the samples. Points (1) - (9) are the samples from test wells and (a) - (j) are from springs and shallow wells.

Most of the points are distributed in the areas with high (Ca+Mg) and (CO_3+HCO_3) concentration. The points of the test well samples have higher (CO_3+HCO_3) concentration in comparison with the points of samples taken from spring water. The difference is mainly attributable to water origin, that is, water from the lower aquifer has existed for a period longer than water from the upper aquifer, thereby consisting of different chemical properties due to geological activities.

7.2.4 Inventory of Wells and Springs

Surveys were conducted on the present condition of the shallow wells and springs located in the vicinity of the proposed boreholes. The following items were surveyed by area.

Number of shallow wells and springs in the vicinity of the proposed boreholes

Water right and utilization of the above shallow wells and springs

Hydrogeological conditions such as aquifer

characteristics, water level and water quality of the above shallow wells and springs

The results are summarized in Table 7.2.7. As shown in this table, there are many shallow wells and springs in the area, which are used for domestic and agricultural purposes. However, no impact is presumed on the utilization of shallow wells and springs from the construction of deep wells, due to the following hydrogeological condition.

- (a) The water of the existing shallow wells and springs in the area is discharged from the shallow (upper) aquifers consisting of alluvial deposits (Qa), pumice sediments (Qp) and weathered upper zone of Tertiary volcanic rocks.
- (b) The screen of the deep wells are installed in the lower aquifers which belong to the formation of Tertiary volcanic rocks (Qv). The lower aquifer is the unconfined and/or semi-confined aquifer.
- (c) An unsaturated dry zone separates the upper and lower aquifers.
- (d) A partial leakage of groundwater will occur from the upper aquifer to the lower aquifer through the unsaturated dry zone, but the artificial leakage can be mostly avoided by cementing.

					(
	Table 7.2.1	EX I S	sting		Kecord		
		Discl	Discharge(Q= 0	, /S.) S	Number	Remarks : Sc(m/day/m)	· · ·
Basin	Aquifer	Average	Maximum	Minimum	of Wells	of Existing Data	· ·
Basin ①	Tv : Tertiary Volcanics	4.44	9.46	0.76	4		
Rio Acuacapa Basin (2)	Tv : Tertiary Volcanics	0.95	0.95	0.95	1	Sc(m/day/m): 1.34	
Rio Las Vacas & Lago de	Qa : Alluvial Sediments	31.62	61.00	20.50	Ś	Sc(m/day/m):233.05-483.84	
Amatitlan Basin ③	Qp : Pleistocene Volcanics	14.23	22.67	1.58	26	Sc(d/day/m): 2.18-893.95	
	Tv : Tertiary Volcanics	13.39	36.09	1.73	10	Sc(m/day/m): 11.41-4980.00	
	Qp/Tv:Ple./Tertiary Volcanics	14.27	28.01	5.80	12	Sc(m/day/m): 4.24-357.89	• • •
	Br : Basement Rocks	3.41	3.41	3, 41	1-4	Sc(d/day/m): 3.00	
Rio Pixcaya Basin ④	Tv : Tertiary Volcanics	7.52	15.14	0.32	r		
	Qp/Tv:Ple./Tertiary Volcanics	8.53	15.77	1.89	6	Sc(fd/day/m): 4.87-78.52	
Rio Guacalate Basin ⑤	Qa : Alluvial Sediments	14.24	31.55	6. 62	6	Sc(m/day/m): 55.78-544.89	
	Qv : Holocene Volcanics	13.81	31.54	6.00	L	Sc(m/day/m): 27.39-88.09	
	Tv : Tertiary Volcanics	7.89	15.14	1.70	9	Sc(m/day/m): 2.57-817.34	
-	Qp/Tv:Ple./Tertiary Volcanics	9.84	17.70	3 78	4	Sc(m/day/m): 54.11-490.32	
Atitlan Basin ()) Qa : Alluvial Sediments	27.13	27.13	27.13	1	Sc(m//day/m):200.34	-
Basin @	Qp : Pleistocene Volcanics	20.86	68.81	3.15	32	Sc(#/day/m): 25.18-726.91	
	Tv : Tertiary Volcanics	7.89	17.41	1.89	1		
	Qv : Holocene Volcanics	9.27	9.27	9.27	1		
	Qp : Pleistocene Volcanics	14.51	14.51	14.51			
	Br : Basement Rocks	4.20	10,09	1.13	33		

Table 7.2.2 Result of Hydrogeological Survey by Municipality (1)(Probability of New Water Sources Development)

Hydrogeological Conditions

		Tater	Sources	(1/s)	Water	Quality	Hydrogeol	ogical Conditi	08	
No.	Munichipality	-				Ec	Productivity of		Geological	Class
		N	P	R	PH	(25° C)	Existing Well	Lithofacies	Structure	
							(1/s)			
1	Santa Catarina Pinula	8.67	20.51	1	6.0	140	11.04 a	a (Qp) >b	8	_; X , ²
2	San Jose Pinula	-	17.78	1944 - S	62	92	5.68 b	b>a (Qp)	C	B
3	San Jose del Golfo	0,31	5.46	- - .	6.2	320	5.93 b	b∙c	Ь	B
4	Palencia									1
5	Chinautla	0.05	1,72	-	7.0	410		a (Qp) >b	а	. A
6	San Pedro Ayampuc	2.03	5.18	· .	7.6	587	2.59 c	b	b c	B
7	Nixco	5.79	30.75	-	7.0	180	7.69 b	b>a (Qp)	b c	B
8	San Pedro Sacatepequez	4.24	3.40	-	6.3	149	3.40 c	b>a (Qp)	b•c	B
9	San Juan Sacatepequez	10.00	12.00	- :	7.0	509	12.00 a	b>a (Qp)	Ъс	A
10	San Raymundo	· -·	22.08	19 - 1	7.5	305	11.04 a	a(Cp) b c	a b	A .
11	Chuarrancho	0.01	-	11.11	6.5	550		C	C	C
12	Fraijanes			а. — А. — А.						
13	Amatitlan									
14	Villa Nueva	4.98	61.51	-	7.0	308	12.30 a	a (Qp) >b	a	A I
15	Villa Canales	45.00	128.70	-	7.0	265	64.35 a	a (Qa>Qp)	8	×.
16	San Niguel Petapa									1 · · ·
					1					

Hydrogeological Conditions

SACATEPEQUEZ

		Tater	Sources	(1/s)	Tater	Quality	Hydroge	ological Conditi	ons].
No.	Munichipality		<u> </u>	·····		Ec	Productivity o	f	Geological	Class
		Ň	P P	R	РН	(25°C)	Existing Well	Lithofacies	Structure	
							(1/s)			
1	Antigua Guatemala			1						
2	Jocotenango		39,40	-	6.5	284	13.13 a	a (Qa)	8	A
3	Pastores].					
4	Sumpango									
5	Sto. Domingo Xenacoj									
6	Santiago Sacatepequez									Ì
7	San Bartolome M. Altas	0.40	13.00		7.0	143	6.50 b	þ	Ь	B
8	San Lucas Sacatepequez									
9	Santa Lucia M. Altas		8.00	· -	6.5	238	4.00 c	b>a (Qp)	C	C
10	Magdalena Milpas Altas	0.81	9.40	-	6.5	173	9.40 b	b>a (Qp)	Ь	B
11	Santa Maria de Jesus	1.50	6.00	:	7.0	328	6.00 b	a (Qa•Qv)	c	В
12	Ciudad Vieja	0.55	40.12	-	6.5	270	13.37 a	a (Qv)	a	A
13	San Miguel Duenas									
14	San Juan Alotenango									
15	San Antonio Aguas Cal.	8.45	1.70				1.70 c	b>a (Qa)	p.c	В
16	Santa Catarina Barahona	17.58	- '	-		1 A. A.		b>a (Qa)	b.c	B
i .	1		1 .		1.1	1			<u> </u>	1

1. Productivity of Existing Well a:More than 10 1/sec b: 5-10 1/sec c:Less than 5 1/sec

2. Lithofacies a:Upper Aquifer(Qa-Op-Qv) b:Lower Aquifer(Tv) c:Basement Bocks 3.Geological Structure a:Basin Structure b:Fractured Zone Deep Weathered Zone c:Local Basin / Weathering 4. Class:Availability of Groundwater in Terms of Quantity/Quality A:High

B:Medium G:Low

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Table 7.2.2 Result of Hydrogeological Survey by Municipality (2) (Probability of New Water Sources Development)

Hydrogeological Conditions

		Water	Sources	(1/s)	Tater	Quality	Hyd	drogeoi	ogical Conditi	ons	
No.	Munichipality	N	P	R	PH	Ec (25°C)	Productiv Existing		Lithofacies	Geological Structure	Class
				ing Santa			(1/s)				
2	Chimaltenango San Jose Poaquil	0.93	-	2.15	6.3	91			b•c(Li∎)	ь	B
3	San Martin Jilotepeque	-	18.90	-	7.0	167	8.90	a	a (Qp) >b	a	Å
4	San Juan Comalapa	34.00	5.80	-	6.3	140	5.80	b	a (Qp) · b	C · · ·	B
- 5	Santa Apolonia					ļ				n e dan din Provinsi	
6	Tecpan Guatemala					1					
7	Patzun	16.90	-	-	6.5	511			a (Qp) · b	C C	B
8	San Miguel Pochuta				}		ľ				
9	Patzicia	8.58	(10.00)	-	6.5	149			b>a (Qp)	b∙c	B
10	Santa Cruz Balanya		. ·		· ·						
11	Acatenango										
]2	San Pedro Yepocapa	· ·			• •	1					
13	San Andres Itzapa								1		
14	Parramos							1	· · ·		
15	Zaragoza	10.42	3.15	-	6.0	155	3.15	c	b>a (Qp)	b·c	·B
16	El Tejar	1	30.70	. - '	6.5	223	10.23	a.	a (Qp) · b	a	A

Hydrogeological Conditions

		Water	Sources	(1/s)	Water	Quality	Hydrogeol	logical Conditi	ons	
No.	Munichipality	N :	P	R	рн	Ec (25°C)	Productivity of Existing Well	Lithofacies	Geological Structure	Class
			1				(1/s)			
1	Solola	30.4	_	-	6.0	106		a (Qp) b	b·c	B
2	San Jose Chacaya								} .	
3	Santa Maria Visitacion				ļ					
4	Santa Lucia Utatlan	1.88	-	-	6.0	82		b>a (Qp)	b∙c	B
Ś	Nahuala	3.47	-	-	-			b∙a(Qp)	b•c ·	B
6	Sta.Catarina Ixtahuacan	7.29		-	6.5	125		b>a (Qp)	b	B
1	Santa Clara la Laguna					:				
8	Concepcion				ļ	{				
9	San Andres Semetabaj	0.95	-	. .	-	-		b	c	C
10	Panajachel									
11	Sta. Catarina Palopo	3.12	· -	· 	7.0	238		b>a (Qa)	c	C
12	San Antonio Palopo	0.42	-	-	6,5	181		b>a (Qa)	c	C
13	San Lucas Toliman	· ·				· .				1
14	Santa Cruz la Laguna				1	1				ļ
15	San Pablo la Laguna]					
16	San Marcos la Laguna					· ·			Į	
17	San Juan la Laguna			·						i
18	San Pedro la Laguna									
19	Santiago Atitlan	14 N 20		$(p,r) \in \mathbb{R}^{n}$	· ·	1	1	1	<u> </u>	l

1. Productivity of Existing Well a:More than 10 l/sec h: 5-10 l/sec 2. Lithofacies a:Upper Aquifer (Da:Op'Qv) b:Lower Aquifer (Tv) c:Less than 5 1/sec

c: Besement Rocks

3.Geological Structure a:Basin Structure b:Fractured Zone

Deep Weathered Zone c:Local Basin / Weathering

4.Class:Availability of Groundwater in Terms of Quantity/Quality A:High

B:Medium C:Low

Table 7.2.2 Result of Hydrogeological Survey by Municipality (3) (Probability of New Water Sources Development)

Hydrogeolog	ica	ł	Condi	tions	
TOTORICADAR	2	÷	·		

		Tater	Sources	(1/s)	Tater	Quality	Hydrogeol	logical Conditi	ons	
No.	Munichipality	N	P	R	PH	Ec (25°C)	Productivity of Existing Well	Lithofacies	Geological Structure	Class
1 2 3 4 5 6 7 8	Totonicapan San Cristobal Totonic. San Francisco el Alto San Andres Xecul Momostenango Santa Maria Chiquimula Santa Lucia la Reforma San Bartolo Aguas Cal.	6.7 2.3 14.2			6, 5 6, 0 6, 5	68 104 94	(1/s)	b>a(Qa) b•a(Qp) b	b	C A B

Hydrogeological Conditions QUETZALTENANGO

_		Tater	Sources	(1/s)	Water	Quality	Hydrog	geological Conditi	ions	
No.	Munichipality	· · · · ·		r te s		Ec	Productivity	of	Geological	Class
		N	Р	R	PH	(25°C)	Existing Wel	1 Lithofacies	Structure	
-1							(1/s)			
1	Quetzaltenango					ľ				
2	Salcaja	÷.,				1 · · ·				
3	Olintepeque	0.94	11.13	- 1	6.5	207	11.13 a	a (Qp)	a•b	A
4	San Carlos Sija	2.80	. - [™] .	-	6.2	100		b>a (Qp)	b،c	B
5	Sibilia									
6	Cabrican	1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 - 1999 -						and a state of the state		
7	Cajola	1.84	-	'	6.0	62		a (Qa) b	a	Å
8	San Miguel Siguila				· · ·					
9	San Juan Ostuncalco									
10	San Mateo									
11	Cpcion. Chiquirichapa	11.57	-		6.0	220		b>a (Qp)	b·c	B
12	San M. Sacatepequez	3.15	-	-				a (Qv) · b	8	: A
13	Almolonga	23.87	36.90	-	6,5	356	12.30	a b>a(Qa-Qv)	à	A
14	Cantel	1		1						
15	Huitan	0.91	-	[-	-	-		b>c	c	C
16	Zunil]								
17	Colomba							a (Qv)	8	A
18	San Francisco la Union	0.59	-	-	6.5	127		a (Qp) b	b+c	B
19	El Palmar									
20	Coatepeque	1 :		1.						
21	Genova	3.03	1		6.0			a (Qv)	C	B
22	Flores Costa Cuca	2.25	(9, 27)) -	6.5	98	9.27	b a (Qv)	C	8
23	La Esperanza									
24	Palestina	-	13.89	-	6.5	146	13.89	a b	b	B

 Productivity of Existing Well a:More than 10 1/sec
 5-10 1/sec
 c:Less than 5 1/sec 2. Lithofacies a: Upper Aquifer (Qa-Qp-Qv) b: Lower Aquifer (Tv) c: Basement Rocks 3. Geological Structure a:Basin Structure b:Fractured Zone Deep Weathered Zone c:Local Basin / Weathering 4. Class: Availability of Groundmater in Terms of Quantity/Quality A:High

B:Medium C:Low

111		Number &	Depth of	Main Aquifer Characteri	stics		Recommended	🤈 Site 🕯	Productivity
No.	Municipality	E/R So	unding		Aparent		Depth for T	est Well	of existing
1. j. t. j.		Number	Depth	Lithofacies	Resistivity	Thickness	Site	Depth	Well
÷.,		(points)	(GL=)		(Ωm)	(m)		(m)	(1/sec)
							About 300ms.		
Gu 2	San José Pinula	5	180~300	Upper rhyolitic welded tuff	32~312	120	South of E-2	150~(200)	5.68
1.1				with thin lava flows (Tv)			(Fig 2.1.2)		
· .				Pumice sediments (Op) and					
				pyroclastic rocks with lava	7~140	70~90	Between E-2		
Gu 8	San Pedro	3	360~380	flows and waterlain sediments (Tv)			and E-3	200	3.40
· .	Sacatepequez.			Andesitic/Besaltic					1. A.
				fractured lava flow (Tv)	532~600	250±	(Fig 2.1.3)		_
		1	1	Andesitic/Basaltic fractured					
Sall	Santa María	8.	180~340	lava flow with pyroclastic	700~1,460	200±	E-3 point	150~ (200)	6.00
• .	de Jesus			rocks (Qv)			(Fig 2.1.4)		
				Tuffaceous sandstone/			About 100ms.		
Ch 3	San Martin	4	260~320	Sandstone with tuffbreccia	26~504	70~90	S.W.of E-2	200	18.90
	Jilotepeque			and tuff (Niocene)			(Fig 2.1.5)		
							About 140ms.		
Ch 4	San Juan	5.5	260~320	Decitic/Andesitic tuffbreccia with	116~675	300±	South of E-2	200	5.80
	Comalapa			lava flows and tuffs (Tv)			(Fig 2.1.6)	:	
		· · · · ·		Decitic/Andesitic/Basaltic					
So 1	Sololá	8	260~360	fractured lava flow with	405~1, 125	90~200	E-4 point	200	
÷.,		. .		pyroclastic rocks (Tv)			(Fig 2.1.7)		
		· [Between E-1		
So 4	Santa Lucia	5	260~360	Decitic/Andesitic lava flow	410~720	260	and E-3	200	-
	Utatlan	}		with pyroclastic rocks (Tv)			(Fig 2.1.8)		
		1					About 500ms.		
To 5	Nomostenango	2	300~340	Andesitic/Basaltic fractured lava	568~1,530	200±	N.E. of E-1	(200)~250	-
· .]	l ·	flow with pyroclastic rocks (Tv)			(Fig 2.1.9)		
							About 350ms.		
Qu18	San Francisco	6	340~400	Andesitic/Basaltic fractured lava	448~1,600	250±	East of E-2	(200)~250	-
	la Union			flow with gyroclastic rocks (Tv)		1	(Fig 2.1.10)		

Table 7.2.3 Result of Electrical Resistivity Sounding (carried out in Phase I)

Table 7.2.3 Result of Electrical Resistivity Sounding (carried out in Phase II)

		Number &	Depth of	Main Aquifer Characteristics		Recommended
No.	Municipality	E/R Sou			Aparent	Drilling
		Number	Depth	Lithofacies	Resistivity	Depth
		(points)	(GL, -18)		(Ωm)	(86)
Gu 3	San Jose del Golfo	3:4	200~240	Pyroclastic rocks with lava flow (Tv)	56~416	150
So 5	Nahua 1a.	3	140~180	Pyroclastic rocks with lava flow (Tv)	23~700	200
1.1					53~700	200
) u 4	San Carlos Sija	3	300	Andesitic lava flow (Tv)		200
նա 7	Cajola	3	320~340	Pumice sediments(Qp) and Andesitic lava flow (Tv)	840~1,500	200
Qu21	Génova	3	200~300	Pyroclastic(Volcanicaud) flow (Qv)	20~140	180
Qu.22	Flores Costa Cuca	4	140~300	Pyroclastic (Volcanicaud) flow (Qv)	63~344	180
To 5	Numostenango	1	160	Andesitic lava flows with Pyreclastics	68~1,080	150

	San Jose Pinula	San Pedoro Sacatepequez	Santa Waria de Jesus	San Wartin Jilotepeque	San Juan Comalapa	Solcia	Santa Lucia Utatlan	Homostenaogo	San Fransisco la Union	Genova
Profundidad (Well depth) (m)	0.00	250	212	195	215	170	199	133	190	152
	14 32 29 10 140 25 10	14°41°05° 90°39°08°	14-29/10*	90.47, 05*	14° 44' 44" 90° 53' 14"	14° 47' 35" 91° 10' 58"	14°45'40° 91°16'40°	15 02 49 19 19 19 10 10 10 10 10 10 10 10 10 10 10 10 10	14°55°15° 91°32°37°	31 49, 58"
(e)	1728	2090	1880	1760	2090	2370	2408	2216	2714	360
lametro del ademe (Diameter of Casing Pipes)		8	*	800	8 -		90	*	*****	-
				ĴΪ	CA Stu	idy Team	e			
6. Fecha de inic.y final de la perfor (Bitinning and Completion Date of Driling)	Aug.24 ~Sep.30 1994	Aug. 22 ~ Sep. 22 1994	Sep. 3 ~0ct.11 1994	Sep. 1 ~0ct. 3 1994	0ct.16 ~Nov.20 1994	0ct.13 ~Nov. 9 1994	0ct.15 ~Nov.12 1994	Nov.18 ~ Dec. 4 1994	Nov. 14 ~Dec. 16 1994	Nov.15 ~Dec. 4 1994
7. Tiempo que tomo (Spent days)	300	38	39	33	36	28	28	17	33	20
<pre>8. Posicion de rejilla(Screen Position) 1) Tipo Puente(Bridge Type) (Mivel de tierra : -m) (Ground Level : -m)</pre>	$\begin{array}{c} 33.5 \\ 33.5 \\ 42.7 \\ 54.9 \\ 91.4 \\ 103.6 \\ 109.7 \\ 109.7 \\ 140.2 \\ 146.3 \end{array}$	182.0~189.0 213.4~219.5 231.6~237.7	140,2~146,3 182,9~189,0 201,2~207,3	115.8~121.9 134.1~146.3 164.6~182.9	$\begin{array}{c} 109.7 - 121.9 \\ 128.0 - 140.2 \\ 152.4 - 158.5 \\ 176.8 - 189.0 \\ 176.8 - 189.0 \end{array}$	152.4~170.2	115.8~121.9 152.4~164.6 182.9~190.2	85.3~103.6 126.5~132.6 134.1~140.2 146.3~158.5	79.2~103.6 115.8~152.4	103.6~121.9 134.1~146.3
Tipo Johnson (Johnson Type)	$21.3 \sim 27.4$ $79.3 \sim 85.4$ $115.8 \sim 121.9$	164.6~182.9	152.4~164.6	88.4~ 91.5 103.6~109.7 146.3~152.4	164.6~170.7		170.7~182.9	164.6~170.7 176.8~179.8	$\begin{array}{c} 103.61 \sim 15.82 \\ 152.41 \sim 76.78 \\ 182.91 \sim 85.9 \end{array}$	91.4~103.6 146.4~149.4
3) Ranurzdo(Slot Type)	97.5~103.6 97.5~103.6 125.0~131.1 134.1~140.2 158.5~154.6	$109.7 \sim 115.8$ $1219 \sim 128.0$ $195.1 \sim 201.2$ $237.7 \sim 244.7$	$\begin{array}{c} 82.3 \\ 37.2 \\ 137.2 \\ 146.3 \\ 154.5 \\ 154.6 \\ 154.6 \\ 170.182.9 \\ 189.0 \\ 201.1 \end{array}$	97.5~103.6 121.9~134.1 152.4~164.6	30.4~36.6 73.1~85.3 91.4~103.6 146.3~152.4 189.0~201.2 210.3~213.4	109.7~131.1 140.2~143.3 149.4~152.4	146.3 - 152.4			2 - 16
 Longitud de rejlla(Screen Length) 1) Tipo Puente(Bridge Type) (m) 2) Tipo Johnson(Johnson Type) (m) 3) Ranurado(Slot Type) 	28 28 21 38 21 38 31 31 31 31 31 31 31 31 31 31 31 31 31	18.3 18.3 18.3	18.3 12.2 48.6	36.6 15.2 30.5	42.7 6.1 51.0	17.8 0 27.5	25.6 12.2 6.1	42.7 9.1 0	61. 9. 6 9. 6	30.5 15.1 6.1
Componente quimico de Agua (Mater quality) DH	0.1	8 °9	7.0	7.2	(* 9	4	0.1	1.0		
 Temperratura de agua : C) (Temperrature of Water : C) Conductividad (un/en) (Conductivity : un/en) 	20,0 263,0	23.4 198,3	19.8 298.0	20. 3 563. 0	19, 8 32, 0	21.2	16.6 151.7	5 3 0		21.5

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Tabie 7.2.5 Results of Pumping Test

-	1	: 						Ţ		· - 1		·				—r				<u></u>	 .	·· • • •
Genova		152		51.82		3	Dec. 11	1994		29.85	201	1096		88.36		12.4		10.74	9.55	15.42		11.99
S.F. la Union		190		100.5		Tv		-1		1				1				L.	ŀ	1		1
Momoste- naogo		183		59.9		Τv	Dec. 8	1994		53.50	200	1090		70.3		15.5	-	15.43	7.12	8.67		10.41
Santa Lu. Utatlan		199		43.91		Tv	Nov. 25	1994	·	131.54	162	883		9.13		96.7		228	359	538		375
Solola		170		48.78		Tv	Nov. 19	1994		71.63	390	2125		54.86		39.7		25.22	25.09	35.35		28.55
San Joan Comalapa		215	 	100.6	 .	Tv	Nov. 30	1994		28.94	250	1363		156.4		8.7		5.51	5.31	7.34		6.05
S.M.Jilo- tepeque		196		82.32		Tv	0ct.28	1994		80.35	401	2185		9.63		227		510	333	834		559
S.Maria S.		212		81.68		6	Nov. 2	1994		163.16	282	1537		3.53		435		150	612	937	· .	567
S.P. Saca- teneguez		250	-	60.97		Tv	0ct. 7	1.994		43.71	320	1744		67.29		26		33	37	68		46
San Jose S		180		79.27		Tv	0ct. 5	1994		6.84	495	2698		11.9		227		299	180	190		223
				(m)	ero principal	quifer)			gua	(G. Lm)	(GPM)	(m³/day)		(m)	1 : C. F.	Sc) (m³/day/m)	cansmissivity)	eis -	Jacob	Recuperacion	(Recovery)	(Average)
Nobre de Pozo (Well Name)	1. Profundidad	(Well depth) (m)	2. Longitud de rejilla	(Total Screen Length)	3. Formation del Aquifero principal	(Formation of Main Aquifer)	4. Fecha de Bombeo	(Pumping Test Date)	5. Nivel estatico de Agua	(Static Water Level)	iudal	(Pumping Rate)	7. Descenso	(Drawdown)	8. Capacidad Especifica : C.F.	(Specific Capacity : Sc)(m ³ /day/m)	9. Transmisibilidad (Transmissivity)	(m ¹ /day) a. Theis	þ.	c. Rec	()	Promedio(Average)
	1. 1	(We	2. Lc	(To	3. Fo	(Fo	4. Fe	ج ح 7-	in . 5 47	(St	6. Caudal	(Pu	7. De	Ū	8°. Ca	(Sp	9. Tr		· .		<u>`</u> .	••••••••
e Na sere e							· · ·						• •									

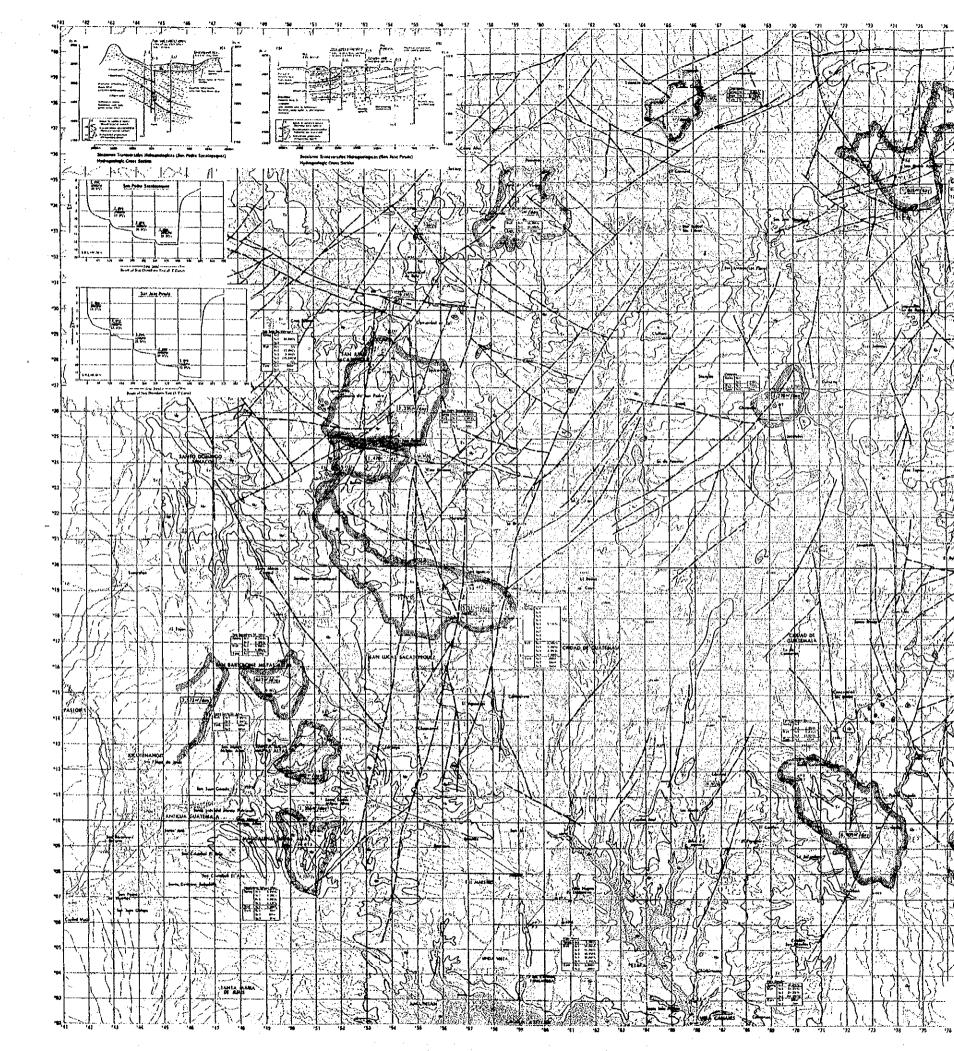
Table 7.2.6 Results of Ion Component Analysis

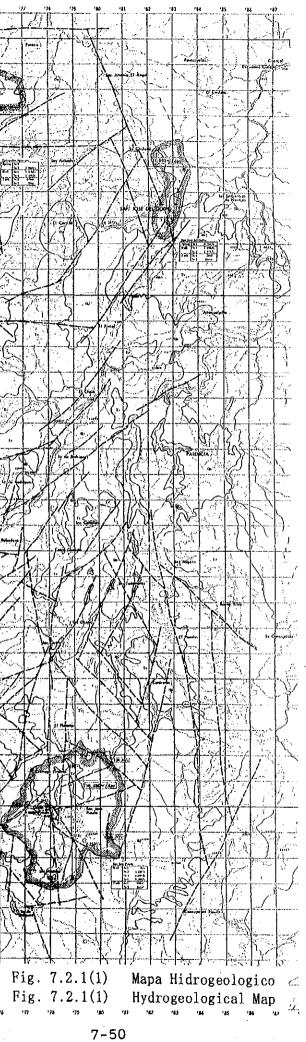
	Location	Ca	Mg	Na	K	C1	S04	Alcali	nity
•							2. j.	pH 8.3	pH 4
EST	DRILLING WELLS					n an Turu thu			
1	S. P. SACATEPEQUEZ	16.29	6.94	21.8	8.67	8.37	0	0	97.1
2	S.J.PINULA	52,12	24.5	11.36	12.12	18.83	0	0	146.5
3	S.M. DE JESUS	44.79	20.33	17.11	6.18	9.76	0	0	140.9
4	S.M.JILOTEPEQUE	22.8	9.82	17.1	7.92	8.37	0	0	112.7
5	S. J. COMALAPA	14.66	6.35	9.16	3.11	7.67	0	0	74.0
6	SOLOLA	16.29	7.93	16.23	7.21	8.37	0	0	111.1
7	S.L.UTITLAN	20.36	8.43	12.32	4.16	6.28	0	0	76.1
8	MOMOSTENANGO	13.03	2.28	7.92	5.66	4.88	0	0	53.5
9	S.F. LA UNION								
10	GENOVA	19.55	12.6	22.36	16.11	13.25	0	12.35	158.4
					n an				
THE	RS								
	S.J.PINULA								
	SPRING NO.1	13.03	5.75	5.13	0.86	8.37	0	0	48.8
	SPRING NO.2					36.26			84.5
	SPRING NO.3	19.55	8.63	12	12.3	22.32	0	0	46.9
	SPRING NO.4	16.29	8.43	11.1	1.35	11.16	0	0	56.3
	DUG WELL	50.49	31.81	18.7	3.9	65.55	0	0	92.0
	COMALAPA						- 		• •
	SPRING NO.1	17.92	8.53	6.46	4.58	9.07	0	0	61.7
						10.46		0	74.0
	SOLOLA				en e				
	EXISTING TANK	30.95	1.88	6.74	3.11	9.76	0	0	59.6
	WATER FALL	30.95		1		9.76		0	92.6
	S.P. SACATEPEQUE	2	· · · ·		e e e e e e Persona de la composición de Persona de la composición de la		e din		
	SPRING	21.28	13.69	5.97	2.31	22.32	0	0	100.8

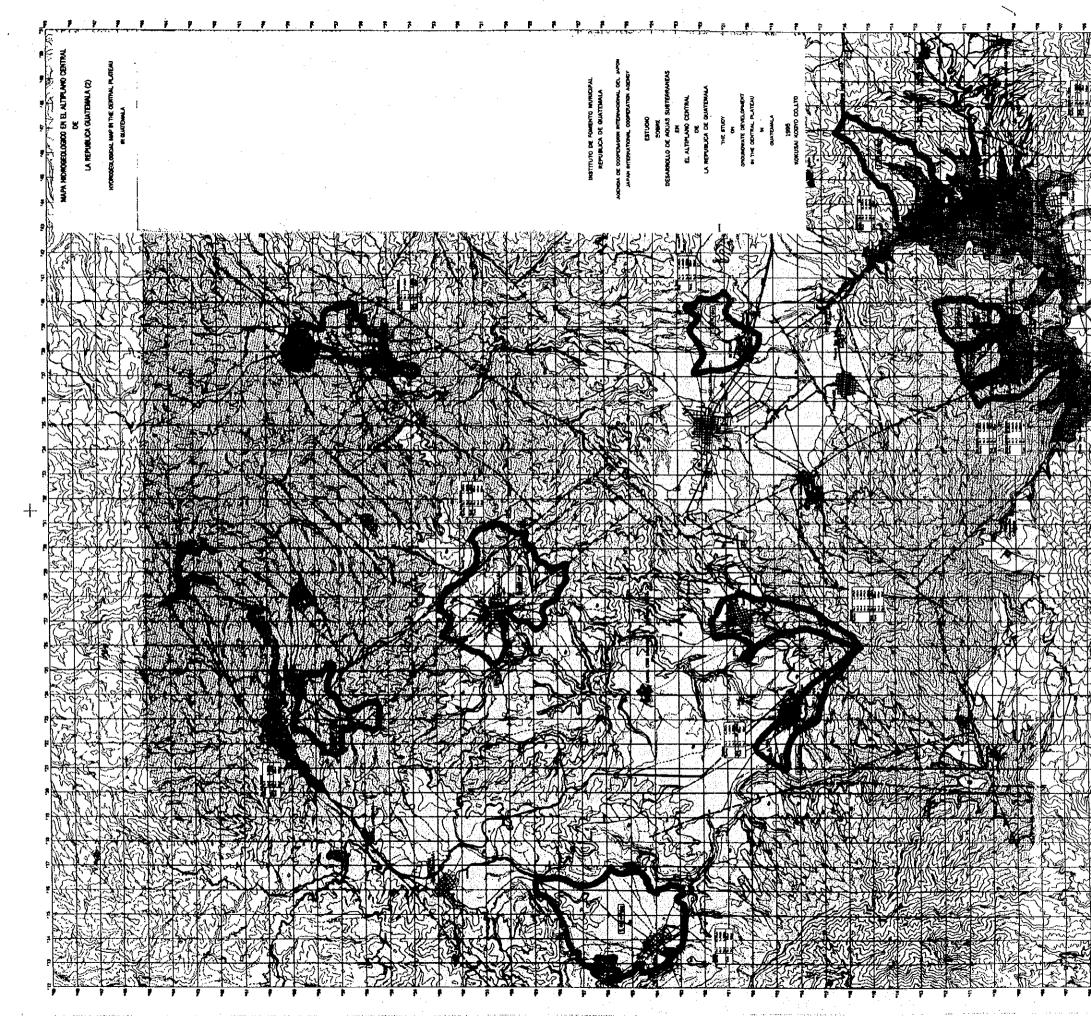
Table 7.2.7 Existing Shallow wells and Springs

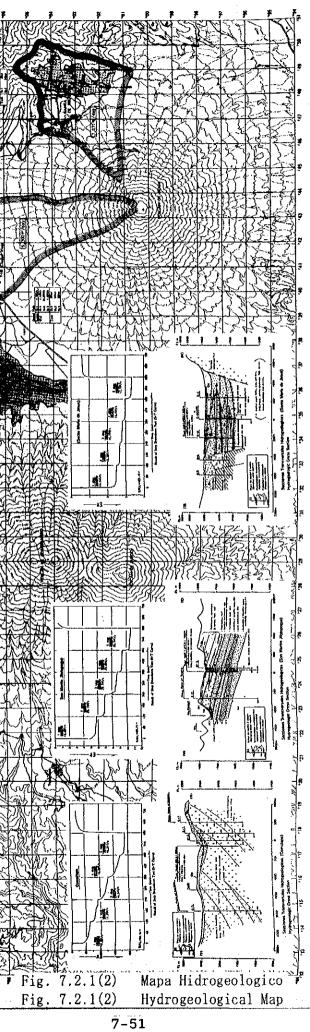
				11				5	Suringe	
			STIAN MOTIFUS	21	W (D) : - 1-1	114-4-20	Minhan	W/0.01:+1	W/Diah+	Wator llea
Municipality	Number	W/Level (GL-m)	Well depth (m)	W/Quality	W/Kight	Mater Use	INUMDEL	ATTEND/#	TIBIL /	nater use
						Agriculture-1	8	Poor	Public	Bathing &
S. J. Pinula	eo.	3-25	1	Poor	Private	Bathing and				washing
						vasnig-2-	4		N.1.1	
S. P.		-	•			Agriculture-2	<u></u>	1-000-1	C-DIIGN	U/S TOT CITY-D
Sacatepéquez	10	1-10	2.6-9	Good	Private	Domestic use-8		POOL-3	Private-5	Agriculture-3
										DURSULT OT STA
S. M. de	0	I			1	1	-	1 •	1	I
Jesús			·	-						
S. M.	ę	3-20	6-22	Good-2	Private	Domestic use-2	ی	Good	Public-4	Domestic use
Jilotepeque				Poor-1		Stand by-1			Private-1	
					•	Agriculture-2	7	Quite-	Public &	Washing
S. J. Comalapa	26	3.6-10	4-12	Quite-good	Private	Domestic use-24		good	private	Drinking &
										domestic use
						Aguriculture-1	ŝ	Quite-	Public-3	Drinking &
		·				Drinking &		good		domestic use
Sololá	ر م	0.6-26	1.6-28	Quite-good	Private	domestic use-1				
	*-3					Washing, bathing				Agriculture
						& cleaning-1				
				Good-1 *-4		Drinking &	2	Good-2	Public-1	Drinking
S. L. Utatlán	4	8.5-14.5	10-17	Quite-	Private	domestic use-4	•		Private-1	Washing &
				good-3		-			-	domestic use
Momostenango	15<	3-16	6.5-18	Good &	Private	Drinking &	1	Quite-	Public	Drinking
				quite-good		domestic use		goog		
					Private-8	Drinking &	2	Quite-	Public	Drinking.
S. F. la Unión	~	5-18	9-22	Quite-good	Public-1	domestic use		goog		washing &
						Only drinking				domestic use
Génova	200	5-10	7-15	Quite-good	Private	Drinking &	0	t	1	ı
						domestic use				
*-1: One spring existed, but it has already dried.	existed. t	wt it has a	Iready drie	d.						

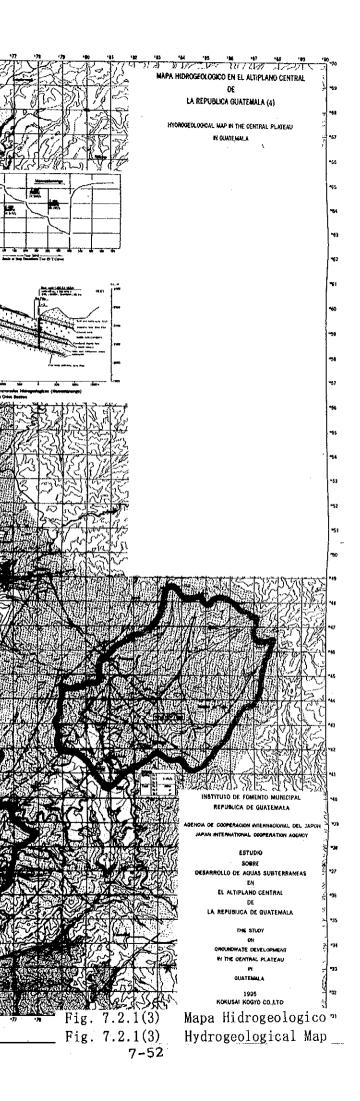
*-2: People drink raw water - good, boiled water - quite-good and do not use for drinking - poor
*-3: One was used until 1993, but it has alredy dried.
*-4: However, 2 % of population have stomach problems every month, and ca. 25 children/month have diarrhea.

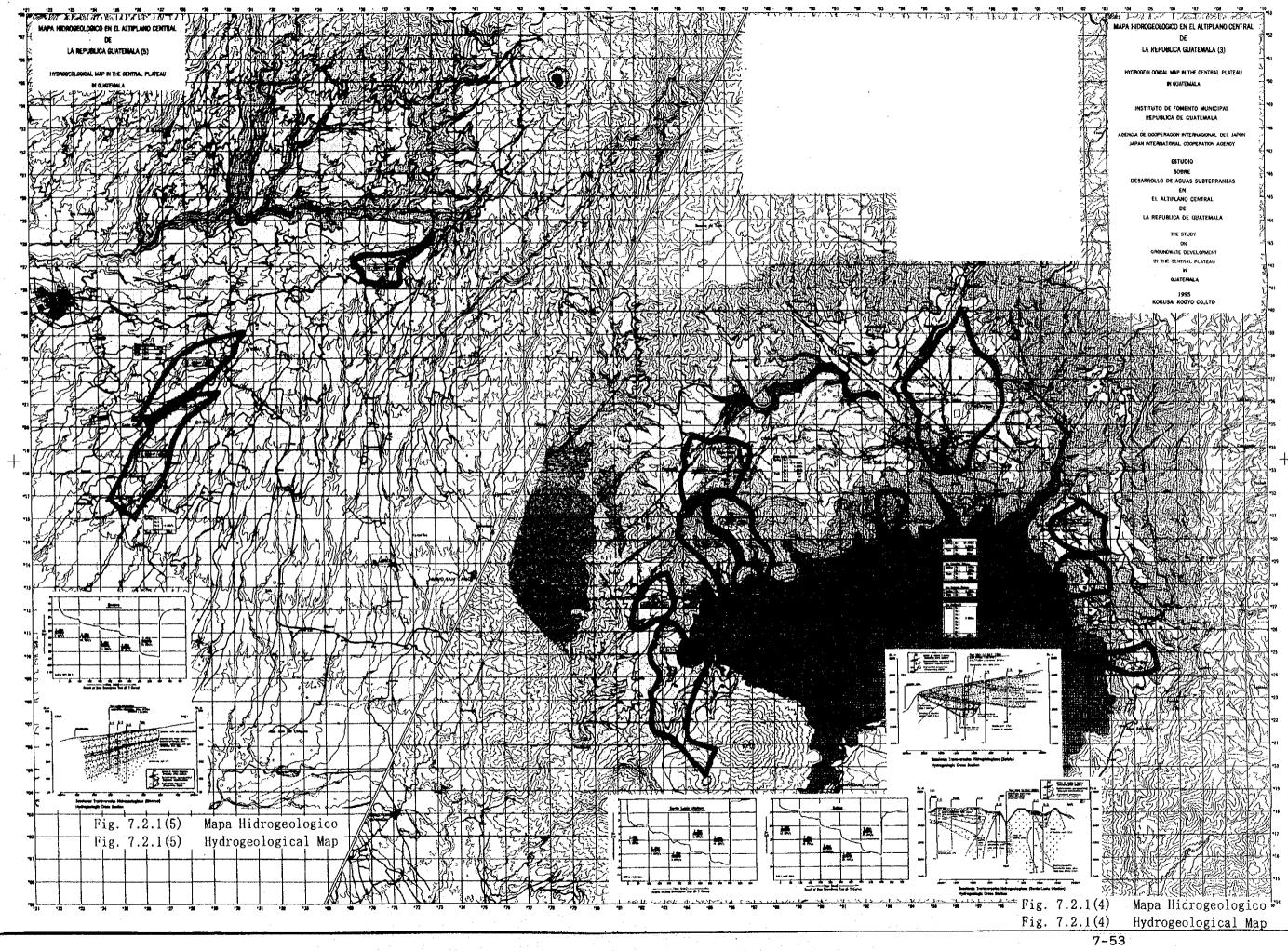












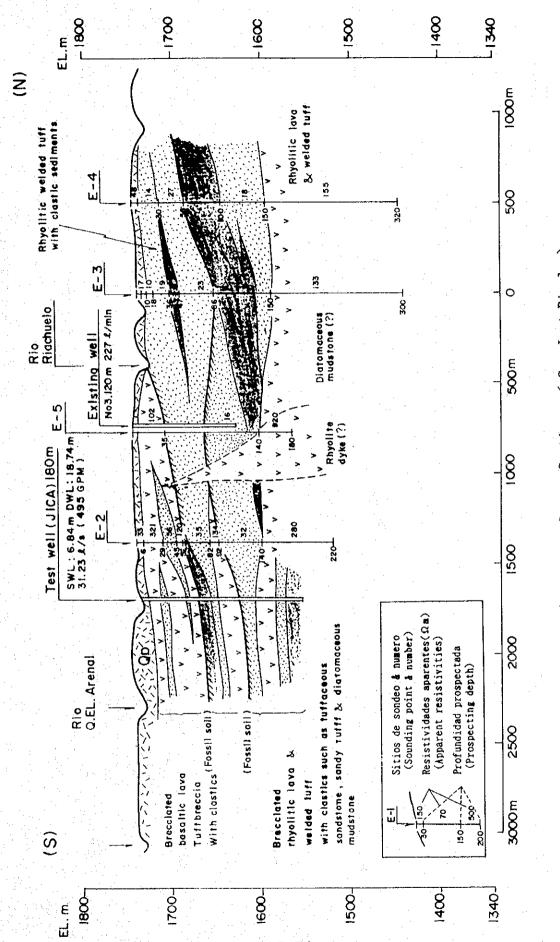
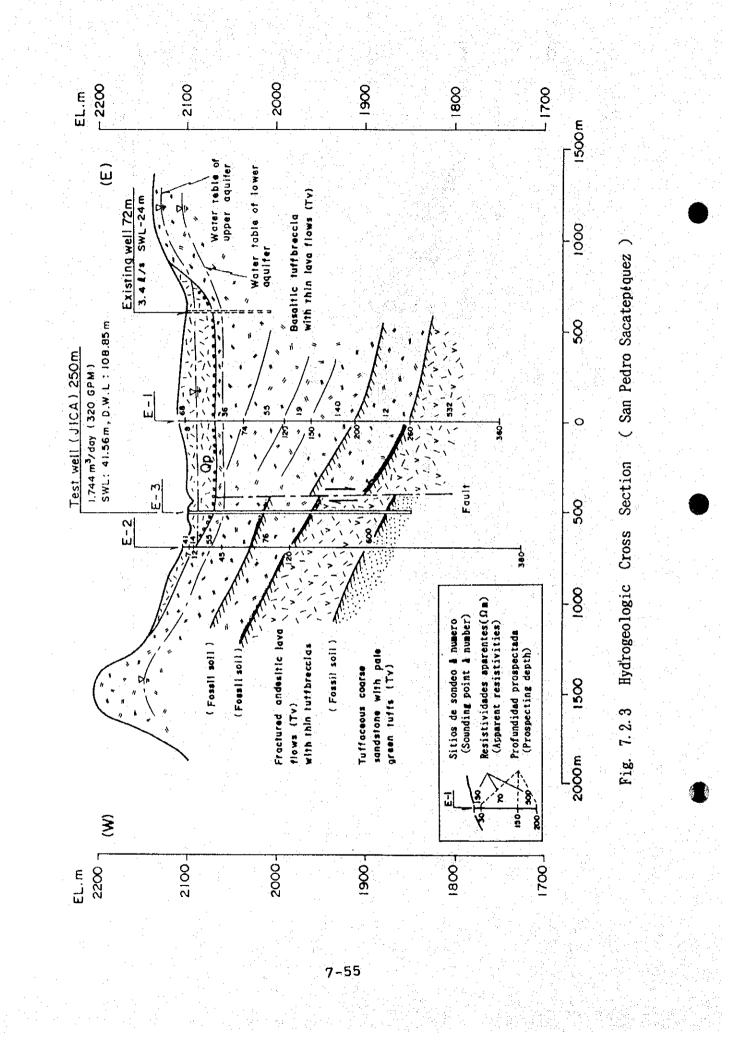
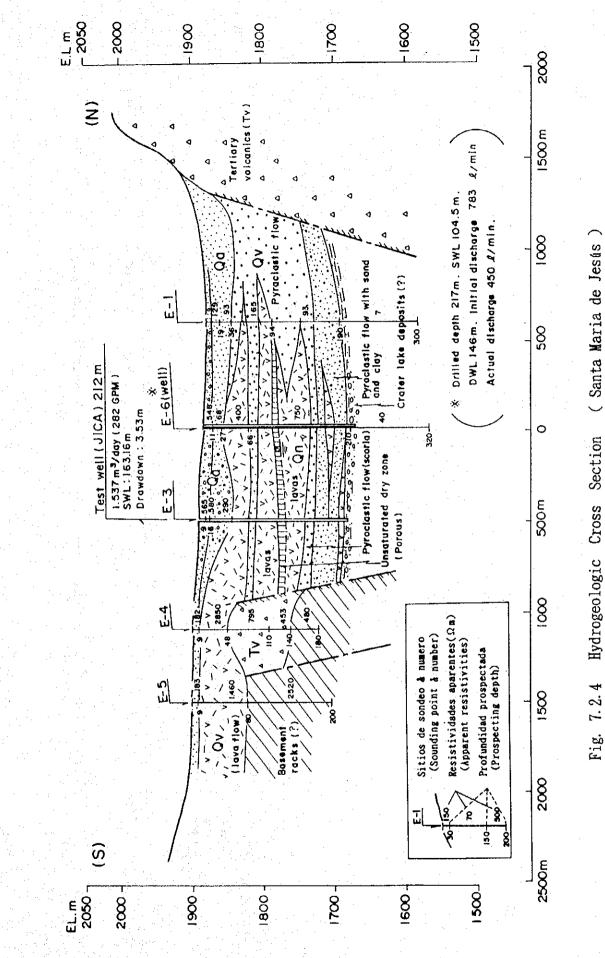
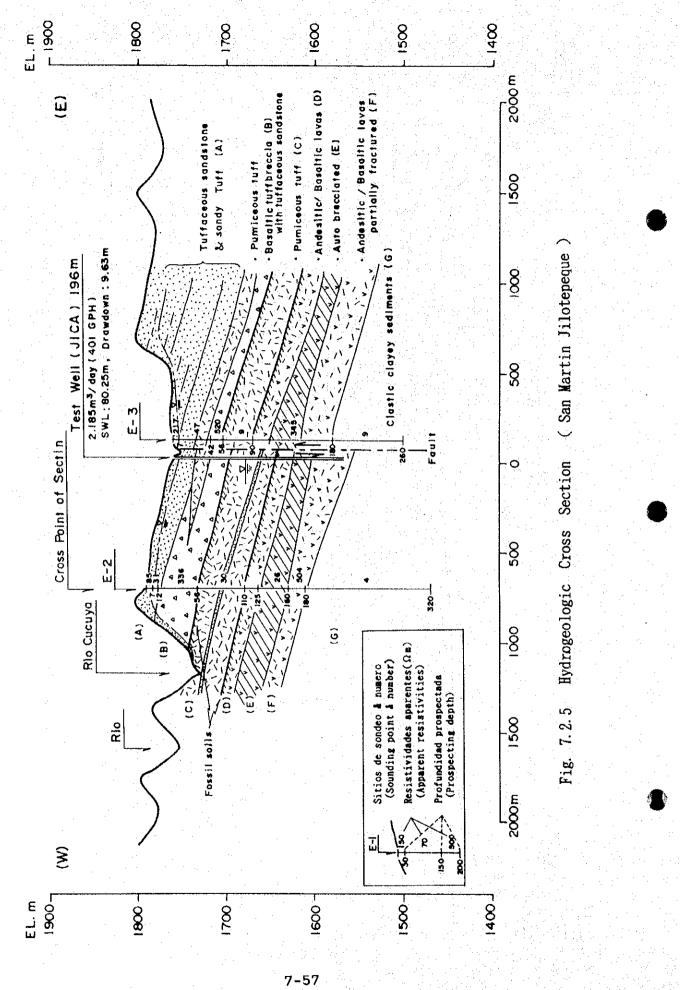
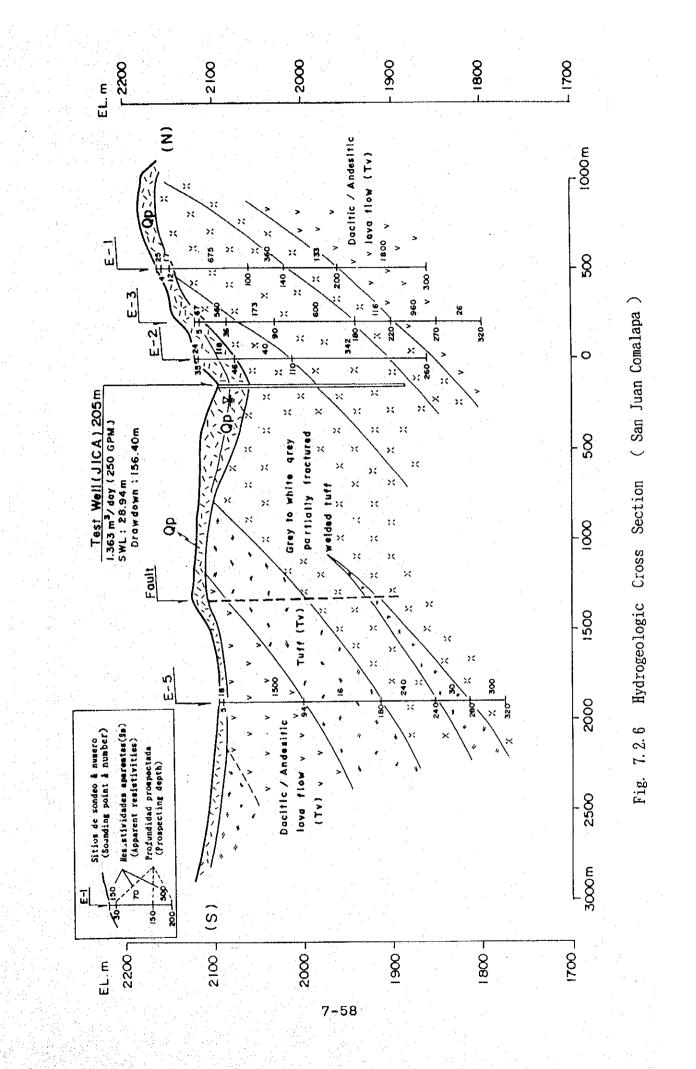


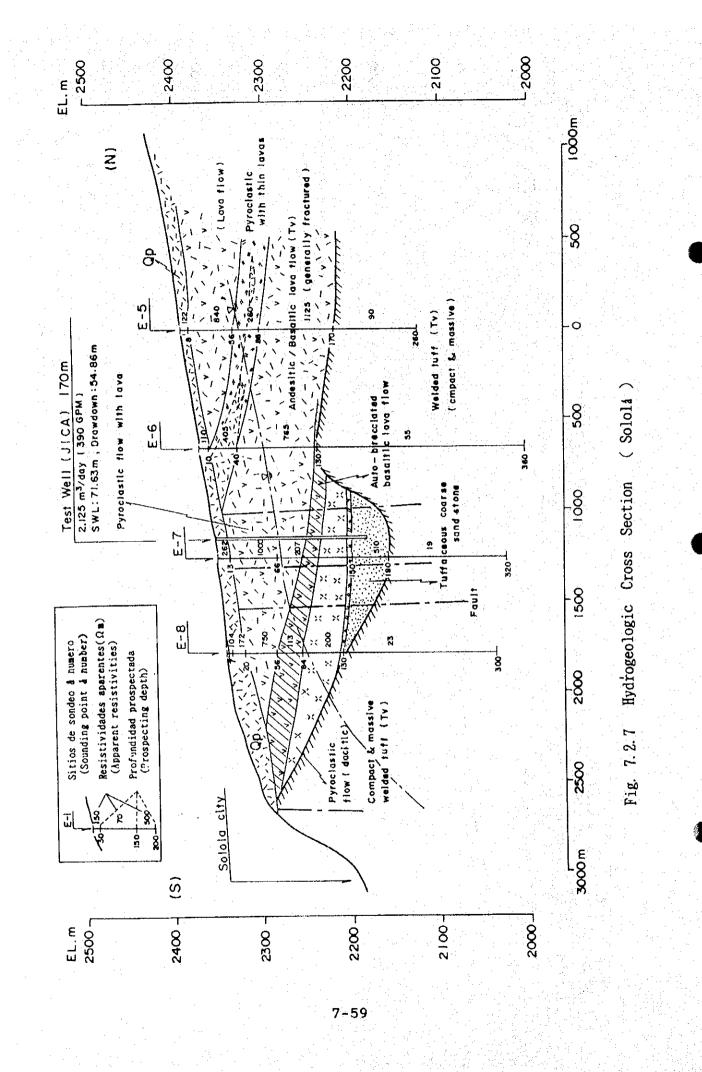
Fig. 7.2.2 Hydrogeologic Cross Section (San José Pinula

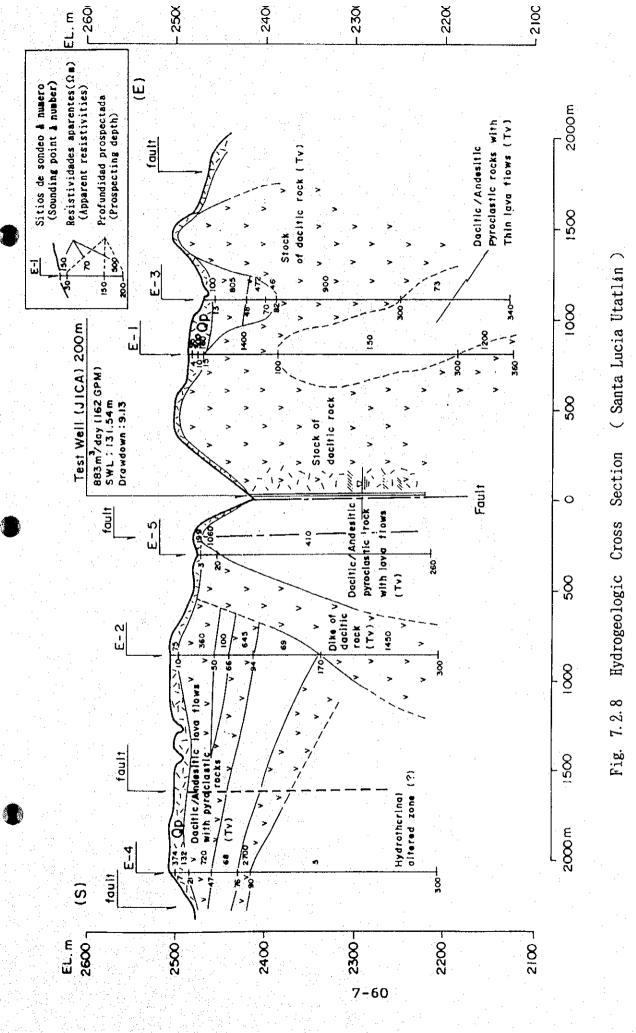


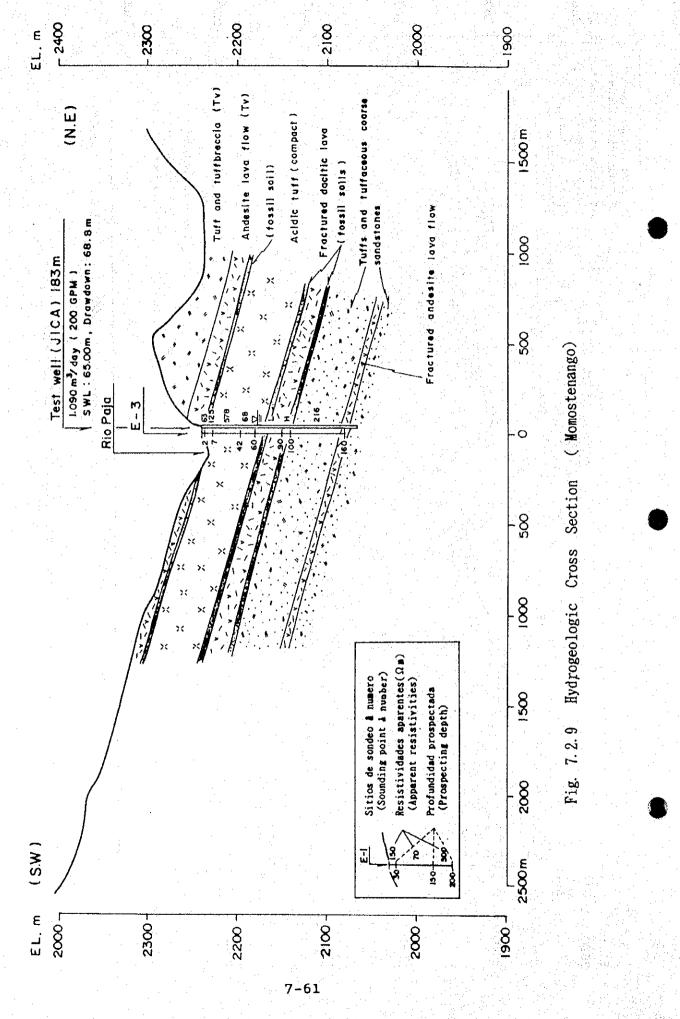


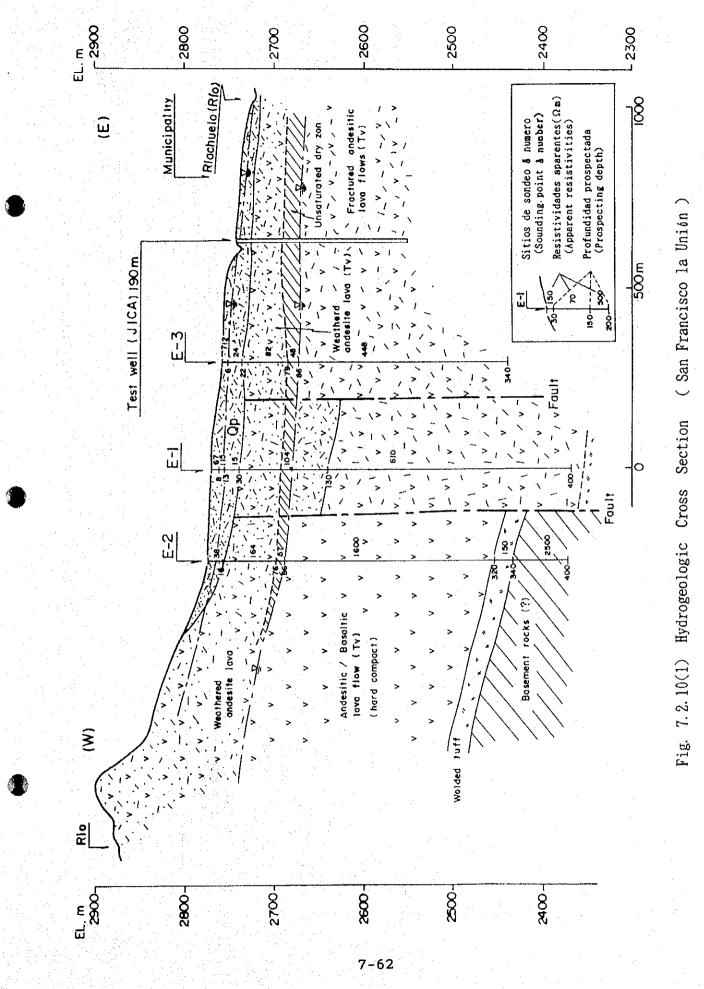


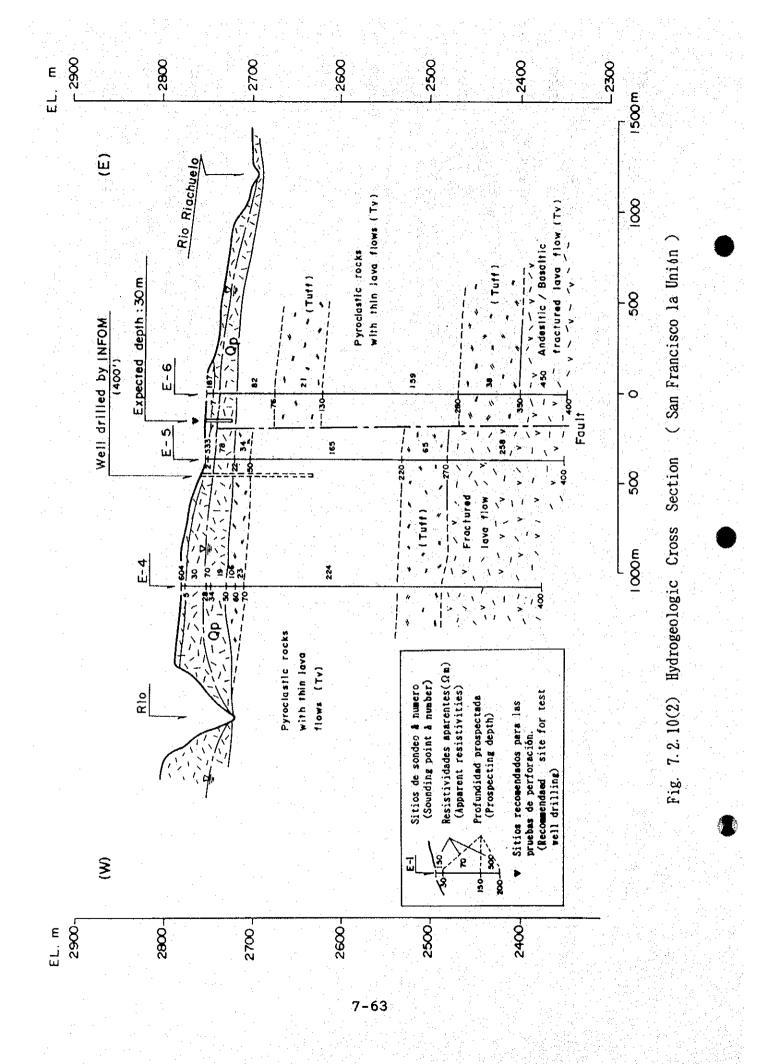


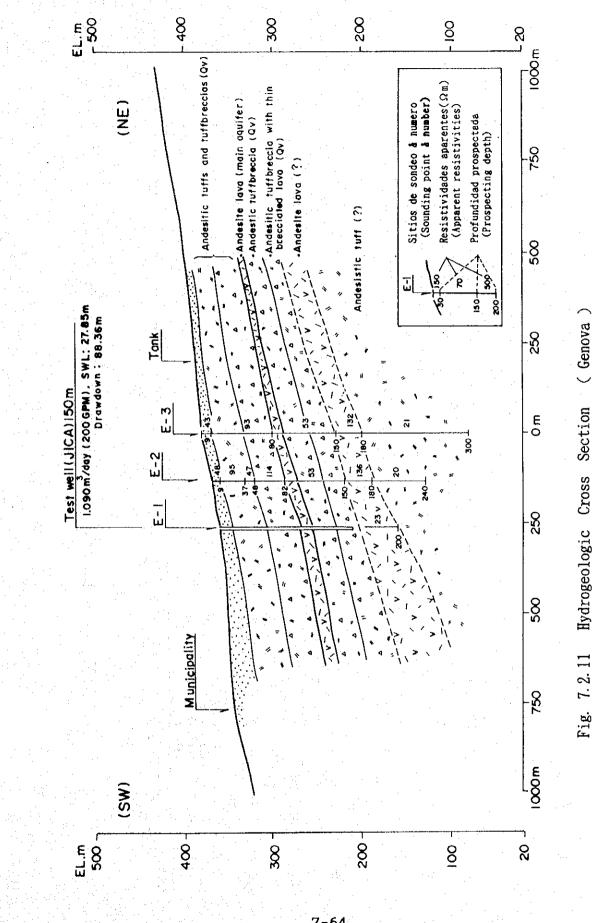


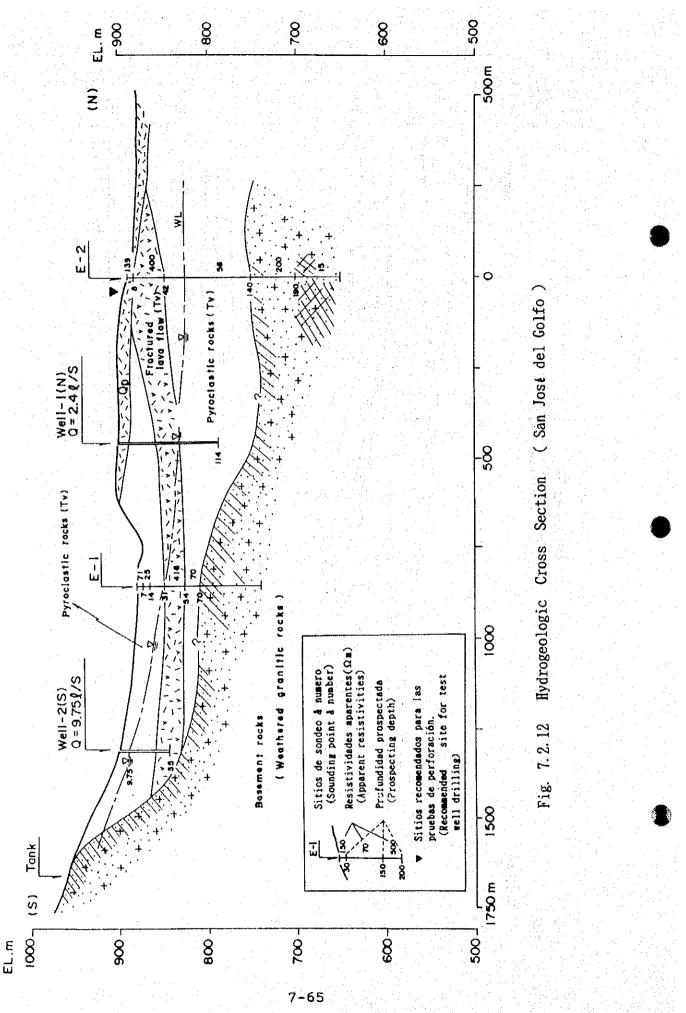


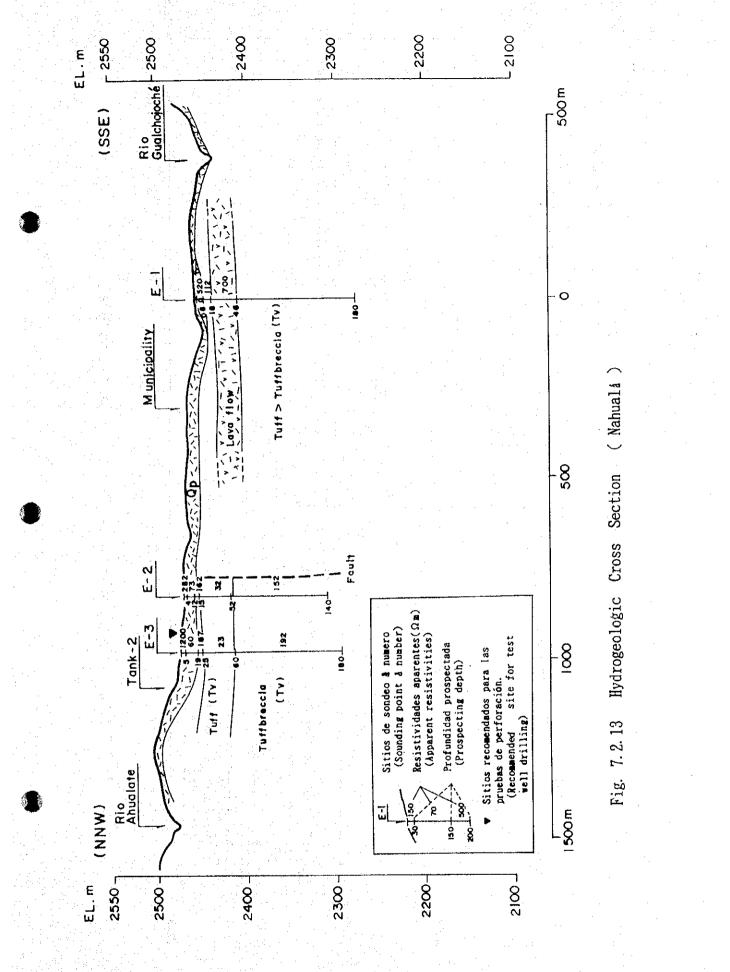


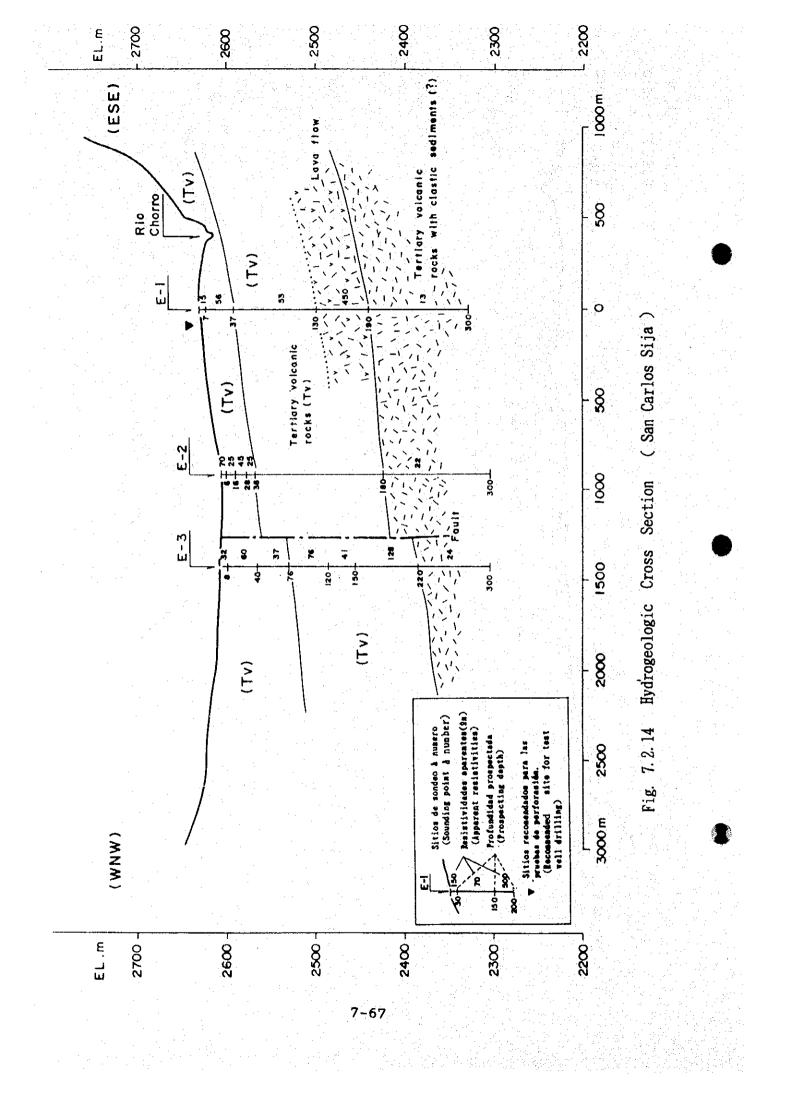


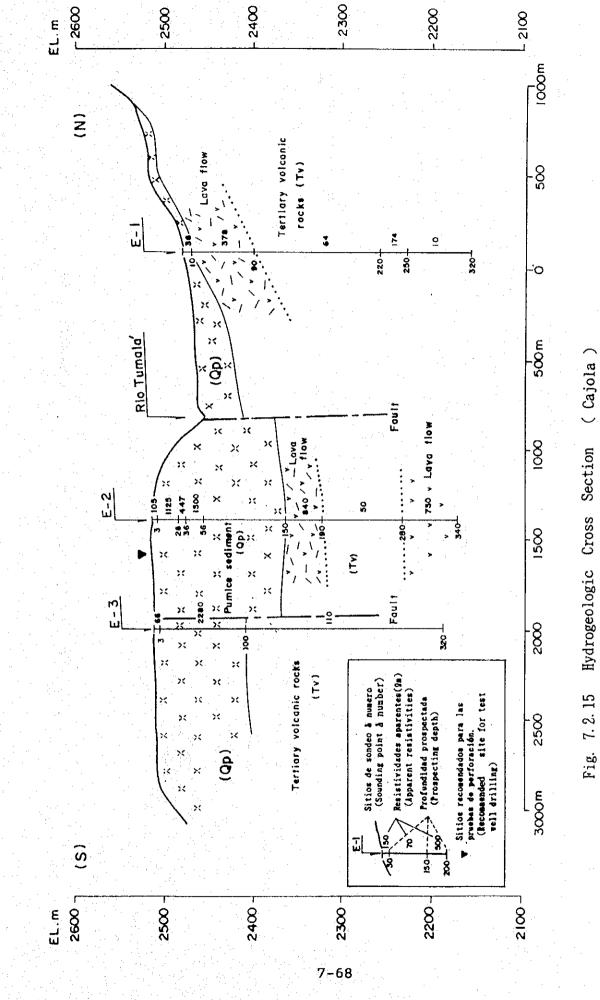


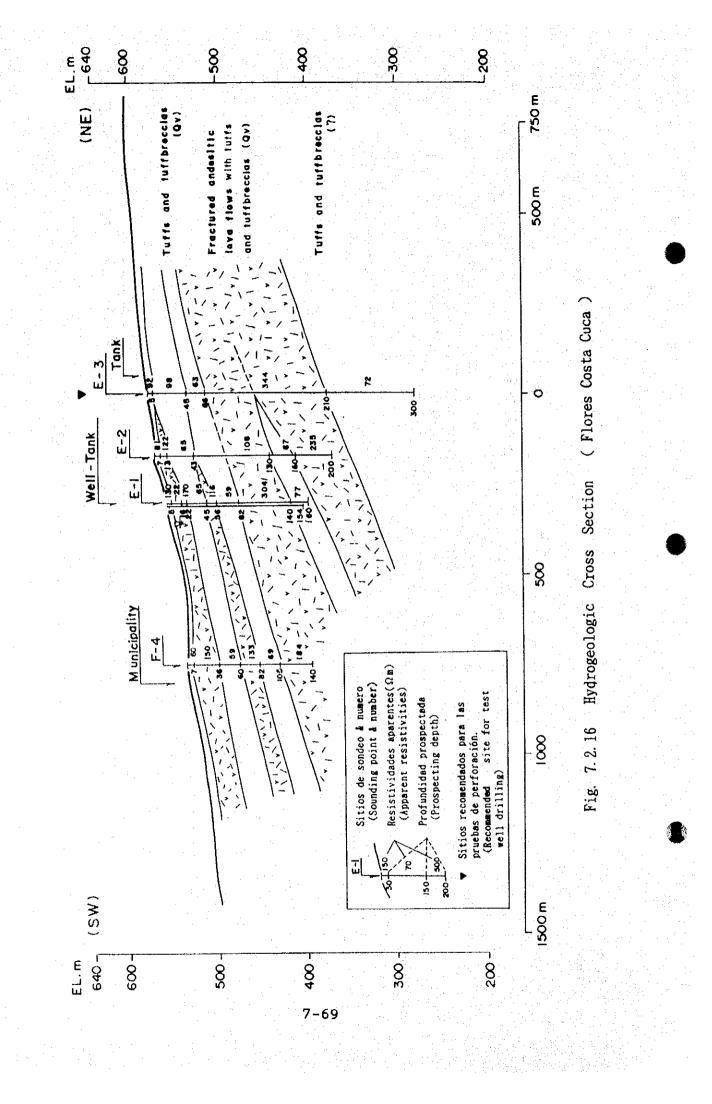


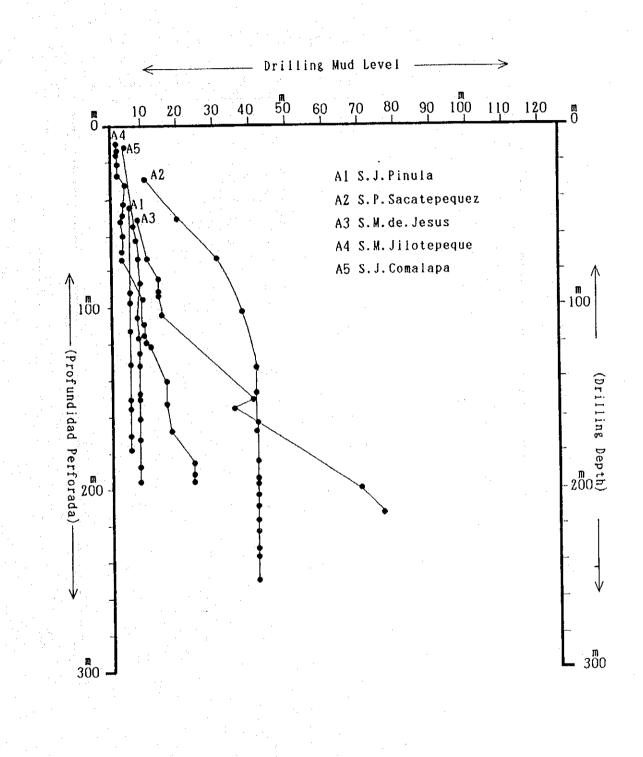


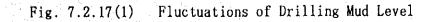


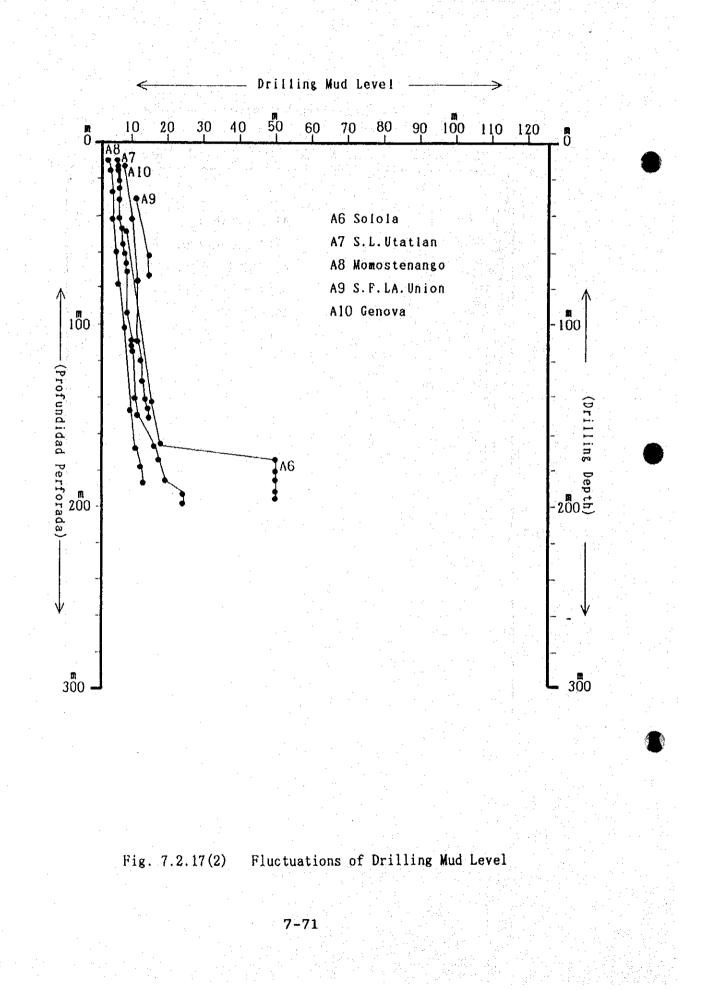












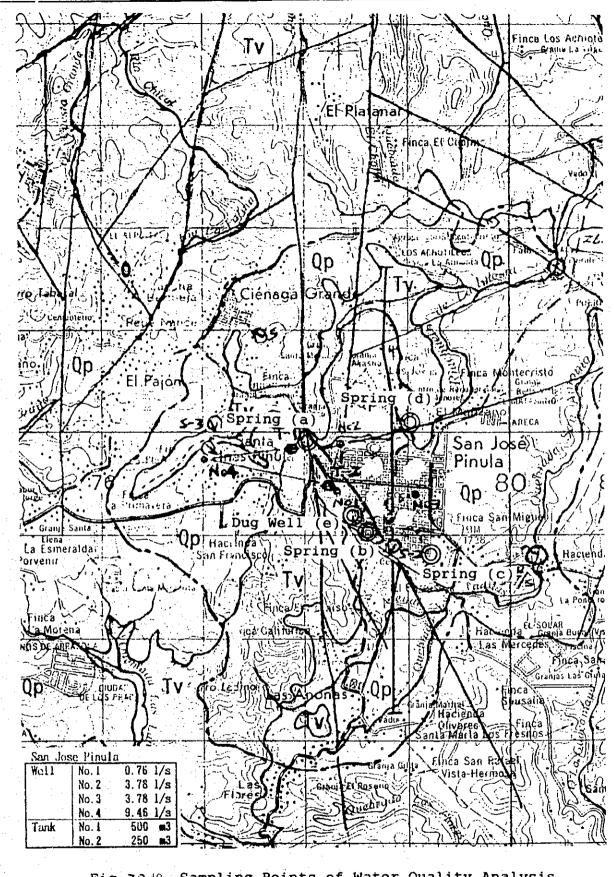
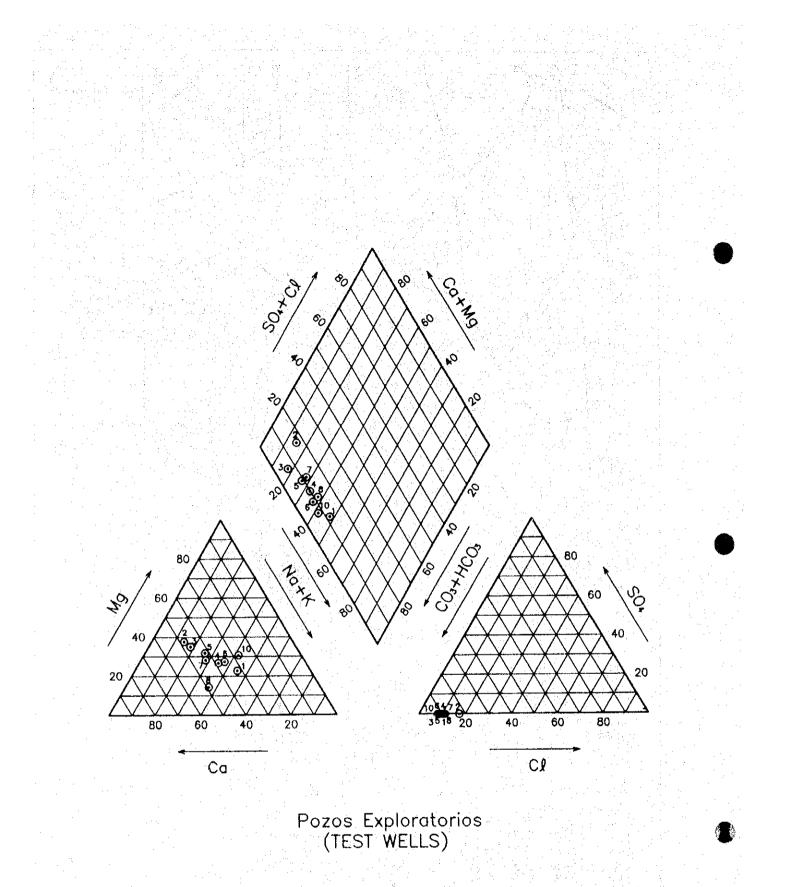
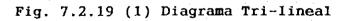


Fig.72.18 Sampling Points of Water Quality Analysis





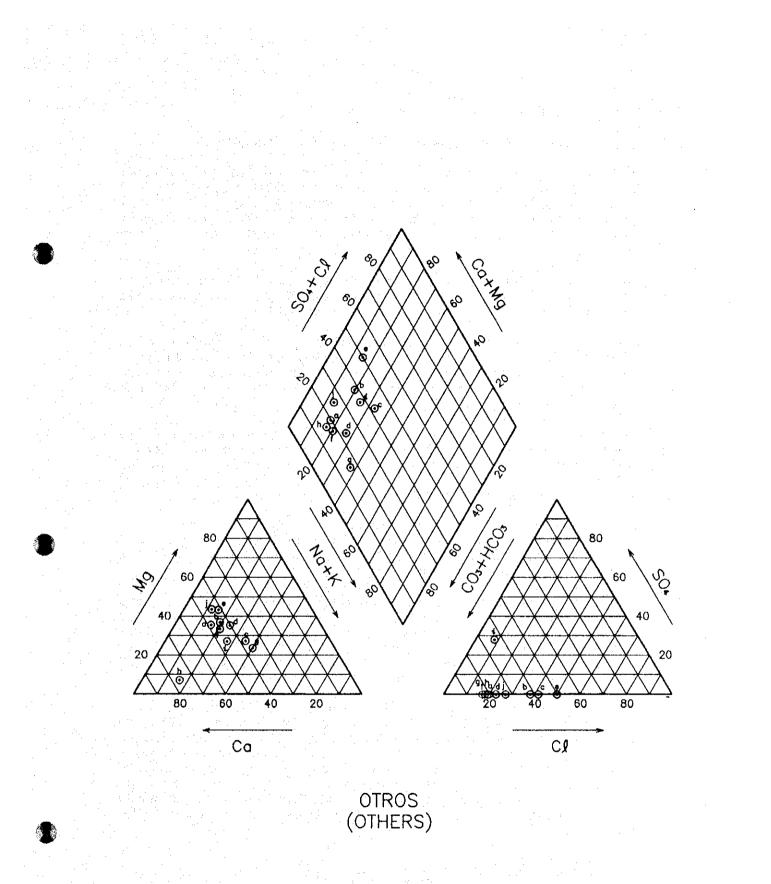


Fig. 7.2.19 (2) Diagrama Tri-lineal

7.3 Groundwater Development Potential

7.3.1 Estimation of Groundwater Development Potential

Little experience has been accumulated on the groundwater development in the Study Area. Hence, insufficient data on hydrogeological information like drilling logs, results of pumping tests and continuous monitoring of the groundwater level and rainfall preclude detailed evaluation of the development scale.

Groundwater development in the Study Area will be based on the upper and lower aquifers in the Pleistocene volcanic and the Tertiary volcanic rocks, respectively.

The aquifer in the Pleistocene volcanic is expected to be especially productive, but its thickness is insufficient for a stable extraction. Therefore, the Tertiary volcanic will be the principal target for future water source development.

For tentative potential evaluation, two methods were applied: the assumed infiltration ratio into the aquifer by geological condition, and the base flow in the dry season.

(1) Estimation of infiltration ratio

Annual rainfall in the municipalities was estimated from the rainfall record. When the municipality did not have a rainfall station, a neighboring station was chosen and its records were adopted for analysis.

Each recharge area was demarcated on the topographic map with a scale of 1:50,000. This area was used for the calculation of groundwater potential. The river basin is basically used as recharge area, however, when the river basin is very big like Villa Canales and Villa Nueva, 50 km2 was applied as the upper bound.

Infiltration factor was applied in the calculation of the groundwater potential. This factor is the ratio of the rainfall that has infiltrated into the aquifer. Adopted to represent this factor is the value indicated in the report "Plan Maestro de Riego Drenaje, Caracterización У· Hidroclimática Hidrogeología" by Ministerio de e Agricultura, Ganadería y Alimentación in 1990. The assumed aquifer recharge from the annual rainfall by geology is as follows.

_	Basement Rock	28
-	Tertiary Volcanic Rocks (Tv)	10%
'	Pleistocene Volcanic (Qp)	15%
-	Alluvial Deposit (Qa)	10%
-	Holocene Volcanic (Qv)	15%
		1 A A A A A A A A A A A A A A A A A A A

Therefore, the annual groundwater potential was calculated as follows.

Annual

Groundwater Annual Recharge Infiltration Potential = Rainfall x Area x Factor

(2) Estimation by river base flow

The value of a river base flow in the dry season is generally regarded as groundwater recharge. Therefore, the groundwater potential is simply estimated from this value and the recharge area.

From the results of the discharge records during 1960 - 1980 and spot measurements in April - May of 1994, specific discharge of the base flow is estimated at about 5 $\ell/\sec/km2$ in most of the Study Area which has about 1,000 mm of annual rainfall, and around 10 $\ell/\sec/km2$ in the southern part of Quetzaltenango which has an annual rainfall of about 3,000 mm.

Annual Groundwater Base Recharge Potential = Flow x Area

Table 7.3.1 shows the results of both calculations. These values represent a tentative evaluation, and the values calculated by infiltration are shown in the hydrogeological map (Fig.7.2.1). This potential should be revised with the monitoring data indicated in Section 7.3.3.

This potential is calculated for the recharge area of each municipality, and corresponds to the total production in the area.

The yielding capacity of one well is estimated at about 5 ℓ/\sec , the same as the value of base flow, because 1 km² is generally regarded as the unit of the recharge area for one well, and, this capacity corresponds to the records of existing wells.

However, the test drilling results showed that the capacity of one well is much higher than this value, as described in Section 7.2., because of the peculiar hydrogeological features of the area, specially of the fault system.

Detailed development potential of the 10 municipalities selected for the Feasibility Study is discussed in the next section. 7.3.2 Groundwater Development Strategy for 10 Municipalities

(1) San Jose Pinula

The town is located in the graben-type groundwater basin where the upper aquifer of Pleistocene pumice sediments and the lower aquifer of Tertiary volcanic rocks with clastic sediments are intercalated. Between these aquifers, there is an unsaturated dry zone. Since the upper aquifer is not thick (several meters) and the water table largely fluctuates seasonally, groundwater in this aquifer is unstable and is not therefore utilized as a public supply source.

The lower aquifer, which consists of basaltic to rhyolitic brecciated lavas and gravel beds with quartz sand, is the main aquifer of the basin. The existing water supply source is the groundwater of this lower aquifer pumped through 3 wells with a depth of 120 meters, and daily production of $613.2m^3$. The yielding capacity of the 3 wells ranges from 1.45 to 9.46 ℓ /s, averaging 4.68 ℓ /s (74.2 GPM).

On the other hand, the yielding capacity of the test well was 31.231/s (495 GPM) with a drawdown of 11.90 meters. The depth of the test well is 180 meters and the static water level is 6.84 maters B.G.L. The difference in the yielding capacities of the existing wells and the test well may be caused by the following two reasons.

- (a) The test well is 60 meters deeper than the existing wells and struck a confined aquifer.
- (b) The test well was properly located, focusing on the fault system, based on hydrogeological and geophysical survey.

Whereas the projected water demand of the municipality in the year 2010 is $3,095m^3/day$ (26.4 ℓ/s), the shortage can be covered by the test well if it is utilized as a production well.

The estimated groundwater potential and water balance of the groundwater sub-basin in the year 2010 are as follows:

-	Recharge area	16km²
		1,650mm
-	Infiltration ratio	14.5 %
-	Groundwater potential	$10,488m^{3}/day$
-	Pumping discharge in 2010	3,095m³/day
	Balance	7,393m³
		and the second

(2) San Pedro Sacatepéquez

The town is situated in a geotectonic intramountain river basin controlled by faults of NW-SE, NS and NE-SW directions. The groundwater bearing layers of the river

basin are Pleistocene pumice sediments (upper aquifer) and locally fractured basaltic to andesitic volcanic rocks of the Tertiary (lower aquifer) formation. In general, an unsaturated dry zone separates the upper and lower aquifers which are unconfined and semi-confined aquifers, respectively.

The thickness of the upper aquifer ranges from several meters to 20 meters and many small scale springs, the important water resources in the area for domestic and agricultural use, gush out from this upper aquifer. The total daily discharge from the springs is estimated at more than $400m^3$, of which $212m^3/day$ is used as municipal water supply.

Groundwater occurrence and the characteristics of the lower aquifer in Tertiary volcanic rocks were unknown, as only the record of one existing well was available. The discharge rate of the existing well is $3.46,49\ell/s$, and the daily production is $97.9m^3$ for an 8 hour pumping rate. The yielding capacity of the test well is $20.19\ell/s$ (320 GPM), with a drawdown of 67.29 meters. The static water level in this well is 41.56 meters B.G.S. in the lower aquifer.

Since the water demand in 2010 is $1,572m^3/day$ and its estimated supply shortage is $1,278m^3/day$ (14.8 ℓ/s), the shortage can be covered by this test well.

The estimated groundwater potential and water balance in the area in 2010 are as follows.

- Recharge area	4.0km ²
- Annual rainfall	1,032mm
- Infiltration ratio	13.0%
- Groundwater potential	1,470(~1,728)m ³ /day *1
- Pumping discharge in 2010	1,760 m ³ /day *2
(including spring water)	
- Balance	290(~-32)m ³
*1: Groundwater potential estimated by infilt	ration factor is 1,470m ³ /day, and 1,728m ³ /day

by base flow flow factor.

*2: Breakdown of pumping discharge (including spring water) in 2010 is as follows:

Pumping discharge from 2 wells	1,572m³/day
Spring water for agricultural use	188m³/day

Since the water balance in this area in 2010 will become negative at a value between 290 to $300m^3/day$, the groundwater monitoring is very important, as described in the next section.

(3) Santa María de Jesús

As shown in Fig. 7.2.1 (2) and 7.2.4, the well is located

on a flat plain (Sabana Grande) surrounded by steep mountains. The basin is mainly filled up with Quaternary volcanic rocks originating from Volcan de Agua, and the basement is formed by slightly permeable Tertiary volcanic rocks.

This basin has two aquifers, the upper and lower aquifers, which are separated by an unsaturated dry zone. The water table is 16 meters B.G.S. in the upper aquifer, and 163 meters B.G.S. in the lower aquifer.

The yielding capacity of the lower aquifer, confirmed by test drilling, is $17.79\ell/s$ (282 GPM) with a drawdown of 3.53 meters, while the yielding capacity of the existing well is 6.0 ℓ/s (95 GPM).

The water demand in 2010 is $2,308m^3/day$, and the supply shortage of $1,617m^3/day$ ($18.7^3/s$) can be covered by the production from the test well.

The estimated groundwater potential and water balance in the groundwater basin in 2010 are as follows:

-	Rechange area	14.0km ²
Here	Annual rainfall	1,229mm
	Infiltration ratio	13.0%
-	Groundwater potential	7,071m ³ /day
-	Pumping discharge in 2010	2,308m ³ /day
-	Balance	4,763m ³

(4) San Martín Jilotepeque

The municipal area is situated in the intramountain basin of the Pixcayá River basin, consisting mainly of Tertiary volcanic rocks.

The main aquifer of the basin is in a layer of fractured and auto-brecciated andesitic to basaltic lavas of Tertiary volcanic rocks. The yielding capacity of the aquifer confirmed by test drilling is $25.30\ell/s$ (400 GPM) with a drawdown of 9.63 meters, and the static water level is 82 meters B.G.S., while the yielding capacity of the existing well is $6.0\ell/s$ (95 GPM).

Production from the test well can cover the estimated municipal water supply shortage of $1,032m^3/day$ (11.95ℓ/s) in 2010.

The estimated groundwater potential and water balance of the basin in 2010 are as follows:

-	Recharge area	7.0km ²
-	Annual rainfall	1,272mm
-	Infiltration ratio	15.0%
_	Groundwater potential	3,659m ³ /day
	Pumping discharge in 2010	

2,109m³

Balance

(5) San Juan Comalapa

The municipal area is located in the intramountain basin of the Pixcayá River system, consisting mainly of Pleistocene pumice sediments (upper aquifer) and Tertiary volcanic rocks (lower aquifer).

The upper aquifer is composed of relatively highly permeable layer of pumice sediments with lake deposits of sandy materials several meters thick. There are many springs gushing out from the upper aquifer, and the estimated total discharge is more than $15\ell/s$.

The lower aquifer is in a layer of partially fractured welded tuffs of the Tertiary. The productivity of the test well drilled down to the lower aquifer is not high at $15.78\ell/s$ (1,363m³/day, 250 GPM), with a huge drawdown of 156.4m from the static water level of 28.94 meters B.G.S. For a cost effective pumping, drawdown should be lowered to within 80~90 meters by reducing the pumping rate to $12.0\ell/s$ (about 1,000m³/day, 190 GPM). The existing well in this town yields 6.421/s (102 GPM).

Since the supply shortage is estimated at $1,954m^3/day$ in 2010, the development of one more well producing about $1,000m^3/day$ is required in order to meet the demand in 2010.

The point recommended for drilling the additional well is shown in Fig. 9.1.5, based on hydrogeological surveys. However, more detailed surveys should be conducted to determine the proper location and depth of the well prior to undertaking construction work.

The estimated groundwater potential and water balance of the basin in 2010 are as follows:

-	Recharge area	16.0km²
	Annual rainfall	1,414mm
_	Infiltration ratio	13.08
	Groundwater potential	8,058m³/day
-	Total discharge in 2010	3,793m³/day
	(from 3 deep wells)	$(2, 493m^3/day)$
	(from springs)	$(1, 300m^3/day)$
-	Balance	4,265m³

(6) Sololá

The area in Sololá for groundwater development is on a flat plateau about 2 kilometers north of the city, where the Tertiary volcanic rocks with clastic sediments (lower aquifer) is unconformably overlain by thin Pleistocene pumice sediments (upper aquifer). The upper aquifer is only several meters thick in the area, but springs gushing out from the aquifer discharge a total amount of more than 45ℓ/s (3,888m³/day). An unsaturated dryzone separates the upper and lower aquifers.

The main lithological units of the lower aquifer are auto-brecciated and fractured basaltic to andestic lavas and tuffaceous coarse grained sandstone. The yielding capacity of the lower aquifer, confirmed by test drilling, is 24.59 l/s (390 GPM) with a drawdown of 54.86 meters. Static water level is 71.63 meters B.G.S.

If this test well is used as a production well, the pumping rate should be reduced from $24.59\ell/s$ to a $13.0-14.0\ell/s$ (1,100-1,200m³/day), in order to keep the pumping level at less than 100m B.G.S., for safe pumping (economical pumping).

To cover up the supply shortage of the municipality in 2010 which is estimated at $2,172m^3/day$ (25.141/s), another well should be constructed in addition to the test well.

The site recommended for the additional well was selected by conducting hydrogeological surveys, and shown in Fig. 9.1.6. However, a more detailed survey is required prior to well construction in order to properly pinpoint the well site and determine well depth.

The estimated groundwater development potential and water balance of the area in 2010 are as follows:

-	Recharge area	18.5km²
·	Annual rainfall	1,081mm
-	Infiltration ratio	14.5%
-	Groundwater potential	7,945m³/day
-	Total discharge in 2010	6,060m ³ /day
	(from deep wells)	$(2, 127m^3/day)$
	(from springs)	$(3,888m^{3}/day)$
-	Balance	1,885m³

(7) Santa Lucía Utatlán

The municipal area is located in the intramountain basin of Rio Quiscab, consisting mainly of Tertiary volcanic rocks conformably overlain by thin pumice sediments of the Pleistocene.

The main aquifer is in the fractured portion of the Tertiary volcanic rocks. The test drilling site was selected targeting the fractured zone of auto-brecciated dacitic rocks along the NE-SW lineament, which sharply controls the flow direction of streams. The yielding capacity of the fractured zone, confirmed by test drilling, is $10.22\ell/s$ (162 GPM) with a drawdown of 9.13 meters. The static water level is 131.45 meters B.G.S. The supply shortage of the municipality in 2010 which is estimated at $344m^3/day$ ($3.98\ell/s$), can be sufficiently covered up by the production of this test well. The estimated groundwater potential and water balance of the area in 2010 are as follows:

- Recharge area	5km ²
- Annual rainfall	1,341mm
- Infiltration ratio	13.5%
- Groundwater potential	$2,480m^{3}/day$
- Total discharge in 2010	506m ³ /day
(from deep well)	(344m ³ /day)
(from springs)	(162m³/day)
- Balance	`1,974m³ ¯́

(8) Momostenango

The municipal area is located on the mountainous highland of Tertiary volcanic rocks. A hot spring (48.1) c, pH 6.5, EC 94 µs/cm) gushes out from the river bed. Test drilling has revealed the possibility of extracting potable water (20.0) c, pH 7.0, EC 53 µs/cm), even in the geothermal area.

The main aquifers of the area, confirmed by test drilling, are found in layers of the fractured dacitic and andestic lavas and tuffaceous coarse sandstone, and the yielding capacity of the well is 12.622/s (200 GPM) with a drawdown of 70.3 meters. The static water level is 63.5 meters B.G.S.

One more well should be constructed to cover up the supply shortage of the municipality in 2010 which is estimated at $1,955m^3/day$ (22.63 ℓ/s). The site recommended for the additional well is shown in Fig. 9.1.8, however the final drilling point and depth shall be determined after conducting further detailed hydrogeological survey.

The estimated groundwater development potential and water balance of the area in 2010 are as follows:

- Recharge area	18.0km²
- Annual rainfall	1,341mm
- Infiltration ratio	10.0%
- Groundwater potential	6,613m³/day
- Total discharge in 2010	3,182m³/day
(from deep wells)	(1,955m³/day)
(from springs)	(1,227m³/day)
- Balance	3.431m ³

(9) San Francisco La Union

The municipal area is situated in the intramountain basin of the Rio Salamá system, consisting of Pleistocene pumice sediments (upper aquifer) and Tertiary volcanic rocks. As described in Section 7.2.2 (3), the test drilling has revealed that this area is very difficult for groundwater development by deep well construction, due to the existence of a very porous unsaturated dry zone between the upper and lower aquifers (see Fig. 7.2.10 (1)). Therefore, the upper aquifer development by the construction of a large but shallow well is recommended to be more effective and economical. Particularly, since the supply shortage of the municipality in 2010 is estimated to be small at $271m^3/day$ (50 GPM). The site and design recommended for the shallow well are shown in Fig. 7.2.10 (2) and Fig. 9.1.9.

The estimated groundwater potential and water balance of the municipality in 2010 are as follows:

-	Recharge area	6km²
••	Annual rainfall	843mm
	Infiltration ratio	13.5%
_	Groundwater potential	1,871m ³ /day
-	Discharge in 2010	271m ³ /day
	Balance	1,600m ³
÷.		

(10) Génova

The municipal area is located on the flat plain at the foot of Quaternary volcances, consisting of pyroclastic rocks with thin lavas and volcanic mud flow layers.

The main aquifer of the area, confirmed by test drilling, is the fractured layer of andesitic lavas of Quaternary volcanic rocks and the yielding capacity is 12.62ℓ/s (200 GPM) with a drawdown of 88.36 meters. The static water level is 27.85 meter B.G.S.

The supply shortage of the municipality in 2010 which is estimated at $770m^3/day$ (8.92 ℓ/s) can be covered up by the production of the test well. The existing spring water source (3.03 ℓ/s) should be utilized for agriculture instead.

The estimated groundwater potential and water balance of the area in 2010 are as follows:

	Recharge area	10km ²
-	Annual rainfall	3,640mm
-	Infiltration ratio	15.0%
	Groundwater potential	14,959m³/day
-	Pumping discharge	770m³/day
-	Balance	14,189m³

7.3.3 Groundwater Level and Monitaring Plan

(1) Objectives of Monitoring

Groundwater resource development in this Study Area is in the initial stage, therefore, no monitoring system has been installed in any of the candidate municipalities.

The objectives of the groundwater monitoring required in

the Study Area are summarized below:

 to collect basic hydrological data to analyze water balance in the hydrogeological basin, and to evaluate future groundwater development potential

to collect basic hydrological and water quality data, as well as their long term variations, for a rational river basin management

(2) Installation of Groundwater Level and Rainfall Recorders

Automatic rainfall and groundwater level recorders were installed at the wells in the 3 municipalities of San José Pinula, San Pedro Sacatepéquez and Comalapa (See Fig. 2.6.1).

The monitoring results so far obtained by December 1994 are described below.

(a) San José Pinula Station

The groundwater level of the existing well in San José Pinula has been periodically measured by use of a handy water level detector from June 1994. This well was drilled by the municipal government to a depth of 213 m, but was abandoned because of low productivity, 0.76 l/sec. The automatic water level recorder was installed at this well and a continuous record has been taken since November 16, 1994.

Fig. 7.3.1 shows the fluctuation in water level in the past 6 months. The groundwater level was around 32.16 m from June to September, and has gradually increased from October.

(b) San Pedro Sacatepéquez

The test drilling well in San Pedro Sacatepéquez was used for water level monitoring. Information on the well is as described in Section 7.2.2.

(c) Comalapa

Monitoring equipment was installed at the test drilling well in Comalapa. Information on the well is as described in Section 7.2.2.

Monitoring will be continued by INFOM, and the results will be utilized for water resources investigation in the future.

Table 7.3.1 Tentative Evaluation of the Groundwater Development Potentiality (1/2) -

. 1	Department	Municipality	A. RAIN	P. Area (km2)	Geology Type		Perc	enta	ge	1	Potentia	Potential (m3/day)		
			(1002)			BR	Tv	Qv.	Qp	Qa	by filtration	by base flow		
				i				1.1						
						{ · ·		· · .		11		a second a second		
1	Guatemala	Chinautla	1135	3	TYOP	10	-	-	90	-	127	3 129		
2		Chuarrancho	1063	4	BR	I	-	-	~	.) - .	-	-		
3		Wixco	1197	16	TVOP2	- 1	80	-	20	· -	577	2 691		
4		San José del Golfo	1063	3.5	BR	40	60	: - .	-	1 n. 4 .	, 59	3 151		
5		San José Pinula	1650	16	TYOP2	-	10		90		1048	8 691		
5		San Juan Sacatepéquez	1032	14	BR	70	. • -• •	1. - 1	30	-	233	5 604		
7		San Raymundo	1122	1	BR	10	30	-	60	-	262	6 302		
8		Santa Catarina Pinula	1343	12	TVOP	-	10	-	90	÷.	640	0 518		
9		Villa Canales	1524	50	TYAL	- 1	60	÷ -	40	÷,≠	2505	2 2160		
10		Villa Nueva	1213	50	TVOP	· - ·	-	-	100	-	2492	5 2160		
11		San Pedro Ayampuc	1063	10	BR	80	20	-	-		104	8 432		
12		San Pedro Sacatepéquez	1032	4	TVOP2	-	40	-	60	- 1	147	0 172		
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										}				
13		Ciudad Vieja	992	15	Qγ	-	30	40	-	40	530	648		
14	Datascierciation	Jocotenango	1031	11.5	TVAL.] +	80	-	20	-	357	3 496		
15		Magdalena Milpas Altas	1031	4		-	60	-	40	-	138	6 172		
16		San Antonio Aguas C.	992	5	TYAL	-	90	-	-	10	135	9 216		
17		San Bartolome M. Altas	1031	3	TV	1 -	100	- 1	-	-	84	17 129		
18		Santa Lucía M. Altas	1031	2.5	TYOP2	1 -	20	-	80	-	98	19 108		
19		Santa María de Jesus	1229	14	QY	-	÷-	100	- 1	- :	707	604		
20		Santa Catarina Barahona	992	3	TVAL	-	70	-	- 1	30	81	15 129		
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21	Chimaltenango	Comalapa	1414	16	TVOP2	-	40	1	60	-	80	58 69		
22	1 · · ·	El Tejar	1234	I .б	TYOP		60	-	40	-	24	34 25		
23		Patzicia	1283	5,5	- 1	· [30	1 -	70	-	26	10 23		
24		Patzun	1283	i	1 .	- 1		-	200	1 -	94	91 77		
29		San Jose Poaquil	1272			· -	100	- 1	-	-	22	65 28		
26		San Martín Jilotepeque	1272		1	-	-	-	100		36	59 30		
20		Zaragoza	1283		TVQP2	1 -	50	-	40		29	53 30		
41		101 05ULA						ł						

Table 7.3.1 Tentative Evaluation of the Groundwater Development Potentiality (2/2)

N	o	Department	Municipality	A. RAIN	P. Area	Geology	Percentage					Potential (m3/day)			
L				(mm)	(km2)	Туре	BR	Τv	٩v	Qp	Qa	by	filtration	by base	flow
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	28	Sololá	SoloTa	1081		TYOP2	} -	10		90	-	ì	7945	ļ	799
	29		Nahuala	1341	41	TYOP2	-	90	-	10	-	ł	15816	1	1771
l	30	and the second second	San Andres Semetabaj	1010	•. 4	TVQP2	-	50	- 1	50] -	ļ.	1384	ł	172
ľ	31		San Antonio Palopo	1010	2.5	עדן	-	70	-	30	- 1	}	796	1	108
	32		San Juan la Laguna	1010	13	l i	-	50	-	.40	10	1	4317		561
	33		San Marcos la Laguna	-1010	6	ÎTVAL 🗆	į -	90	-	-	10	ŀ	1660		259
	34		San Pablo la Laguna	1010	. 6	TYAL .	-	90] -	- 1	10	ļ	1660		259
	35.		Santa Catarina Ixtahuaca	1341	16	rν	-	70	-	30	-	ļ	6760	1	691
	36		Santa Catarina Palopo	1010	3.5	אזן	-	90	-	10	-		1017	1	151
	37		Santa Clara la Laguna	1010	3	TVQP2	-	10	-	90	-	ţ	1204		129
l	38		Santa Cruz la Laguna	1010	2	TYAL	-	5	-	95	-	ţ.	816		86
l	39		Santa Lucia Utatlan	1341	5	TVOP2	-	30	-	70	-	ŀ	2480		218
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	40	Totonicapán	Nomostenango	1341	18	TV	-	100	-	-	ĺ		8613	}	771
	41		San Andres Xecul	843	6	TVOP		50	-	50	-	ļ	1732		259
ł	42		San Francisco el Alto	1341	4.5	TVQP2	-	50	} -	50	ļ -	ļ	2067		194
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ł						1	ł		ł		l	1			
ĺ	43	Quetzal tenango	Almolonga	1594	1 11	TVQP	-	70	-	30	-	1	5524		475
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ĺ	46	· ·	Cajola	1057	34	(TVQP	-	95	-	5	-	ĺ	10092		1468
ĺ	47		Flores Costa Cuca	3640	9	ģγ	ļ -		100	-	-		13463		388
I	48		Genova	3640	10	QY	-	-	100	-	-	ļ	14959		432
l	49		Huitan	936	5.5	BR	50	50	-	-	-		846		237
l	50		Olintepeque	843	3.5	TYOP	-]	70	[_	30	-	{	930		151
	51		Palestina de los Altos	1027	17	TY	-	70	-	30	-	ļ	5501		734
	52		San Carlos Sija	1027		TVQP2	30	30	-	40	-	}	4862		777
I	53		San Francisco la Union	843	6	TVQP2	-	30	-	70	-		1871		259
	54		San W. Sacatepéquez	2100	-	TYOP	- 1	80	} _	20	- 1	Ì	6329		432
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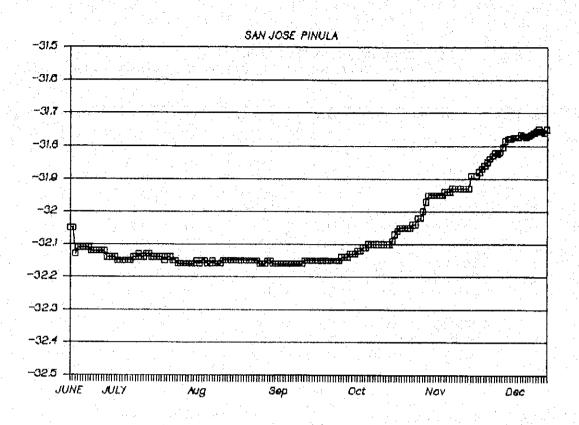


Fig 7.3.1 Daily Groundwater Level in San Jose Pinula