landscape layout of the roundabout shall be established in accordance with design of flyover structure. However some of the original design components and materials would be better reused if possible.

(5) New Establishment of the Roundabout Monument

Some of the roundabout and junction areas are on plane ground, and awaiting establishment of roundabout landscaping with some symbolic monument for the local vicinity areas.

For such roundabouts and junctions, flyover structure design may be considered to meet establishment of new individual monuments on the sites. Major objective of landscaping of the areas may be proposed that these flyovers themselves come to be aesthetic landmarks with some design elements reflecting new landscape design concept.

7.2.4 Evaluation of the Comparative Types A, B, C and D

The comparative Types A, B, C and D will be evaluated from their aesthetics, road alignment, roundabout configuration, and support column arrangement.

(1) Basic Aesthetic Consideration of Flyover Types

Following are 4 types of flyover classification in accordance with 5 categories of landscape consideration and monument criteria.

Landscape analysis and study for flyovers at roundabouts and junction is shown in Table 7.3 (1) \sim 7.3 (2).

① Flyover Type-A

Flyover Type-A will be formulated in a straight overpass over the center of the roundabout or junction. This is the lowest priority for the preservation criteria on the existing monument and roundabout. The construction of flyover will chiefly directly affect landscaped areas. Therefore major landscaped objects must either be relocated or rearranged in order to fix the new layout of the roundabout and flyover structure.

Table 7.3 (1) Landscape Analysis and Study for Flyover at Roundabouts and Junctions

Recommended	I-lyover Lype	Type-D (Type-C)	Type.A	Typc-A or (Typc-B)	Type-B	Type-A	Type-D or (Type-C)	Type-A	Турс-А	Typc-A	Type-A or (Type-B)
T	Type-D	*		•	1	1	*	ı	,	•	1
Pyover Type Consideration	Type-B Type-C	* Mt. side	1		ŧ.	1	* Mt. side	,	1	•	1
ver Type	Type-B	t		*	#	-	-	•	•	ı	* Dawn view
Flyo	Type-A	3	*	*	*	*	•	* Shade prov.	* Shade prov.	* Shade prov.	* Shade prov.
cnts	Continuity	Omani arch design element	Palm grove, Arch-type design	Green and plants	Good view to Mt side	Green elements	Green or horizontal elements	Green and flower elements	Trees and Palms	Trees and Palms	Trees and Palms
Vicinity Landscupe Components	Locality	Even at seaside, Mt. side	Flabitat at seaside	Flabitat at scaside	Township at seaside	Mt. side	Township at seaside	Even at both side	Township at seaside	Even but rather seaside	Even but rather seaside
Vicinity Land	Characteristics	Flat dry land, Date palms row on the road side, view to Mt.	Flat dry land, Date palms and groves at medium distance	Small township with canopy trees	Flat land with palm grove at Mr. side	Township on Mt. side Groves at seaside	Township on seaside	Township on seaside	Township on scaside Groves at Mt. side	Township on seaside	Township on seaside
	Visibility	Clear vertical	•	Lower	Lower	1	Clear vertical horizontal	Lower	Lower horizontal		Lower horizontal
R/A and JN	Perception	Distinctive monument silhouette		Level to downward view	Level to downward view	Florizontal	Distinctive monument silhouette	Level to downward view	Level to downward view	Horizontal expansion	Level to downward view
Monument and Landscape Components at	Dimension	H = 12.1 m W = 12.0 m L = 12.0 m	•	Ii = 0.5 m W = 15.0 m	II = 2.5 m W = 15.5 m L = 16.5 m		H=20.0 m W=15.0 m L=31.0 m	H = 2.0 m Ground level	II = 2.0 m Ground level		H = 2.0 m Ground level
d Landscape	Direction	All directions	•	AJJ directions	All	1	Dhow faces to the sea	All directions	All directions	•	All directions
Monument and	Characteristics	Traditional Omani citadel arch gate	Plane ground, Paved sidewalks (Garden reserve site)	Flat garden and water fountain	Rock piled mound with animal sculptures on the flat garden	Plane ground (Garden reserve site)	Dhow on the water and flat garden surroundings	Flower garden on the slight mounded green	Green mound with flower bed	Plane ground	Mounded garden with shrubs and medium trees
Location	Super- intendency	Bait Al Bamkah (M.M)	Mascem Garden (M.P.E.)	Barka (M.P.E.)	Masna'ah (M.P.E.)	Muladdah (M.P.E.)	Wudam As Sahil (M.P.E.)	Suweiq (M.P.E.)	Khaburah (M.P.E.)	Al Hijari (M.P.E.)	Saham (M.P.E.)
R/A &	z	R/A-1	R/A-2	R/A-3	R/A-4	2-NI 2-NI	R/A-6	R/A-7	R/A-8	R/A-9	R/A-10

Note: (M.M.) Muscat Municipality (M.P.E.) Ministry of Provincial Municipality and Environment

Table 7.3 (2) Landscape Analysis and Study for Flyover at Roundabouts and Junctions

Recommended	Plyover Type	Type-C or (Type-B)	Type-B or (Type-D)	Type-C or (Type-B)	Type-B or (Type-C)	Typc-A	Type-C or (Type-A)	Туре-8	Type-C
			*	•		1	•		
Flyover Type Consideration	Type-A Type-B Type-C Type-D	Mt.		* Mt.	* Mt. side	,	* Mt.	1	* Mt.
ver Type	Type-B	*	*	*	*	1	'	*	1
Flyc	Type-A	1	,	ı	,	*	*	1	1
nents	Continuity	Paims and groves	Tree groves	Omani islamic feature	Forizontal expansion of green	Green	Horizontal green elements	Canopy rees and palms	Florizontal green or focal trees
scape Compx	Locality	Even but rather seaside	Scaside	Even but rather seaside	Even at both side	Even at both side	Seaside	Even but rather seaside	Even at both side
Vicinity Landscape Components	Characteristics	Many groves on seaside Access to Sohar	Township with much groves on seaside	Wadi and flat land Expansion of groves	Township on both sea and Mt. side. Groves continuity	Savanna type dry land expansion and Industrial estate at ML side	Savanna type dry land expansion and wadi	Township on seaside	Township on seaside Savanna type dry land expansion at Mt. side
	Visibility	Clear vertical horizontal	Clear vertical	Clear	Vertical		Horizontal	Soft honzontal	Soft horizontal
R/A and JN		Distinctive monument silhouette	Light structure siluhoette	Distinctive monument silbouette	Omani nature	Florizontal expansion	Focal	Green grove	Green expansion
Monument and Landscape Components at	Dimension	11 = 12.0 m W = 28.0 m L = 28.0 m	H = 12.3 m W = 12.3 m L = 12.3 m /H=40 m	H = 12.0 m W = 20.1 m L = 20.1 m	H = 4.5 m W = 8.0 m L = 15.0 m	,	W = 5.0 m H = 2.5 m W = 10.0 m	H= 2+5m W=25.0m L=30.0m	H= 3+2 m W= 17.0 m L= 30.0 m
d Landscape	Direction	All directions	All directions	All	Rock faces to the sea	,	Horse faces to the sea	All directions	All
Monument an	Churacteristics	Monument of row of palms and water element. Oasis design concept	Blueish globe on the top of triangle tower/Now monument is proposed by Muscat Munic.	Islamic Omani designed observation dome on the air and water fountain	Rock hill with waterfalls and wild animal sculptures on the flat garden	Plane ground (Garden reserve site)	Monument of white horse on the terraced green	Grove on stone paved mound and water fountains	Group planting of Shrubs on the terraced green
Location	Super- intendency	Suweyhrah (M.M.)	Sohar (M.M.)	Sallan (M.M.)	Fataj Al Qabail (M.M.)	Majis (M.M.)	Liwa (M.P.E.)	Shinas (M.P.E.)	Aqr (M.P.E.)
RVA &	z	R/A-11	R/A-12	R/A-13	R/A-14	R/A-15	R/A-16	R/A-17	R/A-18

Note: (M.M.) Muscat Municipality (M.P.E.) Ministry of Provincial Municipality and Environment

There are other areas for flyover Type-A, which have no existing objects in the roundabout areas but spaces reserved for future landscape. In this case, flyover Type-A will become a new landmark for the vicinity area.

② Flyover Type-B

This is the next lowest priority after Type-A, for preserving a monument at the roundabout. Flyover will be shifted to the one side of the monument.

If monument will be situated under the proposed alignment of the flyover, some portion of the monument will be adjusted to relocate to other place in the roundabout area, and will be rearranged closely to the original layout features of the monument composition.

③ Flyover Type-C

This is the next highest priority after Type-D, for preserving the monument at the roundabout area. Flyover shall be shifted to one side of the monument and the other side will be open to view.

If some portion of the monument will be situated under the proposed alignment of the flyover, that portion of the monument will be relocated to other place within the roundabout area and will be kept in harmony with the original condition of the monument composition.

Underpass Type-D

Type-D is proposed as the underpass structure. This type of underpass will be considered when the monument of the roundabout is quite significant and important, and these monuments should be preserved with the highest priority.

Existing scenic conditions and aesthetics of monument and roundabout area will be recognized from all directions. Panoramic view of the site will have important value, and no other obstructing objects should appear around the original monument and surrounding view.

Scenic continuity of the monument and its vicinity are worthy of preservation as the monument itself has become a landmark on the vicinity area.

(2) Evaluation from the Road Alignment Viewpoint

The curve radius of the Batinah Highway surrounding the existing roundabouts are presented in Table 7.4. This table indicates that the curve radius varies from R = 1,000 m to R = 20,000 m. The vertical alignment of Batinah Highway is fairly flat.

From the viewpoint of alignment consideration, there may be no problems to make the grade separation.

Table 7.4 The Relation of Roundabout Site to Horizontal Radius

Roundabout Location	Curve Radius (m)	Roundabout Location	Curve Radius (m)
R/A-1	R = 5,000	R/A-10	R = 4,800
R/A-2	R = ∞	R/A-11	R = 5,000
R/A-3	R = ∞	R/A-12	R = 1,500
R/A-4	R = 20,000	R/A-13	R = 1,000
R/A-5	R = ∞	R/A-14	R = 1,250
R/A-6	R = 20,000	R/A-15	R=∞
R/A-7	R = ∞	R/A-16	R=∞
R/A-8	R = ∞	R/A-17	R = ∞
R/A-9	R = ∞	R/A-18	R = ∞

(3) The Configuration of the Roundabout

In order to accommodate smooth flow of the traffic on the Batinah Highway, the shape of the roundabouts are elliptical in shape (long radius 140 m, short radius 80 m). It will not be necessary to retain this configuration for the grade separation facility, therefore it is recommended to make the roundabout circular to improve traffic flow.

The radius of the simple circle on each roundabout is fixed by following items.

- : To plan within right of way limit
- : To accommodate the span of the grade separation facility

(4) The Pier for the Grade Separation Structure

In making plans for the pier of the grade separation structures within the roundabout, consideration should be given to the monuments, fountains, sodding, and pump rooms. The following consideration should be made in planning the piers within the roundabout:

 when dealing with roundabout aesthetics, prevent the pier from blocking view of the monuments.

7.3 Selection of Grade Separation Facility within Roundabout

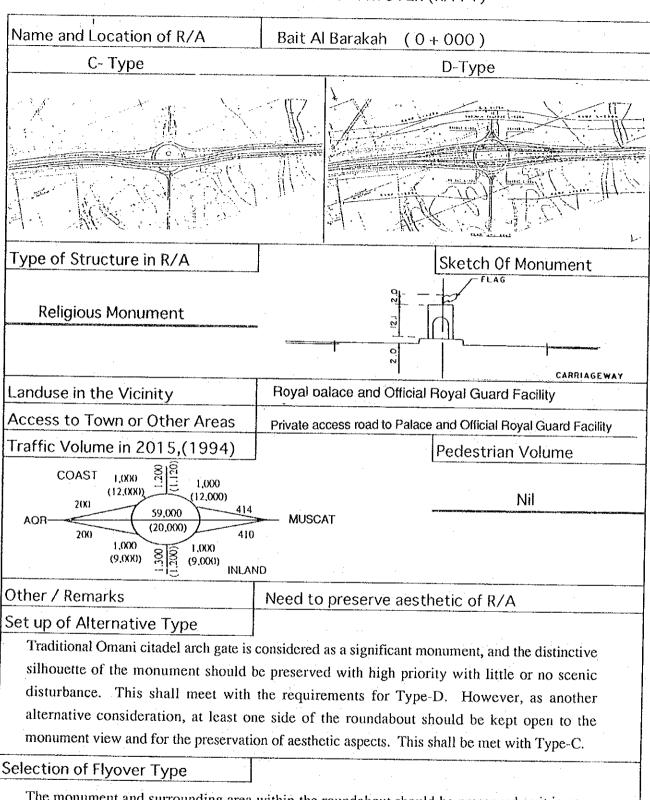
The recommended types are shown below. There are 8 Type A, 4 Type B, 3 Type C, 2 Type D, and 1 Special Type, for a total of 18.

The selection of the type of grade separation will be given as shown in Table 7.5 to Table 7.21.

Intersection	Name	Recommended Type
R/A-1	Bait Al Barakah	D
R/A-2	Naseem Garden	· A
R/A-3	Barka	A
R/A-4	Masna'ah	В
R/A-5	Al Muladdah Junction	Α
R/A-6	Wudam As Sahil	D
R/A-7	Suweiq	Α
R/A-8	Khaburah	Α
R/A-9	Al Hijari Junction	Α
R/A-10	Saham	A
R/A-11	Suwayhrah	$^{\circ}$
R/A-12	Sohar	B (D)
R/A-13	Sallan	. C
R/A-14	Falaj Al Qabail	В
R/A-15	Majis	Α
R/A-16	Liwa	C
R/A-17	Shinas	В
R/A-18	Aqr	Special

Note: () indicates second priority

Table 7.5 SELECTION OF FLYOVER (R/A-1)



The monument and surrounding area within the roundabout should be preserved as it is now, as this significant landmark is a symbol of Bait Al Baraka. Type-D shall be recommended, and underground pass box culvert type is recommended.

Table 7.5-1 SELECTION OF FLYOVER (R/A-2)

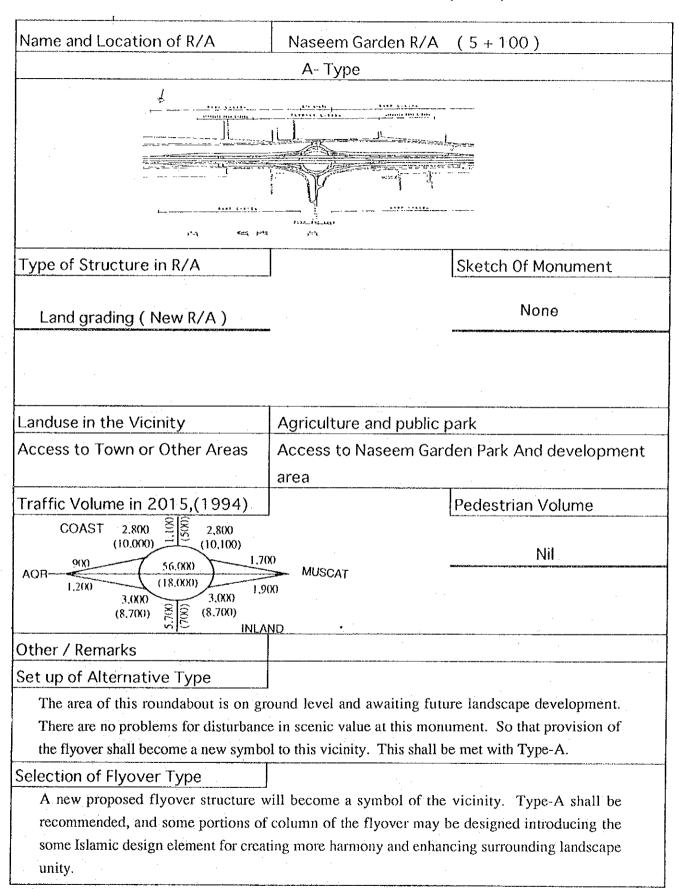


Table 7.6 SELECTION OF FLYOVER (R/A-3)

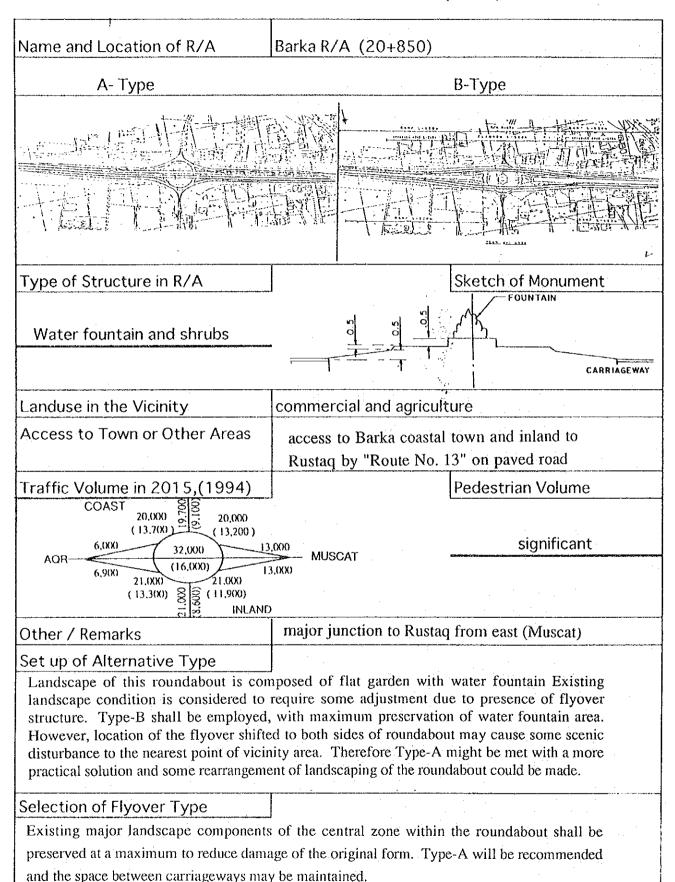


Table 7.7 SELECTION OF FLYOVER (R/A-4)

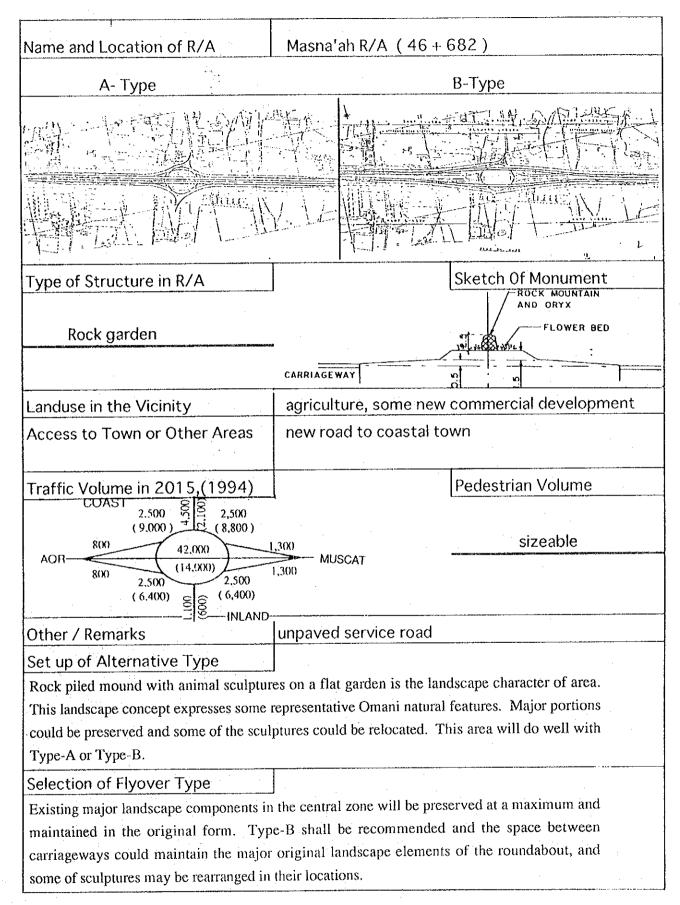
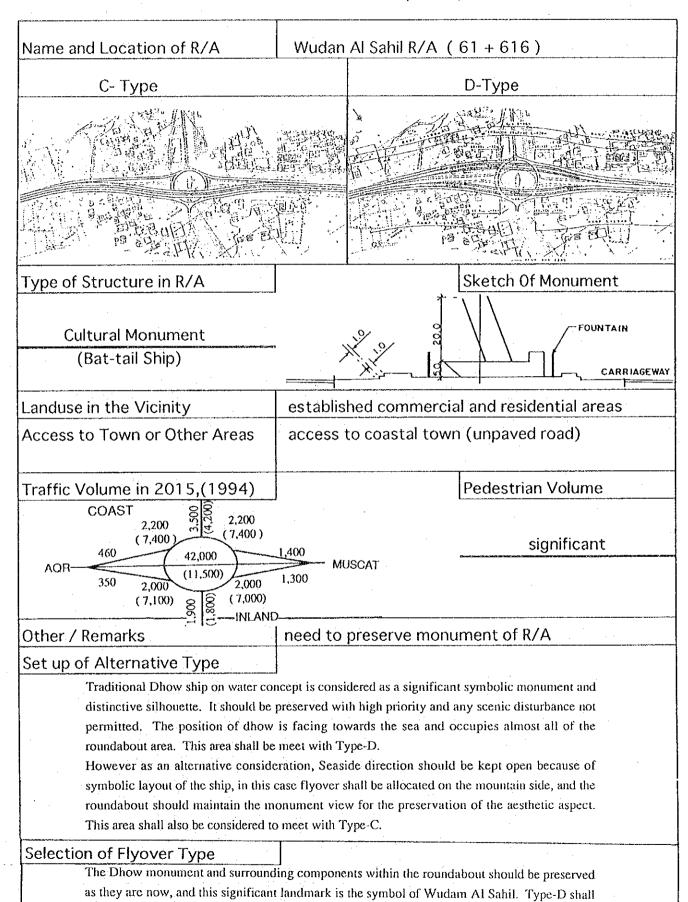


Table 7.8 SELECTION OF FLYOVER (R/A-5)

	C				
Name and Location of R/A	Al Muladdah junction (54 + 156)				
	A- Type				
77. 30 CO 20					
	The state of the s				
Type of Structure in traffic island	Sketch Of Monument				
None	None				
Landuse in the Vicinity	commercial and residential, informal bus ter-				
	minal for Rustag, presence of new development				
Access to Town or Other Areas	access to Rustaq by "Route No.11" on paved road				
Traffic Volume in 2015,(1994)	Pedestrian Volume				
COAST					
3,000 9,000	significant				
AQA (11.700)	MUSCAT				
(0)					
% [€ INLAND					
Other / Remarks	major junction to Rustaq from the west (Sohar)				
Set up of Alternative Type					
· · · · · · · · · · · · · · · · · · ·	d awaiting future landscape development, except for ide. The flyover shall become a new symbol to this e-A.				
Selection of Flyover Type					
	become the symbol of the vicinity. Type-A shall be columns of the flyover may be designed referring to impressive scenery.				

Table 7.9 SELECTION OF FLYOVER (R/A-6)



be recommended, and underground pass of box culvert type is recommended.

Table 7.10 SELECTION OF FLYOVER (R/A-7)

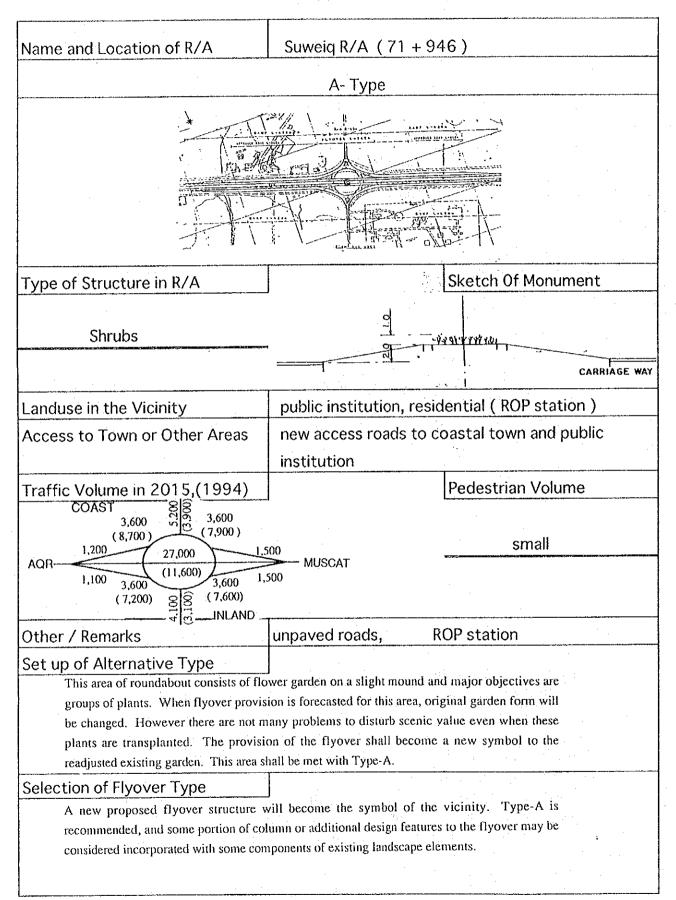


Table 7.11 SELECTION OF FLYOVER (R/A-8)

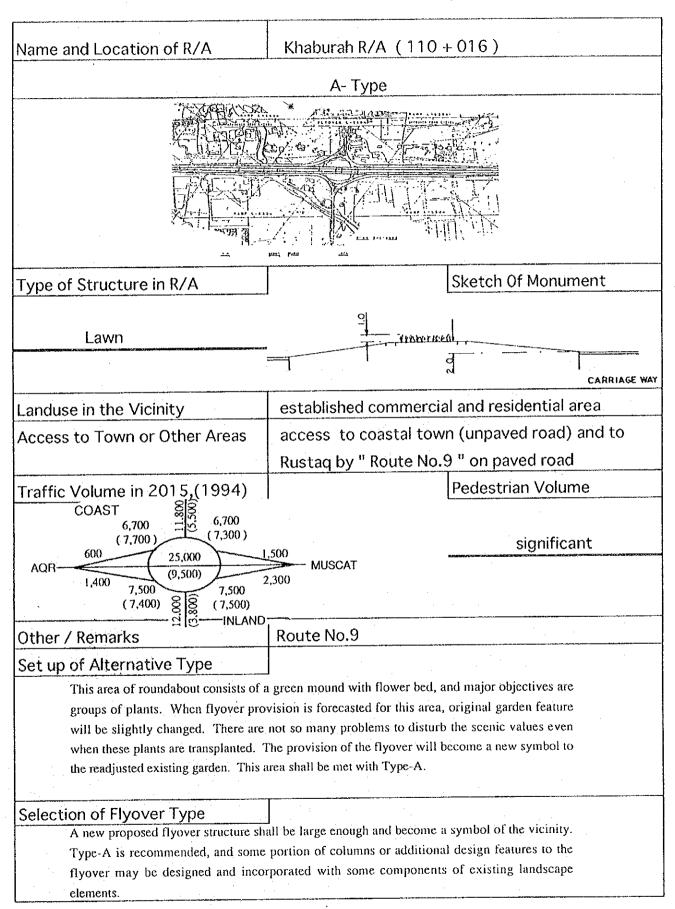


Table 7.12 SELECTION OF FLYOVER (R/A-9)

	·			
Name and Location of R/A	Al Hijari Junction	(122 + 000)		
	A- Type	· · · · · · · · · · · · · · · · · · ·		
	The state of the s	ABF X-3-2-		
Type of Structure in R/A		Sketch Of Monument		
None	-	None		
Landuse in the Vicinity	established commercial and residential areas			
Access to Town or Other Areas	access to Hijari to (unpaved road)	own and to coastal area		
Traffic Volume in 2015,(1994)		Pedestrian Volume		
COAST 700 25,000 900 (8,500) 800 800 101 101 101 101 101	MUSCAT	sizeable		
Other / Remarks				
Set up of Alternative Type				
The area of this roundabout is on gro There are no problems to disturb see shall become a new symbol to this vice	nic values at this moment.	The provision of the flyover		
Selection of Flyover Type				
A new proposed flyover structure shared and some referring to Islamic designed element	e portion of columns of the	ne flyover may be designed		

Table 7.13 SELECTION OF FLYOVER (R/A-10)

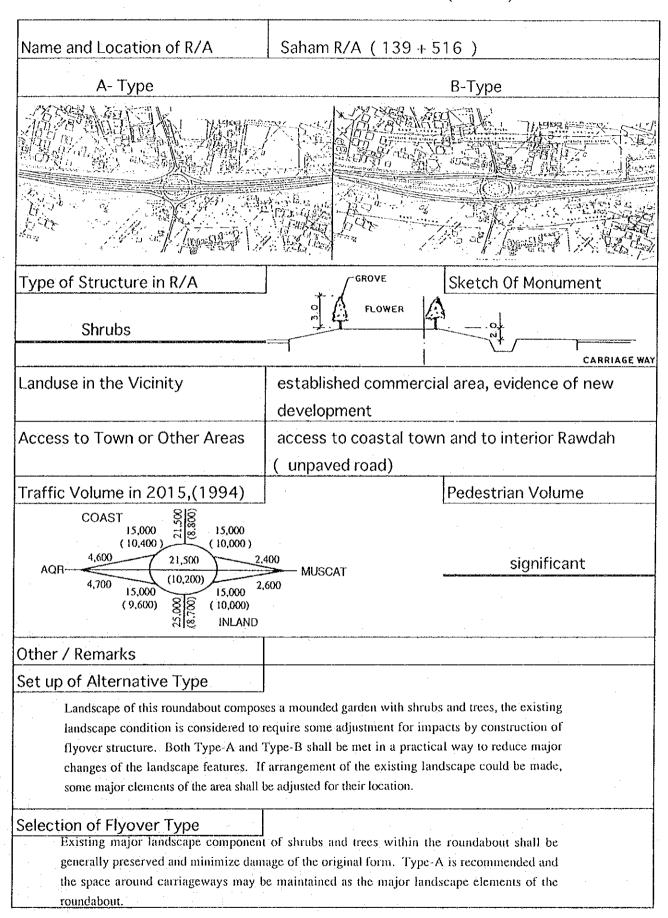


Table 7.14 SELECTION OF FLYOVER (R/A-11)

	1			
Name and Location of R/A	Suweyh	rah R/A (161	+ 566)	
В- Туре	· · · · · · · · · · · · · · · · · · ·		С-Туре	
上		* * * * * * * * * * * * * * * * * * * *		
Type of Structure in R/A			Sketch Of Monument	
Cultural monument		· // // // // // // // // // // // // //		
(date palms & fountain)		1/20	CARRIAGE	WAY
Landuse in the Vicinity	residen	ure		
Access to Town or Other Areas	access	to Sohar (pave	d & unpaved)	
Traffic Volume in 2015,(1994)			Pedestrian Volume	
COAST 8 8 6,000 (6,900) 1,800 25,200 AQR 25,200	2,900 MII	SCAT	small	
1,600 5,900 5,900 (6,600) 8 8 (6,700)	2,700			3
Other / Remarks	preserve	e monument		
Set up of Alternative Type Landscape of this roundabout is conclement, called oasis concept, and the impacts by construction of flyover struction of the area.	e existing la	ndscape will require	some adjustment for	
If maximum preservation of the existing shall be opened to the seaside view a flyover would be better located on the	nd vicinity.	Type-C could meet		
Selection of Flyover Type				
Existing major landscape component preserved to the maximum to reduce dand the space towards the seaside should	lamage of the	e original form. Typ	e-C is recommended	

Table 7.15 SELECTION OF FLYOVER (R/A-12)

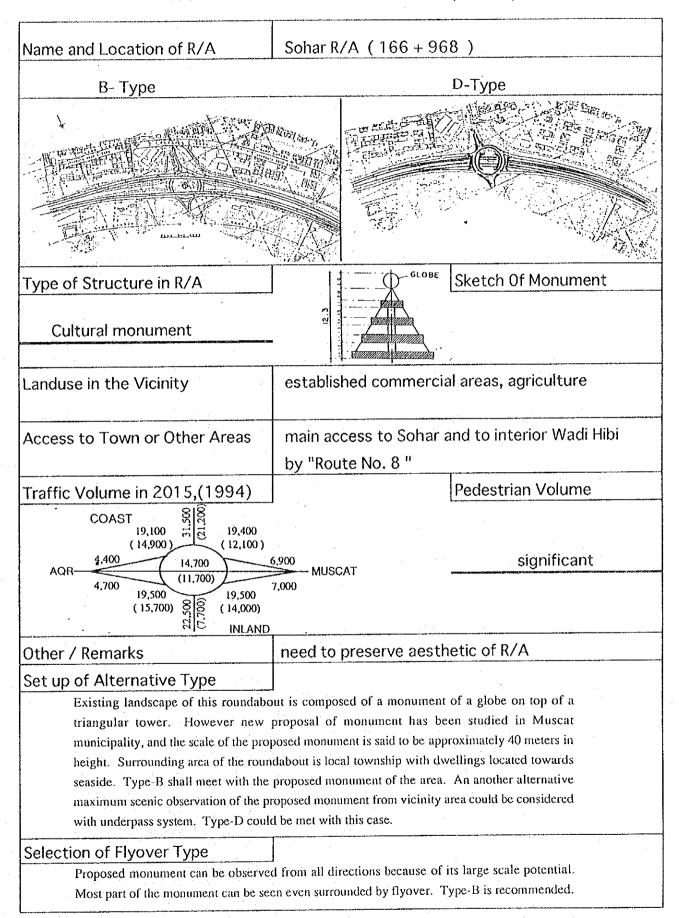
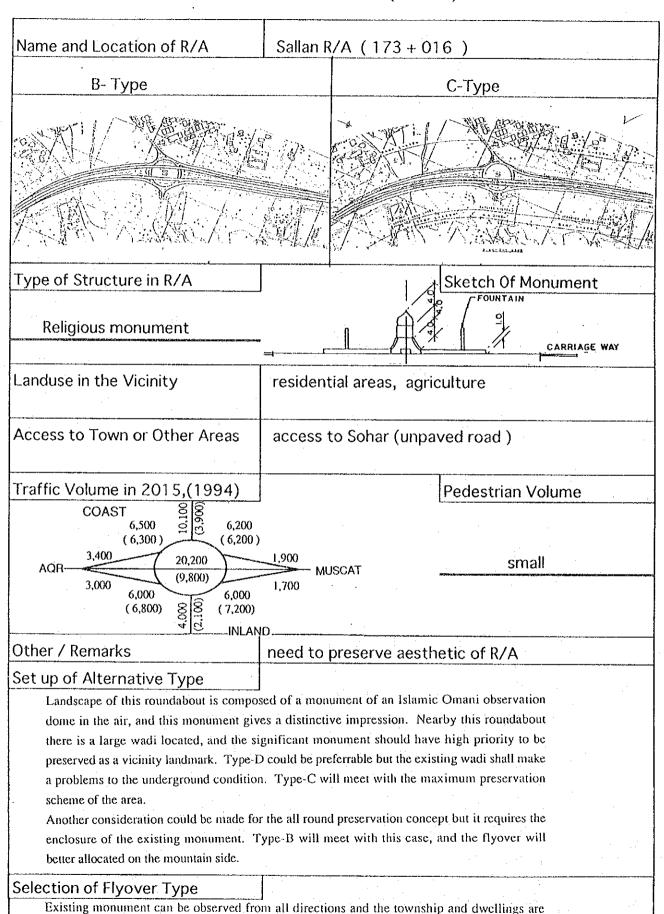


Table 7.16 SELECTION OF FLYOVER (R/A-13)



mostly situated on the seaside. The monument could be observed mostly from the seaside so type-C is recommended. The flyover should be situated on the mountain side in this case.

Table 7.17 SELECTION OF FLYOVER (R/A-14)

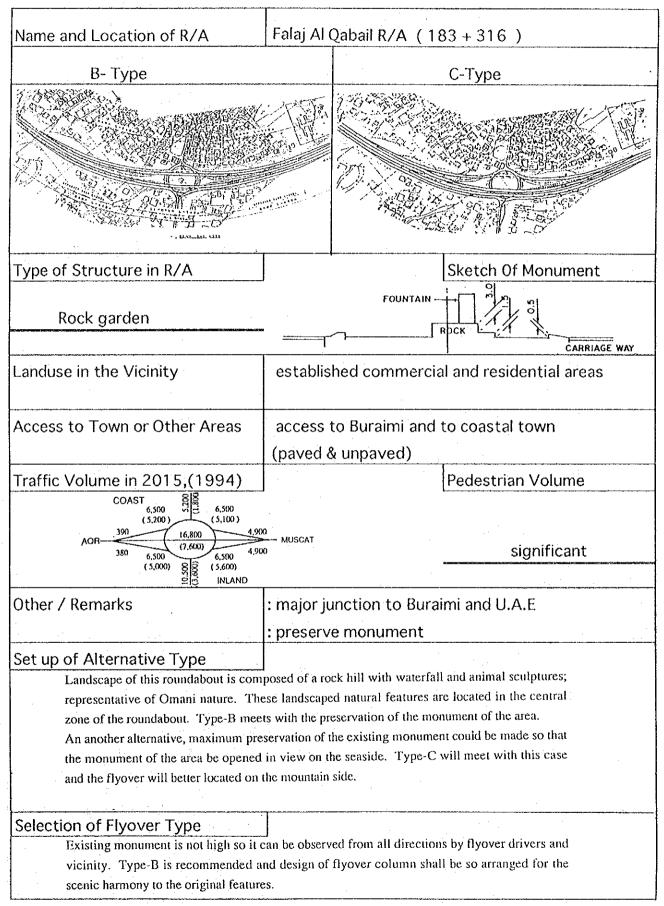


Table 7.18 SELECTION OF FLYOVER (R/A-15)

Name and Location of R/A	Majis R/A (184 + 01	8)
Name and Location of R/A	Majis N/A (104 + OTI	O)
	A- Type	
		The state of the s
	24.1-111	N
Type of Structure in R/A		Sketch Of Monument
Land grading (New R/A)	•	Non
Landuse in the Vicinity	agriculture and new inc	dustrial estate
Access to Town or Other Areas	access to new industri (unpaved)	al estate
Traffic Volume in 2015,(1994)		Pedestrian Volume
COAST (1,000) (3,000) (3,000) (3,000) (3,000) (3,000) (3,000) (3,000) (3,000) (3,000) (3,000) (3,000) (3,000) (3,000) (3,000)	CAT	Nil
Other / Remarks	access to industrial est	ate
Set up of Alternative Type This area of roundabout is on groun There are no problems to disturb the s will become a new symbol of this area	scenic value at this moment. The	provision of the flyover
	T	·
Selection of Flyover Type A new proposed flyover structure recommended, and some portion of commended for creating more amenity	olumns and some impressive feat	ures of the flyover may

Table 7.19 SELECTION OF FLYOVER (R/A-16)

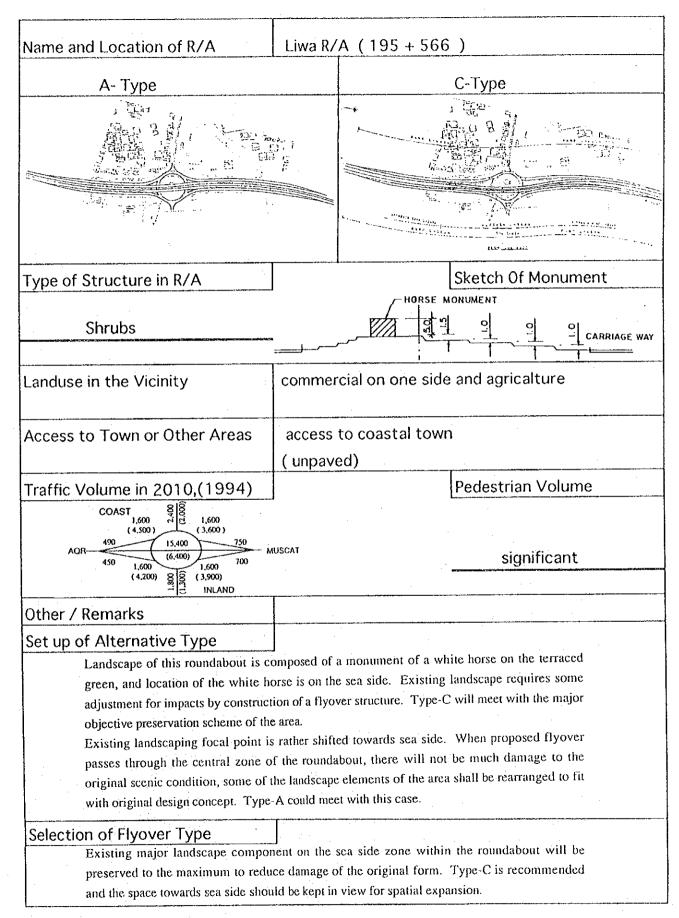


Table 7.20 SELECTION OF FLYOVER (R/A-17)

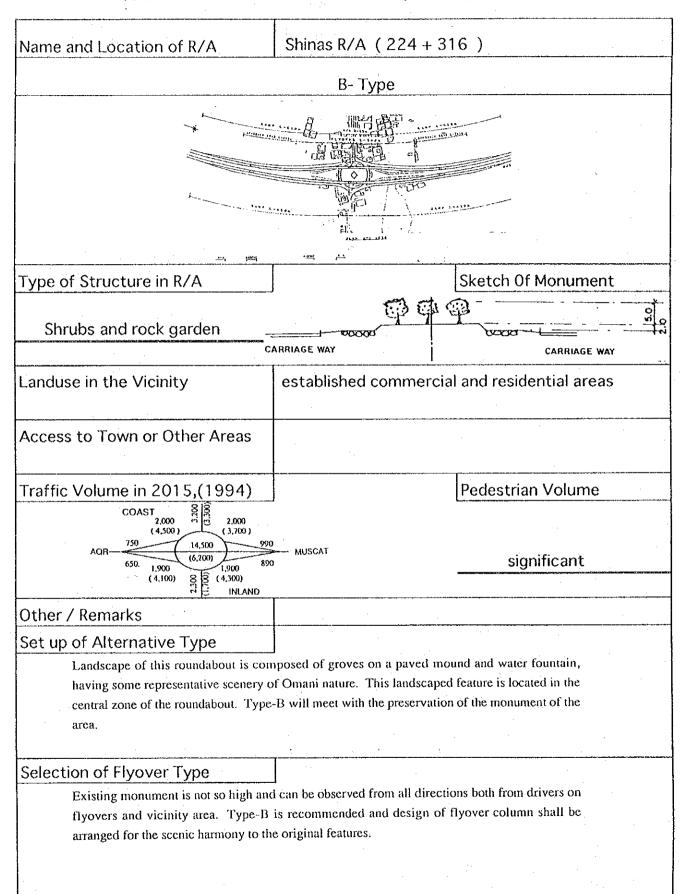
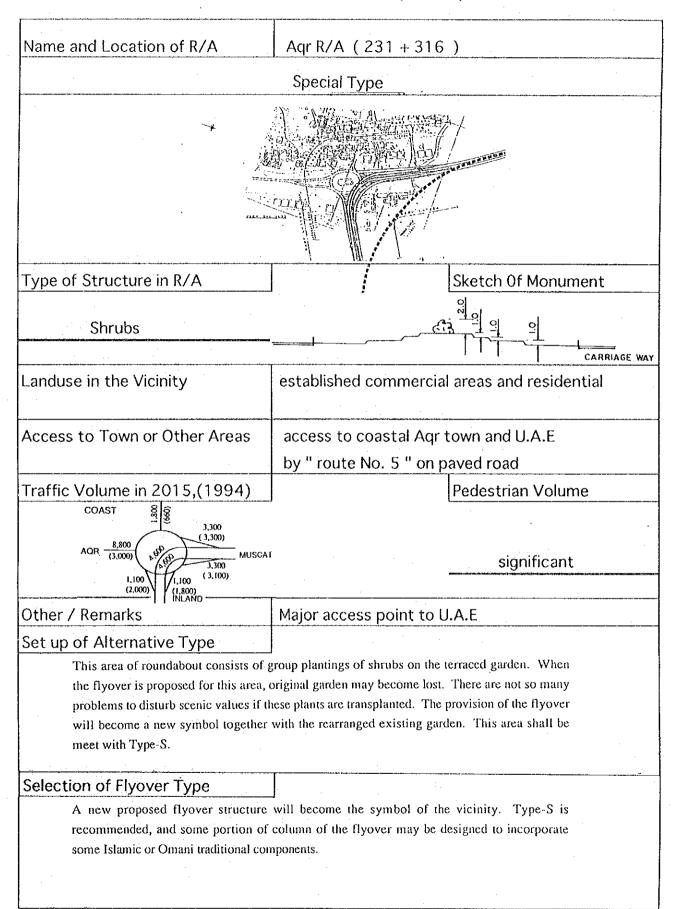


Table 7.21 SELECTION OF FLYOVER (R/A-18)



7.4 Selection of Construction Priority for the Grade Separation of Roundabout

This study selects the roundabout of high priority for the grade separation facility that should be incorporated with the 5-Year Development Programme for the Oman Road Program to begin in 1996.

The order of priority for the grade separation of roundabout is established according to the following 5 selecting criteria with weight.

The study team recommends the order of priority for grade separation of roundabouts based on discussion with the Government of the Sultanate of Oman.

7.4.1 Selection Criteria of Priority Construction

The selecting criteria of priority for the construction of the grade separation of roundabouts will have the following five items. The following 5 items are based on the function of transport of Batinah Highway, the function of linking cities, and the function of contribution to local community integration and industrial development.

[Items of evaluation for Selecting Criteria of Priority]

- evaluation from the road capacity (volume/capacity) at roundabout as a weak point of road.
- 2) evaluation from traffic volume in 2010 on Batinah Highway.
- evaluation from the importance of national road network connected with the Batinah Highway.
- 4) evaluation of contribution to local community integration
- 5) evaluation of contribution to industrial development.

7.4.2 Selection of the Grade Separated Order of Priority

The Order of Priority for 18 roundabouts was calculated from the evaluation items above.

[Amount of weight alotted to each evaluation item]

Each evaluation item was established out of the total amount of 100, having relatively heavy weight to traffic function 1) and network of national road 3), as shown below.

- 1) Road capacity (volume/capacity), 50%
- 2) Traffic volume in 2010 on the Batinah Highway, 10%.
- 3) National road network, 20%.
- 4) Local community integration, 10%.
- 5) Industrial development, 10%.

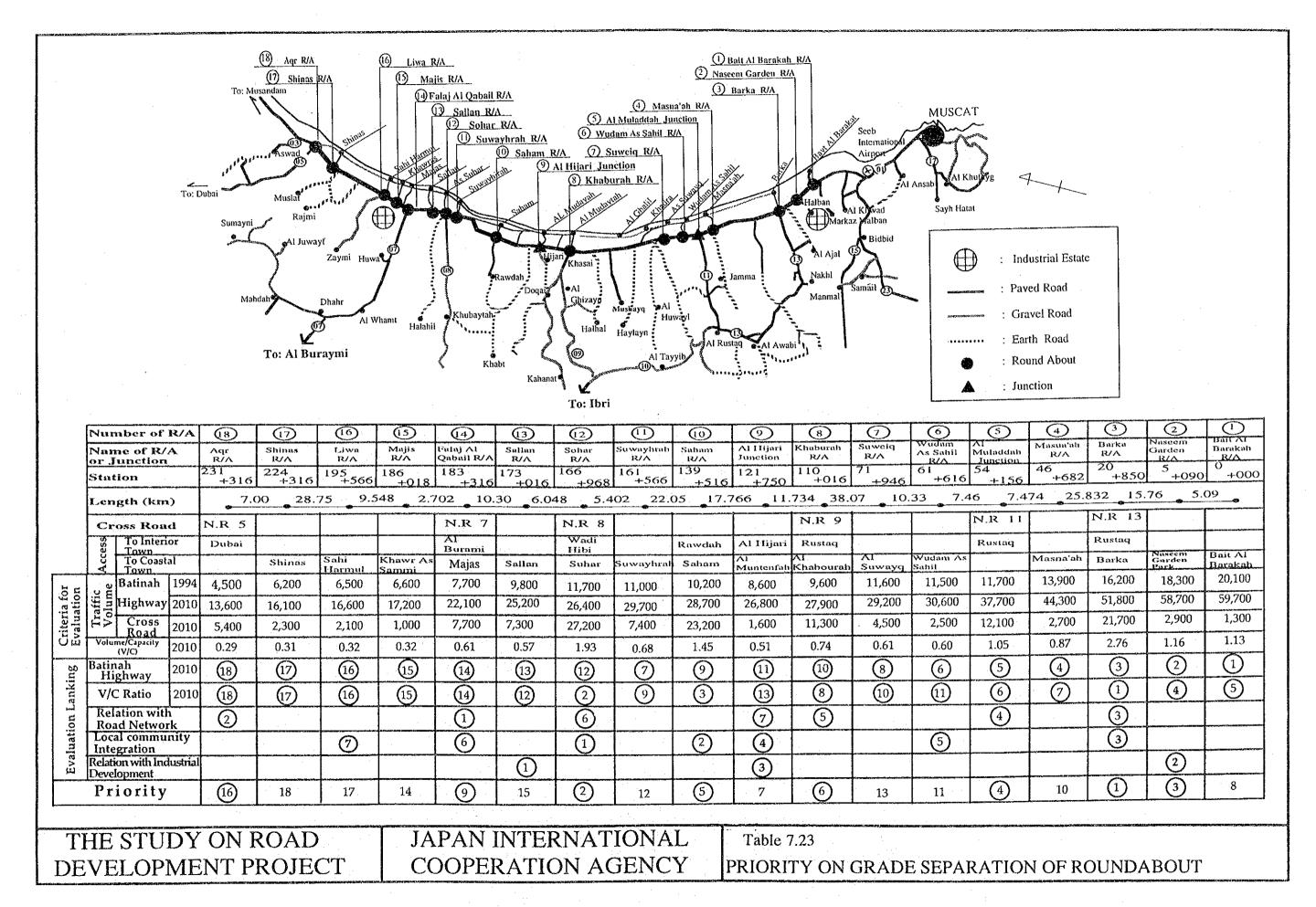
Using the above selecting criteria and amount of weight, the Order of Priority for 18 of roundabouts was calculated as shown in Table 7.22, and listed in Table 7.23.

Roundabout -18 named Aqr, forked to Dubai in UAE though National road NO. 5 and to the Musandam Peninsula of Oman, and also Roundabout -14, named Falaj Al Qabail, forked to Al Buraimi and UAE though National road No. 7 were low ranking (16th and 9th of all), but the study team recommends selecting them in this study as symbols of Oman.

Table 7.22 Calculate for Order of Priority

				Evaluation	Item		Total	
		V/C ratio	Batinah Volume	Highway Network	Local Community	Develop- ment	weighted score	Rank
	Weight	50	10	20	10	10	100	
1.	Bait Al Barakah R/A	70 35.0	90 9.0	0.0	0.0	0.0	44.0	8
2.	Naseem Garden R/A	75 37.5	85 8.5	0.0	0.0	85 8.5	54.5	3
3.	Barka R/A	90 45,0	80 8.0	80 16.0	80 8.0	0.0	77.0	1
4.	Masna'ah R/A	60 30.0	75 7,5	0.0	0.0	0.0	37.5	10
5.	Al Muladdah Junction	65 32.5	70 7.0	75 15.0	0.0	0.0	54.5	4
6.	Wudam As Sahil R/A	40 20.0	65 6.5	0.0	70 7.0	0.0	33.5	11
7.	Suweiq R/A	45 22.5	55 5.5	0.0	0.0	0.0	28.0	13
8.	Khaburah R/A	55 27.5	50 5.0	70 14.0	0.0	0.0	46.5	6
9.	Al Hijari Junction	30 15.0	40 4.0	60 12.0	75 7.5	80 8.0	46.5	7
10.	Saham R/A	80 40.0	45 4.5	0.0	85 8.5	0.0	53.0	5
11.	Suwayhrah R/A	50 25.0	60 6.0	0.0	0.0	0.0	31.0	12
12.	Sohar R/A	85 42.5	35 3.5	65 13.0	90 9.0	0.0	68.0	2
13.	Sallan R/A	35 17.5	30 3.0	0.0	0.0	0.0	20.5	15
14.	Falaj Al Qabail R/A	25 12.5	25 2.5	90 18.0	65 6.5	0.0	39.5	9
15.	Majis R/A	20 10.0	20 2.0	0.0	0.0	90 9.0	21.0	14
16.	Liwa R/A	15 7.5	15 1.5	0.0	60 6.0	0.0	15.0	17
17.	Shinas R/A	10 5.0	10 1.0	0.0	0.0	0.0	6.0	18
18.	Aqr R/A	5 2.5	5 0.5	85 17.0	0.0	0.0	20.0	16

Notes: V/C rate refer to Chapter 5 "Traffic Capacity of Road"
: upper row shows score of each location
: lower row shows the weighted score



7.4.3 Consideration from Traffic Safety Viewpoints

The setting of the construction priority of flyover is examined from the viewpoint of traffic safety. To examine the construction priority of flyovers on roundabouts throughout the Batinah Highway, it carried out a method to examine the existing condition of traffic accidents on the roundabouts and junctions on the study highway.

The data - total 357 accident cases at 75 roundabout and junction locations along the Batinah Highway in 1993 - was collected by the DGR are the data source of examination.

Table 7.24 shows the location names and number of accidents, injuries and fatalities in 357 cases. Fig. 7.7 shows the main locations where more than five (5) cases of accidents have occurred.

Based on these data, it is found out that the accident-prone locations which had more than five (5) accidents are determined as the high priority flyovers on the Highway.

Table 7.25 shows the 20 locations which are ranked highest out of 75 accident locations. These 20 locations, then can be recommended as the high-ranking priority locations of flyover construction on the Batinah Highway from the point of traffic safety.

The Table 7.25 also shows that more than 40 percent (152 cases) of total number of accidents (357) are shared by only ten (10) locations out of total 75.

Especially, three (3) main locations; Sohar, Naseem Garden and Saham roundabouts; are identified as the "Black Spots" on the highway.

These three (3) roundabouts show over 20 cases of accidents, which is much higher than other locations on the highway.

Indeed, combined accident numbers of these three places shared more than 20 percent of total accidents on the highway.

From the view point of accident injury, the three (3) locations Naseem Garden Junction (26 injuries), Hai Aasim Junction (15 injuries) and Al Hijari Junction (14) are ranked as high-injury locations.

In order to set priority for a flyover, it can be recommended to emphasize reduction of number of traffic accidents rather than number of injuries.

The fatality rate is calculated as very high at 57.9%, based on the data. The fatality rate on expressways in Japan was calculated as 25.0% in 1993.

As conclusion of above considerations, it can be said that 8 selected locations out of 18 study locations are almost all reasonable settings for construction priority from the viewpoint of traffic safety.

Table 7.24 Accident Number on Batinah Highway (1993) (1)

	Place	No. of Accidents	No. of Injuries	No. of Fatalities	Center
	Misfit Junction	2	8	1	Rustaq
	Wassail Junction	1	2		Rustaq
0	Aqr - R/A	1			Wajajah
0	Naseem Garden Junction	25	26		Barka
	Talaj A'Sharah Junction	2	3		Rustaq
	Abu Mubawat Junction	2	1	2	Musannah
	Marsi Meesi R/A	15	2		Sohar
0	Barka R/A	12	2		Barka
	A'Sawadi Junction	4	8	1	Musannah
	Al Romais Junction	5	4	1	Barka
	Al Hafri Junction	5	2		Rustaq
0	Sallan R/A	2			Sohar
0	Al Mulladah Junction	13	9		Musannah
	Al Bawarih Junction	2	4		Suwaiq
0	Saham R/A	23	5	1	Saham
0	Suwayrah R/A	11	4	1	Sohar
	Outi Junction	6	2		Sohar
	Al Sharmad R/A	5			Musannah
	Al Waqeebah R/A	. 8	1		Sohar
0	Sohar R/A	29	4		Sohar
0	Al Hijari Junction	6	14		Saham
	Al Haram Junction	5	10		Barka
	Hai Aasim Junction	11	15		Barka
	Wadi Haiby Junction	2			Sohar
	Hazam Junction	3	3		Rustaq
	Majz Al Safry Junction	4			Saham
	Sur Al Shiyadi Junction	6	9		Saham
	Abu Abali Junction	5	16	1	Musannah
	Farfara Junction	4			Khamat
	Saham Hospital Junction	1	1		Saham
····	Tow Junction	4			Barka
0	Liwa R/A	5			Sohar
<u> </u>	Al Radat Junction	3	3		Saham
0	Shinas R/A	2	2	· · · · · · · · · · · · · · · · · · ·	Khamat
0	Al Khaburah R/A	7	1		Khaburah
	Wadi Bani Hoof (AWF) Junction	1	1	2	Khaburah
	Diyan Al Jahawer Junction	2	3		Suwaiq
	Al Oohy Junction	1			Sohar
	Al Multaga Junction	4	5		Sohar

Table 7.24 Accident Number on Batinah Highway (1993) (2)

	Place	No. of Accidents	No. of Injuries	No. of Fatalities	Center
	Al Ooqin Junction	4	111111111111111111111111111111111111111	- dealers.	Suwaiq
	Marjan Junction	1			Wajajah
	B.P. Petrol Station Junction	2	1		Khaburah
0	Suweiq R/A	13	6		Suwaiq
	Suroor Junction	1	1		Barka
	Al Feleij Junction	3			Barka
	Habra Junction	3	2	1	Barka
0	Falaj Al Qabail Junction	7	1		Sohar
	Al Hamiliya Junction	1			Khamat
ļ	A Numan Junction	9	8	1	Barka
	Dabal Abdul Salam Junction	4	4		Saham
	Gheel Al Shubool Junction	3	1		Sohar
	Khamood Al Hamam Junction	5	8		Saham
	Al Kasfa Junction	3			Barka
	Al Sinaiya Junction	6	1		Sohar
	Al Aqda Junction	3	1		Barla
	Wadi Al Hiyool Junction	2	1		Majan
	Hambad Junction	1			Sohar
	Al Alat Junction	1			Rustaq
	Sahil Oman Co Junction	1	2	1	Sohar
	Khadhra Junction	7	6	2	Suwaiq
	Wadi Sahten Junction	1	8	•	Rustag
	Majilif Junction	2	7		Saham
	Ghaleel Junction	4	3		Suwaiq
	Nakhal Junction	2	3		Barka
	New Souq Junction	2			Rustaq
	Al Hoogeen Junction	1			Suwaiq
	Qaseeba Al Busaid Junction	3	5		Khaburah
	Al Fareefah Junction	1			Khamat
	Al Oohy Junction	1	1		Sohar
	Sohar Garden Junction	1	3		Sohar
	Marba'a R/A	2	6	· · · · · · · · · · · · · · · · · · ·	Rustaq
	Al Moutamar Junction	1	4		Suwaiq
<u></u>	Qashiat Al Zou'Ab Junction	2	3		Khaburah
 -	Asrar Ban Amer Junction	2	2		Khatmat
	Bahja Al Anthar Junction	3	1	1	Sohar
	Total	357	260	16	

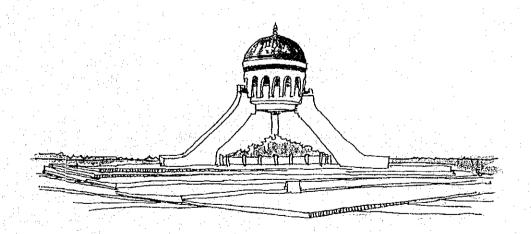
Remark: O = Grade Separation of Roundabout and Junction Recommended by JICA Study Team

Table 7.25 Worst Ranking Places on Batinah Highway (1993)

S. No.	Place		No. of Accidents	No. of Injuries	No. of Fatalities
1.	Sohar R/A	77	29	4	
2.	Naseem Garden Junction	(21.6 %)	25	26	
3.	Saham R/A		23	1	1
4.	Marsi Meesi R/A		15	2	
5.	Al Mulladah Junction	152 (42.6 %)	13	9	
5.	Suweiq R/A		13	6	
7.	Barka R/A		12	2	
7.	Suwayrah R/A		11	4	1
7.	Hai Aasim Junction		11	15	
10.	A Numan Junction		9	. 8	1
11.	Al Waqeebah R/A		8	1	
12.	Al Khaburah R/A		7	1	
12.	Falaj Al Qabail Junction		7	1	
12.	Khadhra Junction		7	6	2
15.	Al Hijari Junction		6	14	
15.	Sur Al Shiyadi Junction		6	9	
15.	Al Sinaiya Junction		6	1	
18.	Al Romais Junction		5	4	1
18.	Al Hafri Junction		5	2	
18.	Al Sharmad R/A		5		
Total	20 Places		214	116	6

7 - 50

CHAPTER 8 SELECTION OF PEDESTRIAN UNDERPASS



CHAPTER 8

SELECTION OF PEDESTRIAN UNDERPASS

There has been a rapid increase of road traffic on the Batinah Highway in recent years with vehicles running in excess of 100 km/h. There are no grade separations on the highway and no facilities provided for pedestrians to cross the highway within city limits. There are many traffic accidents involving people attempting to cross the highway. In order to reduce the number of accidents involving pedestrians, there is a project under implementation to construct on underpass 67 km from Sohar fowards Muscat at A1 Bidaya.

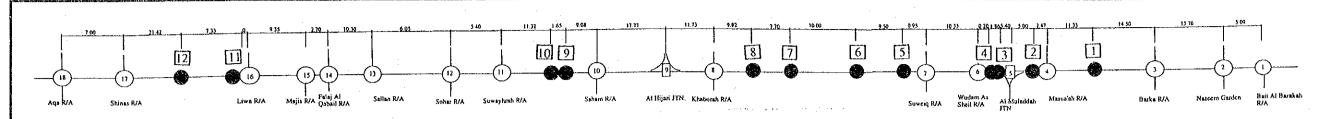
This chapter describes the locations of the pedestrian underpasses similar to the pedestrian underpass described above to be constructed on the Batinah Highway in order to make it safe for pedestrian crossing.

8.1 Candidates for Sites for Pedestrian Underpass

The candidates for the sites for the pedestrian underpasses were selected by considering the following conditions:

- (1) Locations where there are facilities on the other side of the highway from settlements which are imminently related with the daily activities, such as farms and stores.
- (2) Locations where crossing of the highway is required to attend schools and social gatherings.
- (3) Locations where the distance of the settlements and schools from the highways are are within 200 m from the highway. The maximum walking distance for pedestrians will be 500 m (allowing a width of 100 m for the highway.)

Using the above criteria and by selecting the settlements and schools along the Batinah Highway, 40 sites were determined to require crossing underpasses as indicated in Table 8.1.



	tes for Selection			Criteria-1	Criteria-2	Criteria-3	Recommendation	Legend
Location Name of Settlement	Area	a (km2)	School	Settlement area is				Recomme Pedestriai
				more than 0.1km2	There is school at	Side of high priority		Pedestria
No. Sta. N.o	Mountain Side	Sea Side		both side of highway	one side of highway	R/A for flyover		Underpa
1 Sta. 20+400 As Somhan	0.10	0.26	w# PR 444			R/A-3		
2 Sta. 35+400 Al Billah	0.05	0.06	Sea Side		- 0		2	Recomm
3 Sta. 49+200 Al Tarif	0.19	0.14	.:				2	Number
4 Sta. 55+150 Al Muladdah	0.38							ramber
5 Sta. 59+600 Al Qart	0.12	0.12					3 4	
6 Sta. 61+416 Al Tharmad	0.14	0.26	## Amp Am				4	
7 Sta. 63+150 Al Manfash		0.20	Sea Side					
8 Sta. 72+900 Bataha Hilal	0.04	0.08	Sea Side		0		5	
9 Sta. 79+200 Sur Al Hilal		0.10						
10 Sta. 81+000 Al Urig	0.08	0.16						
11 Sta. 82+350 Al Khadra	0.06	0.03	Sea Side		0		6	
12 Sta. 91+700 Dhyan-1	0.20	0.04						
13 Sta. 92+400 Dhyan-2	0.04	0.08	Sea Side				7	
14 Sta. 93+000 Dhyan-3		0.06			:			
15 Sta. 100+100 Al Bidayah	0.14	0.12	Sea Side		O		8	
16 Sta. 110+016 Sur Al Duwahnah	~~~·	0.52				R/A-8		
17 Sta. 115+550 Qasbyat Al Hawashnah	0.16	0.09						-
18 Sta. 139+516 Saham	0.07	0.20	Sea Side		0	R/A-10		,
19 Sta. 141+350 Al Badi		0.20						
20 Sta. 142+400 Al Ghuwaisah		0.20						
21 Sta. 143+400 Al Hadheeb	~	0.10						
22 Sta. 147+500 Hilat Al Rawashid	-+-	0.10						
23 Sta. 148+600 Hilat Al Rawashi 1	0.12	0.22				.,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	9 6	
24 Sta. 150+250 Mujaz As Sughra	0.12	0.14	****				9 10	
25 Sta. 159+000 Khor Siyabi		1.08	· www.					
26 Sta. 159+900 Otab	0.06							
27 Sta. 166+968 Al Waqaybah		0.26	Sea Side			R/A-12		
28 Sta. 170+200 Sallan		0.80				:		
29 Sta. 174+200 Al Gushbah-1		0.30						
30 Sta. 175+400 Al Gushbah-2	·	0.10						
31 Sta. 176+900 Falaj Al Ouhi		0.22						
32 Sta 183+316 Falai Al Oabail	0.20	0.24	Sea Side			R/A-14		
32 Sta. 183+316 Falaj Al Qabail 33 Sta. 195+766 Liwa	N.a	N.a		+				
34 Sta. 200+850 Liwa-1	11,4	0.08		 				
35 Sta. 201+750 Liwa-2		0.08						
36 Sta. 202+900 Liwa-3	0.10	0.13	Sea Side		0		12	
37 Sta. 221+500 Al Hazari	0.10	0.18		Y				
38 Sta. 212+700 Sur Al Abril	0.30	0.09	17-17					
39 Sta. 213+800 Sur Bani Gizmah	0.30	0.08						
40 Sta. 215+100 Tari Hajih	0.50	0.30	.					
AO ISta 215±100/Tari Haiib								

COOPERATION AGENCY

DEVELOPMENT PROJECT

SELECTION OF SITE FOR PEDESTRIAN UNDERPASS

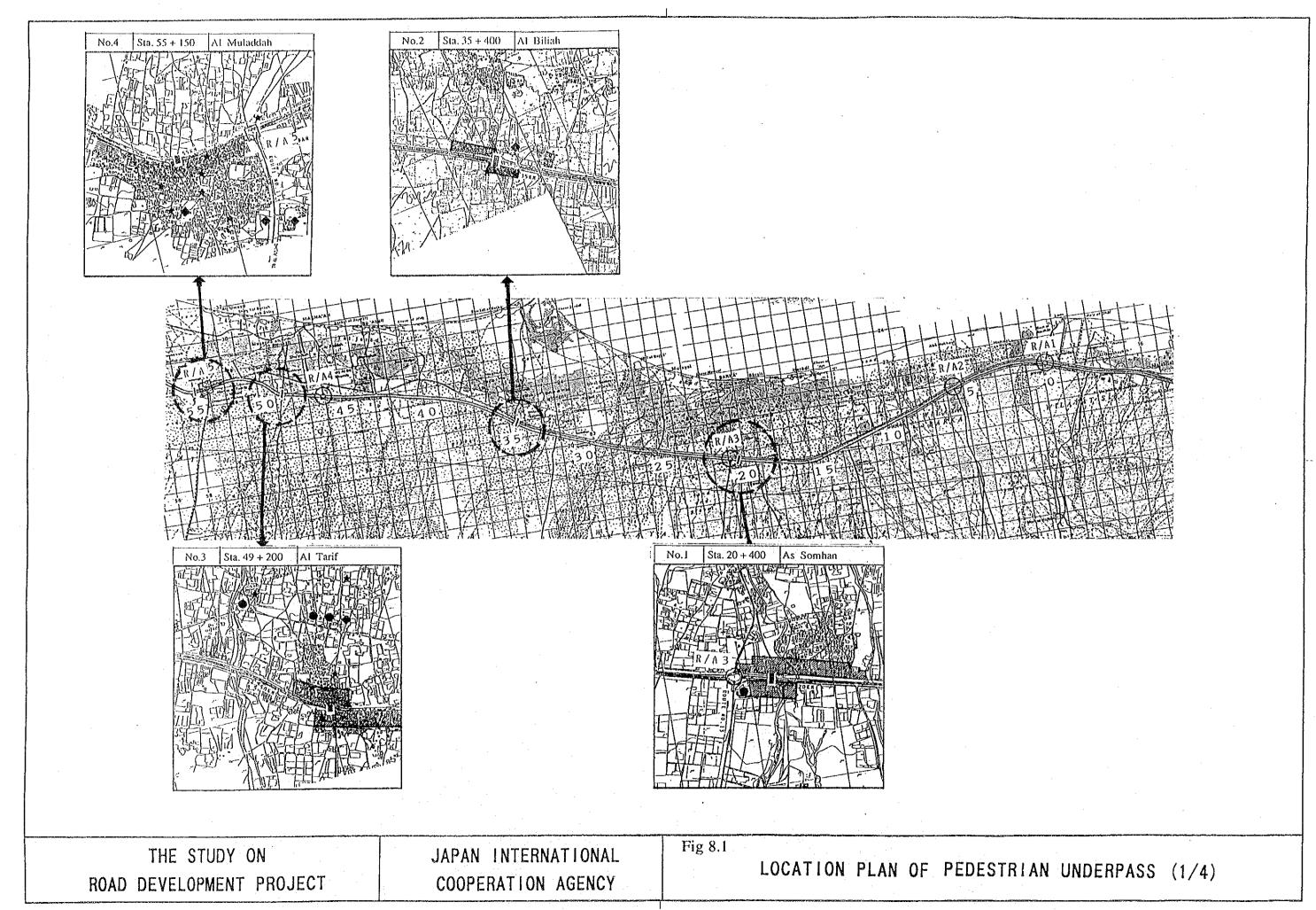
8.2 Criteria of Selection for the Pedestrian Underpass Sites

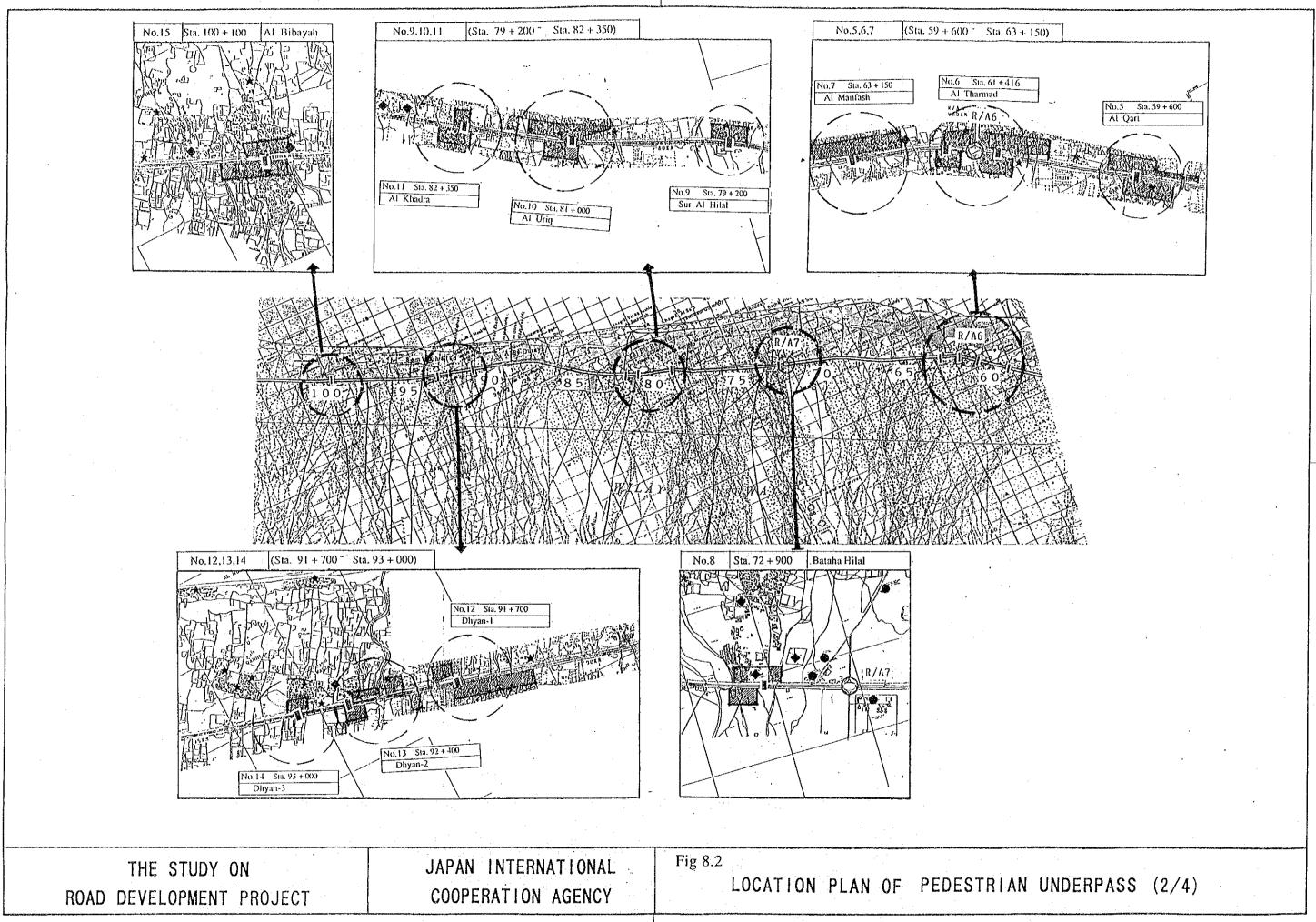
Out of the prospective 40 sites, the following three criteria were established to make the final site determination. See Fig. 8.1 ~ Fig. 8.4.

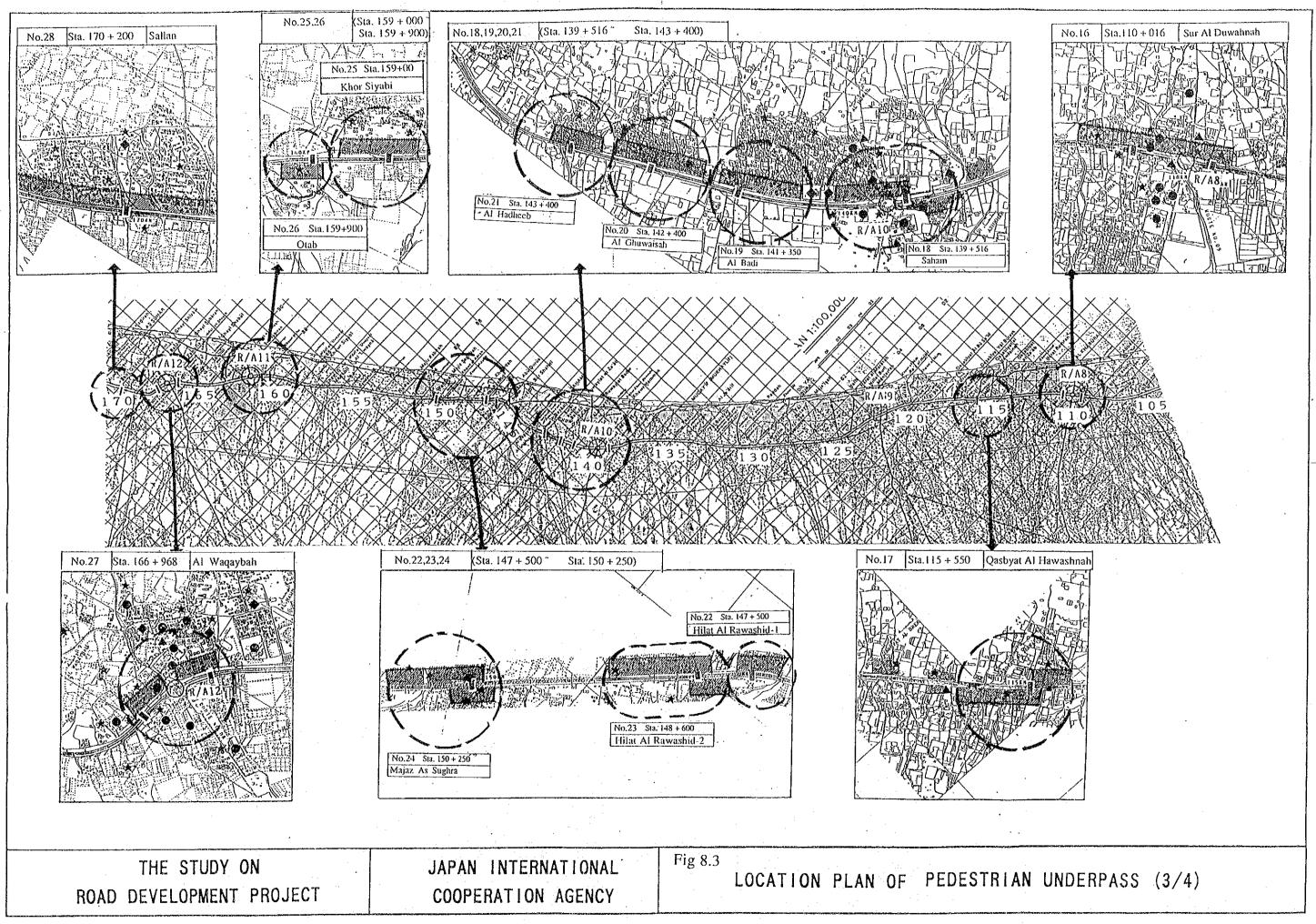
- (1) Criteria-1: Settlements where there are large populations and movements of people across the highway. Area-wise approximately the same size as the settlements where the underpass is being constructed in the wilayat of Bidayah. (Area larger than 0.1 km² both side of highways)
- (2) Criteria-2: Commuting routes to schools requiring crossing of the highway for the safety of students, with further recommendation that the school be near the crossing.
- (3) Criteria-3: No pedestrian underpass should be proposed at locations where there is a high possibility of construction of a flyover at roundabouts, as the vehicle traffic will be reduced.

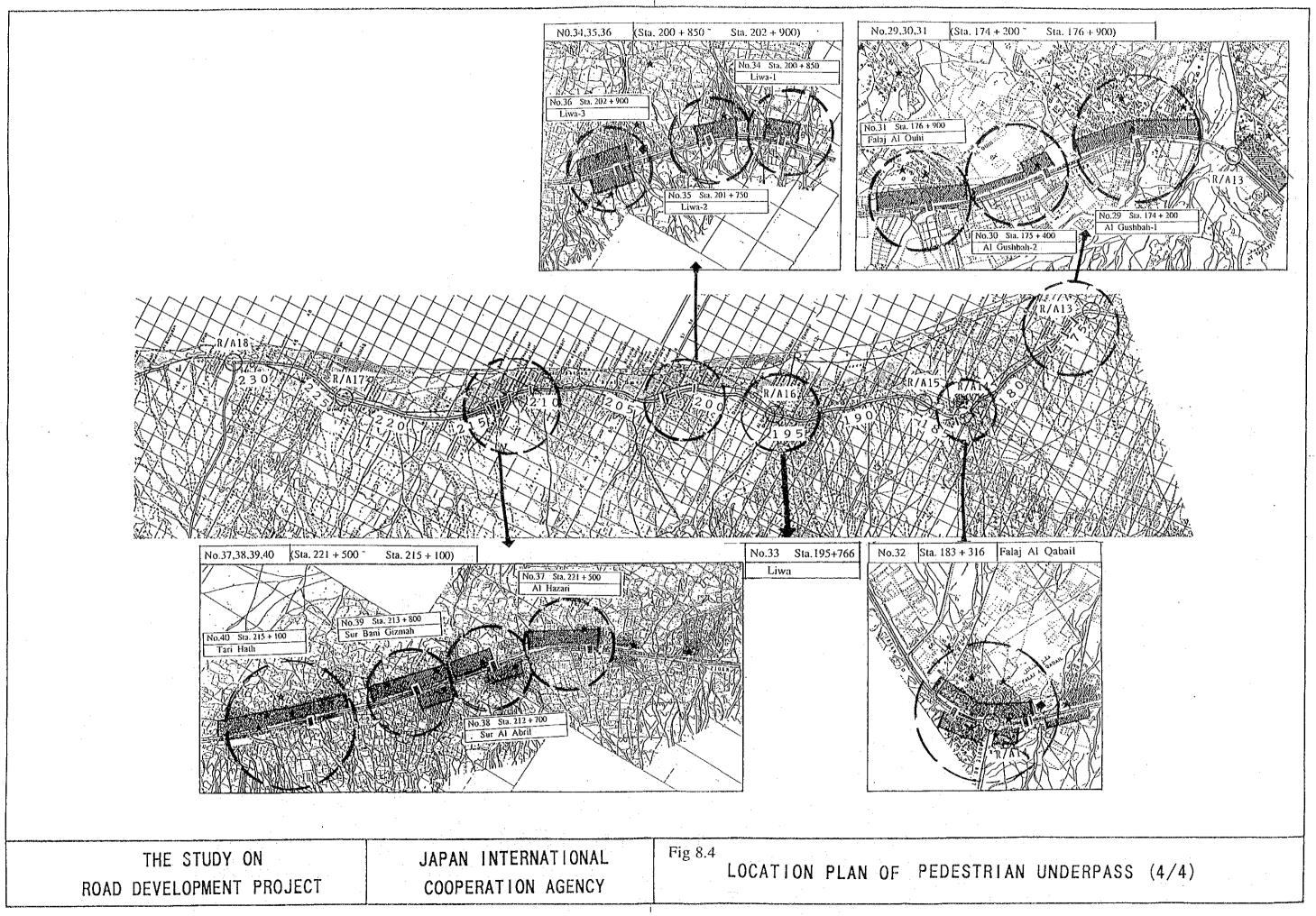
Recommendations: The sites recommended for construction of pedestrian underpasses are the sites selected for Criteria 1 and 2.

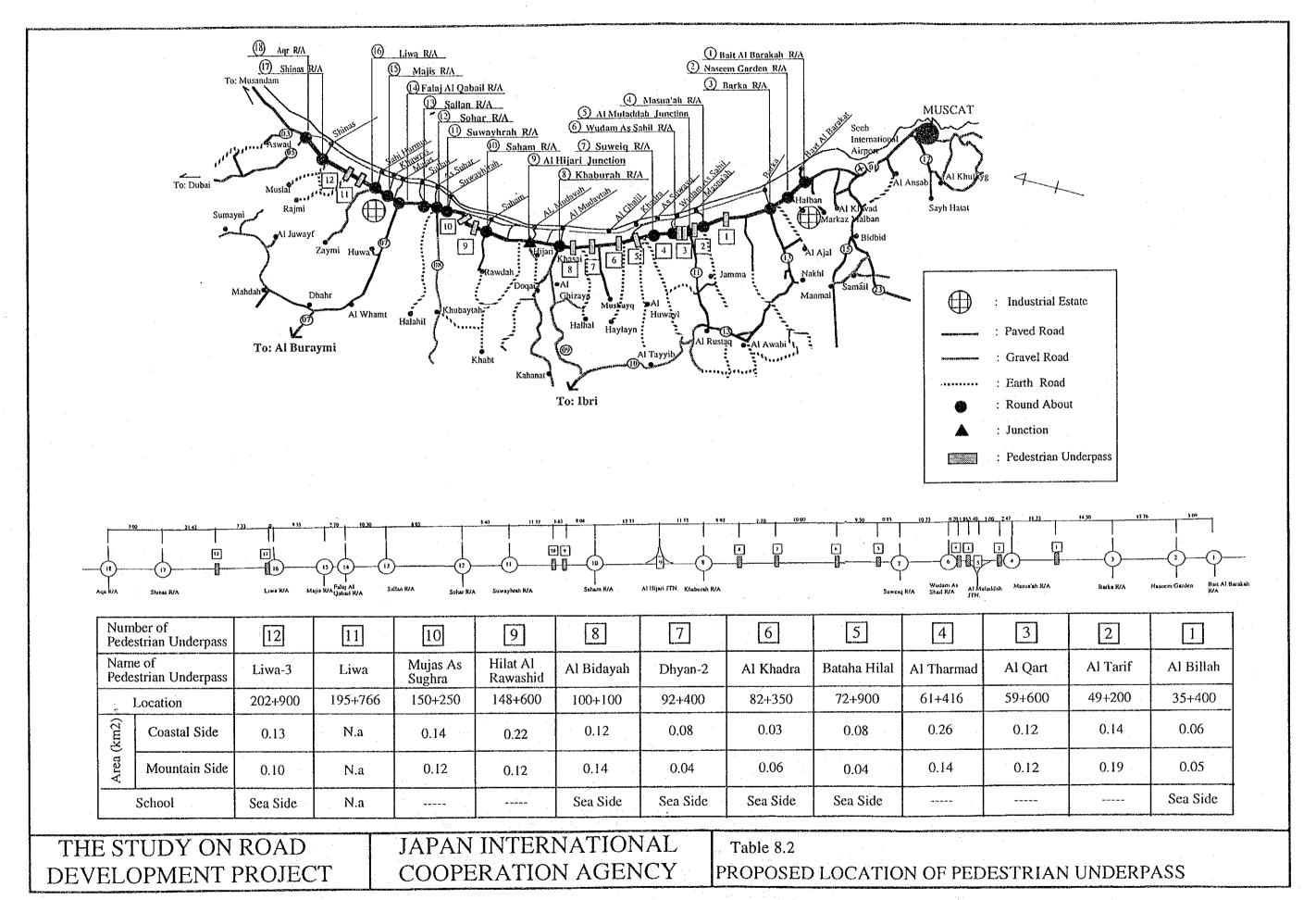
Twelve sites are recommended for construction of the pedestrian underpasses as shown in Table 8.2.











8.3 Consideration from Traffic Safety View Points

1) Observed Conditions of Crossing Pedestrian

In order to decide the construction priority of pedestrian underpasses from the viewpoint of traffic safety, it is needed and required to analyze the existing conditions of pedestrian accidents on the Batinah Highway. But unfortunately, there is no particular pedestrian accident data or statistics available on the Highway.

It is a serious problem and astonishing and that no pedestrian crossing facilities are observed along the Highway that was constructed with high design speed and operated with high speed of about 110-kph (according to the speed survey).

Therefore, any location basically can be recommended to construct the grade separated crossing facilities along the Highway, even after considering properly the characteristics at the site such as social and cultural conditions based on the activities of adjacent inhabitants.

The following inconvenient and dangerous pedestrian situations in crossing the highway were observed by the site survey of JICA study team.

- Many people, especially women, aged or handicapped people had very hard times to cross the guardrail on median.
- ii) Bicyclers, especially children, had difficulty to pass under the guardrail.
- iii) Aged and handicapped people needed long waiting time to cross, the longest time was measured more than five (5) minutes because of the low speed of their crossing and high-speed vehicular traffic.

2) Highly Recommendable Location

The following two (2) locations are urgently recommended as locations to construct pedestrian underpass from viewpoint of safety.

a) Al Bidayah

Al Bidaya is the first priority location to construct pedestrian underpass. There are densely populated villages on both sides of the highway stretching more than one (1) kilometer.

Many pedestrians are observed crossing the Highway. According to the sample survey, the highest pedestrian crossing volume was counted at more than 300 per hour at evening peak hour, as shown in Table 8.3.

Speed of vehicular traffic is observed as considerably high at this location. The speed limit is posted at 50 km/h, although an exact survey was not conducted.

b) Al Tarif

Al Tarif can be the second priority location as there are villages on both side of the Highway stretching more than 500 meter which have a high potential of crossing demands of inhabitants.

Horizontally curved alignment renders poor visibility and worsens the condition, therefore the reason for setting high priority at this location.

Pedestrian Crossing Volume on Batinah Highway Survey Station Al Bidayah Table 8.3

			Day Time (12 Hours) Crossing Volume	0	3,064/day time	* Estimated Volume									
		Crossing Volume is Morning Hour	232/Hour		Crossing Volume in Evening Hour		278/Hour			Side Side					
	Total/h	172	236	288	213	325	298	255.3		Pedestrian Crossing from Bus Stop side to Mosque Side Pedestrian Crossing from Mosque Side to Bus Stop Side					
Д	Total	28	131	148	111	175	174	137.7		Stop side		250 m Range from Bus Stop to Muscat	o Aqr		
Direction B	Part 2	61	81	79	09	87	83	75.2		from Bus		sus Stop t	us Stop t		
	Part 1	26	50	69	51	88	91	62.5		Crossing		ge from E	ge from B	1994	
A	Total	85	105	209	102	150	124	129.2		destrian		0 m Rang	250 m Range from Bus Stop to Aqr	July 16, 17 1994	
Direction /	Part 2	41	43	95	09	99	56	60.2		. , .	•	1: 25	2: 25		
A	Part 1	44	62	114	42	84	89	0.69		Direction A:		Part		Survey Date:	
Direction	Time	6:00 ~ 7:00	7:00 ~ 8:00	8:00 ~ 9:00	15:00 ~ 16:00	16:00 ~ 17:00	17:00 ~ 18:00	Ave. No./Hour	Remarks:	7		2) 1		 3) (6	

8.4 Type of Pedestrian Crossing Facility

There are two types of pedestrian crossing facilities, namely overpass and underpass. These two types are compared in Table 8.4.

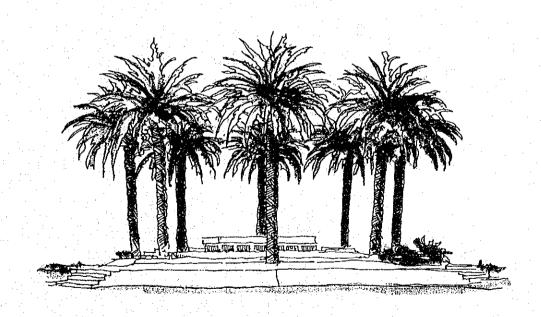
In Oman only the underpass type are used apparently due to aesthetic consideration, also being good refuge from summer heat and provide crossing for livestock. There might be a case in detailed design stage where overpass with necessary shelter will have to be designed because of unfavorable conditions for underpass.

Table 8.4 Comparison of Pedestrian Facility

Criteria	Underpass	
1. User's Convenience	The pedestrians are psychologically liable to use due to less climbing height than flyover	Opposite of underpass due to much more climbing height than underpass
2. Comfort	Good	Fair but bad in summer
3. Crime Prevention	Lighting required and other measures	None required
4. Aesthetic	Good	Required harmony with surroundings
5. Effect on Surroundings	Almost none	Protective measures on highway are required
6. Construction Cost	Low	Slightly High
7. Maintenance	Required	Little
8. Ease of Construction	Normal	Less than Underpass

From the above table, underpass type was selected judging from economical and aesthetic standards.

CHAPTER 9 PRELIMINARY STUDY OF GRADE SEPARATION FACILITIES



CHAPTER 9

PRELIMINARY STUDY OF GRADE SEPARATION FACILITIES

9.1 General

This section describes the results of preliminary study of grade separation facilities for eight (8) selected roundabouts of high priority in Chapter 7 based on the results of traffic demand forecast, topographic survey and soil investigation.

Table 9.1 gives the name, location and structure type of eight (8) roundabouts.

Table 9.1 List of Eight (8) Selected Roundabouts of High Priority

No.	Roundabout	Name	Location	Type of structure
1	R/A - 2	Naseem Garden	5 + 090	Α
2	R/A - 3	Barka	20 + 850	Α
3	R/A - 5	Al Muladdah JUN.	54 + 156	Α
4	R/A - 8	Khaburah	110 + 016	Α
5	R/A-10	Saham	139 + 516	Α
6	R/A-12	Sohar	166 + 968	В
7	R/A-14	Falaj Al Qabail	183 + 316	В
8	R/A-18	Aqr	231 + 316	Special

9.2 Investigation of the Natural Conditions

In order to obtain more detailed field information for the feasibility study, topographic survey, soil investigation and environment investigation were performed. For a detailed description of the environment impact study, refer to Chapter 12.

9.2.1 Topographic Survey

Topographic survey of the roundabouts and pedestrian underpasses were performed, selected in the study as shown in Chapters 7 and 8.

The field survey was implemented during June and July in 1994.

Name of roundabouts, pedestrian underpasses and description of field survey are shown as Table 9.2A, 9.2B and Table 9.3A, 9.3B. Its details are shown in Table 7.23 and Table 8.2.

Table 9.2A List of Roundabouts for Field Survey

Number of R/A	Station	Name of R/A	Remark
No. 2	5 + 090	Naseem Garden	
No. 3	20 + 850	Barka	Route 13
No. 5	54 + 156	Al Muladdah Junction	Route 11
No. 8	110 + 016	Khaburah	Route 9
No. 10	139 + 516	Saham	
No. 12	166 + 968	Sohar	Route 8
No. 14	183 + 316	Falaj Qabail	Route 7
No. 18	231 + 316	Aqr	Route 5

Table 9.2B List of Pedestrian Underpasses

Number of P/U	Station	Name of P/U	Remark
P/U - 1	35 + 400	Al Billah	
P/U - 2	49 + 200	Al Tarif	
P/U - 3	59 + 600	Al Qart	
P/U - 4	61 + 416	Al Tharmad	
P/U - 5	72 + 900	Bataha Hilal	
P/U - 6	82 + 350	Al Khadra	
P/U - 7	92 + 400	Dhyan-2	
P/U - 8	100 + 100	Al Bidayah	
P/U - 9	148 + 600	Hilat Al Rawashid	
P/U-10	150 + 250	Mujas As Sughra	•
P/U-11	195 + 766	Liwa	
P/U-12	202 + 900	Liwa-3	

Table 9.3A Description of Field Survey for Roundabout

Survey Type	Scale	Remark
Centerline and longitudinal	H = 1/500	
Leveling survey	V = 1/100	
Cross section leveling	V.H = 1/100	
Survey		
Plane table survey	1/200, 1/500	0.2 m Contours

Table 9.3B Description of Field Survey for Pedestrian Underpass

Survey Type	Scale	Remark
Centerline and longitudinal	H = 1/200	
Leveling survey	V = 1/200	
Cross section leveling	V.H = 1/100	
Survey Plane table survey	1/200	0.2 m Contours

9.2.2 Soil Investigation

(1) Introduction

The object of the soil investigation is to provide information and data for structure, earthwork and other designs for the feasibility study on the construction of flyovers and pedestrian underpasses along the Batinah Highway. Machine boring, standard penetration tests, samplings, soil laboratory tests, etc., were carried out.

The investigation was carried out for eight roundabouts and three pedestrian underpasses along Batinah Highway from June to July in 1994.

Machine boring was conducted for a total 11 holes (135 m) with 115 standard penetration tests, and before machine boring, survey for buried structure by test pits with depth 2.5 m to 3.0 m was implemented. The samples taken from those surveys were tested.

Summary of field works and laboratory works

1.	Machine boring	11 locations	135 m
	for Roundabouts	8 locations	105 m
	for Pedestrian underpasses	3 locations	30 m
2.	Standard penetration test		115 each
3.	Survey for buried structure by test pits		11 each
4.	Laboratory works		
÷	Physical test		37 tests

(2) Stratigraphy

1) General

The geology of Batinah coastal plain consists of sedimentary formation of tertiary and quaternary age. Tertiary sedimentary rocks are extensively distributed and form baserock in the area, overlain by quaternary sediments which consist of terrestrial sediment, fans deposit fluviatile deposit and coastal deposit. These are tabulated below.

Explanation of sedimentary rocks

Geological time		Formation	Description	
	Holocene	Alluvium	Fluviatile deposit, recent fans coastal deposit	
Quaternary	Pleistocene	Diluvium	Fluviatile deposit, old fans terrace deposit	
Tertiary	Neocene	Mudstone	Mudstone, Gravely mudstone and marlymudstone, marlystone	
	Paleocene	Limestone	Limestone, marlystone	

Tertiary formation

Tertiary sedimentary rocks consist of limestone, marlystone, mudstone, gravely mudstone and marlymudstone. According to deep well data, the sedimentary rocks are confirmed from ground surface depth 50 m to 100 m.

In this machine boring survey. Junction white mudstone from ground surface depth 10 meters was confirmed at Al Muladdah.

Diluvium

Diluvium consists of fluviatile deposit, old fans, terrace deposit and is formed of consolidated and unconsolidated silt, sand, gravel and sandy gravel.

Alluvium

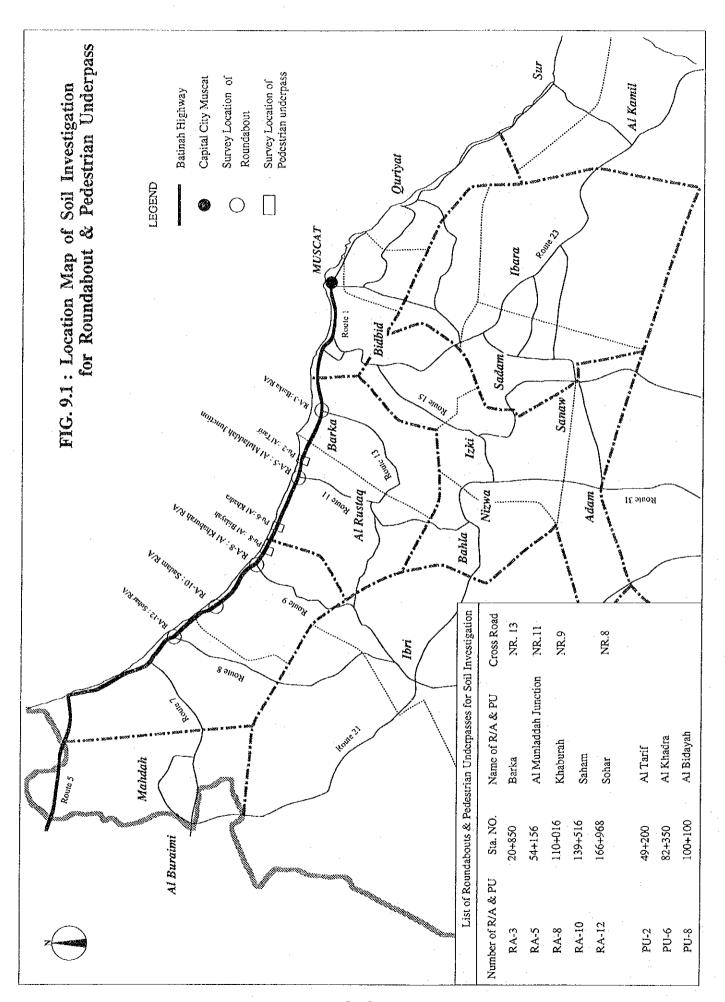
Alluvium consists of Wadi fluviatile deposit, recent fans and coastal deposit. These are formed of coastal sand with dunes or barchans and silt of which faces are marked by the intercalations of sand and granule or lamina of silt, clay and granule.

2) Soil Condition of Survey Site

Soil investigation was carried out at five roundabouts and three pedestrian underpasses. Refer to Fig. 9.1 location map of soil investigation for roundabouts & pedestrian underpasses and list of roundabouts & pedestrian underpasses for soil investigation.

a) R/A - 3 Barka

The deposit of the site consists of silty fine sand with rounded gravel and silty fine sand of solid status.



- Upper silty fine sand bed

From ground surface to 3.0 m depth, is investigated by hand digging test pit. The bed is formed of light brown loose to medium dense silty fine sand (depth $2.2 \sim 2.6$ m) and brown dense to very dense silty fine to medium sand (depth $2.2 \sim 3.0$ m).

Lower silty fine sand bed

The bed is formed of light brown silty fine sand with rounded gravel of diameter 0.5 to 5.0 cm. Thickness of soil bed is 4.0 meters and N-value ranges from 31 to 50/15.

- Fine sand of solid status bed

This bed is located at a depth of 91.2 m and is formed of light brown silty fine sand of solid status with rounded gravel. Thickness of bed is more than 3.3 meters and N-value ranges from 50 to 50/8.

b) R/A - 5 Al Muladdah Junction

Deposit of the site consists of sandy soil and mudstone. At the time of investigation, the ground water level was confirmed at a depth of 9.8 meters from ground surface.

- Sandy Soil

From ground surface to 2.2 m depth is investigated by hand digging test pit. The bed is formed of brown loose silty fine sand (depth $0 \sim 0.7$ m) and brown medium to dense silty fine sand (depth $0.7 \sim 2.5$ m).

The bed of depth from 2.5 m to 6.5 m is formed of brown silty fine sand of solid status with rounded gravel. N-value range from 36 to 50/15 and thickness of bed is 4.0 meters.

Mudstone

The bed is formed of white mudstone and intercalation of sandy soil of solid status. White mudstone is located at a depth of 93.0 meters to 84.4 meters and N-value range from 44 to 50/7. Intercalation of sandy soil is made up of silty fine to medium sand of solid status with rounded gravel and N-value range from 50/10 to 50/9.

c) R/A - 8 Khaburah

The deposit of the site consists of sandy soil and groundwater level was confirmed at a depth of 10.40 meter from ground surface. Ground surface to 3.0 meters depth is investigated by hand digging test pit. The bed is formed of brown loose silty fine sand (depth to 1.7 m), brown soft sandy silt (depth 1.7 m to 2.7 m) and brown medium dense silty fine sand (depth 2.7 m to 3.0 m).

- Upper sandy soil bed

The bed is formed of silty fine sand of semi solid status. This is located at a depth of 96.9 m to 93.1 m and thickness of bed is 3.8 meters with N-value ranges from 10 to 31.

Lower sandy soil bed

The bed is formed of fine to coarse sand with rounded gravel of diameter $0.5 \sim 5.0$ cm max, 10 cm. This is located at a depth of 93.1 m to 84.5 m and thickness of bed is 8.6 m. N-value ranges from 50/29 to 50/8.

d) R/A - 10 Saham

The deposit of the site consists of sandy soil and sand of solid status. At the time of investigation, the groundwater level was not measured at any of the boreholes drilled.

- Upper sandy soil bed

The bed is formed of brown silty fine sand and is located at a depth of 100 m to 92.6 m. Thickness of bed is 7.0 meters with N-value ranges from 7 to 22.

- Lower sandy soil bed

The bed is formed of fine to medium sand of solid status with much rounded gravel of diameter $0.5 \sim 3.0$ cm. This is located at an altitude from 91.3 m to 86 m and thickness of bed is more than 5.4 meters with N-value ranges from 57 to 50/10.

e) R/A - 12 Sohar

The deposit of the site consists mainly of sandy soil and can be divided into two deposit of upper and lower sandy soil bed, its boundary is at a depth of 87.0 meters. At the time of investigation, the groundwater level was confirmed at a depth of 11.5 meters from ground surface.

Upper sandy soil bed

Ground surface to 3.0 meters depth is investigated by hand digging test pit. The bed is formed of light brown silty fine sand and intercalates a brown fine sand bed with gravel. This is located at a depth of 99.5 m to 87.0 m with N-value ranges from 11 to 34. Intercalation of fine sand bed is located an altitude from 93 m to 90 m with N-value from 40 to 50/13.

- Lower sandy soil bed

The bed is made up of medium to coarse sand with rounded gravel and is located at a depth of 87.0 meters. The N-value ranges from 50 to 50/7 and the bed is reasonable bearing stratum for the structural design.

f) Pedestrian Underpass

Soil investigations for pedestrian underpasses were carried out at three location (Al Tarif, Al Khadrra and Al Bidaya). Bearing Strafa for pedestrian underpasses from ground surface to about 5 meters down consist of silty fine sand and sandy silt with N-value ranges from 34 to 43.

At the time of investigation, the groundwater level was not measured at any of the boreholes drilled.

PU-1 Al Tarif

The deposit of the site consists of silty fine sand, sandy silt and fine to medium sand with rounded gravel.

- Upper sandy soil bed

From ground surface to 1.6 meters depth is investigated by hand digging test pit. The bed is formed of light brown silty fine sand and yellowish grey sandy silt from ground surface to 5.5 meters with N-value ranges from 11 to 34.

- Lower sandy soil bed

The bed is made up of fine to coarse sand with rounded gravel sandy silt and silty fine sand of solid status, this is located at a depth of 93.4 meters to 88.47 meters with N-value ranges from 41 to 50/15.

PU-6 Al Khadra

The deposit of the site consist of silty fine to coarses sand with rounded gravel.

- Upper sandy soil bed

From ground surface to 2.5 meters depth is investigated by hand digging test pit.

The bed is formed of brown silty fine sand with partially rounded gravel and is located at a depth of 100.05 meters to 96.7 meters with N-value of 16.

- Lower sandy soil bed

The bed is made up of silty fine to coarse sand with rounded gravel and is located at a depth of 96 meters to 89.81 meters with N-value ranges from 37 to 50/24.

PU-8 Al Bidayah

The deposit of the site consist of brown silty fine to coarse sand with rounded gravel.

- Upper sandy soil bed

From ground surface to 2.5 meters depth is investigated by hand digging test pit. The bed is formed of silty fine sand with rounded gravel, and is located at a depth of 99.6 meters to 96.3 meters.

Thickness of the bed is 3.3 meters and N-value is 26.

Lower sandy sotl

The bed is made up of fine to coarse sand with rounded gravel and is located at a depth of 96.3 meters to 89.4 meters. Thickness of the bed is 7.0 meters and N-value ranges from 30 to 50/13.

(3) Soil Laboratory Test

1) General

The soils subject to analysis for the earth work design are sandy soil and cohesive soil bed of which a total 37 disturbed samples were taken by split-spoon sampler. Test results are referred to in the analysis as the following soil categories.

Gravelly sand soil 3 samples
Sandy soil 17 samples
Cohesive soil 17 samples
Total 37 samples

Table 9.4 Contents of Laboratory Test

Test	Standard	Unit	Quantities
Natural Water Content	BS 1377: Part 2: 1990: 3.2	Test	37
Specific Gravity	BS 1377: Part 2: 1990: 8.3	Test	20
Grain Size Analysis	BS 1377: Part 2: 1990: 9.2	Test	20
Atterberg Limit Determination	BS 1377: Part 2: 1990: 4.3	Test	4
In-Situ Dry Density	BS 1377 (1990) "method of test for soils for Civil Engineering Purposes"	Test	22

2) Soil Laboratory Test Result

A) Particle Size Gradation

The gradation of three categories is shown in Table 9.5 and Fig. 1-1 in Appendix.

The deposit of sandy soil contains coarse particles more than $76.4 \sim 90.6\%$ and cohesive soil contains fine particles of clay and silt more than 64.2% of total weight.

Table 9.5 Result of Particle Size Gradation

Items of Gradation	Gravel	Sand	Silt - Clay	No. 10 (2.00 mm)	No. 40 (0.425 mm)	No. 200 (0.075 mm)
and Average	(%)	(%)	(%)	(%)	(%)	(%)
	Average Value	Average Value	Average Value	Average Value	Average Value	Average Value
Deposit	Representa- tive Range					
Gravelly Soil	41.7	48.9	9.4	58.3	42.5	9.4
	35.6 ~ 47.8	42.1 ~ 55.7	8.4 ~ 10.4	52.2 ~ 64.4	32.5 ~ 52.5	8.4 ~ 10.4
Sandy Soil	9.0	67.4	23.6	91.0	83.6	23.6
	1.3 ~ 19.5	57.7 ~ 76.9	12.9 ~ 34.3	80.4 ~ 100	68.8 ~ 98.4	12.9 ~ 34.3
Cohesive Soil	5.3	30.5	64.2	94.7	93.6	64.2
	1.6 ~ 9.0	17.8 ~ 43.2	51.9 ~ 76.5	90.9 ~ 98.5	89.7 ~ 97.8	51.9 ~ 76.5

B) Character of Consistency

The objective of the test is to classify soil by Particle Size Gradation. The character of consistency is summarized in Table 9.6 and is shown Fig. 1-2 in Appendix.

- No change of consistency is observed with increasing depth below ground.
- According to the consistency chart, cohesive deposit is to be classified into ML or CL.
- Colloidal activity

Cohesive deposits are to be classified as non-active clay (mainly kaolinite) and ordinary clay (mainly illite).

Cohesive deposits are classified as being in a stable condition as
 WL > Wn and Ic = 1.04 (0.90 ~ 1.18).

Table 9.6 Result of Consistency

Items of Consistency	WH (%)	W1 (%)	Ip	If	It	Ic	Activity Ratio
	Average Value						
Deposit	Represen- tative Range	Represen- tative Range	Represen- tative Range	Represen- tative Range	Represen- tative Range	Represen- tative Range	Represe- ntative Range
Cohesive	16.9	34.5	9.9		· -	1.04	0.72
Soil	8.5 ~ 25.3	32.9 ~ 36.1	9.0 ~ 10.8	-	•	0.90 ~ 1.18	0.58 ~ 0.82

Note:

CH: High plasticity and cohesive clay, non organic

OH: Organic clay with medium plasticity

MH: Non-organic silt, mica or diatomaceous fine sand/silt and elastic silt

ML: Non-organic silt, fine sand, silty or clayey fine sand

OL: Low to medium plasticity non-organic silt, sandy clay low-cohesive clay

Low to medium plasticity silt, clay with sand or gravel, and low cohesive clay

Wn: Natural water content

WL: Liquid limit
Ip: Plasticity index
If: Flow index

It: Toughness index (It = Ip/If)

Degree of shear strength at plastic limit

Ic: Consistency index (Toughness and stability of cohesive soil)

Ic = WL - Wn/Ip Ic ≥ 1 Stable condition

Ic = 0 Unstable condition: Liquidizes by disturbance

Colloidal activity:

Colloidal activity has deep ties with clay mineral and geological condition of sediment, and is defined by Skempton.

Clay is classified into four groups from non-active clay to high active clay as more than 2. It is shown as the following formula.

Colloidal activity =

Plasticity index Ip Soil particle (%) of less than 2µ

Table 9.7 Classification by Colloidal Activity

Activity Ratio	Kind of Cohesional Soil by Activity Ratio	Main Clay Mineral	Deposit Condition
A < 0.75	Non active clay	Kaolinite	 Clay of aqueous and fresh water sediment Clay of marine deposit which has been leached.
A = 0.75 ~ 1.25	Ordinary clay	Illite	Clay of marine and estuarine deposit
A > 1.25	Active clay	 Including organic colloid A = 2 is including Montmorillonite 	

C) Specific Gravity, Wet Density and Void Ratio

The above are summarized in Table 9.8.

Table 9.8 Result of Gs y t e

Items of Soil	Specific Gravity	Wet Density	Void Ratio
Properties	Gs	γt (t/m³)	e
	Average Value Ave		Average Value
Deposit	Representative	Representative	Representative
	Range	Range	Range
Gravelly Soil	2.524 2.422 ~ 2.626	-	- :
Sandy Soil	2.538	1.832	0.447
	2.448 ~ 2.628	1.702 ~ 1.962	0.350 ~ 0.544
Cohesive Soil	2.440	1.750	0.479
	2.380 ~ 2.500	1.649 ~ 1.851	0.405 ~ 0.553

- Specific Gravity

The test results yield reasonable value with standard deviation of less than 0.090.

Wet Density

The tests was carried out by in-situ dry density with test pit and test results was obtained reasonable value.

Other factors are shown in the following formula:

$$\gamma t = \frac{1 + Wn/100}{\frac{1}{Gs} + \frac{Wn/100}{Sr/100}} \bullet \gamma w$$

where, γ t: Wet density of soil (t/m³)

Wn: Natural water content (%)

Sr: Degree of saturation (%) γ w: Density of water (= 1.00 t/m³)

Gs: Specific gravity

The average values are shown below:

Deposit	Dry Density γ d(t/m³)	Wet density γt (t/m³)	Degree of Saturation Sr
Sandy Soil	1.76	1.83	10.9
Cohesive Soil	1.52	1.75	30.5

Void Ratio

The average values of void ratio are shown below:

Sandy Soil

 $\ddot{e} = 0.447$

Cohesive Soil

 $\bar{e} = 0.479$

(4) Foundation Strata for Structural Design

A) Criteria and Distribution of Bearing Strata

The load bearing strata for structures is assessed depending on importance of the structure and the longitudinal forces to be carried by the structure.

In general, the criteria for spread or piled foundations of bridge abutments and piers is defined as the following N-values:

Sandy Soil

N > 30

Cohesive Soil

N > 20

Result of soil investigation, the depth of bearing strata of the above N-values and more than 50 are shown as Table 9.9.

Table 9.9 Bearing Strata for Structural Design

	Bearing Strata					
Site	N-Value mo	ore than 30	N-Value more than 50			
	Depth (Altitude)	Soil Bed	Depth (Altitude)	Soil Bed		
Barka R/A	3.0 ~ 3.5 (95.5 ~ 96.5)	Silty fine sand	3.0 ~ 6.0 (93.5 ~ 96.5)	Silty fine sand		
Al Muladdah Junction	3.0 (97.0)	Silty fine sand	4.0 (95.5)	Silty fine sand		
Khabu Rah R/A	5.0 (94.5)	Silty fine sand	6.0 (93.5)	Silty fine sand		
Saham R/A	7.0 (92.5)	Silty fine sand	8.0 ~ 9.0 (9.5)	Fine to medium sand		
Sohar R/A	5.0 ~ 6.0 (92.5 ~ 93.5)	Silty fine sand	12.0 ~ 13.0 (86.5)	Medium to coarse sand		
Al Tarif P/U	5.0 (94.0)	Sandy site	6.0 (93.0)	Fine to medium sand		
Al Khadra P/U	4.0 (96.0)	Silty fine sand	6.0 (94.0)	Silty fine sand		
Al Bidaya P/U	4.0 (96.0)	Silty fine sand	7.0 (93.0)	Medium to coarse sand		

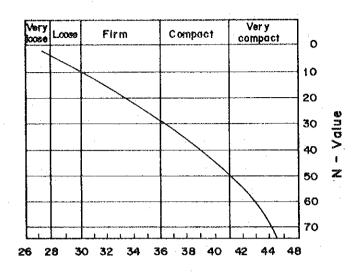
B) Soil Values of Bearing Strata

Soil values to decide the bearing strata for structures are summarized in Table 9.10.

Table 9.10 Soil Values of Bearing Strata

Division of Soil	Wet Density	Cohesion of the First Stage	Internal Friction Angle	Modulus of Elasticity
	γt (t/m3)	c (t/m²)	Ø (Degree)	Eo (kg/cm²)
Sandy Soil	1.83	•	Fig. 9.2	-
Cohesive Soil	1.75	0.625N	-	28N

Note: N-value and ground water level are shown in the soil profile.



INTERNAL FRICTION ANGLE (DEGREE)

Fig. 9.2 Relative Chart for N-Value and Internal Friction Angle

9.3 The Required Number of Lanes

The number of lanes will be determined from the ratio of Traffic Demand Forecast (refer to Chapter 5) and the Traffic Capacity (refer to Chapter 6) of the Batinah Highway.

In this Study the number of lanes will be planned in the following manner in consideration of the present number of lanes and the planned traffic volume.

- Main Highway

The design traffic volume on the Batinah Highway in 2010 was forecast from 59,000 to 13,700 pcu/day, and the maximum design traffic volume is predicted to be 60,000 pcu/day, so it has been determined to plan for 4 lanes the same as for the present Batinah Highway.

Interchange Ramps

The forecast traffic volume on the Muscat-side of the Barka Roundabout (R/A-3) is 13,000 pcu/day, and 9,000 to 400 pcu/day for the others. The maximum traffic volume is forecast for 12,000 pcu/day, so the number of lanes for the Muscat-side of the Barka Roundabout (R/A-3) has been decided as 2, with the others planned as one lane.

- Section of Inflow of Roundabout

The width of the connection of the inflow lane and the roundabout will be w = 9 m.

9.4 Geometric Design of Grade Separation Facilities

9.4.1 General

The horizontal and vertical alignment for the grade separation facilities to be used at the eight selected roundabouts will be as follows while making use of the design criteria, survey data, and results of the geological (soils) data in Paragraph 9.2.

9.4.2 Horizontal and Vertical Alignment

(1) Horizontal Alignment

The horizontal alignment will be designed as follows:

- Changes in horizontal and vertical alignment will be gradual for safe travel at high speeds (120 km/h).
- Horizontal alignment will be planned so that the grade separation facilities can be designed within the right-of-way of the present roadways and roundabouts.
- The roundabout will be planned so that all intersection facilities can be confined within the outer circle of the present roundabout; therefore the connections of ramps with the intersecting roads and roundabout as well as the spacing of the footings and foundations, can all be contained within the circular space. The sight distance in the vicinity of the intersection (where there will be no supports) shall be greater than those shown in Fig. 9.3 for the Stopping Sight Distance on Horizontal Curve.

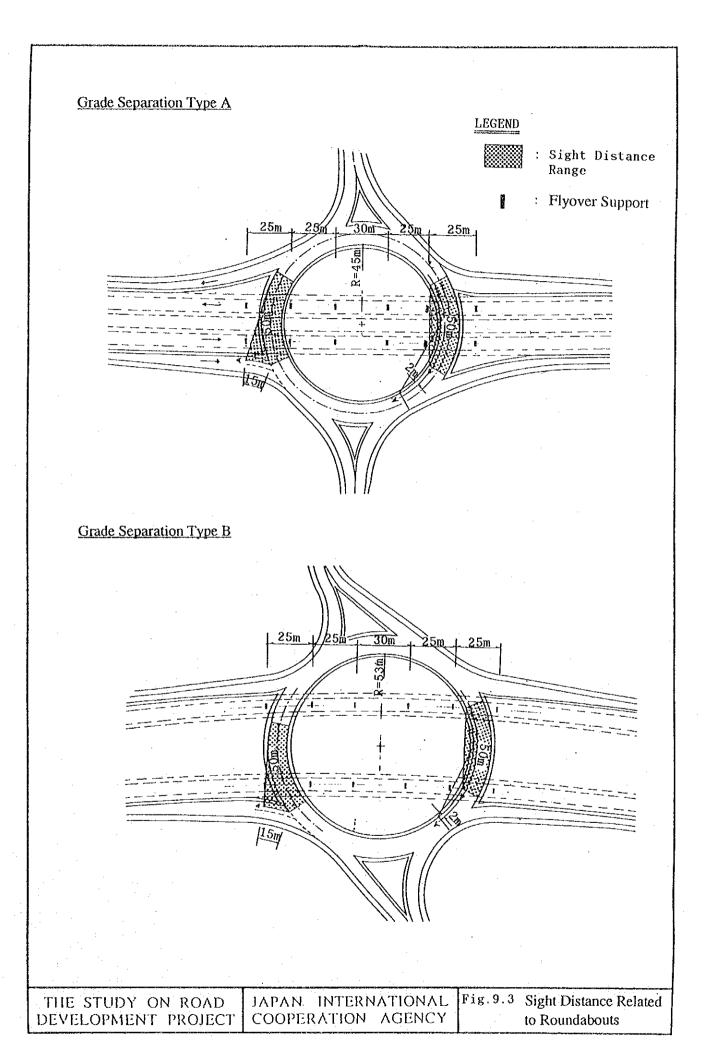
The grade separation facility (flyover) for Roundabout R/A-18 will have a special shape. Refer to paragraph 9.4.3 for a comparison of the alternate proposed structures.

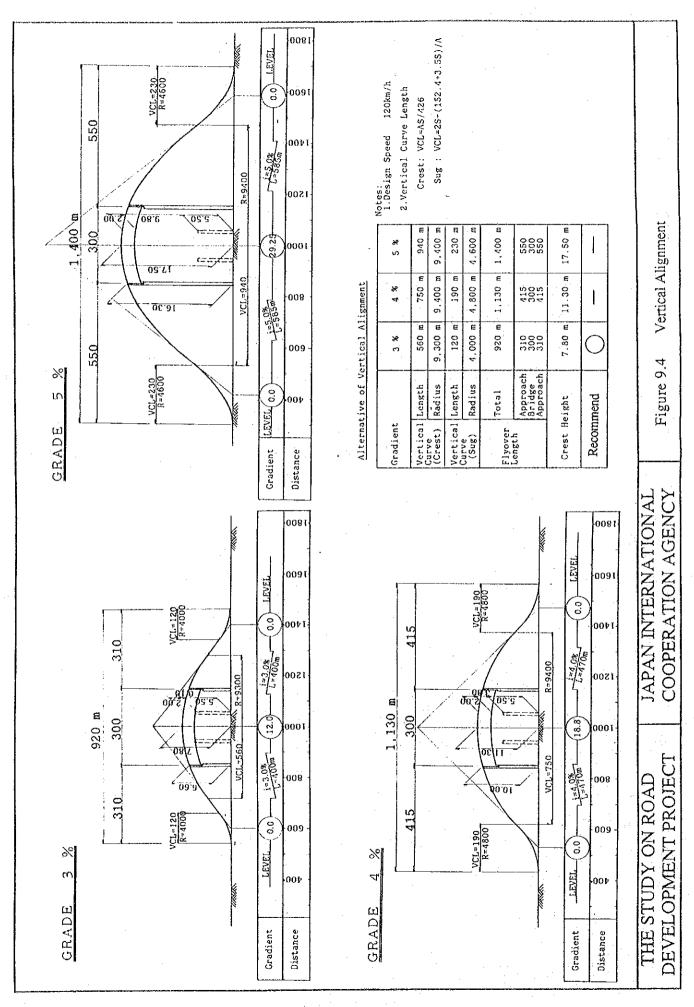
(2) Vertical Alignment

- The vertical alignment for a vehicle travelling at 120 km/h with grades of 3 %, 4 % and 5 % are given in Fig. 9.4.
- Of the alternate grades, the one with the lowest height of grade separation facility will be selected.

(3) Location for the Pedestrian Walkway.

The pedestrian walkway will be planned for the convenience of the villagers while keeping in mind the concepts given in Chapter 8.





9 - 22

9.4.3 Preliminary Design for Grade Separation Facility

The preliminary design for the grade separation facility based on the above consideration is as shown in Volume IV: Drawings.

9.4.4 Alternate Plans for Grade Separation Facilities

The proposed basic plan for the grade separation facility is given in paragraph 9.4.3. Alternate plans are made for R/A-5 (Al Muladdah Junction), and R/A-12 (Sohar), and R/A-18 (Aqr) which have been provided. These alternate plans should be given further consideration with the development of project and its implementation.

(1) Roundabout R/A-5

The Proposed Concept.

R/A-5 is presently of the T-shaped junction configuration. In view of the traffic conjunction in this area, the Oman Government has already planned work to improve the intersection. The preliminary plan given in paragraph 9.4.2, based on this concept is a grade separated version.

Alternate Plan

Compared to the other roundabouts, the volume of traffic on R/A-5 has a large volume of traffic in the directions of Muscat and Al Rustaq (Route 11). One concept proposed places emphasis on this traffic. The plan placing emphasis on the Muscat - Al Rustaq traffic (Route 11) is based on the following concept:

- To allow passage of the heavy traffic without obstructing other traffic and allowing smooth passage:
- Mandatory stopping for secondary traffic;
- To clarify the primary and secondary traffic flows.

Comments:

The above two comments have the following merits:

- The proposed plan permits the traffic from the village to flow smoothly through the roundabout.
- The alternate plan allows the largest flow of traffic to flow smoothly.

The two proposals above possess the following characteristics:

- The original proposal enables the roundabout to ameliorate traffic from the neighboring community.
- 2) The comparative proposal prioritizes major traffic directions other than the trunk line for smooth flow.

Either of the above proposals call for a grade separation of the Batinah Highway, leaving the role of the ground level lanes to be one of service to the local community. Appropriate planning should be conducted to ensure maximum convenience.

The Fig. 9.5 gives a comparative plan which gives priority to the traffic on Batinah Highway and Route 11.

(2) Roundabout R/A-12

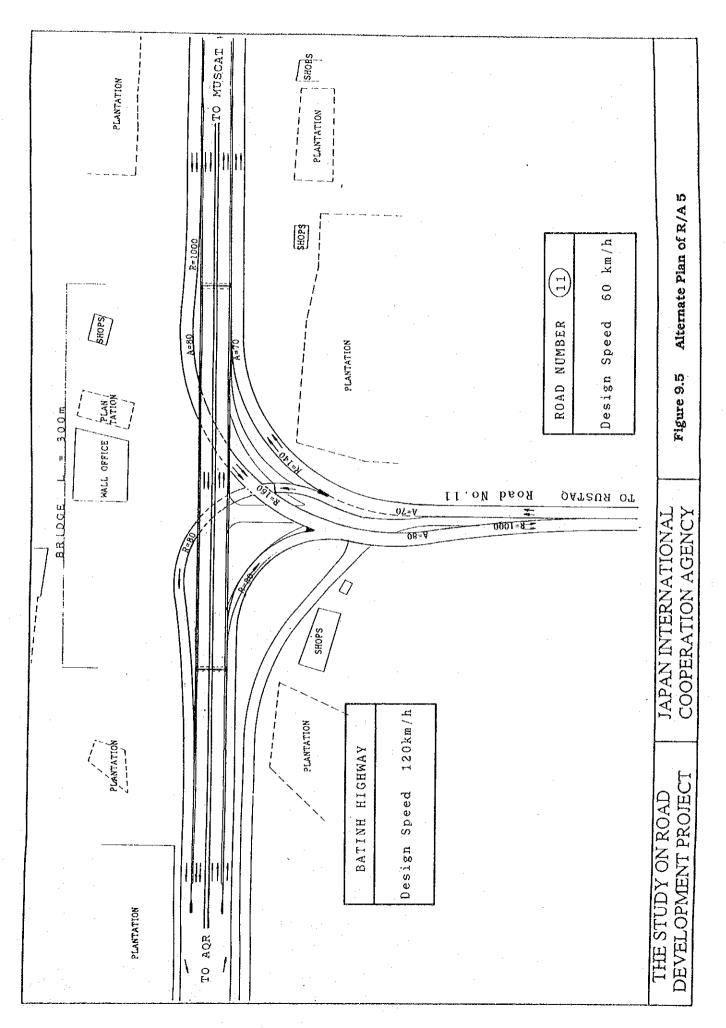
The proposed concept

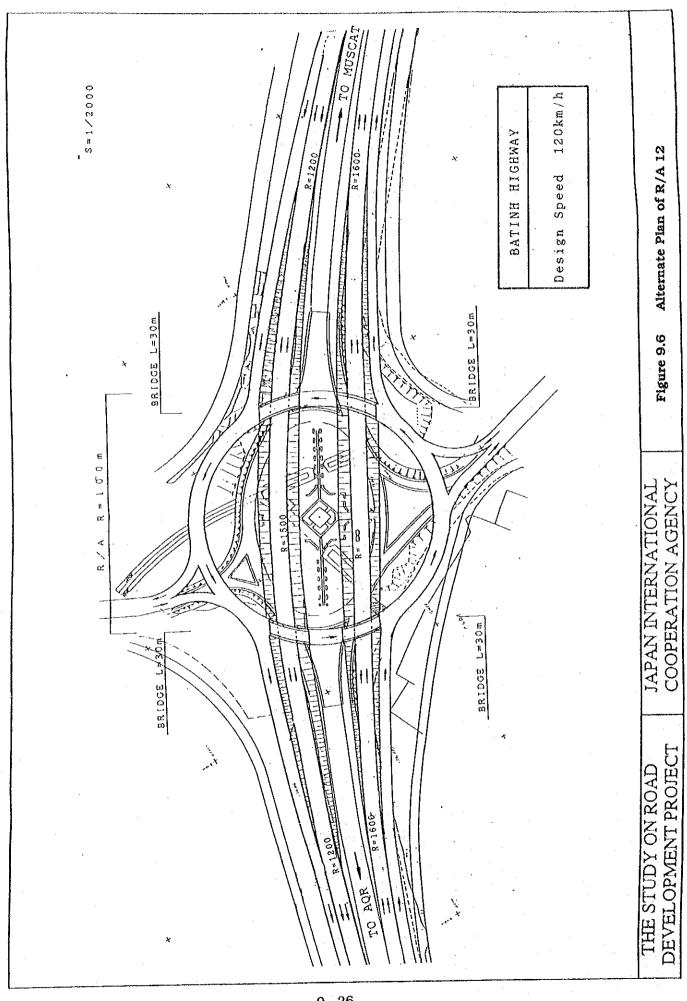
The location of R/A-12, Sohar, will be sited in the center of the Batinah Highway where the land around the roundabout is most advanced. There is an existing monument, but after completion of the study a new monument will be erected by the government, and will be one of the most important roundabouts in the government.

For these reasons the proposed monument and grade separation facilities will become one of the most outstanding landmarks in the Sultanate.

Alternate plan

From the point of aesthetics, it has been proposed to construct the grade separation an underground facility. The underground facility for the grade separation will make the monument stand out but will required special studies for the concentration of the various structures. For the local residents it will not be possible to cross over the Batinah Highway in the vicinity of the grade separation facility except at the intersecting highway. Fig. 9.6 shows the proposed underground plan.





Comments

The new proposed monument is expected to be some 40 m high and is expected to be visible by both drivers and local residents and is not considered to obstruct the view.

(3) Roundabout R/A-18 (Aqr)

Proposed plan

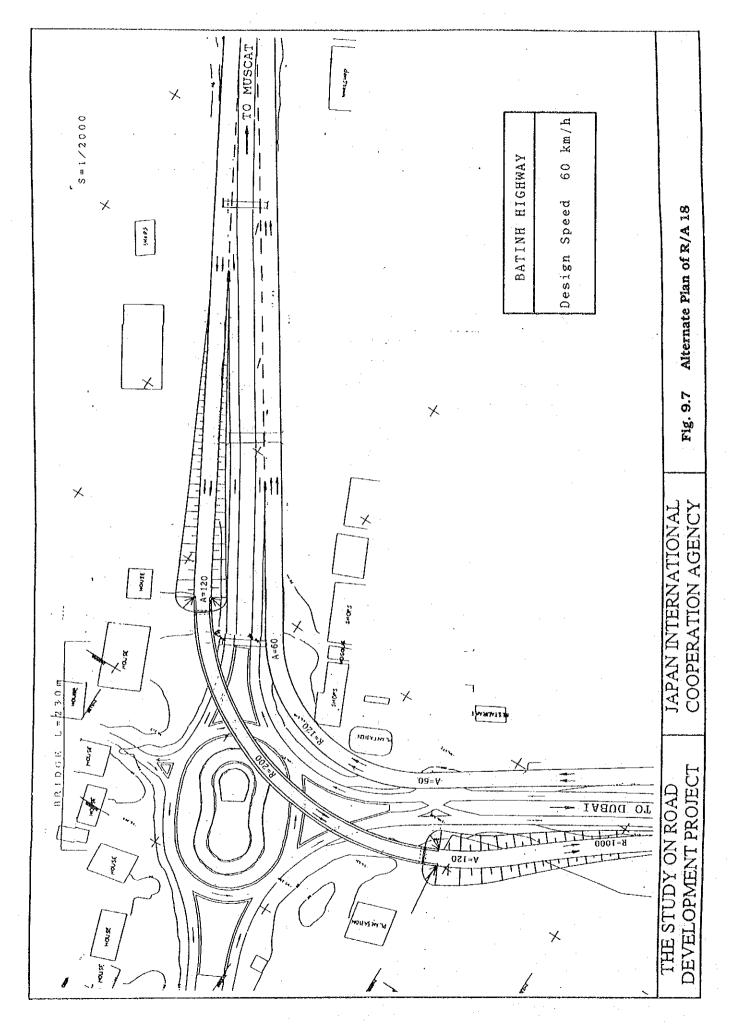
The main flows of traffic at R/A-18 (Aqr) are towards Dubai and Muscat, and it is proposed to make the grade separation facility in those directions. Therefore, the plan proposed in paragraph 9.4.2 places emphasis in the Muscat and Dubai directions and the interconnection therewith. It is a shortcut route which allows traffic to move at 120 km/h, and requires no relocation of the local residents. It will be necessary to elevate the grade separation facility at 2 locations, one on the Batinah Highway side, and the other on the Route No. 5 side.

Alternate plan

The comparative plan is to make maximum use of the existing roadway for the grade separation facility. In order to keep the relocation of local residents to a minimum, it will be necessary to reduce the design speed (60 km/hr) of the vehicles. Fig. 9.7 gives a plan of the comparative concept.

Comments

R/A-18 (Aqr) is the branch-off junction for the Dubai and Musandam directions, and is an important symbol for Oman. For this reason, for its importance as an interconnection to Muscat and Dubai the speed of 120 km/h has not been changed, and as a symbol a new grade separation facility with a monument is recommended to be built at this intersection.



9.5 Preliminary Design of Bridges

9.5.1 General

In paragraph 9.4, (Geometric Design of Grade Separation Facilities) the type of facility structure was determined at the time the field survey and boring for the foundations was conducted at the project roundabout structures. Preliminary design for a total of eight roundabouts was performed. After the type of structure was determined, the aesthetics were considered.

After the evaluation of bridge, types was determined, preliminary design was performed, preliminary quantities were estimated, cost estimates were performed as described in another chapter, construction planning/implementation planning prepared, and the data for economic analysis was obtained.

The flow chart for the grade separation structure together with the selection of scale was determined in accordance with the following flow chart in Fig. 9.8.

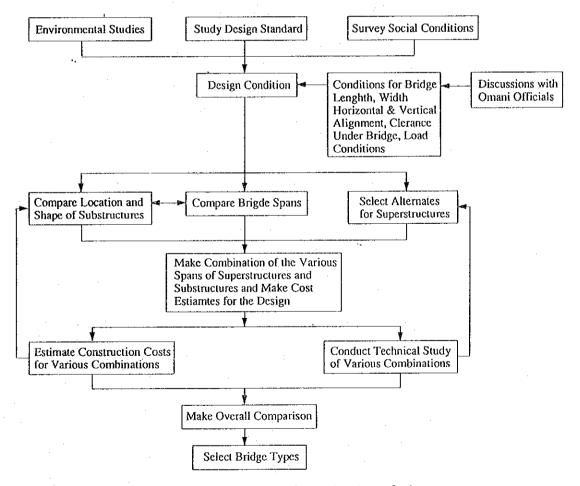


Figure 9.8 Flow Chart for Selection of Flyover

9.5.2 Bridge Design Criteria

The Bridge Design Criteria used in this study is as follows:

(1) Superstructure

Bridge Type

RC or PC Bridge

Bridge Live Load

Omani Standards, revised in 1994 (refer to Chapter

6.2)

Bridge Width

12.7 m

Material Strength

Concrete

 $\sigma_{28} = 350 \text{ kg/cm}^2 \text{ (precast)}$

 $\sigma_{28} = 350 \text{ kg/cm}^2 \text{ (precast)}$

Reinforcing Bar:

 $\sigma_1 = 1.800 \text{ kg/cm}^2$

: $\sigma_1 = 1,400 \text{ kg/cm}^2 \text{ (bridge slab)}$

(2) Substructure

Abutment

Inverted T-Type, RC Abutment

Pier

Inverted T-Type, or Rigid Frame π Shape Pier

Material Strength

Concrete

 $\sigma_{28} = 240 \text{ kg/cm}^2 \text{ (precast)}$

 $\sigma_{28} = 80 \text{ kg/cm}^2 \text{ (precast)}$

Reinforcing Bar:

 $\sigma_1 = 1.800 \text{ kg/cm}^2$

(3) Foundation

The foundation will be constructed with RC piles locally available.