Comparative Study of Pumps and Pumping Station Table G.3.3.3.

(1/6)

- Abstract of Facilities and Cost Comparison in Case Full (100%) Water of Regulating Ponds is Available (1)

				Case	38		
	l	1-1	I-2	I-3	4.1	г п	
Pimn Bore	mm	2,600	2,200	2,000	1,650	2,600	1,650
Canacity year [fait	m3/s	14.40	12.00	00.6	900.9	15.00	90.9
Capacity yel Olik	m3/min	864.0	720.0	540.0	360.0	0.006	360.0
Number of Units	100	Ϋ́	9	9 0	12	च	7
Total Mand of Dumas	E	22.7	22.7	22.7	22.7	22.7	7.22
Motor Douger nor Thit	<u>\$</u>	4.200	3,600	2,700	1,800	4,400	1,800
Gize of Pump House (Width * Length)	m * m	70.0 * 32.5	72.0 * 32.5	80.0 * 32.5	96.0 * 32.5		72.0 * 32.5
Size of Settling Basin (Width * Length)	# #	69.0 * 100.0	71.0 * 100.0	79.0 * 100.0	95.0 * 100.0		71.0 * 100.0
Annual Total Water Remirement	MCM	1.307.7	1,307.7	1,307.7	1,307.7		1,307.7
Annual Total Water Sumiv by Primis	MCM	1,373.6	1,355.1	1,323.5	1,308.4		1,313.4
Annual Total Frees Water	MCM	63.9	47.4	15.8	8.0		5.7
Author Total Oceration House of Pumps	Ŀ	26.496	31,368	40,848	925'09	21,432	8,472
Funn Faniaments Cost	1.000Rs.	935,050	943,640	1,006,660	1,068,280		937,800
Pinns	1,000Rs.	313,000	337,200	360,800	361,200		310,600
Motors	1,000Rs.	207,500	214,200	224,800	235,200		212,000
Valves	1,000Rs.	153,150	125,040	130,160	135,480	٠	145,100
Pines	1,000Rs.	100,000	102,000	112,000	130,000		102,000
Others	1,000Rs.	161,400	165,200	178,900	206,400		168,100
Construction Cost of Pirmo House	1,000Rs.	27,300	28,080	31,200	37,050		27,690
Construction Cost of Settling Basin	1,000Rs.	6,540	069'9	7,310	8,530		069'9
Construction Cost of Suction Pit		the second contract of	The second secon	The second section of the second section is a second section of the second section of the second section secti	The second section is a second section of the second section of the second section of the second section secti	to the state of th	
and Basement of Pump House	1,000Rs.	104,940	108,660	118,840	142,730		108,660
Concrete Work	1,000Rs.	87,350	90,710	99,670	120,490		90,710
Earthwork	1,000Rs.	4,130	4,190	4,410	4,940		4,190
In-situ RC Piling	1,000Rs.	13,460	13,760	14,760	17,300	prompty or any specific policy and a second second second section of Administration and the second s	13,760
Miscoelaneous Works (5%)	1,000Rs	53,690	54,350	58,200	62,830		54,040
Direct Cost	1,000Rs.	1,127,520	1,141,420	1,222,210	1,319,420		1,134,880
Indirect Cost (16 % of Direct Cost)	1,000Rs.	180,400	182,630	195,550	211,110		181,580
Contingencies (22 % of Direct Cost)	1,000Rs.	248,050	251,110	268,890	290,270		249,670
Total Construction Cost	1,000Rs	1,555,970	1,575,160	1,686,650	1,820,800		1,566,130
Replacement Cost of Pump Equipments	1,000Rs.	929,640	933,490	987,750	1,009,990		921,430
Annual O & M Cost							
Electricity	1,000Rs.	171,365	174,031	170,210	168,394		168,948
Maintenance for Pump Equipments	1,000Rs.	18,701	18,873	20,133	21,366		18,756
Total O& M cost							
in the Early Stage of the Project Life (50%)) 1,000Rs.	95,033	96,452	95,172	94,880		93,852
in the Late Stage of the Project Life	1,000Rs.	190,066	192,904	190,343	189,760		187,704
Net Present Value	1,000Rs.	3,425,500	3,462,600	3,615,600	3,769,200		3,415,800

Comparative Study of Pumps and Pumping Station **Table G.3.3.3.**

(2/6)

- Abstract of Facilities and Cost Comparison in Casc Full (100%) Water of Regulating Ponds is Available (2)

				Case					
	1	11 -2		П -3а		П -3Ъ		п -3с	ļ
			(05)	0000	1 650	2 000	1.350	2,000	1,650
Pump Bore	HI H	2,200	1,050	2,000	96.5	2000	00 7	10.00	5.50
Capacity per Unit	m3/s	12.00	9.00	00.01	9.00	10.00	7 6	00:07	3300
	m3/min	720.0	360.0	0.009	360.0	600.0	240.0	900.0	0.000
Mumber of Haite	sou	S	73	9	Ġ	9	<u></u>	n j	1 1
Total first of the control of the co		22.7	22.7	22.7	22.7	22.7	22.7	22.7	22.7
Ioual read of Fumps	 <u></u>	3 600	1.800	3,000	1,800	3,000	1,200	3,000	1,700
Motor Power per Unit	***	2226	760 * 32 \$		76.0 * 32.5		81.0 * 32.5		82.0 * 32.5
Size of Pump House (Width * Length)	==== =================================		0.000		75.0 * 100.0		80.0 * 100.0		81.0 * 100.0
Size of Settling Basin (Width * Length)	m * m		75.0 * 100.0		7.00 100.0		1 202 2		1 307 7
Annual Total Water Requirement	MCM		1,307.7		1,307.7		1,307.7		1,000,1
Ammel Total Water Supply by Pumps	MCM		1,313.6		1,308.3		1,312.4		1,510.7
Amend Total Water Capper) of a cappe	MCM		0.9		9.0		4.7		3.0
Annual Total Excess water	h	28 704	3.408	31.416	8,208	30,888	13,920	29,544	12,480
Annual Total Operation froms of Funitys	1 0000	2,0	964 780		963,500		980,700		1,003,310
Pump Equipments Cost	1,000ps		341 200		330,800		332,400		345,900
Pumps	1,000ns		217.700		227,000		230,700		231,300
Motors	1,000KS.		136 700		120,200		119,100		126,510
Valves	1,000Rs.		08/97		000 201		113.000		114,000
Pipes	1,000Rs.		10/,000		000,001		195 500		185.600
Others	1,000Rs.		172,100		1005,871		002,001		31.080
Construction Cost of Pump House	1.000Rs.		29,640		29,640		OKC, IC		00,10
Constant Cost of Contino Bosin	1 000Rs		7,000		7,000		7,380		0.40
Construction Cost of Setuming Dasail				The second second section of the second seco			 :		
Construction Cost of Suction Pit	4000		114 240		116.080		122,730		123,730
and Basement of Fump House	1,000Ks.		04,441		97.410		103,180		103,990
Concrete Work	1,000Rs.;		0/9,5%		4 320		4,470		4,510
Earthwork	1,000Rs.		026,4		045, 41		15.080		15,230
In-situ RC Pilmg	1,000Rs.		14,350	A	0.00.5		061.75	op page - manager out of the second of the second	58 320
Miscelaneous Works (5%)	1,000Rs.		55,790		018,00		0,71,7		1 224 800
	1,000Rs.		1,171,550		1,172,030		1,199,520		000,000
Indirect Cost (16 % of Direct Cost)	1,000Rs.		187,450		187,520		076,181		0,000
9	1.000Rs		257,740		257,850		263,890		004,407
, c	1 000Rs		1,616,740		1,617,400	!	1,655,330		1,690,230
Total Couls duchon Cost	1 000 P		046 240		935.640		941,440		971,120
Replacement Cost of Pump Equipments	1,000Ks.		213,014						
Annual O& M Cost					27.0		169 974		169.665
Electricity	1,000Rs.		169,020		100,575		100,001		30.066
Maintenance for Pump Equipments	1,000Rs.		19,296		19,270		19,614		20067
Total O& M Cost					000		200		94 866
in the Early Stage of the Project Life (50%) 1,000Rs.) 1,000Rs.		94,158		778,66		44.24.44		100,731
in the Late Stage of the Project Life	1,000Rs.		188,316		187,643		188,488		167,731
Net Present Value	1.000Rs.		3,494,200		3,480,900		3,528,800		3,599,600

Comparative Study of Pumps and Pumping Station Table G.3.3.3.

(9/8)

181,580 249,670 18,756 188,852 310,600 212,000 145,100 102,000 27,690 108,660 90,710 4,190 13,760 54,040 1,134,880 ,566,130 170,096 94,426 1,329.2 71.0 * 100.0 3,421,400 1,307.7 72.0 * 32.5 T II 2,600 15.00 900.0 21,936 235,200 135,480 130,000 211,110 360.0 22.7 1,800 1,329.2 21.5 61,536 361,200 206,400 37,050 142,730 120,490 4,940 17,300 170,899 21,366 96,133 3,781,500 95.0 * 100.0 ,319,420 290,270 ,820,800 192,265 1,307.7 96.0 * 32.5 - Abstract of Facilities and Cost Comparison in Case 50% Water of Regulating Ponds is Available (1) 4 Case 196,263 118,840 99,670 4,410 14,760 20,133 98,132 360,800 224,800 130,160 112,000 178,900 195,550 268,890 3,644,900 79.0 * 100.0 1,372.5 **\$**. 42,360 ,686,650 80.0 * 32.5 Ę 1,408.0 943,640 337,200 214,200 125,040 102,000 251,110 4,190 180,420 3,600 28,080 54,350 182,630 3,494,200 32.592 199,293 72.0 * 32.5 71.0 * 100.0 ,575,160 933,490 3 97,956 1,423.3 115.6 27,456 935,050 313,000 207,500 153,150 100,000 104,940 87,350 13,460 ,127,520 180,400 026,555, 216,261 3,454,400 14.40 69.0 * 100.0 4,130 53,690 248,050 70.0 * 32.5 I ,000Rs. ,000Rs. 1,000Rs. 1,000Rs. ,000Rs 1,000Rs. ,000Rs. 1,000Rs. in the Early Stage of the Project Life (50%) 1,000Rs. 1,000Rs. 1,000Rs. ,000Rs. ,000Rs. ,000Rs ,000Rs. ,000Rs m3/min E .E MCM ,000Rs. ,000Rs. ,000Rs 000Rs .000Rs. MCM MCM nos Ħ <u>홍</u> (16 % of Direct Cost) (22 % of Direct Cost) in the Late Stage of the Project Life Size of Settling Basin (Width * Length) Annual Total Operation Hours of Pumps Size of Pump House (Width * Length) Replacement Cost of Pump Equipments Maintenance for Pump Equipments and Basement of Pump House Annual Total Water Supply by Pumps Construction Cost of Settling Basin Construction Cost of Suction Pit Construction Cost of Purp House Annual Total Water Requirement Misceelaneous Works (5%) Annual Total Excess Water Fotal Construction Cost Pump Equipments Cost Fotal O & M Cost Motor Power per Unit Total Head of Pumps In-situ RC Piling Annual O & M Cost Concrete Work Net Present Value Capacity per Unit Number of Units Contingencies Earthwork Indirect Cost Electricity Valves Direct Cost Pump Bore Motors Pumps Pipes

6,690

21.5

7,680

1,800

Comparative Study of Pumps and Pumping Station Table G.3.3.3.

- Abstract of Facilities and Cost Comparison in Case 50% Water of Regulating Ponds is Available (2)

				Case					
	l	П -2		п -3а		48- II		Л -3с	
Dan Dan	8	2,200	1.650	2.000	1,650	2,000	1,350	2,000	1,650
runip Dote	3/6	12.00	00.9	10.00	90.9	10.00	4.00	10.00	5.50
Capacity per cont	m3/min	720.0	360.0	600.0	360.0	0.009	240.0	0.009	330.0
Number of Huite	Sou	'n	77	9	2	9	3	\$	4
Total Hand of Prints	E	22.7	22.7	22.7	22.7	22.7	7.22	22.7	22.7
Motor Downer nor I'mit	<u>.</u>	3,600	1,800	3,000	1,800	3,000	1,200	3,000	1,700
Size of Pump House (Width * Length)	m * m		76.0 * 32.5		76.0 * 32.5		81.0 * 32.5		82.0 * 32.5
Size of Settling Basin (Width * Length)	- H		75.0 * 100.0		75.0 * 100.0		80.0 * 100.0		81.0 * 100.0
Annual Total Water Requirement	MCM		1,307.7		1,307.7		1,307.7		1,307.7
Annual Total Water Sumbly by Pumps	MCM		1,324.0		1,312.4		1,321.1		1,324.0
Annual Total Excess Water	MCM	•	16.3		4.7		13.4		16.4
Annual Total Operation Hours of Punps	щ	28,704	4,128	31,704	7,920	31,656	12,600	30,984	10,536
Pump Equipments Cost	1,000Rs		964,780		963,500		980,700		1,003,310
Pumps	1,000Rs.		341,200		330,800		332,400		345,900
Motors	1,000Rs.		217,700		227,000		230,700		231,300
Valves	1,000Rs.		126,780		120,200		119,100		126,510
Pipes	1,000Rs.		107,000		107,000		113,000		114,000
Others	1,000Rs.		172,100		178,500	and the state of t	185,500		185,600
Construction Cost of Pump House	1.000Rs.	and the second second second second	29,640		29,640		31,590.		31,980
Construction Cost of Settling Basin	1.000Rs.		7,000		7,000		7,380		7,460
Construction Cost of Suction Pit	-								000
and Basement of Pump House	1,000Rs.		114,340		116,080		122,730		123,730
Concrete Work	1,000Rs		95,670		97,410		103,180		105,900
Earthwork	1,000Rs.		4,320		4,320		4,470		4,510
In-situ RC Piling	1,000Rs.		14,350		14,350	and the second s	15,080		05,251
Miscelaneous Works (5%)	1,000Rs.		\$5,790	The state of the s	55,810		57,120		58,320
Direct Cost	1,000Rs.		1,171,550		1,172,030		1,199,520		1,224,800
Indirect Cost (16 % of Direct Cost)	1,000Rs.		187,450		187,520		191,920		195,970
Contingencies (22 % of Direct Cost)	1,000Rs.		257,740		257,850		263,890		269,460
tion Cos	1,000Rs		1,616,740		1,617,400		1,655,330		1,690,230
Replacement Cost of Pump Equipments	1,000Rs.		946,240		935,640		941,440		971,120
Annual O & M Cost							1		60.
Electricity	1,000Rs.		170,899		168,874		169,918		1/1,13/
Maintenance for Pump Equipments	1,000Rs.		19,296		19,270		19,614		20,066
Total O & M Cost									
in the Early Stage of the Project Life (50%) 1,000Rs.	6) 1,000Rs.		860,56		94,072		94,766		200,56
in the Late Stage of the Project Life	1,000Rs.		190,195		188,144		189,532		191,203
Net Present Value	1.000Rs		3,503,500		3,483,400		3,534,000		3,606,900
Incit i worth value									

Table G.3.3.3. Comparative Study of Pumps and Pumping Station

- Abstract of Facilities and Cost Comparison in Case Less than 10% Water of Regulating Ponds is Available (1)

(9/9)

					Case		
	l	II.	I-2	I-3	4	II -la	
Pump Bore	mm	2,600	2,200	2,000	1,650	2,600	1,650
Capacity per Unit	m3/s	14.40	12.00	6006	00.9	15.00	90.9
	m3/min	864.0	720.0	540.0	360.0	0.006	360.0
Number of Units	nos	'0	9	000	12	4	~
Total Head of Pumps	E	22.7	22.7	22.7	22.7	22.7	22.7
Motor Power per Unit	kw	4,200	3,600	2,700	1,800	4,400	1,800
Size of Pump House (Width * Length)	m * m	70.0 * 32.5	72.0 * 32.5	80.0 * 32.5	96.0 * 32.5		72.0 * 32.5
Size of Settling Basin (Width * Length)	# H	69.0 * 100.0	71.0 * 100.0	79.0 * 100.0	95.0 * 100.0		71.0 * 100.0
Annual Total Water Requirement	MCM	1,307.7	1,307.7	1,307.7	1,307.7		1,307.7
Annual Total Water Supply by Pumps	MCM	1,511.7	1,459.8	1,449.5	1,386.2		1,372.2
Annual Total Excess Water	MCM	204.0	152.1	141.8	78.5		\$. 5.
Annual Total Operation Hours of Pumps	 #	29,160	33,792	44,736	64,176	22,656	8,136
Pump Equipments Cost	1,000Rs.	935,050	943,640	1,006,660	1,068,280;		937,800
Pumps	1,000Rs.	313,000	337,200	360,800	361,200		310,600
Motors	1,000Rs.	207,500	214,200	224,800	235,200		212,000
Valves	1,000Rs.	153,150	125,040	130,160	135,480		145,100
Pipes	1,000Rs.	100,000	102,000	112,000	130,000		102,000
Others	1,000Rs.	161,400	165,200	178,900	206,400		168,100
Construction Cost of Pump House	1,000Rs.	27,300	28,080	31,200	37,050		27,690
	1,000Rs.	6,540	969'9	7,310	8,530		9,690
Construction Cost of Suction Pit				_			-
and Basement of Pump House	1,000Rs.	104,940	108,660	118,840	142,730		108,660
Concrete Work	1,000Rs.	87,350	90,710	029,670	120,490		90,710
Earthwork	1,000Rs.	4,130	4,190	4,410	4,240		4,190
In-situ RC Piling	1,000Rs.	13,460	13,760	14,760	17,300		13,760
Misccelaneous Works (5%)	1,000Rs.	53,690	54,350	58,200	62,830		54,040
Direct Cost	1,000Rs.	1,127,520	1,141,420	1,222,210	1,319,420		1,134,880
Indirect Cost (16 % of Direct Cost)	1,000Rs.	180,400	182,630	195,550	211,110		181,580
Contingencies (22 % of Direct Cost)	1,000Rs.	248,050	251,110	268,890	290,270		249,670
Total Construction Cost	1,000Rs.	1,555,970	1,575,160	1,686,650	1,820,800		1,566,130
Replacement Cost of Pump Equipments	1,000Rs.	929,640	933,490	052,786	1,009,990		921,430
Annual O & M Cost		,,,, <u>,</u>					
Electricity	1,000Rs.	187,589	186,684	185,432	177,790		175,880
Maintenance for Pump Equipments	1,000Rs.	18,701	18,873	20,133	21,366		18,756
Total O & M Cost							
in the Early Stage of the Project Life (50%	6) 1,000Rs	103,145	102,779	102,783	875,66		97,318
in the Late Stage of the Project Life	1,000Rs.	206,290	205,557	205,565	199,156		194,636
Net Present Value	1,000Rs.	3,505,700	3,525,100	3,690,900	3,815,600		3,450,000

Table G.3.3.3. Comparative Study of Pumps and Pumping Station

- Abstract of Facilities and Cost Comparison in Case Less than 10% Water of Regulating Ponds is Available (2)

				Case					
		п -2		П -3а		48- ∏		П -3с	
Pump Bore	un.	2,200	1,650	2,000	1,650	2,000	1,350	2,000	1,650
Capacity per Unit	m3/s	12.00	90.9	10.00	90.9	10.00	4.00	10.00	5.50
	m3/min	720.0	360.0	600.0	360.0	0.009	240.0	0.009	330.0
Number of Units	soa	5	61	9	2	9	m	S	4
Total Head of Pumps	Ħ	22.7	22.7	22.7	22.7	22.7	22.7	7.22	22.7
Motor Power per Unit	kw	3,600	1,800	3,000	1,800	3,000	1,200	3,000	1,700
Size of Pump House (Width * Length)	m.m		76.0 * 32.5		76.0 * 32.5		81.0 * 32.5		82.0 * 32.5
Size of Settling Basin (Width * Length)	m * m		75.0 * 100.0		75.0 * 100.0		80.0 * 100.0		81.0 * 100.0
Annual Total Water Requirement	MCM		1,307.7		1,307.7		1,307.7		1,307.7
Annual Total Water Supply by Pumps	MCM		1,386.2		1,345.9		1,351.5		1,348.1
Annual Total Excess Water	MCM		78.5		38.3		43.8		40.5
Annual Total Operation Hours of Pumps	且	30,144	3,888	32,592	7,992	31,656	14,712	30,624	12,408
Pump Equipments Cost	1,000Rs.		964,780		963,500		980,700		1,003,310
Pumps	1,000Rs		341,200		330,800		332,400		345,900
Motors	1,000Rs.		217,700		227,000		230,700		231,300
Valves	1,000Rs.		126,780		120,200		119,100		126,510
Pipes	1,000Rs.	,	107,000		107,000		113,000		114,000
Others	1,000Rs.	-	172,100		178,500		185,500		185,600
Construction Cost of Pump House	1,000Rs.		29,640		29,640		31,590		31,980
Construction Cost of Settling Basin	1,000Rs.		7,000		7,000		7,380	The same of the sa	7,460
Construction Cost of Suction Pit	A CONTRACTOR OF THE CONTRACTOR		THE RESERVE OF LITTLE STATE ST	made & co. of females of manufacture & Williams & Williams	To the state of th	added francisco codd a fill community of months among allows community of the community of	The state of the s	on a delectronistis formation of Marketine and Marketine a	dentité : la attache : c : celle : cotons corrers ha
and Basement of Pump House	1,000Rs.		114,340		. 116,080		122,730		123,730
Concrete Work	1,000Rs.		95,670		97,410		103,180		103,990
Earthwork	1,000Rs.		4,320		4,320		4,470		4,510
	1,000Rs.		14,350		14,350		15,080		15,230
Miscelaneous Works (5%)	1,000Rs.	-	55,790;		55,810		57,120	A STATE OF THE PERSON AND THE PERSON	58,320
Direct Cost	1,000Rs.	i	1,171,550		1,172,030		1,199,520		1,224,800
Indirect Cost (16 % of Direct Cost)	1,000Rs.		187,450		187,520		191,920		195,970
Contingencies (22 % of Direct Cost)	1,000Rs.		257,740		257,850		263,890		269,460
Total Construction Cost	1,000Rs.		1,616,740		1,617,400		1,655,330		1,690,230
Replacement Cost of Pump Equipments	1,000Rs.		946,240		935,640		941,440		971,120
Amual O & M Cost									
Electricity	1,000Rs.		177,790		172,925		173,593		174,186
Maintenance for Pump Equipments	1,000Rs.		19,296		19,270		19,614		20,066
Total O & M Cost							/ 1 Tu u 4 m		
in the Early Stage of the Project Life (50%) 1,000Rs.	%) 1,000Rs.		98,543		860'96		96,604		97,126
in the Late Stage of the Project Life	1,000Rs.		197,086		192,195		193,207		194,252
Net Present Value	1,000Rs.		3,537,600		3,503,400		3,552,200		3,621,900

Table G.3.3.4.	Annual Operation Hours of Each Pump	of Each Pump	
	Feb. Mar.	1 May Jun Jul. Aug. Sep. Oct. Nov. Dec.	Annual Operation Hours
No of Days	- e - =	10 10 11 11 10 10 11 11 10 10 11 11 10 10	
NO. Of Days	Operation Pattern in Ca	se Full (100%) Water of Regulating Ponds is Available	
1	* * * * *		
ΙÍ	* * * * * * * * * * * * * * * * * * * *		292 days × 24 hr 7,008 hr
m3/s Pump	***	# #	
10.00 m3/s Pump (4)	*	*	
m3/s Pump		***	41 days × 24 fu= 984 nr
	# # # # # # # # # # # # # # # # # # #	***	
6.00 m3/s Pump ①	*		hr=
			10tal 8,208 III
	▼ Primp Operation Pattern in Case 5	Pumo (meration Pattern in Case 50% Water of Regulating Ponds is Available	
10.00 m34 Dimm		* * * * * * * * * * * * * * * * * * * *	365 days
	* * * * * * *	* * * * * * * * * * * * * * * * * * * *	
m3/s Pump		* * * * * * * * * * * * * * * * * * * *	× 24 hr=
m3/s Pump	*	* * * * * *	
10.00 m3/s Pump ③	*	* *	93 days × 24 hr
10.00 m3/s Pump ®			
	9 9 9 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	***	
6.00 m3/s Pump (1)	• • • • • • • • • • • • • • • • • • •	*	90 days × 24 hr 2,160 hr
m3/s Pump			
			And the second s
		The Contract of the 10% Wester of Bernitative Donde is Assipable	
	V Pump Operation Fattern at Case	20 FCSS (11111 1 7 7 7 4 4 4 4 4 4 4 4 4 4 4 4 4	365 days × 24 hr = 8,760 hr
10.00 m3/s rump	* * * * * * * * * * * * * * * * * * * *	*	344 days × 24 hr= 8,256 hr
ma/s Primi	· ~~	* * * * * * * * * * * * * * * * * * * *	24 hr=
	*		4,584
	*	* * *	2,472
m3/s Pump			Total 32,592 hr
- 1/. P.	* * * * * * * * * * * * * * * * * * *	· · · · · · · · · · · · · · · · · · ·	hr= 6,048
6.00 m3/s Pump	A		1,944
dina recom			Total 7,992 lrr

Table G.3.3.5. Comparative Study of Pipeline

Case		A-1	A-2	A-3	B-1	B-2	B-3	C-1	C-2	C-3
Velocity of pipeline	s/m		3.5			3.0			2.5	
Series of pipeline	,		2	3		2	3	,	5	3
Diameter of pipes	mm	5,200	3,600	3,000	5,600	4,000	3,200	6,000	4,200	3,400
Unit weight of pipes	t/m	4.00	2.22	1.75	4.42	2.59	1.90	4.98	2.75	2.03
Total head of pumps	E	22.7	23.3	23.5	22.4	22.7	23.0	22.3	22.5	22.7
Motor power per unit (for large pump)	kw	3,000	3,100	3,100	3,000	3,000	3,000	3,000	3,000	3,000
(for small pump)	kw	1,800	1,900	1,900	1,800	1,800	1,800	1,800	1,800	1,800
Pump equipments cost	1,000Rs.	785,000	789,400	789,400	785,000	785,000	785,000	785,000	785,000	785,000
Pums	1,000Rs.	330,800	330,800	330,800	330,800	330,800	330,800	330,800	330,800	330,800
Motors	1,000Rs	227,000	231,400	231,400	227,000	227,000	227,000	227,000	227,000	227,000
Valves	1,000Rs.	120,200	120,200	120,200	120,200	120,200	120,200	120,200	120,200	120,200
Pipes	1,000Rs.	107,000	107,000	107,000	107,000	107,000	107,000	107,000	107,000	107,000
Pipeline cost	1,000Rs.	165,260	180,890	212,710	182,760	211,030	230,980	205,850	224,170	246,920
Material	1,000Rs	70,000	77,700	91,880	77,350	90,650	99,750	87,150	96,250	106,580
Manufacturing	1,000Rs.	61,600	68,380	80,850	68,070	79,770	87,780	76,690	84,700	93,790
Transportation	1,000Rs.	6,580	7,300	8,640	7,270	8,520	9,380	8,190	9,050	10,020
Installation	1,000Rs	19,740	21,910	25,910	21,810	25,560	28,130	24,580	27,140	30,060
Earthwork	1,000Rs.	7,340	5,600	5,430	8,260	6,530	5,940	9,240	7,030	6,470
Miscoelaneous Works (5%)	1,000Rs.	47,510	48,510	50,110	48,390	49,800	50,800	49,540	50,460	51,600
Direct cost	1,000Rs.	997,770	1,018,800	1,052,220	1,016,150	1,045,830	1,066,780	1,040,390	1,059,630	1,083,520
Indirect cost (16% of Direct Cost)	1,000Rs	159,640	163,010	168,360	162,580	167,330	170,680	166,460	169,540	173,360
	1,000Rs.	219,510	224,140	231,490	223,550	230,080	234,690	228,890	233,120	238,370
Total Construction Cost	1,000Rs.	1,376,920	1,405,950	1,452,070	1,402,280	1,443,240	1,472,150	1,435,740	1,462,290	1,495,250
Replacement Cost of Pump Equipments	1,000 Rs .	935,640	941,710	941,710	935,640	935,640	935,640	935,640	935,640	935,640
Annual O & M cost								,		
Electricity	1,000 Rs .	172,932	178,817	178,817	172,932	172,932	172,932	172,932	172,932	172,932
Maintenance for pump equipments	1,000Rs.	15,700	15,788	15,788	15,700	15,700	15,700	15,700	15,700	15,700
Total O & M Cost		,	1						710	212.00
in the Early Stage of the Project Life	1,000Rs.	94,316	97,303	97,303	94,316	94,316	94,316	94,316	94,316	94,316
in the Late Stage of the Project Life	1,000Rs.	188,632	194,605	194,605	188,632	188,632	188,632	188,632	188,632	188,632
Net Present Value	1,000Rs.	3,245,300	3,310,000	3,356,100	3,270,700	3,311,700	3,340,600	3,304,200	3,330,700	3,363,700

Table G.3.3.6. Diameter & Thickness of Delivery Pipes with Ribs in Each Case

$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	Case	Velocity of Pipeline	Diameter of Pipes	Height of Ribs	Distance between Ribs	Thickness of Pipes	Thickness of Ribs	Unit Weight of Pipes
3.5 $2 \times \phi 3,600$ 40.0 100.0 1.9 2.8 3.5 $2 \times \phi 3,600$ 25.0 100.0 1.9 2.1 $3 \times \phi 3,000$ 20.0 100.0 1.9 2.1 3.0 $2 \times \phi 4,000$ 40.0 100.0 1.9 2.2 $3 \times \phi 3,200$ 25.0 100.0 1.9 1.8 2.5 $2 \times \phi 4,200$ 40.0 100.0 1.9 3.4 2.5 $2 \times \phi 4,200$ 30.0 100.0 1.9 2.3 $3 \times \phi 3,400$ 25.0 100.0 1.9 1.9 1.9		(s/m)	D (mm)	ћ (сm)	(cm)	t (cm)	t ₁ (cm)	w (t/m)
3.5 $2 \times \phi 3,600$ 25.0 100.0 1.9 2.1 $3 \times \phi 3,000$ 20.0 100.0 1.9 2.1 3.0 $1 \times \phi 5,600$ 40.0 100.0 1.9 3.0 3.0 $2 \times \phi 4,000$ 30.0 100.0 1.9 1.8 3.0 $2 \times \phi 4,200$ 25.0 100.0 1.9 3.4 2.5 $2 \times \phi 4,200$ 30.0 100.0 1.9 2.3 3 $\times \phi 3,400$ 25.0 100.0 1.9 1.9 1.9	A-1		1 × 45,200	40.0	100.0	1.9	2.8	4.00
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	A-2	3.5	$2 \times \phi 3,600$	25.0	100.0	1.9	2.2	2.22
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	A-3		$3 \times \phi 3,000$	20.0	100.0	1.9	2.1	1.75
3.0 $2 \times \phi 4,000$ 30.0100.01.92.2 $3 \times \phi 3,200$ 25.0 100.01.91.8 $1 \times \phi 6,000$ 40.0 100.01.93.42.5 $2 \times \phi 4,200$ 30.0100.01.92.3 $3 \times \phi 3,400$ 25.0100.01.91.9	B-1		×	40.0	100.0	1.9	3.0	4.42
$3 \times \phi 3,200$ 25.0 100.0 1.9 1.8 $1 \times \phi 6,000$ 40.0 100.0 1.9 3.4 2.5 $2 \times \phi 4,200$ 30.0 100.0 1.9 2.3 $3 \times \phi 3,400$ 25.0 100.0 1.9 1.9	B -2	3.0	×	30.0	100.0	1.9	2.2	2.59
$1 \times \phi 6,000 \qquad 40.0 \qquad 100.0 \qquad 1.9 \qquad 3.4$ $2.5 \qquad 2 \times \phi 4,200 \qquad 30.0 \qquad 100.0 \qquad 1.9 \qquad 2.3$ $3 \times \phi 3,400 \qquad 25.0 \qquad 100.0 \qquad 1.9 \qquad 1.9$	B-3		×	25.0	100.0	1.9	89.1	1.90
2.5 $2 \times \phi 4,200$ 30.0 100.0 1.9 2.3 3 $\times \phi 3,400$ 25.0 100.0 1.9 1.9	5		×	40.0	100.0	1.9	3.4	4.98
$3 \times \phi 3,400$ 25.0 100.0 1.9	C-2	2.5	×	30.0	100.0	1.9	2.3	2.75
	C-3		$3 \times \phi 3,400$	25.0	100.0	1.9	1.9	2.03

Table G.4.1.1 Details of Main Canal Alignment

•	TP-NO.	Inter Angle	Radius	Curve Length	STA	A of B.C.	STA	A of E.C.
		-	(m)	(m)				
1	TP- 0		-	-	STA	0+000.00	STA	0+000.00
2	TP- 0'	3° 04′ 49″ L	1,000	53.76	STA	0+974.97	STA	1 + 28.73
3	TP- 1	43° 29′ 29″ L	1,000	759.07	STA	1+899.04	STA	2+658.11
4	TP- 2	54° 05′ 39″ R	500	472.06	STA	3+259.98	STA	3+732.04
5	TP- 3	22° 11′ 58″ L	1,000	387.45	STA	5+117.78	STA	5+505.24
6	TP- 4	41° 41′ 00″ R	800	582.01	STA	8+914.91	STA	9+496.92
7	TP- 5	19° 33′ 32″ L	1,000	341.37	STA	13+932.07	STA	14+273.44
8	TP- 6	60° 44′ 24″ L	500	530.06	STA	17+779.01	STA	18+309.07
9	TP- 7	00° 37′ 38″ R	1,000	10.95	STA	24+384.96	STA	24+395.91
10	TP-8	14° 31′ 36″ L	1,000	253.54	STA	28+469.45	STA	28+722.98
11	TP- 9	39° 22′ 57″ R	500	343.68	STA	33+165.61	STA	33+509.28
12	TP-10	57° 23′ 28″ L	1,000	1001.66	STA	35+964.17	STA	36+965.83
13	TP-11	16° 10′ 42″ R	1,000	282.36	STA	45+746.86	STA	46+ 29.23
14	TP-12	37° 20′ 46″ R	500	325.91	STA	50+424.82	STA	50+750.73
15	TP-13	36° 16′ 40″ L	1,000	633.17	STA	53+840.07	STA	54+473.23
16	TP-14	12° 10′ 41″ L	800	170.04	STA	66+633.04	STA	66+803.08
17	TP-15	36° 07′ 18″ L	1,000	630.44	STA	68+414.05	STA	69+ 44.49
18	TP-16	15° 46′ 00″ R	1,000	275.18	STA	74+257.35	STA	74+532.52
19	TP-17	27° 51′ 53″ R	1,000	486.33	STA	80+921.08	STA	81+407.40
20	TP-18	32° 07′ 37″ L	1,000	560.72	STA	84+777.58	STA	85+338.29
21	TP-19	35° 13′ 18″ L	1,000	614.73	STA	87+893.03	STA	88+507.76
22	TP-20	49°01′49″R	1,000	855.74	STA	89+ 82.68	STA	89+938.43
23	TP-21	31° 51′ 56″ L	1,000	556.16	STA	93+279.47	STA	93+835.64
24	TP-22	56° 49′ 00″ L	500	495.82	STA	94+784.18	STA	95+280.00
25	TP-23	79° 42′ 03″ R	1,000	1391.04	STA	96+856.90	STA	98+247.94
26	TP-24	22° 27′ 15″ L	1,000	391.90	STA	106+148.80	STA	106+540.76
27	TP-25	26° 32′ 05″ R	1,000	463.12	STA	111+559.59	STA	112+ 22.71
28	EP		-	-	STA	113+250.00	STA	113+250.00
	Total Leng	th				113.250 Km		

Table G.4.1.2 Location of TP and BM in Main Canal

<u></u>	Easting	Northing	Elevation
	X	Y	(m)
TP-0	3020135.00	876631.11	205.850
TP-0'	3019137.66	876726.19	208.600
TP-1	3017842.69	876779.68	208.430
TP-2	3016896.58	875953.57	207.710
TP-3	3015106.23	876365.86	208.470
TP-4	3011246.45	875738.55	207.200
TP-5	3007064.24	878398.01	208.600
ВМ	3004614.87	878957.34	210.548
TP-6	3003193.43	879284.07	208.356
ВМ	3000796.71	876548.15	205.150
BM	2999553.61	875262.53	206.545
TP-7	2998914.32	874557.14	206.881
BM	2997239.03	872579.18	207.956
TP-8	2996057.75	871470.24	206.916
ВМ	2995141.07	869806.45	205.946
BM	2994004.44	867631.88	203.435
TP-9	2993810.16	867287.98	203.093
TP-10	2990868.18	866077.53	205.992
BM	2990259.78	864404.81	207.494
BM	2989538.22	858719.94	206.632
TP-11	2989183.69	856758.00	205.152
BM	2988536.69	855432.50	203.424
BM	2987515.86	853603.09	204.566
TP-12	2987089.37	852543.41	203.107
TP-13	2983872.71	850958.51	206.740
BM	2982634.34	848596.74	210.670
BM	2981404.58	846216.02	205.260
TP-14	2978068.81	839805.47	203.530
BM	2977694.47	838439.19	206.852
TP-15	2977534.68	837854.87	207.745
TP-16	2979551.76	832547.05	204.346
BM	2979648.70	831701.77	204.300
BM	2979789.64	829789.05	204.392
BM	2979951.25	827932.55	203.953
TP-17	2980136.10	825797.21	200.115
BM	2979392.25	824037.76	200.322
TP-18	2978615.06	822199.35	200.846
BM	2978837.91	820824.77	199.989
TP-19	2979120.86	819079.94	200.104
TP-20	2980064.79	818117.10	200.356
TP-21	2979737.49	814047.67	200.803
TP-22	2980427.11	812709.67	201.376
TP-23	2983094.81	812433.23	197.839
TP-24	2983777.57	803525.33	197.942
TP-25	2986239.44	798659.49	194.428
EP	2986247.07	797213.31	195.500

Table G.4.1.3 Comparison of Station Between Survey and Design

TP. No.	BC/EC	STA. of Survey	STA. of Design
0'	EC-0	0+515.10	1+028.74
1	BC-1	1+383.41	1+899.05
	EC-1	2+183.15	2+658.12
2	BC-2	2+785.02	3+259.98
	EC-2	3+295.58	3+732.05
3	BC-3	4+681.32	5+117.78
	EC-3	5+073.70	5+505.24
4	BC-4	8+483.37	8+914.92
	EC-4	9+092.49	9+496.93
5	BC-5	13+527.64	13+932.07
	EC-5	13+808.49	14+273.44
6	BC-6	17+312.33	17+779.01
	EC-6	17+898.33	18+309.07
7	BC-7	23+975.95	24+386.69
	EC-7	23+986.89	24+397.64
8	BC-8	28+060.43	28+470.54
	EC-8	28+315.33	28+724.08
9	BC-9	32+757.96	33+165.63
	·EC-9	33+131.43	33+509.30
10	BC-10	35+586.33	35+964.25
	EC-10	36+681.07	36+965.90
11	BC-11	45+462.11	45+746.95
	EC-11	45+503.13	46+029.31
12	BC-12	49+898.72	50+424.47
	EC-12	50+236.68	50+750.38
13	BC-13	53+326.02	53+839.71
	EC-13	53+981.22	54+472.88
14	BC-14	66+141.03	66+632.75
	EC-14	66+311.71	66+802.79
15	BC-15	67+922.68	68+413.76
	EC-15	68+574.86	69+044.20
16	BC-16	73+786.88	74+256.98
	EC-16	74+065.48	74+533.79
17	BC-17	80+453.19	80+921.50
	EC-17	80+949.35	81+407.84
18	BC-18	84+319.50	84+777.99
	EC-18	84+895.42	85+338.74
19	BC-19	87+450.15	87+893.52
	EC-19	88+084.99	88+508.24
20	BC-20	88+659.93	89+083.14
	EC-20	89+572.01	89+938.87
21	BC-21	92+913.07	93+279.92
'	EC-21	93+484.01	93+836.08
22	BC-22	94+432.56	94+785.43
	EC-22	94+973.44	95+281.25
23	BC-23	96+550.34	96+858.13
	EC-23	98+219.66	98+249.17
24	BC-24	106+120.52	106+150.04
	EC-24	106+517.52	106+541.94
25	BC-25	111+536.41	111+560.83
	EC-25	112+007.99	112+023.95

Table G.4.2.1 Design Capacity and Section of Main Canal

STA.	Distance	Escape		Distributary		Seepage		Main Ca		
		No.	No.	C.C.A	Discharge	Loss		Discharge	Section	Slope
0+000	(m)		·····	(ha)	(m3)	(m3)	(ha)	(m3)	No.	(1:)
0.000	1,850					0.19	115,600	71.18	1	14,00
1+850	C 000		D - 1	1,700	1.72	0.26	112 000	69.27	1	14,00
7+850	6,000		D - 2	2,430	2.27	0.26	113,900	09.27	1	14,00
	5,350			•		0.19	111,470	66.74	1	14,000
13+200	6,350		D - 3	2,390	1.76	0.08	109,080	64.79	1	14,000
19+550	0,550		D - 4	1,260	0.66	0.50		4,		.,,
24.500	4,950		D - 5	9,280	4.87	0.54	107,820	64.05	i	14,000
24+500	5,897		נ - ע	7,200	4.07	0.59	98,540	58.64	1	14,00
30+397			D-6	10,150	5.33		99.200	60.70	,	14.00
32+800	2,403 E -	1					88,390	52.72	1	14,00
34+700	1,900		D - 7	2,410	1.26	0.14	88,390	52.72	2	14,00
J	6,050					0.42	85,980	51.32	2	14,00
40+750	4,700		D - 8	7,060	3.70	0.66	78,920	47.20	2	14,00
45+450	4,700		D-9	11,400	5.98	0.00	70,720	17.20		
47.750	2,300		D - 10	2 200	1.25	0.14	67,520	40.56	2	14,00
47+750	2,350		Ð - 10	2,380	1,23	0.15	65,140	39.17	2	14,00
50+100			D - 11	2,660	1.40	0.70	60 400	27.62	0	1400
53+400	3,300		D - 12	13,620	7.15	0.79	62,480	37.62	2	14,00
	3,150						48,860	29.68	2	14,00
56+550	Е	- 2			·			<u> </u>		
	1,550					0.37	48,860	29.68	3	9,00
58+100	5,100		D - 13	6,240	3.27	0.22	42,620	26.04	3	9,00
63+200	5,100		D - 14	3,630	1.90					
70+200	7,000		D - 15	1,860	0.98	0.10	38,990	23.92	3	9,00
701200	3,650		D-13	1,000	0.70	0.35	37,130	22.84	3	9,00
73+850	1.160		D - 16	5,910	3.10		21 220	10.20	2	0.00
75±000	1,150 E	- 3					31,220	19.39	3	9,00
	4.50					0.52	21.000	10.20		7.00
78+150	3,150		D - 17	9,150	4.80	0.53	31,220	19.39	4	7,00
	7,400					0.15	22,070	14.06	4	7,00
85+550	4,550		D - 18	2,560	1.34	0.16	19,510	12.57	4	7,00
90+100			D - 19	2,560	1.34					
93+150	3,050		D - 20	2,440	1.28	0.14	16,950	11.07	4	7,00
734130	1,150		D = 20	2,440	1.20	•	14,510	9.65	4	7,00
94+300	E	- 4	 							
	1,200					0.21	14,510	9.65	5	4,00
95+500			D - 21	3,650	1.92					
102+900	7,400		D - 22	3,250	1.95	0.21	10,860	7.52	5	4,0
	5,550				•	0.07	7,610	5.36	5	4,0
108+450	3,750		D - 23	1,180	0.62	0.19	6,430	4.67	5	4,0
112+200			D - 24	2,950	1.67	0.17				
112.250	1,050	E - 5					3,480	2.81	5	4,0
113+250		3 - 3	·····			 	•			
				المعارضين		0.28	3,480	2.81		
			D - 25	3.48	2.53					

Table G.4.3.1 Section Properties of Main Canal

Section No.	Design Discharge	Roughness Coefficient	Slope (1:)	Area	Wetted Perimeter	Water Depth	Base Width	Velocity	Freeboard
	(m3/s)			(m2)	(m)	(m)	(m)	(m/s)	(m)
1	72	0.016	14,000	70.57	26.21	3.94	12.00	1.02	1.21
2	53	0.016	14,000	55.76	23.23	3.53	10.50	0.95	1.22
3	30	0.016	9,000	30.94	17.37	2.60	8.00	0.97	1.20
4	20	0.016	7,000	20.78	14.22	2.14	6.50	0.96	1.06
5	10	0.016	4,000	10.04	9.87	1.49	4.50	1.00	0.91

Note: Canal Side Slope 1:1.5

Table G.4.4.1 Head Loss and F.S.L. of Main Canal (1/3)

			20.0	7	V Designation	Ļ	Distv	Rridge	Ferane	Head Loss	Loss	Full Supply Level	oly Level	Bed Level	evel
SIA.	Discance	Discance Imain Canal		A-D10	Tries	1 / Z	O(m3/c)	- A	Z	Frict (m)	Str (m)	(m) S/[]	D/S (m)	U/S (m)	D/S (m)
	(u)	Q(m3/s)		.ov	13/25	2	K(III)))	24	7	(111)) 1				
(Section 1/Slope=1:14,000)	:lope=1:14,((00)											0000		30.500
000+0	0	72								,	(0	203.00	0	203.00
1+850	1,850	72				ᅼ	1.72			0.13	3	708.87	208.87	204.93	204.93
3+350	1,500	72						VR-1		0.11	0.03	208.76	208.73	204.82	204.79
3+750		72						VR-2		0.03	0.03	208.70	208.67	204.76	204.73
7+850	4	72				D-2	2.27			0.29	0	208.38	208.38	204.44	204.44
7+888		72	CR-1							0	0.02	208.38		204.44	204.42
10+100	2,	72		CD-1	SP					0.16	0.02	208.20		204.26	204.24
13+200		72				D-3	1.76			0.22	0	207.96		204.02	204.02
13+800		7.						VR-3		0.05	0.03	207.91	208.88	203.97	203.94
15+165		7.2						VR-4		0.09	0.03	207.79		203.85	203.82
16+600		72						AR-1		0.11	0.05	207.65	207.60	203.71	203.66
19+550	·	72				Д 4	99.0			0.21	0	207.39	207.39	203.45	203.45
19+588		72	CR-2							0	0.02	207.39		203.45	203.43
21+700	C		! !	CD-2	U					0.15	0	207.22	207.22	203.28	203.28
23+450								VR-5		0.13	0.03	207.09	207.06	203.15	203.12
24+500						D-5	4.87			0.07	0	206.99	206.99	203.05	203.05
24+538		- 22	CR-3							0	0.02	206.99	206.97	203.05	203.03
30+307	۸.		; ;			9-D	5.33			0.42	0	207.55	207.55	203.61	202.61
32±800			CR-4					,	E-1	0.17	0.02	206.38	206.36	202.44	202.42
	(1)						16.61			2.34	0.30				
	,	í													
(Section 2/Slope=1:14,000)	slope=1:14,((6	¢	30 700		00000	200 69
33+000				CD-3	ပ					0.01		200.33		202.92	202.02
34+700	1,700					D-7	1.26			0.13	3	206.22	200.02	202.69	202.09
38+100	3,400	53		CD4	SP					0.24	0.02	202.98	202.36	202.45	202.43
38+465	365	53						DR-1		0.05	0.04	205.94		202.41	202.37
40+750	7	53				P-8	3.7			0.17	0	205.73		202.20	202.20
40+788		53	CR-5							0	0.02	205.73		202.20	202.18
42+500	1.712	53		CD-5	SP					0.12	0.03	205.59		202.06	202.03
44+650	-	53						VR-6		0.16	0.03	205.40		201.87	
45+450		53				D-9	5.98			0.05	0	205.32		201.79	
3CV+CV	C			CD-6	Ö				-	0.15	0	205.17	205.17	201.64	201.64
1.1.1				^ *)										

Table G.4.4.1 Head Loss and F.S.L. of Main Canal (2/3)

STA	Discance	Discance Main Canal	Х-Rед	X-Dra		Disty	sty	Bridge	Escape	Head Loss	Loss	Full Supply Level	ly Level	Bed Level	evel
	(E)	O(m3/s)		ò	1	No.	O(m3/s)	Type	Š.	Frict (m)	Str (m)	U/S (m)	D/S (m)	U/S (m)	D/S (m)
47+750	275	53				D-10	1.25			0.02	0	205.15	205.15	201.62	201.62
47+788		53	CR-6							0	0.02	205.15	205.13	201.62	201.60
48+200	4	53						VR-7		0.03	0.03	205.10	205.07	201.57	205.54
49+620	_	53		CD-7	U					0.10	Ó	204.97	204.97	201.44	201.44
50+100		53				D-11	1.4			0.04	0	204.93	204.93	201.40	201.40
51+400	_	53		CD-8	ပ					60.0	0	204.84	204.84	201.31	201.31
51+600		53						VR-8		10.01	0.03	204.83	204.80	201.30	201.27
53-400	_	53				D-12	7.15			0.13	0	204.67	204.67	201.14	201.14
55+550		53		-				DR-2		0.16	0.04	204.51	204.47	200.98	200.94
56+550		53	CR-7						丏	0.07	0.02	204.40	204.38	200.87	200.85
	23,750						20.74			1.70	0.28				
3/2 20:4003/	(Cont. or 2/8/2007)	ć													
(C/C 1)(C)(C)(C)(C)(C)(C)(C)(C)(C)(C)(C)(C)(C)	150	% ()		0°-0	ďS					0.05	0.0	204.36	204.32	201.76	201.72
58+100		? Ç))	;	D-13	3.27			0.15	0	204.17	204.17	201.57	201.57
59+400		300		DC-10	SP					0.15	0.03	204.02	203.99	201.42	201.39
63+200		30				D-14	1.9			0.42	0	203.57	203.57	200.97	200.97
63+238		30	CR-8							0	0.02	203.57	203.55	200.97	200.95
65+710	2,	30						DR-3		0.28	0.03	203.27	203.24	200.67	200.64
66+950		30		DC-11	SP					0.14	0.03	203.10	203.07	200.50	200.47
68+100		30		DC-12	SP					0.12	0.05	202.95	202.90	200.35	200.30
68+730		30						DR-4		0.07	0.03	202.83	202.80	200.23	200.20
70+200	1,470	30				D-15	0.98			0.17	0	202.63	203.63	200.03	200.03
70+238		30	CR-9							0	0.05	202.63	202.61	200.03	200.01
73+850	3	39				D-16	3.1			0.40	0	202.21	202.21	199.61	199.61
74+850		30						VR-9		0.11	0.03	202.10	202.07	199.50	199.47
75+000		30	CR-10						E-3	0.05	0.02	202.05	202.03	199.45	199.43
	18						9.25			2.05	0:30				
(Section 4/S	(Section 4/Slope=1:7,000)	(6)											3		6
75+200		20		CD-13	SP			;		0.03	0.06	202.00	201.94	199.86	199.80
77+000		50				ţ	0	VR-10		0.26	0.03	201.68	201.65	45.991 45.001	100 35
78+150	1,150	20				À	8.4			0.10		45,102	44.107	22.22	55.55

Table G.4.4.1 Head Loss and F.S.L. of Main Canal (3/3)

ST.	Disconce	Discource Main Canal	X-Reg	X-Drainage	nage	Disty	Ą	Bridge	Escape	Head Loss	Loss	Full Supply Level	ly Level	Bed Level	evel
Ç	(m)	O(m3/s)		No.	Type	No.	Q(m3/s)	Type	No.	Frict (m)	Str (m)	U/S (m)	D/S (m)	U/S (m)	D/S (m)
78+188			CR-11							0.01	0.02	201.48	201.46	199.34	199.32
00T+27	œ.							VR-11		0.54	0.03	200.92	200.89	198.78	198.75
82+350		20 70		CD-14	U					0.05	0	200.84	200.84	198.70	198.70
85+550	"					D-18	1.34			0.46	0	200.38	200.38	198.24	198.24
85+588			CR-12							0	0.02	200.38	200.36	198.24	198.22
88+600	3.	20		CD-15	ပ					0.43	0	199.93	199.93	197.79	197.79
90+100		20				C-19	1.34			0.22	0	199.71	199.71	197.57	197.57
90+138			CR-13							0	0.02	199.71	199.69	197.57	197.55
90+500	\·	- 50		CD-16	ပ					0.05	0	199.64	199.64	197.50	197.50
93+150	2					C-20	1.28			0.38	0	199.26	199.26	197.12	197.12
94+300			CR-14						E-4	0.17	0.02	199.09	199.07	196.95	196.93
	1						8.76			2.76	0.20				
(Section 5/S	(Section 5/Slope=1:4.000)	(0)													
04+550	250	10		CD-17	SP					90.0	0.03	199.01	199.98	197.52	197.49
95+500						D-21	1.92			0.24	0	198.74	198.74	197.25	197.25
95+532			CR-15						-	0.01	0.02	198.73	198.71	197.24	197.22
100+900	5.3							VR-12		1.32	0	197.37	197.37	195.88	195.88
102+900						D- 22	1.95			0.50	0	196.87	196.87	195.38	195.38
102+932		10	CR-16							0.01	0.02	196.86	196.84	195.37	195.35
105+620	2.0	10		CD-18	ပ					0.67	0	196.17	196.17	194.68	194.68
108+450						D-23	0.62			0.71	0	195.46	195.46	193.97	193.97
108+482			CR-17							0.01	0.02	195.45	195.43	193.96	193.94
110+100	1,0			CD-19	ပ					0.40	0	195.03	195.03	193.54	193.54
112+200						D-24	1.67			0.53	0	194.50	194.50	193.01	193.01
113+250			CR-18		-				E-5	0.26	0.02	194.24	194.22	192.75	192.73
							6.16			4.72	0.11				
	-					25	2.53								

Table G.4.5.1 Location and Gate Size of Cross Regulator in Main Canal

			Canal		Sate of Cros		
CR-No.	STA.	Discharge (m3/s)	Section No.	Туре	Span (m)	Height (m)	Nos.
CR-1	7+888						
CR-2	19+588	72		Roller Gate With	. 8	4.1	2
CR-3	24+538		1	Counter Weight	_	4.1	2
CR-4	32+800						
CR-5	40+788	***************************************		h.11. C		######################################	
CR-6	47+788	53	2	Roller Gate With Counter Weight	7	3.7	2
CR-7	56+550						
CR-8	63+238			Roller Gate			
CR-9	70+238	30	3	With Counter Weight	6	2.8	2
CR-10	75+000				_		
CR-11	78+188						
CR-12	85+588	20	. 4	Roller Gate With	5	2.3	2
CR-13	90+138	20	7	Counter Weigh		2.3	-
CR-14	94+300						
CR-15	95+532						
CR-16	102+932	10	. 5	Roller Gate With	6	1.7	1
CR-17	108+482	10	, ,	Counter Weigh		1./	
CR-18	113+250						

(Note) CR: Cross Regulator

Table G.4.5.2 Location and Discharge of Cross Drainage in Main Canal

		Main	Canal		Cross Drainag	e
CD-No.	STA.	Discharge (m3/s)	Section No.	Туре	Nala	Discharge (m3/s)
CD-1	10+100	72	. 1	SP	Paniala	306
CD-2	21+700	72	. 1	C	Nose	1048
CD-3	33+000	53	2	С	Budh	722
CD-4	38+100	53	2	SP	Takawala	142
CD-5	42+500	53	2	SP	Gumal	558
CD-6	47+475	53	2	C	Shahid	198
CD-7	49+620	53	2	C	Rada	71
CD-8	51+400	53	2	C	Bhuar	156
CD-9	56+700	30	3	SP	Luni	912
CD-10	59+400	30	3	SP	Swan	538
CD-11	66+950	30	3	SP	Rod Kohi	487
CD-12	68+100	30	3	SP	Toe	977
CD-13	75+200	20	4	SP	Mochiwal	1243
CD-14	82+350	20	4	С	Ali Garah	82
CD-15	88+600	20	4	С	Bhcda	212
CD-16	90+500	20	- 4	С	Rod Kohi	161
CD-17	94+550	10	5	SP	Gajistan	295
CD-18	105+620	10	5	С	Sheranna	411
CD-19	110+100	10	5	С	Ramak Div.	. 49

(Note) CD: Cross Drainage

SP: Culvert Super Passage

C: Nala Culvert

Table G.4.5.3 Location and Discharge of Escape in Main Canal

		Main	Canal	Esca	oe Structure
E-No.	STA.	Discharge (m3/s)	SectionNo.	Discharge (m3/s)	Connecting Nala
E-1	32+800	72	1	72	Budh
E-2	56+550	53	2	53	Luni
E-3	75+000	30	3	30	Mochiwal
E-4	94+300	20	4	20	Gajistan
E-5	113+250	10	5	10	Ramak

(Note) E: Escape Structure

Table G.4.5.4 Section and Head Loss of Head Regulator

		HEAD	HEAD REGULATOR	TOR		Main (Janai		H	Head Loss				Distributary	utary		
	No. of	Unit	Width	Height	Bed	F.S.L. C.W.J	C.W.L.	Length	Frict.	Exit/En Transsi	Franssi	Total	F.S.L.	Bed	Water	Slope	N.S.L.
	Barrels	Discharge	ф	Н	Level									Level	Depth	(1:)	
		(m3/s)	(m)	(E)	(m)	(m)	(m)	(m)	(m)	(m)	(m)	(m)	(m)	(m)	(E)		(m)
												:					
D-1 (O=1.72 m3/s)	-	1.72	1.35	0.00	207.10	208.87	208.38	15	0.05	0.15	0.01	0.21	208.17	207.28	0.89	2,000	208.10
D-2 (O=2.27 m3/s)	61	1.14	1.35	06.0	207.3	208.38	208.38	15	0.05	0.07	0.00	0.0	208.29	207.25	1.04	2,000	208.50
D-3 (0= 1.76 m3/s)	-	1.76	1.35	0.90	206.10	207.96	207.39	15	0.05	91.0	0.01	0.22	207.17	206.28	0.89	2,000	208.10
D-4 (0= 0.66 m3/s)	_	0.66	1.35	0.90	206.40	207.39	207.39	15	0.01	0.05	0.01	0.04	207.35	206.74	0.61	1,000	205.25
D-5 (0= 4.87 m3/s)	4	1.22	1.35	0.90	205.90	206.99	206.99	15	0.02	0.08	0.00	0.10	206.89	205.53	1.36	3,000	206.60
D-6 (Q= 5.33 m3/s)	4	1.33	1.35	0.00	205.30	206.55	206.38	15	0.03	60.0	0.00	0.12	206.26	204.76	1.50	3,500	205.60
D-7 (Q= 1.26 m3/s)	1	1.26	1.35	0.90	204.60	206.22	205.73	15	0.05	0.08	0.00	0.10	205.63	204.87	0.76	1,500	204.00
D-8 (Q= 3.70 m3/s)	3	1.23	1.35	0.90	204.60	205.73	205.73	15	0.02	0.08	0.00	0.10	205.63	204.43	1.20	2,500	207.00
D-9 (Q= 5.98 m3/s)	4	1.50	1.35	0.90	204.00	205.32	205.15	15	0.03	0.12	0.00	0.15	205.00	203.50	1.50	3,500	205.65
D-10 (Q= 1.25 m3/s)	_	1.25	1.35	06.0	204.10	205.15	205.15	15	0.02	80.0	0.01	0.11	205.04	204.28	0.76	1,500	203.00
D-11 (Q= 1.40 m3/s)		1.40	1.35	0.90	203.30	204.93	204.40	15	0.03	0.10	0.01	0.14	204.26	203.50	0.76	1,500	203.45
D-12 (Q= 7.15 m3/s)	5	1.43	1.35	0.90	203.20	204.67	204.40	15	0.03	0.11	0.00	0.14	204.26	202.55	1.71	4,000	205.24
D-13 (Q= 3.27 m3/s)	2	2 .	1.35	0.90	202.40	204.17	203.57	15	0.04	0.14	0.00	0.18	203.39	202.19	1.20	2,500	207.30
D-14 (Q= 1.90 m3/s)	2	0.95	1.35	0.90	202.50	203.57	203.57	15	0.01	0.05	0.00	90.0	203.51	202.62	0.89	2,000	204.10
D-15 (Q= 0.98 m3/s)	_	0.98	1.35	0.90	201.60	202.63	202.63	15	0.01	0.05	0.01	0.07	202.56	201.95	0.61	1,000	203.90
D-16 (Q= 3.10 m3/s)	5	1.55	1.35	0.90	200.90	202.21	202.05	15	0.0	0.12	0.00	0.16	201.89	200.69	1.20	2,500	204.70
D-17 (Q= 4.80 m3/s)	3	1.60	1.35	0.90	200.30	201.49	201.48	15	0.0	0.13	0.00	0.17	201.31	199.95	1.36	3,000	203.60
D-18 (Q= 1.34 m3/s)	-	1.34	1.35	0.90	200.30	201.38	201.38	15	0.03	0.09	0.01	0.13	201.25	200.49	92.0	1,500	200.80
D-19 (Q= 1.34 m3/s)	1	1.34	1.35	0.90	198.60	199.71	199.71	22	0.03	60.0	0.01	0.13	199.58	198.82	0.76	1,500	200.50
D-20 (Q= 1.28 m3/s)	_	1.28	1.35	0.90	198.00	199.26	199.09	15	0.03	0.08	0.00	0.11	198.98	198.22	0.76	1,500	200.10
D-21 (Q= 1.92 m3/s)	2	0.96	1.35	0.90	197.70	198.74	198.73	15	0.01	0.05	0.00	90:0	198.67	197.78	68.0	2,000	199.80
D-22 (Q= 1.95 m3/s)	2	0.98	1.35	0.00	195.80	196.87	196.86	15	0.01	0.05	0.00	90.0	196.80	195.91	0.89	2,000	196.70
D-23 (Q= 0.62 m3/s)	-	0.62	1.35	0.90	194.50	195.46	195.45	15	0.01	0.02	0.01	0.04	195.41	194.88	0.53	90,	196.45
D-24 (Q= 1.67 m3/s)	1	1.67	1.35	0.00	193.00	194.50	194.24	15	0.04	0.14	0.01	0.19	194.05	193.16	0.89	2,000	194.60

Table G.4.5.5 Location and Type of Bridge in Main Canal

	_	Main	Canal	_	
No.	STA.	Discharge (m3/s)	Section No.	Road Type	Remarks
VR-1	3+350	72	1	Village Road	
VR-2	3+750	72	1	Village Road	
VR-3	13+800	72	1	Village Road	
VR-4	15+165	72	1	Village Road	٠
AR-1	16+600	72	1	Arterial Road	Bannu Road
VR-5	23+450	72	1	Village Road	
FP-1	28+670	72 :	1	Foot Path	
FP-2	35+450	53	2	Foot Path	
DR-1	38+665	53	2	District Road	Tank Road
VR-6	44+650	53	2	Village Road	
VR-7	48+200	53	2	Village Road	•
VR-8	51+600	53	2	Village Road	
DR-2	55+550	53	2	District Road	Kulachi Road
DR-3	65+610	30	3	District Road	Daraban Roa
DR-4	68+730	. 30	3	District Road	Daraban Roa
VR-9	74+850	30	3	Village Road	
VR-10	77+000	20	4	Village Road	
VR-11	82+000	20	4	Village Road	
FP-3	98+220	10	5	Foot Path	
VR-12	100+900	10	5	Village Road	
FP-4	105+710	10	5	Foot Path	

Table G.5.1.1 C.C.A and Discharge of Each Distributary

Disty	Name of	G.C.A.		C.C.A.		Discharge
No.	Disty	-	Good & Mod	Margi.	Total	
		(ha)	(ha)	(ha)	(ha)	(m3/s)
D- 1	SAKHI MARDAN	1,930	0	1,700	1,700	1.72
D- 2	UMAR KHAN WANDA	2,770	400	2,030	2,430	2.27
D- 3	YARIK (1)	2,740	1,350	1,040	2,390	1.76
D- 4	YARIK (2)	1,480	1,260	0	1,260	0.66
D- 5	RODI KHEL	10,470	9,280	0	9,280	4.87
D- 6	REHMAN DHERI	11,470	10,150	0	10,150	5.33
D- 7	BUDH	2,720	2,410	0	2,410	1.26
D-8	KOT ISA KHAN	8,090	7,060	0	7,060	3.70
D- 9	POTAH	13,430	11,400	0	11,400	5.98
D-10	SHAHID	3,130	2,380	0	2,380	1.25
D-11	SIKANDAR	3,120	2,660	0	2,660	1.40
D-12	MADDI	15,510	13,620	0	13,620	7.15
D-13	KOT ZAFAR	7,500	6,240	0	6,240	3.27
D-14	SWAN	4,470	3,630	0	3,630	1.90
D-15	GANDI ASHIQ	2,300	1,860	0	1,860	0.98
D-16	MOCHIWAL	6,900	5,910	0	5,910	3.10
D-17	GARAH ISA KHAN	11,000	9,150	0	9,150	4.80
D-18	ALI GARAH	3,010	2,560	0	2,560	1.34
D-19	BABRAN	3,060	2,560	0	2,560	1.34
D-20	GAJISTAN	2,800	2,440	0	2,440	1.28
D-21	KAURI HOT	4,110	3,650	0	3,650	1.92
D-22	SHAH GHARBI	3,760	2,750	500	3,250	1.95
D-23	SHERANNA	1,440	1,180	0	1,180	0.62
D-24	CHIRRI BHUHAR	3,370	2,700	250	2,950	1.67
D-25	JHANGI	4,020		1,440	3,480	2.53

(Note) C.C.A.(1): Good & Moderate irrigable land (Ordinary Land)

C.C.A.(2): Marginal irrigable land (Sandy Land)

Table G.5.2.1 Sump Well Area of Each Distributary

						(Unit:ha)
Disty	Gross	Cultivable	Sump We	ll Area	Gravity	Агеа
No.	Irrigable	Command				
	Area	Area	G.I.A.	C.C.A.	G.I.A.	C.C.A.
•	(G.I.A.)	(C.C.A.)				
	(1)	(2)	(3)	(4)	(5)	(6)
	•				·	
D - 1	1,850	1,700	0	0	1,850	1,700
D - 2	2,640	2,430	0	0	2,640	2,430
D - 3	2,600	2,390	50	50	2,550	2,340
D - 4	1,370	1,260	- 0 -	. 0	1,370	1,260
D - 5	10,090	9,280	0	0	10,090	9,280
D - 6	11,030	10,150	150	140	10,880	10,010
D - 7	2,620	2,410	130	120	2,490	2,290
D - 8	7,680	7,060	270	250	7,410	6,810
D - 9	12,390	11,400	0	0	12,390	11,400
D - 10	2,590	2,380	0	0	2,590	2,380
D - 11	2,900	2,660	0	0	2,900	2,660
D - 12	14,800	13,620	630	580	14,170	13,040
D - 13	6,780	6,240	900	830	5,880	5,410
D - 14	3,950	3,630	280	260	3,670	3,370
D - 15	2,020	1,860	390	360	1,630	1,500
D - 16	6,430	5,910	280	260	6,150	5,650
D - 17	9,950	9,150	360	330	9,590	8,820
D - 18	2,790	2,560	0	0	2,790	2,560
D - 19	2,780	2,560	0	0	2,780	2,560
D - 20	2,650	2,440	50	50	2,600	2,390
D - 21	3,970	3,650	200	180	3,770	3,470
D - 22	3,530	3,250	0	0	3,530	3,250
D - 23	1,290	1,180	200	180	1,090	1,000
D - 24	3,210	2,950	80	70	3,130	2,880
D - 25	3,790	3,480	0	0	3,790	3,480
Total	125,700	115,600	3,970	3,660	121,730	111,94(

(Note)

: (1)=(3)+(5)

: (2)=(4)+(6)=(1)*0.92

: (4)=(3)*0.92 : (6)=(5)*0.92

Table G.5.3.1 Standard Section Properties for Distributary (1/3)

Distributary	Design	Area	Section	on Properties - N	Manning n=0.0	16	Freeboard
Range	Ū	_	Wetted	Depth	Base	Velocity	
Q	Q		Perimeter		Width		
(m3/s)	(m3/s)	(m2)	(m)	(m)	(m)	(m/s)	(m)
0.00 - 0.25	0.250	0.29	1.57	0.31	0.45	0.91	0.49
0.25 - 0.50	0.500	0.48	2.02	0.41	0.55	1.07	0.49
0.50 - 0.75	0.750	0.65	2.36	0.47	0.65	1.18	0.53
0.75 - 1.00	1.000	0.80	2.62	0.53	0.70	1.27	0.52
1.00 - 1.50	1.500	1.08	3.05	0.61	0.85	1.40	0.54
1.50 - 2.00	2.000	1.34	3.39	0.69	0.90	1.51	0.50
2.00 - 3.00	3.000	1.81	3.95	0.80	1.05	1.66	0.60
3.00 - 4.00	4.000	2,25	4.40	0.89	1.20	1.79	0.6
4.00 - 5.00	5.000	2.66	4.78	0.97	1.30	1.89	0.63
5.00 - 6.00	6.000	3.04	5.11	1.04	1.35	1.98	0.6
6.00 - 7.00	7.000	3.42	5.42	1.10	1.45	2.06	0.70
7.00 - 8.00	8.000	3.78	5.70	1.15	1.55	2.13	0.70

(*)Canal Side Slope: 1:1.5

SLOPE OF 1:750

Distributary	Design	Area	Section	on Properties - N	Manning n=0.01	16	Freeboard
Range		-	Wetted	Depth	Base	Velocity	
Q	Q		Perimeter		Width		
(m3/s)	(m3/s)	(m2)	(m)	(m)	(m)	(m/s)	(m)
0.00 - 0.25	0.250	0.33	1.68	0.34	0.45	0.77	0.46
0.25 - 0.50	0.500	0.55	2.19	0.44	0.60	0.91	0.46
0.50 - 0.75	0,750	0.75	2.54	0.51	0.70	1.01	0.49
0.75 - 1.00	1.000	0.93	2.82	0.58	0.75	1.09	0.52
1.00 - 1.50	1.500	1.25	3.28	0.66	0.90	1.20	0.54
1.50 - 2.00	2.000	1.56	3.66	0.74	1.00	1.29	0.56
2.00 - 3.00	3,000	2.11	4.26	0.86	1.15	1.43	0.59
3.00 - 4.00	4.000	2.61	4.74	0.95	1.30	1.53	0.60
4.00 - 5.00	5.000	3.09	5.16	1.04	1.40	1.62	0.61
5.00 - 6.00	6.000	3.54	5.52	1.12	1.50	1.70	0.63
6.00 - 7.00	7.000	3.97	5.84	1.19	1.55	1.76	0.66
7.00 - 8.00	8.000	4.39	6.15	1.25	1.65	1.82	0.65

(*)Canal Side Slope: 1:1.5

SLOPE OF 1:1,000

Distributary	Design	Агеа	Section	on Properties - N	Manning n=0.0	16	Freeboard
Range	_		Wetted	Depth	Base	Velocity	
Q	Q		Perimeter		Width		
(m3/s)	(m3/s)	(m2)	(m)	(m)	(m)	(m/s)	(m)
0.00 - 0.25	0.250	0.37	1.78	0.36	0.50	0.69	0.44
0.25 - 0.50	0.500	0.62	2.31	0.46	0.65	0.82	0.49
0.50 - 0.75	0.750	0.83	2.67	0.53	0.75	0.91	0.47
0.75 - 1.00	1.000	1.03	2.98	0.61	0.80	0.97	0.49
1,00 - 1.50	1.500	1.40	3.47	0.70	0.95	1.08	0.50
1.50 - 2.00	2.000	1.73	3.86	0.78	1.05	1.16	0.52
2.00 - 3.00	3.000	2.35	4.49	0.91	1.20	1.28	0.54
3.00 - 4.00	4.000	2.91	5.00	1.01	1.35	1.38	0.59
4.00 - 5.00	5.000	3.44	5.44	1.11	1.45	1.46	0.59
5.00 - 6.00	6.000	3.94	5.82	1.18	1.55	1.52	0.62
6.00 - 7.00	7.000	4.42	6.16	1.25	1.65	1.58	0.60
7.00 - 8.00	8.000	4.89	6.49	1.31	1.75	1.64	0.64

(*)Canal Side Slope: 1:1.5

Table G.5.3.1 Standard Section Properties for Distributary (2/3)

SLOPE OF 1:1,5	500			-			-
Distributary	Design	Area _	Section	on Properties - N	Manning n=0.0	16	Freeboard
Range		_	Wetted	Depth	Base	Velocity	
Q	Q ·		Perimeter	-	Width		
(m3/s)	(m3/s)	(m2)	(m)	(m)	(m)	(m/s)	(m)
0.00 - 0.25	0.250	0.43	1.92	0.38	0.55	0.60	0.47
0.25 - 0.50	0.500	0.72	2.49	0.50	0.70	0.71	0.45
0.50 - 0.75	0.750	0.97	2.89	0.58	0.80	0.78	0.47
0.75 - 1.00	1.000	1.20	3.21	0.65	0.85	0.84	0.50
1.00 - 1.50	1.500	1.63	3.74	0.76	1.00	0.93	0.49
1.50 - 2.00	2.000	2.01	4.15	0.85	1.10	1.00	0.50
2.00 - 3.00	3.000	2.73	4.84	0.98	1.30	1.10	0.52
3.00 - 4.00	4.000	3.39	5.40	1,10	1.45	1.18	0.55
4.00 - 5.00	5.000	4.00	5.86	1.20	1.55	1.25	0.55
5.00 - 6.00	6,000	4.59	6.29	1.27	1.70	1.31	0.58
6.00 - 7.00	7.000	5.15	6.66	1.35	1.80	1.36	0.60
7.00 - 8.00	8.000	5.69	6.99	1.43	1.85	1.41	0.57

(*)Canal Side Slope: 1:1.5

SLOPE OF 1: 2.000

Distributary	Design	Area	Secti	on Properties - N	Manning n=0.0	16	Freeboard
Range		-	Wetted	Depth	Base	Velocity	
Q	Q		Perimeter		Width		
(m3/s)	(m3/s)	(m2)	(m)	(m)	(m)	(m/s)	(m)
0.00 - 0.25	0.250	0.48	2.02	0.41	0.55	0.54	0.44
0.25 - 0.50	0.500	0.80	2.62	0.53	0.70	0.63	0.47
0.50 - 0.75	0.750	1.08	3.05	0.61	0.85	0.70	0.49
0.75 - 1.00	1.000	1.34	3.39	0.69	0.90	0.75	0.46
1.00 - 1.50	1.500	1.81	3.94	0.80	1.05	0.83	0.50
1.50 - 2.00	2.000	2.25	4.40	0.89	1.20	0.89	0.51
2.00 - 3.00	3.000	3.04	5.11	1.04	1.35	0.99	0.51
3.00 - 4.00	4.000	3.77	5.70	1.15	1.55	1.06	0.55
4.00 - 5.00	5.000	4.46	6.19	1.26	1.65	1.12	0.54
5.00 - 6.00	6.000	5.12	6.64	1.34	1.80	1.18	0.56
6.00 - 7.00	7.000	5.74	7.03	1.42	1.90	1.22	0.58
7.00 - 8.00	8.000	6.34	7.39	1.49	2.00	1,26	0.56

(*)Canal Side Slope: 1:1.5

SLOPE OF 1: 2,500

Distributary	Design	Area	Section	on Properties - N	Manning n=0.01	16	Freeboard
Range			Wetted	Depth	Base	Velocity	
Q	Q		Perimeter		Width		
(m3/s)	(m3/s)	(m2)	(m)	(m)	(m)	(m/s)	(m)
0.00 - 0.25	0.250	0.52	2.11	0.42	0.60	0.49	0.43
0.25 - 0.50	0.500	0.87	2.73	0.55	0.75	0.58	0.45
0.50 - 0.75	0.750	1.17	3.17	0.64	0.85	0.64	0.46
0.75 - 1.00	1.000	1.45	3.53	0.72	0.95	0.69	0.48
1.00 - 1.50	1.500	1.97	4.12	0.84	1.10	0.76	0.51
1.50 - 2.00	2.000	2.44	4.58	0.92	1.25	0.82	0.48
2.00 - 3.00	3.000	3.31	5.34	1.08	1.45	0.91	0.52
3.00 - 4.00	4.000	4.10	5.94	1.20	1.60	0.98	0.55
4.00 - 5.00	5.000	4.85	6.46	1.31	1.75	1.03	0.54
5.00 - 6.00	6.000	5.56	6.92	1,41	1.85	1.08	0.54
6.00 - 7.00	7.000	6.24	7.32	1.49	1.95	1.12	0.56
7.00 - 8.00	8.000	6.90	7.70	1.57	2.05	1.16	0.58

(*)Canal Side Slope: 1:1.5

Table G.5.3.1 Standard Section Properties for Distributary (3/3)

SLOPE OF 1:3,000

Distributary	Design	Area	Section	on Properties - l	Manning n=0.0	16	Freeboard
Range			Wetted	Depth	Base	Velocity	
Q	Q		Perimeter		Width		
(m3/s)	(m3/s)	(m2)	(m)	(m)	(m)	(m/s)	(m)
0.00 - 0.25	0.250	0.55	2.19	0.44	0.60	0.45	0.46
0.25 - 0.50	0.500	0.93	2.82	0.58	0.75	0.54	0.47
0.50 - 0.75	0.750	1.25	3.28	0.66	0.90	0.60	0.49
0.75 - 1.00	1.000	1.56	3.66	0.74	1.00	0.65	0.46
1.00 - 1.50	1.500	2.11	4.26	0.86	1.15	0.71	0.49
1.50 - 2.00	2.000	2.61	4.74	0.95	1.30	0.77	0.50
2.00 - 3.00	3.000	3,54	5.52	1.12	1.50	0.85	0.53
3.00 - 4.00	4.000	4.39	6.15	1.25	1.65	0.91	0.50
4.00 - 5.00	5.000	5.19	6.69	1.36	1.80	0.96	0.54
5.00 - 6.00	6.000	5,95	7.15	1.46	1.90	1.01	0.54
6.00 - 7.00	7.000	6.68	7.58	1.54	2.05	1.05	0.56
7.00 - 8.00	8.000	7.38	7.97	1.62	2.15	1.08	0.58

(*)Canal Side Slope: 1:1.5

SLOPE OF 1: 3,500

Distributary	Design	Area	Section	on Properties - N	Manning n=0.01	16	Freeboard
Range	-	_	Wetted	Depth	Base	Velocity	
Q	Q		Perimeter		Width		
(m3/s)	(m3/s)	(m2)	(m)	(m)	(m)	(m/s)	(m)
0.00 - 0.25	0.250	0.58	2.23	0.45	0.60	0.43	0.45
0.25 - 0.50	0.500	0.98	2.90	0.58	0.80	0.51	0.47
0.50 - 0.75	0.750	1.33	3.38	0.69	0.90	0.57	0.46
0.75 - 1.00	1.000	1.65	3.76	0.77	1.00	0.61	0.48
1.00 - 1.50	1.500	2.23	4.38	0.88	1.20	0.67	0.47
1.50 - 2.00	2.000	2.77	4.88	0.99	1.30	0.72	0.51
2.00 - 3.00	3.000	3.75	5.69	1.15	1.55	0.80	0.50
3.00 - 4.00	4.000	4.66	6.33	1.28	1.70	0.86	0.52
4.00 - 5.00	5.000	5.50	6.88	1.40	1.85	0.91	0.55
5.00 - 6.00	6.000	6.30	7.36	1.50	1.95	0.95	0.55
6.00 - 7.00	7.000	7.08	7.80	1.58	2.10	0.99	0.57
7.00 - 8.00	8.000	7.82	8.20	1.67	2.20	1.02	0.58

(*)Canal Side Slope: 1:1.5

SLOPE OF 1: 4,000

Distributary	Design	Area	Section	on Properties - I	Manning n=0.01	16	Freeboard
Range		_	Wetted	Depth	Base	Velocity	
Q	Q		Perimeter		Width		
(m3/s)	(m3/s)	(m2)	(m)	(m)	(m)	(m/s)	(m)
0.00 - 0.25	0.250	0.62	2.31	0.46	0.65	0.41	0.44
0.25 - 0.50	0.500	1.03	2.97	0.60	0.80	0.49	0.45
0.50 - 0.75	0.750	1.40	3.47	0.70	0.95	0.54	0.45
0.75 - 1.00	1.000	1.73	3.86	0.78	1.05	0.58	0.47
1.00 - 1.50	1.500	2.35	4.49	0.91	1.20	0.64	0.49
1.50 - 2.00	2.000	2.91	5.00	1.01	1.35	0.69	0.49
2.00 - 3.00	3.000	3.94	5.82	1.18	1.55	0.76	0.52
3.00 - 4.00	4.000	4.89	6.49	1.31	1.75	0.82	0.54
4.00 - 5.00	5.000	5.79	7.06	1.43	1.90	0.87	0.52
5.00 - 6.00	6.000	6.63	7.55	1.54	2.00	0.91	0.56
6.00 - 7.00	7.000	7,44	8.00	1.62	2.15	0.94	0.53
7.00 - 8.00	8.000	8.22	8.41	1.71	2.25	0.97	0.54

(*)Canal Side Slope: 1:1.5

Table G.5.3.2 Hydraulic Design of Distributary No. 6 (1/2)

Name of	STA.	Gross	Cultivable	Discha	rge	Remarks
Miner and Mogha	2000	Irrigable Area (G.I.A.)	Command Area (C.C.A.)	Mogha	Disty	
		(G.I.A.) (ha)	(C.C.A.) (ha)	(m3/s)	(m3/s)	
В.Р.	0+000				5.330	
Sump Well	01000	148	136	0.072	5.258	
1R	0+050	174	160	0.084	5.174	
M-1	0+720	504	464	0.244	4.930	1L-4L
2R	0+720	174	160	0.084	4.846	
3R	1+320	174	160	0.084	4.762	
5L	2+180	174	160	0.084	4.678	
M-2	2+180	870	800	0.420	4.258	4R-8R
6L	2+640	174	160	0.084	4.174	
7L	3+200	174	160	0.084	4.090	
8L	3+680	174	160	0.084	4.006	
9L	4+140	170	157	0.082	3.924	
9R	4+140	142	131	0.069	3.855	
10L	4+600	174	160	0.084	3.771	
11L	4+920	174	160	0.084	3.687	
12L	5+450	174	160	0.084	3.603	
10R	5+450	174	160	0.084	3.519	
11R	5+680	174	160	0.084	3.435	
13L	6+150	174	160	0.084	3.351	
12R	6+150	174	160	0.084	3.267	
14L	6+640	174	160	0.084	3.183	
13R	6+640	174	160	0.084	3.099	
14R	7+100	174	160	0.084	3.015	
15L	7+590	114	105	0.055	2.960	
15R	7+590	174	160	0.084	2.876	
16L	8+070	174	160	0.084	2.792	
16R	8+070	174	160	0.084	2.708	
17L	8+570	174	160	0.084	2.624	
17E	8+570	169	156	0.084	2.542	
18L	9+070	149	138	0.082	2.469	•
18R	9+070	174	160	0.073	2.385	
19L	9+510	174	160	0.084	2.301	
19R	9+510	174	160	0.084	2.301	
20L	9+850	149	138	0.034	2.144	
20R	9+850	174	160	0.073	2.060	
21L	10+260	174	160	0.084	1.976	*
21R	10+260	174	160	0.084	1.892	
22L	10+708	174	160	0.084	1.808	94
22R	10+708	174	160	0.084	1.724	
M-3	10+708	3,005		1.452		23L-30L,28R-37
M-4	10+708	564		0.272	0.272	23R-27R
		11,030	10,150	5.330		

Table G.5.3.2 Hydraulic Design of Distributary No. 6 (2/2)

Name of	STA.	Gross	Cultivable	Dischar	rge	Remarks
Miner and Mogha		Irrigable Area (G.I.A.)	Command Area (C.C.A.)	Mogha	Disty	
		(ha)	(ha)	(m3/s)	(m3/s)	
(M-1)					0.244	
1L	0+020	126	116	0.061	0.183	
2L	1+550	126	116	0.061	0.122	
3L	2+770	126	116	0.061	0.061	
4L	3+800	126	116	0.061	0.000	
(34.2)					0.420	
(M-2)	0+020	. 174	160	0.084	0.420	
4R	0+020 1+000	174	160	0.084	0.330	
5R 6R	1+000	174	160	0.084	0.252	
ok 7R	3+100	174	160	0.084	0.188	
8R	3+100	174	160	0.084	0.000	
A4.3\					1.452	
(M-3)	0.000	174	160	0.084	1.452	
23L	0+600		129	0.068	1.300	
24L	1+200	140	160	0.084	1.216	
28R	1+200	174	141	0.084	1.142	
25L	1+700	153		0.074	1.142	
29R	1+700	174 174	160 160	0.084	0.974	
26L	2+260	174	160	0.084	0.890	
30R	2+260	174	160	0.084	0.896	
31R	2+780	174		0.064	0.733	
27L	3+270	174		0.073	0.733	
32R	3+270	174		0.084	0.565	
28L 33R	4+030 4+030	174		0.084	0.303	
29L	4+030 4+800	174		0.084	0.397	
29L 34R	4+800	174		0.084	0.313	
34R 35R	5+620	174		0.084	0.229	
30L	6+540	127		0.061	0.168	
36R	6+540	174		0.084	0.084	
37R	7+360	174		0.084	0.000	
(M-4)		-	_		0.272	
23R	0+020	174	160	0.084	0.188	
24R	0+800	95		0.046	0.142	
25R	1+440	95		0.046	0.096	
25R 26R	3+150	100		0.048	0.048	
27R	5+350	100		0.048	0.000	

Table G.5.3.3 Hydraulic Design of Distributary No. 18

Name of	STA.	Gross	Cultivable	Discha	arge	Remarks
Miner and		Irrigable	Command	Mogha	Disty	
Mogha		Area	Area			
		(G.I.A.)	(C.C.A.)			•
		(ha)	(ha)	(m3/s)	(m3/s)	
B.P.	0+000				1.340	
1R	0+400	174	160	0.084	1.256	
1L	0+400	136	125	0.065	1.191	
2R	1+050	174	160	0.084	1.107	
2L	1+050	123	113	0.059	1.048	
3R	1+650	174	160	0.084	0.964	
3L	1+650	135	124	0.065	0.899	
4R	2+250	174	160	0.084	0.815	
4L	2+250	131	120	0.063	0.752	
5R	2+850	174	160	0.084	0.668	
5L	2+850	125	114	0.059	0.609	
6R	3+450	174	160	0.084	0.525	
6L	3+450	137	125	0.065	0.460	
7R	4+220	174	160	0.084	0.376	
7L	4+220	151	138	0.072	0.304	
8R	5+000	174	160	0.084	0.220	
8L	5+000	141	129	0.067	0.153	
9R	5+880	170	156	0.082	0.071	
9L	5+880	149	136	0.071	0.000	
		2,790	2,560	1.340		

Table G.5.3.4 Outline of Model Command Areas of Distributary No. 6 & No. 18 and Irrigation Facilities

Item		D-6	D-18
1. Gross Command Area (G.C.A).	(ha)	11,470	3,010
2. Gross Irrigable Area (G.I.A.)	(ha)	11,030	2,790
3. Cultivable Command Area (C.C.A)	(ha)	10,150	2,560
4. Discharge	(m^3/s)	5.33	1.34
5. Structures			
a. Length of Distributary	(km)	10.707	7.680
b. Length of Minor	(km)	20.85	-
c. Head Regulator for Minor	(nos.)	3	-
d. Mogha	(nos.)	37	18
e. Fall with V.R. Bridge (Drop Struc	ctures)		
Type - I	(nos.)	3	-
Type - II	(nos.)	4	-
Type - III	(nos.)	2	5
f. Side Spillway	(nos.)	3	-
g. Culvert Road Bridge	(nos.)	3	1
h. Village Road Bridge	(nos.)	1	~
i. Tail Cluster	(nos.)	4	1

Table G.6.2.1 Capacity and Size of Regulating Pond in Distributary

Distributary		Size			
	Dead	Effective	Total		
<u></u>	(m3)	(m3)	(m3)	(m)*(m)	
D- 1	23,600	148,600	172,200	230*230	
D- 2	33,100	206,800	239,900	270*270	
D- 3	25,800	162,200	188,000	240*240	
D- 4	9,400	61,000	70,400	150*150	
D- 5	71,200	439,000	510,200	390*390	
D- 6	75,000	462,200	537,200	400*400	
D- 7	17,500	111,200	128,700	200*200	
D-8	53,500	331,700	385,200	340*340	
D- 9	87,000	535,600	622,600	430*430	
D-10	17,500	111,200	128,700	200*200	
D-11	19,500	123,100	142,600	210*210	
D-12	104,500	641,800	746,300	470*470	
D-13	47,200	293,000	340,200	320*320	
D-14	28,100	176,500	204,600	250*250	
D-15	14,000	89,300	103,300	180*180	
D-16	44,200	274,600	318,800	310*310	
D-17	67,400	416,300	483,700	380*380	
D-18	19,500	123,100	142,600	210*210	
D-19	19,500	123,100	142,600	210*210	
D-20	17,500	111,200	128,700	200*200	
D-21	28,100	176,500	204,600	250*250	
D-22	28,100	176,500	204,600	250*250	
D-23	9,400	61,000	70,400	150*150	
D-24	23,600	148,600	172,200	230*230	
D-25	35,700	222,800	258,500	280*280	

(*)Size

: Length of Top Bank

Side Slope : 1:1.5

Table G.7.2.1 Hydraulic and Earthwork Calculation for Flood Carrier Channels

No.	Bed	Berm	Water	n	Area	Wetted	Velocity	Discharge	_	Height	Ex.	Fill
	Width	Width	Depth		<i>(</i> 0)	Perimeter	(1-)	(21-)	of River	of Emb.	(*10^6 m3)	(*10^6 m3)
	(m)	(m)	(m)		(m2)	(m)	(m/s)	(m3/s)	(Km)	(m)	(-10-0103)	(.10.0103)
A 1	20.0	83.0	4.0	0.034	270.0	200.4	1.13	305	5.0	2.0	0.184	0.180
1-A	34.5	33.0	5.0	0.033	342.0	118.5	1.94	663				
B 2	55.0	57.5	5.0	0.033	542.5	188.0	1.94	1048				
2-A	36.0	34.5	5.0	0.033	355.5	123.0	1.94	690				0.252
C 3	38.0	35.5	5.0	0.033	369.5	127.0	1.95	722	10.5	3.0	0.669	0.662
3-A	45.5	45.0	5.0	0.033	445.0	153.5	1.95	864	10.5	3.0	0.788	
D 4	7.0	38.0	4.0	0.034	128.0	97.4	1.12	142				
E 5	29.0	26.0	5.0	0.033	286.5	99.0	1.95	558	24.5	3.0	1.232	1.544
F 6	10.0	60.0	4.0	0.034	184.0	144.4	1.09	198				
G 7	8.5	10.0	3.5	0.033	58.1	41.1	1.21	71	17.0	1.5	0.332	0.421
H 8	7.0	46.0	4.0	0.034	144.0	113,4	1.09					
19	48.0	48.0		0.033	469.5	162.0	1.95					
9-A	22.0	52.0		0.034	285.4		1.48					
9-B	26.0	60.0		0.034	327.4	162.2	1.49					
J 10	28.0	69.5	4.5	0.034	364.9	183.2	1.47	538	14.0	2.5	0.683	0.683
K 11	26.0	60.0		0.034	327.4	162.2	1.49					
L 12	52.0	51.0		0.033	501.5	172.0	1.96					
12-A	78.0	81.5		0.033	753.5	259.0	1.95					
12-B	106.0			0.033	1039.5	360.0	1.94					
12-C	102.0	77.0	5.0	0.033	855.5	274.0	2.05	1750	5.0	3.0	0.730	0.315
12-D	12.0	60.0	4.0	0.033	192.0	146.4	1.15					
12-E	89.0	66.0	5.0	0.033	746.5	239.0	2.05					
12-F	14.0			0.033	220.0	168.4						
M 13	66.0			0.033	639.5	220.0						
13-A	36.0	21.0	5.0	0.033	301.5	96.0	2.05	618	3 10.5	3.0	0.578	0.662
13-B	41.0	26.0	5.0	0.033	346.5	111.0						
13-C	36.0			0.033	309.5	100.0						
N 14	10.0			0.034	95.4							
14-A	14.5			0.034	197.6							
0 15	18.0	32.0	4.0	0.034	160.0	96.4	1.30	213	2 8.0) 2.(0.270	0.288
P 16	12.0			0.034	128.0							
Q 17	14.5			0.034	197.6							
R 18	21.0			0.034	277.9							
S 19	3.5			0.033	40.6							
Т 20	25.5	63.5	4.5	0.034	335.6	168.7	1.47	490	6.5	5 2.5	0.293	3 0.317
Total						·			321.5	5	16.383	3 15.996

Table G.7.3.1 List of Supplemental Drains

	No. of	Type	Length		o. of Typ	
	Drainage		(Km)	Dra	inage	(Km)
			1.5	26 GD 1	D11.0 A	. 10
1	SD-D 1-1	A	1.5		D11-2 A	1.8
2	SD-D 1-2	A	4.4		D12-1 A	8.8
3		A	7.2		D12-2 A	
4	SD-D 2-1	A	3.3		D12-3 A	11.4
5	SD-D 3-1	A .	6.1	30 SD-l	D12-4 A	8.4
6	SD-D 3-2	\mathbf{A}^{\perp}	0.7	31 SD-	D12-5 A	3.5
7	SD-D 4-1	Α	6.1		D12-6 A	8.2
8	SD-D 5-1	Α	10.1	33 SD-1	D12-7 A	5.0
9	SD-D 5-2	Α	6.1	34 SD-1	D13-1 A	4.0
10	SD-D 5-3	A	4.7		D14-1 A	6.7
11	SD-D 5-4	Α	7.5	36 SD-	D16-1 A	1.1
12	SD-D 5-5	Α	4.5		D17-1 A	6.2
13	SD-D 5-6	Α	5.3		D17-2 A	7.0
14	SD-D 6-1	Α	4.5	39 SD-	D17-3 B	1.7
15	SD-D 6-2	Α	3.9	40 SD-	D19-1 A	3.4
16	SD-D 6-3	Α	1.5	41 SD-	D20-1 A	5.7
17	SD-D 8-1	A	7.4		D21-1 A	7.4
18	SD-D 8-2	Α	6.4	43 SD-	D21-2 A	
19	SD-D 9-1	Α	8.9	44 SD-	D22-1 A	
20	SD-D 9-2	A	12.7		D24-1 A	
21	SD-D 9-3	A	3.0	46 SD-	D25-1 A	5.0
22	SD-D 9-4	В	5.5		D25-2 A	
23	SD-D 9-5	В	1.5		D25-3 A	
24	SD-D10-1	Ā	2.2			
25	SD-D11-1	A	3.6			

(*) Type A: Type A is provided to drains out the surplus irrigation water.

Total Length = 249.3 Km

Type B: Type B is provided to drains out the surplus irrigation water and flood discharge.

Total Length = 8.7 Km

FIGURES

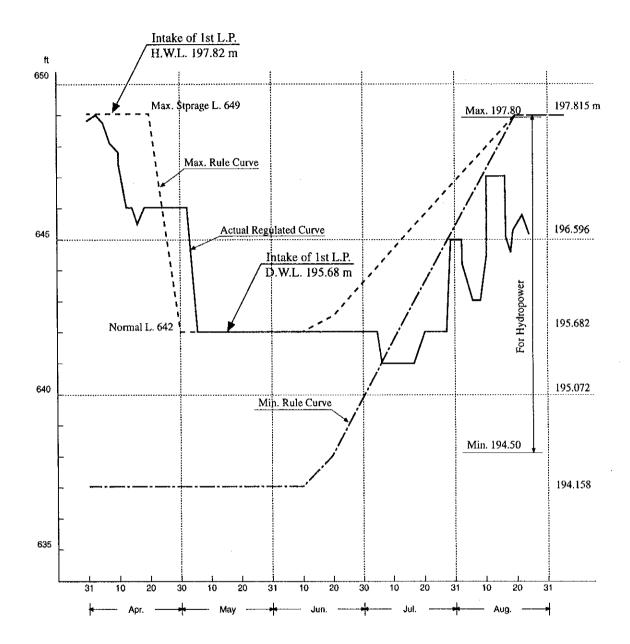


Fig. G.1.2.1 Water Level of Chashma Pond

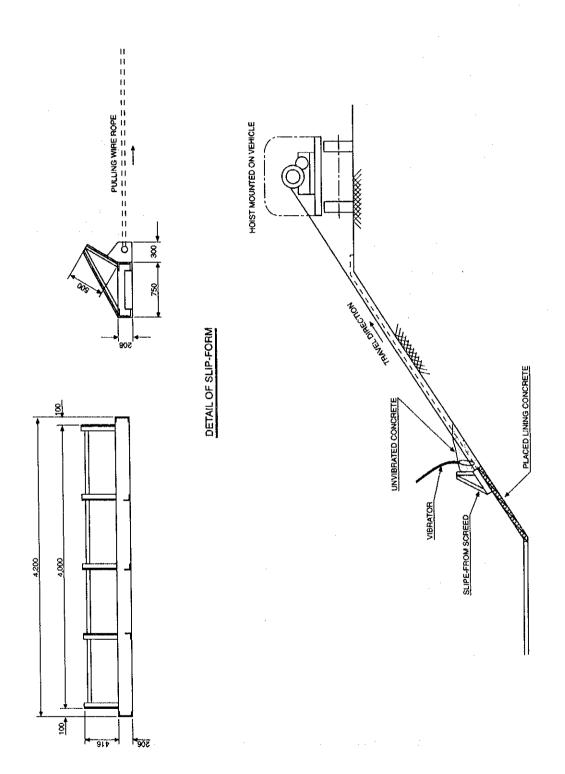


Fig. G.2.2.1 Canal Lining Method by Slipe-form Screed

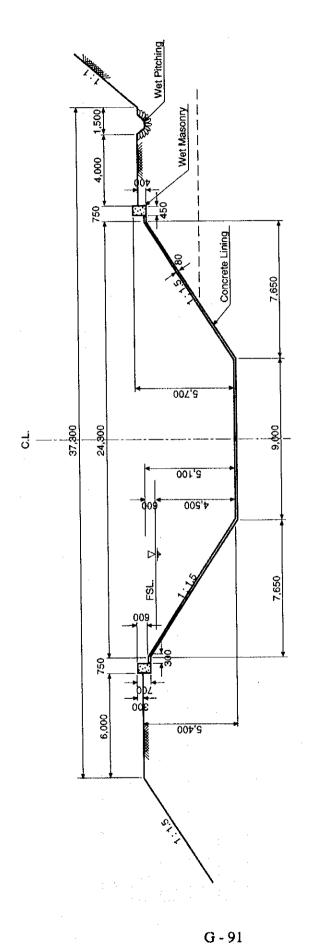
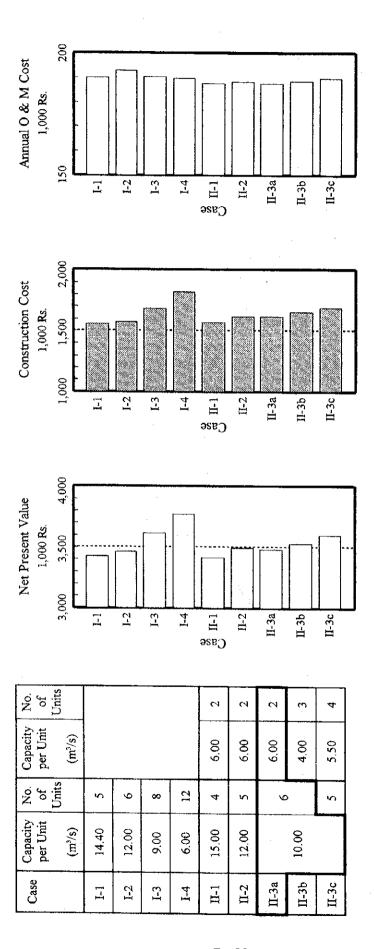
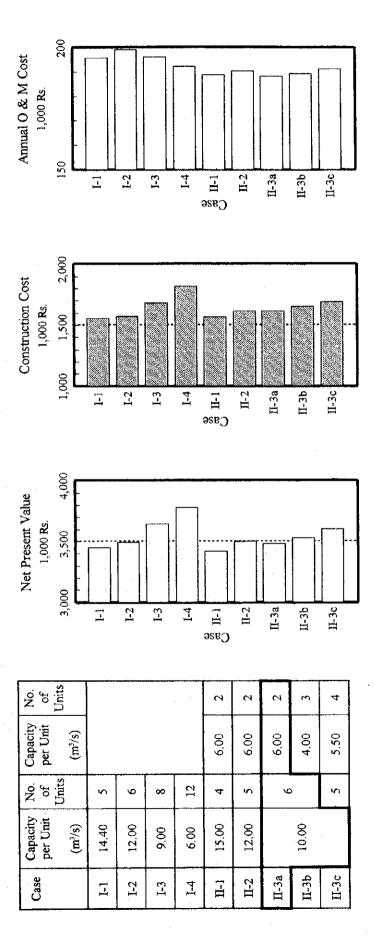


Fig. G.2.3.1 Standard Cross-section of Feeder Canal



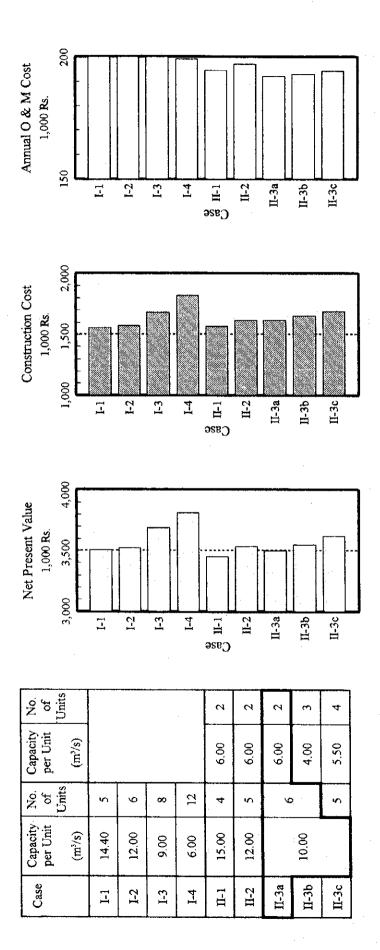
(1) Full (100%) Water of Regulating Ponds Assumed to be Available

Fig. G.3.3.1. Comparison of Net Present Value, Construction Cost and O&M Cost of Pumping Station (1/3)



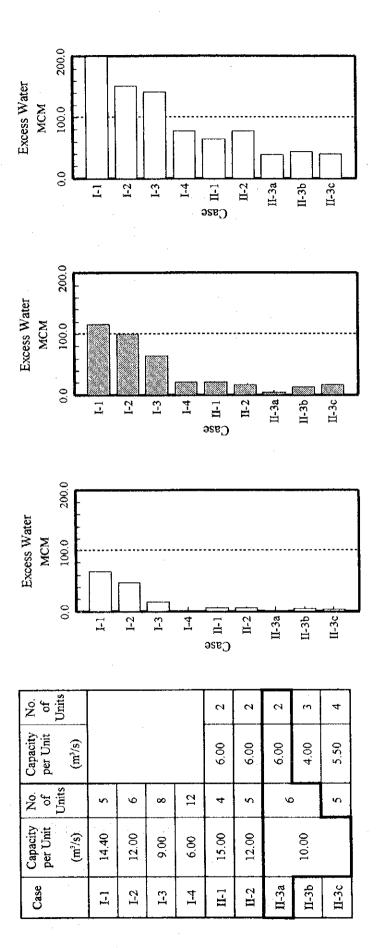
(2) 50% Water of Regulating Ponds Assumed to be Available3.3.1. Comparison of Net Present Value, Construction Cost

Fig. G.3.3.1. Comparison of Net Present Value, Construction Cost and O&M Cost of Pumping Station (2/3)



(3) Less than 10% Water of Regulating Ponds Assumed to be Available

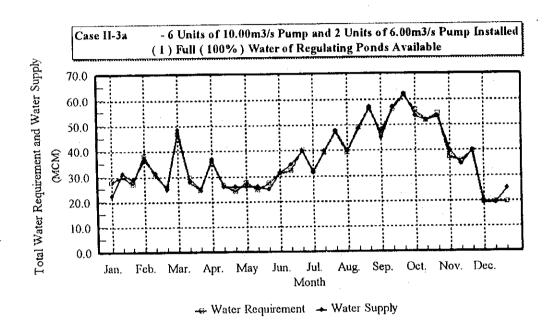
Fig. G.3.3.1. Comparison of Net Present Value, Construction Cost and O&M Cost of Pumping Station (3/3)



of Regulating Ponds Available (3) Less than 10% Water of Regulating Ponds Available (2) 50% Water

of Regulating Ponds Available (1) Full (100%) Water

Comparison of Excess Water Supply by Pumps in Each Case Fig. G.3.3.2.



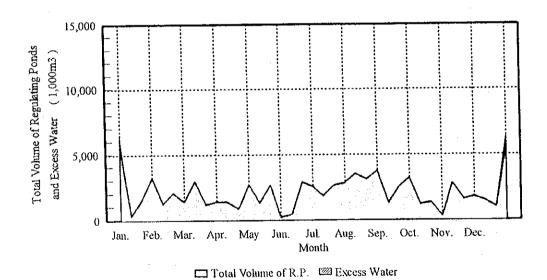
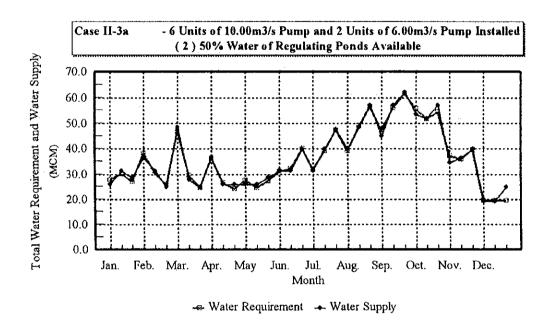


Fig.G.3.3.3. Results of Water Supply Simulatiom (1/3)



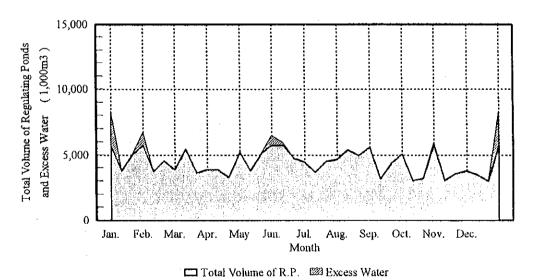
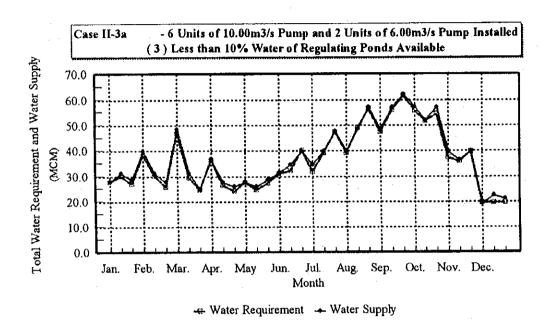


Fig.G.3.3.3. Results of Water Supply Simulatiom (2/3)



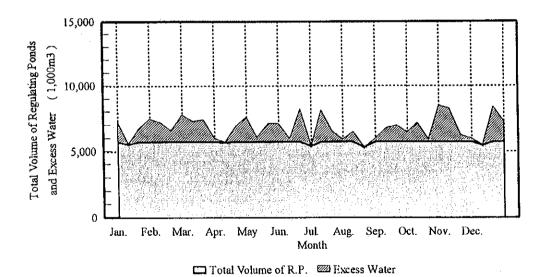


Fig.G.3.3.3. Results of Water Supply Simulatiom (3/3)

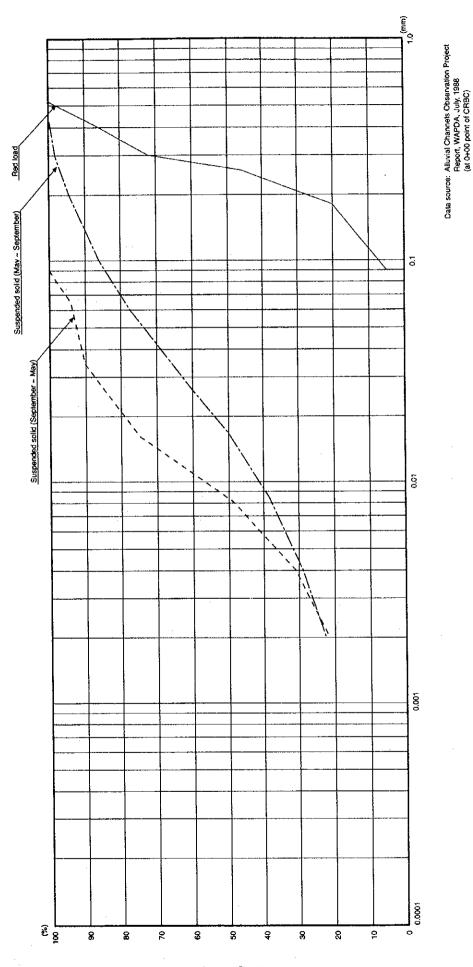
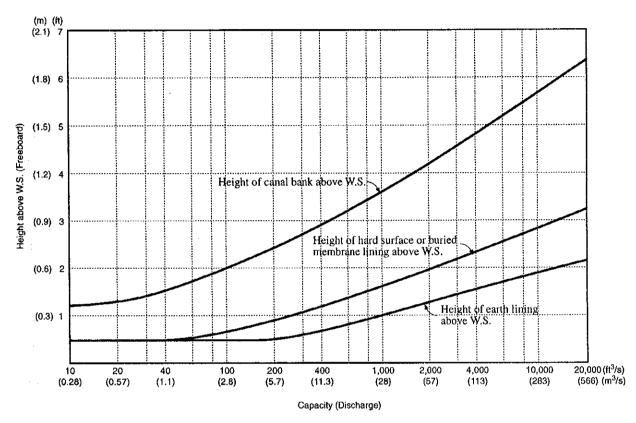


Fig. G.3.4.1 Gradation of Suspended and Bed Loads in CRBC at RD 0+00

PERCENT RETAINED

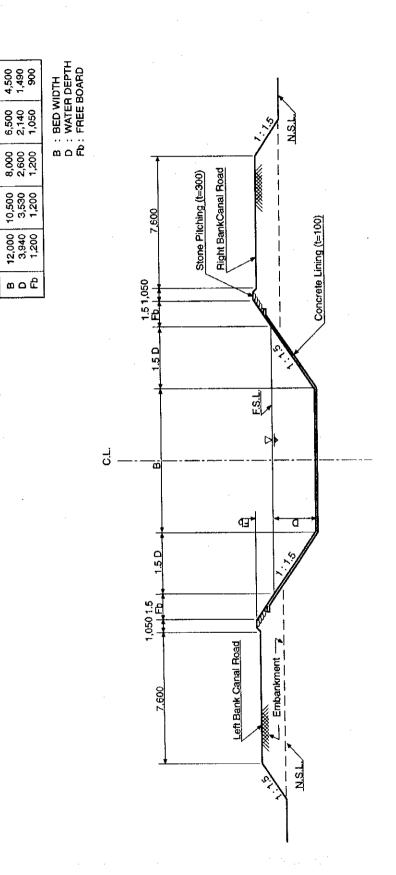
Fig. G.3.4.2 Gradation of Dune Sand in Pumping Site

РЕАСЕМТ FINER BY WEIGHT



(Note) Source: Design of Small Canal Structures (United Stated Department of the Interior Bureau of Reclamation)

Fig. G.4.2.1 Freeboard for Main Canal



(Unit: mm)

DIMENSION

SECTION OF CANAL

Fig. G.4.3.1 Standard Cross-section of Main Canal

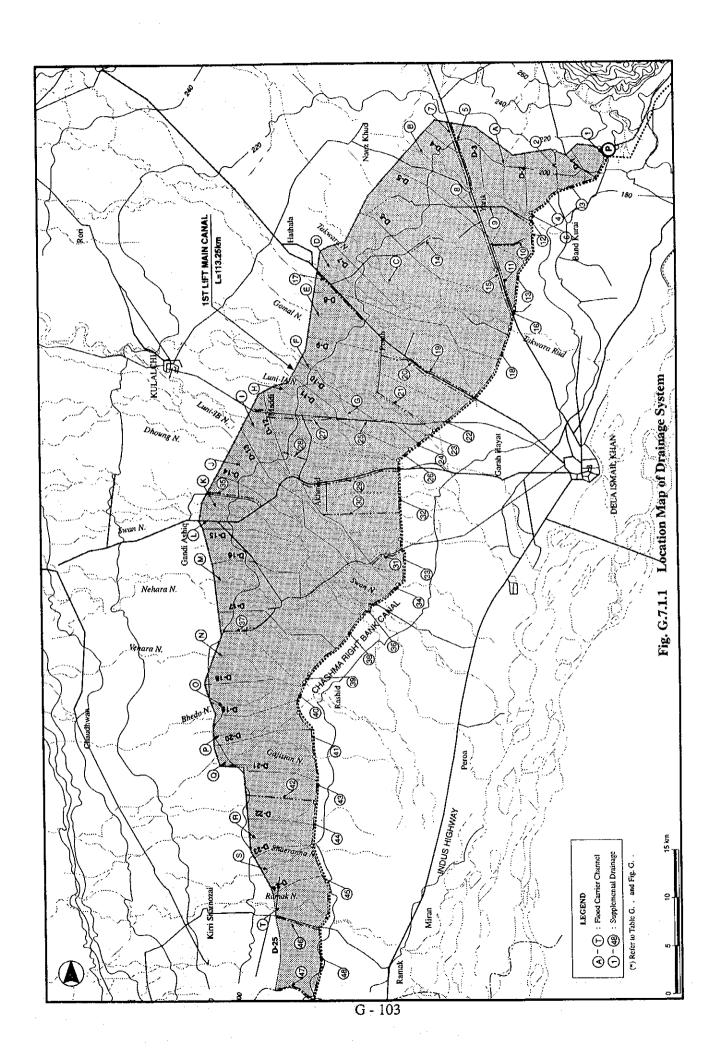
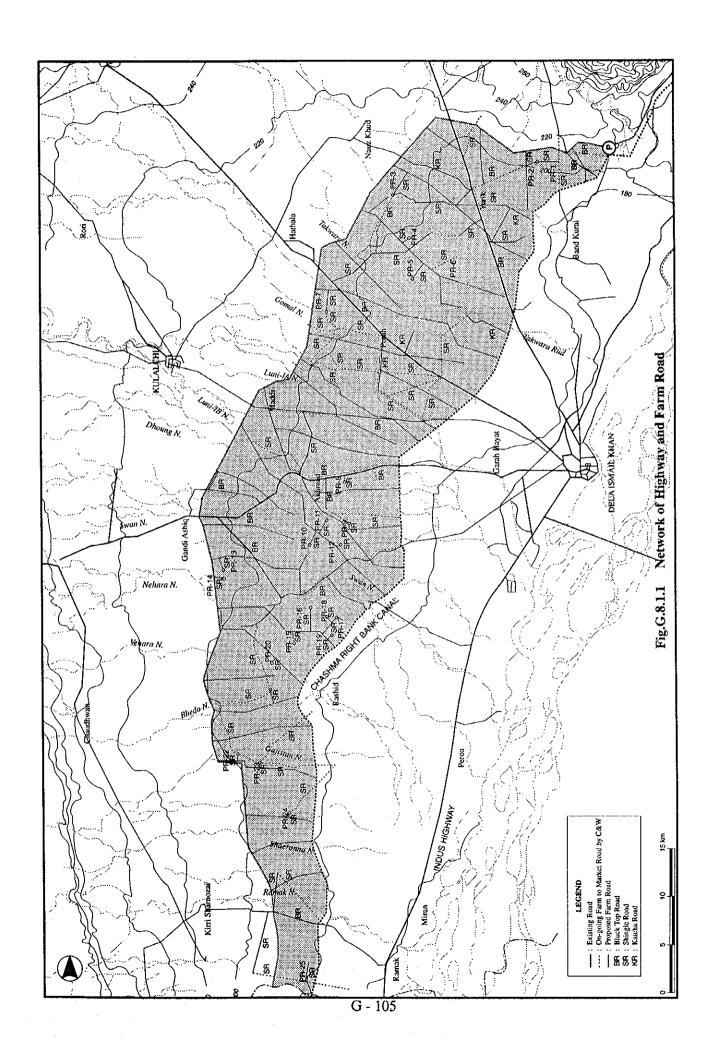
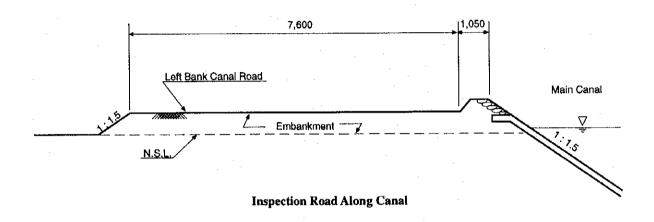
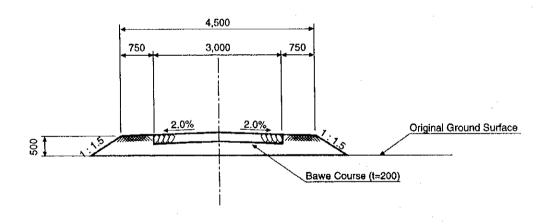


Fig. 7.2.1 Network of Flood Carrier Channel

SUPPLEMENTAL DRAINAGE







Farm Road
(Proposed Additional Farm Road: Total length 32.5 km)

Fig. G.8.3.1 Standard Cross-section of Proposed Road

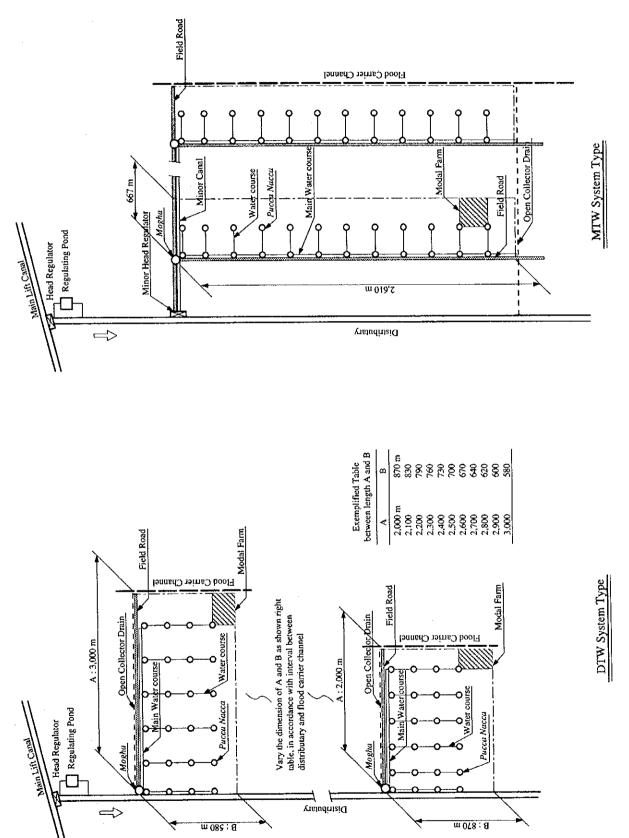


Fig. G.9.1.1 Typical Layout of Command Area Development System

ANNEX H

CONSTRUCTION PLAN AND COST ESTIMATE

ANNEX H

CONSTRUCTION PLAN AND COST ESTIMATE

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Implementation Plan

ANNEX H CONSTRUCTION PLAN AND COST ESTIMATE

H.1 Introduction

This ANNEX presents all the results of field and home studies for the construction plan and cost estimate of the Project comprising the project components of a feeder canal, a main canal, river treatments and drainage canals, distributory canals, on-farm development, additional farm roads, regulating ponds and other related facilities, based on the results of the comparative study discussed in ANNEX G. Some parts referring to the project organization and implementation arrangement are based on the study results discussed in ANNEX F.

H.2 Construction Plan

H.2.1 General

The construction works of the project are divided into two phases cost wise evenly considering work volume and role of each components so that the project will be benefiting right after completion of Phase I. Phase I consists of the feeder canal including a intake facility, the pump station with pump equipments for Phase I, section 1 of the main canal and other works in Phase I area. Phase II consists of remaining part of the main canal and the pump station and other works in Phase II. Each Phases are planned to be implemented partly in parallel for eighteen (18) months for acceleration of the project. Construction procedures for each components are stated in Chapter H.2.3 later. Major construction works having big earthwork volume are basically planned to be executed by the heavy duty construction machinery and equipment and minor construction works would be executed by combination of heavy equipment and manpower.

H.2.2 Basic Assumption of Construction Plan

H.2.2.1 Workable Days

Earth works are mostly affected by rainfall. Since embankment of canal band is controlled by moisture contents, special attention must be paid to execute the construction woks during rainy days. Flooding period is also to be considered for scheduling. Suspension days of these earth works caused by rainfall are assumed as following criteria according to the daily rainfall intensity.

D	aily Rainfall Intensity (mm/day)			Suspension (day)	
	0 - 10			0	
	10 - 30			1	10000
• :	30 - 50	1. 1. 1. 1.		2	
	50 - 100			3	
	more than 100		a de la composición della comp	4	

Annual mean workable days were estimated based on the above criteria and the rainfall records in D. I. Khan region for recent 10 years, and the computed result is shown in Table H.2.1 and its summary is as follows:

Month	Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.
W. Days	31.0	27.5	29.8	29.5	30.6	29.9	28.4	29.0	28.8	30.9	30.0	30.4

The result shows that the above workable days of all months are more than 25 days of standard workable days of civil works. Therefore, workable days for the construction works, are decided to be 25 days throughout a year, and total workable days are 280 days in a year, taking public holidays into account.

H.2.2.2 Conversion Rate of Earth Volume

Earth volumes are changeable according to the natural conditions as they are. Earth materials naturally placed would increase in volume after excavation and decrease after compaction. These changes of volume should be considered for estimate of produced volumes by construction equipment and machinery or earth moving plan. The conversion rates of earth volumes are assumed as follows:

	Class <u>Conversion Rate</u>			te
Abbreviation	of	Natural	Loose	Compacted
	Earth	Condition	Condition	Condition
E	Earth material	1.00	1.15	0.90
R	Rock	1.00	1.60	1.30
S	Sand	1.00	1.10	0.95

H.2.2.3 Basic Method of Earth Works

Earth works consist of excavating, loading, hauling, spreading and compacting. Since there are various methods for these earth works, due consideration must be given to the choice of the suitable combination of a heavy duty equipment. Suitable thickness of spreading and compaction, way of mixing and water content should be

studied again according to mechanical and physical testing of actual earth material at the detailed design stage.

Following equipment would be basically introduced on these earth works of the project.

Earth-works	Earth Materials	Proposed Equipment
Excavation	Common Soil	Bulldozer (11-21t), back-hoe shovel (0.6/1.2m ³)
	Rock	Blasting and Bulldozer with ripper (21t)
Loading	Any kind of Materials	Wheel loader (2.2m ³)
Ü	•	Back-hoe shovel (0.6/1.2m ³)
Hauling	Any kind of Materials	Dump truck (11/20t)
•	Sod, Wood etc.	Tractor trailer (5t)
Spreading	Any kind of Materials	Bulldozer and Grader
Compacting	Common Soil	Tamping and Sheep foot Roller
	Road Paving Material	Vibration Roller and Tire Roller

H.2.3 Construction Procedure and Method

H.2.3.1 General

According to the optimization study for project implementation described in ANNEX G, introduction of phase wise implementation into Phase I and II and construction schedule of each components are decided as described in the sub-chapter H.2.4 and as shown in Fig. H.2.2. Principal feature of each components are described in ANNEX G.

H.2.3.2 Earth Moving Plan

From the results of earth moving investigations, earth material of each section/component would be obtained from the following places. Re-usage of excavated material is planned to be maximized unless affecting for degradation of embankment to reduce construction cost. Aggregate material are partly obtained from excavated rock in the feeder canal reach and partly from market supply. Spoiling work within short range are basically included within excavation work.

Taking into account available materials, most economical construction method, combination of suitable construction machinery, etc., the earth moving plan for Feeder canal and pump station is illustrated in Fig. H.2.1 (1/2) and for Main canal, regulating ponds, river treatments and drainage canals and distributory canals in Fig. H.2.1 (2/2). Earth borrow areas are planned at three (3) spots, one of which, borrow area 1, is at hilly area along the feeder canal, one of which, borrow area 2, is

at hilly area near from the pump station and another one, borrow area 3 is to be arranged at about 5 km apart from the main canal section 1 (B) towards to north-west along Bannu road as shown in the aforementioned Figures.

Component	Section	Borrow Area (Hauling Distance)
Feeder Canal	Entire reach	Borrow Area 1 (10 km)
Pump Station	Settling basin	Borrow Area 2 (5 km)
Main Canal	Section 1 (A)	Borrow Area 3 (10 km) and Section 1 (B)
	•	as well as Regulating Ponds (16 km)
Distributory Canal	Entire reach	Side Borrow (0.5 km)
Drainage Dikes	Entire reach	Side Borrow (0.3 km)

H.2.3.3 Construction Procedure and Method for Phase I

Construction procedure and method of major works and work volumes in each components for Phase I are as described below. Work volume at borrow area and haulage are not included. Required area of land acquisition would be about 1,000 ha.

(1) Intake Structure

The intake structure is planned to be completed within Phase I and its operation will start from middle of the fifth year. Prior to construction work of the intake facility, temporary coffer dike is to be filled up at about 100 m upstream connecting from Spur 1 to the embankment of the low-hydro power station to have been completed and land draining work will follow by pumps. Temporary road will be arranged at downstream till completion of Chashma dike crossing work.

Foundation excavation by bulldozer and back hoe and sheet piling works using vibro hammer while draining and concrete placing will start after complete draining of the site. Gate installation and back filling work will be done carefully for function and durability of the intake facility. Dredging work along the approach canal towards to the intake will be continuously conducted by sand pumps on deck of dredging ships. Major item and volume of construction works for the intake facility are tabulated below.

Work Item	Unit	Work Volume
Embankment	1,000 m ³	30.0
Dredging Work	1,000 m ³	184.0
Dry Masonry	1,000 m ³	5.6
Concrete Work	$1,000 \text{ m}^3$	8.3
Sheet Piling	1,000 m ²	1.6
Radial Gate	nos	4

(2) Feeder Canal

The feeder canal is planned to be completed within Phase I and its operation will starts from middle of the fifth year. Construction works will be initiated by land clearing and stripping works along canal stretch and followed by excavation works. Weathered rock excavation would be done by bulldozer with ripper and blasting would be done for hard rock. Stepwise excavation like about 5 m gap stairs shall be applied for huge or mountainous section for safety execution. Replacement of foundation of embankment would be requested in case embankment would be partly saddle on right bank of CRBC or foundation soil would not be suitable. Embankment work should be done carefully observing water contents and soil properties of material.

Construction of cross drain structures (11 nos of super passages and 1 no of drainage culvert) are tightly scheduled within possible shortest period excluding flooding season and temporary closing dike should be arranged even in dry season for protection of construction sites.

Prior to placing of lining concrete of 80 mm thick, surface of slope and bottom should be smoothened, compacted well and pre-wetted and sand drain be completed. Concrete lining work will be conducted by about 4 m-width panel using motor driven steel slip form. Consolidation by vibrator and finishing by manual surfacing will be carefully done. Timely water supply to the surface shall be arranged while curing stage.

Temporary road will be pre-arranged for a part of Chashma road of about 4.0 km which is running on the canal stretch while construction and replacement will be done according to the design criteria of district road along the feeder canal.

Work Item	Unit	Work Volume
Embankment	1,000 m ³	7,250.3
Excavation common	$1,000 \text{ m}^3$	4,103.8
Excavation Rock	1,000 m ³	3,329.0
Base Coarse	$1,000 \text{ m}^3$	146.1
Dry Masonry	1,000 m ³	132.0
Concrete lining	1,000 m ²	1,611.5
Blasting Work	1,000 m ²	499.4
Structure*	nos	42

^{*:} excluding intake and berm drains

(3) Pump Station

Pump house, settling basin with gates and spillway, outlet pond and necessary amount of excavation and back-filling for installation of a delivery pipeline are scheduled to be completed within Phase I. Pump equipment as listed below and other auxiliary equipments including all electric items are accordingly installed at the ending stage. Detailed construction scheduling is required at the design stage for arrangement of machinery, equipment and materials within limited construction period of about one (1) and half year and limited construction site. Concrete lining is proposed for the settling basin and connecting channels.

Excavation of foundation of pump station and construction of settling basin will be commenced at first and followed by foundation work of in situ concrete piles and building works of the station. Outlet pond can be executed independently. Prearrangement of procurement, production, transportation and fabrication of necessary equipment installed are necessary for timely completion of the pump station. Pump equipments, pipes and valves delivered to the site by several packages considering transportation convenience are to be installed using ceiling crane in the pump house.

Careful treatment of delivery pipes and casings while traveling are proposed for protection by arrangement of supporting bars or equivalent material inside. The delivery pipe is to be buried after installation temporarily for protection until completion of Phase II works.

Work Item	Unit	Work Volume
Embankment	1,000 m ³	84.4
Excavation Common	1,000 m ³	491.2
Back-filling	1,000 m ³	46.6
Concrete Lining	1,000 m ²	103.5
Concrete Work	1,000 m ³	35.5
RC Piling	m	5,414
Pump House	nos	1
Pump (d=2000)	nos	2
Pump (d=1650)	nos	1
Delivery Pipe (d=3200)	nos	. 1

(4) Main Canal

Section 1 of 32.8 km of the main canal is scheduled within Phase I. Major work items and procedures are as same as the feeder canal. Section 1 is divided into two (2) sub-section as mentioned in the sub-chapter of earth moving plan, upstream section of which is section 1 (A) of 16.6 km and running through sand dune area and

downstream section of which is section 1 (B) of 16.2 km in normal soil area. Procedure and method for concrete lining is as same as the feeder canal.

In the section 1 (A), embankment material is in short much and borrow from section 1 (B), regulating pond sites and borrow area 3 and quality control of embankment material is especially required. Work volume for each major items are listed below.

Work Item	Unit	Work Volume
Embankment	1,000 m ³	1,735.4
Excavation common	1,000 m ³	2,058.7
Base Coarse	1,000 m ³	79.9
Dry Masonry	$1,000 \text{ m}^3$	53.1
Concrete lining	1,000 m ²	930.5
Structure*	nos	18

^{*:} excluding intake and berm drains

(5) Distributory Canals

Construction of distributory canals of D-1 to D-6 and their minor canals (81.5 km) are included within Phase I. Major work items and volumes estimated using sample area method as described in Chapter 6 of the main report are listed below. Survey, mapping and design works are required for estimate of actual work volume and cost in the design stage.

Main work is embankment of canal band and structures using smaller machinery and equipment and more manual procedures comparing the feeder and main canal. After clearing and stripping, embankment work will be commenced using borrow material nearby within 500 m. Pre-fabricated steel forms are proposed for placing of concrete for the related structures to reduce construction period.

Prior to placing of lining concrete of 50 mm thick, surface of slope and bottom should be smoothened, compacted well and pre-wetted and sand drain be completed. Concrete lining work will be conducted by about 4 m-width panel using motor driven or manual steel slip form. Consolidation by vibrator and finishing by manual surfacing will be carefully done. Timely water supply to the surface shall be arranged while curing stage.

Work Item	Unit	Work Volume
Embankment	1,000 m ³	911.6
Excavation common	1,000 m ³	28.6
Base Coarse	1,000 m ³	41.7
Concrete lining	1,000 m ²	361.6
Regulating Pond	nos	6
Structure	nos	251

(6) Flood Carrier Channel and Supplemental Drainage

Major work items and volumes of flood carrier channels and supplemental drainages within Phase I are as listed below. Flood carrier channels have flood protection dikes at both side filled using excavation material from riverain expansion works. Excavation of riverain and compaction of the dikes are proposed to be done mainly by bulldozer. Supplemental drainages are planned to drain excess water in the project area and flow sections are much smaller than the flood carrier channels, therefore protection dikes are not proposed except few large drainages and drainage excavation mainly using bulldozer is main item of the construction work.

Work Item	Unit	Work Volume
Embankment	1,000 m ³	3,993.5
Excavation	1,000 m ³	5,057.1
Bridge	nos	4

(7) On-farm Development, Additional Farm Roads

On-farm development of 27,210 ha and additional farm roads in the area are scheduled within Phase I. Construction procedures are based on farmer's participation for major work except excavation work for collector drains by back hoe. All materials such as cement, bricks, pre-casted structures are given to them on project account and their manual execution of the rest of all works including compaction of canal band will be following.

Unit area (1.0 ha) work volumes are estimated as listed below.

Work Item	Unit	Work Volume
Excavation	1,000 m ³	11.2
Brick Work	1,000 nos	144
Cement	ton	30
Aggregate	m3	115
Structure (Pacca)	nos	60

H.2.3.4 Construction Procedure and Method for Phase II

Construction procedure and method of major works and work volumes in each components for Phase II are as described below. Work volume at borrow area and haulage are not included. Required area of land acquisition would be about 2,200 ha.

(1) Pump Station

Excavation, installation of two (2) delivery pipelines, back-filling and installation of remaining pump equipment as listed below are Phase II work for the pump station. Pre-arrangement of procurement, production, transportation and fabrication of necessary equipment installed are necessary for timely completion of the pump station also in Phase II

Work Item	Unit	Work Volume
Excavation Common	1,000 m ³	56.1
Back-filling	1,000 m ³	45.1
Pump (d=2000)	nos	4
Pump (d=1650)	nos	. 1.
Delivery Pipe (d=3200)	nos	22

(2) Main Canal

Section 2 to 5 of 80.5 km of the main canal is scheduled within Phase II. Major work items and procedures are as same as the feeder canal. Procedure and method for concrete lining is as same as the feeder canal. Remaining work items and volumes are as listed below.

Work Item	Unit	Work Volume
Embankment	1,000 m ³	2,547.6
Excavation common	1,000 m ³	4,215.6
Base Coarse	1,000 m ³	196.6
Dry Masonry	1,000 m ³	130.7
Concrete lining	1,000 m ²	1,604.8
Structure	nos	68

(3) Distributory Canals

Construction of distributory canals of D-7 to D-25 and their minor canals (361.1 km) are included within Phase II. Major work items and volumes are as listed below. Procedures and methods are as same as Phase I.

Work Item	Unit	Work Volume
Embankment	1,000 m ³	2,961.4
Excavation common	1,000 m ³	92.8
Base Coarse	$1,000 \text{ m}^3$	135.5
Concrete lining	1,000 m ²	1,174.5
Regulating Pond	nos	19
Structure	nos	816

(4) Flood Carrier Channel and Supplemental Drainage

Major work items and volumes of flood carrier channels and supplemental drainages within Phase I are as listed below. Procedures and methods are as same as Phase I.

Work Item	Unit	Work Volume
Embankment	1,000 m ³	12,972.5
Excavation	$1,000 \text{ m}^3$	16,427.8
Bridge	nos	12

(5) On-farm Development, Additional Farm Roads

On-farm development of 88,390 ha and additional farm roads in the area are scheduled within Phase II. Procedures and methods are as same as Phase I.

H.2.3.5 Major Construction Equipment and Machinery Required

Major works required for the project are excavation works of about $45 \times 10^6 \text{ m}^3$ including common excavation, excavation at borrow area and drainage excavation, rock blasting or excavation of about $3 \times 10^6 \text{ m}^3$, common embankment, drainage dike embankment and back-filling works of about $40 \times 10^6 \text{ m}^3$, concrete lining works of $6 \times 10^6 \text{ m}^2$, stone works such as stone pitching and gabion works of about $690 \times 10^3 \text{ m}^3$, base course works of about $630 \times 10^3 \text{ m}^3$ for roads and other concrete works of $190 \times 10^3 \text{ m}^3$.

The major construction equipment and machinery required for timely completion of the construction work volume above are shown in Table H.2.2 based on the implementation schedule of Fig. H.2.2.

H.2.4 Implementation Schedule

The project implementation schedule for the project is shown in Fig. H.2.2. Phase wise implementation schedule are as described below.

Phase I, for five (5) and half years, will be started by detailed design, construction of camp facilities and procurement of vehicles and office equipment for approximately one (1) year. Procurement of O&M equipment is scheduled within the third year prior to the completion of components. Arrangement and setting-up of the farmer's associations and construction works of other facilities will also start from the first year and continue through to the end of Phase I.

Civil works for the feeder canal and the pump station will be commenced at the middle of the second year while the intake structure starts from the fourth year. Works for the main canal and distributory canals will start from the third year and be followed by commencement of the on-farm development including construction of additional farm roads and drainage works from middle of the fourth year and from the fifth year respectively.

Installation of radial gate for the intake and pump equipment are scheduled from middle of the fourth year to middle of the fifth year. Whole works for the feeder canal and intake facility as well as Phase I part of the main canal (section 1), pump station and distributory canals (D1 to D6) are planned to be completed by middle of the fifth year and the rest of all works for Phase I are to be completed by middle of the sixth year.

Phase II, for four (4) and half years, will be commenced by detailed design and arrangement for farmer's associations for approximately one (1) year. Phase II part of the main canal and distributory canals and other facilities will be commenced from the fifth year and followed by Phase II part of the pump station, drainage works and on-farm development including additional farm roads from middle of the fifth year. Installation of pump equipment is scheduled within the sixth year. Whole works are planned to be completed by the end of the seventh year.

H.3 Cost Estimate

H.3.1 Basic Conditions and Assumption for the Cost Estimate

The project cost comprises direct construction cost including project components, compensation cost for the land acquisition and Miscellaneous works, indirect construction cost including consultancy services, procurement of Office and O&M equipment and administration cost, physical and price contingency and interest and service charge. Following basic conditions and assumption are made for the estimate of the project cost.

- (1) The unit prices are analyzed on current price basis at the time of March, 1994 for the cost estimate.
- (2) The exchange rate used in the cost estimate is shown as follows.
 US\$ 1.0 = Rs. 30.0 = ¥ 107.1 as average during the period from July, 1993 to March, 1994.
- (3) Construction works would be executed by full contract basis through international competitive bidding. The machinery and equipment required for construction works would be provided by the contractors themselves. Therefore, depreciation costs of machinery and equipment are considered in the estimate of the construction unit cost.
- (4) Taxes on the construction materials, machinery and equipment to be imported from abroad are excluded from the cost estimate but taxation for machinery and equipment are separately listed up in Table H.2.2 based on current Excise and Taxation condition in Pakistan.
- (5) The construction cost integrated by construction unit costs is divided into both foreign and local currency portions. Local currency portion is estimated on the basis of the current price as at March, 1994 and of the data collected from the on-going projects and markets around the project area. Foreign currency portion is estimated based on the CIF prices at Karachi.
- (6) The physical contingency estimated at 10 % of the direct construction cost is included in the construction cost of both foreign and local currency portions.

H.3.2 Estimate of the Project Cost

Based on the conditions and assumption mentioned above, the construction cost for the project is summarized below. Details of each items are shown in Table H.3.1.

Cost Component	Foreign Currency (1,000Rs.)	Local Currency (1,000Rs.)	Total (1,000Rs.)
1. Direct Construction Cost	6,251,158	3,869,243	10,120,402
2. Indirect Construction Cost	1,000,185	619,079	1,619,264
3. Physical Contingency	625.116	386,924	1,012,040
Base Construction Cost	7,876,459	4,875,247	12,751,706
4. Price Contingency	2,281,202	1,779,755	4,060,957
5. Interest and Service Charge	218,907	134,183	353,090
Project Cost	10,376,568	6,789,185	17,165,753

Phase wise cost are summarized below and details are shown in Table H.3.2.

Phase wise Cost	Foreign Currency (1,000Rs.)	Local Currency (1,000Rs.)	Total (1,000Rs.)
1. Phase I 2. Phase II	5,343,524 5,033,045	3,498,414 3,290,771	8,841,938 1,619,264
Project Cost	10,376,568	6,789,185	17,165,753

H.3.3 Breakdown of Project Cost

(1) Land Acquisition and Compensation Cost

Cost for land acquisition is and compensation for the project is estimated based on the land price data from Excise and Taxation office and construction cost for normal house. Land acquisition cost and compensation cost are 97.9 million Rs. and 75.0 million Rs. respectively and shown in Table H.3.3. The cost is figured as a component of the direct construction cost for the project.

(2) Direct Construction Cost

Direct construction cost was estimated for the individual components by unit cost basis as discussed in the following Subsection H.3.5. Cost breakdown for major components such as Feeder canal, pump station, Main canal and river treatment and drainage canals are described in Table H.3.4, H.3.5, H.3.6 and H.3.7 respectively. List of pump equipment proposed are shown in Table H.3.8.

(3) Consultancy Service Costs

Consultancy services by foreign Consultants is required for the detailed design and construction supervision stages. Total required man-month of the engineers

including local consultant is 193 M/M for the detailed design and 964 M/M for the construction supervision for Phase I and 193 M/M for the detailed design and 864 M/M for the construction supervision for Phase II. The breakdown of the consultancy service costs and required man-month are shown in Tables H.3.10 and H.3.11.

(4) O&M Equipment

All the construction equipment and materials necessary for the construction of the Project would be provided by the contractors. While, vehicles, O&M equipment and office equipment are planned to be procured within the implementation cost by the Government for the smooth operation and maintenance of the Project facilities after the completion of the construction works.

The number of the vehicles, O&M equipment and office equipment and their procurement costs are estimated as listed in Table H.3.9.

(5) Administration Cost

Organization of the project office for implementation are as described in ANNEX F. Administration costs during implementation stage comprise staff salary, and direct costs such as office equipment, operation and maintenance costs for the vehicles and office operation costs.

H.3.4 Annual Disbursement Schedule

The annual disbursement schedule for the project is worked out as shown in Table H.3.12 based on the project implementation schedule illustrated in Fig. H.2.2 and the summary is as follows.

			Unit: Rs. 10 ⁶
Financial Year	Total	Foreign Currency	Local Currency
year l	638.0	458.9	179.1
year 2	932.2	539.8	392.4
year 3	1,655.2	1,002.5	652.7
year 4	2,918.7	1,711.0	1,207.7
year 5	4,374.1	2,602.2	1,771.9
year 6	4,000.5	2,497.0	1,503.5
year 7	2,647.0	1,565.1	1,081.9
Total	17,165.8	10,376.6	6,789.2

H.3.5 Unit Cost Analysis

Construction cost was calculated by use of detailed unit cost. Each unit cost is composed of the basic unit cost and working rate of labor and/or construction machinery. Basic costs of and materials surveyed and those classifications of foreign and local portions are shown in Table H.3.13.

Summary of CIF prices of the major construction machinery and equipment, and their hourly costs including the depreciation costs, operation and maintenance costs are shown in Table H.3.14. Unit cost was calculated by each, according to the proposed work items which were designed by construction method. Analyzed unit cost is summarized in Table H.3.15.

H.3.6 Annual Operation and Maintenance Cost

Annual operation and maintenance cost comprise of the salaries for administrative and technical staff, the materials and costs for replace and maintenance of the project facilities, and the costs for operation and maintenance of O&M equipment. The summary of the annual operation and maintenance cost is shown in Table H.3.16.

H.3.7 Replacement Cost

Some of the facilities installed or constructed in the Project have some shorter useful life than the Project life and will require replacement at a certain time within the project useful life. The replacement costs and the useful lives of these facilities are listed in Table H.3.20.

TABLES

Table H.2.1 Monthly Workable Days

Year	Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sep.	Oct.	Nov.	Dec.
	31	28	31	30	31	30	28	30	29	31	30	31
	31	28	31	30	31	30	28	31	30	31	30	30
	31	28	29	28	28	30	56	28	30	30	30	31
	31	26	59	30	31	30	30	31	30	31	30	31
	31	28	30	30	31	30	56	31	30	31	30	30
	31	28	31	30	31	30	29	27	30	31	30	56
	31	29	30	30	30	29	56	28	56	31	30	30
	31	27	30	29	31	30	31	56	30	31	30	31
	31	56	29	29	31	30	29	29	24	31	30	31
	31	27	28	29	31	30	1		1	ı		
	310	275	298	295	306	299	256	261	259	278	270	274
ays	Workable Days 31.0 27.5	27.5	29.8	29.5	30.6	29.9	28.4	29.0	28.8	30.9	30.0	30.4

Note; Hyphone shows lack of data

Table H.2.2 List of Major Construction Equipment and Machinery and Taxation
Unit: 1,000 Rs.

					U.	nit: 1,000 Ks	<u></u>
No.	Equipment	Specifica Spec.	PS	Tax Ratio	CIF Price for each Machine	Nos of Machine Required	Amount of Tax and Duty
A Farth Mi	oving & Excavation	· · · · · · · · · · · · · · · · · · ·					
	ozer, D6 with Ripper	11t	160	90%	3,300	50	148,500
	ozer ,D7 with Ripper	17 t	220	90%	4,950	60	267,300
	ozer ,D8 with Ripper	21 t	290	90%	6,600	50	297,000
	or shovel	1.2m3	100	70%	2,000	20	28,000
5. Excav		0.6m3	140	90%	3,500	125	393,750
6. Excav	· ·	1.2m3	210	90%	5,250	125	590,625
	l loader	2.2m3	160	90%	2,500	50	112,500
10. Dum		11 t	210	90%	1,000	60	54,000
11. Dum	•	20 t	290	90%	1,500	50	67,500
B. Compact	tio n						97.000
12. Tire	roller	11-17t	100	90%	1,000	30	27,000
13. Tire	roller	21-31t	150	90%	1,250	10	11,250
16. Vibra	ation roller	3t	25	90%	1,000	10	9,000
17. Vibra	ation roller	15t	160	90%	1,100	10	9,900
	t-Foot Roller	11-17t	100	90%	1,000	5	4,500
19. Pneu	matic Vibrater	-	-	90%	20	50	900
20. Trac	tor Water Bowser,4m3	5 t	60	90%	500		22,500
	er tank rolly, 10m3	11 t	210	90%	1,000		45,000
	er Grader of 3.7m brade	17 t	150	90%	1,500	40	54,000
C. Other E	quipment		L.S.			100	35,000
		sub-tota	ıl			<u>945</u>	2,178,22
	<u>Additi</u>	onal Tax(Sal	es and ot	<u>her)</u>			<u>501.95</u>
	TOTAL AM	OUNT OF	FAX AN	D DUTY			2,680,17

Note: in addition to above, 21% of sales or other kind of tax would be charged CIF Karachi price is estimated as sarae as domestic price in Japan. US\$1.00 = Rs. 30.0 = Yen 107.1

Table H.3.1 Project Cost

			Toject Cost		
Project Cost Component	Working Volume	unit	Foreign Currency (1,000 Rs.)	Local Currency (1,000 Rs.)	Total Cost (1,000 Rs.)
I. Direct Construction Cost					
a) Land Acquisition, Compensation & Pro	eliminary		35.604.8	226,407.2	262,012.0
Land Acquisition	3,200.0	ha	0.0	97,850.0	97,850.0
Compensation(House)	130	nos	0.0	75,000.0	75,000.0
Preliminary(Construction Camp)		L.S.	35,604.8	53,557.2	89,162.0
b) Feeder Canal			1,479,685.8	808.072.7	2.287,758.4
Earthwork	58.6	km	1,080,540.4	281,637.3	1,362,177.7
Structure	241	nos	399,145.3	526,435.4	925,580.7
c) Pump Station	1	sta.	1.193.602.8	395,015.2	1.588.618.0
Pump Equipment		set	909,964.7	162,494.9	1,072,459.5
Other Works*1		set	283,638.1	232,520.3	516,158.5
d) Main Canal			1.084.801.2	1,044,497.6	2,129,298,8
Earthwork	113.3	km	453,934.3	219,631.6	673,565.9
Structure	463	nos	630,866.9	824,866.0	1,455,732.9
e) Distributory Canals			413.238.5	402,778.4	816,016.8
Earthwork	442.6	km	272,449.2	179,933.3	452,382.5
Structure	1,093	nos	140,789.3	222,845.1	363,634.4
f) Regulation Pond	25	nos	330,307.0	265,176.4	595,483.5
h) River Treatment & Drainage Canals	579.5	km	1,247,968.5	273,147.7	1,521,116.2
i) Commercial Roads	32.5	km	11,014.5	7,045.6	18,060.1
j) On-farm Development Cost	115,600.0	ha	346,915.6	292,202.0	639,117.6
k) Sump Well & Domestic Water Supply	,	nos	10,915.6	9,244.5	20,160.2
l) Other and Miscellaneous Works		L.S.	97,104.0	145,656.0	242,760.0
Sub-total			6,251,158.3	3,869,243.3	10,120,401.6
II. Indirect Construction Cost					
a) Consultancy Service Cost(10% to D.C	est)	L.S.	625,115.8	386,924.3	1,012,040.2
b) Implementation Cost(6% to D.Cost)		L.S.			607,224.1
 Procurement of Office and O&M E 	quipment		208,500.0		208,500.0
- Administration Cost			166,569.5	232,154.6	398,724.1
Sub-total			1,000,185.3	<u>619,078.9</u>	1,619,264,3
III. Physical Contingency	10%	,	625,115.8	386,924.3	1,012,040.2
Total Base Construction Cost			7,876,459.4	4,875,246.6	12,751,706.0
IV. Price Contingency*2		F.C.		1,779,755.1	4,060,956.9
V. Interest and Service Charge a) Interest During Construction Period (I	No interest f	or GO	218,907.1 P is proposed)	134,183.2	353,090.3
b) Bank Service Charge(1 %)	3%	•	218,907.1	134,183.2	353,090.3
GRAND TOTAL COST			10,376,568.4	6,789,184.9	17,165,753.3

Note; *1: Construction cost of the substation is counted within the Other Works.

However counteraction work of transmission line is not considered because installation of transmission line for the Project should be done by the GOP in consideration of nationwide facilitating plan of national grid of electricity supply.

^{*2:}with annual escalating of 4.5% for F.C. and 5.5% for L.C.

Table H.3.2 Phasewise Construction Cost

					TOL CO. T			Dhose II	
Project Cost Component	Foreign Currency (1,000 Rs.)	Local Currency (1,000 Rs.)	Total Cost (1,000 Rs.)	Foreign Currency	Local Currency	Total Cost	Foreign Currency	Local Currency	Total Cost
I. Direct Construction Cost									
a) Land Acquisition, Compensation & Preliminary (3200ha)	35,604.8	226,407.2	262,012.0	35,604.8	119,407.2	155,012.0	0.0	107,000.0	107,000.0
(1:Camp, 1,000th and 72 houses II: 2,200th and 58 houses) b) Feeder Canal (58.6km)	1,479,685.8	808,072.7	2,287,758.4	1,479,685.8	808,072.7	2,287,758.4	0.0	0.0	0.0
c) Pump Station (1 station)	1,193,602.8	395,015.2	1,588,618.0	679,383.4	282,032,4	961,415.8	514,219.3	112,982.8	627.202.1
(I: Major part of Pump equipment and Delivery Pipe) d) Main Canal (113.3km)	1,084,801.2	1,044,497.6	2,129,298.8	471,888.5	454,356.4	926,245.0	612,912.7	590,141.1	1,203,053.8
(I: Sta.O-Sta.32+800 II: Further to the End) e) Distributory Canals (442.6km)	413,238.5	402,778.4	816,016.8	97,111.0	94,652.9	191,764.0	316,127.4	308,125.4	624,252.9
(I: D-1 to D-6 II: D-7 to D-25) f) Regulation Pond (25 nos)	330,307.0	265,176.4	595,483.5	77,622.2	62,316.5	139,938.6	252,684.9	202,860.0	455,544.9
(Same as above) h)Drainage Canals (579.5km)	1,247,968.5	273,147.7	1,521,116.2	293,272.6	64,189.7	357,462.3	954,695.9	208,958.0	1,163,653.9
(Same as above) i) Commercial Roads (32.5km)	11,014.5	7,045.6	18,060.1	2,588.4	1,655.7	4,244.1	8,426.1	5,389.9	13,816.0
(Same as above) j) On-farm Development Cost (32.5km)	346,915.6	292,202.0	639,117.6	81,525.2	68,667.5	150,192.6	265,390.4	223,534.5	488,925.0
(Same as above) k) Sump Well & Domestic Water Supply (L.S.)	10,915.6	9,244.5	20,160.2	2,565.2	2,172.5	4,737.6	8,350.5	7,072.1	15,422.5
(Same as above) I) Other and Miscellaneous Works (L.S.)	97,104.0	145,656.0	242,760.0	38,841.6	58,262.4	97,104.0	58,262.4	87,393.6	145,656.0
(Same as above) Sub-total of Direct Construction Cost	6.251,158.3	3.869.243.3	10,120,401.6	3,260,088,7	2.015.785.8	5,275,874,5	2,991.069.6	1.853.457.5	4.844.527.1
II. Indirect Construction Cost									
a) Consultancy Service Cost(10% to D.Cost)	625,115.8	386,924.3	1,012,040.2	326,008.9	201,578.6	527,587.5	299,107.0	185,345.7	484,452.7
b) Implementation Cost(6% of D. Cost)	375,069.5	232,154.6	607,224.1	195,605.3	120,947.2	316,552.5	179,464.2	111,207.4	290,671.6
Sub-total	1.000,185,3	619,078.9	1,619,264,3	521.614.2	322,525.7	844,139,9	478.571.1	296.553.2	775,124.3
III. Physical Contingency (10%)	625.115.8	386,924.3	1.012.040.2	326,008.9	201.578.6	527.587.5	299,107.0	185,345.7	484.452.7
Total Base Construction Cost	7,876,459.4	4,875,246.6	12,751,706.0	4,107,711.7	2,539,890.2	6,647,601.9	3,768,747.7	2,335,356.4	6,104,104.1
IV. Price Contingency*1	2,281,201.8	1.779,755.1	4,060,956,9	1,148,159.6	905,819.8	2,053,979.4	1,133,042.2	873,935.3	2,006,977.5
V. Interest and Service Charge	218.907.1	134,183,2	353,090.3	87,652.2	52,704.1	140,356.3	131,254.9	81,479.2	212,734.1
GRAND TOTAL COST	10,376,568.4	6,789,184.9	17,165,753.3	5,343,523.5	3,498,414.1	8,841,937.6	5,033,044.8	3,290,770.9	8,323,815.7

Note; *1: with annual escalation of 4.5% for F.C. and 5.5 % for L.C.

Table H.3.3 Cost of Land Aquisition and Compensation

Item	Unit	Unit Rate (1,000Rs.)	Q'ty	Amount (1,000Rs.)
I. Land Acquisition				
a) Rod kohi Area	ha	3,000	3,200	<u>95,850</u>
- Feeder canal	- do -	~ do -	322	9,660
- Main canal	- do -	- do -	574	17,220
- Distributory canals	- do -	- do -	709	21,270
- Drainage canals	- do -	- do -	1,248	37,440
- Quarry and other area	- do -	- do -	347	10,260
b) Village Area	ha	400,000	<u>5</u>	2,000
- Feeder canal	- do -	- do -	2	800
- Main canal	- do -	- do -	2	800
- Distributory canals & and other area	- do -	- do -	1	400
<u>su</u> b-total		·		<u>97.850</u>
II. Compensation				
a) Residence	nos	440,000	<u>130</u>	57,200
- Feeder canal area	- do -	- do -	54	23,760
- Main canal area	- do -	- do -	54	23,760
- Distributory canals & other area	- do -	- do -	22	9,680
b) Miscellaneous expense	L.S.			17.800
sub-total				75.000
Total				172,850

Table H.3.4 Breakdown of Construction Cost for Feeder Canal Including Intake Structure

Unit: Pakistan Rs.

Dry Masonry m3 Gabbion Mattress m3	2.5 37.2 38.8 31.7	L.C. 0.5 6.4 8.6	Work Volume 785,640 4,103,799	F.C.	Construction Cost L.C.	Total
A. Earthwork & Stonework Stripping of Top Soil Excavation Common(Backhoe),type 1 Excavation Common(Backhoe),type 2 Excavation Common(Composite),Type 1 Excavation Common(Composite),Type 2 Excavation Manual Excavation of Common Rock(ripper-dozer) Excavation for Structure(Small Backhoe) Excavation for Drain Haulage of Earth Material,type 1 Haulage of Earth Material,type 2 Embankment and Compaction,type 1 Embankment and Compaction, type 2 Embankment for Drain Backfill&Compaction, type 1 Backfill&Compaction, type 1 Backfill&Compaction,Type 2 Sod Facing(turfing) Dredging Work Sand Filter Loading or Unloading of Earth Material,1 Loading or Unloading of Earth Material,2 Gravelling & Compaction of sub-base Base Coarse filling and Compaction M3 M4 M5 M6 M6 M7 M7 M8 M8 M9 M9 M9 M9 M9 M9 M9 M9	2.5 37.2 38.8	0.5 6.4 8.6	785,640		L.C.	Total
Stripping of Top Soil Excavation Common(Backhoe),Type 1 Excavation Common(Backhoe),type 2 Excavation Common(Composite),Type 1 Excavation Common(Composite),Type 2 Excavation Manual Excavation of Common Rock(ripper-dozer) Excavation for Structure(Small Backhoe) Excavation for Drain Haulage of Earth Material,type 1 Haulage of Earth Material,type 2 Embankment and Compaction,type 1 Embankment and Compaction,type 1 Embankment for Drain Backfill&Compaction,type 1 Backfill&Compaction,type 1 Backfill&Compaction,Type 2 Sod Facing(turfing) Dredging Work Sand Filter Loading or Unloading of Earth Material,1 Loading or Unloading of Earth Material,2 Gravelling & Compaction M3 Back Compaction for Sub-base Base Coarse filling and Compaction M3 Metalling and Tarring M3 M5 M6 M6 M6 M6 M7 M7 M7 M8 M8 M9	37.2 38.8	6.4 8.6				
Excavation Common(Backhoe),Type 1 Excavation Common(Backhoe),type 2 Excavation Common(Composite),Type 1 Excavation Common(Composite),Type 2 Excavation Manual Excavation of Common Rock(ripper-dozer) Excavation for Structure(Small Backhoe) Excavation for Drain Haulage of Earth Material,type 1 Haulage of Earth Material,type 2 Embankment and Compaction,type 1 Embankment and Compaction,type 1 Embankment for Drain Backfill&Compaction,type 1 Backfill&Compaction,Type 2 Embankment for Drain Backfill&Compaction,Type 2 Embankment for Drain Backfill&Compaction,Type 2 Bod Facing(turfing) Dredging Work Sand Filter Loading or Unloading of Earth Material,1 Loading or Unloading of Earth Material,2 Gravelling & Compaction Gravelling & Compaction Manual Manual Manual Manual Manual Markin Mark	37.2 38.8	6.4 8.6				
Excavation Common(Backhoe),Type 1 Excavation Common(Backhoe),type 2 Excavation Common(Composite),Type 1 Excavation Common(Composite),Type 2 Excavation Manual Excavation of Common Rock(ripper-dozer) Excavation for Structure(Small Backhoe) Excavation for Drain Haulage of Earth Material,type 1 Haulage of Earth Material,type 2 Embankment and Compaction,type 1 Embankment and Compaction,type 1 Embankment for Drain Backfill&Compaction,type 1 Backfill&Compaction,Type 2 Embankment for Drain Backfill&Compaction,Type 2 Embankment for Drain Backfill&Compaction,Type 2 Bod Facing(turfing) Dredging Work Sand Filter Loading or Unloading of Earth Material,1 Loading or Unloading of Earth Material,2 Gravelling & Compaction Gravelling & Compaction Manual Manual Manual Manual Manual Markin Mark	37.2 38.8	6.4 8.6		1,964,100	400,676	2,364,776
Excavation Common(Backhoe),type 2 Excavation Common(Composite),Type 1 Excavation Common(Composite),Type 2 Excavation Manual Excavation of Common Rock(ripper-dozer) Excavation for Structure(Small Backhoe) Excavation for Drain Haulage of Earth Material,type 1 Haulage of Earth Material,type 2 Embankment and Compaction,type 1 Embankment and Compaction,type 2 Embankment and Compaction,Manual Embankment for Drain Backfill&Compaction,type 1 Backfill&Compaction,Type 2 Sod Facing(turfing) Dredging Work Sand Filter Loading or Unloading of Earth Material,1 Loading or Unloading of Earth Material,2 Gravelling & Compaction of sub-base Base Coarse filling and Compaction Manual Man	38.8	8.6	4.101.7991	152,661,306	26,264,311	178,925,617
Excavation Common(Composite), Type 1 Excavation Common(Composite), Type 2 Excavation Manual Excavation of Common Rock(ripper-dozer) Excavation for Structure(Small Backhoe) Excavation for Drain Haulage of Earth Material, type 1 Haulage of Earth Material, type 2 Embankment and Compaction, type 2 Embankment and Compaction, Manual Embankment for Drain Backfill&Compaction, Type 1 Backfill&Compaction, Type 2 Sod Facing(turfing) Dredging Work Sand Filter Loading or Unloading of Earth Material, 1 Loading or Unloading of Earth Material, 2 Gravelling & Compaction of sub-base Base Coarse filling and Compaction Manual Manu		1	1,105,777	152,001,500	20,201,511	1,0,525,017
Excavation Common(Composite), Type 2 Excavation Manual Excavation of Common Rock(ripper-dozer) Excavation for Structure(Small Backhoe) Excavation for Drain Maulage of Earth Material, type 1 Haulage of Earth Material, type 2 Embankment and Compaction, type 2 Embankment and Compaction, type 2 Embankment for Drain Backfill&Compaction, Manual Embankment for Drain Backfill&Compaction, Type 2 Sod Facing(turfing) Dredging Work Sand Filter Loading or Unloading of Earth Material, 1 Loading or Unloading of Earth Material, 2 Gravelling & Compaction of sub-base Base Coarse filling and Compaction Metalling and Tarring Wet Masonry Dry Masonry Gabbion Mattress m3		6.3	3,556,875	112,752,947	22,408,314	135,161,262
Excavation Manual Excavation of Common Rock(ripper-dozer) Excavation for Structure(Small Backhoe) Excavation for Drain Haulage of Earth Material,type1 Haulage of Earth Material,type2 Embankment and Compaction,type 1 Embankment and Compaction,type 2 Embankment and Compaction,Manual Embankment for Drain Backfill&Compaction,type 1 Backfill&Compaction,type 2 Sod Facing(turfing) Dredging Work Sand Filter Loading or Unloading of Earth Material,1 Loading or Unloading of Earth Material,2 Gravelling & Compaction of sub-base Base Coarse filling and Compaction Manual	26.6	6.8	3,550,015	0	22,100,514	155,101,202
Excavation of Common Rock(ripper-dozer) Excavation for Structure(Small Backhoe) Excavation for Drain Haulage of Earth Material,type 1 Haulage of Earth Material,type 2 Embankment and Compaction,type 1 Embankment and Compaction, type 2 Embankment and Compaction,Manual Embankment for Drain Backfill&Compaction,type 1 Backfill&Compaction,type 1 Backfill&Compaction,Type 2 Sod Facing(turfing) Dredging Work Sand Filter Loading or Unloading of Earth Material,1 Loading or Unloading of Earth Material,2 Gravelling & Compaction of sub-base Base Coarse filling and Compaction Metalling and Tarring Met Masonry Dry Masonry Gabbion Mattress m3 m3 m3 m3 m3 m3 m3	0.0	25.2		ol	ار	Č
Excavation for Structure(Small Backhoe) Excavation for Drain Haulage of Earth Material,type 1 Haulage of Earth Material,type 2 Embankment and Compaction,type 1 Embankment and Compaction, type 2 Embankment and Compaction,Manual Embankment for Drain Backfill&Compaction,type 1 Backfill&Compaction,Type 2 Sod Facing(turfing) Dredging Work Sand Filter Loading or Unloading of Earth Material,1 Loading or Unloading of Earth Material,2 Gravelling & Compaction of sub-base Base Coarse filling and Compaction Metalling and Tarring Met Masonry Dry Masonry Gabbion Mattress m3 m3 m3 m3 m3 m3 m3 m3 m3	92.8	14.3	3,328,997	308,930,922	47,604,657	356,535,579
Excavation for Drain Haulage of Earth Material,type 1 Haulage of Earth Material,type 2 Embankment and Compaction, type 2 Embankment and Compaction, type 2 Embankment and Compaction, Manual Embankment for Drain Backfill&Compaction, type 1 Backfill&Compaction,Type 2 Sod Facing(turfing) Dredging Work Sand Filter Loading or Unloading of Earth Material,1 Loading or Unloading of Earth Material,2 Gravelling & Compaction of sub-base Base Coarse filling and Compaction Metalling and Tarring Met Masonry Dry Masonry Gabbion Mattress m3*km	36.1	11.4	3,320,331	300,950,922	47,004,037	7,0,00,00
Haulage of Earth Material,type 1 Haulage of Earth Material,type 2 Embankment and Compaction,type 1 Embankment and Compaction, type 2 Embankment and Compaction, Manual Embankment for Drain Backfill&Compaction, type 1 Backfill&Compaction,Type 2 Sod Facing(turfing) Dredging Work Sand Filter Loading or Unloading of Earth Material,1 Loading or Unloading of Earth Material,2 Gravelling & Compaction of sub-base Base Coarse filling and Compaction Metalling and Tarring Wet Masonry Dry Masonry Gabbion Mattress m3 m3 m3 m3 m3	33.8	4.2		0	, ,	· ·
Haulage of Earth Material, type 2 m3*km Embankment and Compaction, type 2 m3 m3 Embankment and Compaction, type 2 m3 Embankment and Compaction, Manual m3 Embankment for Drain m3 m3 Earth Material, 2 m3 Earth Material, 3 m3 Earth Material, 4 m3	4.9	0.9	35,568,753	174,286,890	32,011,878	206,298,768
Embankment and Compaction, type 1 Embankment and Compaction, type 2 Embankment and Compaction, Manual Embankment for Drain Backfill&Compaction, type 1 Backfill&Compaction, Type 2 Sod Facing(turfing) Dredging Work Sand Filter Loading or Unloading of Earth Material, 1 Loading or Unloading of Earth Material, 2 Gravelling & Compaction of sub-base Base Coarse filling and Compaction Metalling and Tarring Wet Masonry Dry Masonry Gabbion Mattress m3 m3 m3 m3 m3	3.8	2.3	33,300,733	174,200,030	32,011,076	200,290,700
Embankment and Compaction, type 2 Embankment and Compaction, Manual Embankment for Drain Backfill&Compaction, type 1 Backfill&Compaction, Type 2 Sod Facing(turfing) Dredging Work Sand Filter Loading or Unloading of Earth Material, 1 Loading or Unloading of Earth Material, 2 Gravelling & Compaction of sub-base Base Coarse filling and Compaction Metalling and Tarring Wet Masonry Dry Masonry Gabbion Mattress m3 m3 m3 m3 m3	33.9	5.7	7,250,294	245,784,967	41,326,676	. 007.111.640
Embankment and Compaction, Manual m3 Embankment for Drain m3 Backfill&Compaction, type 1 m3 Backfill&Compaction, Type 2 m3 Sod Facing(turfing) m2 Dredging Work m3 Sand Filter m3 Loading or Unloading of Earth Material, 1 m3 Loading or Unloading of Earth Material, 2 m3 Gravelling & Compaction of sub-base m2 Base Coarse filling and Compaction m3 Metalling and Tarring m2 Wet Masonry m3 Dry Masonry m3 Gabbion Mattress m3		l.		· · ·]		287,111,642
Embankment for Drain m3 Backfill&Compaction type 1 m3 Backfill&Compaction,Type 2 m3 Sod Facing(turfing) m2 Dredging Work m3 Sand Filter m3 Loading or Unloading of Earth Material,1 Loading or Unloading of Earth Material,2 Gravelling & Compaction of sub-base m2 Base Coarse filling and Compaction m3 Metalling and Tarring m2 Wet Masonry m3 Dry Masonry m3 Gabbion Mattress m3	33.6	8.0	13,200	443,520	105,600	549,120
Backfill&Compaction type 1 m3 Backfill&Compaction,Type 2 m3 Sod Facing(turfing) m2 Dredging Work m3 Sand Filter m3 Loading or Unloading of Earth Material,1 m3 Loading or Unloading of Earth Material,2 m3 Gravelling & Compaction of sub-base m2 Base Coarse filling and Compaction m3 Metalling and Tarring m2 Wet Masonry m3 Dry Masonry m3 Gabbion Mattress m3	0.0	23.0		U	٥	U
Backfill&Compaction,Type 2 m3 Sod Facing(turfing) m2 Dredging Work m3 Sand Filter m3 Loading or Unloading of Earth Material,1 m3 Loading or Unloading of Earth Material,2 m3 Gravelling & Compaction of sub-base m2 Base Coarse filling and Compaction m3 Metalling and Tarring m2 Wet Masonry m3 Dry Masonry m3 Gabbion Mattress m3	19.3	4.5	į	U	٥	
Sod Facing(turfing) m2 Dredging Work m3 Sand Filter m3 Loading or Unloading of Earth Material,1 m3 Loading or Unloading of Earth Material,2 m3 Gravelling & Compaction of sub-base m2 Base Coarse filling and Compaction m3 Metalling and Tarring m2 Wet Masonry m3 Dry Masonry m3 Gabbion Mattress m3	43.3	7.4	ŀ	U,	0	Ü
Dredging Work Sand Filter Loading or Unloading of Earth Material,1 Loading or Unloading of Earth Material,2 Gravelling & Compaction of sub-base Base Coarse filling and Compaction Metalling and Tarring Wet Masonry Dry Masonry Gabbion Mattress m3 m3 m3 m3 m3	42.6	9.9		0	0	
Sand Filter m3 Loading or Unloading of Earth Material,1 m3 Loading or Unloading of Earth Material,2 m3 Gravelling & Compaction of sub-base m2 Base Coarse filling and Compaction m3 Metalling and Tarring m2 Wet Masonry m3 Dry Masonry m3 Gabbion Mattress m3	0.0	11.0	225,000	0	2,475,000	2,475,000
Loading or Unloading of Earth Material, 1 Loading or Unloading of Earth Material, 2 Gravelling & Compaction of sub-base m2 Base Coarse filling and Compaction m3 Metalling and Tarring m2 Wet Masonry m3 Dry Masonry m3 Gabbion Mattress m3	55.7	6.2		0	0	C
Loading or Unloading of Earth Material,2 Gravelling & Compaction of sub-base Base Coarse filling and Compaction Metalling and Tarring Wet Masonry Dry Masonry Gabbion Mattress m3 m3 m3	36.6	226.5		0	0	. (
Gravelling & Compaction of sub-base m2 Base Coarse filling and Compaction m3 Metalling and Tarring m2 Wet Masonry m3 Dry Masonry m3 Gabbion Mattress m3	14.6	4.0	3,556,875	51,930,379	14,227,501	66,157,881
Base Coarse filling and Compaction m3 m2 m2 m3 m2 m3 m3 m3	0.0	20.3		0	0	C
Metalling and Tarring m2 Wet Masonry m3 Dry Masonry m3 Gabbion Mattress m3	7.5	13.0	19,200	143,232	250,176	393,408
Wet Masonry m3 Dry Masonry m3 Gabbion Mattress m3	110.8	219.5	146,076	16,185,190	32,063,621	48,248,810
Dry Masonry m3 Gabbion Mattress m3	142.3	106.4	19,200	2,732,160	2,042,880	4,775,040
Gabbion Mattress m3	557.8	1,093.6		0	. 0	C
1 1	96.4	458.0	132,000	12,724,800	60,456,000	73,180,800
Blick Masonry m3	259.1	473.1		of	o	(
	250.0	1,100.0		o	0	(
Riprap m3	26.0	196.0	ĺ	o	l ol	(
SUB-TOTAL		1		1.080,540,413	281,637,290	1,362,177,703
	-					
B. Structure and Other Work	- 1					
a) Lining Work	- 1			191,768,500	275.566.500	467,335,000
Reinforced Concrete Lining at Structure m2	611.6	630.0	. 1	0	0	(
Concrete Lining Type 1 m2	119.0	171.0	1,611,500	191,768,500	275,566,500	467,335,000
Concrete Lining Type 2 m2	72.7	120.5	ı İ	0	o	(
b) Other Work				15.661.497	48.334.688	63,996,185
Building m2 1	,400.0	3,000.0	200	280,000	600,000	880,000
Reforestation 1,000nos 7	,160.9	5,294.3	56	401,010	1 '1	697,491
Blasting of Rock m3	30.0	95.0	499,350	14,980,487	l I	62,418,694
In-situ RC Piling(0.75m in dia) by Auger m 1	,485.0	1,215.0		0	0	
c) Related Structures		,]		191,715,347	202,534,208	394,249,55
Intake Structure nos 42,006	,426.6	37,054,158.1	1	42,006,426.6		79,060,585
<u> </u>	,934.3	4,791.7		775,062.6		1,719,023
	,106.4	8,785,013.1	1	7,831,106.4	1	16,616,119
Cross Drain(Super Passage) nos 11,490		12,766,764.7	11	126,391,628.7		
	342.9	3,732,001.4	1	3,639,342.9	1 1	7,371,344
	762.9	1,001,772.6	1	2,091,525.9	1 ' ' 1	
1	,638.0	419,376.4	17	6,827,846.6		
	,673.4	222,883.6	11			
173	,,,,,,,,	222,005.0		4,134,TV1,1	2,451,719.3	4,004,120
SUB-TOTAL				399.145.344	834 43E 304	Q1E EBA 744
	-			227.142.244	526.435,396	925 <u>,580.740</u>
GD LVD TOTAL	1		, 1	1		
GRAND TOTAL				1,479,685,756	808,072,686	

Table H.3.5 Brakedown of Construction Cost for Pump Station

Unit: Pakistan Rs. Construction Cost Unit Price Work Unit Description F.C. Total L.C. Volume FC A. Earthwork & Stonework 0.5 m2 Stripping of Top Soil 12,629,132 10,775,315 1,853,818 6.4 289,659 Excavation Common(Backhoe), Type 1 37.2 m3 8.6 38.8 Excavation Common(Backhoe),type 2 m36.3 160,959 5,102,400 1,014,042 6,116,442 31.7 Excavation Common(Composite), Type 1 m3 26.6 6.8 Excavation Common(Composite), Type 2 m3 0 0.0 25.2 Ð m_3 Excavation Manual 14.3 0 92.8 Excavation of Common Rock(ripper-dozer) m3 11.4 36.1 Excavation for Structure(Small Backhoe) m3 4.2 **33 S** Excavation for Drain m_3 1,754,886 322,326 2,077,212 358,140 0.9 m3*km 4.9 Haulage of Earth Material, type ! Haulage of Earth Material, type 2 m3*km 3.8 2.3 2,836,469 408,280 33.9 5.7 71,628 2,428,189 Embankment and Compaction, type 1 m3 8.0 Embankment and Compaction, type 2 m3 33.6 Û 23.0 Embankment and Compaction, Manual m3 0.019.3 4.5 Embankment for Drain m3 1,387,101 202,457 1.184.645 Backfill&Compaction ,type 1 43.3 7.4 27.359 m39.9 42.6 m3 Backfill&Compaction,Type 2 n 11.0 Sod Facing(turfing) m2 0.0 6.2 Dredging Work m3 55.7 226.5 m3 36.6 Sand Filter 1,332,281 286,512 1,045,769 Loading or Unloading of Earth Material, I m314.6 4.0 71,628 20.3 0.0 m3 Loading or Unloading of Earth Material,2 13.0 Gravelling & Compaction of sub-base m2 7.5 110.8 219.5 Base Coarse filling and Compaction m3 106.4 m2 142.3 Metalling and Tarring 557.8 1,093.6 Wet Masonry m3 96.4 458.0 m3 Dry Masonry 68.900 24,381 44.519 259.1 473.1 Gabbion Mattress m3 250.0 1.100.0 Blick Masonry m3 m3 26.0 196.0 Riprap 26,447,537 22,315,585 4.131.952 SUB-TOTAL B. Structure and Other Work 68,685,968 39,603,965 29.082.003 a) Concrete and other 157,483.2 182,199.6 339,682.8 610.4 706.2 258.0 Concrete A (140kgf/cm2,1:4:8) m3 0.0 0.0 0.09654 Concrete B (170kgf/cm2,1:3:6) m3 761.8 14,769,363.4 6.439,946,3 8,329,417.1 7 497 9 Concrete C (210kgf/cm2,1:2:4) m3 858.9 1.110.9 997,586.7 1,172,751.8 175,165.1 93.4 10,680.8 m2 16.4 Wooden Form 6,480,839.0 13,792,401.0 10.8 599,800.0 7,311,562.0 12.2 Reinforcement Bar kα 1,445,843.2 712,208.2 733,635.0 630.0 1,164.5 Reinforced Concrete Lining at Structure 611.6 m2 171.0 100,603.0 11,971,757.0 17,203,113.0 29,174,870.0 119.0 Concrete Lining Type 1 m2 0.0 0.0 0.0 120.5 Concrete Lining Type 2 m2 72.7 33,975.3 7,480.8 26,494.5 25.5 1.039.0Steel Slip Form m2 7,563,780.0 2,192,400.0 5,371,380.0 26,100.0 63,945.0 84.0 Fixed Wheel Gate Installation(W/O C/W) m2 393,300.0 114,000.0 279,300.0 22,800.0 55,860.0 m2 Slide Gate Installation 1.142.999.952 350.776.242 1.493.776.194 b) Related Structures 80,037,946.8 87,060,471.3 167,098,418 80,037,946.8 87.060,471.3 nos Suction Pit and Pump House 162,494,850.0 1,072,459,500 909.964.650.0 Pump and Auxiliary Equipment nos 909,964,650.0 162,494,850.0 97.346,234.8 245.389.322 148,043,087.4 97,346,234.8 148.043.087.4 Delivery Pipeline nos 8,828,954 4,954,268.0 3,874,685.8 3,874,685.8 4.954.268.0 Outlet Pond nos 1,172,081,955 390,380,207 1,562,462,162 SUB-TOTAL 1,588,618 1,193,603 395.015 GRAND TOTAL

Table H.3.6 Breakdown of Construction for Main Canal

Unit: Pakistan Rs. Description Unit Unit Price Work Construction Cost F.C L.C. Volume -F.C. L.C. Total A. Earthwork & Stonework Stripping of Top Soil m2 0.5 1,467,700 3,669,250 748,527 4.417.777 Excavation Common(Backhoe), Type 1 6,274,318 37.2 233,404,630 m3 6.4 40,155,635 273,560,265 Excavation Common(Backhoe),type 2 38.8 8.6 m3 Excavation Common(Composite), Type 1 m3 31.7 6.3 Excavation Common(Composite), Type 2 m3 26.6 6.8 0 Excavation Manual m3 0.0 25.2 0 Excavation of Common Rock(ripper-dozer) 0 92.8 m3 14.3 ol Excavation for Structure(Small Backhoe) m3 36.1 11.4 Λ Excavation for Drain m3 33.8 4.2 Haulage of Earth Material, type1 m3*km 4.9 0.9 8,000,000 39,200,000 7,200,000 46,400,000 Haulage of Earth Material, type 2 m3*km 3.8 2.3 Embankment and Compaction,type 1 33.9 5.7 m3 2,694,932 91,358,195 15,361,112 106,719,307 Embankment and Compaction, type 2 m3 33.6 8.0 Embankment and Compaction, Manual m3 0.0 23.0 Embankment for Drain m3 19.3 1,588,094 30,650,217 4.5 7,146,424 37,796,640 Backfill&Compaction ,type 1 m3 43.3 7.4 Backfill&Compaction,Type 2 42.6 9.9 m3 Sed Facing(turfing) m2 0.0 11.0 195,000 2,145,000 2,145,000 Dredging Work m3 55.7 6.2 Sand Filter m3 36.6 226.5 Loading or Unloading of Earth Material, 1 m3 14.6 500,000 7,300,000 2.000,000 4.0 9,300,000 Loading or Unloading of Earth Material, 2 m3 0.0 20.3 Gravelling & Compaction of sub-base m₂7.5 13.0 Base Coarse filling and Compaction m3 110.8 219.5 276,452 30,630,882 60,681,214 91,312,096 Metalling and Tarring m2 142.3 106.4 Wet Masonry m3 557.8 1,093.6 Dry Masonry 458.0 m3 96.4 183,829 17,721,118 84,193,691 101,914,809 Gabbion Mattress m3259 1 473.1 Blick Masonry m3 250.0 1,100.0 Riprap m3 26.0 196.0 SUB-TOTAL 453,934,290 673.565.894 219,631,603 B. Structure and Other Work a) Lining Work 301,703,080 433,539,720 735.242.800 Reinforced Concrete Lining at Structure m2 611.6 630.0 Concrete Lining Type 1 m2 119.0 171.0 2,535,320 301,703,080 433,539,720 735,242,800 Concrete Lining Type 2 m2 72.7 120.5 b) Other Work 811.330 599,844 1.411.174 Building m2 1,400.0 3.000.0 Reforestation .000no 7,160.9 5,294.3 113 811,330 599,844 1,411,174 Blasting of Rock m3 30.0 95.0 In-situ RC Piling(0.75m in dia) by Auger m 1,485.0 1,215.0 c) Related Structures 328,352,483 390.726.419 719,078,901 Drainage Culvert 9.574.021.6 nos 9 560 608 5 10 95.740.216 95,606,085 191,346,301 Super Passage(Canal Culvert) 16,626,141.9 nos 19,082,572.0 149,635,277 171,743,148 321,378,425 Cross Regulator nos 2,549,845.8 4,558,419.0 18 45,897,224 82,051,542 127,948,766 Escape 4,053,182.3 4,131,665.7 nos 20.265,912 20.658,329 40,924,240 Berm Drain 3,934.3 4,791.7 377 поѕ 1,483,242 1,806,462 3,289,704 Head Regulator to Distributory 237,327.1 384,381.1 nos 5.695.852 24 9,225,148 14,920,999 A.R. Bridge 1,292,899.2 1,237,247.0 nos 1,292,899 1,237,247 2,530,146 D.R. Bridge nos 944,333,6 904,609.9 3,777,334 3,618,440 7,395,774 V.R. Bridge nos 358,792.9 372,228.3 3,946,722 4,094,511 8,041,233 Footpath Bridge 154,451.4 171,376.9 617.805 685,507 1.303.313 SUB-TOTAL 630.866.893 824.865.983 1,455,732,876 GRAND TOTAL 1,084,801,183 1,044,497,586 2,129,298,769

Table H.3.7 Breakdown of Construction Cost for River Treatment and Drainage Canals

Unit: Pakistan Rs.

	17.7.	¥1_1.	n I	Work		C	Unit: Pakistan Rs.
Description	Unit	F.C.	Price L.C.	Volume	F.C.	Construction Cost L.C.	Total
A. Earthwork & Stonework		2.5	0.5		0.0	0.0	0.0
Stripping of Top Soil Excavation Common(Backhoe) Type 1	m2 m3	37.2	6.4		0.0	0.0	0.0
	m3	38.8	8.6		0.0	0.0	0.0
Excavation Common(Backhoe), type 2 Excavation Common(Composite), Type 1	m3	31.7	6.3	1,234,900	39,146,330.0	7,779,870.0	46,926,200.0
Excavation Common(Composite), Type 2	m3	26.6	6.8	1,234,500	0.0	0.0	0.0
Excavation Manual	m3	0.0	25.2	ļ	0.0	0.0	0.0
Excavation of Common Rock(ripper-dozer)	m3	92.8	14.3		. 0.0	0.0	0.0
Excavation for Structure(Small Backhoe)	m3	36.1	11.4		0.0	0.0	0.0
Excavation for Drain	m3	33.8	4.2	20,250,000	684,450,000.0	85,050,000.0	769,500,000.0
Haulage of Earth Material, type I	m3*km	4.9	0.9	370,470	1,815,303.0	333,423.0	2,148,726.0
Haulage of Earth Material, type 2	m3*km	3.8	2.3	,	0.0	0,0	0.0
Embankment and Compaction type 1	m3	33.9	5.7	970,000	32,883,000.0	5,529,000.0	38,412,000.0
Embankment and Compaction, type 2	m3 .	33.6	8.0	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	0.0		0.0
Embankment and Compaction, Manual	m3	0.0	23.0		0.0	1	0.0
Embankment for Drain	m3	19.3	4.5	15,996,000	308,722,800.0	71,982,000.0	380,704,800.0
Backfill&Compaction ,Type 1	m3	43.3	7.4	,,	0.0		0.0
Backfill&Compaction, Type 2	m3	42.6	9.9		0.0		0.0
Sod Facing(turfing)	m2	0.0	11.0		0.0	0.0	0.0
Dredging Work	m3	55.7	6.2		0.0	0.0	0.0
Sand Filter	m3	36.6	226.5	1	0.0	0.0	0.0
Loading or Unloading of Earth Material, I	m3	14.6	4.0	1,234,900	18,029,540.0	4,939,600,0	22,969,140.0
Loading or Unloading of Earth Material,2	m3	0.0	20.3	1,20 1,5 00	0.0	0.0	0.0
Gravelling & Compaction of sub-base	m2	7.5	13.0	ì	0.0	0.0	0.0
Base Coarse filling and Compaction	m3	110.8	219.5		0.0	0.0	0.0
Metalling and Tarring	m2	142.3	106.4		0.0	0.0	0.0
Wet Masonry	m3	557.8	1.093.6		0.0	0.0	0.0
Dry Masonry	m3	96.4	458.0		0.0	0.0	0.0
Gabbion Mattress	m3	259.1	473.1		0.0	0.0	0.0
Blick Masonry	m3	250.0	1,100.0		0.0	0.0	0.0
Riprap	m3	26.0	196.0		0.0	0.0	0.0
SUB-TOTAL					1,085,046,973.0	175,613.893.0	1.260.660.866.0
B. Concrete Work	1					İ	
Concrete A (140kgf/cm2,1:4:8)	m3	610.4	706.2		0.0	0.0	0.0
Concrete B (170kgf/cm2,1:3:6)	m3	761.8	965.4	i	0.0	1	0.0
Concrete C (210kgf/cm2,1:2:4)	m3	858.9	1,110.9		0.0	0.0	0.0
Wooden Form	m2	16.4	93.4		0.0	0.0	0.0
Reinforcement Bar	kg	12.2	10.8	,	0.0	0.0	0.0
Reinforced Concrete Lining at Structure	m2	611.6	630.0		0.0	0.0	0.0
Concrete Lining Type 1	m2	119.0	171.0		0.0	0.0	0.0
Concrete Lining Type 2	m2	72.7	120.5		0.0	0.0	0.0
Steel Slip Form	m2 -	7.2	25.5		0.0	0.0	0.0
Mortar	m3	839.1	1,138.9		0.0	0.0	0.0
SUB-TOTAL					0.0	0.0	0.0
C. Other Work		1					
A.R. Bridge (76.5m length ave.)	nos	3,878,697.7	3,878,697.7	2	7,757,395.3	7,757,395.3	15,514,790.7
D.R. Bridge (195.4m length ave.)	nos	8,366,103.5		7		1	117,125,448.6
V.R. Bridge (32.8m length ave.)	nos	594,190.9	1	7	4,159,336.3		15,139,847.1
Causeway	nos	i		0	0.0	0.0	0.0
Divider of Flood	nos			0	0.0	0.0	0.0
			1				
SUB-TOTAL					70.479.455.9	77.300.630.5	147.780.086.4
D. Additional Item(Protection)	9%			5%	<i>57,77</i> 6,321,4		70.422.047.6
E. Misccelaneous Works	9%	1 4 4 5 1 T		3%			42.253.228.6
TOTAL STREET,	70			3%	J+303.172.5	1.781.477.1	###\$\$## \$
GRAND TOTAL					1,247,968,543.2	273,147,685.3	1,521,116,228.6

Table H.3.8 List of Pump Equipment

Description	Unit	**-:4	Deine	W-1		0	Unit: Pakistan Rs.
Description	1 om	F.C.	Price L.C.	Work Volume	F.C.	Construction Cost L.C.	Total
	+ +	r.c.	L.C.	A Offittie	r.c.	L.C.	10(a)
A. Pump Equipment							(-1,-1,-1,-1,-1,-1,-1,-1,-1,-1,-1,-1,-1,-
Large Pump, Vertical Volute Mixed Flow	nos	40,590,000.0	4,510,000.0	1. 1	243,540,000.0	27,060,000.0	270,600,000.0
Small Pump, Vertical Volute Mixed Flow	nos	27,090,000.0			54,180,000.0	6,020,000.0	60,200,000.0
Main Motor for Large Pump	nos	28,170,000.0	3,130,000.0	1 . 1	169,020,000.0	18,780,000.0	187,800,000.0
Main Motor for Small Pump	nos	16,650,000.0	1,850,000.0	1 1	33,300,000.0	3,700,000.0	37,000,000.0
Check Valve (1800mm in dia)	nos	11,070,000.0	1,230,000.0	1 I	66,420,000.0	7,380,000.0	73,800,000.0
Check Valve(1200mm in dia)	nos	7,299,000.0	811,000.0	2	14,598,000.0	1,622,000.0	16,220,000.0
Motor Operated Butterfly Valve,4000mm	nos	19,260,000.0	2,140,000.0	. 2	38,520,000.0	4,280,000.0	42,800,000.0
Motor Operated Butterfly Valve, 1800mm	nos	3,573,000.0	397,000.0	. 6	21,438,000.0	2,382,000.0	23,820,000,0
Motor Operated Butterfly Valve, 1200mm	nos	2,637,000.0	293,000.0	2	5,274,000.0	586,000.0	5,860,000.0
Discharge Pipe	lot	16,520,000.0	7,080,000.0	1	16,520,000.0	7,080,000.0	23,600,000.0
Auxiliary Equipments	lot	9,100,000.0	3,900,000.0		9,100,000.0	3,900,000.0	13,000,000.0
Emergency Generator (150KVA)	nos	3,857,000.0	1,653,000.0	1	3,857,000.0	1,653,000.0	5,510,000.0
Electlic Equipments	lot	88,200,000.0	37,800,000.0	1	88,200,000.0	37,800,000.0	126,000,000.0
Motor Operated Butterfly Valve,3200mm	nos	12,060,000.0	1,340,000.0	. 3	36,180,000.0	4,020,000.0	40,200,000.0
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SUD TOTAL							
SUB-TOTAL					800,147,000,0	126.263.000.0	<u>926,410,000.0</u>
B. Bar Screen and Stop Log							
Bar Screen, 4.0mW-5.5mH	lot	4,739,000.0	2,031,000.0	1	4,739,000.0	2,031,000.0	6,770,000.0
Bar Screen, 3.8m & 2.2mW-5.5mH	lot	11,620,000.0	1	I 1	11,620,000.0	4,980,000.0	16,600,000.0
Stop Log	lot	3,857,000.0		1 1	3,857,000.0	1,653,000.0	5,510,000.0
			,		.,,		5,0 10,00010
SUB-TOTAL					20 21 6 202 0	0.664.000.0	40 000 000 0
SOB-TOTAL					20,216,000.0	. 8 <u>.664.000.0</u>	28,880,000.0
C. Substation							
Sub-station	lot	46,270,000.0	19,830,000.0	1	46,270,000.0	19,830,000.0	66,100,000.0
		,,,	***	1	10,210,00010	12,030,000.0	00,100,000.0
							•
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SUB-TOTAL					46,270,000.0	19.830.000,0	66.100.000.0
· ···-	_						
D. Additional Item(than above)	%			2.5%	21.665.825.0	<u>3.868.925,0</u>	25.534.750,0
E. Misccelaneous Works	%			2.5%	21.665.825.0	3.868.925.0	25.534.750.0
GRAND TOTAL					900 074 780 0	150 101 000 1	
GRAID IVIAL				<u> </u>	909,964,650.0	162,494,850.0	1,072,459,500.0

Table H.3.9 List of Vehicle, O&M Equipment and Office Equipment

N	ło.	Name of Equipment	Price (1,000Rs.)	nos	Total (1,000Rs.)
. c	OFFICE	USE			
Į.	-1	Utility Vehich(4WD)	2,000	10	20,000
Į.	-2	Light Geep(4WD)	600	78	46,800
I	:-3	Pickup Truck(4WD)	1,200	25	30,000
		Sub total		113	<u>96,800</u>
II. C	General		•		
I	II-1	Vibration Roller (3ton)	1,000	2	2,000
1	11-2	Back hone(0.6m3)	3,500	4	14,000
1	II-3	Bull Dozer (11ton)	3,000	4	12,000
]	II-4	Pickup Truck (4WD)	1,200	4	4,800
]	II-5	Wheel Loader (1m3)	2,500	3	7,500
]	II-6	Dump Truck	1,000	6	6,000
;	11-7	Tructor	350	6	2,100
		Sub total		<u>29</u>	<u>48,400</u>
III.	Intake				
	III-1	Pump-ship for Dredging	4,000	2	8,000
IV.	Pump S	Station			
	IV-1	Jet Pump for Cleaning	300	3	900
V.	On-far	m Development (for construction also)			
	V-1	Tructor with Trenching Attachment	500	50	25,000
VI.	OFFIC	E & Communication EQUIPMENT	L.S.		29,400
IV.	Spare :	Parts other Eqwpment(30%)	L.S.		62,550
	*************	Total			208,500

Table H.3.10 Cost of Consultancy Services

The stage of the state of the s

	· .	ι	Jnit: 1,000Rs
Îtem	Foreign Currency	Local Currency	Total
(Phase I)			
I. Detailed Design stage	<u>47.800</u>	<u>39.240</u>	<u>87.040</u>
1 Proposition (Proping 45 MAA)	39,000	:	39,000
1 Remuneration (Foreign 65 M/M) 2 " (Local 128 M/M)	39,000	10,240	10,240
3 Direct Cost	5,000	9,000	14,000
4 Other Study Cost & Special Equipment	3,800	20,000	23,800
4 Offici Study Cost & Special Equipment	3,000	20,000	23,800
II. Construction Supervision Stage	278,209	<u>162.339</u>	<u>440,548</u>
1 Remuneration (Foreign 316 M/M)	189,600		189,600
2 " (Local 648 M/M)	-	51,840	51,840
3 Other Study Cost & Special Equipment	38,609	30,499	69,108
4 Direct Cost	50,000	80,000	130,000
Phase I Total	326,009	201,579	527,588
			·
(Phase II)	47,000	20.240	
I. Detailed Design stage	<u>47.800</u>	<u>39.240</u>	<u>87.040</u>
1 Remuneration (Foreign 65 M/M)	39,000	-	39,000
2 " (Local 128 M/M)	-	10,240	10,240
3 Direct Cost	5,000	9,000	14,000
4 Other Study Cost & Special Equipment	3,800	20,000	23,800
II. Construction Supervision stage	251.307	146,105	<u>397.412</u>
1 Remuneration (Foreign 258 M/M)	154,800	-	154,800
2 " (Local 606 M/M)	-	48,480	48,480
3 Other Study Cost & Special Equipment	56,507	37,625	94,132
4 Direct Cost	40,000	60,000	100,000
Phase II Total	299,107	185,345	484,452
Total Cost	625,116	386,924	1,012,040

Table H.3.11 Required Man-months of Consultant Engineers

							Unit: Man	-Month	
]	Phase I]	Phase II			Total	
Specialist	Foreign	Local	Total	Foreign	Local	Total	Foreign	Local	Total
	Staff	Staff		Staff	Staff		Staff	Staff	
I. Detailed Design stage									
					•				_
1 Project Director	1		1	1	-	1	2	-	2
2 Team Leader	12		12	12	-	12		-	24
3 Senior Design Engineer	12		12	12	-	12	1		24
4 Structure Engineer	12		60	ļ	48	60	1	96	120
5 Soil Mechanical Engineer	4		16	4	12	16	ł .	24	32
6 Engineering Geologist	4	12	16	4	12	16	I .	24	32
7 Drainage & Hydrological Engineer	4	12	16	!	12	16	ł .	24	. 32
8 On-farm Design Engineer	4	8	12	4	8	12	1	16	24
9 Mechanical Engineer	4	8	12	4	8	12	8	16	24
10 Institutional Coodinator	4	12	16	4	12	16	8	24	32
11 Specialist as required	4	16	20	4	16	20	. 8	32	40
Sub Total	65	128	193	65	128	<u>193</u>	130	<u>256</u>	<u>386</u>
II. Construction Supervision Stage		•							
1 Project Director	4		4	3	-	3	7	_	
2 Resident Project Engineer	54		54	36	-	36	90	-	90
3 Senior Construction Engineer	54	-	54	36	-	36	90	-	9
4 Construction Supervisor	108	432	540	108	432	540	216	864	1,08
5 Engineering Geologist	12	24	36	6	12	18	18	36	5
6 Mechanical Engineer	12	24	36	9	18	27	21	42	. 6
7 Institutional Coodinator	24	72	96	24	72	96	48	144	19
8 Specialist as required	48	96	144	36	72	108	84	168	25
Sub Total	316	648	964	258	606	864	574	1,254	1.82
Total Man-Month	381	776	1,157	323	734	1,057	704	1,510	2,21

Table H.3.12 Disbursement Schedule

Unit; 1,000 Rs.

	Pro	Project Total Cost	7,2						Anmusil	Disburser	Annual Disbursement Schedule	edule					
1				1ct(1006)	8	2nd(1997)	8	3rd(1998)	95	4th(1999)	ĵ.	5th(20)	96	6th(2001)	901)	7th(2002)	(05)
Project Cost Compone	ዄ	Ŋ	Total Cost	FC	<u>2</u>	FC	,, rc	FC	Ľ	FC) J	. S	3	2	2	5	2
1. Direct Cost							;	•	,	¢		c	41166		37117	<	
a) Land Acquisition	35,604.8	226,407.2	262,012.0	35605	20582	5	41163	>	C0114	>	6114	>	COLIT	•	3	۰ د	· •
b) Feeder Canal	1,479,685.8	808,072.7	2,287,758.4	0	0	246614	134679	493229	269358	493229	269358	246614	134679	0	0	0	0
c) Pump Station	1,193,602.8	395,015.2	1,588,618.0	0	0	67938	28203	135877	56406	271753	112813	306659	107206	411375	90386		
d) Main Canal	1,084,801.2	1.044,497.6	2,129,298.8							188755	181743	393060	378456	298682	287585	204304	196714
e) Distributory Canals	413,238.5	402,778.4	816,016.8							38844	37861	144220	140570	124798	121639	105376	102708
f) Regulation Pond	330,307.0	265,176.4	595,483.5							31049	24927	115277	92547	99753	80083	84228	67620
h)Drainage Canals	1,247,968.5	273,147.7	1,521,116.2							-		386454	84585	479636	104980	381878	83583
i) Farm Roads	11,014.5	7,045.6	18,060.1							547	414	2979	9061	4018	2570	3370	2156
j) On-farm	346,915.6	292,202.0	639,117.6							20381	12167	93841	79041	126537	106581	106156	89414
Development k) Sump Weil	10,915.6	9,244.5	20,160.2							1026	869	3810	3226	3297	2792	2783	2357
1) Miscellaneous	97,104.0	145,656.0	242,760.0	13872	20808	13872	20808	13872	20808	13872	20808	13872	20808	13872	20808	13872	20808
Sub-total	6.251.158.3	3,869,243,3	10.120.401.6	49477	41390	328425	224855	642977	387737	1059557	707124	9829021	1084188	1561968	828289	901969	1983981
II. Indirect Cost																	
a) Consultancy	625,115.8	386,924.3	1,012,040.2	131603	81458	82252	50911	98702	61093	164504	101822	65802	40729	49351	30547	32901	20364
Service b) Implementation	375,069.5	232,154.6	607,224.1	232296	33165	23796	33165	23796	33165	23796	33165	23796	33165	23796	33165	237%	33165
Sub-total	1,000,185,3	619.078.9	1.619.264.3	363899	114623	106048	84076	122498	94258	188300	134987	16568	73894	73147	53712	26692	53529
III. Phisical	625,115.8	386,924.3	1.012,040.2	4948	4139	32842	22485	84298	38774	105956	2027.7	170679	108419	156197	85859	20197	26236
Contingency																	
Base Cost	7,876,459.4	4,875,246.6	12,751,706.0	418323	160152	467315	331416	829773	520769	1353813	912823	1967062	1266501	1791311	1008159	1048862	675426
IV. Price Contingency	2.281.201.8	1.779,755.1	4,060,956,9	38496	18101	69659	57746	159747	124372	333284	280200	594564	479805	646416	458389	442725	361141
V. Service Charge	218,907.1	134,183,2	353,090.3	2092	801	6520	3259	13005	7520	23923	14687	40528	25584	59319	36957	73520	45375
TOTAL COST	10,376,568.4	6.789.184.9	17.165.753.3	458911	179054	539804	392422	1002525	(522661	1711020	12021	2602154	1771890	2497046	1503506	1565108	1081942
VI. Annual O&M	0.0	229 721.6	229,721.6	0	0	0	0	0	0	•	0	0	33210	0	49710	0	146802
TOTAL	10,376,568.4	7,018,906.5	17,395,474.9	458911	179054	539804	392422	1002525	652661	1711020	1207711	2602154	1805100	2497046	1553215	1565108	1228744
FC: Foreign Currency, LC: Local Currency Development Area(ha)	Local Currency		115,600.0	0		0		0		6,800		38,100		80,300	. 8 :	115,600	8 (
Area Under Imigation(ha)			115,600.0	0		0		0		0		18,140	a	017/2	2	80,210	

Table H.3.13 Unit Cost of Labour and Construction Materials

				conent	Unit C		D
No. Item	Unit	Cost (Rs.)	F (%)	L (%)	F (Rs.)	L (Rs.)	Remarks
		(NS.)	(70)	(70)	(Ks.)	(13.)	
. Labour 1 Forman	man-day	150.0	0%	100%	0	150	(Normal Market)
2 Assist, forman/Semi-skilled	man-day	90.0	0%	100%	0	90	(unless otherwise)
3 Heavy equi, ope	man-day	200.0	0%	100%	0	200	(stated hereinafter
4 Assist. heavy equi. ope	man-day	80.0	0%	100%	0	-80	•
5 Dump truck driver	man-day	150.0	0%	100%	0	150	
6 Assist, dump driver	man-day	70.0	0%	100%	0	70	
7 Common driver	man-day	120.0	0%	100%	0	120	
8 Carpenter/Mason	man-day	130.0	0%	100%	0	130	
9 Bar bender(cut and bind)	ton	1050.0	0%	100%	0	1050	Local Style
10 Common labour(unskilled)	man-day	60.0	0%	100%	0	60	
. Construction Materials							
1 Aggregates and rock							
a) Sand(normal)	m3	60.0	20%	80%	12	48	Indus Riverbed
b) Sand(coarse)	m3	160.0	20%	80%	32	128	Panyala sand
c) Coarse Aggregate/Gravel	m3	250.0	30%	70%.	75	175	6-20mmdia
d) Rock, Gabion/Mason	m3	210.0	20%	80%	42	168	
e) Rock, Riprap	m3	120.0	20%	80%	24	96	
2 Lumber							
a) Plywood ,5mm	m2	165.0	30%	70%	49.5	115.5	WAPDA rate
b) Timber (Plank, 1"*12")	m	25.0	20%	80%	5	20	
c) Timer(Scaffolding,4")	m	35.0	20%	80%	7	28	
3 Rainforced from bar	ton	16500.0	60%	40%	9900	6600	
4 Portrand cement	ton	3300.0	40%	60%	1320	1980	
5 Fuel and Oil Product							
a) Gasoline	lit	14.4	50%	50%	7.2	7.2	
b) Diesel	lit	6.2	50%	50%	3.1	3.1	
c) Engine oil	lit	58.3	50%	50%	29.15	29.15	
d Bitumen 80/100	kg	11.0	50%	50%	5.5	5.5	
6 RC Pipe							
a) Dia. 6"	m ·	65.6	40%	60%	26.24	39.36	
b) Dia. 12"	m	164.1	40%	60%	65.64	98.46	
c) Dia. 18"	m	240.0	40%	60%	96	144	WAPDA rate
d) Dia. 24"	m	390.0	40%	60%	156	234	WAPDA rate
e) Dia. 30"	m	610.0	40%	60%	244	366	WAPDA rate
7 Steel							
 a) Steel Plate/products 	ton	17500.0	70%	30%	12250	5250	
b) Hand Rail	ton	20000.0	70%	30%	14000	6000	
c) Steel Pipe(600mm)	· m	75000.0	70%	30%	52500	22500	
d) Steel Pipe(4,000mm)	m	1500000.0	70%	30%	1050000	450000	•
8 Blasting	_			F0.04	20	20	377 A 17 M - L - 1/D
a) TNT,Dynamite	kg	76.0	50%		38	38	WAH Nobel(Pv
b) Detonator	рс	37.0	50%		18.5	18.5	WAH Nobel(Pv WAH Nobel(Pv
c) Fuse/Detonator Code	m	11.0	50%		5.5	5.5 1700	•
d) Drilling Rod	pc	3400.0	50%		1700		Foreign Market Foreign Market
e) Drilling Bit	pc	4600.0	50%	50%	2300	2300	roreign warker
9 Other	1.000	1000.0	0.01	100%	0	1000	
a) Brick	1,000pc	1000.0 16500.0	0%		6600		
b) Neoplene Bearing Pad	m2(t=30mm)	62.0	40% 50%		31		WAPDA rate
c) G.I. Pipe 1" dia	m	121.0	50% 50%		60.5	60.5	WAPDA rate
d) G.I. Pipe 2" dia	m 1.000ma	1500.0	30% 0%		0.00		
e) Roof Tile	1,000pc	6.0	0%		0		
f) Sod Glass	. m2	1000.0	0%		0		
g) Tree for Planting 10 Gate, Fixed Wheel Type(Co	1,000nos				U	1000	Official Linesof
Set of Gate	m2	87000.0	30%		26100	60900	Mech. Circle.lr
			30%	, 1070	20100	50700	moon, oneioni
11 Gate, Normal Slide Type &	MISHING ODEL	anni					Mech. Circle.In

Table H.3.14 Operation Cost of Construction Equipment

	Specifica	tions		eration Co		
No. Equipment	Spec.	PS	FC	LC	Total	Remarks
			(Rs./hr)	(Rs./hr)	(Rs./hr)	
A Earth Moving & Excavation						
1. Bulldozer ,D6	11t	160	733	78	811	e de la composition della comp
2. Bulldozer ,D7	17 t	220	917	112	1,029	
3. Bulldozer ,D8 with Ripper	21 t	290	1,653	154	1,808	
4. Tractor shovel	1.2m3	100	569	109		E/Market
5. Excavator	0.6m3	140	954	81		WAPDA
6. Excavator	1.2m3	210	1,804	112	1,916	E/WAPDA
7. Dredger of 1.2m3 backet	21 t	230	1,804	112	1,916	E/WAPDA
8. Tractor Trailer	5 t	60	140	30		Market
9. Wheel loader	2.2m3	160	914	94	1,008	WAPDA
10. Dump truck	11 t	210	660	130	790	WAPDA
11. Dump truck	20 t	290	1,265	178	1,443	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,
	201	2,70	1,200	170	1,110	
B. Compaction						
12. Tyre roller	11-17t	100	290	60		WAPDA
13. Tyre roller	21-31t	150	505	75		E/WAPDA
14. Tamping roller (C. dozer)	17t	150	1,070	104	1,174	N 442
15. Tamping roller (C. dozer)	21t	230	1,675	177	1,852	1000
16. Vibration roller	3t	25	339	43	382	•
17. Vibration roller	15t	160	1,359	109	1,468	
18. Sheep-Foot Roller	11-17t	100	400	85	485	E/WAPDA
Pneumatic Vibrater	•	•	45	5	50	Market
20. Tractor Water Bowser,4m3	5 t	60	150	30	180	Market
21. Water tank rolly, 10m3	11 t	210	600	75	675	WAPDA
22. Motor Grader of 3.7m brade	17 t	150	1,070	104	1,174	WAPDA
C. Other Equipment						
23. Normal Truck	10t	160	300	45	345	Market
24. Normal Truck	2 t	100	200	30	230	Market
25. Jeep,4-wheel drive	2 t	120	275	40	315	Market
26. Truck w/ 2t crane	10 t	160	350	45	395	E/Market
27. Tractor w/ 1t crane	5t	60	180	30	210	E/Market
28. Compressor	11m3	110	373	87	460	Lividiket
29. Compressor	17m3	190	549	130	678	
30. Concrete mixer, 1bag	0.16m3	-	330	30	360	Market
31. Concrete mixer, 3bag	0.5m3	_	600	50	650	E/Market
32. Truck Mixer(4m3)	11t	210	600	75	675	E/WAPDA
33. Batching Plant, 0.6m3	41kw	210	1,118	38	1,156	L/WAFDA
34. Generator	10kVA		1,116		200	Market
35. Generator	35kVA		320	75	395	Market
36. Crusher Plant,dia=600mm	45kw	_	465		503	Market
37. Secondary Crusher	0.5m3	-	110		125	Market
38. Fuel Bowser, 10m3	0.31113 11t	210	600		675	WAPDA
39. Low-bed Trailer	35t	320	1,200			
40. Bitumen Sprayer, Truck Mounted		210	600		675	
	111	-10	0.00	13	013	i warda
D. Boring, Blasting & Piling	0.01.					
41. Boring Machine	3.7kW	-	81		112	
42. Augar Machine	45kw	-	465			
43. Pile Driver	-	210	600			
44. Pneumatic Jack Hammer	2" bit		190			
45. Rock Drill			190	20	210	

Note: 1. Blank Remarks means etimated cost according to international price

^{2.} WAPDA: Price Listed from WAPDA 3. Market: Price quated or checked from Local Market

^{3.} E/Market(WAPDA): estimated from Local Market(WAPDA) Price of Equivalent or similar item

Table H.3.15 Summary of Unit Construction Cost (1/2)

No.	Description	Unit		Unit Price		Remarks
			Total	F.C.	L.C.	
A STANFORM	0.04					
and the second second	& Stonework		2.0	2.5	0.5	General
A1 Stripping of		m2	3.0	2.5		
1 2 2	ommon(Backhoe),Type 1	m3	43.6	i i	***	Main/Feeder Dist/Minor
and the second second	ommon(Backhoe),type 2	m3	47.4	38.8		Borrow/Wide
4.4	common(Composite),Type 1	m3	38.0			
	Common(Composite),Type 2	m3	33.4	26.6		Borrow/Narrow
A6 Excavation N		m3	25.2	0.0		General
	f Common Rock(ripper-dozer)	m3	107.1	92.8	14.3	
	or Structure(Small Backhoe)	m3	47.5	36.1	11.4	
A9 Excavation for		m3	38.0		4.2	
A10 Haulage of E	arth Material,type1	m3*km	5.8			by Dump-truck
	arth Material, type 2	m3*km	6. 1	3.8		by Tractor trail
A12 Embankmen	and Compaction, type 1	m3	39.6	33.9		Main/Feeder
A13 Embankmen	and Compaction, type 2	m3	41.6	33.6		Dist/Minor
A14 Embankmen	t and Compaction, Manual	m3	23.0	0.0	23.0	General
A15 Embankmen	t for Drain	m3	23.8	19.3	4,5	
A16 Backfill&Co	mpaction ,type 1	m3	50.7	43.3	7.4	Main/Feeder
A17 Backfill&Co	mpaction, Type 2	m3	52.5	42.6	9.9	Dist/Minor
A18 Sod Facing(t	urfing)	m2	11.0	0.0	11.0	
A19 Dredging W	ork	m3	61.9	55.7	6.2	by dredger
A20 Sand Filter		m3	263.1	36.6	226.5	Lining
A21 Loading or U	Inloading of Earth Material, 1	m3	18.6	14.6	4.0	Dump Truck
A22 Loading or U	Inloading of Earth Material,2	m3	20.3	0.0	20.3	Tractor
A23 Gravelling &	Compaction of sub-base	m2	20.5	7.5	13.0	Road
A24 Base Coarse	filling and Compaction	m3	330.3	110.8	219.5	Road
A25 Metalling an	d Tarring	m2	248.8	142.3	106.4	Road
A26 Wet Masonr	y	m3	1,651.4	557.8	1,093.6	with mortar
A27 Dry Masonr	y	m3	554.4	96.4	458.0	Stone-pitching
A28 Gabbion Ma	ttress	m3	732.2	259.1	473.1	slope protection
A29 Blick Masor	ıry	m3 -	1,350.0	250.0	1,100.0	Transition, wa
A30 Riprap		m3	222.0	26.0	196.0	slope protection
3 Concrete W	'ork					
B1 Concrete A	(140kgf/cm2,1:4:8)	m3	1,316.7	610.4	706.2	Foundation
B2 Concrete B	(170kgf/cm2,1:3:6)	m3	1,727.2	761.8	965.4	Lining,structu
B3 Concrete C	(210kgf/cm2 ,1:2:4)	m3	1,969.8	858.9	1,110.9	Structure
B4 Wooden For	m	m2	109.8	16.4	93.4	non-standard
B5 Reinforceme	ent Bar	kg	23.0	1	10.8	deformed bar
B6 Reinforced	Concrete Lining at Structure	m2	1,241.6	611.6	630.0	at Structure

F.C. : Foreign Currency Portion L.C. : Local Currency Portion

Table H.3.15 Summary of Unit Construction Cost (2/2)

			·····	· · · · · · · · · · · · · · · · · · ·	. (Unit : Pakistan Rs.
No.	Description	Unit		Unit Price		Remarks
			Total	F.C.	L,C.	
В7 Со	ncrete Lining,type 1	m2	290.0	119.0	171.0	Main/Feeder
B8 Lir	ning for Distributory	m2	193.2	72.7	120.5	Dist/Minor
B9 Ste	eel Slip Form	m2	32.7	7.2	25.5	for Lining
B10 Mc	ortar	m3	1,978.0	839.1	1,138.9	supplemental
B11 Ste	eel Form	m2	85.1	27.6	57.5	standard structure
B12 RC	Pipe, 300mm in dia.	m	284.1	125.6	158.5	mogha, causeway
B13 RC	C Pipe, 200/150mm in dia.	m	170.0	76.0	94.0	Sump Well
C Ot	her Work				* .	
C1 Ha	and Rail/Safty Ladder	m	120.0	63.0	57.0	Bridge/Ope. deck
C2 Tri	ush Rack	m2	1,050.0	600.0	450.0	
C3 Bu	uilding (Normal office or House)	m2	4,400.0	1,400.0	3,000.0	general
C4 Bu	uilding (Upgraded Office or House)	m2	6,000.0	2,000.0	4,000.0	for const. camp
C5 Bu	uilding (Pomp House)	m2	15,000.0	5,000.0	10,000.0	
C6 Re	eforestation	1,000nos	12,455.2	7,160.9	5,294.3	Along Canal
C7 Ste	eel Piling(dia=400 or H-350mm)	m	19,000.0	15,000.0	4,000.0	
C8 Sh	eet Piling	m2	4,300.0	2,900.0	1,400.0	·
C9 Bl	asting of Rock	m3	125.0	30.0	95.0	Blasting Only
C10 In-	-situ RC Piling(0.8m dia)	m	2,700.0	1,485.0	1,215.0	
C11 Ne	coprene Bearing Pad	m2	18,900.0	7,000.0	11,900.0	Bridge
C12 M	ild Steel Plate (t=10mm)	m2	1,932.0	1,176.0	756.0	operation deck
C13 Ov	verhead Crane	set	12,100,000.0	8,470,000.0	3,630,000.0	for pump house
C14 Ste	eel Pipe for Pump (4000mm in dia.)	m.	162,400.0	96,520.0	65,880.0	delivery pipe
C15 Ins	stallation of motor-operate-Roller Gate	m2	90,045.0	26,100.0	63,945.0	
C16 Ins	stallation of manual-operate-Slide Gate	m2	78,660.0	22,800.0	55,860.0	·

F.C.: Foreign Currency Portion L.C.: Local Currency Portion

Table H.3.16 Annual O&M Cost

Unit: 1,000Rs. Total **Project Cost Component** Foreign Cost Local Cost Public Expenses 174,260 96,070 270,330 59,130 (*) 59,130 **Administration Staff** 1,400 700 Office Operation Cost 700 O&M Cost for Pump Station 18,800 188,000 169,200 O&M Cost for Others 17,440 21,800 4,360 Associations' Expenses 4,990 18,000 22,990 Manpower(**) 23,760 23,760 Office operation 650 650 1,300 21,690 O&M Cost for facilities 4,340 17,350 **Total Cost** 179,250 114,070 293,320

^{*:} Annual Salary; 47,304 + Additional allowances; 9,461 + Duty travel 2,365

^{** :} Substituted by farmers' activity and not calculated for the total cost

Table H.3.17 Array for Operation and Maintenance

	Building	Staff			Equip			
Name of Office	-		Vechicles	Wireless		Office eq		
	(m2)	(persons)	(nos.)		Copy mch.		Fax	Phone
CRBDA Main Office	3150	210	30	[C.L.1]	4	8	9.	30
Pump Operation Office	260	26		[C.L.2]			11 -	. 1
Intake Operation Office	30	. 3		[C.L.2]				1
Feeder canal O&M Office (F-1)	50	8		[C.L.2]	-		1,	
Feeder canal O&M Office (F-2)	50	. 8		[C.L.2]		·	 ',	
Main canal O&M Office (M-1)	60	- 10	. 2	[C.L.2]				
Main canal O&M Office (M-2)	60	10	2	[C.L.2]		- 1	1000	
Main canal O&M Office (M-3)	60	10	2	[C.L.2]				
Main canal O&M Office (M-4)	60	10	1	[C.L.2]				
Main canal O&M Office (M-5)	60	10	1	[C.L.2]				•
Main canal O&M Office (M-6)	60	10	1	[C.L.2]	·			
Distributary O&M Office (D-1)	180	5	5 1	[C.L.3]		1		
Distributary O&M Office (D-2)	200		5 1	[C.L.3]		1		
Distributary O&M Office (D-3)	180		5 1	[C.L.3]	•	1		
Distributary O&M Office (D-4)	160) :	5 1	[C.L.3]		1		
Distributary O&M Office (D-5)	260) :	5 1	[C.L.3]		1		
Distributary O&M Office (D-6)	270) :	5 1	[C.L.3]		i		
Distributary O&M Office (D-7)	170) :	5 !	[C.L.3]	ļ	i		
Distributary O&M Office (D-8)	230) :	5 .	[C.L.3]	l	1		÷
Distributary O&M Office (D-9)	280) .	5	[C.L.3]	l	. 1		
Distributary O&M Office (D-10) 170)	5	[C.L.3]]	1		
Distributary O&M Office (D-11) 176)	5	[C.L.3])	1	:	
Distributary O&M Office (D-12	310)	5	1 [C.L.3])	1		
Distributary O&M Office (D-13		o	5	1 [C.L.3)	1		
Distributary O&M Office (D-14		0	5	[C.L.3		1	l	
Distributary O&M Office (D-15		0	5	1 [C.L.3	1	1	i	
Distributary O&M Office (D-16		0	5	1 [C.L.3	3	:	1	
Distributary O&M Office (D-17		0	5	1 [C.L.3		;	l	
Distributary O&M Office (D-18	•		5	1 [C.L.3]		l	
Distributary O&M Office (D-19		0	5	1 [C.L.3	3]		1	
Distributary O&M Office (D-20			5	1 (C.L.3			1	
Distributary O&M Office (D-2)	•		5	1 (C.L.3			1	
Distributary O&M Office (D-22	1		5	1 [C.L.3			1	
Distributary O&M Office (D-2)	•		5	1 (C.L.:			1	
Distributary O&M Office (D-2-	•		5	1 [C.L.:			1	
Distributary O&M Office (D-2		30	5	1 [C.L.:			1	
	8,9			54		4 3	3	9

Communication Line (1) [C.L.1]

: Pump Operation Office ~ Intake Operation Office

Communication Line (II) [C.L.2]

: Pump Operation Office ~ CRBDA Main Office

Communication Lines (III-1~25) [C.L.3] : each Distributary O&M Office ~ CRBDA Main Office

Table H.3.18 Number of O&M Staff by Grade

Name of Office				Grad	le		·	
	1~16	17	18	19	20	21	22	Total
CRBDA Main Office	139	23	22	22	2	. 1	1	210
Pump Operation Office	21	5						26
Intake Operation Office	3							3
Feeder canal O&M Office (F-1)	8							8
Feeder canal O&M Office (F-2)	8							{
Main canal O&M Office (M-1)	9.5	0.5						10
Main canal O&M Office (M-2)	9.5	0.5						10
Main canal O&M Office (M-3)	9.5	0.5						10
Main canal O&M Office (M-4)	9.5	0.5						10
Main canal O&M Office (M-5)	9.5	0.5						1
Main canal O&M Office (M-6)	9.5	0.5						10
Distributary O&M Office (D-1)	5							
Distributary O&M Office (D-2)	5							
Distributary O&M Office (D-3)	5							
Distributary O&M Office (D-4)	5							
Distributary O&M Office (D-5)	5							
Distributary O&M Office (D-6)	5							
Distributary O&M Office (D-7)	5							
Distributary O&M Office (D-8)	5							
Distributary O&M Office (D-9)	5							
Distributary O&M Office (D-10)	5							
Distributary O&M Office (D-11)	5							
Distributary O&M Office (D-12)	5							
Distributary O&M Office (D-13)	5							
Distributary O&M Office (D-14)	5							
Distributary O&M Office (D-15)	5							
Distributary O&M Office (D-16)	. 5							
Distributary O&M Office (D-17)	5							
Distributary O&M Office (D-18)	5							
Distributary O&M Office (D-19)	5							
Distributary O&M Office (D-20)	5							
Distributary O&M Office (D-21)	5							
Distributary O&M Office (D-22)	5							
Distributary O&M Office (D-23)	5							
Distributary O&M Office (D-24)	5							
Distributary O&M Office (D-25)	5							
Number of Staff	361	31	22	22	2	1	1	44
		17,000	21,000		34,000		_	-4-
Unit Salary by Grade (Rs./month) Annual Salary (,000 Rs.)	25,992	6,324	5,544	7,656	816	468		47,30

Table H.3.19 Summary for Operation and Maintenance Office

Distributary	Command Area L	ength of Disty Le	ngth of Minor F	lead Regulator M	linor Regulator	Mogha	Gata keeper	Ditch tender	Driver
O&M Office	(ha)	(m)	(m)	(Nos.)	(Nos.)	(Nos.)	(Persons)	(Persons)	(Persons)
D-1	1,700	4,000	0	1	0	24	11	3	1
D - 2	2,430	5,600	0	1	0	32	14	. 4	1
D - 3	2,390	10,200	0	. 1	0	24	11	7	1
D-4	1,260	3,700	0	1	0	9	6	3	1
D - 5	9,280	12,700	13,700	1	2	61	26	17	1
D-6	10,150	10,700	20,850	1	3	67	29	20	1
D - 7	2,410	9,900	0	1	0	17	9	7	1
D - 8	7,060	22,000	4,000	1	1	47	20	17	1
D-9	11,400	9,300	29,800	1	2	75	30	25	1
D - 10	2,380	16,700	0	1	0	17	9	11	1
D - 11	2,660	14,300	0	1	0	19	10	9	. 1
D - 12	13,620	22,500	57,700	1	6	90	39	50	. 1
D - 13	6,240	20,900	8,100	1	1	41	. 18	19	1
D - 14	3,630	9,800	5,000	1	1	26	13	10	
D - 15	1,860	9,900	0	1	0	13	. 8	. 7	. 1
D - 16	5,910	11,800	14,800	1	2	39	. 18	17	1
D - 17	9,150	15,100	13,000	1	1	60	24	18	. 1
D - 18	2,560	7,600	0	1	0	18	9	5	. ,
D - 19	2,560	9,400	0	1	0	18	9	. 6	:
D - 20	2,440	10,600	0	1	0	17	9	7	1
D - 21	3,650	10,600	0	1	0	26	12	7	
D - 22	3,250	7,000	0	1	0	23	11	5	
D - 23	1,180	5,700	0	1	0	8	6	4	:
D - 24	2,950	6,000	0	1	0	21	10	4	
D - 25	3,480	9,600	0	111	0	24		6	
		275,600	166,950	25	19	816	(372)*	(288)*	25

*: These duties will be carried out by farmers.

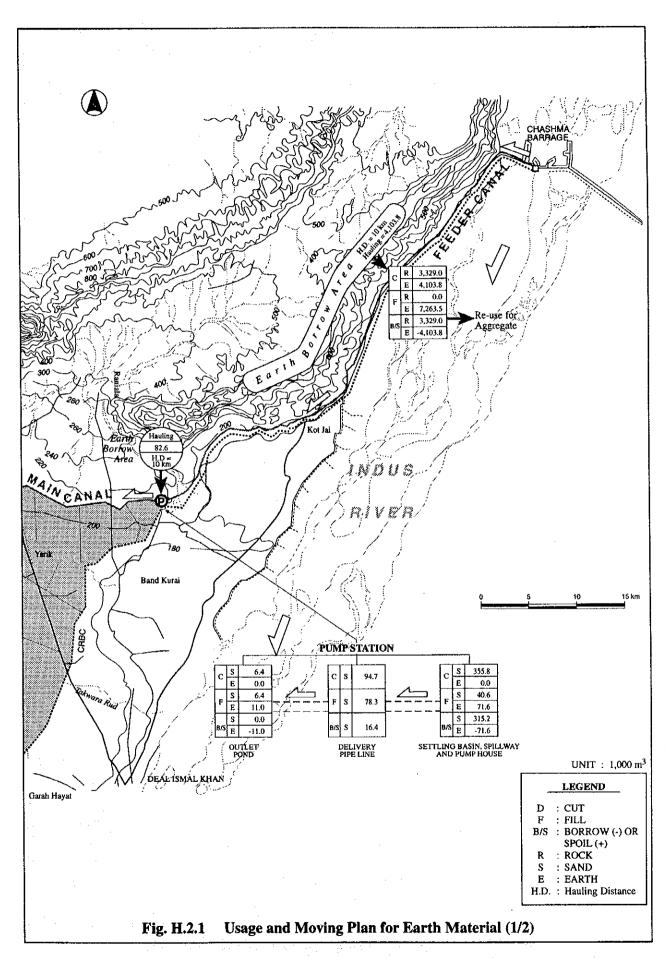
Main Canal	Length of Canal	Head Regulator C	ross Regulator	Canal tender	Gate Keeper	Driver
O&M Office	(m)	(Nos.)	(Nos.)	(Persons)	(Persons)	(Persons)
M - 1	15,000	3	1	-	-	1.5
M - 2	17,800	3	3	-	-	1.5
M - 3	23,750	6	.3	-	÷ .	1.5
M - 4	18,450	4	3	-	-	1.5
M - 5	19,300	4	4	-	•	1.:
M - 6	18,950	4	4	-	-	, 1.5
					•	9

Feeder Canal	Length of Canal	Canal tender	Driver
O&M Office	(m)	(Persons)	(Persons)
F - 1	27,360	•	
F - 2	31,205	_	-

Table H.3.20 Replacement Cost

			Cost Re	equired (1,000	Rs.)
	Item	NOS of	Foreign	Local	Total
	·	Structure	Currency	Currency	
Ī.	Pump station	·			
	(1 time at the middle of Project Life)				
	Pump Equipment	1 set	910,000	170,000	1,080,000
II.	Gates (1 times at the Middle)		31,500	74,000	105,500
II1	Radial Gate				
-	In take	1	4,000	10,000	14,000
П2	2 Roller Gate with counter weight		17,000	39,500	56,500
	Feeder Canal	1	2,000	3,500	5,500
-	Main Canal	23	15,000	36,000	51,000
II:	3 Slide Gate		10,500	24,500	35,000
	Main Canal	25	3,500	7,000	10,500
_	Regulating Pond	25	5,000	10,500	15,500
•	Distributory Canals	19	2,000	7,000	9,000
	Total Cost		941,500	244,000	1,185,500

FIGURES



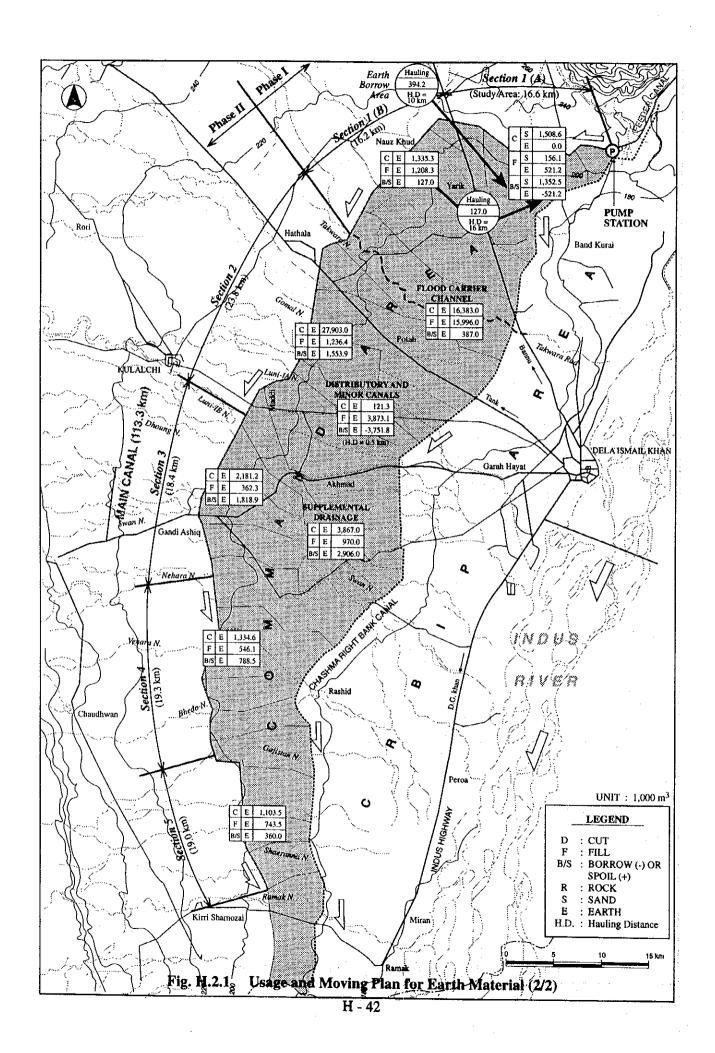


Fig. H.2.2 Implementation Schedule for the Project

Project Year Order		1st Year	2nd Year	3rd Year	4th Year	5th Year	6th Year	7th Year	8th Year
Project Preparatory Works									
1. Project organization set-up									
2. Financial arrangement	Phase I		Phase II						
3. Land acquisition									
Phase I (27,210 ha)		Detail Design		Construction	on Stage	Phase I)	A		
1. Intake structure			3	neugilig & Civi m	Name of Section	e de la companya de l			
2. Feeder canal			Section 1990			► Installatin of Pumps		-	
3. Pump station				A TOTAL SELECTION OF THE SELECTION OF TH	a programma de la companya del companya de la compa				
4. Main canal and disty (D-1 ~ D-6)				Court salegoethers, Horkey Belling on the	miragina ekera jiga Jawayi Kiringgar	op iii dayya iii o			
5. Drainage (I)						englistika maja gjad mekketikiti	S Total Same		
6. On-farm and farm road (I)			-				e programme a constant		
7. Other facilities	Cami	Camp Construction ——		Co. Salar Sa	A CONTRACTOR OF STREET OF STREET	The second secon	a de la companya de l		
8. FA set-up assistance		Атапдете	A SECULIA	A STEEL CONTROLLED STORY STORY STORY	execution to the second second second	William St. Commonweal St. Common	1 2 20 20 20 20 20 20 20 20 20 20 20 20 2		
9. Procurement of Equipment) }	Vehicle & Office Equipment		North Edministra		A SAME AND A SAME AND			
-Developped Area by the Project(ha)					6,800	18,140	27,210	27,210	27,210
-Area Under Irrigation Practice(ha)						18,140	27,210	27,210	27,210
Phase II (88,390 ha)				Detai	Detail Design		Construction Stage (Phase II)	ige (Phase II)	
1. Pump station							Instantation		
2. Main canal and disty (D-7 ~ D-25)						The second secon	A CONTRACTOR OF THE PROPERTY O	A CONTRACTOR OF THE STATE OF TH	
3. Drainage (II)						Mile of the state	on may of Migration scale in State	The second secon	4
4. On-farm and farm road (II)						A Company			
5. Other facilities				Δ	Arrangement			Kiddle Assessment on the Sawa	
6. FA set-up and assistance				*	- Remem	A printed in the printed in the printed of the second		N. 30	
-Developped Area by the Project(ha)						19,960	53,090	88,390	88,390
-Area Under Irrigation Practice(ha)								53,000	88,390

Note: Other facilities comprise O&M, FA offices, support facilities, etc.

ANNEX I

PROJECT EVALUATION

ANNEX I

PROJECT EVALUATION

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ANNEX I PROJECT EVALUATION

I.1 Introduction

The results from the assessment of alternative pump irrigation and agricultural plans lead to the selection of proposed 60 feet lift irrigation plan covering canal command area of 115,600 ha with a cropping intensity of 160%. In this Annex-I, overall project evaluation is discussed in detail for this selected plan.

Chashma Right Bank 1st Lift Irrigation Project primarily aims at increasing agricultural productivity by irrigation and drainage development, organization of self-reliance farmer organizations, and provision of adequate agricultural support services provision, and re-organization of provincial government institutions. The Project has other significant impact on the rural community taking the present less development progress of the area into consideration. The rural village road is poor and hinterareas from rural road are hard to access after occasional rains. The villagers face difficulty to get municipal water throughout a year. Some villages become vacant in the dried and hot season due to lack of water. Evaluation of the Project, hence is required to count other development effects beside crop production benefit.

The project evaluation involves making an assessment of project feasibility in view of economic, financial and socio-economic aspects. The economic feasibility is firstly evaluated by calculating the economic internal rate of return (EIRR) and sensitivity analysis against possible adverse changes in the future. Financial evaluation is carried out by analyzing the effect to farm budgets and requirement of O&M and replacement costs according to the typical crop rotation systems. The social impact of the project is also briefly studied.

I.2 Economic Evaluation

I.2.1 Basic Assumption

The basic assumptions applied for economic evaluation of the Project are summarized as follows:

- (1) The economic useful life of the Project is 50 years,
- (2) All prices are expressed at March 1994 prices in Rupee,
- (3) The exchange rate of US\$ 1.00 = Rs. 30.0 = Yen 107.1 as of average during July to March, 1993/4 is applied,
- (4) A standard conversion factor (SCF) with 0.9 is applied to domestic cost

elements such as transport, handling and processing for estimation of economic value, and

(5) The transfer payment such as tax, duty, subsidy and interest are excluded for the estimation of economic costs and prices,

I.2.2 Crop Production Benefit

(1) Economic Prices of Agricultural Outputs and Inputs

Economic prices of farm inputs (urea, triple super phosphate and muriate potash) and tradable farm produce (wheat, maize, sugar cane, cotton, cotton seed, and sunflower) are estimated on the basis of IBRD projection of world market prices for 2005 in constant 1994 terms. Economic prices of other non-tradable farm outputs (pulses, fodder, oilseeds, fruits, vegetables, by-products) and farm inputs (seed/seedling, machinery) are set at same financial prices.

Assuming an opportunity cost (shadow wage rate) for farm labor of 0.75 during the lean labor demand period of 6 months (June to August and November to January), the weighted economic farm labor price is estimated at Rs. 44/day against the financial price of Rs. 50/day. Economic prices of farm inputs and outputs are shown in Table I.2.2.1 to I.2.2.7 and summarized in Table I.2.2.8.

(2) Crop Production Benefit

Crop production benefit could accrue from the pumping irrigation water supply to rainfed and barani area, organized irrigation activities by farmer associations, and improvement of farming practices and productivity. Livestock production benefits are indirectly estimated through the value assessment of fodder crops and by-products of other crops.

Economic crop production budgets per ha both for irrigated and barani/rainfed conditions are prepared under Without (WO) and With (W) project conditions on the basis of farm input requirement, present and future yields, and economic farm gate prices of farm inputs and outputs. Economic net crop production value (ENCPV) per ha under (WO) and (W) project conditions is estimated on the basis of the present and proposed cropping patterns and cropping intensities as shown in Table I.2.2.9 and summarized as follows. Incremental crop production benefit will be expected to increase year by year after the completion of development according to the implementation schedule. It is assumed that the built-up period to achieve full benefit is seven (7) years after the completion of physical works (first year 50%, second

65%, third 75%, forth 85%, fifth 90%, sixth 95%):

	Cropping	Econo	mic Net Crop Pro	oduction
Item	Area (ha)	Total (Rs.'000)	Per Cropping Area (Rs./ha)	Per Project Area (Rs./ha)
1. Without Project Condition (WO)	20,470	86,540	4,228	749
2. With Project Condition (W)	170,500	2,592,619	15,206	22,428
3. Incremental (W) - (WO)	150,030	2,506,080	10,978	21,679

I.2.3 Other Development Benefits

(1) Farm road development benefit

Farm road of 555.9 km along the main, distributary and minor canals and 32.5 km connecting between canals and villages will be constructed under the Project. The road density will increase from the present 129m/km² to 702m/km². Traffic volume covering the future crop production, required farm inputs and consumer goods are estimated on the basis of the future crop production plan and crop budgets. Transport cost savings between the future (WO) and (W) project conditions are assessed by mode of transportation (vehicle and animal) and estimated at Rs. 92/ton per 6 km which is average distance from farm or farm gate to local markets. The farm road development benefit is estimated at Rs. 224, 154 thousand including cost saving for passenger traffic which is assumed at 10% of the commodity traffic as follows (Ref. Table I.2.3.1):

1. Quantity of Commodity Traffic ('000)	2,461.1
Crop production	2,303.3
Farm inputs	143.5
Consumer goods	14.3
2. Financial Benefit (Rs.'000)	249,060
Commodity traffic	226,418
Passenger traffic	22,642
3. Economic Benefit / (2) x 0.9 (Rs. 000)	224,154

(2) Benefit from transfer of Rod Kohi water right to upper hill torrent

Rod Kohi is an unique system of irrigation in which the hill torrent (nullah) water is diverted and spread into the fields for cultivation. It is a form of water harvesting technique. Hill torrent water is diverted and spread into the fields for cultivation. There are 25 nullahs (hill torrents) in D. I. Khan division to irrigate the area. For

diverting water from these nullahs into the field, big dams called Sads are constructed in their beds. Then there is a distributary channel called Kinda from a dam. After that another channel called Khula, Munha or Kas takes the water to different fields which are embanked and called Bandra.

The general rule of rights is that known as the "Saroba Paina". This means that the man at the head of the stream has the first right to use as much water as he requires for his cultivated land, and when he has taken this, it is his duty to clear the bed of the stream and allow the water to pass on to the village next below and so on to the tail of the stream. There are some cases where the area irrigable at the head of the stream is so large that if it is all irrigated, the lower villages will get no water. In such cases, limits of area have been fixed, beyond which the **Saroba** lands (areas at the head of a Nullah) are not allowed to irrigate until the **Paina** villages (areas at the tail of a Nullah) have irrigated an area similarly fixed. The Deputy Commissioner (Collector) has the powers on all "Rods" to cut a dam as soon as the land belonging to it have been irrigated or as soon as the water begins to run from it into the ravines.

After the commissioning of 1st lift irrigation canal, 27,100 ha of Rod Kohi area in the Project area will come under the irrigation. This area will no more require flood water irrigation. Therefore, the water rights in the CCA area will be transferred and reallocated to the non-command area. Re-allocation of water rights will be ordered by the Deputy Commissioner keeping in view the cultivated area of the villages above the command area of 1st Lift canal. The upstream dams will be further improved and strengthened to divert larger quantities of floods water to the increased area brought under cultivation according to re-allocation of the water rights.

Benefit accrued by the transfer of Rod Kohi water right to the upper stream is considered as the net production value in the present Rod Kohi area under the Project area. Out of the Rod Kohi area, 10,700 ha is harvested in a normal year and produces the economic net production value of Rs. 43,729 thousand. The transferable value is assumed at 60% of the total value, hence the annual water right benefit will be Rs. 26,237 thousand (Ref. Table I.2.3.2).

(3) Water resource development benefit for domestic water supply (Domestic water supply benefit)

Domestic water supply facilities in the Project area are less developed and their water quality, especially shallow tube well (less than the depth of 91.4 m), is usually saline. Actual beneficiary population by the tube wells is limited at around 25% in 104

mouzas concerned the Project area. Utilization of irrigation canal water as domestic water is prevailing in the villages under the gravity irrigation system. This phenomena will accrue in the Project area after the commencement of lift canal irrigation. The water quality of canal water is more suitable than those saline tube well water if the water is properly treated, hence irrigation water supply is considered as water resource development for domestic water supply in the Project area.

The water resource development benefit for domestic water supply through the irrigation development accounts for alternative development cost for deep tube wells and their O&M cost including domestic water charges. It is assessed that another 66 deep tube wells will be required covering the rest of non-beneficiary population of around 92,100. Total economic domestic water supply benefit is estimated at Rs. 42,754 thousand (Ref. Table I.2.3.3).

(4) Benefit from reduction of seasonal migration mainly due to lack of water, no farming activities, and no feed for livestock

Based on the farm survey, around 18% of household in the Project area migrate to other area during March to June, when are most dried and hot season, mainly due to shortage of water and feed for livestock. Migrants spent additional livelihood and feed expenditures which were estimated at Rs. 9,902 thousand/year. After the Project completion, these cost will be never required for the villagers and be considered as the Project benefit. (Ref. Table I.2.3.4).

(5) Environmental improvement benefit

The survey on villagers' concerns on environmental conditions and residential land prices was implemented in the developed area (stage I area), newly developed area (stage II area) under the gravity irrigation system, and the Project area during July to August in 1994. The total sampling number became 126 households after excluding the samples with abnormal land prices. As for the environmental factors, five major items covering water supply, road/accessibility, medical care, education and communication, which were considered major elements for village livelihood improvement, were evaluated by the respondents' scoring from zero (0) to five (5). There was significant correlation between the sum of environmental scoring of the respective respondents and their residential prices as follows (Ref. Fig.-I.2.2.1 and 2.2.2):

Polynomial Regression; $Y = 989.7 - 229.6 X + 20.8 X^2$

X = Environmental Factor (sum of scoring for 5 items)

Y = Residential land price (Rs. 000)

R-squared = 0.958

The environmental improvement is hard to assess in terms of monetary value. The residential land prices were obviously reflected by the villagers' evaluation on the environmental factors covering the above five items. It is concluded that the difference of residential land prices between the Project area and the stage II area (Rs. 11,000/ha) is environmental improvement value by the Project. The environmental benefit is estimated at Rs. 44,000 thousand covering the future residential area of 4,000 ha as follows:

Area	Environmental Factors	Residential Land
	(Average Sum of Scoring Number)	Price (Rs.'000)
Project Area	6.3	371
Gravity Irrigation Area		
Stage II Area (Newly developed)	6.6	382
Stage I Area (Developed)	14.2	1,932

I.2.4 Economic Cost

The financial costs for the construction components are grouped into two parts of local and foreign costs. The local cost comprises three (3) items such as transfer payment, unskilled labor cost, and other costs for material and skilled labor. Construction Conversion Factors (CCFs) that are the weighted average of the respective cost items by applying other conversion factors are estimated as the following procedure (Ref. Table I.2.4.1):

- (1) Financial foreign cost accounts for the economic cost,
- (2) Transfer payment in the local cost at the rate of 10% is excluded from the financial cost,
- (3) The rest 90% of financial cost is split into unskilled labor and other costs,
- (4) The part of unskilled labor is converted to the economic value applying the conversion factor of 0.88,
- (5) The standard conversion factor of 0.9 is applied for the conversion of other costs, and
- (6) The CCFs by the project components are calculated as the sum of economic

shares by cost items after the conversion of those financial shares.

Conversion factor for O&M cost is estimated same as the above procedure. Replacement costs are converted applying the related CCFs to the financial costs. The economic project costs are estimated as follows (Ref. Table I.2.4.2):

	Financial	Economic
Item	Cost	Cost
	(Rs. 000)	(Rs.'000)
1. Project Cost	12,751,706	11,727,250
Construction	10,120,402	9,292,282
Engineering/Administration	1,619,264	1,498,831
Physical contingency	1,012,040	936,137
2. Annual O&M Cost	<u>317,080</u>	<u> 266,664</u>
3. Replacement Cost		
Pump	1,080,000	1,042,200
Gate	105,500	91,363
Others	4,655	4,747

I.2.5 Economic Evaluation

Economic evaluation is made through the estimation of (i) Economic Internal Rate of Return (EIRR), (ii) Net Present Value (NPV) and (iii) Benefit-Cost Ratio (B/C) both at the discount rate of 10% as shown in Table I.2.5.1. The project benefits on farm road development, water right transfer, domestic water, reduction of migration and environmental improvement are included in the crop production benefit one by one for the evaluation as follows:

Item	Crop	Farm	Water	Domestic	Migration	Total
•	Production	Road	Right	Water	•	
		+(A)	+(B)	+(C)	+(D)	
	=(A)	=(B)	=(C)	=(D)		
1. EIRR	13.6	14.7	14.8	15.0	15.1	15.3
2. NPV(Rs. Million)						
- Benefit	12,505	13,624	13,755	13,969	14,018	14,238
- Cost	9,066	9,066	9,066	9,066	9,066	9,066
3. B/C	1.38	1.50	1.52	1.54	1.55	1.57

In order to evaluate soundness of the project against possible adverse changes in the future, sensitivity analysis is made for the following cases:

Item	Crop Production	Farm Road	Water Right	Domestic Water	Migration	Total
		+(A)	+(B)	+(C)	+(D)	
: 	=(A)	=(B)	=(C)	=(D)		
1. Project cost overrun by 20%	11.8	12.7	12.8	13.0	13.1	13.2
2. Benefit decrease by 20%	11.0	12.0	12.1	12.3	12.3	12.5
3. Delay in construction for 2 year	s 11.2	12.0	12.0	12.2	12.2	12.4
4. Case 1 and 2	9.5	10.3	10.4	10.6	10.6	10.8
5. Case 1 and 3	9.8	10.5	10.6	10.7	10.8	10.9
6. Case 2 and 3	9.2	9.9	10.0	10.1	10.2	10.3
7. Case 1, 2 and 3	7.9	8.6	8.7	8.8	8.9	9.0

I.3 Financial Evaluation

In order to evaluate the Project from the financial aspect of the farmers, the farm budget analysis on different sizes of farms are made under the representative crop rotation systems in the future (W) project condition as follows:

Years / Seasons	Type -I	Type-II	Type-III
Ist Year			
Rabi	-	Wheat	Fodder
Spring	Maize	-	
Kharif	Fodder	Maize	Sugarcane (September)
2nd Year		* *	
Rabi	Wheat	Oilseeds	Sugarcane (ratoon)
Spring	. - .		- do -
Kharif	Maize	Maize	- do -
3rd Year			
Rabi	Wheat	Wheat	
Spring	-	-	Maize
Kharif	Cotton	Maize	-

For the assessment of farmers' capacity to pay by the respective farm budget surplus (balance of gross income and expenditure covering non-farm expenditures), water charges and replacement costs for the Project are estimated on the basis of crop water requirement to the total O&M costs from the intake to on-farm level and replacement facilities (pumps are excluded due to their heavy burden to the farmers) as shown in Table I.3.1.1. The replacement cost will be collected from the farmers according to the crop water requirement and amortized in 25 years by the interest rate of 10 %/year. The assessment results are shown in Table I.3.1.2 and summarized as follows:

Item	Small	Medium	Large	Average
	(2.31 ha)	(4.70 ha)	(18.89 ha)	(12.94 ha)
I. Crop Rotation-I (Maize-Fodder-Wheat-Ma	ize-Wheat-Cotte	on)		
a) Farm Budget Surplus	15,680	39,550	208,040	138,390
b) Water Charge	4,750	9,670	38,870	26,620
c) Replacement Charge	1,370	2,780	11,170	7,650
Share of (b+c) to (a)	(39%)	(31%)	(24%)	(25%)
II.Crop Rotation-II (Wheat-Maize-Oilseeds-M	laize-Wheat-Ma	ize)	•	
a) Farm Budget Surplus	24,580	57,650	280,810	188,240
b) Water Charge	5,250	10,670	42,890	29,380
c) Replacement Charge	1,510	3,070	12,320	8,440
Share of (b+c) to (a)	(28%)	(24%)	(20%)	(20%)
III.Crop Rotation-III (Fodder-Sugarcane-Sugar	rcane-Maize)			
a) Farm Budget Surplus	11,600	31,320	174,620	115,500
b) Water Charge	8,040	16,360	65,740	45,030
c) Replacement Charge	2,310	4,700	18,890	12,940
Share of (b+c) to (a)	(89%)	(67%)	(48%)	(50%)

Based on the assessment on the future farm budget surplus, water charges and replacement cost, the followings could be clarified:

- (1) Crop rotation-II is most suitable for small scale farmers holding 2.3 ha and the water charges and replacement costs will be within 30% of the future farm budget surplus,
- (2) Crop rotation-I and II are economical for medium scale farmers with 4.7 ha and the water charges and replacement costs will be around 30% of the future farm budget surplus,
- (3) Crop rotation-III including sugarcane production is not economically suitable for any scale of farmers due to the high water consumption of sugarcane compared to its low profitability. Sugarcane production in the Project area could be manageable by the commercialized farming through reduction of crop production costs beside the water charges, and
- (4) Appropriate crop rotation systems according to the scale of farms should be introduced and rational irrigation water allocation not only at the distributary and water course levels but also among the farmers be practised.