER PREVENTION PLAN (RDPPI

Salate : Gen-Technology Consultante Co., Lt

Borehole:

BH-7

Site location:

Rt 4 km 44.7

Co-ords (x,y):

G.W. Table [m]: Date finished:

3.20

20-06-94

Project number:

4017

Vertical scale:

1:50

Elevation [m]: Date started:

25.75 18-06-94

Total depth [m]:

15.45

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N 1		a		y								
	Rec-	m.		m								
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(m)	overy		Description	0	- -	•				1.5		
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0.00	1111			<u> </u>		1	T	Ť				
25.76			Medium silty clay, greyish]		- [- [
			brown, low to medium plasticity				: :	: :	:	::::	# : : : :	::
Ī			, trace of sand		1							
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1.45	: : : :	SS-1		掛	1	: : :	: :	: :		ğ : : :		: :
24.30		<u> </u>	Stiff silty clay, greyish brown,	甘								: ; l
				F	1				:			
1			low to medium plasticity	 	1	: : :	: :	: :	- :			: :
:		SS-2		E]			: :	_,			
2.45	 			III]							
23,30			Very stiff silty clay, greyish	盽	1		: :		. :			: : [
3.00			brown, low plasticity		}	: : :	: :	1 :	: .			
22.75		\square	∇	FF]					ा । संस्थान		: :
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4.00	 	ka			1					12		
		X ss-4				•	: :	,	:			
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1				EE					- []		1411	
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]:::::	M		1	1				_;	10	:::::	: [
5.45		SS-5		胜]	•		-				
20.30			Very stiff silty clay, greyish	E								
6.00			brown, trace of fine sand	##	1	: : :						
19.76	<u> </u>	\(\begin{array}{c} \operatorname{\text{T}} \operat		胜						21		
		SS-6		H						43.0		
		تع		‡ ‡	1		: :	· : :		A CHARLES		
				H								
1				FE]							
		SS-7		 - -	1		1.1		: :	20		
7.45	1:::::	Д,		壯								
10.30			Hard silty clay, greyish brown,-	Æ	1							
			trace of fine sand	##	1		: :	. :				
	:::::	\square		##	}			· : :	: :	and Subsequence	33	
8.45		S-8a		E								
17.30	† : : : : : : : : : : : : : : : : : : :		Hard silty clay, greyish brown,	Œ						cradica	لكتك	
1.000			l	1							. 1. 1 1 2. 1. 193	
9.00	10 1 1 1	<u> </u>	low plasticity	<u>rt-t-</u>	1		1 . 1			logy Consul	tenti Cal	

Rec- Depth (m) 0 (%300	Description	S y m b o	Lab tests	SPT - N
9.04	Hard silty clay, greyish brown,- trace of fine gravel, low to medium plasticity Hard silty clay, greyish brown, low plasticity			30 30 30 30 30 30 30 30 30 30 30 30 30 3
Thirwal tabe Cood length Split speen Rock core Augur OTHER	SPT [Blows/0.3m] Recovery RQD RQD Recovery			

Borehole:

BH-8

Rt 410 km 76.42

Project number: Vertical scale:

4017 1:50

Site location: Co-ords (x,y):

Elevation [m]:

44.00

G.W. Table [m]: Date finished:

3.25 22-06-94 Date started:

21-06-94

Total depth [m]: 10.05

		S		S			
		a m		У			
	Rec-	P		m	Lab tests		SPT - N
Depth [m]	overy	·	Description	b			
12		8					
	0 (%)00				0 w,Lt,PL [%]	50	0 SPT (Blows/0.3m) 50
0.00			Stiff to hard silty clay, greyish-	1-1-			
			brown, low plasticity, mixed				
			with gravel, percentage of	#		: :	
1.00			gravel less than 10%				
43.00		SS-1					7
		Maari		H			
				H			
2.00	: : : : : : : : : : : : : : : : : : :			H			3
		X ss-2		##			4
				#			**************************************
3.00							
3.00		h	7	₩.			76
		SS-3		排		-	
4.00				盽			
40.00		SS-4		盽		<u>, i i i</u>	13
•		M334		盽			
				H			
5.00				H			24
33.11		Ss-5		H			
•							AND PARTY.
6.00				#			
38.00	 	М					27
		Se-e		H			BUILD BA
		_					
		ss-7		#			36
		Д.					ANSWERS OF
7.75 34.26	ļ i			#			
-		h	Hard silty clay, greyish brown,-	H			
		Ss-8	trace of fine gravel, low	H			
		للم	plasticity (Original ground ?)	揖			
9.00				H			

[m]	0 (%)00	S a m p I e	Description	y m b o	Lab tests	SPT - [
9,04 35.00		4		++		-
38.00		SS-9				
10.05	<u> </u>	== SS-1¢		11		
33.95			End of Borehole at 10.05 m			
	n-waii tuba Caeed length		SPT (Blows/0.3m) Recovery RQD,			
		other	Atterberg limits			

Borehole:

BH-9

Site location:

Rt 410 km 97.8

Co-ords (x,y):

G.W. Table [m]: Date finished:

0.00 26-06-94 Project number:

4017

Vertical scale: Elevation [m]: 1:50 35.00

Date started:

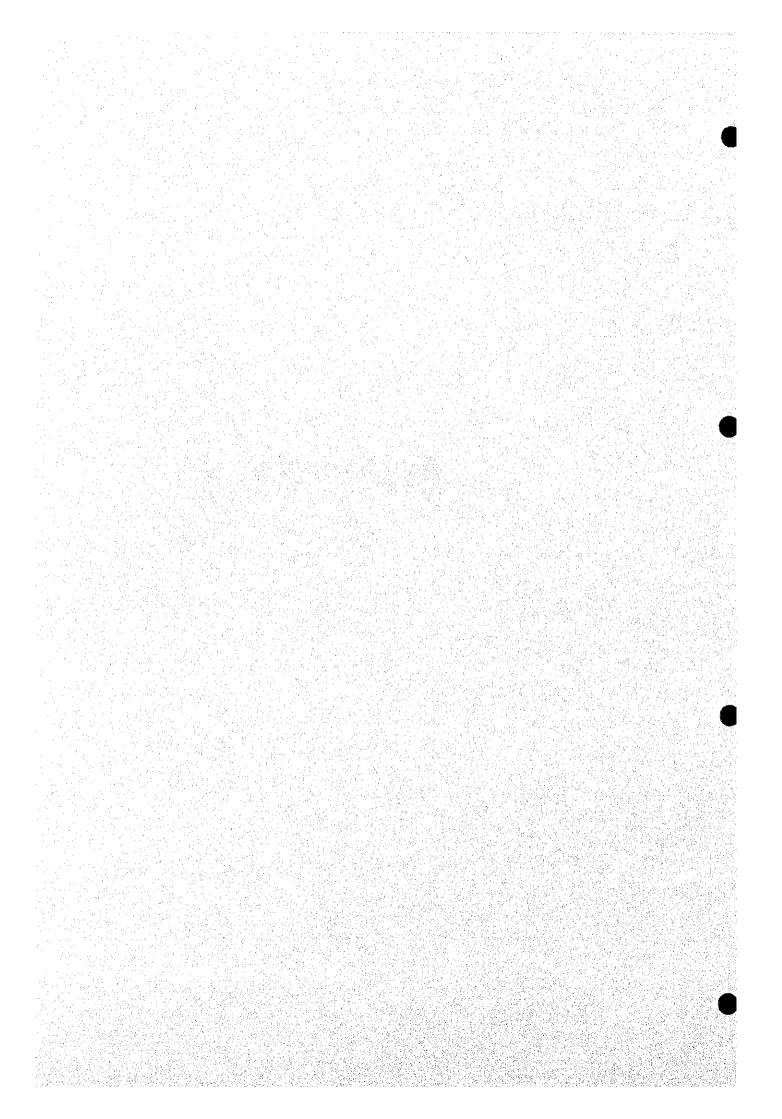
26-06-94

Total depth [m]:

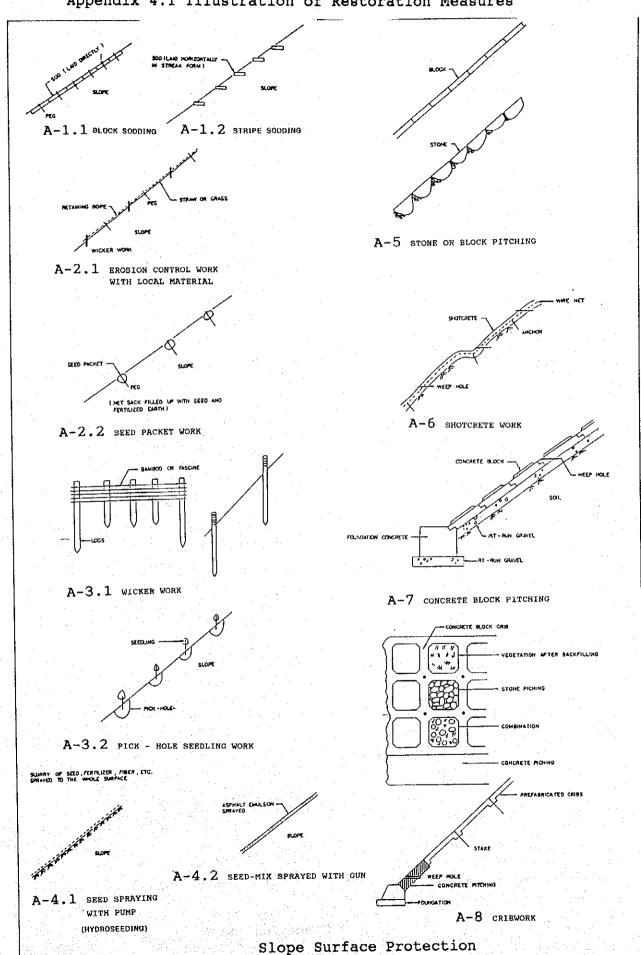
5.00

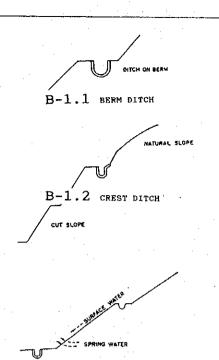
Depth [m]	Rec- overy	S a E p - e s	Description	S y m b o	Lab tests	SPT - N							
0.00	0 1%100				0 w.LL.PL (%) 60	0 SPT (Blows/0.3m) 60							
1.00			Medium hard schist, highly weathered, light grey, 3 joint sets 25,45 and 60 degree from core axis, Fe - oxide staining and clay filling in some joint										
3.50		NX		\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\									
5.00 30.00	The second of th									Hard schist, moderately to highly weathered, light grey to- grey, 3 joint sets 20,30 and 80 degree from core axis	\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\		
			End of Borehole at 5.00 m										
Car	all tube ed length pilt spoon Rock core Auger	нєя	SPT [Blows/0.3m] Recovery ROD ROD ROD Note: BH-9 was dridegree from vertical	lled 7	75-								

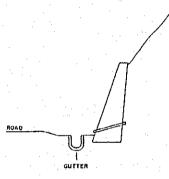
Appendix 4



Appendix 4.1 Illustration of Restoration Measures

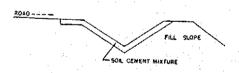




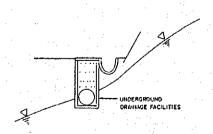


В-1.3 тое вітсн

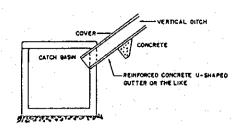
B-2.1 GUTTER



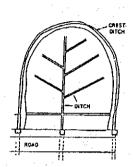
B-2.2 ditch made with soil cement



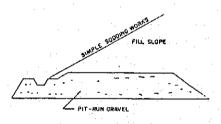
B-5.1 UNDERGROUND DRAINAGE WITH PIT/PIPE B-5.2 UNDERGROUND DRAINAGE WITH BLANKET



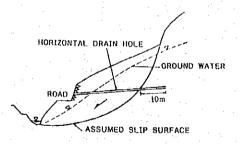
B-3 VERTICAL DITCH



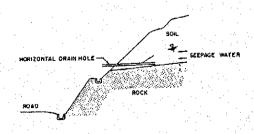
B-4 SURFACE DRAINAGE



B-6 DRAINAGE WITH PIT-RUN GRAVEL

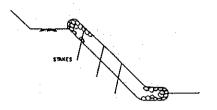


B-7.1 HORIZONTAL DRAIN HOLE

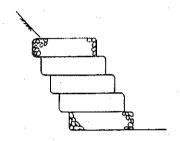


B-7.2 HORIZONTAL WEEP HOLE

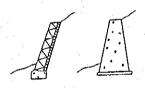
Slope Drainage

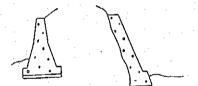


C-1.1 CYLINDER GABION

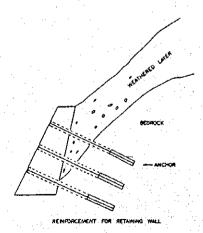


C-1.2 MAT GABION

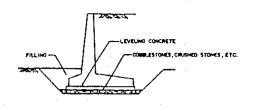




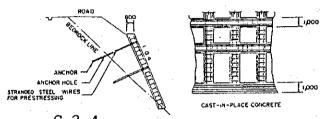
C-2.1 GRAVITY TYPE RETAINING WALL



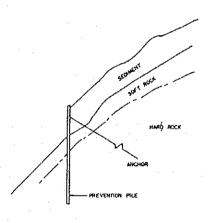
C-2.2 GRAVITY TYPE RETAINING WALL WITH ANCHOR



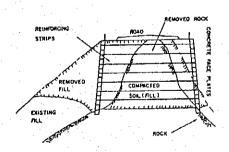
C-2.3 T-SHAPED RETAINING WALL



C-2.4 CRIB RETAINING WALL
C-2.5 CRIB RETAINING WALL WITH ANCHOR

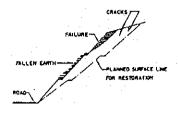


C-3.1 PREVENTION PILE
C-3.2 PREVENTION PILE WITH ANCHOR



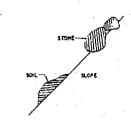
C-4 REINFORCED EMBANKMENT

Slope Support

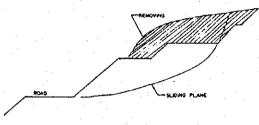


RESTORATION AFTER CRACKS AND FAILURE

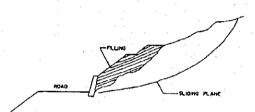
D-1 RECUTTING



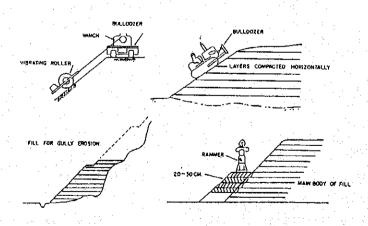
D-2.2 REMOVAL OF UNSTABLE MATERIAL



D-2.1 EARTH REMOVAL

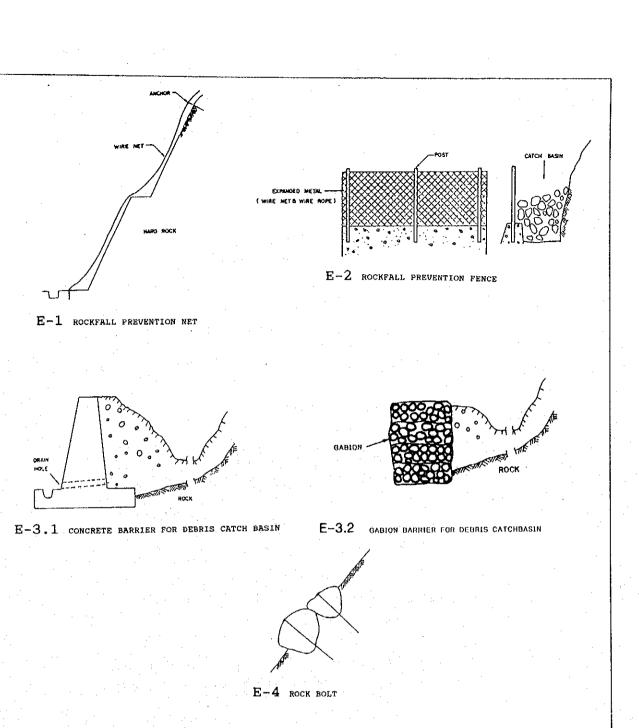


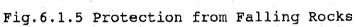
D-3 COUNTERWEIGHT FILL

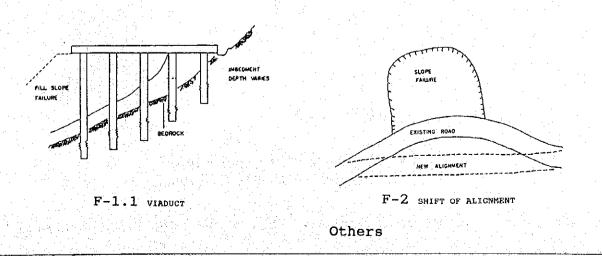


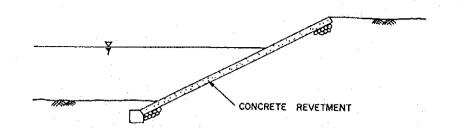
D-4 REFILLING WITH COMPACTION

Slope Stabilization by Earth Work

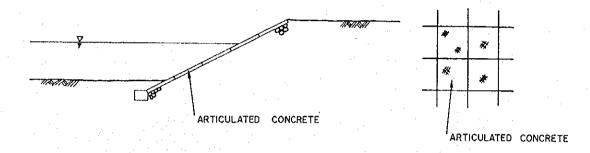




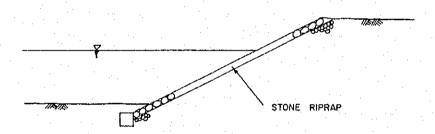




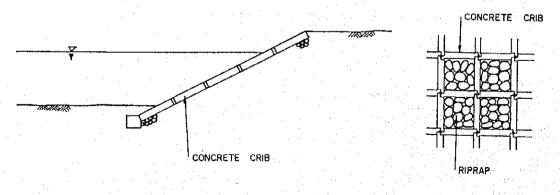
J-1.1 CONCRETE REVETMENT K-1.1 CONCRETE REVETMENT



J-1.2 ARTICULATED CONCRETE REVETMENT K-1.2 ARTICULATED CONCRETE REVETMENT

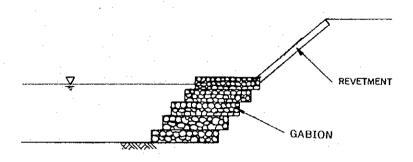


J-1.3 STONE RIPRAP REVETMENT (WITH MORTAR) K-1.3 STONE RIPRAP REVETMENT (WITH MORTAR)

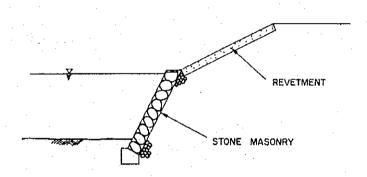


J-1.4 CRIBWORK WITH STONE RIPRAP K-1.4 CRIBWORK WITH STONE RIPRAP

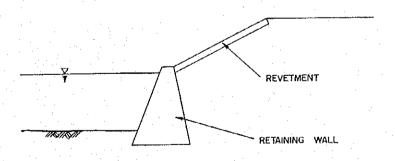
Securing of Abutment Fill Slopes and River Banks(1)



J-1.5 GABION

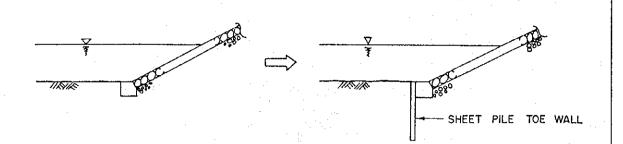


J-1.6 STONE MASONRY

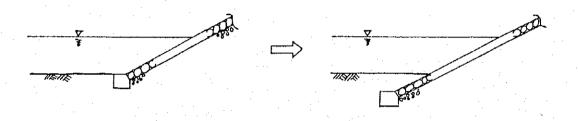


J-1.7 GRAVITY TYPE RETAINING WALL

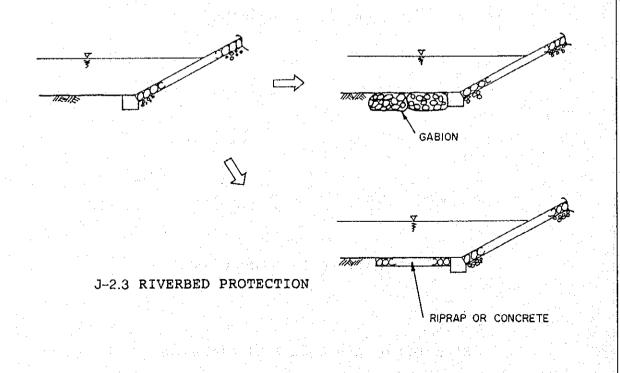
Securing of Abutment Fill Slopes and River Banks(2)



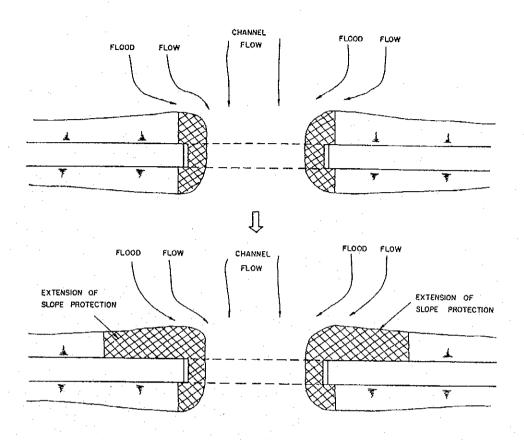
J-2.1 SHEET PILE TOE WALL



J-2.2 DEEP EMBEDMENT OF BASE

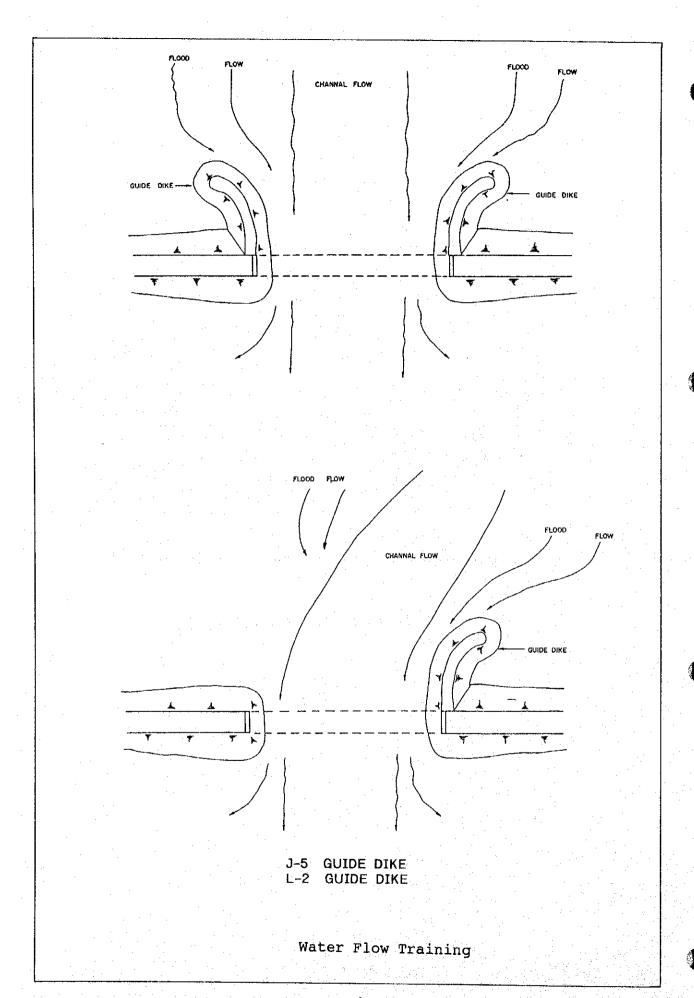


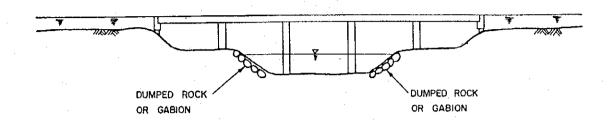
Improvement of Existing Revetments



J-4 EXTENTION OF PROTECTION ON UPSTREAM SIDE

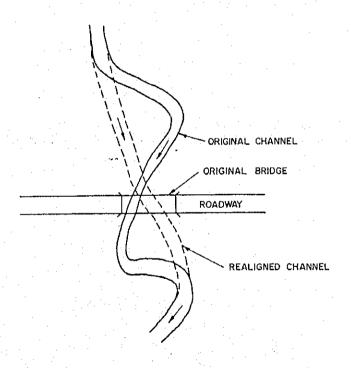
Extension of Fill-Slope Protection





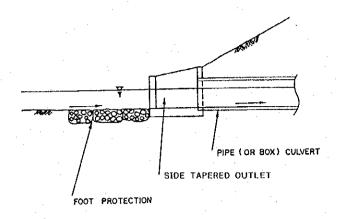
L-1.1 STABILIZATION OF THE STREAM BANK WITH DUMPED ROCK L-1.2 STABILIZATION OF THE STREAM BANK WITH GABION

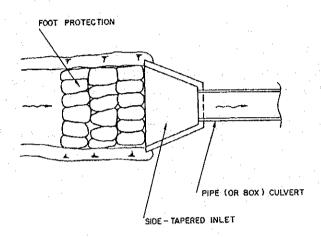
Fig. 6.1.12 Stream Bank Stabilization



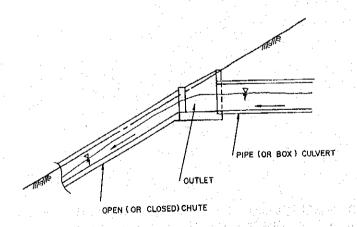
L-3 REALIGNMENT OF THE TRAILWATER CHANNEL N-1 REALIGNMENT OF THE CHANNEL

Channel Realignment





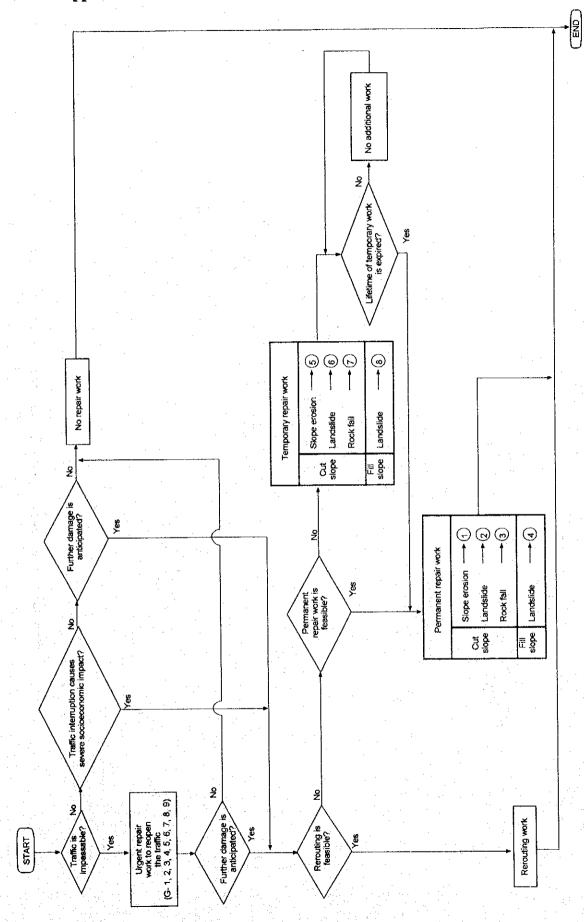
O-1 SIDE-TAPERED INLET O-2 FOOT PROTECTION AT CULVERT ENTRANCE



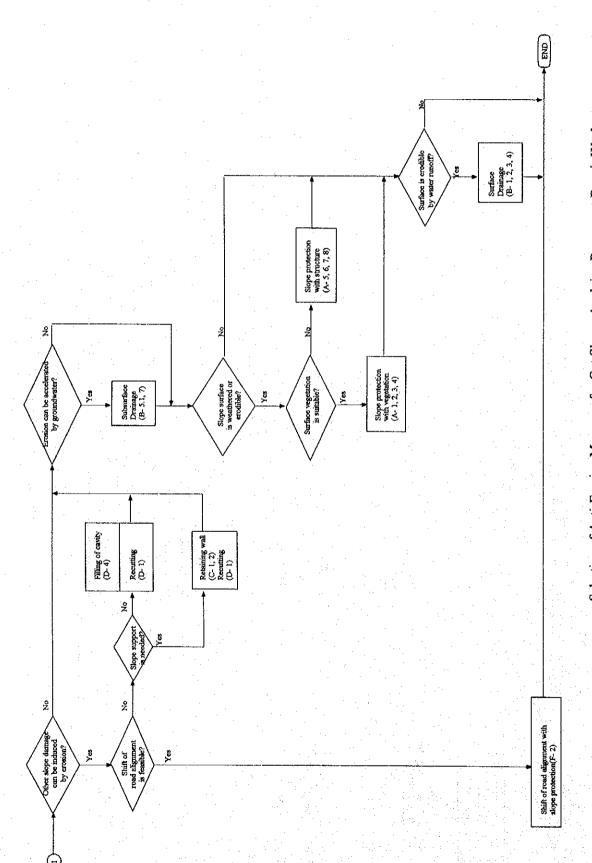
O-5 SIDE-TAPERED OUTLET O-6 OPEN OR CLOSED CHUTE FOR TRAILWATER

Inlet and Outlet Control

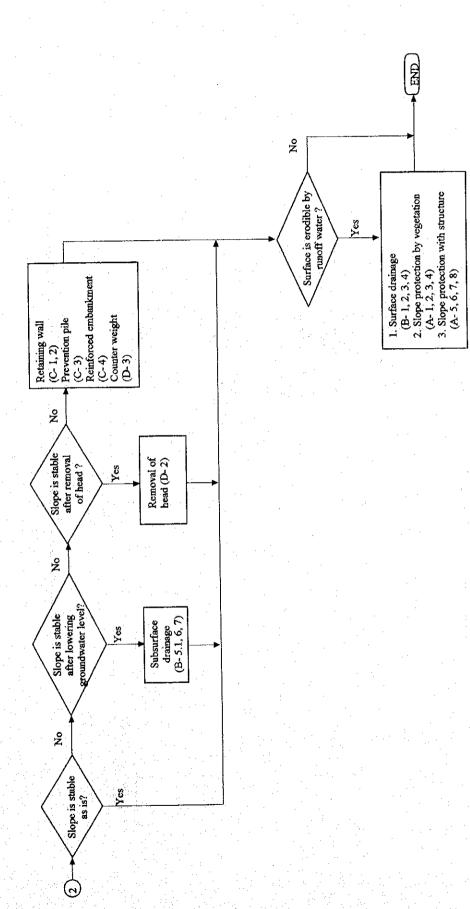
Appendix 4.2 Flowchart of Selection on Restoration Measure



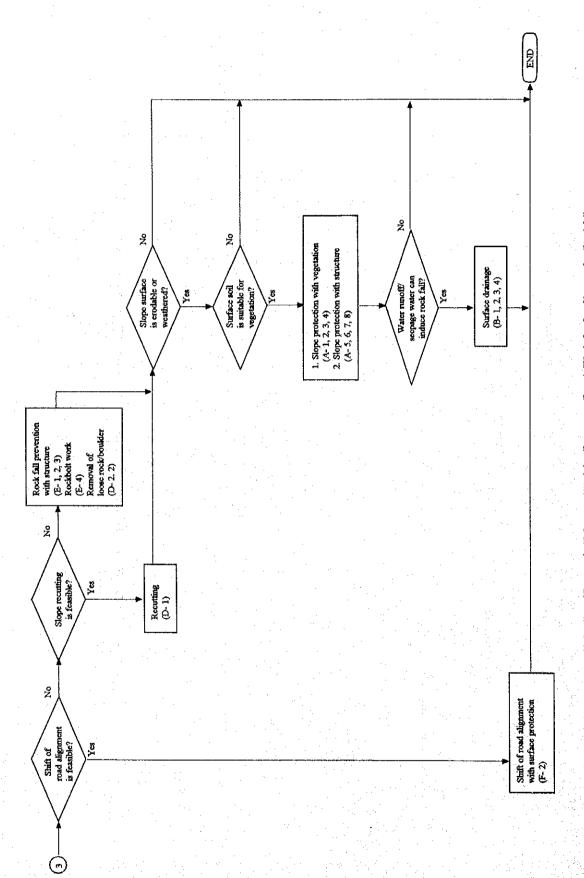
Selection of Restoration Measures for Slope & Embankments



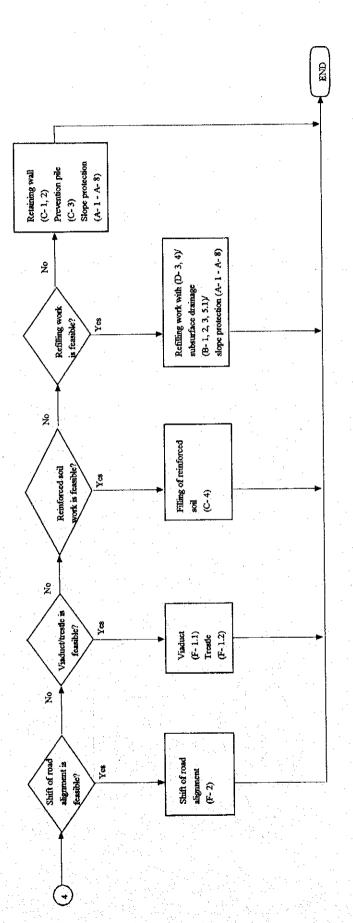
Selection of Anti-Erosion Measures for Cut Slopes Applying Permanent Repair Work



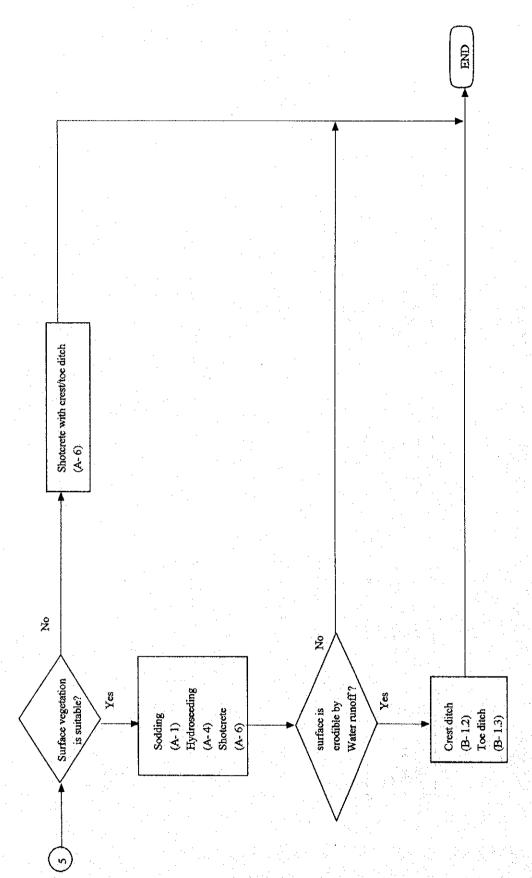
Selection of Restoration measures Applying Permanent Repair Work for Cut Slopes after a Landslide



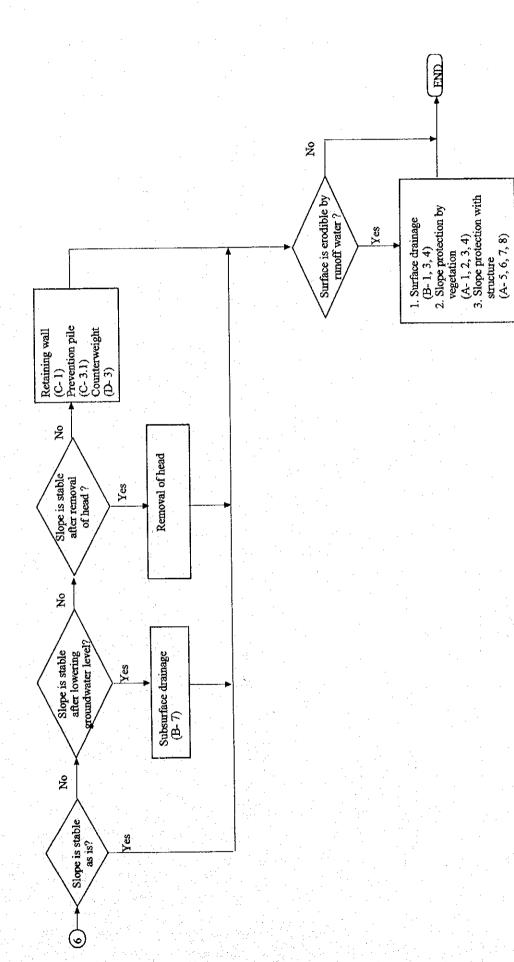
Selection of Restoration Measures Applying Permanent Repair Work for Cut Slopes after Rockfalls



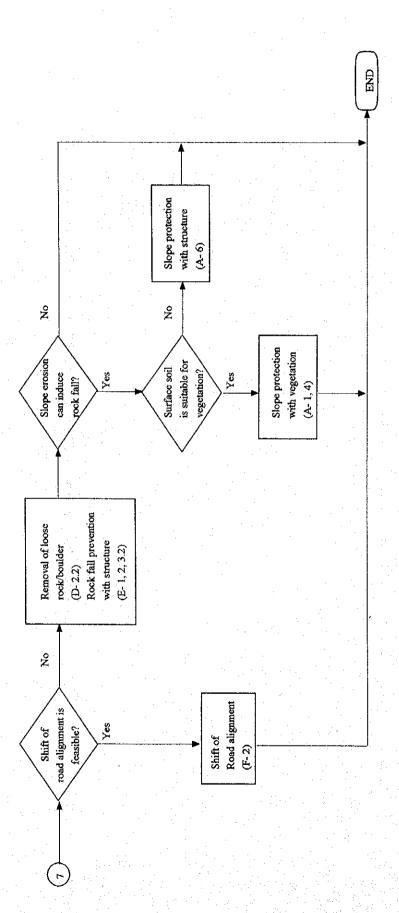
Selection of Restoration Mesures Applying Permanent Repair Work for Fill Slopes after a Landslide



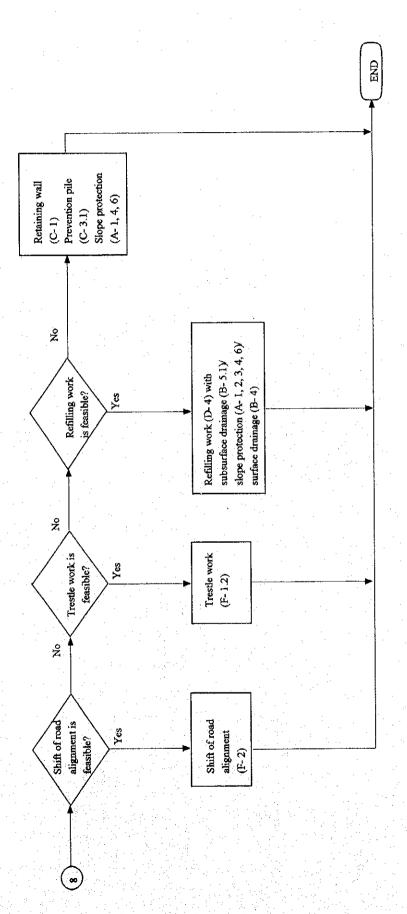
· Selection of Anti-Erosion Measures Applying Temporary Repair Work for Cut Slopes



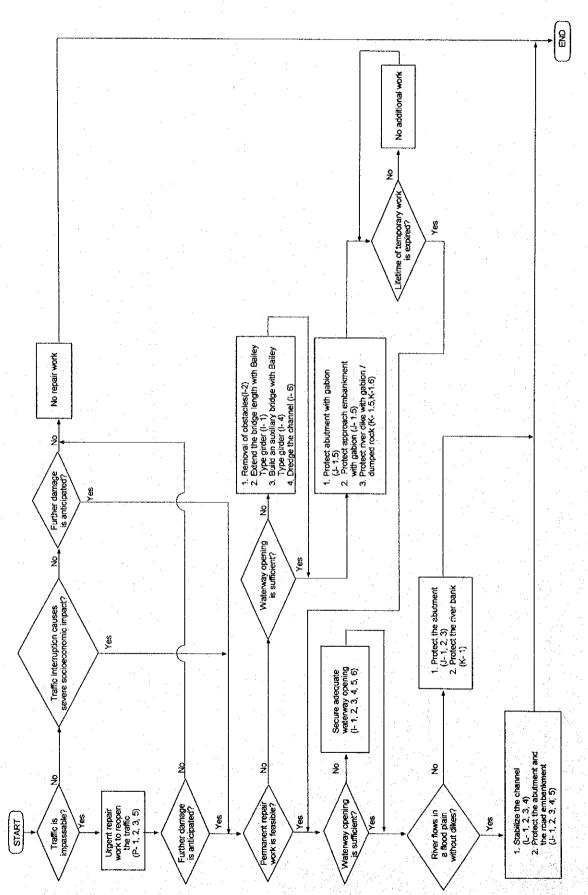
Selection of Restoration Measures Applying Temporary Repair Work for Cut Slopes after a Landslide



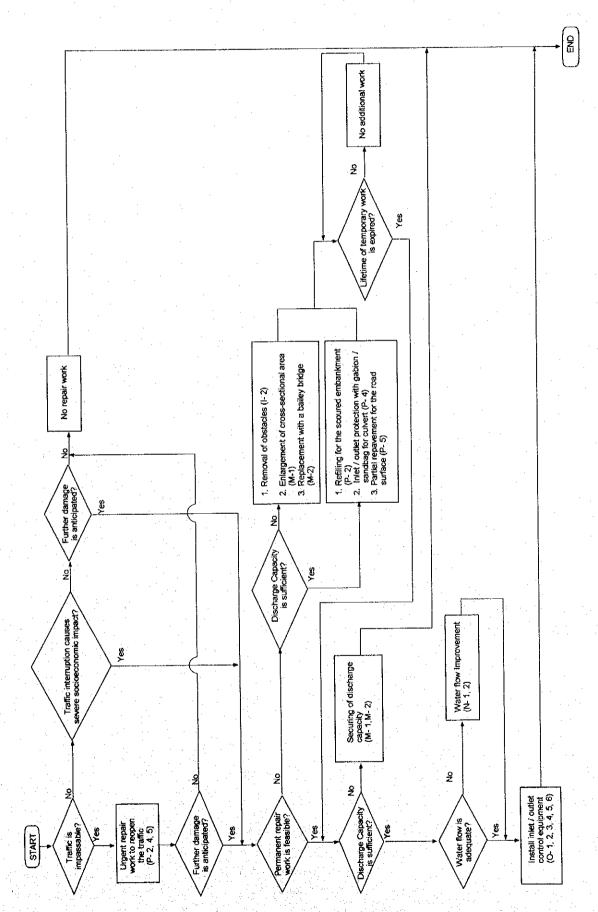
Selection of Restoration Mesures Applying Temporary Repair Work for Cut Slopes after Rockfalls



Selection of Restoration Measures Applying Temporary Repair Work for Fill Slope after a Landslide



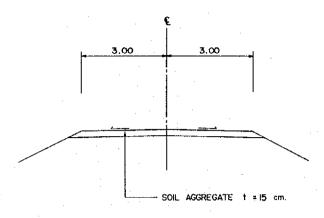
Selection of Restoration Measures for a Bridge Collapsing



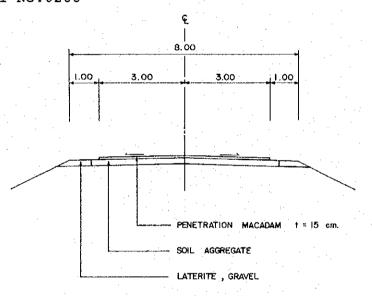
Selection of Restoration Measures for a Culvert Collapsing

Appendix 4.3 Standard Cross Section for Each Project Roads

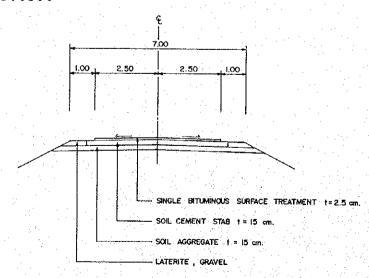
Route 109 Control No.0100



Route 109 Control No.0200

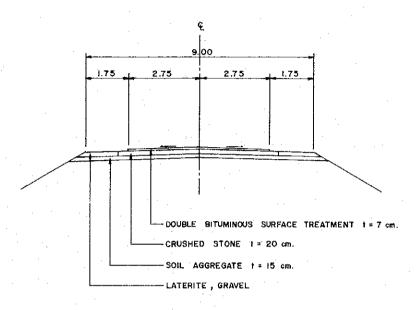


Route 1095 Contorl No.0500

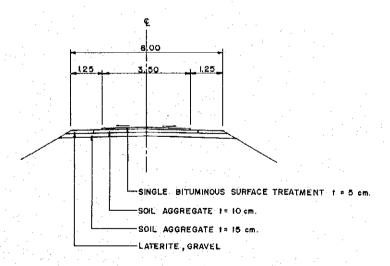


Standard Cross section (1)

Route 1149 Control No.0100



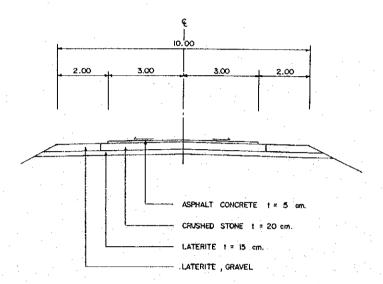
Route 1256 Control No.0101



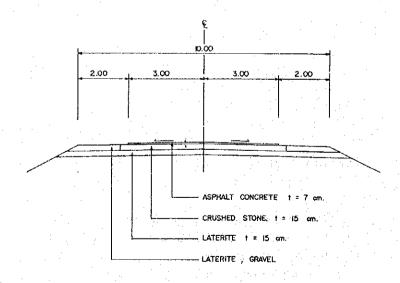
Standard Cross Section (2)

4 - 25

Route 4 Control No.3800

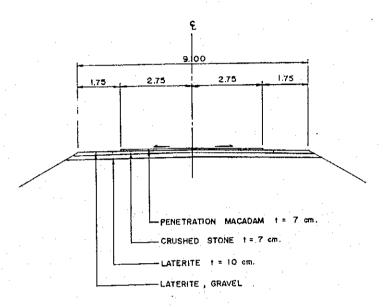


Route 4 Control No.3900

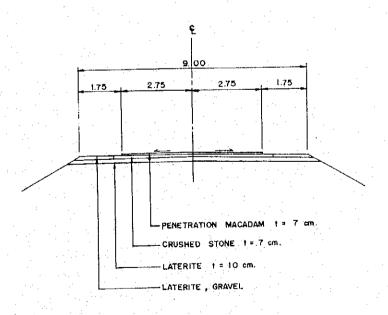


Standard Cross Section (3)

Route 410 Control No.0301

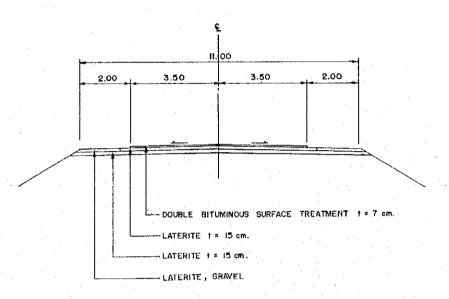


Route 410 Control No.0302

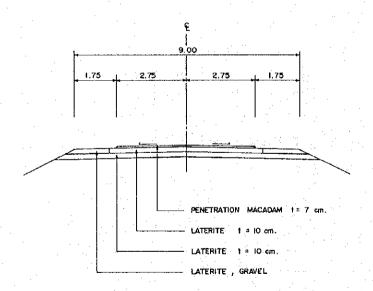


Standard Cross Section (4)

Route 410 Control No.0401

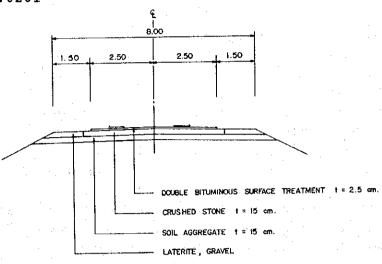


Route 410 Control No.402

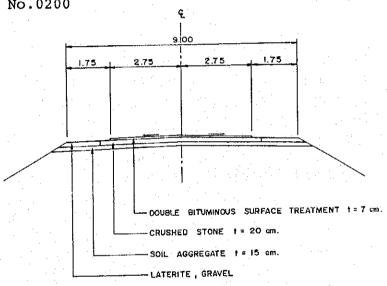


Standard Cross Section (5)

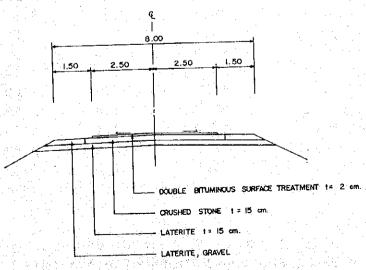
Route 4015 Control No.0201



Route 4107 Control No.0200



Route 4058 Control No.0100



Standard Cross Section (6)

Appendix 4.4 Discharge Capacity

	Spot No.	n	A	P	R	i	Qc	Q	
1	1256/2/3	0.05	220.8	52.2	4.23	0.020	1663.4	169.5	٦
1	4/2/3	0.03	44.0	24.0	1.83	0.020	310.7	32.9	
1	4015/1	0.03	322.7	132.4	2.44	0.030	3374.3	100.1	1
Í	4015/4	0.05	112.7	. 41.0	2.75	0.020	625.5	455.1	1
	4015/5	0.04	45.0	67.0	0.67	0.020	122.0	313.7	1
	4015/c1	0.04	80.0	30.0	2.67	0.025	608.1	263.0	1
	4015/c2	0.04	276.0	58.0	4.76	0.020	3680.9	183.2	1
1	4107/1/2	0.035	57.5	30.5	1.89	0.020	354.5	335.5	ŀ
1	4058/1	0.045	57.0	31.0	1.84	0.010	190.1	188.5	1.

Where,

n = Roughness Coefficient (sec/m¹/₃)

A = Cross-sectional Area of Water Flow (m²)

P = Length of Wetted Perimeter (m)

R = A/P : Hydraulic Radius (m)

i = Hydraulic Gradient

Qc = Discharge Capacity (m^3/sec)

Q = Discharge Volume (m³/sec)

Appendix 4.5 Ratio of River Flow Blockade by Pier

Spot No.	M	В	N
1256/2/3	36.5	1.4	3.84
4/2/3	19.5	0.7	3.59
4015/1	96.8	3.5	3.62
4015/4	35.0	1.1	3.14
4015/5	59.0	1.7	2.88
4015/C1	27.5	1.4	5.09
4015/C2	53.0	2.4	4.53
4107/1/2	28.2	0.7	2,48
4058/1	29.5	1.0	3.39

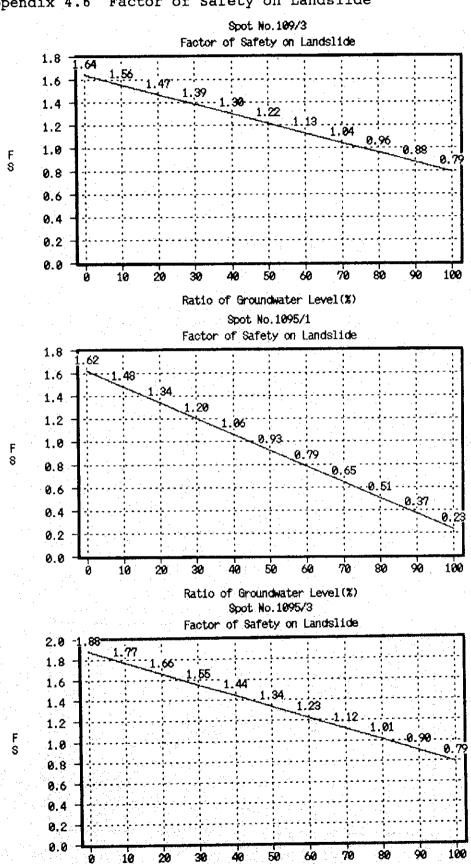
Where,

W = Width of Water Flow at Flood Level (m)

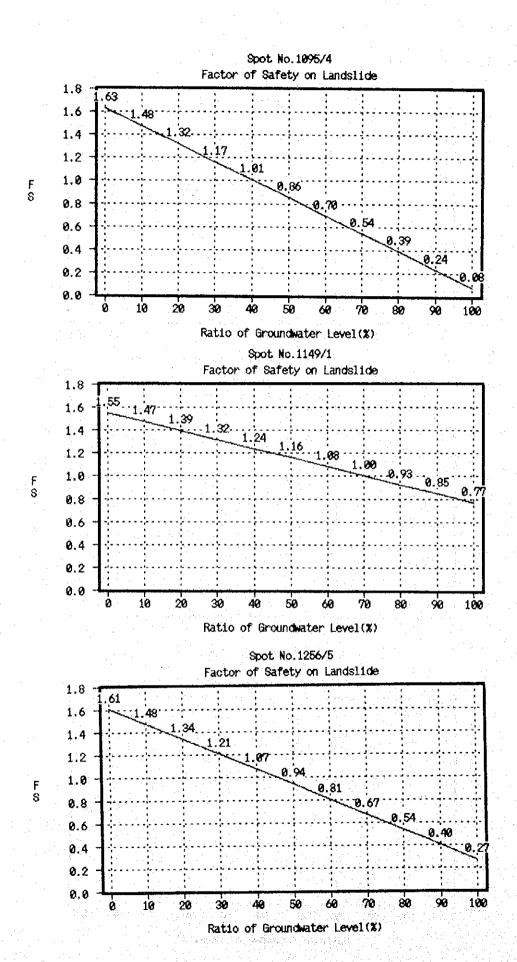
B = Total Sectional Width of Piers (m)

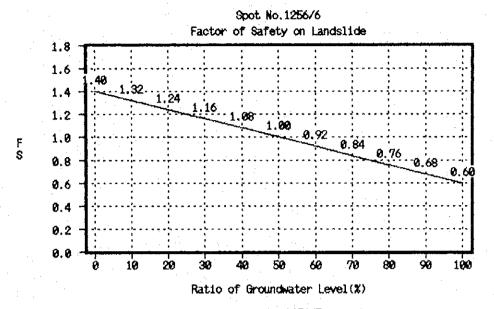
N = Ratio of River Flow Blockade by Pier

Appendix 4.6 Factor of Safety on Landslide



Ratio of Groundwater Level(%)





Spot No.1256/7 Factor of Safety on Landslide 1.8 1.6 1.4 1,2 1.0 F 0.8 0.6 0.4 0.2 0.0 50 70 30 90 10 Ratio of Groundwater Level(%)

