B-2.2 Rainfall Analysis

(1)Design Rainfall

The design rainfall will be determined by examining the following items in order:

- Rainfall Probability;
- Correlation among the Stations; and
- Estimates of Areal Average Rainfall.
- Pattern of Hyetograph

(a)Rainfall Probability

The probability analysis for rainfalls have been made by Iwai method to calculate the probability of exceedance in the logarithm normal distribution, as follows:

Table B-2.3 Rainfalls Probable for Each Return Period

[YEN PHONG Station] Period: 1960~1992 (Data missing in 1982)

		Return Period (year)								
Consecutive D	2	3	5	1 0	1 5	2 0	3 0	4 0	5 0	100
Daily Max. R.	118.3	136. 4	157.6	185.4	201.7	213.3	229. 9	241.8	251.1	280.8
2-day Conse. R	154.6	179. 9	208.4	244.5	265. 1	279.6	299. 9	314.4	325.6	360.8
3-day Conse.R	164.4	190.3	219.9	257.7	279.3	294.6	316. 2	331.6	343.6	381.2
4-day Conse. R	177.6	205.6	237. 7	279.0	302.7	319.5	343. 3	360.4	373.6	415.4
5-day Conse. R	191.1	219.7	252. 9	296. 2	321.3	339. 2	364. 7	383. 0	397. 2	442.6

[DONG ANH Station] Period: 1962~1992 (Data missing in 1965~1969)

		Return Period (year)								
Consecutive D	2	3	5	1 0	1 5	2 0	3 0	4 0	5 0	100
Daily Max. R.	114.3	136.5	164.5	204.2	228. 8	246.9	273. 5	293. 2	308.9	360.7
2-day Conse.R	147.9	176. 4	213.9	269.5	305.1	331.8	371.8	401.9	426.1	507.6
3-day Conse. R	167.2	199.6	239.9	296.3	330.9	356.3	393. 4	420.8	442.5	513.7
4-day Conse. R	182.1	219.7	264. 1	323.0	357.7	382.5	418.1	443.8	463.9	528. 4
5-day Conse. R	196.0	233. 9	279.8	342.4	379.9	407.1	446. 5	475.3	497.9	571.3

Table B-2.3 Continued

[YEN VIEN Station]

Period: 1962~1992 (Data missing in1990)

	Return Period (year)								din janga Ngarangan	
Consecutive D	2	3	5	10	1 5	2 0	3 0	4 0	5 0	100
Daily Max. R.	129.3	151.0	176.6	210.8	230. 9	245.3	266. 0	281.0	292.7	330. 4
2-day Conse. R	161.3	194.5	235. 3	291.8	326.0	351.0	387. 3	413.9	435.0	503.7
3-day Conse.R	175.6	211.8	255.6	315.4	351.4	377.4	415.1	442.7	464.3	534.7
4-day Conse. R	191.7	228.5	272. 2	330. 4	364.8	389.6	425.0	450.7	470.8	535.5
5-day Conse. R	208.9	249.9	297.5	359.4	395. 3	420.9	457. 2	483.4	503.7	568.3

[BAC NINH Station]

Period: 1960~1992

	Return Preiod (year)										
Consecutive D	2	3	5 .	1 0	1 5	2 0	3 0	4 0	5 0	100	
Daily Max. R.	119.4	136.5	155.4	178. 7	191.7	200.8	213.4	222. 3	229.1	250. 4	
2-day Conse. R	151.4	175.1	201.1	233. 0	250.7	263. 1	280. 2	292. 2	301.4	330. 1	
3-day Conse.R	165.9	189.6	215.0	246.0	262. 3	273.9	289.8	300.9	309.3	335. 4	
4-day Conse. R	179.1	204.5	230.8	261.2	277. 4	288.3	303.1	313.3	321.0	344. 3	
5-day Conse.R	194.0	222. 3	252.0	286. 9	305.6	318.4	335.8	347. 9	357. 0	384.8	

[QUE YO station]

Period: 1960~1992

	Retun Period (year)									
Consecutive D	2	3	5	1 0	1 5	2 0	3 0	4 0	5 0	100
Daily Max. R.	124. 1	143.3	165. 2	193. 2	209. 2	220.6	236. 6	248. 0	256.8	284.8
2-day Conse.R	151.6	176. 2	203.3	236.8	255.5	268.5	286. 6	299.3	309.1	339.6
3-day Conse.R	163.3	190.1	219.2	254.5	274. 0	287. 4	306. 0	319.0	329.0	359.7
4-day Conse. R	175.4	202.9	232. 2	267. 3	286. 4	299.5	317. 6	330. 1	339.7	369.1
5-day Conse.R	192.0	221.4	251.8	286. 9	305.5	318.1	335. 2	347.0	355.9	382.7

Table B-2.3 Continued

[TU SON Station]

Period: 1960~1981

	Return Period (year)									
Consecutive D	2	3	5	1 0	1 5	2 0	3 0	4 0	5 0	100
Daily Wax. R.	120.5	135. 9	152. 1	171.5	182. 0	189.1	199.0	205.8	211.0	226.9
2-day Conse. R	151.4	170.3	190.3	214.0	226. 9	235.6	247. 6	255.9	262. 2	281.5
3-day Conse. R	164.5	184.8	206.3	231.8	245.6	255.1	268. 0	277. 0	283.8	304.6
4-day Conse. R	174.3	195.8	218.0	243.8	257.5	266.8	279. 4	288. 1	294.7	314.6
5-day Conse. R	186.7	209.0	231.6	257.3	270.7	279.7	291.8	300.1	306.3	325.0

(b) Rainfalls Correlation Among Stations

The rainfalls are poorly correlated among the rainfalls observed at the respective stations, as disclosed in Table B-2.4 The correlation coefficient was 0.60 in the highest.

Table B-2.4 Correlation Coefficient Among the Stations

Daily Rainfall

Sta.	YEN PHONG	DONG ANH	YEN VIEN	BAC NINH	QUE YO	TU SON
YEN PHONG		0.684	0.626	0.536	0.341	0.686
DONG ANH	0.606		0.706	0. 452	0. 355	0.399
YEN VIEN	0.599	0.774		0.548	0. 281	0.481
BAC NINH	0.462	0.453	0.618		0.623	0.331
QUE VO	0.412	0.445	0.313	0.620		0.278
TU SON	0.738	0.514	0.666	0.393	0.391	

Table B-2.4 Continued

3 Days Consecutive Rainfall

ali	Sta.	YEN PHONG	DONG ANH	YEN VIEN	BAC NINH	QUE YO	TU SON
Kainiali	YEN PHONG		0.614	0.584	0.567	0.443	0.717
	DONG ANH	0.644		0. 779	0.452	0. 455	0.543
consecutive	YEN VIEN	0.643	0.770		0.615	0.408	0.605
	BAC NINH	0.598	0.471	0.606		0.619	0. 429
nays	QUE YO	0.525	0.518	0.420	0.645		0.389
ť	TU SON	0.769	0.566	0.605	0.374	0.376	

5 Days Consecutive Rainfall

Sta.	YEN Phong	DONG ANH	YEN YIEN	BAC NINH	QUE YO	TU SON
YEN PHONG		0.626	0.627	0.645	0.456	0.724
DONG ANH			0.738	0.469	0.510	0.624
YEN VIEN				0.511	0.346	0.552
BAC NINH		,			0.627	0.517
QUE VO		1 1 2				0.474
TU SON						

(c)Estimates of Areal Average Rainfall

Thiessen method has been selected to estimate the areal average rainfall among the various method, such as Thiessen method, Isohyetal method, Representative rainfall method, etc., because of nearly flat topography in the study area and poor correlation of rainfalls among the stations.

Thiessen polygon has been drawn up, as shown in Figure B-2.3 and Thiessen coefficient for each station are calculated as follows:

Table B-2.5 Thiessen Coefficient for Each Station

Rainfall	YEN	DONG	YEN	BAC	QUE	TU
Station	PHONG	ANH	YIEN	NINH		SON
Thiessen C	0. 0559	0. 0000	0. 0390	0.3112	0. 3175	0. 2764

By applying the above coefficients to the rainfalls for each station, the areal average rainfalls for the study area have been computed as tabulated in Table $B-2.6\,$

Table B-2.6 Areal Average Rainfalls

Concount:	Return Period (year)										
Consecutive Rainfall Days	2	3	5	1 0	1 5	2 0	3 0	4 0	5 0	100	
Daily Max. R.	121.5	139. 1	158.5	182.9	196.7	206.3	219.8	229.3	236.6	259.6	
2-day Conse. R	152.0	175, 1	200.6	231.9	249.4	261.6	278. 5	290.4	299.6	328. 2	
3-day Conse.R	165.0	189.3	215.8	248.1	265.8	278. 2	295. 3	307.3	316.5	344.9	
4-day Conse. R	177.0	202.6	229.7	262. 0	279.6	291.6	308. 2	319.7	328.5	355. 4	
5-day Conse.R	191.8	219.3	248. 1	282. 1	300. 3	312.8	329. 8	341.6	350.6	378.0	

Figure B-2.6 Thiessen Polygon Map

(d)Pattern of Hyetograph

The consecutive days for the design rainfall applies 3 days on condition that any rainfall losses would not be claimed in the runoff analysis. As for the hyetograph daily pattern, three (3) type in the location of the peak/biggest rainfall, the first day, middle day and last day, has been worked out and looked of the actual rainfall pattern. The frequency of each pattern are tabulated as follows:

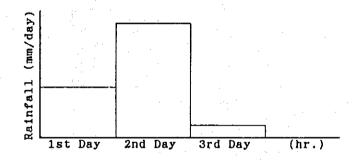
Table B-2.7 Frequency of Hyetograph Daily Pattern

	Peak in F	irst Day	Peak in I	Middle Day	Peak in Last Day		
Station	1-2-3	1-3-2	2-1-3	3-1-2	2-3-1	3-2-1	
YEN PHONG	2	4	4	11	0_	11	
DONG ANH	3	2	7	4	3	7	
YEN VIEN	3	1	4	7	1.	14	
BAC NINH	2	5	6	7	4	9	
QUE VO	2	3	7	9	1	11	
TU SON	4	1	3	6	3	5	
TOTAL	16	16	31	44	12	57	

Note: 1-2-3 express the biggest, second and third intensity of rainfall among consecutive rainfall for 3-day

As shown in the above table, the daily pattern of hyetograph has been applied to the middle in peak type (2-1-3) which is of frequent occurrence and the same hyetograph pattern brought about the heavy flood damages in 1985, as follows:

Figure B-2.7 Design Daily pattern of Hyetograph



where: First day rainfall = probable consecutive 2 days rainfall - Probable daily max. rainfall;

Second day rainfall = Probable daily rainfall;

Third day rainfall = Probable consecutive 3 days rainfall - Probable consecutive 2 days rainfall.

The hyetograph of hourly rainfall is hardly concluded based on the actual records because of data unavailable. As the rainfalls in the study area are general-rainfalls are computed by applying Sherman type, as expressed below:

$$Rt = \left(\frac{R24}{24}\right) \cdot \left(\frac{24}{t}\right)^n$$

where: rt = Hourly average rainfall intensity (mm/hr) R24 = 24 hour rainfall (mm) n = Factor (1/2 - 2/3)

Table B-2.8 shows the computed hourly rainfall by applying n=0.6.

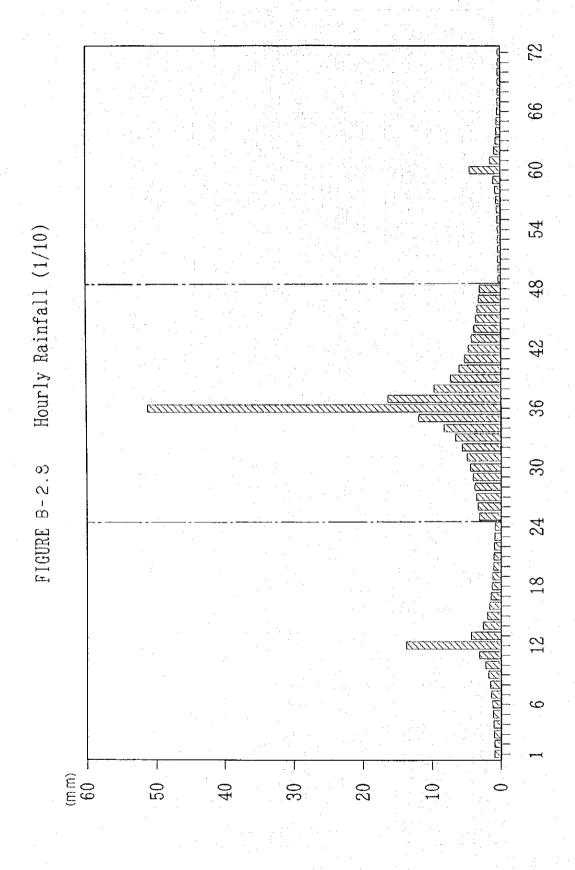
Table B-2.8 Computed Hourly Rainfall Hyetograph

. <u> </u>	<u> 2011 - 2014 - </u>	<u> </u>	And the second		
1		For	First Day		
t	(24/t)**0.6	r t	r t*t	R	r
1	6.7317	13.744	13.744	13.7	0.8
2	4. 4413	9.068	18.135	4.4	0.9
3	3. 4822	7.109	21.328	3. 2	1.0
4	2. 9302	5.982	23.930	2. 6	1.0
- 5	2. 5630	5. 233	26.164	2. 2	1.1
6	2. 2974	4.691	28.143	2.0	1.2
_ 7	2.0944	4. 276	29.933	1.8	1. 3
8	1.9332	3.947	31.575	1.6	1.5
9	1.8013	3.678	33.099	1.5	1.8
10	1.6909	3.452	34.523	1.4	2. 2
11	1.5969	3.260	35.865	1.3	3. 2
12	1. 5157	3, 095	37.135	1.3	13.7
13	1. 4446	2. 949	38.343	1. 2	4.4
14	1. 3818	2. 821	39.497	1.2	2. 6
15	1.3258	2.707	40.602	1.1	2.0
16	1. 2754	2.604	41.664	1.1	1.6
17	1. 2299	2.511	42.687	1.0	1.4
18	1.1884	2. 426	43.674	1.0	1.3
19	1. 1505	2.349	44.629	1.0	1. 2
20	1. 1156	2. 278	45.554	0.9	1.1
21	1.0834	2. 212	46.451	0.9	1.0
22	1.0536	2.151	47. 324	0.9	0.9
23	1.0259	2.094	48. 173	0.8	0.9
24	1.0000	2.042	49.000	0.8	0.8
[[ota]				49.0	49.0

Table B-2.8 Continued

			For Sec	ond Day	•
t	(24/t)**0.6	r t	r t*t	R	r
1	6.7317	51.301	51.301	51.3	3. 2
2	4.4413	33.846	67.693	16.4	3.4
3	3.4822	26.537	79.612	11.9	3.6
4	2.9302	22.330	89.321	9.7	3. 8
5	2.5630	19.532	97.660	8.3	4.1
6	2. 2974	17.508	105.048	7.4	4.5
7	2.0944	15.961	111.730	6.7	5.0
8	1. 9332	14.732	117.860	5.1	5. 7
9	1.8013	13.727	123.545	5.7	6. 7
10	1.6909	12.886	128.863	5. 3	8.3
11	1.5969	12.170	133.871	5, 0	11.9
12	1.5157	11.551	138.612	4.7	51.3
13	1.4446	11.009	143, 122	4. 5	16.4
14	1.3818	10.531	147.428	4.3	9.7
15	1.3258	10.104	151.553	4.1	7.4
16	1. 2754	9. 720	155. 517	4.0	6.1
17	1. 2299	9.373	159.334	3. 8	5. 3
18	1.1884	9.057	163.019	3. 7	4. 7
19	1.1505	8.768	166.583	3.6	4. 3
20	1.1156	8.502	170.036	3.5	4.0
21	1.0834	8. 257	173.387	3.4	3.7
22	1.0536	8.029	176.644	3.3	3.5
23	1.0259	7.818	179.813	3. 2	3. 3
24	1.0000	7.621	182.900	3, 1	3. 1
Total				182.9	182.9

			For Thi	rd Day	
t	(24/t)**0.6	r t	r t*t	R	r
1	6.7317	4.544	4. 544	4.5	0.3
2	4.4413	2.998	5.996	1.5	0.3
3	3, 4822	2.350	7.051	1.1	0.3
4	2. 9302	1.978	7. 911	0.9	0.3
5	2.5630	1.730	8.650	0.7	0.4
6	2. 2974	1.551	9.304	0.7	0.4
7	2.0944	1.414	9.896	0.6	0.4
8	1.9332	1.305	10.439	0.5	0.5
9	1,8013	1.216	10.943	0.5	0.6
10	1.6909	1.141	11.414	0.5	0.7
11	1.5969	1.078	11.857	0.4	1.1
12	1.5157	1.023	12. 277	0.4	4. 5
13	1. 4446	0.975	12.677	0, 4	1.5
14	1. 3818	0.933	13.058	0.4	0.9
15	1. 3258	0.895	13.424	0.4	0.7
16	1. 2754	0.861	13.775	0.4	0.5
17	1. 2299	0.830	14. 113	0.3	0.5
18	1.1884	0.802	14.439	0.3	0.4
19	1.1505	0.777	14.755	0.3	0.4
20	1.1156	0.753	15.061	0.3	0.4
21	1.0834	0.731	15.357	0.3	0.3
22	1.0536	0.711	15.646	0.3	0.3
23	1.0259	0.692	15.927	0.3	0.3
24	1.0000	0.675	16.200	0.3	0.3
Total				16.2	16.2



B-2.3 Runoff Analysis

(1) Runoff Model

The conceptual runoff models may be categorized to the lumped model, such as unit hydrograph method, runoff function method, series tank model, storage function method, etc., and the rainwater flow method. In either way, every methods are possible precisely to reproduce the flood events, when the actual observation records/data are available. However, application of the lumped model may be unreliable and difficult in determination of the factors/coefficients to be applied, because the discharge records/data actually observed and hydraulic data/information on the flood events are scarcely available within the area. Hereby, the rainwater flow method would be applied for the runoff analysis in the Study Area, which is complicated in calculation but be the most applicable method corresponding with the various physical conditions. (For detail explanation, See B-3 (1))

(2) Design Rainfall

The design rainfall for the runoff analysis in the Study Area would be applied with 3-day consecutive areal average rainfall of 284.1 mm and the middle day in peak type in the pattern of hyetograph. (See B-2.2)

(3) Base Flow and Rainfall Losses

The base flow may consist of wasted-water of irrigation and domestic water, while the rainfall losses may be involved in the initial losses, seepage losses and retention in the area. Considering that the drainage problems are taken place only in the rainy season, and that the antecedent rainfall to the 3-day consecutive rainfall is expected to be substituted for the initial losses, it assumes that the base flows are negligibly small and rainfall losses ignore.

(4) Characteristics of Drainage Area

(a) Equivalent Roughness

The equivalent roughness, which is equivalent to roughness coefficient of Manning formula applied to the flow on the saturated land, have been determined to be 0.6 for mountainous land, 1.0 for paddy field, 0.4 for upland field and 0.025 for residential area.

(b) River Gradient and Land Slope

The gradient of rivers/creeks/channels and slope of land have been decided based on the 1/50,000 or 1/10,000 topographical maps.

(c) River Flow Factors (K,P value) Estimate

For the runoff analysis by applying the kinematic wave model to the rainwater flow method, the river flow factors (K & P values) for each creek and drainage channels have been estimated by plotting the correlation between flow area (W) and water discharge (Q) in the logarithm scale coordinates, as shown in Figure B-2.9.

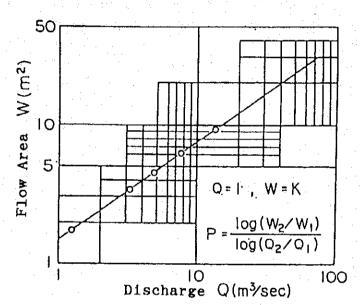


Figure B-2.9 Estimation of River Flow Factors

(5) Hydraulic Factors for Runoff Analysis

(a) Topography

Seeing that the water logging area depends on the topography of land, the land slope have been evaluated, based on the topographical maps with a scale of 1/50,000 or 1/10,000 indicating every 1.0 meter contour line. An area and volume every 1.0 meter in an elevation have been computed for drainage analysis, which would be regarded as an area and volume of flooded water.

(b) Land Category

For estimation of runoff discharge from the various land categories, such as regional area, paddy field, upland field, etc., The land categories for each drainage block area estimated based on the present land use maps (Figure C-1.5).

(c) Average Equivalent Roughness

The average equivalent roughness, as mentioned previously, have been calculated for each drainage block as a weighted average of each land category and equivalent roughness as tabulated in Table B-2.9.

(d) Hydraulic Factors for Land

The diagrammatic land area and land slope have been computed for runoff analysis, based on the said topographical maps, as show in Table B-2.10.

(e) Waterway Characteristics

The river flow factors (K & P values) of the waterway for each drainage blocks have been computed based on the surveyed data as shown in Table B-2.10.

Name of	HillyLand	Paddy F.	Upland	ResidentA.	Drain. A	Average
Drainage Block	0.600	1.000	0.400	0.025	ha	N
HIEN LUONG D/S		3005.8		816.2	3822.0	0.792
HIEN LUONG U/S		2083.0	56.0	524.0	2663.0	0.796
HAN QUANG		1761.2		300.8	2062.0	0.858
TAN CHI	169.0	4660.7	55.7	1643.6	6529.0	0.739
KIM DOI D/S	58.0	3121.6	31.0	1244.4	4455.0	0.718
KIM DOI M/S	8.5	219.3	67.0	174.2	469.0	0.545
KIM DOI U/S		1262.3		567.7	1830.0	0.698
LAIDA		1470.3		314.7	1785.0	0.828
GIALAM		2054.3		588.7	2643.0	0.783
TRI PHUONG		1073.5		320.5	1394.0	0.776
TRINH XA AREA		3237.8	101.0	723.2	4062.0	0.811
XUAN VIEN		1292.8	0.9	616.3	1910.0	0.685
CHAU CAU	104.0	984.0	75.8	308.2	1472.0	0.737
VIET THONG		864.3	17.0	255.7	1137.0	0.772
QUE TAN		386.9		136.1	523.0	0.746
PHA LAI	96.0	1678.2	115.0	315.8	2205.0	0.812

Table B-2.9 Average Equivalent Roughness

Table B-2.10 Hydraulic Factors for Land and Waterway.

				Cloped Land	7			Waterway	WAV	:	
4. 4. V.	01221	(La)	010	oloped na	Equ. Dough	elono B	Divor I	Piwer S	Manning's	×	ρ
Name of Drainage block	BIOCK	(lia)	Nope L.	orope o.	Egu. rougn		L(m)	MIVEL D.		Value	Value
HIEN LUONG D/S	Left B.	3030	3367	0.000225	0.792	0006	0006	0.0000526	0.045	1.10	0.838
. *.	Right B.	1080	1200	0.000600					:		
HIEN LUONG U/S	Left B.	1990	3980	0.000370	0.796	2000	2000	0.0000526	0.045	1.10	0.838
	Right B.	088	1760	0.000560						:	
HAN QUANG	Left B.	1320	3771	0.000171	0.858	3500	3500	0.0000526	0.045	1.10	0.838
	Right B.	800	2286	0.000333							
TAN TRI	Left B.	4860	3738	0.000067	0.739	13000	13000	0.0000526	0.045	1.10	0.838
	Right B.	1400	1077	0.000400							
KIM DOI D/S	Left B.	1260	1938	0.000833	0.718	0099	6500	0.0000540	0.045	1.22	0.795
	Right B.	2700	4154	0.000320							
KIM DOI M/S	Left B.	220	880	0.000500	0.545	2500	2500	0.0000540	0.033	1.22	0.795
	Right B.	310	1240	0.002101							
KIM DOI U/S	Left B.	1090	1557	0.000467	0.698	0002	7000	0.0000540	0.033	1.22	0.800
	Right B.	750	1557	0.000500							
LAIDA	Left B.	895	1071	0.000979	0.828	7300	2000	0.0005400	0.033	1.20	0.800
	Right B.	968	1226	0.000979							The state of the s
GIALAM	Left B.	1310	1226	0.001071	0.783	11700	11700	0.3000000	0.033	1.20	0.800
	Right B.	1310	1120	0.001071							
TRI PHUONG	Left B.	685	1120	0.001051	0.776	0009	0009	0.0001667	0.033	1.20	0.800
	Right B.	685	1142	0.001051					:		Control of Control
TRINH XA AREA	Left B.	088	1100	0.000909	0.881	8000	8000	0.0003638	0.033	1.20	0.800
	Right B.	3180	3975	0.000909							
XUAN VIEN	Left B.	096	1352	0.000592	0.685	7100	71000	0.0002222	0.033	1.20	0.800
	Right B.	096	1352	0.000592						- 1	
CHAU CAU	Left B.	1100	2200	0.001000	0.737	5000	2000	0.0001400	0.033	1.20	0.800
	Right B.	280	1160	0.000517							
VIET THONG	Left B.	380	092	0.001000	0.746	2000	2000	0.0001000	0.033	1.20	0.800
	Right B.	068	1780	0.001000							
QUE TAN	Left B.	008	857	0.001050	0.746	3500	3500	0.0001714	0.033	1.20	0.800
	Right B.	300	857	0.001050							
PHA LAI	Left B.	1930	4289	0.000220	0.812	4200	4500	0.0001556	0.033	1.20	0.800
	Right B.	410	911	0.000220					-		

(5) Runoff Analysis

Taking into account the hydraulic factors for runoff analysis mentioned above, runoff analysis have been carried out in the Study Area. Concerning the Tan Chi Area, the total runoff is shown in Figure B-2.10.

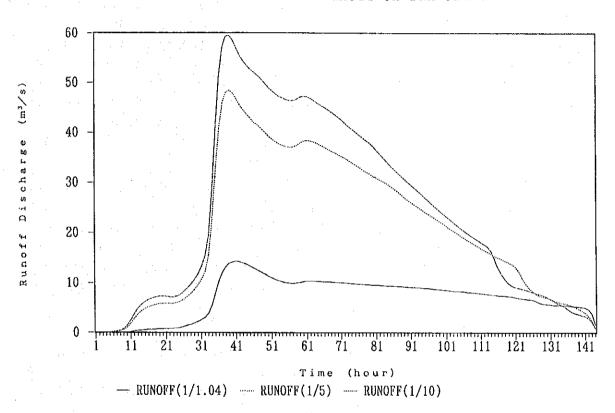


Figure B-2.10 Total Runoff on Tan Chi Case

B-2.4 River Water Levels

(1) Design River Water Levels

The Study Area is the polder surrounded by the three rivers, Duong, Cau and Ngu Huyen Khe rivers. The drainage in the area is able to drain the excess water by gravity through the drainage sluices in the dry season, while the pumping drainage is imperative in the rainy season due to the higher water level in the rivers. Along the Duong river, Chau Cau, Thai Hoa, Tan Tri, Tri Phuong, Thinh Lien, Phu ong and Duong Ha pumping stations are located from the downstream in order. The major drainage stations along the Cau river are Xuan Vien, Huu Chap, Kim Doi, Vien thong, Que Tan, and Hien Luong stations located from the upstream in order. Along the Ngu Huyen Khe river, Dan Xa, Phu Lam and Trinh Xa pumping stations are constructed.

The design river water levels for each pumping station are generally estimated by the rating curve and/or the river cross-section and profile base on the respective water discharges. However, the design water levels have been calculated by proportioning to the distance from the related gauging stations, since the data necessary for the estimate of water discharges were not available.

In order to evaluate the design river water levels which have a hydrography corresponding to the 3-day consecutive rainfall in a probability of 1/10 year and more, the rainfall data have been evaluated as listed below:

Table B-2.11 3-day Consecutive Rainfall With a probability of 1/10 Year & More for Each Rainfall Station

	3-day R	ainfall 1/20	i/10 yr. Rain (3-d. Consecu. R. Equivalent)	1/20 yr. Rain (3-d. Consecu. R. Equivalent)	(3-d. Consecu-
YEN PHONG	257.7	294.6	1975:246.4 mm	1983:285.8 mm	1984:326.9 mm 1/40yr. Equiv. 1985:336.2 mm 1/40yr. more
DONG ANH	296.3	356.3	1980:304.5 mm 1985:315.3 mm	1986:354.5 mm	1984:466.6 mm 1/50yr. more
YEN VIEN	315.4	377.4	1967:339.0 mm 1980:332.9 mm 1989:332.0 mm	1985:390.9 mm	1984:420.9 mm 1/30yr. more
BAC NINH	246.0	273.9	1975:257.2 mm 1990:257.0 mm	1986:261.3 mm	1985:346.3 mm 1/100yr. more
QUE YO	254.5	287. 4	1972:267.1 mm 1975:249.5 mm	1971:271.7 mm 1980:277.0 mm 1985:287.5 mm 1986:295.6 mm	
TU SON	231.8	255. 1	1965:243.0 mm	1968:269.3 mm	
Over All Area	248.1	278. 2			

In addition to the above rainfall at each station, the 3-day consecutive rainfall in regard to the areal average rainfall have also been evaluated. As shown in Table B-2.12, the 3-day consecutive areal rainfall with an exceedance probability of 1/10 year or equivalent are occurred in 23-25/July, 1965 and 26-28/July, 1975. The rainfall exceeded a probability of 1/10 year is taken place only on 11-13/September, 1985. Taking into account pattern of hyetograph in 1985 and allowance of pump operation capacity, The water levels recorded on 11-13/September, 1985 would be applied as the design river water levels.

Table B-2.12 Hyetograph Pattern of 3-day Consecutive Areal Rainfall

		and the second second		
Yea	r Wonth/day	Areal Rain	Rain Pattern	Probability
196	5 7/23- 7/25	225.9	Wid.D. Peak	1/10yr.below
196	7 6/8-6/10	158.3	lst D. Peak	1/2yr. below
196	8 10/ 9-10/11	179. 4	Mid.D. Peak	1/3yr. below
197	1 7/12- 7/14	194.1	3rd D. Peak	1/3yr. below
197	2 8/28- 8/30	174. 2	Wid.D. Peak	1/3yr. below
197	8/26-8/28	238.5	lst D. Peak	1/10yr.below
198	0 7/23- 7/25	217. 1	Wid.D. Peak	1/5yr. Equiv.
1980	8/19- 8/21	169.5	Wid.D. Peak	1/2yr. Equiv.
198	8/1-8/3	144.1	lst D. Peak	1/2yr. below
1984	4 11/ 8-11/10	178.7	3rd D. Peak	1/3yr. below
198	9/11- 9/13	331.0	Wid.D. Peak	1/50yr. more
1986	5/25- 5/27	188.6	3rd D. Peak	1/3yr.equiv.
1986	6/17-6/19	203. 3	Mid.D. Peak	1/5yr. below
1989	6/10-6/12	118.9	3rd D. Peak	1/2yr. below
1990	9/19- 9/21	174.7	Wid.D. Peak	1/3yr. below

Table B-2.13 Selected Hydrograph at Each Pumping Station

	-1						-	_
HOA CAU CAU	Pump Sta.	4.80	4.15	4.01	4.63	5.37	5. 20	4.76
	의	- 1			5.11	1	5.64	نی
TAN TRI	Pump Sta.	6.19	5. 75	5.56	5.83	6.36	6. 29	5.96
TRI PHUONG	Pump Sta.	6.52	6.14	5.93	6.12	6.59	6.55	6.25
THINH LIEN	Pump Sta.	6.63	6.26	6.05	6. 22	6. 57	6.64	6.35
A PHU DONG THINH LIENTRI PHUONG TAN TRI THA	Pump Sta.	6.97	6.65	6. 42	6.51	6.91	6.90	6.64
DUONG H	Pump St	7.3	7.03	6.80	-	-	7.16	6
BEN HO	Station	6.02	5.56	5.37	5.69	6.24	6. 15	5.82
G CAT	ion	7.69	7.48	7. 23	7.13	7.42	7.47	7. 26
Station LHUON		1/22	7/23	1/24	7/25	7/26	7/27	1/28
	Date	1965						

CAU	Pump			5. 57			-	
THAI HOA	Pum							
TAN TRI	Pump Sta.	6.40	6.47	7.32	8.12	9.02	9.52	9.39
FRI PHUONG	Pump Sta.	6.73	6.82	7.74	8.59	9.50	9.97	9.81
G THINH LIENTRI PHUONG TA	Pump Sta.	6.84	6.94	7.88	8.74	9.66	10.13	9.95
PHU DON	Pump St	7.1	7.2	8.3	9.21	10.15	10.58	10.37
DUONG HA	žt.	22	7.65	8.72	9.68	10.63	11.04	10.79
BEN BO	Station	6.23	6.29	7.11	7.88	8.78	9.29	8
THUONG CAT		7, 90	8.06	9. 21	10.23	11.19	11.57	11.28
Station THUONG		8/6	6/6	9/10	9/11	9/12	9/13	9/14
	Date	1985						

CAU	Sta.	6.30	6.54	7.12	7.45	7. 21	6.95	6. 60
2 2 3								
II HOA		6.94	7.28	7.95	8. 18	7.86	7.54	7.15
THA								
TAN	Pum		8.38	9. 20	9. 28	8.84	8. 42	7.96
RI PHUONG	Pump Sta.	8.27	8.82	9.70	9.72	9. 23	8.77	8. 29
THINH LIENT	Pump Sta. Pump Sta.	8.40	8.	6	9.86	9.	∞	ထ
PHU DONG	Pump Sta.	8.78			10.30			
<u>000</u>	Pun		9.85	10,86	10.74	10.14	9.59	9,05
BEN HO	Station	7.70			9.06			
THUONG CAT	Station	9.61	10.37	11.44	11.25	10.59	10.00	9.43
Station	Station			7/25	7/26	7/27	7/28	7/29
	Date	1980						

B-2.5 Water Balance Analysis

(1) Basic Approach to Drainage Improvement

The drainage system area in the study area are grouped into the followings, as stated in the Section 3.3.2. of the main text:

Ngu Huyen Khe River drainage system area(5,850ha); Duong River and Tao Khe Creek drainage system area(21,300ha); Drainage system areas along Cau River (12,460ha).

Among other, three drainage system mentioned above except Tao Khe creek drainage systems are composed of the independent drainage area equipped with the pumping station. The improvement scheme of these drainage areas would be able to draw up for each system. While, Tao Khe Creek areas are split into several drainage blocks by not only the topography and polder dikes but also the regulating sluices, as shown in Figure B-2.12.

The basic approach to the drainage improvement would be focused on the independent drainage areas where are frequently flooded, taking into account regional and decentralized drainage systems as well as the existing water control rules.

(2) Method of Analysis

In drainage command area, the runoff is controlled by the topographical conditions and appears in the river or drainage canal with some delay. And then, the runoff in the canal is controlled by hydraulics of the canal. In case, the canal capacity does not meet the runoff or in case, the flow is restricted due to backwater from downstream, the excess water causes inundation. Such phenomenon emerges not only in canals but also in culverts across roads, siphons, gateways, drainage pumping stations, etc. Such phenomena are commonly observed in the lowlying flat areas. In order to simulate the inundation precisely, so-called Continuous Reservoir Model method would be applied. (For detail explanation, See B-3 (2))

(3) Hydraulic Factors for Water Balance Simulation

(a) Design Field Elevation

The lowest elevation in the Study Area ranges between +2.0 and +4.0 m. On the other hand, the land use in the lowest area is mainly swamp land and partly utilized for fish pond. Consequently,

on drainage planning the lowest elevation is not considered. Design field elevation is determined base on land use map, which is covered about 10% of the drainage area.

(b) Allowable Inundated Water depth and Duration

Assuming that the growing of paddy may be tilling to booting stage in the month of July through September, the inundated water depth and duration to be allowed are applied as follows;

- * A water depth of over 0.246 m should not be lasted beyond one day; and
- * A water depth of over 0.159 m should not be lasted beyond three days.
- * A runoff of 3-day consecutive rainfall should be drained out within 5-day.

(c) Methodology for Determination of Pump Capacity

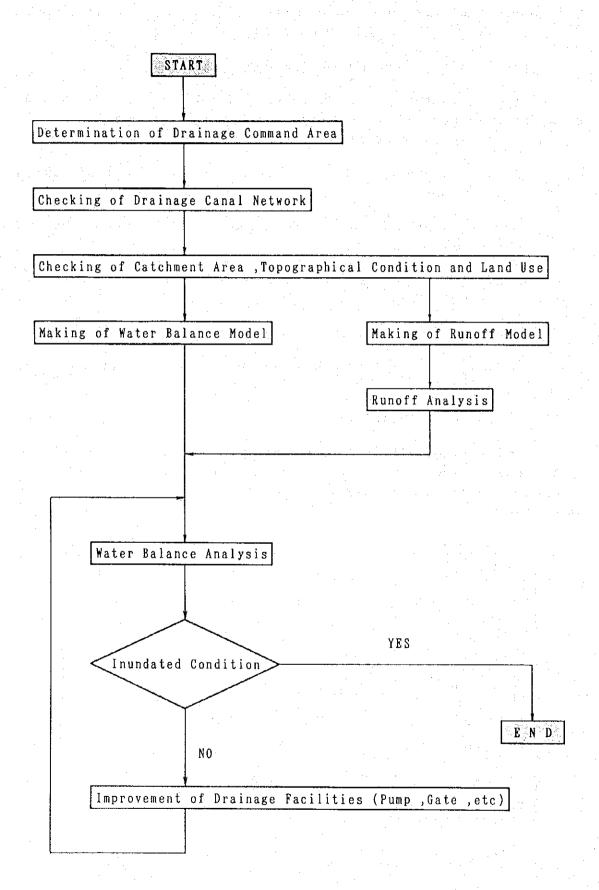
The pump capacity is determined based on the flow chart of drainage planning as shown in Figure B-2.11.

(4) Water Balance Simulation

For the study alternatives, following drainage schemes have been contemplated taking into account present water control rules, gravity drainage in the dry season and original plan of drainage improvement.

Table B-2.14 Drainage Schemes on Study Area

بالكالم فيستف كالباب والمستحدد والمستحدد والمستحدد			
Name of Drainage Area	Area(ha)	Name of Drainage Area	Area(ha)
Pha Lai	2,340	Trinh Xa	4,060
Que Tan	600	Lai Da	1,790
Viet Thong	1,090	Kim Doi	6,510
Chau Cau	1,680	Hien Luong	6,980
Tri Phuong	1,480	Tan Chi	6,420
Gia Lam	2,620	Tan Chi	8,540
Xuan Vien	1,920	+ Han Quang	



(a) Study Area

According to the above alternative schemes, the water balance analysis have been done, the results of each drainage area are shown in Table B-2.16, B-2.17 and Figure B-2.12.

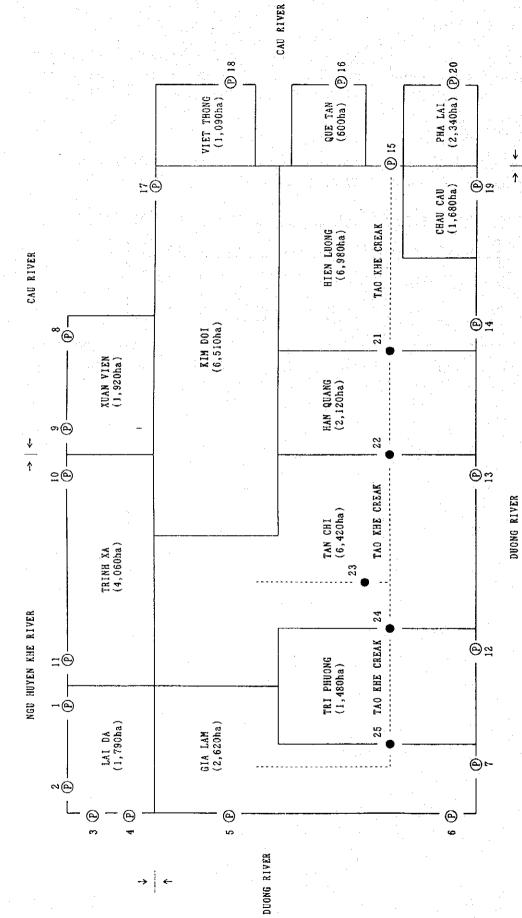
(b) Project Area

For Tan Chi area, the following 2 alternatives study have been done based on the detailed data.

Table B-2.15Alternative Schemes on Project Area

Case	Name of Drainage Area	Area(ha)
1	Tan Chi Only	6,420
2	Tan Chi + Han Quang	8,540

According to the above alternative schemes, the water balance analysis have been done, the results of each drainage area are shown in Table B-2.18 \cdot \cdot B-2.21 and Figure B-2.13 \cdot \cdot B-2.16.



., ,	25 THINH LIEN SLUICE	25	PHA LAI PUMP STA.				HIEN LUONG PUMP S.	15 HIEN LUONG PUMP S.	15 HIEN LUONG PUMP S.
UICE	24 TRI PHUONG SLUICE	24	CHAU CAU PUMP STA.	19		THAI HOA PUMP STA.	14 THAI HOA PUMP STA.		14
Ι΄.	23 BA CAY SUUCE		VIET THONG PUMP S.	18		TAN CHI PUMP STA	13 TAN CHI PUMP STA.	HUU CHAP PUMP STA. 13 TAN CHI PUMP STA	
	22 TRAM BRIDGE	22	KIM DOI PUMP STA.	17	S.	TRI PHUONG PUM	12 TRI PHUONG PUM	THINH LIEN PUMP S. 12 TRI PHUONG PUMP S.	7 THINH LIEN PUMP S. 12 TRI PHUONG PUM
	21 LA MIET SEUICE	21	16 QUE TAN PUMP STA.	16	TA.	TRINH XA PUMP S	11 TRINH XA PUMP STA.	PHU DONG PUMP STA. 11 TRINH XA PUMP S	
	NAME OF STRUCTURES	No.	NAME OF STRUCTURES No. NAME OF STRUCTURES		ES	NAME OF STRUCTURES No.	No. NAME OF STRUCTUR		E OF STRUCTURES No.

Table B-2,16 Inundated Condition at Present Status

Note: (*1) Shown the time inundated over 0.246 m in a water depth.

(*2) Shown time inundated over 0.159 m in a water depth.

Pumping Capacity: PHA LAI AREA ... PHA LAI 5.00m3/s

QUE TAN ARE ... QUE TAN 1.56m3/s

VIET THONG AREA ... VIET THONG 3.56m3/s

CHAU CAU AREA ... CHAU CAU 44m3/s

TRI PHUONG AREA ... TRI PHUONG 3.11m3/s

GIALAM AREA ... THINH LIEN 4.16m3/s , YUAN VIEN 2.22m3/s

XUAN VIEN AREA ... TRINH XA AREA ... TRINH XA 20.00m3/s , PHU LAM 3.33m3/s

LAIDA AREA ... LIEN DAN 2.22m3/s , LOC HA 1.17m3/s , DONG DAU 2.00m3/s , LAIDA 1.00m3/s

Table B-2/7 Inundated Condition after Project

CPLAN									
				Drainage Area	ಡ				
Tem					C::: C	AT TIO	NOIN WILL DAY THE	TALAL	1 1 1 T 4 T
	IV'I VHG	OUE TAN	VIET THONG	HAU CAU	IKI PHUUNG	עם עום חאת	AUAN TAUN		no Tun
	0 0 0 0 0 0	800 hs	2240 hz 600 hz 1000 hz	1680 ha	1480 ha	na 2620 ha	1920 ha	4060 ha j	1790 ha
Drainage Area	DII 07:09	3 000	3 0	1 000	# CC # ~	L		3 000 m	4.500 m
Designed Field EL.	2.050 ■	2.050 m 2.300 m	E 000.2			1	į		1 000
4 4 4 5	E	0 279 m	0.283 =	0.309	0.285	m 0.7.0 m	E 027.0	E 0C2.0	E 602.0
Max, Fater Depth	2 4		100			421 ha	236 ha	824 ha	263 ha
Max. Inundated Area	248 118	BH C71	817 001		•		54 .0.	:	* + 5
000000	8+ Q6 hr	At 82 hr.	At 100 hr.			בים אני אלי חוד.	AC 101 III.	:	
Max. w. D. Occurred		1 7 6	F 4 V C	24 hr	73 57	7. 24 hr.	24 hr. 47 hr.	21 hr.	24 hr.
Duration time	71 nr.	. Ju 67	. 111 1.7	111 12			001 001047	101 101	141104 100
Trundation	(*2)66-136	(*1)72-95	(*1)89-112	(*1)80-103	(*1)84-1	10 (*1.88-111	021-00(2*)	(*1/01-101	101-40/14/
01 INUNCACATOR (19.0 m3/s 9.0 m3/s	2/61	0 0 0 0 0	7 2 E 27 / 6	x 7 m3/s	6.5 33	/s 14.2 m3/s	9.0 m3/s	19.0 m3/s	9.0 m3/s
Pumping Capacity	2 / DE C . B	0 / C III C - 7			- 000	4/ 4/ 1 007 2 11	4 600 1/4/50	A 600 1/e//26	028 1/c/ba
+ + + + + + + + + + + + + + + + + + + +	4 060 1/s/ha	4.833 1/s/ha	5.046 1/s/hal	5.179 1/s/ns	14.336 1/5	/ nam - 4 c U 1 / S / 11	14.000 1/5/110	4.000 I/S/11ap	. 040 1/3/114
Urainage pury	2. /2/I 000 E								

Note : (*1) Shown time inundated over 0.246 m in a water depth. (*2) Shown time inundated over 0.159 m in a water depth.

Table B-2.16 Inundated Condition at Present Status

Note: (*1) Shown the time inundated over 0.246 m in a water depth. (*2) Shown time inundated over 0.159 m in a water depth. (*3) Shown case include TAN CHI Area

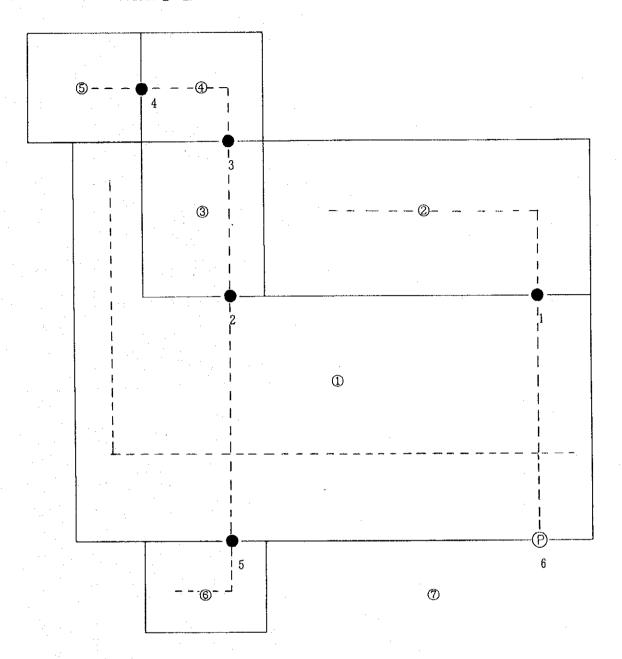
Pumping Capacity: KIM DOI AREA ··· KIM BOI 12.50m3/s
HIEN LUONG ARE ··· THAI HOA 4.67m3/s ,HIEN LUONG 26.50m3/s
TAN CHI AREA ··· TAN CHI 15.11m3/s
HAN QUANG (Inc.TAN CHI) AREA ··· TAN CHI 15.11m3/s

Table B-2.17 Inundated Condition after Project

	7 5 7 7					
	Item		Drainage Area		(*3)	
		KIM DOI	HIEN LUONG		TAN CHI HAN QUANG	
•	Drainage Area	6510 ha	6980 ha	6420 ha	8540 ha	
	Designed Field EL.		2.100 Ⅲ	3.060 m	2.500 ₪	-
	Max. Water Depth		,		0.264 m	
	Max. Inundated Area	585 ha	916 ha	936 ha	1395 ha	
	Max.W.D.Occurred At 84 hr. At 100 hr. At 93 hr. At 107 hr.	At 84 hr.	At 100 hr.	At 93 hr.	At 107 hr.	
	Duration Time	62 hr.	70 hr.	47 hr.	24 hr.	
	of inundation	(*2)57-118	(*1)63-132	(*2)70-116	(*1)95-118	•
	Pumping Capacity 35.50 m3/s 31.17 m3/s 31.11 m3/s 41.11 m3/s	35.50 m3/s	31.17 m3/s	31.11 m3/s	41.11 m3/s	
	Drainage Duty	5.453 1/s/ha	4.466 1/s/ha	4.846 1/s/ha	4.814 1/s/ha	

Note : (*1) Shown time inundated over 0.246 m in a water depth. (*2) Shown time inundated over 0.159 m in a water depth.

FIGURE B-2.13 WATER BALANCE SIMULATION MODEL (TAN CHI ONLY)



No.	NAME OF STRUCTURES	No.	NAME OF STRUCTURES
1	KT CAU NAU-S GATE	6	TANCHI PUMPING STATION
2	BA CAY SLUICE		
3	LIEN MAO SYPHON		
4	CAU SAT SLUICE		
5	CANH HUNG SLUICE		

①~⑥ Drainage Command Area
⑦ Duong River

Table B-2,18 Inundated Condition at Present Status at TAN CHI Area

[PRESENT]						
Item			Drainage Area	e Area		
	Θ	0	0	(P)	(2)	0
rainage Area	2682 ha	1410 ha	696 ha	632 ha	730 ha	270 ha
Designed Field EL.		2.900 ₪	3.150 m	3.450 m	4.650 m	4.700 m
fax. Water Depth		0.647 m	0.832 m	0.556 ш	0.00 m	0.013 m
fax. Inundated Area		752 ha	289 ha	313 ha	4 ha	29 ha
fax.W.D.Occurred	At 112 hr.	At 139 hr.	At 107 hr.	At 102 hr.	At 66 hr.	At 67 hr.
Uration Time	0ver 64 hr.	0ver 64 hr. 0ver 104 hr 0ver 110 hr0ver 94 hr. 0 hr.	0ver 110 hr	Over 94 hr.	0 hr.	0 hr.
of inundation	(*2)81-144	(*2)81-144 (*1)41-144 (*1)35-144 (*1)51-144	(*1)35-144	(*1)51-144		

(*1) Shown the time inundated over 0.246 m in a water depth. (*2) Shown time inundated over $0.159~\mathrm{m}$ in a water depth. Note

Pumping Capacity : TAN CHI 15.11m3/s (2.354 1/s/ha)

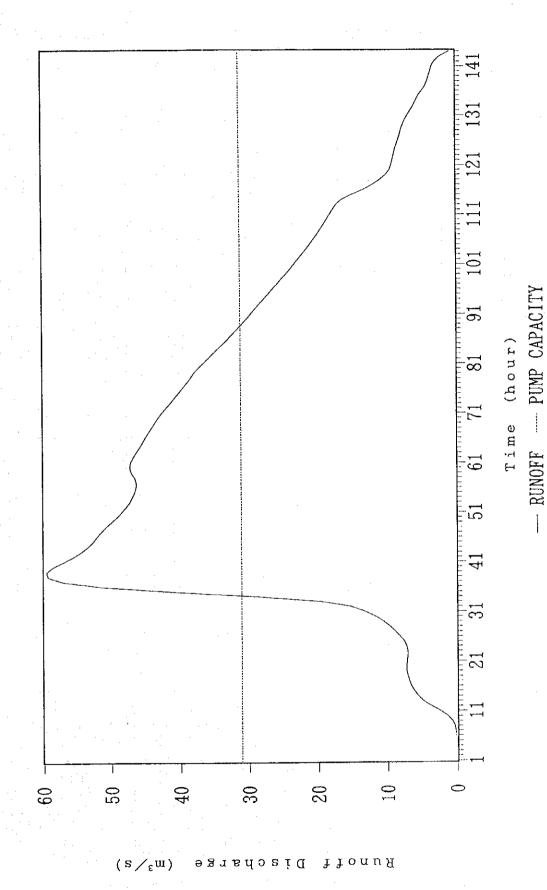
Table B-2,19 Inundated Condition after Project at TAN CHI Area

Item			Drainage Area	e Area		
	0	©	(B)	Ф	®	©
Orainage Area		1410 ha	696 ha	632 ha	730 ha	270 ha
Designed Field EL.	!	(*3)3.060 m	3.150 ₪	3.450 m	(*3)4.000 m	(*3)4.600 m
fax. Water Depth	:	0.247 m	0.204 m	0.042 m	0.375 ш	0.113 Ⅲ
Max. Inundated Area		191 ha 514 ha	127 ha	71 ha	71 ha 5 ha 29	29 ha
fax.W.D.Occurred	~~	At 93 hr.	At 82 hr.	At 70 hr.	At 60 hr.	At 67 hr.
Duration time		47 hr.	35 hr.	0 hr.	20 hr.	0 hr.
of Inundation	-	(*2)70-116	(*2)65-99		(*1)46-65	

Shown time inundated over 0.246 m in a water depth. (*1)

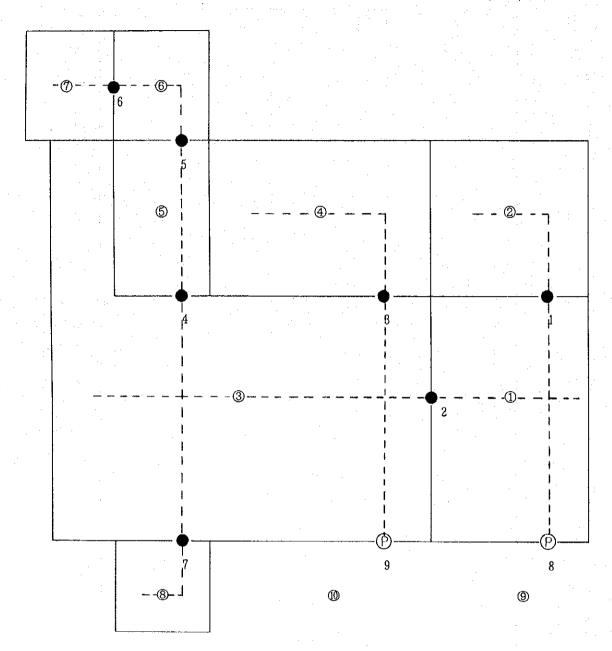
Different designed field elevation between present and plan in Block No. 3, 5 and Shown time inundated over 0.159 m in a water depth (*3) (*5)

oumping Capacity : TAN CHI 31.11m3/s (4.846 1/s/ha)



B-55

FIGURE B -2.15 WATER BALANCE SIMULATION MODEL (TAN CHI AND HAN QUANG)



No.	NAME OF STRUCTURES	No.	NAME OF STRUCTURES
1	BA CUA SLUICE	6	CAU SAT SLUICE
2	TRAM BRIDGE	7	CANH HUNG SLUICE
3	KT CAU NAU-2 GATE	. 8	HAN QUANG PUMPING ST.
4	BA CAY SLUICE	. 9	TANCHI PUMPING STATION
5	LIEN MAO SYPHON	1	

Drainage Command Area Duong River

⑨~⑩

Table B-2,20 Inundated Condition at Present Status at TAN CHI and HAN QUANG Area

[PRESENT]				-				
Ttom				Drainage Area	e Area			
1	€	0	©	(9	©	0	∞
Orange Area	1391 ha	729 ha	2682 ha			632 ha	730 ha	270 ha
Designed Field FI		2	:					4.700 m
Zov Water Death				0.446 m			0.000	0.013 m
May Trundated Area	61.0 h	372 ha		558 ha	291 ha	314 ha		29 ha
Mak W D Occurred	At 141 hr	At 137 hr.	At 117 hr.	At 116 hr.	At 108 hr.	At 104 hr.	At 66 hr.	At 67 hr.
Duration time Over 102 hr. Over 90 hr.	Over 102 hr.	Over 90 hr.	0 hr.	Over 82 hr.	0 hr. Over 82 hr. Over 109 hr. Over 94 hr.	Over 94 hr.		0 hr.
of Inundation (*1)43-144 (*1)55-144	(*1)43-144	(*1)55-144		(*1)63-144	(*1)63-144 (*1)36-144 (*1)51-144	(*1)51-144		

(*1) Shown the time inundated over 0.246 m in a water depth. Note:

(*2) Shown time inundated over 0.159 m in a water depth.

Pumping Capacity : TAN CHI 15.11m3/s (1.769 1/s/ha)

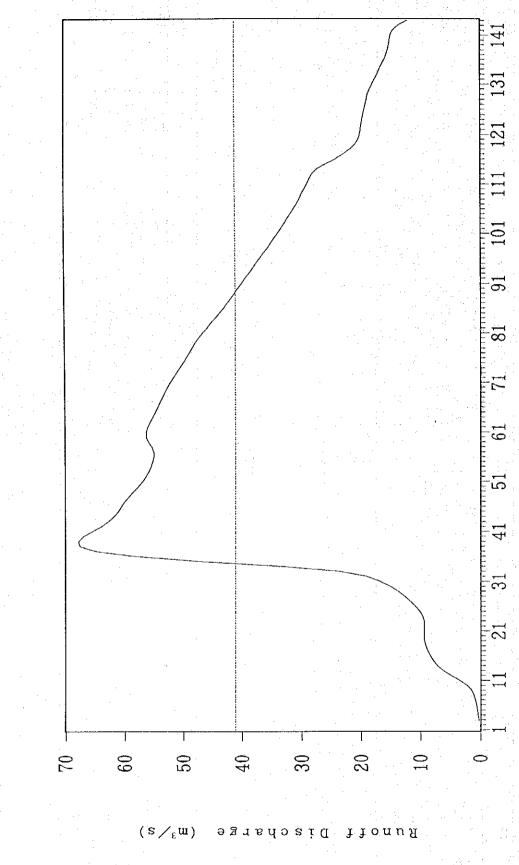
Table B-2.21 Inundated Condition after Project at TAN CHI and HAN QUANG Area

LPLANI				-				
T 4 D				Urainage Area	Area			
	€	6	60	4	9	9	0	@
, , , , , , , , , , , , , , , , , , ,	1301 ha	720 ha	2682 ha		696 ha	632 ha	730 ha	270 ha
Urainage Alea		\$ CO & C	3 400 =	(*3)3 060 m	3.150 m	٠	(*3)4.000 m	(*3)4.600 m
Designed fletd b.	H 700	1 1 000		E 000 0	0.166 =		0.025 m 0.375 m 0.113 m	0.113 m
Max. Water Deptn	E 502.0	100.	1 000 0	# 701.0	118 ha		5 12	29 ha
Max.Inundated Area 417 ha 173 ha	41 / na	175 na	144 IIG	714 00 77	A+ 75 hn		4+ 60 hr	At 67 hr.
Max.W.D.Occurred	At 107 hr.	At 108 hr.	At &3 nr.	W. O . III.	77. (2)		200	بر د
Duration time	24 hr.	36 hr.	0 hr.	24 hr.	12 nr.	o nr.	.JII 07	0 111
of Inundation (*1)95-118 (*2)91-126	(*1)95-118	(*2)91-126		(*2)76-99	(*1)70-81		(*1)40-05	

Note : (*1) Shown time inundated over 0.246 m in a water depth.

Pumping Capacity : TAN CHI(15.11 m3/s), HAN QUANG(26.00 m3/s), Total 41.11m3/s (4.814 l/s/ha (*2) Shown time inundated over 0.159 m in a water depth. (*3) Different designed field elevation between present and plan in Block No. (4), (7) and (8)

Quang) Han જ Сhì (Tan Capacity Pump and Runoff B-2.16 Total FIGURE



--- PUMP CAPACITY

Time (hour)

B-58

(c) Drainage Coefficient

The drainage coefficient has been determined by following procedures.

Design Standard for Drainage Planning

- Allowable depth 246 mm for 24 hours
- Allowable depth 159 mm for 72 hours
- To drain the excess water of 3-day consecutive rainfall within 5-day

Item (1) and (2) will be checked by water balance simulation which are shown as follows.

Drainage Duty on Case Study

Case	Pump Capacity (m3/sec)	Drainage Duty (1/s/ha)
Tan Chi Only (6,420 ha)	31.11 (15.11+16.00)	4.814
Tan Chi + Han Quang (8,540 ha)	41.11 (15.11+26.00)	4.846

Item (3) has been checked based on following calculation.

- · 3-day consecutive rainfall is 248 mm
- Evaporation(3mm) and percolation(2mm) losses for 5 day
 is 25 mm

$$248 - 25$$

q=---- x 10000 = 5.16 1/sec/ha
5x86400

Determination of Drainage Duty

On drainage planning, drainage duty would be varied depend on the topographic conditions. Accordingly, sophisticated drainage analysis is required. On the other hand, the practical method (Item 3) mentioned above is also required to check the result briefly.

By water balance simulation, the drainage duty is about 4.8 1/s/ha, and on the other hand drainage duty by practical method is about 5.2 1/s/ha. There is not so big difference of drainage duty between two methods. The drainage duty of 4.8 1/s/ha is adopted, because water balance simulation method is more precisely in comparison with practical method.

B-2.6 Water Balance Analysis for Irrigation

(1) Water Balance Model (Input2)Trin Xa Pumping St. A (Area)=39,610 ha Rainfall(Input1) Irrigation(Output) (Input3)Cong Thon P.St. (Input4) Thai Hoa P.St. (2)Condition for Calculation (a)Catchment Area A=39,610 ha (b)Rainfall(Bac Ninb Station: 1960~1992) - Average Anual Rainfall R=1474 mm - 1/2 Non-Exceedance of Probable Rainfall R=1465 mm - 1/10 Non-Exceedance of Probable Rainfall R=1106 mm (c)Average Evaporation Ep=984 mm (d)Runoff Coeficient - 0.389 : Based on river discharge (Phon Day River Quang Cu St.) (e)Coefficient of Return Flow - 0.65 : Based on observeed data at lowlying area in Japan (f)Inflow from outside of Project Area - Trinh Xa Pumping Statiom (Ngu Huyen Khe River) Q=20.0 m3/s - Thai Hoa Pumping Station (Duong River) Q=4.67 m3/s - Con Thong Pumping Station (Duong River) Q=2.52 m3/s Total Volume = 118 MCM (Nov.--Mar.) (g) Irrigated Area and Period (Case A) A=5,084 ha (Jan.--Jun) - Winter Spring Paddy(1) (Jul.--Oct) - Summar Paddy(2) A= 5,295 ha (Nov.--Jan.) - Maize A= 600 ha - Sweet Potato A= . 600 ha (Nov.--Jan.) - Potato 300 ha (Jan.--Dec.) A= - Ground Nut 456 ha (Feb.--Jun.) A= 100 ha (Sep.--Dec.) - Soy Bean A=

Vegetable(1)Vegetable(2)

- Others

A=-

A=

A≃

195 ha

280 ha

140 ha

(Sep.--Dec.)

(Jul.--Jan.) (Jan.--Dec.)

```
(Case B)
     - Winter Spring Paddy(1)
                                    A = 4,000 ha
                                                   (Jan. -- Jun)
     - Summar Paddy(2)
                                    A = 4,000 \text{ ha}
                                                   (Jul: --Oct)
     - Maize(1)
                                    A= 600 ha
                                                   (Sep.--Dec.)
     - Maize(2)
                                          400 ha
                                                   (Jan. -- Apr.)
     - Sweet Potato
                                     A = 1,050 \text{ ha}
                                                   (Oct.--Dec.)
                                     A=
     - Potato
                                          800 ha
                                                   (Nov.--Feb.)
     - Ground Nut
                                          900 ha
                                                   (Feb.--Jun.)
                                     A=
     - Soy Bean
                                          700 ha
                                     A=
                                                   (Nov. -- Jan.)
     - Vegetable(1)
                                                   (Oct. -- Jan.)
                                     Α=
                                          367 ha
     - Vegetable(2)
                                     A=
                                          450 ha
                                                   (Oct. -- Mar.)
     - Vegetable(3)
                                     A = .
                                          280 ha
                                                   (Jan.--Jun.)
     - Others
                                                   (Jan.--Dec.)
                                     A=
                                          217 ha
(h)Water requirement
     - Paddy (Land Preparation)
                                     q=1.79 l/s/ha
              (After Planting)
                                     q=1.29 l/s/ha
     - Subsidiary (Maximum)
                                    q=1.03 l/s/ha
                                     q=0.31 l/s/ha
                   (Minimum)
```

Table B-2.22 Water Balance Table (Uit:MCM)

	① Runoff	② Pump Input	③ Avairable Water	④ Water Req.	⑤ Balance
Average Year	227	118	659	411	+ 248
1/2 Year	225	118	566	411	+ 155
1/10 Year	170	118	475	411	+ 64

Note: ① shown the runoff in the project area

- 2 shown the inplow from outside of project area by pump
- 3 shown the available water ((1+2)*1.65)
- 4 shown the water requirement of paddy and crop
- 6) shown the balance (3-4)

Table B-2.23 Water Requirement at Study Area (Case A)

	Nov	Dec	Jan.	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Total
Daddy 1 (*1)		23, 757	46	20.366	0	0	43,821	42,407	0	0	0	0	176,656
Paddy 2 (*2)	C	0	00	47,242	68, 575	58,204	8,713	30,603	63,247	55,211	7,525	0	357,337
Marze	1.071	988		0	0	0	0	0	0	0	0	1,010	3,287
Maige 2	C	0	747	1,425	1,827	1,165	0	0	0	0	0	0	5,164
You Rean	006	753	213	0	:		0	0	0	0	0	931	2,798
Sweet Potato	1:	4	2.850	0	0	0	0	0	0	0	0	4,566	18,073
Dotato	670	:		475	0	C	0	0	0	0	0	0	3,304
Vocato	386		80	C	0	0	0	0	0	0	0	403	1,200
Vestianie 1	3	255	149	970	749	225	0	0	0	0	0	0	993
Vegetable 2	2 057	1 771	797	011	0	0	0	0	1,949	3,454	2,782	1,329	14,139
Cacuad Nut	200	`}	797	1.530	1.993	2.507	3,786	3,214	519	0	0	0	14,346
Others	1.650	1.420	-	1,155	1,491	1,924	2,983	2,887	3,125	2,770	2,543	2, 131	25,358
Total		34 953	7.9	72 463	74 235	64,026	59,303	79,111	68.840	61,435	12,850	10,369	622,655

Note: (*1) Land Preparation (*2) Maintenance Water

Table B - 2.24 Water Requirement at Study Area (Case B)

												r : inn)	000 M3/
	Nov	Dec	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	0ct	Total
Paddy 1 (*1)	0	17,899	34,	15,345	0	0	35,798	34,643	0	0	0	0	138,573
Paddy 2 (*2)	0	0	13,573	35,594	51,667	37,705	6,564	25,000	51,667	38,538	6,148	0	266,455
Maize 1	5,357	1,179		0	0	0	0	0	0	0	2,959	6,864	16,359
Maize 2	0	0		190	244	155	0	0	0	0	0	0	689
Soy Bean 1	1,350	692	0	0	0	0	0	0	0	0	396	1,794	4,232
Soy Bean 2	0	0	0	400	1,052	1,339	1,024	0	0	0	0	0	က
S.Potato 1	6,943	4,950	460	0	0	0	0	0	0	0	359	8,262	20,974
S.Potato 2	0	0	0	950	2,524	3,257	4,943	2,392	0	0	0	0	14,067
Potato	2,812	2,441	686	0	0	0	0	0	0	0	0	2,826	9,069
Vegetable 1	1,029	ì	531	0	0	0	0	0	0	0	0	883	3,328
Vegetable 2	0	0	498	006	1,163	750	0	0	0	0	0	-	3,311
Vegetable 3	2,700	2,325	1,046	0	0		0	0	2,557	4,534	4,163	1,744	19,069
Ground Nut	0	0	0	1,105	2,879	3,621	5,469	2,646	0	0	0	0	15,721
0thers	1,650	1,420	1,278	1,155	1,491	1,924	2,983	2,887	3, 125	2,770	2,543	2,131	25,358
Total	21,841	31,792	53,363	55,639	61,019	48,753	56,782	67,568	57,349	45,842	16,567	24,503	541,017

Note : (*1) Land Preparation (*2) Maintenance Water

Table B -2.25 Water Requirement at Project Area (Case A)

						}			ì			(Unit: 1	1000 m3)
	Nov	Dec	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	0ct	Total
Paddy 1 (*1)	0	6,067	11,824	5,201	0	0	12,637	12,229	0	0	0	0	47,957
Paddy 2 (*2)	0	0	4,378	11,863	17,512	12,780	2,225	8,825	18,238	13,604	2,242	0	91,667
Maize		78	0	0	:	0	:	0	;	0	550	1,030	2,461
Soy Bean	112	24	0	0	0	0	0	0	0	0	69	149	355
Sweet Potato		288	0	0	0	0	0	0	0	0	439	1,129	2,723
Potato	· •	349	281	0	0	0	0	0	0	0	0	0	1,032
Vegetable 1	, , , ,	119	0	0	0	0	0	0	0	0	96	324	789
Vegetable 2		155	52	0	0	0	0	0	170	302	278	116	1,254
Ground Nut		0	0	194	505	635	959	464	0	0	0	0	2,757
Others	180	155	140	126	163	210	325	315	341	302	278	232	2,767
Total	2,796	7,234	16,675	17,384	18,179	13,625	16,146	21,833	18,750	14,208	3,951	2,981	153,763

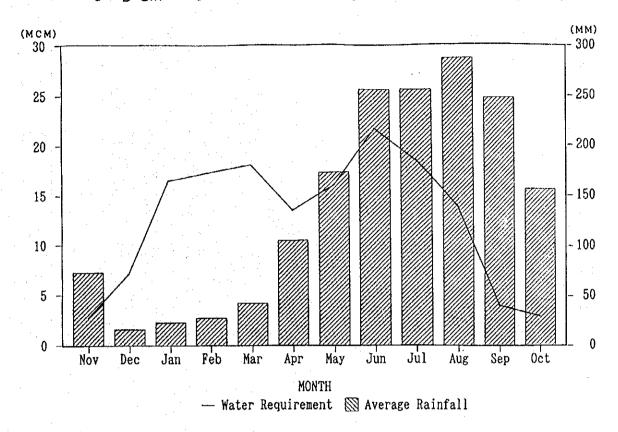
Note: (*1) Land Preparation (*2) Maintenance Water

Table B-2.26 Water Requirement at Project Area (Case B)

									• :)	(Unit: 1	: 1000 m3)
	Nov	Dec	Jan	Feb	Mar	Apr	May	Jun	Jul	any	dəş	0ct	Total
Paddy 1 (*1)	0	1,551	11,067	5,551	0	0	9,546	9, 238	0	0	0	0	36,953
Paddy 2 (*2)	0	Φ	1,227	8,439	13,778	11,694	1,751	6,667	13,778	12,027	3,278	0	72,638
Maize 1	804	407	0	0	0	0	0	0	0	0	517	1,030	2,757
Maize 2	0	0	199	380	487	311	0	0	0	0	0	0	1,377
Soy Bean	787	564	0	0	0	0	0	0	0	0	320	1,046	2,718
S. Potato 1	898	365	71	0	0	0	0	0	0	0	126	1,129	2,559
S. Potato 2	651	548	498	112	0	0	0	0	0	0	0	741	2,551
Potato	1,071	930	583	0	0	0	0	0	0	0	0	1,289	3,580
Vegetable 1	472	406	213	0	0	0	0	0	0	0	0	472	1,563
Vegetable 2	579	498	448	304	0	0	0	0	0	0	0	467	2,296
Vegetable 3	0	0	70	126	102	184	325	157	0	0	0	0	964
Ground Nut	0	0	0	382	966	1,254	1,893	916	0	0	0	0	5,442
Others	279	240	216	195	252	325	505	488	529	468	430	360	4,289
Total	5,511	5,511	14,299	15,489	15,615	13,767	14,020	17,467	14,306	12,496	4,671	6,535	139,687

Note: (*1) Land Preparation (*2) Maintenance Water

Figure B-2.17Average Rainfall and Water Requirement at Project Area (Case A)



FigureB-2.18 Average Rainfall and Water Requirement at Project Area (Case B)

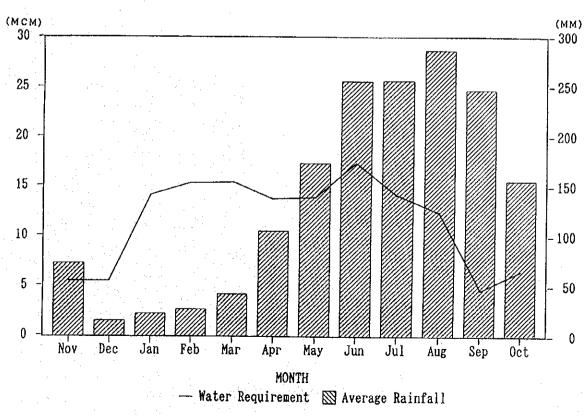


Figure B-2.19Average Rainfall and Water Requirement at Study Area (Case A)

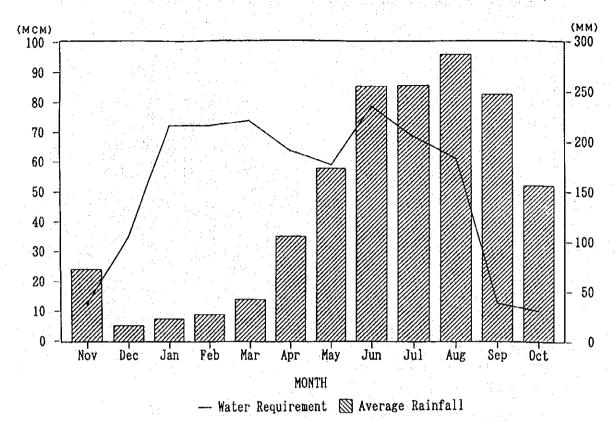


Figure B-2.20 Average Rainfall and Water Requirement at Study Area (Case B)

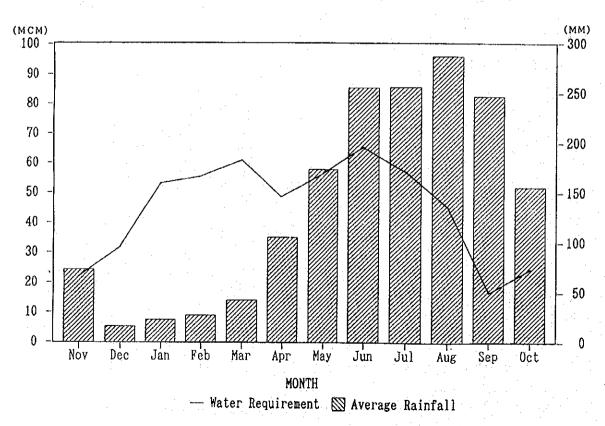


Table B-2.27 Average Monthly Discharge of Pho River at Quang Qu Station

(UNIT:M3/S) (CA:1,190km2) FEB MAR APR MAY JUN JUL AUG SEP OCT NOV DEC AVERAGE YEAR JAN 44.60 18.90 12.90 28.75 1960 6.08 5.28 6.22 4.53 10.10 10.10 51.20 79.80 95.30 13.40 24.09 8.77 7.62 10.80 19.50 9.93 30.80 23.60 45.40 66.90 31.70 20.70 1961 9.11 22.68 11.40 60.20 49.70 31.40 21.00 12.40 1962 11.90 8.54 7.549.33 39.60 6.27 14.70 | 15.00 | 46.30 | 58.50 | 36.20 | 18.70 | 27.80 12.00 21.19 6.27 6.52 5.96 1963 35.70 18.20 49.90 22.60 14.70 1964 9.37 8.38 8.12 11.30 56.40 34.40 49.40 23.00 1965 9.21 7.94 7.06 24.80 27.50 67.0038.10 25.10 19.10 17.00 9.09 5.33 6.98 47.30 53.60 33.20 23.40 21.40 11.60 6.76 19.54 1966 7.43 6.23 11.20 14.80 5.6317.10 16.30 30.00 13.00 1967 6.36 5.74 8.52 45.20 8.58 6.08 14.78 32.10 112.00 1968 4.58 5.69 6.289.40 14.60 28.90 40.00 29.00 16.10 8.53 25.60 17.50 | 14.50 45.30 19.80 10.10 6.76 5.40 9.72 35.90 86.20 30.10 23.91 1969 5.66 50.50 64.20 9.67 29.50 38.40 51.40 25.10 12.40 9.061970 8.60 7.52 6.48 7.46 7.10 7.26 24.80 26.60 95.30 126.00 46.30 34.20 16.40 12.30 34.30 1971 7.88 .26.80 17.80 107.00 58.30 30.20 18.00 12.40 27.03 1972 8.67 7.78 7.22 8.54 21.60 1973 8,60 9.59 15.40 18.50 34.50 69.10 | 83.60 | 112.00 | 33.50 16.80 11.60 35.25 9.81 25.10 8.29 7.25 39.00 39.20 24.70 32.60 12.30 8.02 18.76 1974 10.00 9.85 8.83 1975 9.86 7.50 21.10 53.00 89.00 26.70 38.40 47.20 20.00 | 11.30 8.81 | 28.28 6.4921.70 10.70 7.86 13.30 19.30 12.00 10.30 60.90 15.80 5.99 | 15.78 1976 6.01 5.4510.05 7.17 7.04 11.73 19.20 33.75 41.28 63.15 47.78 27.35 16.69AVERAGI 8.05 10.10 | 10.30 | 25.10 | 15.80 13.00 8.58 5.99 MIN 4.58 5.28 5.33 4.53 6.989.63 10.80 24.80 53.00 89.00 95.30 126.00 112.00 49.40 30.10 14.70 52.13 11.90 8.60

RUNNOFF COEFFICIENT=24.44*86400*365/1000000/(1661*1190*1000/1000000)
38.9 %

ANNUAL AVERAGE RAINFALL AT HANOI STATION = 1661 mm

TABLE B-2.28 Sediment Yield at THUONG CAT Station

						·												
Total	(mil.ton)	17.1	17.9	17.6	31.5	25.1	19.8	.27.8	21.9	45.9	26.3	41.4	27.9	35.6	54.1	27.3	_	29.9
Average		543.0	566.7	555.4	1000.0	795.2	627.4	879.2	695.1	1454.3	835.5	1309.6	883.6	1127.8	1713.9	863.0	1	948.2
	12	8.1	21.2	64.7	160.0	11.9	128.0	48.3	257.0	38.6	107.0	57.0	24.0	581.0	24.1	485.8	l	20.2
	I	33.9	267.0	50.7	341.0	52.3	1080.0	151.0	1380.0	188.0	309.0	559.0	134.0	111.0	340.7	2.793	1	93.8
	2	220.0	620.0	305.0	0 606	412.0	600.0	808.0	1240.0	2120.0	1020.0	1810.0	202.0	587.0	724.2	954.0	1	665.7
	ග	1130.0	1170.0	1320.0	1540.0	934.0	858.0	1480.0	454.0	3370.0	2300.0	2990.0	984.0	1550.0	3519.3	1596.0	_	2961.0
	∞	2200.0	2510.0	2370.0	5260.0	3300.0	2610.0	2900.0	1500.0	4340.0	4190.0	4460.0	6480.0	3340.0	9256.7	2410.4	1	3315.7
	7	2110.0	1080.0	1640.0	1290.0	2560.0	1870.0	3610.0	2140.0	5410.0	1450.0	4270.0	1910.0	5700.0	4083.3	3170.9	-	2665.5
	9	724.0	795.0	819.0	2140.0	1910.0	271.0	1260.0	1210.0	1710.0	382.0	1100.0	636.0	930.0	1923.2	903.1	1	1467.8
	5	5.5	224.0	29.0	86.7	114.0	11.0	166.0	63.5	49.8	103.0	196.0	67.9	509.0	318.9	103.1	1	68.3
}	4	2.3	26.5	2.5	63.1	38.0	0.9	16.0	19.9	14.8	27.3	82.3	7.5	17.9	73.0	13.6	1	9.6
	က	3.0	19.6	5.5	33.6	16.3	11.0	5.9	4.2	7.0	12.6	12.2	3.3	6.6	19.1	10.1	1	8.9
	2	14.0	5.3	10.5	39.0	12.7	9.9	7.1	3.5	11.4	15.4	32.9	2.8	12.7	37.6	19.6	-	13.8
	1	5.3	7.2	11.7	47.9	97.8	7.1	22.4	11.3	34.7	20.1	46.9	6.7	16.1	52.7	25.9	1	22.0
month	year	1958	1959	1960	1961	1962	1963	1964	1965	1966	1967	1968	1969	1970	1971	1972	1973	1974

TABLE B-2, 29 Sediment Yield at THUONG CAT Station

month													
rear		2	က	4	3	9	2	0 0	6	10	11	12	Average
1958	52.3	103.0	50.0	53.2	58.9	1170.0	1290.0	991.0	0.008	328.0	114.0	51.9	423.2
1959	60.2	58.6	123.0	148.0	507.0	657.0	710.0	1080.0	760.0	0.699	578.0	93.8	456.1
1960	66.1	70.0	54.2	42.3	189.0	930.0	882.0	1030.0	733.0	343.0	120.0	187.0	388.2
1961	241.0	203.0	158.0	172.0	391.0	1670.0	1090.0	1910.0	852.0	684.0	421.0	367.0	682.8
1962	261.0	61.0	120.0	248.0	361.0	1220.0	1250.0	1710.0	865.0	0.909	176.0	77.3	583.3
1963	59.2	67.2	112.0	72.8	104.0	0.699	1300.0	1340.0	819.0	704.0	939.0	350.0	547.7
1964	117.0	57.6	63.2	114.0	491.0	1020.0	1390.0	1420.0	0.808	553.0	216.0	120.0	541.5
1965	54.6		36.6	75.7	204.0	818.0	1020.0	0.606	504.0	1200.0	1190.0	539.0	551.2
1966	144.0		73.7	125.0	31.0	1080.0	5290.0	1850.0	1560.0	1720.0	309.0	138.0	1043.3
1967	104.0	93.9	107.0	181.0	363.0	578.0	1160.0	1900.0	1490.0	1090.0	532.0	296.0	662.1
1968	180.0	157.0	59.8	256.0	421.0	905.0	1640.0	1680.0	1410.0	1350.0	624.0	201.0	742.6
1969	62.8		44.6	86.1	320.0	0.968	1170.0	1640.0	724.0	320.0	238.0	109.0	473.3
1970	98.8		116.0	136.0	781.0	782.0	1690.0	1440.0	852.0	645.0	239.0	842.0	648.5
1971	163.7		100.4	252.8	490.9	1227.4	1401.7	1829.7	1150.5	529.7	420.1	77.1	651.8
1972	106.6	104.1	68.3	75.6	186.9	807.3	1473.0	1128.0	977.6	738.4	722.2	583.5	582.9
1973	i	ı	1	ı	ŀ	1	-	1	ı	1	1	1	ı
1974	107.7	988.6	84.9	71.9	196.9	1157.6	1320.8	1644.9	1376.0	599.2	213.4	98.3	582.7

Water Requirement for Irrigation

[BASED ON FAO]

(1)Upland Crop

By modified penman method

(Unit:mm)

Month	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	0ct	Nov	Dec
ETO	2.1	2.2	2.6	3.4	5.1	5.1	5.4	4.8	4.5	3.7	2.9	2.4
Soy bean	1.5	1.6	1.9	2.5	3.7	3.7	3.9	3.5	3.3	2.7	2.1	1.7
S.Potato	2.0	2.0	2.4	3.2	4.7	4.7	5.0	4.5	4.2	3.4	2.7	2.2
Potato	1.8	1.9	2.3	3.0	4.5	4.5	4.7	4.2	3.9	3.2	2.5	2.1
Maize	1.8	1.9	2.2	2.9	4.3	4.3	4.6	4.1	3.8	3.1	2.5	2.0
G.Nuts	1.6	1.7	2.0	2.6	3.8	3.8	4.1	3.6	3.4	2.8	2.2	1.8
Vegetable	1.8	1.8	2.1	2.8	4.2	4.2	4.4	3.9	3.7	3.0	2.4	2.0

(2)Paddy

Based on MWR Data(See next page)

Land Soaking 120 mm Standing water 70 mm 190 mm

Average consumptive use 5.5 mm

Percoltion rate 1.5 mm

7.0 mm

(3) Irrigation Efficiency

Taking consideration of well techniqued water management, the following efficiency is adopted based on the FAO paper.

For Paddy

E=0.9(conveyance efficiency)*0.7(water application eff.)=0.63 For Upland Crop

E=0.8(conveyance efficiency)*0.7(water application eff.)=0.56

Table B-2.30 Computation of Evapo-Transpiration

SMAN HORLOGG +		I HPROVEMENT		PROJECT OF	DRAINAGE		OR IN 8(SYSTEM IN SOUTH BAC DUONG AGRICALTURAL	DECONG	AGRICAL		AREA	
* METEOROLOGICAL STATION * LATITUDE (-: SOUTHERN HEMISPHERE * ALTITUDE * METHOD OF COMPUTATION	SPHERE)	EAMOI 21.01 5.00 PENHAN METHOD PETECHAN METHOD	METHOD W+(1-M)*F(U)*	**************************************	- MM/DAY	<u> </u>						
* HEIGHT OF WIND MEASURMENT (CORRECTION PACTOR OF WIND-SPR * RATIO OF UDAY/UDAILY * RATIO OF UDAY/UNIGET	(M) SPRED	1.30	9	N N	E E			JULY	AUG	SEPT	oct.	NON	3
* MAXIMUM RERATIVE HUMIDITY RADIATION AT LATITUDE MAXIMUM POSSIBLE SUNSHINE	(*) (MR/DAY) (BOUR)		12.5	97.0 14.3 12.0	96.0 15.5 12.6	95.0 16.3 13.1	95.0 16.4 13.4	955.0 15.0 4.0 4.0	96.0 15.8 12.8	96.0 14.7 12.3	96.0 13.1 11.7	95.0 11.3 11.1	95.0 10.4 10.8
AVERAGE YEAR OF 1960-1992	TIND	JAK	FEB	MAR	APR	KAY	JUNE	JULY	AUG	SKPT	OCT.	NOV	DEC
gs.	(CC)	36,3	17.1	20.0	23.7 2.9	27.4 6.1	28.8 5.8	29.1 6.5	283.7	27.5	24.8 5.3	21.3	18-0 4.1 78:4
* MEAN KERATIVE HUMIDIT * MEAN WIND-SPEED VAPOUR PRESSUR	(XOX/DAY)	168.0 18.56	19.52	23.40	192.0 29.29	189.0 36.54	158.0 39.64	40.33	132,0 38.95	135.0 36.75	146.0 31.32 25.40	142.0 25.35 20.03	148.0 20.60 16.15
MIND FUNCTION F(U)	(Maken)	61.64	6.6	69. 69.	. 73 . 73	76.	95.7.7	27.59 8.7.79	54	52.	57.	70	.57
SOLAR RADIATION RS	(MOK/DAY)	3.94	2.51	2.93	3.70	5.07	5.02	12.0 12.0 13.0 13.0 13.0 13.0 13.0 13.0 13.0 13	4.89	4.39	3.77	2.88	7.7
SPEED UDAY CTOR C RATION ET	(M/BBC)	1.90 .96 2.10	2.17	2.60	2.17 .99 3.40	2.13 5.03 10.10	1.05 1.05 5.10	1.06	4.4 90.1 90.4	1.05 4.50	3.70	2.90	2.40

Water Requirement for Irrigation

[BASED ON M.W.R.]

The water requirement has been calculated based on the ducument of "Irrigation Coefficient in Hai Hung Province" prepared by the Ministry of Water Resources, Institute of Water Resources Planning and Management on December 1992.

1.Net Water Requirement

(1)Paddy

- Pre-planting water requirement 1,700/86.4/20=0.984 1/s/ha
- Irrigation water requirement after planting 400/86.4/6=0.772 1/s/ha

(2)Upland Crop

400/86.4/8=0.578 1/s/ha

2. Irrigation Efficiency

0.65--0.70 (Average: 0.675)

3. Gross Water Requirement

If the value of 0.675 is adopted as irrigation efficiency, the gross water requirement is shown as follows.

- Pre-planting water = 0.984/0.675=1.46 l/s/ha
- After planting = 0.772/0.675=1.14 1/s/ha
- Upland Crop = 0.578/0.675 = 0.86 1/s/ha

B-3.1 Flood Runoff Method

The mechanism of surface runoff is generally classified into two parts; namely (1) the behaviour of rain water which flows down a sloping surface and pours directly into river channel and (2) the behaviour of lateral inflow which pours into such a stream. As a simplified stream condition, the behaviour of unsteady flow in an open channel with distributed lateral inflow along a channel is studied hydraulically to establish the basic relationship between the rate of inflow and runoff in a stream or on a sloping surface. Hydrographs under this simplified condition are easily computed for both laminar and turbulent flows, and the hydraulic character of hydrographs resulting from simulated inflow at a given rate are investigated. The characteristic curve method was employed to express this phenomenon. Below is a brief explantion.

On the other hand, if the law of resistance of Manning's type is used, unsteady flow in an open channel with a given rate of lateral inflow would be expressed, for the practical purposes, by the following equations:

$$A = n \times I^{-1/2} \times R^{2/3} \times Q = kQ^p \text{ and}$$

$$A = n \times I^{-1/2} \times R^{2/3} \times Q = kQ^p \text{ and}$$

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$$A = n \times I^{-1/2} \times R^{2/3} \times Q = kQ^p \text{ and}$$

To solve the above equation, the characteristic curve method (So-called kinematic run off model method) is applied and the characteristic curves are given as follows:

$$\frac{dX}{I} = \frac{dt}{dA/dQ} = \frac{dt}{pkQ^{p-1}} = \frac{dQ}{q}$$

where, A: cross-sectional area of flow (sq.m)

n: Manning's roughness coefficient

I: water surface slope of flow

R: hydraulic radius (m)

Q: discharge (cu.m/sec)

k,p: constants

t: time

X: distance along channel (m)

q: lateral inflow per unit length of channel
 (cu.m/sec/m)

Based on the above two equations of kinematic runoff model method, the characteristic curves can be solved by applying the following equation; as

$$dx/dt = Q^{1-p}/pk$$
.
 $qdt = pkQ^{p-1}$ or $qt = kQ^p$ + constant, and
 $qdx = dQ$ or $qX = Q$ + constant

Taking the constant = 0, the flow condition is expressed for a given magnitude of lateral inflow q, as;

$$t = kQ^{p}/q$$
 and
 $t = kXQ^{p-1}$

When q=0, the above equations is expressed on a characteristic curve method given above that A= constant and Q= constant = $(A/K)^{1/p}$. The flow condition is shown as follows.

$$X = (Q^{1-p}/pk)t.$$

Some Consideration on Effect of Storage on a Paddy Plot.

The time lag of concentration of runoff is generally recognized to be remarkable for drainage area mainly composed of low flat paddy because of storage capacity on a paddy plot. A paddy plot surrounded by levees with certain depth of flooding water can be regarded as a small reservoir and, therefore, the conception of simplified reservoir operation could be introduced to take into account the effect of rain water deposit on a paddy plot.

The storage function is introduced to calculate the specific runoff capacity from a paddy plot by the following equation:

$$\frac{dV}{dt} = I - 0$$

where V denotes storage on a paddy plot; I and O denote inflow into and outflow from a paddy plot; respectively, and denotes time. The above equation can be divided by the water surface area on a plot, (A,) and then transformed as follows:

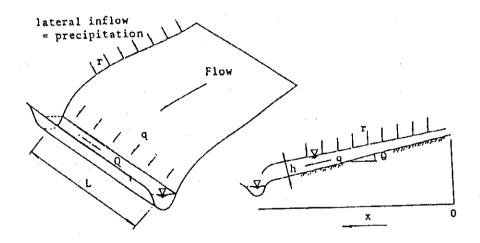
where H represents the ponding depth on a plot; i as specific inflow corresponding to effective rainfall on a plot, and o as specific outflow corresponding to the specific runoff capacity from a paddy plot. A different another type of equation is constructed to solve the above equation by a computer as follows;

$$H_{t+1} = H_t + (RE_t, t+1 - \frac{o_t - o_{t+1}}{2}) \triangle t$$

where RE_t, $_{t+1}$ represents effective rainfall between time t and t+1 and $\triangle t$ represents the time interval given for computation. The specific runoff capacity from paddy fields is thus computed at corresponding time (t), which is then considered as a lateral inflow of drainade canal or stream.

Application of the Characteristic Curve Method to a Particular Case

(1) For a Slope



- In the case when $r \neq 0$

 $t = kq^p/\delta r$

 $t = kXq^{p-1}$

- When r = 0

$$t = pkX/q^{1-p} = 0.6q^{-0.4}(N/I^{1/2})^{0.6}X$$

where, δ : conversion rate from mm/hr to $m^3/\text{sec} = 0.2778 \times 10^{-6}$

r : effective rainfall (mm/hr)

q: discharge per unit width of slope (m³/sec/m)

N: equivalent roughness coefficient of slope

I : slope = sin
X : flow distance

- (2) For River or Channel: As stated previously with theoretical concept.
 - (3) For Paddy Field

- for ditch

$$A_m = kQ_m^p$$

$$\frac{\Im A_m}{\Im t} + \frac{\Im Q_m}{\Im X} = (2b0)\alpha, \text{ and}$$

- for lateral drainage canal

$$A_b = kQ_b^p$$

$$\frac{\Im A_b}{\Im t} + \frac{\Im Q_b}{\Im X} = \frac{Qm}{2b}$$

where, A_m , Q_m : flow area and discharge in a ditch

 A_b , Q_b : flow area and discharge in a laretal canal

k,p : constants

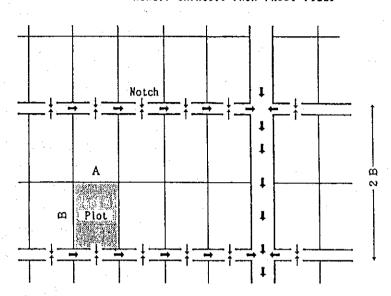
 α : = 0.2778 x 10⁻⁶

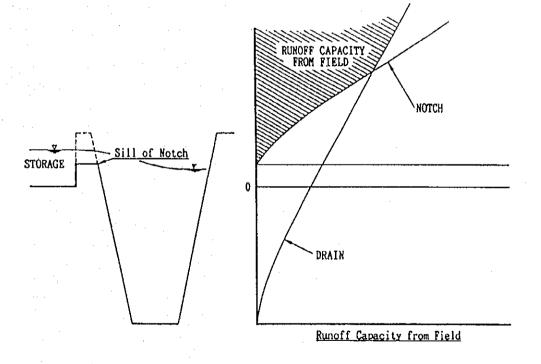
b : see Figure

O: runoff capacity per unit area (mm/hr)

Following figure shows the concept of runoff capacity from the paddy field.

RUNOFF CAPACITY FROM PADDY FIELD





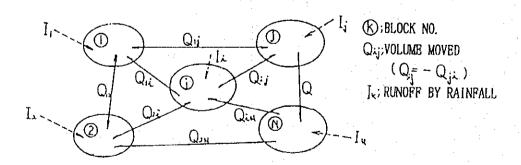
B-3.2 Continuous Reservoir Model

Method of Analysis

In a watershed, runoff caused by rainfall is governed by the topographical characteristics and appeared in the river or artificial channel with some delay in time. The runoff in the channel is then governed by the hydraulics of the channel. In case flow capacity of the channel does not meet the runoff or in case the flow in the channel is restricted due to backwater from the downstream, the runoff remained from the discharge causes inundation at the place. Such phenomenon emerges not only in channels but also in culverts across roads, siphons, gateways, drainage pumping stations, etc. Such situations are commonly observed in the low flat areas. In order to precisely simulate the phenomenon, the so-called continuous reservoir model method can be applied. The water balance study is therefore made below.

Continuous Reservoir Model

The "continuous reservoir model" assumes that each of a number of blocks divided into from a watershed is a reservoir with storage functions characterized by H-V and H-A curves. Drainage facilities such as channels, culverts, siphons, gateways, pumps, weirs, etc. are connecting the twe reservoirs. The movement of water between the two reservoir is generated by the difference in water levels and governed by the hydraulic functions and dimensions of the facilities, causes difference in stored volumes. The differences in the storage are then calibrated by the differences in the water levels to pursue further changes in the storage and water levels. The conceptual image of the model is shown below.



CONCEPTUAL IMAGE: CONTINUOUS RESERVOIR MODEL

A continuity equation for an arbitrary i'th block is given as follows:

$$\frac{d v_i}{d t} = I_{\lambda} - \sum_{j=1}^{H} Q_{\lambda j} (\lambda = 1) \qquad (1)$$

where V_i = storage volume in i'th block,

I_i = flow into I'th block; i.e. direct runoff within the block,

 $Q_{i,j}$ = flow from i'th block to j'th block (reverse is negative), and

t = time

For application of the model to the blocks in a watershed, the change of water level in a unit time of calculation interval, Δt is generally infinitesimal. Accordingly, the equation below may be applied.

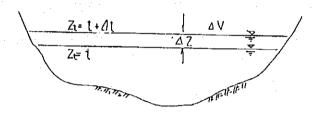


IMAGE OF CROSS-SECTION OF BLOCK RESERVOIR

$$\Delta V = A \cdot (Z_{\ell-\ell}) \cdot \Delta Z \quad \cdots \quad (2)$$

where

 ΔV = incremental storage volume,

A = inundated areas (given by a function of Z),

 ΔZ = incremental inundated water level, and

Z = inundated water level.

Substituting the equation (1) and (2), the following equation may be used then

$$\frac{dZ_i}{dt} = \frac{1}{A(Z_A)} (I_i - \sum_{j=1}^{d} Q_{ij}) \qquad (3)$$

Changes in water levels may thus be calculated. In the above equation, $A(Z_1)$ and I_1 are derived from the water level-storage curve and runoff analysis in each block respectively, while $Q_{1,j}$ are derived from water level differences between the reservoirs and functions and dimensions of the hydraulic facilities.

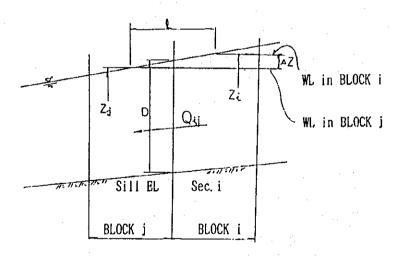
The equation (3) is formed simultaneously in each of whole N blocks, and therefore the study in changes of inundated water levels is to solve N numbers of simultaneous differential equations. Some of numerical analysis methods such as Runge-Kutter-Gill method for instance, can be employed.

Flow Condition Equation

Each reservoir block is connected by an arbitral number of drainage facilities and the flows through them are governed by water level differences. Flow condition equations are equations to calculate the flows through the various drainage facilities, and are expressed in terms of water level difference by substituting hydraulic functions and dimensions of the facilities.

The flow condition equations for various drainage facilities are given below.

i) River channel



The flow between i'th and J'th blocks is assumed to run by gravity by the water level difference ΔZ and by Manning's Formula, then

$$Q_{ij} = \frac{A_{ij}}{\Omega} \cdot R_{ij}^{ij} \cdot T_{ij}^{ij}$$

$$= \frac{1}{\Omega} \cdot A_{ij} \cdot R_{ij}^{ij} \cdot (\frac{\Delta Z_{ij}}{L_{ij}})^{ij}$$

$$= \frac{1}{\Omega \cdot L_{ij}} \left(A_{ij} \cdot R_{ij}^{ij} \right) \cdot \left[A Z_{ij} - \dots - \dots \right]$$
(1)

Generally in the flow channels, $AR^{2/3}$ equals aD^b (a and b are the constants while D is the water depth) and substituting this for the equation (1), then

$$Q_{ij} = \frac{1}{\prod I_{ij}} \cdot a_{ij} \cdot p_{ii}^{bij} \cdot \sqrt{\Delta Z_{ij}}$$

$$= \frac{1}{\prod \sqrt{I_{ij}}} \cdot a_{ij} \cdot (\frac{Z_{i} + Z_{i}}{2} - SILL_{ij})^{bij} \cdot \sqrt{Z_{i} - Z_{j}} \cdot \dots (2)$$

In the equation (2) above, I_{ij} , a_{ij} , b_{ij} and Sill-EL, are identical in each block so that Q_{ij} can directly be calculated from water levels in the blocks by giving those identical figures in the equation in advance.

In the above equations (1) and (2), Z_i and Z_j are the water levels in i'th and j'th blocks, respectively, $a_{i,j}$ and $b_{i,j}$ are the constants, n is the Manning's coefficient of roughness, ΔZ is the water level difference, 1 is the distance between the two blocks, and $D_{i,j}$ is the water depth.

B-3.3 Outputs of Water Balance Computation

(1) Tan Chi Only Case

*****	RUNOFF	******* INSIDE	OUTSIDE	DRAINAGE	*****		DATION	* ********	*****	*****	*****
TIME	DISCHARGE	W.L.	W.L.	DISCHARGE	PONDAGE	DEPTH	AREA	DRAINAGE	DISCHARGE of	FACILITIES	$(1000m^3)$
		(EL.m)	(EL.m)	(1000m³) (1000*cum) (m)	(ha)	1 2	5 G		
*****	********	*****	*******	*******	******	*****	*******		*********	********	******
1	.00	3 338	7.910	103.75		.000	223.45	.00 .00	8.25112.00		
. 2	.00	3.275	7.931	103.98	462.1	.000	182.41	.00 .00	8.01112.00		*
3	.00	3.214	7.951	101.21	360.9	.000	142.46	.00 3.00	7.78112.00		
4	.02	3.166	7.972	79.69	281.2	.000	111.01	00 24 74	7.56112.00		
5	. 05	3.123	7.992	72.28	209.0	.000	82.50 55.08	.00 32.91	6.81112.00		
6	.09	3.081	8.012	69.57	139.5 77.6	,000	30.64	.00 36.92 6.17 39.23	5.50112.00		
7 8	.18	3.044	8.033 8.053	62.09 44.45	33.4	.000	13.22	24.66 39.15	4.51112.00 3.74112.00		
9.	.57	2.896	8.053	30.05	4.0	.000	1.58	39.53 42.06	3.37112.00		
10	1.04		8.094		3.7	.000	1.49	60.47 50.02	3.37112.00		
11	1.94	2.859		2.08	3.6	.000	1.44	61.64 47.24	3 37112 00		
12	3.27	2.831	8.135	3.55	3.3	.000	1.32	63.61 38.11	3.37112.00		
. 13	4.77	2 902	8.155	4.06	4.0	.000	1.61	63.91 20.48	3.37112.00		
14	6.08	2.922	8.176	5.88	4.2	.000	1.69	52.53 18.98	3.37112.00		
15	6.98	2.901	8.196	7.19	4.0	.000	1.60	43.40 18.11	3.37112.00		
16	7.56	2.879	8.217	7.77	3.8	.000	1.52	41.90 16.13	3.37112.00		+,
17	7.93	2.861	, 8.237	8.12	3 6	.000	1.44	40.34 14.72	3,37112.00		
18	8.14	2.845	8.257	8.31	3.4	.000	1.38	38.74 13.73	3.37112.00		
19	8.20	2.817	8.278	8.47	3.2	.000	1.27	36.50 12.60	3.37112.00	1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 -	
20	8.13	2.793	8.298	8.37	2.9	.000	1.17	33.78 11.46	3.37112.00		
21	7.98	2.771	8.319	8.20	2.7	.000	1.08	31.13 10.66	3.37112.00		•
22	7.80	2.751	8.339	8.00	2.5	.000	1.00	28.55 10.09	3.37112.00		• *
23	7.68		8.360	7.86	2.3	.000	. 93	26.03 9.72	3.37112.00		
24	7.75	2.717	8.380	7.90	2.2	.000	.87	23.59 9.58	3.37112.00	•	•
25	8.06	2.705	8.414	8.18	2.0	.000	.82	21.28 9.72 19.17 10.11	3.37112.00		
26 27	8.58 9.27	2.695 2.689	8.448 8.482	8.68 9.33	2.0 1.9	,000 ,000	.78 .75	17.31 10.70	3.37112.00 3.37112.00		
28	10.10	2.685	8.517	10.13	1 9	.000	.74	15.75 11.45	3.37112.00		
29	11.14	2.685	8.551	11.14	1 9	.000	.74	14.54 12.34	3.37112.00	*	
30	12.45	2.689	8.585	12.41	1 9	.000	.76	13.70 13.39	3.37112.00		
31	14.12	2.698	8.619	14.03	2.0	.000	.79	13.24 14.65	3.37112.00		-
32	16.27	2.712	8.653	16.13	2.1	.000	.85	13.18 16.23	3.37112.00		
33	19.94	2.739	8.687	19.67	2.4	.000	.96	13.54 18.51	3.38112.00		
34	27.48	2.641	8.722	28.46	1.4	.000	.56	6.52 29.26	3.52112.00	*	
35	39.62	2.674	8.756	39.30	1.7	.000	.69	.00 43.18	3.94112.00		
36	52.85	2.786	8.790	51.73	2.9	.000	1.14	.00 51.83	4.52112.00		
37	62.58	2.906	8.824	61.38	4.1	.000	1.62	.00 45.46	5.16112.00		
38	67.34	2.955	8.858	66.84		.000	1.82	.00 39.36	5.81112.00		
39	68.49	2.989	8.892	68.15	4.9	.000	1.96	.00 37.41	6.43112.00		
40	67.77	3.001	8.927	66.76	5.9	.000	2.36	.00 38.20	7.04112.00		
41	66.36	3.003	8.961	62.34	9.9	.000	3.94	.00 42.21	7.45112.00		
42	64.95	3.010	8.995 9.029	53.33 48.90	21.5 36.5	.000	8.53 14.41	6.20 44.93 9.10 46.38	7.53112.00 7.62112.00		
43	63.81 62.95	3.019	9.063	47.60	51.8	.000	20.47	9.32 47.37	7.70112.00	•	
44 45	62.44	3.037	9.097	47.12	67.1	.000	26.52	8.96 48.12	7.80112.00		
46	62.23	3.046	9.132	47.08	82.3	.000	32.50	8.31 48.70	7.90112.00		
47	62.05	3.055	9.166	47.22	97.1	.000	38.35	7.57 49.20	8.00112.00		
48	61.64	3.064	9.200	47.32	111.4	.000	44.00	6.95 49.63	8.10112.00		
49	60.99	3.072	9.203	47.25	125.2	.000	49.43	6.54 50.00	8.20112.00		
50	60.21	3 080	9.207	47.02	138.3	.000	54.63	6.36 50.31	8.30112.00		
51	59.43	3.088	9.210	46.71	151.1	000	59.65	6.33 50.56	8.40112.00		
52	58.71	3.095	9.213	46.37	163.4	.000	64.52	6.37 50.76	8.49112.00		
53	58.07	3.103	9.217	46.04	175.4	.000	69.27	6.46 50.91	8.58112.00		
54	57.52	3.110	9.220	45.74	187.2	.000	73.93	6.56 51.02	8.68112.00		
55	57.05	3, 117	9.223	45.46	198.8	.000	78.50	6.66 51.10	8.77112.00		
56	56.67	3.124	9.227	45.22	210.3	.000	83.02	6.75 51.16	8.86112.00		
57	56.49	3.130	9.230	45.04	221.7	.000	87.54	6.79 51.22	8.95112.00		
58	56.65	3.137	9.233	44.98		.000	92.15	6.70 51.28	9.04112.00		
59.	57.08	3.145	9.237	45.04	245.4	.000	96.90	6.46 51.37	9.12112.00		
60	57.50	3.152	9.240	45.17	257.7	.000	101.76	6.14 51.48	9.20112.00		
61	57.70	3 160	9.243	45.28	270.2	.000	106.66	5.84 51.61	9.27112.00		
62 63	57.63	3 167	9.247	45.28	282.5	.000	111.54 116.36	5.67 51.72 5.65 51.81	9.32112.00		
63 64	57.38 57.03	3.174 3.182	9.250	45.16 44.98	294.7	,000	121.12	5.77 51.84	9.41112.00		
65	56.67	3.189	9.257		306.8 318.7	.000	121.12	5.96 51.81	9.43112.00		
66	56.32	3 196	9.260	44.62	330.4	.000	130.42	6.22 51.71	9.45112.00		
67	55.99	3 203	9.263	44.49	341.9	.000	134.96	6.52 51.53	9.46112.00		
68	55.66	3.209	9.267	44.40	353 1	,000	139.41	6.86 51.28	9.46112.00	4	
69	55.35	3.216	9.270	44.36	364.1	,000	143.75	7.21 50.97	9.45112.00		
70	55.03	3.222	9.273	44.37	374.8	,000	147.96	7.58 50.61	9.44112.00		
71	54.70	3.229	9.277	44.40	385.1	.000	152.02	7.97 50.21	9.42112.00		
72	54.34	3 235	9.280	44.43	395 0	.000	155.94	8.40 49.78	9.38112.00		

****		*******		*****	
***	INTINDATION	ANALYSIS	BLOCK NO	1 ***	CAS

****	RUNOFF	INSIDE	OUTSIDE	DRAINAGE	*****	TAILIN	DATION	*****		******	*****
TIME	DISCHARGE		W.L.	DISCHARGE	PONDAGE		AREA	DRAINAGE	DISCHARGE of	FACILITIES	(1000m ³)
(hr)	(1000m ³)			(1000m³) ((ha)		5 6		(1000m)
****		******	*******				*******		*****	********	*******
73	53.96	3.240	9.262	44.47	404.5	.000	159.68	8.86 49.32	9.34112.00		
.74	53.57	3.246	9.243	44.52	413.5	.000	163.25	9.35 48.84	9.29112.00		
75	53.20	3.251	9.225	44.60	422.1	.000	166.65	9.88 48.28	9.24112.00		
76	52.84	3.256	9.207		430.3	.000	169.85	10.45 47.66	9.17112.00	1.	
77	52.50	3.260	9.188	44.85	437.9	.000	172.87	11.06 46.99	9.10112.00		
78	52.19	3.265	9.170	44.99	445.1	.000	175.72	11.68 46.31	9.02112.00	of the second second	
79	51.89	3.269	9.152	45.14	451.9	.000			8.93112.00		
80	51.58		9.133	45.33	458.1	.000	180.84	12.94 44.89	8.84112.00		
81	51.22	3.276	9.115	45.54	463.8	.000	183.09	13.56 44.15	8.75112.00		-
. 82	50.79	3.279	9.097	45.76	468.8	.000	185.07	14.21 43.38	8.65112.00		• •
83 84	50.30	3.282	9.078	45.98	473.1	.000	186.77	14.88 42.59	8.54112.00	4.5	
85	49.78 49.25	3.284	9.060	46.17 46.35	476.7	000	188.20	15.60 41.79	8.44112.00		
86	48.73	3.286 3.287	9.042			.000		16.33 40.98	8.33112.00		•
87	48.21	3.288	9.005	46.52 46.67	481.9 483.4	.000		17.09 40.17	8.22112.00		
88	47.68	3.288	8.987	46.79	484.3	.000	190.82 191.18	17.85 39.37	8.11112.00		
89	47.13	3.289	8.968	46.88	484.5	.000		18.62 38.59 19.38 37.84	8.00112.00		
90	46.55	3.288	8.950	46.92	484.2	.000	191.13	20.14 37.15	7.90112.00		
91	45.91	3.288	8.932	46.91	483.2	.000	190.73	20.14 37.15	7.79112.00 7.68112.00		* *
92	45.21	3.287	8.913	46.84	481.5	.000	190.09		7.57112.00		
93	44.47	3.285	8.895	46.75	479.3	.000	189.19	22.46 35.37		1.0	
94	43.73	3.283	8.877	47.02	476.0	.000	187.90		6.79112.00		
95	42.99	3.281	8.858	47.14	471.8	.000	186.26	24.23 34.51	6.12112.00		
96	42.25	3.278	8.840	47.10	467.0	.000	184.34	25.18 34.15	5.57112.00	1	
97	41.52	3.275	8.823	46.98	461.5	.000	182.18	26.11 33.80		and the second	
98	40.78	3.271	8:805	46.83	455.5	.000		27.01 33.44	5.11112.00		
99	40.06	3.267	8.788	46.65	448.9	.000	177.20	27.89 33.07	4.38112.00		
100	39.36	3.263	8.770	46.47	441.8	.000	174.39	28.74 32.70	4.09112.00	4	et jaron taring
101	38.66	3.258	8.753	46.28	434.1	.000	171.38	29.57 32.32	3.82112.00	100	V
102	37.95	3.253	8.735	46.09	426.0	.000	168.17	30.38 31.93	3.59112.00	100	100
103	37.24	3.248	8.718	46.36	416.9	.000	164.57	31.21 31.58	3.40112.00	The second	
104	36.53	3.242	8.700	46.34	407.1	.000	160.70	32.14 31.32	3.37112.00	and the second	
105	35.82	3.236	8.683	45.86	397.0	.000	156.74	33.01 31.01	3.37112.00	100	
106	35.12	3.230	8.665	45.49		.000	152.65	33.81 30.64	3.37112.00		
107	34.44	3.223	8.648	45.17	375.9	.000	148.41	34.57 30.26	3.37112.00	and the second	
108	33.76	3.217	8.630	44.87	364.8	.000	144.03	35.29 29.89	3.37112.00		
109	33.10	3.210	8.613	44.57	353.4	.000	139.50	36.00 29.55	3.37112.00		
110	32.44	3.202	8.595	44.28	341.5	.000	134.83	36.68 29.23	3.37112.00	10 to	
111	31.79	3.195	8.578	43.98	329.3	.000	130.02	37.34 28.93	3.37112.00	**	
112	31.16	3.188	8.560	43.67	316.8	.000	125.08	37.97 28.66	3.37112.00		7.4
113	30.55	3.180	8.543	43.38	304.0	.000	120.01	38.59 28.40	3.37112.00		
114	29.95	3.172	8.525	43.14	290.8	.000	114.81	39.17 28.11	3.37112.00		
115	29.35	3.164	8.508	43.12	277.0	.000	109.38	39.68 27.65	3.37112.00		
116	28.55	3.155	8.490	43.49	262.1	.000	103.48	40.16 26.85	3.37112.00		
117	27.22	3.145	8.473	44.03	245.3	.000	96.85	40.71 25.80	3.37112.00		
118	25.36	3.133	8.455	44.19	226.5	.000	89.42	41,45 24.94	3.37112.00		•
119	23.56 22.30	3.121	8.438 8.420	43.81	206.2	.000	81.42	42.38 24.42	3.37112.00	The second second	
120	21.60	3.109 3.096		43.15	185.4	.000	73.19	43.38 24.13	3.37112.00		
121 122	21.17	3.084	8.401 8.382	42.45 41.84	164.5 143.9	.000	64.96 56.81	44.33 23.92 45.18 23.71	3.37112.00		•
123	20.82	3.071	8.362	41.34	123.3	.000	48.71		3.37112.00		
124	20.42	3.059	8.343	40.90	102.9	.000	40.63	45.94 23.49 46.63 23.27			
125	19.87	3.046	8.324	40.47	82.3	.000	32.49		3.37112.00 3.37112.00		
126	19.20	3.034	8.305	40.01	61.4	.000	24.28	47.88 22.97	3.37112.00		
127	18.60	3.021	8.286	39.52	40.5	.000	16.02	48.48 22.88	3.37112.00		
128	18.09	3.009	8.267	39.07	19.5	.000	7.74	49.05 22.80	3.37112.00		
129	17.38	2.951	8.247	32.42	4.5	.000	1.80	52.37 26.16	3.37112.00		•
130	16.35	2.929	8.228	16.56	4.3	.000	1.72	60.38 34.03	3.37112.00		
131	15.07	2.855	8.209	15.81	3.6	.000	1.42	65.88 29.30	3.37112.00		
132	13.68	2.765	8.190	14.58	2.7	,000	1.06	74.72 18.46	3.37112.00	100	
133	12.36	2.746	8.171	12.56	2.5	.000	. 98	75.03 12.12	3.37112.00		
134	11.32	2.729	8.152	11.49	2.3	.000	. 92	74.85 10.11	3.37112.00		
135	10.58	2.720	8.132	10.67	2.2	.000	.88	74.25 8.67	3.37112.00		
136	10.10	2.721	8.113	10.09	2.2	.000	. 88	74.59 2.01	3.37112.00	*.	
137	9.84	2.612	8.094	10.93	1.1	.000	.45	59.64 12.97	3.37112.00		
138	9.74	2.538	8.075	10.48	. 4	.000	. 15	44.26 14.78	3.37112.00		
139	9.67	2.509	8.056	9.97	. 1	.000	. 03		3.37112.00		
140	9.53	2.500	8.037	9.62	.0	.000	.00	39.93 9.79	3.37112.00		•
141	9.14	2.500	8.017	9.14	.0	.000	.00	36.60 6.16	3.37112.00		
142	7.94	2.500	7.998	7.94	. 0	.000	.00	33.13 3.50	3.37112.00	er i gegen er	
143	5.49	2.500	7.979	5.49	.0	.000	.00	29.66 2.15	3.37112.00		
144	1.99	2,500	7.960	1.99	.0	.000	.00	26.18 .91	3.37112.00		1.0

CASE PLAN

				*******				****		
*****	********	******		********	*******	and the second second		*****	********	******
	RUNOFF		OUTSIDE	DRAINAGE	DOUD! OB		DATION	DDA	INION DISCULDED IN DIGIT	TTTTC (1000-3)
	DISCHARGE	W.L.	W.L.	DISCHARGE			AREA	1	INAGE DISCHARGE of FACIL	111F2 (IAAAW.)
(hr) *****	(1000m ³)	(EL m)	(EL.m)	(1000m³) ((ha) *******		******	********
1	.00	3.060	3.338	.00	699.0	.000	237.96	.00		
2	.00	3.060	3.275	.00	699.0	.000	237.96	.00		
3	.00	3.060	3.214	.00	699.0	.000	237.96	.00		
4 .	.01	3.060	3.166	.00	699.0	.000	237.96	.00		
5	.07	3.060	3.123	.00	699.1	.000	237.98	.00		
6	. 17	3.060	3.081	.00	699.3	.000	238.02	.00	•	
7	. 27	3.059	3.044	6.17	693.4	.000	236.56 230.55	6.17		
8	. 42	3.053	3.017	24.66 39.53	669.1 630.2	.000	220.00	24.66 39.53		
9 10	.64 1.12	3.045	2.896 2.873	60.47	570.9	.000	206.18	60.47		
11	2.19	3.018	2.859	61.64	511.4	.000	191.44	61.64		
12	3.66	3.005	2.831	63.61	451.5	000	176.57	63.61		
13	4.86	2.957	2.902	63.91	392.4	.000	156.42	63.91		
14	5.60	2.903	2.922	52.53	345.5	.000	137.71	52.53		
15	6.12	2.859	2.901	43.40	308.2	.000	122.85	43.40		
16	6.55	2.818	2.879	41.90	272.9	.000	108.76	41.90		
17	6.93	2.779	2.861	40.34 38.74	239.4 208.0	.000	95.44 82.91	40.34 38.74		
18 19	7.29 7.60	2.742	2 845 2 817	36.50	179.1	.000	71.39	36.50		
20	7.79	2.678	2 793	33.78	153.1	.000	61.03	33.78		
21	7.82	2.651	2.771	31.13	129.8	.000	51.74	31.13		
- 22	7.72	2.627	2.751	28.55	109.0	.000	43.44	28.55		
23	7.65	2.606	2.733	26.03	90.6	.000	36.11	26.03		
24	7.75	2.587	2.717	23.59	74.7	000	29.80	23.59		
25	8.06	2.572	2.705	21.28	61.5	.000	24.53	21.28		
26	8.56	2.559	2.695	19.17	50.9	000	20.29	19.17		
27	9.20	2.550	2.689	17.31 15.75	42.8 37.0	.000	.17.06 14.75	17.31 16.75		
28 29	.9.96 10.81	2.543	2.685 2.685	14.54	33.3	.000	13.26	14.54		
30	11.81	2.537	2.689	13.70	31.4	.000	12.51	13.70		
31	13.13	2.536	2 698	13.24	31.3	.000	12.47	13.24		
32	15.02	2.539	2.712	13.18	33.1	.000	13.20	.13.18		
33	18.59	2.544	2.739	13.54	38.2	.000	15.21	13.54		
34	26.76	2.568	2.641	6.52	58.4	, 000	23.28	6.52		
35	39.78	2.614	2.674	.00	98.2	.000	39 13	.00		
36 37	51.40 57.34	2.674	2.786 2.906	.00	149.6 206.9	.000	59.62 82.48	.00		
38	59.34	2.810	2.955	.00	266.3	.000	106.13	.00		
39	59.41	2.880	2.989	.00	325.7	.000	129.82	.00		
40	58.22	2.947	3.001	.00	383.9	.000	153.02	.00	. *	
41	56.18	3.002	3.003	.00	440.1	.000	173.75	.00		
42	53.95	3.013	3.010	6.20	487.8	.000	185.59	6.20		
43	52.00	3.023	3.019	9.10	530.7	.000	196.23	9.10		
44	50.28 48.61	3.032	3.028 3.037	9.32 8.96	571.7 611.4	.000	206.39 216.22	9.32 8.96		
45 46	47.04	3.049	3.046	8.31	650.1	.000	225.83	8.31		
47	45.68	3.058	3.055	7.57	688.2	.000	235.28	7.57		
48	44.37	3.066	3.064	6.95	725.6	.006	244.56	6 95		
49	43.06	3.074	3.072	6.54	762.1	.014	253.61	6.54		
50	41.85	3.082	3.080	6.36	797.6	022	262.42	6.36		
51	40.81	3.090	3.088	6.33	832.1	030	270.97	6.33		
52	39.96	3.097	3.095	6.37	865.7	.037	279.30	6.37		
53	39.27	3.104	3.103	6.46	898.5	.044	287.44	6.46 6.56		
54	38.72	3.111	3.110	6.56	930.7	.051	295.41 303.26	6.66		
55 56	38.28 37.94	3.119 3.125	3.117 3.124	6.66 6.75	962.3 993.5	.059 .065	310.99	6.75		
57	37.75	3.132	3.130	6.79	1024.4	.072	318.67	6.79		
58	37.86	3.139	3,137	6.70	1055.6	.079	326.40	6.70	•	
59	38.32	3.146	3.145	6.46	1087.5	.086	334.30	6.46		
60	38.76	3.154	3.152	6.14	1120.1	.094	342.39	6.14	•	
61	38.94	3.161	3.160	5.84	1153.2	.101	350.60	5.84		
62	38.93	3.168	3.167	5.67 5.65	1186.4	.108	358 84 367 08	5.67		
63	38.85	3.176	3.174	5.65 5.77	1219.6 1252.6	.116 .123	367.08 375.26	5.65 5.77		
64 65	38.75 38.65	3.183 3.190	3.189	5.77	1285.3	,130	383.36	5.96	•	•
66	38.55	3.197	3.196	6.22	1317.6	.137	391.38	6.22	÷	
67	38.45	3.205	3.203	6.52	1349.6	.145	399.30	6.52		
68	38.26	3.212	3.209	6.86	1381.0	.152	407.09	6.86		
69	37.95	3.218	3.216	7.21	1411.7	.158	414.71	7.21		
70	37.55	3.225	3.222	7.58	1441.7	.165	422.14	7.58		
71	37.11	3.232	3.229 3.235	7.97 8.40	1470.8 1499.1	.172 .178	429.37 436.37	7.97 8.40		
72	36.64	3.238	5.230	0.40	1400,1	.110	400.01	0.40	·	

*** INUNDATION ANALYSIS BLOCK No. 2 *** CASE P

******	********	********		******	*******	******	********	****	*****	***********	*
	RUNOFF			DRAINAGE			DATION	· .		D D700111000 0 D10111000 11000	3 .
(hr)	DISCHARGE (1000m³)	W.L. (EL.m)	W.L.	DISCHARGE	PONDAGE	DEPTH	AREA		ta i na G	E DISCHARGE of FACILITIES (1000m	°).
*****	*******	*******	(EL.m)	(1000m")	(1000*cum)	(111)	(ha) ********	l		*****	•
73	36.15	3.244	3.240	8.86	1526.3		443.14	8.86		*****************	**
74	35.68	3.250	3.246		1552.7	184	443.14				
75	35.22	3.255	3.251	9.88	1578.0	195	455.95	9.35			
76	34.79	3.261	3.256		1602.3	. 201		9.88 10.45			
77	34.38	3.266	3.260	11.06	1625.7	206		10.40			
78	33.98	3.271	3.265	11.68			467.77				
79	33.51	3.276	3.269	12.32		~211		11.68			
80	32.91	3.280	3.273	12.94	1669.1	.216	478.56	12.32			
81	32.17	3.284	3.276		1689.1	.220	483.51	12.94			
82	31.36	3.288	3.279	13.56	1707.7	.224	488.12	13.56	i		
83	30.51	3.291		14.21	1724.9	. 228	492.38	14.21			
84	29.66		3.282	14.88		.231	496.25	14.88			
85	28.82	3.295	3.284		1754.6		499.74	15.60			
		3.297	3.286	16.33	1767.1		502.84	16.33			
86 87	28.00	3.300	3.287		1778.0	.240	505.54	17.09			
	27.19	3.302	3.288	17.85	1787.3	. 242	507.86	17.85			
88	26.41	3.304	3.288		1795.1	.244	509.79	18.62			
89	25.67	3 305	3.289	19.38		. 245	511.35	19.38			
90	24.97	3.306			1806 2		512.55	20.14	7		
91	24.29	3.307	3.288		1809.6	. 247	513.39	20.91			
92	23.63	3.307	3.287		1811.6	.247	513.87	21.68			
93	22.98	3.307	3.285	22.46	1812.1	. 247	514.00	22.46		and the state of t	
94	22.34	3.307	3.283	23.30	1811.1	.247	513.77	23.30			
95	21.69	3.307	3.281	24.23	1808 6	.247	513.14	24.23			
96	21.04	3.306	3.278	25.18	1804.5	. 246	512.11	25.18			
97	20.39	3.304	3.275	26.11			510.70	26.11		and the second second second second	
98	19,77	3.303	3.271	27.01	1791.5	.243	508.90	27.01			
99	19.18	3.301	3.267	27.89	1782.8	.241	506.74	27.89			
100	18.64	3.299	3.263		1772.7	239	504.23	28.74			ž
101	18.12	3.296	3.258	29.57		.236	501.39	29.57			
102	17.60	3.293	3.253	30.38		233	498.22				
103	17.11	3.290	3.248	31.21	1734.3	.230	494.72	30.38			
104	16.62	3.287	3.242	32.14		.227		31.21			
105	16.14	3.283	3.236	33.01	1702.0		490.88	32.14			
106	15.68	3.279	3.230	33.81	1683.8	.223	486.69	33.01			
107	15.22	3.275	3.223	34.57		.215	482.20	33.81			
108	14.78	3.270	3.217	35.29	1644.0		477.40	34.57			
109	14.34	3.265	3.210	36.00	1622.3	.205	472.31	35.29			
110	13.92	3.260	3.202				466.94	36.00			
111	13.50	3.255	3.195	37.34	1599.6	.200	461.30	36.68			
112	13.10	3.249	3.188	37.97	1575.7	.195	455.39	37.34			
113	12.62	3 244	3.180		1550.8	. 189	449.22	37.97			
114	11.61	3.237	3.172	38.59 39.17	1524.9	184	442.78	38.59	· 14		
115	9.65	3.231			1497.3	.177	435.94	39.17			
116	7.14	3.223	3.164 3.155	39.68	1467.3	.171	428.50	39.68			
117	5.04	3.215		40.16	1434.3	.163	420.31	40.16			
118	3.86	3.213		40.71	1398.6	.155	411.47	40.71			
119	3.38	3.198	3.133	41.45	1361.0	.147	402.14	41.45			
120	3.19	3.190	3.121	42.38	1322.0	.138	392.47	42.38	• "		
121	3.06	3.180	3.109	43.38	1281.8	.130	382.50	43.38			
122	2.94	3.171	3.096	44.33	1240.6	.120	372.27	44.33			
123	2.83	3.161	3.084	45.18	1198.3	.111	361.79	45.18			
124	2.75		3.071	45.94	1155.2	.101	351.10	45,94		and the stage of the first section is	
		3.152	3.059	46.63	1111.3	.092	340.22	46.63	5		
125 126	2.68	3.142	3.046	47.26	1066.8	.082	329.17	47.26			
	2.62	3.132	3.034	47.88	1021.5	.072	317.94	47.88			
127	2.56	3.121	3.021	48.48	975.6	.061	306.55	48.48		and the second of the second o	
128	2.50	3.111	3.009	49.05	929.0	.051	295.01	49.05			
129	2.44	3.100	2.951	52.37	879.1	.040	282.63	52.37			
130	2.38	3.087	2.929	60.38	821.1	.027	268.24	60.38			•
131	2.32	3.073	2.855	65.88	757.5	.013	252.48	65.88			
132	2.26	3.057	2.765	74.72	685.1	.000	234.50	74.72			
133	2.19	3.041	2.746	75.03	612.2	.000	216.44	75.03			
134	2.13	3.025	2.729	74.85	539.5	.000	198.41	74.85			
135	2.00	3.009	2.720	74.25	467.3	,000	180.49	74.25			
136	1.69	2.960	2.721	74.59	394.4	.000	157.19	74.59			
137	1.17	2.891	2.612	59.64	335.9	.000	133.88	59.64			
138	. 60	2.841	2.538	44.26	292.2	.000	116.48	44 26	:		
139	. 26	2.792	2.509	41.79	250.7	.000	99.93	41.79			*
140	. 11	2.746	2.500	39,93	210.9	.000	84.06	39.93			
141	. 04	2.703	2.500	36.60	174.3	.000	69.48	36.60	- 1		-
142	. 02	2.665	2.500	33.13	141,2	.000	56.29	33.13			4.1
143	. 02	2.630	2.500	29.66	111.6	.000	44.47	29.66	1.		
144	.01	2.600	2,500	26.18	85.4	.000	34.04	26.18			
									1.5		

				******				*******	****	*******
Í	RUNOFF	INSIDE		DRAINAGE		INUNDA			DIOCULDOD - C	D. O. I.
TIME I	DISCHARGE	. W.L.	W.L.	DISCHARGE	PONDAGE	DEPTH	AREA		DISCHARGE OF .	FACILITIES (1000m ³)
	$(1000m^3)$	(EL.m)	(EC.m)	$(1000m^3)$	1000*cum)	(m)	(ha)	2 3		
	*******		******	*******	*******	******	*******	********	**********	*******
1	00	3.198	3.338	-48.40	288.3	. 048	87.11	.00 48.40		
		3.224	3.275	-26.37	314.7	.074	93.88	.00 26.37		•
2	01				317.3	.077	94.56	3.00 5.62		
3	04	3.227	3.214	-2.62						* · · · · · · · · · · · · · · · · · · ·
4	.11	3.208	3.166	19.12	298.3	.058	89.68			
5 -	.19	3.184	3.123	24.81	273.7	.034	83.37	32.91 8.10		
6	30	3.157	3.081	27.57	246.4	.007	76.38	36.92 9.35		
7		3 128	3.044	29.11	217.8	.000	69.02	39.23 10.11		
	.43			28.78	189.6	.000	61.80	39.15 10.37		
8	.63	3.100	3.017					42.06 10.40		
9	1.05	3.070	2.896	31.66	159.0	.000	53.95			
10	2.06	3.034	2.873	37.53	123.5	.000	44.86	50.02 12.49		•
11	3.48	3.003	2.859	34.61	92.4	.000	36.87	47.24 12.63		
	4.66	2.874	2.831	30.57	66.5	.000	26.89	38.11 24.35		
12		2 800	2.902	18.54	53.4	.000	21.58	20.48 43.58		•
13	5.40				42.5	.000	17.17	18.98 51.39		•
14	5.90	2.738	2.922	16.81				18.11 57.71		
15	6.25	2.685	2.901	15.76	32.9		13.33			
16	6.48	2.645	2.879	13.63			10.44	16.13 62.25		
17	6.64	2.614	2.861	12.08	20.4	.000	8.24	14.72 65.47		
18	6.71	2.591	2.845	10:96	16 1	.000	6.52	13.73 67.84		
	6.68	2.573	2.817	9.71	13.1	.000	5.29	12.60 69.60		
19					11.1	.000	4.50	11.46 70.76		
20	6.51	2.563	2.793	8.46						
21	6.27	2.555	2.771	7.55	9.9	.000	3.99	10.66 71.50		
22	6.04	2.551	2.751	6.87	9.0	.000	3.65	10.09 71.98		*
23	5.93	2.548	2.733	6.34	8.6	:000	3.49	9.72 72.27		
24	5.97	2.549	2.717	5.92	8 7	.000	3.51	9.58 72.35		•
			2.705	5.66	9.2	.000	3.72	9.72 72.23		
25	6.19	2.552				000	4.12	10.11 71.89		
26	6.56	2.557	2.695	5.59	10.2					
27	7.05	2.565	2.689	5.67	11.6	.000	4.67	10.70 71.35		
28	7.61	2.575	2.685	5.85	13.3	.000	5.39	11.45 70.63		
.29	8.28	2.587	2.685	6.07	15.5	.000	6.28	12.34 69.71		
30	9.19	2.603	2.689	6.34	. 18.4	.000	7.43	13.39 68.53		
		2.624	2.698	6.66	22.2	.000	8.96	14.65 66.95		
31	10.44						11.14	16.23 64.72		
32	12.45	2.655	2.712	7.06	27.5	.000				
33	17.55	2.712	2.739	7.38	37.7	.000	15.26	18.51 60.92		
34	27.64	2.791	2.641	13.62	51.7	.000	20.93	29.26 53.51		
35	38.39	2.890	2.674	20.67	69.5	.000	28.09	43.18 43.71	*	
36	44.40	2.985	2.786	27.54	86.3	.000	34.91	51.83 26.40	*	
		3.015	2.906	27.56	104.5	.000	39.97	45.46 17.90		
37	45.71						46.73	39.35 21.34		
38	44.38	3.042	2.955	18.01	130.8	.000		37.41 23.20		
39	41.64	3.069	2.989	14.21	158.3	.000	53.77			
40	38.16	3.093	3.001	13.39	. 183.1	.000	60.12	38.20 24.81		
41	34.52	3.113	3.003	15.33	202.2	.000	65.04	42.21 26.88		
42	31.28	3.128	3.010	15.72	217.8	000	69.03	44.93 29.21		
43	28.62	3.142	3.019	15.02	231.4	.000	72.52	46.38 31.37		
		3.154	3.028	14.09	243.6	.004	75.66	47.37 33.29		
44	26.32						78.52	48.12 35.02		
45	24.27	3.165	3.037	13.10	254.8	.015				
46	22.57	3.175	3.046	12.12	265.3	.025	81.20	48.70 36.59		
47	21.13	3.185	3.055	11.22	275.2	.035	83.75	49.20 37.98		
48	19.79	3.194	3.064	10.45	284.5	.044	86.14	49.63 39.18		
		3.203	3.072	9.83	293.2	.053	88.38	50.00 40.17		
49	18.55				301.3	.061	90.46	50.31 40.98		
50	17.45	3.211	3.080	9.34			92.40	50.56 41.63		
51	16.52	3.219	3.088	8.93	308.9	.069				
52	15.73	3.226	3,095	8.59	316.1	.076	94.23	50.76 42.17		
63	15.06	3.232	3.103	8.29	322.8	.082	95.97	50.91 42.62		•
54	14.49	3.239	3.110	8.02	329.3	.089	97.63	51.02 43.00		
55	14.00	3.245	3.117	7.76	335.5	.095	99.23	51.10 43.34		
		3.251	3.124	7.52	341.6	.101	100.79	51.16 43.65		
56	13.62				347.8	107	102.36	51.22 43.93	•	
5 7	13.42	3.257	3.130	7.29				51.28 44.22		
58	13.42	3.264	3.137	7.06	354.1	. 114	103.99			
59	. 13.47	3.270	3.145	6.80	360.8	.120	105.71	51.37 44.58		
60	13.41	3.277	3.152	6.50	- 367.7	.127	107.48	51.48 44.98		
61	13.23	3.284	3.160	6.25	374.7	.134	109.27	51.61 45.35		4
62	12.99	3.291	3.167	6.12	381.6	141	111.03	51,72 45,60		
				6.13	.388.2	.147	112.72	51.81 45.68		
63	12.74	3.297	3.174				114.32	51.84 45.60		
. 64	12.49	3.304	3.182	6.24	394.4	154				
65	12.25	3.309	3.189	6.42	400.2	159	115.82	51.81 45.39		
66	12.04	3.315	3.196	6.63	405.6	. 165	117.21	51.71 45.08		
67	11.83	3.320	3.203	6.83	410.6	. 170	118.49	51.53 44.70		* *
68	11.63	3.324	3.209	7.02	415.3	.174	119.67	51.28 44,26		
69	11.43	3.329	3.216	7.18	419.5	.179	120.76	50.97 43.79		
		3.332	3.222	7.30	423.4	.182	121.77	50.61 43.31		
70	11.23				427.1	. 186	122.70	50.21 42.82		
71	11.04	3.336	3.229	7.40			123.56	49.78 42.31		
72	10.83	3.339	3.235	7.47	430.4	.189	160.00	10110 42101		

					and the second of the second	
***	INUNDATION	ANALYSIS	BLOCK	No. 3 ***	CASE	PLAN

*****	RUNOFF'	*******	********* ^!!T@!D@	********** DRAINAGE				************
TIME	DISCHARGE	W.L.	W.L:	DISCHARGE			ATION AREA	DRAINAGE DISCHARGE of FACILITIES (1000m3)
	(1000m ³)		(EL.m)	(1000m ³)			(ha)	2 3
*****	*******	******	*******	********	*******	******	*******	***************
73 74	10.63 10.43	3.342 3.345	3.240	7.55 7.72	433.5	.192	124.35	49.32 41.77
75	10.45	3.347	3.246	8.06	436.2 438.4		125.05 125.61	48.84 41.11 48.28 40.22
76	10.08	3.349	3.256	8.28	440.2		126 07	47.66 39.38
77	9.92	3.351	3.260	8.42	441.7		126.45	46.99 38.57
78	9.77	3.352	3.265	8.52	442.9	. 202	126.77	46.31 37.78
79	9.63	3 353	3.269	8.62	443.9			45.61 36.99
80	9.49	3.354	3.273	8.75	444.7	.204	127.22	44.89 36.13
81 82	9.33 9.14	3.354 3.354	3.276 3.279	8.93 9.12	445.1	204		44.15 35.22
83	8.94	3.354	3.282	9.30	445.1 444.7	.204	127.33 127.24	43.38 34.26 42.59 33.29
84	8.74	3.353	3.284	9.47	444.0	.203		41.79 32.32
85	8.54	3.352	3.286		442.9	.202	126.77	40.98.31.36
86	8.35	3.350	3.287	9.74	441.5	.200	126.41	40.17 30.43
87	8.17	3.349	3.288	9.83	439.9	.199	125.99	39.37 29.54
88	7.99 7.83	3.347	3.288	9.87	438.0	.197	125.51	38.59 28.71
89°	7.67	3.345 3.343	3.289 3.288	9.88 9.89	436.0 433.7	195	124.98	37.84 27.96
91	7.52	3.340	3.288	9.91	431.3	.193	124.41 123.80	37.15 27.26 36.50 26.60
92	7.37	3.338	3.287	9.95	428.8	.188	123.14	35.91 25.96
93	7.23	3.335	3.285	10.02	426.0	185	122.42	35.37 25.35
94	7.09	3.332	3.283	10.14	422.9	182	121.64	34.90 24.75
95	6.95	3.329	3.281	10.35	419.5	179	120.77	34.51 24.16
96	6.81	3.325	3.278	10.57		.175	119.81	34.15 23.59
97	6.66	3.321	3.275	10.78	411.7	.171	118.75	33.80 23.02
98 99	6.52	3.316	3.271	10.99 11.19	407.2		117.60	33.44 22.45
100	6.39	3.312 3.306	3.267	11.19	402.4 397.3	.162 .156	116.37 115.06	33.07 21.88 32.70 21.32
101	6.14	3.301	3.258	11.56	391.9	151	113.67	32.32 20.76
102	6.03	3.295	3.253	11.74	386.1	145	112.21	31.93 20.20
103	5.92	3.289	3.248	11.94	380.1	.139	110.66	31.58 19.64
104	5.81	3.283	3.242	12.22	373.7	.133	109.02	31.32 19.11
105	5.70	3.276	3.236	12.42	367.0	126	107.30	31.01 18.58
106 107	5.60 5.50	3.269 3.262	3.230	12.57 12.69	360.0 352.9	.119 .112	105.51 103.67	30.64 18.07
108	5.41	3.256	3.217	12.76	345.5	.105	101.78	30.26 17.58 29.89 17.13
109	5.31	3.248	3.210	12.82	338.0	098	99.86	29.55 16.73
110	5.23	3.240	3.202	12.87	330.3	.090	97.90	29.23 16.36
111	5.14	3.232	3.195	12.93	322.6	.082		28.93 16.00
112	5.06 4.98	3.224	3,188 3,180	12.99 13.09	314.6	.074	93.86	28.66 15.66
113 114	4.91	3.216 3.208	3.172	13.42	306.5 298.0	.066 .058	91.79 89.60	28.40 15.30 28.11 14.68
115	4.83	3.198	3.164	14.39	288.4	.048	87.15	27.65 13.26
116	4.66	3.187	3.155	15.82	277.3	.037	84.29	26.85 11.03
117	4.21	3.175	3.145	16.50	265.0	025	81.14	25.80 9.29
118	3.55	3.162	3.133	16.13	252.4	012	77.91	24.94 8.80
119	3.04	3.150	3.121	15.75	239.7 227.0	.000	74.65	24.42 8.67
120 121	2.83 2.79	3.137 3.125	3.109 3.096	15.56 15.46	214.3	.000	71.38	24.13 8.58
122	2.78	3.112	3.084	15.37	201.7	.000	68 14 64 91	23 92 8.46 23.71 8.34
123	2.77	3.100	3.071	15.28	189.2	.000	61.70	23.49 8.21
124	2.76	3.087	3.059	15.19	176.8	.000	58.51	23.27 8.08
125	2.75	3.075	3.046	15.14	164.4	000	55.33	23.09 7.96
126	2.74	3.063	3.034	15.12	152.0	.000	52.16	22.97 7.85
127 128	2.73 2.72	3.050 3.038	3.021 3.009	15.12 15.12	139.6 127.2	000	48.98	22.88 7.76
129	2.71	3.022	2.951	18.30	111.6	.000	45.80 41.80	22.80 7.68 26.16 7.86
130	2.70	3.001	2.929	24.01	90.3	.000	36.33.	34 03 10.02
131	2.69	2.895	2.855	22.63	70.4	.000	28.46	29.30 22.36
132	2.68	2.823	2.765	15.58	57.5	.000	23.24	18.46 41.12
133	2.67	2.786	2.746	9.31	50.8	.000	20.56	12.12 47.89
134	2.66	2.759	2.729	7.44	46.0	.000	18.63	10.11 51.58
135 136	2.65 2.63	2.737 2.748	2.720	6.48 .76	42.2 44.1	.000	17.07	8.67.54.20
137	2.63	2.693	2.612	12.41	34.3	.000 .000	17.83 13.87	2.01 54.90 12.97 56.87
138	2.60	2.626	2.538	14.42	22.5	.000	9.10	14.78 62.90
139	2.65	2.572	2.509	12.23	12.8	.000	5.18	12.44 68.32
140	2.48	2.531	2.500	9.73	5.6	.000	2.25	9.79 72.28
141	2.44	2.510	2.500	6.16	1.8	000	75	6.16 74.72
142 143	2.29 1.74	2.504 2.501	2.500	3.50 2,15	. 6 . 2	000		3.50 75.73 2.15 76.02
144	.66	2.500	2.500	,89	.0	.000	09 00	.91 76.15
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* * *	INUNDATION	ANALYSIS	BLOCK	No.	4	***	CASE	PLAN				

****	RUNOFF	INSIDE	OUTSIDE	DRAINAGE	******	INUND	Λ'ΓΙΟΝ			
TIME	DISCHARGE	W.L	W.L.	DISCHARGE	PONDAGE		AREA	DRAINAGE	DISCHARGE of FACILI	TIES (1000m ³)
		(EL.m)	(EL.m)	(1000m ³) ((ha)	3 4		
****	******	******	*******	*******	******	******		*********	*************	*********
1	.00	3.316	3.198	48.40	114.5	.000	45.55	48.40 22.79		
. 2	.00	3.243	3.224	26.37	88.1	.000	35.06	26.37 22.79		4.3
3	.01		3.227	5.61	82.5	.000	32.83	5.62 22.79	•	
4	. 02	3.213	3.208	5.60	77.0	.000	30.61	5,62 22.79		
5	.03	3.190	3.184	8.06 9.30	68.9 59.7	.000	27.41 23.73	8.10 22.79 9.35 22.79		
6 7	.04	3.165 3.137	3.157 3.128	10.03	49.7	.000	19.76	10.11 22.79		
8	.08	3.109	3.100	10.05	39.5	.000	15.72	10.37 22.79		
. 8	.13	3.081	3.070	10.21	29.4	.000	11.71	10.40 22.79		* *
10	. 25	3.048	3.034	12.15	17.5	.000		12.49 22.79		
11	. 44	3.017	3.003	11.99	6.0	.000	2.38	12.63 22.79		
12	.61	3.000	2.874	6 55	. 0	.000	. 02	24.35 22.79		
13	.71	3.000	2.800	.71	.0	.000	. 02	43.58 22.79		
14	.79	3.000	2 738	. 78	. 0	.000	.02			
15	.84	3.000	2.685	.84	. 1	.000	.02	57.71 22.79		
16	.90	3.000	2.645	.89	1	.000	.02	62.25 22.79 65.47 22.79		
17	.94	3.000	2.614 2.591	.94 .99	. 1 . 1	.000	.02 .02	67.84 22.79		
18 19	.99 1.03	3.000	2.573	1.03	. 1	.000	. 03	69.60 22.79		
20	1.07	3.000	2.563	1.07	. 1	.000	.03	70.76 22.79		
21	1.11	3.000	2.555	1.11	. 1	.000	.03	71.50 22.79		
22	1.15	3.000	2.551	1 14	. 1	.000	.03	71.98 22.79		
23	1.21	3.000	2.548	1.20	.1	.000	.03	72.27 22.79		
24	1.32	3.000	2.549	1.31	. 1	.000	.03	72.35 22.79		
25	1.46	3.000	2.552	1.45	. 1	.000	. 04	72.23 22.79		
26	1.63	3.000	2.557	1.62	1	.000	.04	71.89 22.79	•	
27	1.81	3.000	2.565	1.80	.1	.000	. 05	71.35 22.79	4	
28	2.01	3.000	2.575	2.00	- 1	000	.05	70.63 22.79		
29	2.25	3.000	2.587	2.23 2.51	.1	.000	.06	69.71 22.79 68.53 22.79		
30	2.53	3.000	2.624	2.84	2	.000	.07	66.95 22.79		
31 32	2.87 3.30	3.001	2.655	3.27	.2	.000	. 08	64.72 22.79		
33	3.98	3.001	2.712	3.92	.3	.000	.11	60.92 22.79		-
34	5.77	3.001	2.791	5.64	. 4	.000		53.51 22.79		
35	8.42	3.002	2.890	8.27	. 6	.000	. 22	43.71 22.79		
36	10.31	3.013	2.985	6.29	4.6	.000	1.82	26.40 22.79		* * * * * * * * * * * * * * * * * * *
37	11.31	3.050	3.015	-2.36	18.2	.000	7.25	17.90 22.79		
38	12.05	3.085	3.042	40	30.7	.000	12.21	21.34 22.79		
39	12.68	3.119	3.069	. 45	42.9	.000	17.07	23.20 22.82		
40	13.24	3.151	3.093	1.60 2.95	54.6 65,4	.000	21.71 26.00	24.81 23.21 26.88 23.92		
41	13.74 14.21	3.181	3.113 3.128	4.46	75.1	.000	29.87	29.21 24.75		
42 43	14.65	3.232	3.142	5.78	84.0	.000	33.40	31.37 25.59		
44	15.06	3.255	3.154	6.89	92.1	,000	36.65	33.29 26.40		
45	15.45	3.276	3.165		99.8	.000	39.68	35.02 27.18		
46	15.80	3.295	3.175	8.66	106.9	.000	42.52	36.59 27.93		
47	16.04	3.314	3.185	9.40	113.5	.000	45.17	37.98 28.59		
48	16.13	3.330	3.194	10.11	119.6	.000	47.56	39.18 29.08		
49	16.17	3.345	3.203	10.81	124.9	.000	49.70	40.17 29.37		
50	16.20	3.358	3.211	11.44	129.7	.000	51.59	40.98 29.53		
51	16.24	3.370	3.219		133.9	.000	53.28 54.82	41.63 29.64 42.17 29.73		
52	16.29	3.381	3.226 3.232		137.8 141.3	.000	56.22	42.62 29.82		
53 54	16.33 16.39	3.400	3.232		144.6	.000	57.53	43.00 29.91		
55	16.45	3.408	3.245		147.7	.000	58.77	43.34 30.00		
56	16.53	3.416	3.251	13.54	150.7	.000	59.96	43.65 30.11		
57	16.65	3.425	3.257	13.68	153.7	.000	61.14	43.93 30.25		
58	16.91	3.433	3.264	13.75	156.9	.000	62.39	44.22 30.48		
59	17.22	3.443	3.270	13.74	160.3	.000	63.78	44.58 30.84		
60	17.44	3.453	3.277	13.81	164.0	.003	65.23	44.98 31.18		
61	17.55	3.462	3.284	14.15	167.4	.012	66.58	45.35 31.21		
62	17.63	3.470		14.70 15.33	170.3 172.7	.020	67.74 68.68	45.60 30.90 45.68 30.35		
63 64	17.69 17.75	3.477 3.482	3.297	15.93	174.5	.027	69.41	45.60 29.67		
65	17.75	3.486		16.45	175.8	.032	69.94	45.39 28.94		
66	17.84	3.488	3.315		176.8	.038	70.32	45.08 28 20		
67	17.89	3.490	3.320		177.4	.040	70.58	44.70 27.46		
68	17.92	3.491	3.324	17.52	177.8	.041	70.74	44.26 26.74		
69	17.96	3.492	3.329		178.1	.042	70.83	43.79 26.06		
70	18.00	3.492		17.90	178.2	.042	70.87	43.31 25.41	•	•
71	18.02	3.492			178.1	.042	70.86	42 82 24 77		
72	18.02	3,492	3.339	18.19	178.0	.042	70.79	42.31 24.12		

***	INUNDATION ANALYSIS	BLOCK No. 4 ***	CASE	PLAN

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	RUNOFF			DRAINAGE			DATION	DD L THE OD	DICOM DOD O DICOM DE COMO
	DISCHARGE	W.L.		DISCHARGE	PONDAGE .	DEPTH	AREA		DISCHARGE of FACILITIES (1000m3)
(nr) *****	(1000m ³)	(EL.M)	(EL.m)	(1000m ³) ((1000*cum)	(m)	(ha)	3 4	*************
73	18.02							41.77 23.41	
74	18.02	3.491	3.342	18.36	177.6	041	70.66		
	10.02	3.487	3.345		176.5			41.11.22.83	
75	18.02	3.484	3.347	19.42	175.1	034	69 64	40.22 22.79	
76		3.480	3.349	19.39		030	69.10	39.38 22.79	
77	18.02	3.476	3.351	19.39	172.3			38.57 22.79	
78	17.99	3.472	3.352	19.39	170.9			37.78 22.79	and the second of the second o
79		3.468	3.353		169.4	.018	67.38		
80	17.53	3.463	3.354	19.26	167.7	.013		36.13 22.79	
81	17.15	3.458	3.354	19.07	165.7	008			
82	16.75	3.452	3.354	18.81	163.7 161.5	.002	65.11	34.26 22.79	
83	16.35	3.446	3.354	18.51	161.5	000	64.26	33.29 22.79	
84	15.97	3.440	3.353	18.17	159.3	.000	63.38	32.32 22.79	
85	15.59	3.434	3.352	17.83	157.1	.000	62.49	31.36 22.79	
86	15.21	3.428	3.350	17.45		.000	61.60	30.43 22.79	
87	14.84	3.422	3.349	17.02	152.7	.000	60.73	29.54 22.79	
88	14.49	3.416	3.347		150.6	.000	59.90	28.71 22.79	
89	14.17	3.410	3.345	16.17	148.6	.000		27.96 22.79	
90	13.86	3 405	3.343	15.81	146.6	.000		27.26 22.79	
91	13.56	3.400	3.340	15.48	144.7	000	57 56		
								26.60 22.79	
92	13.26	3.394	3.338	15.19	142.8	.000		25.96 22.79	
93	12.96	3.389	3.335	14.92		.000		25.35 22.79	
94	12.64	3.383	3.332	14.65	138.8	.000	55.21	24.75 22.79	
95	12.31	3.378	3.329	14.41	136.7		54.38	24.16 22.79	
96	11.97	3.372	3.325	14.17	134.5	.000	53.50	23.59 22.79	and the first of the second of
97	11.64	3.365	3.321	13.93	132.2	.000	52.59	23.02.22.79	
98	11.31	3.359	3.316	13.71	129.8	.000	51 64	22.45 22.79	
99	10.98	3.352	3.312	13.48	127.3	.000	50.64	21.88 22.79	
100	10.66	3.345	3.306	13.25	124.7	.000	49.61	21.32 22.79	
101	10.35	3.337	3.301		122.0	000	48.55	20.76 22.79	
102	10.05	3.330	3.295	12.80	119.3	000	47.45	20.20 22.79	
103	9.75	3.322	3.289	12.59	116.4	.000	46.32	19.64 22.79	
104	9.46	3.314	3.283	12.39	113.5	.000	45.16	19.11 22.79	
105	9.18	3.305	3.276	10.00				18.58 22.79	
105	8.90	3.297	3.269		110.5	.000	43.96		
107	8.64	3.288	3.262	11.98	107.4	.000	42.73	18.07 22.79	
				11.74	104.3	.000	41.50	17.58 22.79	
108	8.38	3.280	3.255	11.48	101.2	000	40.27	17.13 22.79	
109	8.13	3.271	3.248	11.23	98.1	.000	39.03	16.73 22.79	and the second of the second o
110	7.89	3.262	3.240		95.0	.000	37.79	16.36 22.79	
111	7.66	3.254	3.232	10.81	91.9	.000		16.00 22.79	
112	7.44	3.245	3.224	10.62	88.7	.000	35.27	15.66 22.79	
113	7.10	3.236	3.216	10.41	86.4	000	33.96	15.30 22.79	アン・ディング しょうしゅ おんしょ ディー・ディー
114	6.01	3.225	3.208	9.94	81.4	.000	32.39	14.68 22.79	
115	3.60	3.211	3.198	8.67	76.4	000	30.38	13.26 22.79	and the second s
116	1.04	3.196	3.187	6.59	70.8	.000	28.17	11.03 22.79	and the second of the second o
117	.00	3.182	3.175	5.00	65.8	.000	26 18	9.29 22.79	
118	.00	3.169	3.162	4.64	61.2	.000	24 33	8.80 22.79	
119	.00	3.156	3.150	4.63		.000		8 67 22 79	
120	.00	3.143	3.137	4.63	51.9	000		8.58 22.79	
121	.00	3.131	3.125		47.3		18.81	8.46 22.79	
122	, 00	3.118	3.112			. 000	16.98	8.34 22.79	
123	.00	3.105	3.100	4.57	38.1	000	15.17	8.21 22.79	
124	.00	3.093	3.087	4.54	33.6	.000	13.36	8.08 22.79	the second control of
125	.00	3.080	3.075	4.52	29.1		11.66	7.96 22.79	
126	.00	3.068	3.063	4.50	24.6	000	9.77		
	.00	3.055	3.050		20.1			7.85 22.79	
127				4.50			7.98	7.76 22.79	
128	.00	3.043	3.038	4.50	15.6	.000		7.68 22.79	
129.	.00	3.030	3.022	4.76	10.8	.000	4.30	7.86 22.79	
130	.00	3.011	3.001	6.99	3.8	.000	1.52	10.02 22.79	
131	.00	3.000	2.895	3.72	. 1	.000	.04	22.36 22.79	
132	.00	3.000	2.823	.00	. 1	.000	04	41.12 22.79	
133	.00	3.000	2.786	.00	. 1	.000	04	47.89 22.79	and the second of the second o
134	.00	3.000	2.759	.01	.1	.000	03	51.58 22.79	
135	.00	3.000	2.737	.03	. 1	000	02	54.20 22.79	
136	.00	3.000	2.748	.04	0	.000	01	54.90 22.79	
137	.00	3.000	2.693	.01	. 0	.000	01	56.87 22.79	
138	.00	3.000	2.626	.01	. 0	000		62.90 22.79	
139	.00	3.000	2.572	.01	. 0	.000	.00	68.32 22.79	
140	, 00	3.000	2.531	.00	. 0	.000	.00	72.28 22.79	to a supplied that the supplied of the supplied of
141	.00	3.000	2.510	.00	. 0	.000	.00	74.72 22.79	
142	.00	3.000	2.504	.00	0	.000	.00	75 73 22.79	
143	.00	3.000	2.501	.00	. 0	.000	.00	76.02 22.79	
144	.00	3.000	2.500	.00	. 0	.000	.00	76.15 22.79	

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	RUNOFF	INSIDE	OUTSIDE	DRAINAGE			OATION		
TIME	DISCHARGE :	W.L.	W.L.	DISCHARGE	PONDAGE	DEPTH	AREA	DRAII	NAGE DISCHARGE of FACILITIES (1000m ³)
(hr)	(1000m³)	(EL.m)	(EL.m)	$(1000m^3)$ (1000*cum)	(m)	(ha)	4	
*****	********	******	*******	********			*******		***************
. 1	.00	4.000	3.316	.00	. 0	. 000	.00	22.79	
2	.00	4.000	3.243	.00	. 0	. 000	.00	22.79	
3	.01	4.000	3.228	.01	0	.000	.00	22.79	
4	.02	4.000	3.213	.02	. 0	.000	.00	22.79	•
5	.03	4.000	3.190	.03	.0	.000	.00	22.79	· · · · · · · · · · · · · · · · · · ·
6	.06	4.000	3.165	.06	0	.000	.00-	22.79	
7	.08	4.000	3.137	.08	.0	.000	.00	22.79	v i
8	.12	4.000	3.109	.12	. 0	.000	.00	22.79	
9	.19	4.000	3.081	. 19	.ŏ	.000	.00	22.79	
	. 34	4.000	3,048	.34	. 0	.000	.00	22.79	
10			3.017	.64	.0	. 000	.00	22.79	
11	.64	4.000		.99	.0	.000	.00	22.79	•
12	99	4.000	3.000		.0	.000	.00	22.79	
13	1.24	4.000	3.000	1.24				22.79	
14	1.39	4.000	3.000	1.39	٠٥.	.000	.00		
15	1.51	4.000	3,000	1.51	.0	.000	.00	22.79	
16	1.61	4.000	3.000	1.61	0	.000	.00	22.79	
1.7	1.70	4.000	3.000	1.70	. 0	.000	.00	22.79	
18	1.79	4.000	3.000	1.79	.0	.000	.00	22.79	
19	1.87	4.000	3.000	1.87	.0	.000	.00	22.79	
20	1.94	4.000	3.000	1.94	.0	. 000	.00	22.79	· ·
21	2.01	4.000	3.000	2.01	.0	.000	.00	22.79	
22	2.08	4.000	3.000	2.08	.0	.000	.00	22.79	
23	2.18	4.000	3.000	2.18	.0	.000	.00	22.79	
24	2.35	4.000	3.000	2.35	. Ó	.000	.00	22.79	· ·
25	2.60	4.000	3.000	2.60	. 0	.000	.00	22.79	
26	2.90	4,000	3.000	2.90	. 0	.000	.00	22.79	
27	3.24	4.000	3.000	3.24	. 0	.000	.00	22.79	
. 28	3.61	4.000	3.000	3.61	.0	.000	.00	22.79	· ·
29	4.04	4.000	3.000	4.04	. 0	.000	.00	22.79	
30	4.54	4.000	3.000	4.54	Ŏ	.000	.00	22.79	
31	5 14	4.000	3.001	5.14	ő	.000	,00	22.79	· · · · · · · · · · · · · · · · · · ·
32	5.91	4.000	3.001	5.91	.0	.000	,00	22.79	
33	7.21	4.000	3.001	7.21	.0	.000	.00	22.79	
34	10.00	4.000	3.001	10.00	. 0	.000	.00	22.79	•
	14.25	4.000	3.002	14.25	.0	.000	. 00	22.79	
35								22.79	
36	18.00	4.000	3.013	18.00	.0	.000	.00		
37	20.26	4.000	3.050	20.26	.0	.000	.00	22.79	•
38	21.74	4.000	3.085	21.74	0	.000	.00	22.79	
39	22.95	4,007	3.119	22.75	. 2	.007	.09	22.82	
40	24.00	4.035	3.151	23.21	1.0	. 035	.42	23.21	i e
41	24.95	4.072	3.181	23.92	2.0	.072	.86	23.92	
42	25.82	4.110	3.207	24.75	3.1	.110	1.32	24.75	
43	26.63	4.147	3.232	25.59	4.1	.147	1.77	25.59	
44	27.40	4.183	3.255	26.40	5 1	. 1.83	2.20	26.40	
45	28 13	4.217	3.276	27.18	6.1	.217	2.60	27.18	
46	28.80	4.248	3.295	27.93	6.9	. 248	2.98	27.93	
47	29.30	4.273	3.314	28.59	7,7	.273	3.28	28.59	
48	29.54	4.290	3.330	29.08	8.1	.290	3.48	29.08	
49	29.62	4.299	3.345	29.37		`,299	3.59	29.37	
50	29.69	4.304	3.358	29.53	8.5	. 304	3.65	29.53	
51	29.76	4.309	3.370	29.64	8.6	. 309	3.70	29.64	
52	29.84	4.312	3.381	29.73	8.7	.312	3.75	29.73	
53	29.92	4.316	3.390	29.82	8.8	.316	3.79	29.82	
54	30.01	4.320	3.400	29.91	9.0	.320	3.84	29.91	
55	30.12	4.324	3.408	30.00	9 1	.324	3.89	30.00	
56	30.25	4.329	3.416	30.11	9 2	.329	3.95	30.11	
57	30.45	4.336	3.425	30.25	9.4	. 336	4.04	30.25	
58	30.84	4.349	3.433	30.48	9.8	. 349	4.19	30.48	
.59	31.31	4.366	3.443	30.84	10.3	.366	4.40	30.84	
60	31.42	4.375	3.453	31 18	10.5	.375	4.50	31.18	
61	31.02	4.368	3.462	31.21	10.3	.368	4.42	31.21	
62	30.36	4.349	3.470	30.90	9.8	.349	4.19	30.90	
63	29.60	4.322	3.477	30.35	9.0	.322	3.87	30.35	
			3.482	29.67	8.2	. 292	3.50	29.67	
64 65	28.82	4.292	3.486	28.94	7.3	. 260	3.12	28.94	•
65	28.04	4.260		28.20	6.4	. 227	2.73	28.20	
66	27.28	4.227	3.488	27.46	5.5	.195	2.34	27.46	•
67	26.55	4.195	3.490	26.74	4.6	.164	1.97	26.74	
68	25 87	4 164	3.491		3.8	.134	1.61	26.06	
69	25.23	4.134	3.492	26.06	3.0	. 105	1.01	25.41	
70	24,60	4.105	3.492	25.41		.076	.92	24.77	
71	23.95	4.076	3.492	24.77	2.1	.045	.54	24.12	
72	23.24	4,045	3.492	24.12	1.3	,040	. 04	£4.16	

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***	INUNDATION	ANALYSIS	BLOCK NO	5 . 5	. * * *	

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CASE

DRAINAGE RUNOFF INSIDE OUTSIDE --INUNDATION--W.L. W.L. PONDAGE DEPTH TIME DISCHARGE DISCHARGE AREA DRAINAGE DISCHARGE of FACILITIES $(1000m^3)$ (EL.m) (EL.m) (1000m³) (1000*cum):(m) (ha) ******* 3.491 ***** 73 ****** 22.45 ****** 4.011 *************** 23.41 .3 **** 011 ***** 23.41 74 21.63 4.000 3.487 21.93 . 0 .000 .00 22.83 .000 75 76 20.80 4.000 3.484 20.80 . 0 .00 22.79 22.79 19.98 4.000 .000 19.98 . 0 .00 4.000 3.476 19.18 .000 22.79 19,.18 .00 3.472 3.468 3.463 78 79 18.39 4.000 18.39 17.62 . ი .000 .00 22.79 22.79 17.62 4.000 .000 . n .00 16.88 4.000 16.88 .00 22.79 .000 81 16.15 4.000 3.458 16.15 . 0 .000 .00 22.79 82 15.45 4,000 3.452 15.45 . n . 000 . 00 22,79 14.78 3.446 14.78 .00 22.79 4.000 . 0 .000 84 14.14 4.000 3.440 14.14 . 0 .000 .00 22.79 85 13.54 4.000 3.434 13.54 ÷ά .000 . 60 22.79 22.79 12.98 4.000 3.428 12.98 , o .000 .00 87 12.52 4.000 3.422 12.52 Ō. .000 .00 22.79 22.79 22.79 88 12.13 4.000 3.416 12.13 . 0 .000 .00 11.79 11.79 89 4.000 .000 .00 . 0 90 11.45 4.000 3.405 11.45 . 0 .000 .00 91 11.11 10.77 4.000 3.400 $\frac{11.11}{10.77}$. o .000 .00 22.79 3.394 22.79 22.79 92 4.000 . 0 .000 .00 93 10.43 4.000 3.389 10.43 . o .000 .00 94 10.10 9.76 4.000 3.383 10.10 Ö. .000 .00 22.79 4.000 95 3.378 9.76 .000 . o .00 22.79 96 9.42 4.000 3.372 9.42 . 0 .000 .00 22,79 97 9.08 4.000 3.365 9.08 .000 .00 . 0 22.79 8.74 8.41 98 4.000 3.359 8.74 22.79 . 0 .000 .00 99 4.000 3.352 8.41 . 0 .000 .00 22.79 100 8.07 4.000 3.345 8.07 . 0 .000 22.79 22.79 .00 7.73 7.73 101 4.000 3.337 . 0 .000 .00 102 4.000 3.330 .0 .000 .00 103 4.000 3.322 7.06 . 0 22.79 22.79 .000 .00 6.72 104 4.000 3.314 6.72 .0 .000 .00 105 4.000 3.305 6.39 . 0 .000 .00 106 6.08 4.000 3.297 6.08 22.79 22.79 . 0 .000 .00 107 5.84 4.000 3.288 5.84 . 0 .000 .00 108 5.66 4.000 3.280 5.66 . 0 ,000 .00 109 5.50 4.000 3.271 22.79 22.79 5.50 . n .000 .00 110 5.35 4.000 3.262 5.35 .0 .000 .00 111 5.19 4.000 3,254 5.19 . 0 .000 .00 22.79 5.04 4.000 3.245 5.04 . 0 .000 .00 22.79 113 4.89 4.000 3.236 4.89 ٠.0 .000 .00 22 79 4.74 114 4.000 3.225 4.74 .000 .00 22.79 4.59 4.000 4.59 3.211 . 0 .000 .00 22.79 116 117 4.44 4.29 4.000 3.196 4.44 . 0 .000 22.79 . 00 4.000 3.182 4.29 .000 .00 22.79 118 4.16 4.000 3.169 4.16 , o .000 .00 22.79 119 4.05 4.000 3.156 4.05 ٠Ò. .000 .00 22,79 120 3.94 4.000 3.143 3.94 . 0 22.79 .000 .00 3.84 4.000 3.131 3.84 . O .000 .00 22.79 122 3.74 4.000 3.74 3.118 . 0 .000 .00 22.79 22.79 123 3.64 4.000 3.105 3.64 .0 .000 .00 3.54 4.000 3.093 3.54 3.44 ٠0 .000 .00 22.79 125 3.44 4.000 3.080 . 0 22.79 22.79 .000 .00 126 4.000 3.068 3.35 . 0 .000 .00 3.26 4.000 3.055 .0 3.26 .000 .00 128 3.18 4.000 3.043 3.18 22.79 22.79 .000 .00 129 3.10 4.000 3.030 3.10 . 0 .000 .00 3.03 4.000 .0 3.011 3.03 .000 .00 131 2.95 4.000 3.000 2.95 .000 22.79 22.79 . 00 132 2.88 4.000 3.000 2.88 .000 .00 2.81 4.000 .0 2.81 .000 .00 134 2.66 4.000 3.000 2.66 .000 .00 22.79 135 2.16 4.000 3.000 2.16 . 0 .000 22.79 22.79 .00 136 1.21 4.000 3.000 . 0 .000 .00 137 .55 4.000 3.000 .55 . 0 .000 . 00 22.79 138 .36 4.000 3.000 . 36 . 0 22.79 22.79 .000 . 00 139 .20 4.000 3.000 . 20 .000 .00 140 .06 4.000 3.000 .06 .0 .000 .00 22.79 141 .00 4.000 3.000 .00 22.79 22.79 .000. 00 142 .00 4.000 3.000 .00 .000 .00 143 .00 4.000 3.000 .00 . 0 .000 .00 .00 4.000 3.000 .00 .00 . 0 .000

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	RUNOFF	INSIDE	OUTSIDE	DRAINAGE			KOITAC	
TIME	DISCHARGE	w.L.	W.L.	DISCHARGE			AREA	DRAINAGE DISCHARGE of FACILITIES (1000m ³)
		(EL.m)		$(1000m^3)$			(ha)	5
				********				******************
1	.00	4.574	3.338	8.25	44.0	.000	15.29	8.25
2	.00	4.550	3.275	8.01	35.9	.000	12.85	8.01
3	.01	4.525	3.214		28.2	.000	10.49	7.78
4	01	4.502	3.166	7.56	20.6	.000	8.19	7.56
5	.02	4,346	3.123	6.81	13.8	.000	5.53	6.81
6	.03	4.209	3.081	5.50	8 4	.000	3.35	5.50
7	.05	4.098	3.044	4.51	3.9	.000	1.56	4.51
8	.07	4.006	3.017	3.74	. 2	.000	.09	3.74
9	. 12	4.000	2.896	. 35	.0	.000	.00	3.37
10	. 24	4.000	2.873	. 24	.0	.000	.00	3.37
11	.38	4.000	2.859	38	0	.000	.00	3.37
12	.48	4.000	2.831	.48	0	.000	.00	3.37
13	. 54	4.000	2.902	. 54	.0	.000	.00	3.37
14	58	4.000	2.922	. 58	.0	.000	.00	3.37
15	. 6 2	4.000	2.901	. 62	0	.000	.00	3.37
16	.66	4.000	2.879	. 66	.0	.000	.00	3.37
17	. 69	4.000	2.861	. 69	.0	.000	.00	3.37
18	.72	4.000	2.845	.72	.0	.000	.00	3.37
19	.75	4.000	2.817	. 75	.0	.000	.00	3.37
20	.78	4.000	2.793	.78	. 0	. 000	.00	3.37
21	.81	4.000	2.771	.81	. 0	.000	.00	3.37
22	. 84	4.000	2.751	.84	. 0	.000	.00	3.37
23	90	4.000	2.733	90	.0	.000	.00	3.37
24	1.00	4.000	2.717	1.00	. 0	.000	00	3.37
25	1 11	4.000	2.705	1.11	.0	.000	.00	3.37
26	1.24	4.000	2.695	1.24	. 0	.000	.00	3.37
27	1.38	4.000	2.689	1.38	0	.000	.00	3.37
28	1.54	4.000	2.685	1.54	. 0	.000	.00	3.37
29	1.73	4.000	2.685	1.73	. 0	.000	.00	3.37
30	1.95	4.000	2.689	1.95	.0	.000	.00	3.37
31	2.22	4.000	2.698	2.22	.0	.000	.00	3.37
32	2.59	4.000	2.712	2.59	. 0	.000	.00	3.37
33	3.48	4.006	2.739	3.26	. 2	.000	.09	3.38
34	5.15	4.046	2.641	3.52	1.8	.000	.74	3.52
35	6.77	4.117	2.674	3.94	4 7	.000	1.87	3.94
36	7.74	4.197	2.786	4.52	7.9	.000	3.16	4.52
37	8.35	4.277	2.906	5.16	11.1	.000	4.44	5.16
38	8.84	4.353	2.955	5.81	14.1	.000	5.65	5.81
39	9.26	4.424	2.989	6.43	17.0	.000	6.78	6.43
40	9.64	4.489	3.001	7.04	19.6	.000	7.82	7.04
41	9.99	4.507	3.003	7.45	22.1	.000	8.64	7.45
42	10.31	4.515	3.010	7.53	24.9	.000	9.48	7.53
43	10.61	4.524	3.019	7.62	27.9	.000	10.40	7.62
44	10.90	4.534	3.028	7.70	31.1	.000	11.37	7.70
45	11.18	4.545	3.037	7.80	34.5	.000	12.40	7.80
46	11,40	4.556	3.046	7.90	37.9	.000	13.46	7.90
47	11.51	4.567	3.055	8.00	41.5	.000	14.53	8.00
48	11.55	4.577	3.064	8.10	44.9	.000	15.58	8.10
49	11.58	4.588	3.072	8.20	48 3	.000	16.61	8.20
50	11.60	4.598	3.080	8.30	51.6	.000	17.61	8.30
51	11.63	4.608	3.088	8.40	64.8	800.	18.60	8.40
52	11.66	4.618	3.095	8.49	58.0	.018	19.56	8.49
53	11.70	4.628	3.103	8.58	61.1	.028	20.51	8.58
54	11.74	4.637	3.110	8.68	64.2	.037	21.44	8.68
55	11.79	4.647	3.117	8.77	67.2	.047	22.36	8.77
56	11.85	4.656	3.124	8.86	70.2	.056	23.27	8.86
67	11.91	4.665	3.130	8.95	73.1	. 065	24.17	8.95
58	11.93	4.674	3.137	9.04	76.0	. 074	25.05	9.04
59	11.82	4.682	3.145	9.12	78.7	.082	25.87	9.12
60	11.58	4.690	3.152	9.20	81.1	.090	26.60	9.20
- 61	11.29	4.696	3.160	9.27	83.1	.096	27.22	9.27
62	11.00	4.701	3.167	9.32	84.8	.101	27.73	9.32
63	10.70	4.705	3.174	9.37	86.1	.105	28.13	9.37
64	10.41	4.709	3.182	9.41	87.1	.109	28.43	9.41
65	10.13	4.711	3.189	9.43	87.8	.111	28.65	9.43
66	9.88	4.712	3.196	9.45	88 3	.112	28.78	9.45
67	9.63	4.713	3.203	9.46	88 4	.113	28.83	9.46
68	9.39	4.712		9.46	88.4	.112	28.81	9.46
69	9.12	4.711	3.216	9.45	88.0	.111	28.71	9.45
70	8.83	4.709	3,222	9.44	87.4	.109	28.52	9.44
71	8.51	4.707	3.229	9.42	86.5	.107	28.25	9.42
7.2	8.18	4.703	3,235	9.38	85.3	.103	27.88	9.38

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***	INUMPATION	WUMP1919	BLOCK No.	ь	***	1	CASE	PLAN

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	UNOFF ISCHARGE		OUTSIDE			I NUNDA		DOLLAR OF PROGRESS OF PROGRESS OF A COMMENT
	(1000m ³)	(EL.m)	W.L. (EL.m)	DISCHARGE (1000m ³) (AREA (ha)	DRAINAGE DISCHARGE of FACILITIES (1000m ³)
*****	*******	******	******	/ CHROOFF	******	* (III)	(IIc.)	***************
73	7.85	4.698	3.240	9.34	83.8	.098	27.43	9.34
74	7.52	4.693	3.246	9.29	82.1		26.89	9.29
75	7.20	4.686	3.251	9.24	80.0	.086	26.27	9.24
76	6.89		3.256	9.17	77.7	.079		9.17
77	6.59	4.672	3.260		75.2	.072	24.81	9.10
78	6.29	4.663	3.265	9.02	72.5		23.98	9.02
79	6.01	4,654	3.269	8.93	69.6	.054	23.09	8.93
80	5.73	4.644	3.273	8.84	66.5	.044	22.14	8.84
81	5.48	4.634	3.276	8.75	63.2	.034	21.15	8.75
82	5.23	4.624	3.279	8.65	59.8	.024	. 20.11	8.65
83	5.01	4.613	3.282	8.54	56.2	013	19.03	8.54
84	4.83	4.601	3.284	8.44	52.6	.001	17.93	8.44
85	4.68	4.590	3.286	8.33	49.0	.000	16.82	8.33
86	4.55	4.579	3.287	8.22	45.3	.000	15.70	8.22
87	4.41	4.567	3.288	8.11	41.6	. 000	14.58	8.11
88	4.27	4.556	3.288	8.00	37.9	.000	13.44	8.00
89	4.13	4.544	3.289	7.90	34.1	.000	12.29	7.90
90	3.99	4.532	3.288	7.79	30.3	.000	11.14	7.79
91	3.85	4.520	3.288	7.68	26.5	.000	9.97	7.68
92	3.71	4.508	3.287	7.57	22.6	.000	8.80	7.57
93	3.58	4.470	3.285	7.42	18.8	.000	7.52	7.42
94	3.44	4.386	3.283	6.79	15.4	.000	6.18	6.79
95	3.30	4.316	3.281	6.12	12.6	.000	5.05	6.12
96	3.16	4.265	3.278	5.57	10.2	.000	4.09	5.57
97	3.02	4.203	3.275	5.11	8.1	.000	3.25	5.11
98	2.88	4.158	3.271		6.3	.000	2.52	4.72
99	2.75	4.117	3.267	4.38	4.7	.000	1.87	4.38
100	2.61	4.080	3.263	4.09	3.2	.000	1.27	4.09
101	2.47	4 046	3.258	3.82	1.8	.000	. 73	3.82
102	2.35	4.015	3.253	3.59	6	.000	. 24	3.59
103	2.25	4.000	3.248	2.85	. 0	.000	.00	3.40
104	2.19	4.000	3.242	2.19	.0	.000	.00	3.37
105	2.12	4.000	3.236	2.12	0	.000	.00	3.37
106 107	2.06	4.000	3.230	2.06	.0	.000	.00	3.37
108	2.00 1.94	4.000	3.223	2.00	.0	.000	.00	3.37
109	1.88	4.000	$3.217 \\ 3.210$	1.94 1.88	. 0	.000	.00	3.37
110	1.81	4.000	3.202	1.81	.0	.000	.00	3.37
111	1.75	4.000	3.195	1.75	.0	.000	.00	3.37
112	1.69	4.000	3.188	1.69	.0	.000	.00	3.37
113	1.64	4.000	3.180	1.64	.0	.000	.00	3.37 3.37
114	1.59	4.000	3.172	1.59	.0	.000	.00	3.37
115	1.54	4.000	3.164	1.54	.0	.000	.00	3.37
116	1.50	4.000	3.155	1.60	Ŏ	.000	.00	3.37
117	1.46	4.000	3.145	1.46	.0	.000	.00	3.37
118	1.42	4.000	3.133	1.42	.0	.000	.00.	3.37
119	1.38	4.000	3.121	1.38	. 0	.000	.00	3.37
120	1.34	4.000	3.109	1.34	.0	.000	.00	3.37
121	1.30	4.000	3.096	1.30	. 0	.000	.00	3.37
122	1.26	4.000	3.084	1.26	.0	.000	.00	3.37
123	1.23	4.000	3.071	1.23	.0	.000	.00	3.37
124	1.20	4.000	3.059	1.20	.0	.000	.00	3.37
125	1.17	4.000	3.046	1.17	. 0	.000	.00	3.37
126	1.14	4.000	3.034	1.14	. 0	.000	00	3.37
127	1.11	4.000	3.021	1.11	.0	.000	.00	3.37
128	1.08	4.000	3.009	1.08	.0	.000	.00	3.37
129	1.05	4 000	2.951	1.05	.0	.000	.00	3.37
130	1.02	4.000	2.929	1.02	. 0	.000	.00	3.37
131	1.00	4.000	2.855	1.00	. 0	.000	.00	3.37
132	. 98	4.000	2.765	. 98	. 0	000	.00	3.37
133 134	. 95 . 93	4.000	2.746	. 95	. 0	.000	.00	3.37
135	.93	4.000	2.729	.93	.0	.000	.00	3.37
136	.89	4.000	2.720 2.721	.91	.0	.000	. 00	3.37
137	.80	4.000	2.612	.89	0	. 000	.00	3.37
138	.56	4.000	2.538	.80	0	.000	.00	3.37
139	.25	4.000	2.509	. 56 . 25	.0	.000	.00	3.37
140	.05	4.000	2.500	.05	.0	.000	.00	3.37
141	.00	4.000	2.500	.00	.0	.000	.00	3.37
142	.00	4.000	2.500	.00	.0	.000	.00	3.37
143	,00	4 000	2.500	.00	.0	.000	.00	3.37 3.37
144	.00	4.000	2.500	.00	.0	.000	.00	3.37

(2) Tan Chi and Han Quang Case

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69.33 93.60 10.79 69.68 93.60 10.73 70.01 93.60

70.61 93.60

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631.3 657.7

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39.75

39.52

39.31

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69

70

2.604

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2.617

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9.042

9.046

9.051

9.055

9.060

*	INUNDATION	ANALYSIS	BLOCK	No.	1	***	CASE	PLAN		

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	RUNOFF	INSIDE		DRAINAGE			KOLTA	
TIME	DISCHARGE	W.L.	W.L. 1	DISCHARGE	PONDAGE	DEPTH	AREA	DRAINAGE DISCHARGE of FACILITIES (1000m ³)
(hr)	(1000m ³)	(EL.m)	(EL.m)	(1000m³)	(1000*cum)	(m)	(ha)	1 2 8
****	********	******	******	*******	********	******	******	***********
73	38.39	2.637	9.043	12.24	710.2	.137	247.60	10.47 70.89 93.60
74	37.94	2.643	9.025	12.09	736.1	.143	256.22	10.37 71.14 93.60
75	37.44	2.650	9.008	11.97	761.6	. 150	264.72	10.26 71.37 93.60
76	36.93	2.656	8.990	11.87	786.6	156	273.08	10,16 71.58 93.60
77	36.43	2.662	8.973	11.79	811.3	.162	281.30	10.05 71.77 93.60
78	35.93	2.668	8.956	11.73	835.5	.168	289.38	9.94 71.93 93.60
. 79		2.674	8.938	11.70	859.2	.174	297.30	9.82 72.07 93.60
	35.45		8.920	11.69	882.5	.180	305.07	9.71 72.20 93.60
80	34.97	2.680		11.71	905.3	.186	312.68	9.60 72.29 93.60
81	34.51	2.686	8.903			.191	320.13	9.49 72.37 93.60
82	34.07	2.691	8.885	11.75	927.6			
83	33.65	2.697	8.868	11.81	949.5	.197	327.41	9.38 72.41 93.60
84	33.22	2.702	8.850	11.91	970.8	. 202	334.53	9.27 72.42 93.60
85	32.72	2.707	8.833	12.02	991.5	.207	341.43	9.17 72.40 93.60
86	32.09	2.712	8.815	12.16	1011.4	.212	348.09	9.08 72.36 93.60
87	31.34	2.717	8.798	12.30	1030.4	.217	354.44	9.01 72.28 93.60
88	30.54	2.722	8.780	12.45	1048.5	.222	360.47	8.97 72.18 93.60
89	29.75	2.726	8.763	12.61	1065.7	.226	366.19	8.94 72.05 93.60
90	29.02	2.730	8.745	12.77	1081.9	.230	371.61	8.93 71.90 93.60
91	28.30	2.734	8.728	12.94	1097.3	.234	376.74	8.94 71.72 93.60
92	27.60	2.738	8.710	13.11	1111.8	. 238	381.57	8.97 71.52 93.60
	26.91	2.741	8.693	13.30	1125.4	.241	386.12	9.01 71.29 93.60
93				13.51	1138.1	.244	390.36	9.06 71.04 93.60
94	26.24	2.744	8.675				394.31	9.10 70.74 93.60
95	25.58	2.747	8.658	13.76	1149.9	.247		
96	24.94	2.750	8.640	14.04	1160.8	.250	397.95	9.15 70.41 93.60
97	24.33	2.752	8.623	14.35	1170.8	.252	401.28	9.19 70.06 93.60
98	23.73	2.755	8.607	14.67	1179.9	.255	404.30	9.24 69.69 93.60
99	23.15	2.757	8.590	15.02	1188.0	.257	407.01	9.28 69.30 93.60
100	22.56	2 759	8.573	15.39	1195.2	.259	409.40	9.32 68.90 93.60
101	21.98	2.760	8.557	15.76	1201.4	.260	411.48	9.36 68.48 93.60
102	21.39	2.761	8.540	16.15	1206.8	.261	413.22	9.40 68.04 93.60
103	20.79	2.763	8.523	16.56	1210.9	.263	414.63	9.45 67.59 93.60
					1214.0	.263	415.70	9.50 67.10 93.60
104	20.19	2.763	8.507	17.00		. 264	416.42	9.55 66.60 93.60
105	19.61	2.764	8.490	17.45	1216.2		416.81	9.61 66.10 93.60
106	19.05	2.764	8.473	17.89	1217.4	. 264		
107	18.51	2 764	8.457		1217.5	.264	416.87	9.68 65.59 93.60
108	17.98	2.764	8.440	18.78	1216.7	.264	416.60	9.75 65.07 93.60
109	17.47	2.764	8.423	19.23	1215.0	. 264	416.01	9.82 64.55 93.60
110	16.98	2.763	8.407	19.69	1212.3	.263	415.11	9.89 64.02 93.60
111	16.53	2.762	8.390	20.16	1208.6	. 262	413.90	9.96 63.48 93.60
112	16.10	2.761	8.373	20.63	1204 1	.261	412.39	10.03 62.94 93.60
113	15.68	2.759	8.357	21.11	1198.7	.259	410.57	10.10 62.39 93.60
114	15.27	2.758	8.340	21.59	1192.4	.258	408.46	10.17 61.84 93.60
		2.756	8.323	22.08	1185.1	. 256	406.05	10.24 61.28 93.60
115	14.86		8.307	22.59	1177.0	.254	403.34	10.32 60.69 93.60
116	14.46	2.754			1167.9	.252	400.31	10.40 60.05 93.60
117	14.07	2.752	8.290	23.15				10.48 59.34 93.60
118	13.69	2.749	8.273	23.77	1157 8	.249	396.95	10.58 58.57 93.60
119	13.32	2.745	8.257	24.45	1146.7	. 245	393.23	
120	12.95	2.743	8.240	25.15	1134.5	.243	389.17	10.68 57.78 93.60
121	12.60	2.740	8.222	26.12	1121.0	.240	384.66	10.78 56.69 93.60
122	12.13	2.736	8.203	27.30	1105.8	.236	379.59	10.91 55.38 93.60
123	11.18	2.731	8.185	31.39	1085 6	.231	372.85	11.09 51.11 93.60
124	9.53	2.724	8.167	36.59	1058.6	. 224	363.82	11.45 45.55 93.60
125	7.10	2.716	8 148	38.54	1027.1	.216	353.33	11.91 43.15 93.60,
126	4.58	2.707	8.130	39.90	991.8	. 207	341.54	12.40 41.30 93.60
127	3.30	2.698	8.112	40.67	954.4	.198	329.07	12.90 40.03 93.60
128	3.07	2.688	8.093	42.96	914.5	.188	315.76	13.37 37.27 93.60
	2.97	2.676	8.075	50.81	866.7	.176	299.80	13.88 28.92 93.60
129		2.663	8.057	55.62	813.9	.163	282.20	14.48 23.50 93.60
130			8.038	58.45	758.2	.149	263.59	15.06 20.09 93.60
131	2.70	2.649		61.36	699.1	.134	243.89	15.62 16.62 93.60
132		2.634.	8.020					
133		2.618	8.002	64.51	636.4	.118	222.94	16.16 12.93 93.60
134		2.601	7.983	68.40	569.3	.101	200.56	16.68 8.52 93.60
135		2.583	7,965	75.66	494.8	.083	175.72	17.20 .74 93.60
136		2.564	7.947	75.98	420.0	.064	150.75	17.62 .00 93.60
137		2.545	7.928	75.69	345.4	,045	125.88	17.91 .00 93.60
138		2.526	7.910	75.49	271.1	.026	101.06	18.11 .00 93.60
139		2.508	7.892	75.32	196.8	.008	76.27	18.28 .00 93.60
140		2.388	7.873	68.80	128.7	.000	51.18	19.54 10.71 93.60
141		2.199	7.855	62.83	66.2	.000	26.32	21.63 26.89 93.60
142		2.005	7.837	64.55	1.7	.000	. 67	21.11 26.75 93.60
		2.002	7.818	.86	.8	.000	.33	20.82 26.75 93.60
143		2.002	7.800	.14	.7	.000	.27	20.53 26.75 93.60
144	.00	". UUZ	, , , , , ,	•				

***	INUNDATION	ANALYSIS	BLOCK No.	2 ***	CAS

PLAN.

RUNOFF DRAINAGE INSIDE OUTSIDE --INUNDATION--TIME DISCHARGE W.L DISCHARGE PONDAGE DEPTH AREA DRAINAGE DISCHARGE of FACILITIES (EL.m) $(1000m^3)$ (EL.m) (hr) (1000m³) (1000*cum) (m) (ha) ****** 1 . on 2.590 2.501 14.61 132.8 .000 53.15 14.61 .00 119.0 2.581 2.501 13.83 13.49 .000 47.62 13.83 2.572 2.483 .01 . 000 42.22 13.49 14.57 .02 2.562 2.451 14.57 90.9 .000 36.40 15.90 17.17 .03 2.551 2.413 15.90 17.17 75.1 .000 30.05 2.539 .05 58.0 .000 23,20 .07 2.527 2.322 18.29 39.7 .000 15.91 18.29 8 .10 2.514 $2.271 \\ 2.202$ 19.19 19.24 20.7 .000 8.27 2.501 1.6 .000 . 63 19.24 10 .30 2.500 2 078 1.87 . 0 .000 .00 18.65 11 .54 .54 .74 . 0 2.500 2 005 .000 .00 18.65 2.500 2.004 .000 . 0 .00 18.65 13 .85 2.500 2.004 85 .000 .00 18.65 . 93 . 93 2.500 2.004 . 0 .000 .00 18.65 1.00 2.500 2.003 1.00 . 0 : 000 . 00 18.65 16 1.06 2.500 2.003 1.06 . 0 .000 .00 18.65 17 1.12 2,500 2.003 1.12 1.17 . 0 .000 .00 2.500 2.003 .000 . 0 . 00 18.65 19 1.22 2.500 2.002 , õ .000 . 00 18.65 20 1.26 2.001 2.001 1.26 . 0 .000 .00 18.65 2.500 1.31 . 0 .000. 00 18.65 22 1.35 2.500 2.001 1.35 .000 .00 18.65 23 1.44 2.500 1.44 1.58 2.001 . 0 .000 .00 18.65 1.58 2.500 2.001 .000 . 0 . 00 18.65 25 1.76 2.500 2.001 1.76 . 0 . 000 .00 18.65 26 1.96 2.500 1.96 2.18 . 0 .000 .00 18.65 2.18 2.500 2.001 . 0 .000 .00 18.65 2.43 28 2.43 2.500 2.001 . 0 .000 .00 18.65 29 2.500 2.001 . 0 .000 .00 18.65 30 3.07 2.500 2.002 3:07 . 0 .000 :00 18.65 31 3.50 2.500 2.002 3.50 . o .000 .00 18.65 32 4.05 2.500 2.002 0 .000 4.05 .00 18.65 33 5.19 2.500 2.003 5,19 .00 18.65 34 7.63 2,500 2.004 7.63 . 0 .000 .00 18.65 35 10.51 2.500 2.005 .000 10.51 . 0 .00 18.65 36 12.41 2.500 2.006 12.41 . 0 .00 18.65 37 13.49 2.500 2.007 13,49 . 0 .000 .00 18.65 2.500 2.017 38 14.33 14.33 . 0 .000 .00 18.65 39 15.05 2.500 2.040 15.05 . 0 .00 18.65 40 15.68 2,500 2.072 15.68 .000 .00 18.65 2.500 41 16.26 2.109 16.26 . 0 .000 .00 18.65 42 16.80 2.500 2.152 16.80 . 0 . 00 18.65 2,500 2.201 2.255 17.30 17.78 43 17.30 .000 .00 18,65 17.78 44 . 0 .000 .00 19.02 45 18.24 2.500 2.315 18.14 . 1 .04 18.66 46 18.62 2.501 2.375 16.66 2.1 .000 .82 16.66 2.505 18.85 2.430 13.99 6.9 15.3 .000 2.77 13.99 48 18.94 2.510 2.477 10.60 6.11 10.60 2.519 49 18.98 2.501 6.83 27.4 .000 10.97 6.83 19.02 2.527 2.504 50 6.72 7.49 39.7 .000 15.89 6.72 51 19.07 2.535 2.508 51.3 .000 20.53 7.49 2.512 52 19.12 2.542 8.09 62.3 .000 24.94 8.09 19.18 2.549 53 8.57 72.9 .000 29.19 8.57 54 19.24 2.556 2.520 8.97 83.2 .00033.30 8.97 55 19.32 2.563 2.525 9.30 93.2 .000 37.31 9.30 2.570 19.41 56 2.530 9.58 103.0 .000 41.24 9.58 2.577 2.535 9.82 57 19.60 $112.8 \\ 122.7$.000 45.16 9.82 10.03 58 19.93 2.583 2.540 .000 49.12 10.03 $2.546 \\ 2.552$ 10.21 10.37 59 20.28 2.590 132.8 .000 53.15 10.21 60 2.597 20.49 142.9 .000 57.20 10.37 2.558 2.565 61 20.61 2.604 10.50 153.0 .004 61.25 10.50 20.66 2.611 $10.62 \\ 10.72$ 163.1 173.0 62 .011 65.27 10.62 63 20.61 2.617 2.571 .017 69,23 10.72 20.45 2.624 2.577 64 10.79 182.6 .024 73.10 10.79 66 192.0 201.2 10.83 .030 76.86 10.83 66 20.02 2.636 2.590 10.86 . 036 80.53 10.86 67 19.80 2.643 2.597 10.85 210.1 .043 84.11 10.85 2.648 68 19.58 2.604 218.9 227.5 10.83 .048 87.61 10.83 69 19.36 2.654 2.610 10.79 .054 91.04 94.41 10.79 10.73 70 71 19.14 2.660 2.617 10.73 235.9 .060 18.93 2.666 2.624 244.1 252.3 10.65 .066 97.72 2.671 2.630 10.56 .071 100.98 10.56

CASE PLAN

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•	RUNOFF	INSIDE	OUTSIDE	DRAINAGE			OATION					
TIME	DISCHARGE	w.L.	W.L.	DISCHARGE.	PONDAGE	DEPTH	AREA	DRA I NAGI	E DISCHARGE	of FACIL	ITIES (1000m³)
(hr)	$(1000m^3)$	(m.13)	(EL.m)	$(1000n^3)$ (1000*ວແສ)	(m)	(ha)	1				
*****		*****	*******	********	******	******	*******		*********	*******	******	******
73	18.50	2.677	2.637	10.47	260.3	.077	104.20	10.47				
74	18.29	2.682	2.643	10.37	268.2	.082	107.37	10.37				
75	18.07	2.687	2.650	10.26	276.1	. 087	110.50	10.26	•			
76	17.84	2.692	2.656	10.16	283.7	.092	113.57	10.16				
77	17.59	2.698	2.662	10.05	291.3	.098	116.59	10.05				
78	17.34	2.703	2.668	9.94	298.7	.103	119.56	9.94				
						.108	122.46	9.82				
79	17.09	2.708	2.674									
80	16.85	2.712	2.680	9.71	313.1	.112	125.32	9.71				
81	16.61	2.717	2.686	9.60	320.1	.117	128.13	9.60				
82	16.37	2.722	2.691	9.49	327.0	.122	130.88	9.49				
83	16.14	2.726	2.697	9.38	333.7	126	133.59	9.38				
84	15.92	2.731	2.702	9.27	340.4	. 131	136.25	9.27				
85	15.71	2.735	2.707	9,17	346.9	.135	138.87	9.17	•			
86	15.50		2.712		353.4	.140	141.44	9.08				
87	15.30	2.744	2.717	9.01	359.6	.144	143.95	9.01				
88	15.11	2.748	2.722	8.97	365.8	148	146.41	8.97				
			2.726	8.94	371.8	152	148.81	8 94				
89	14.93	2.752										
90	14.76	2.756	2.730	8.93	377.6	.156	151.15	8.93				
91	14.61	2.760	2.734	8.94	383.3	.160	153.42	8,94				
92	14.48	2.764	2.738	8.97	388.8	.164	155.62	8.97				
93	14.29	2.767	2.741	9.01	394.1	.167	157.74	9.01				
94	14.03	2.771	2.744	9.06	399.1	171	159.73	9.06				
95	13.74	2.774	2.747	9.10	403.7	174	161.58	9.10				
96	13.44	2.777	2.750	9.15	408.0	. 177	163.30	9.15				
97	13.13	2.779	2.752	9.19	411.9	.179	164.88	9.19				
98	12.83	2.782	2.755	9.24	415.5	.182	166.32	9.24				
- 99		2.784	2.757	9.28	418.8	.184	167.62	9.28				*
	12.53											
100	12.24	2.786	2.759	9.32	421.7.	.186	168.79	9.32				
101	11.95	2.788	2.760	9.36	424.3	.188	169.83	9.36				
102	11.66	2.789	2.761	9.40	426.5	.189	170.74	9.40				
103	11.38	2.791	2.763	9.45	428.5	.191	171.51	9.45				
104	11.11	2.792	2.763	9.50	430.1	. 192	172.15	9.50		•		
105	10.85	2.793	2.764	9.55	431.4	.193	172.67	9.55				
106	10.59	2.793	2.764	9.61	432.4	.193	173.06	9.61				
107	10.34	2.794	2.764	9.68	433.0	.194	173.32	9.68				
108	10.08	2.794	2.764	9.75	433.4	.194	173.46	9.75				
109	9.83	2.794	2.764	9.82	433.4	.194	173.46	9.82				
110	9.57	2.794	2.763	9.89	433.1	.194	173.34	9.89				
111	9.31	2.793	2.762	9.96	432.4	.193	173.08	9.96				
112	9.05	2.793	2.761	10.03	431.4	.193	172.69	10.03				
113	8.81	2.792	2.759	10.10	430.1	. 192	172.17	10.10				
114	8.59	2.791	2.758	10.17	428.6	.191	171.54	10.17				
115	8.39	2.789	2.756	10.24	426.7	.189	170.80	10.24				
116	8.18	2.788	2.754	10.32	424.6	. 188	169.95	10.32				
117	7.98	2.786	2.752	10.40	422.2	.186	168.98	10.40				
118	7.79	2.785	2.749	10.48	419.5	. 185	167.90	10.48				
119	7.59	2.783	2.746	10.58	416.5	.183	166.71	10.58				
120	7.40	2.780	2.743	10.68	413.2	.180		10.68				
							165.40					
121	7.21	2.778	2.740	10.78	409.6	.178	163.97	10.78				
122	7.03	2.775	2.736	10.91	405.8	175	162.41	10.91				
123	6.85	2.772	2.731	11.09	401.5	172	160.72	11.09				
124	6.67	2.769	2.724	11.45	396.7	.169	158.80	11.45				
125	6.51	2.765	2.716	11.91	391.3	165	156 64	11.91				
126	6.35	2.761	2.707	12.40	385.3	. 161	154.22	12.40				
127	6.20	2.757	2.698	12.90	378.6	. 157	161.53	12.90				
128	6.05	2.752	2.688	13.37	371.3	.152	148.60	13.37				
129	5.90	2.746	2.676	13.88	363.3	146	145.41	13.88				
130	5.76	2.741	2.663	14.48	354.6	141	141.92	14.48				
			2.649	15.06		134		15.06				
131	5.62	2.734			345.1		138.14					
132	5.49	2.727	2.634	15.62	335.0	.127	134.09	15.62				
133	5.34	2.720	2.618	16.16	324.2	120	129.75	16.16				
134	4.88	2.712	2.601	16.68	312.4	.112	125.03	16.68				
135	3.62	2.703	2.583	17.20	298.8	.103	119.60	17.20				
136	1.76	2.692	2.564	17.62	282.9	.092	113.25	17.62				
137	.40	2.680	2.545	17.91	265.4	.080	106.24	17.91				
138	.00	2.668	2.526	18.11	247.3	.068	98.99	18.11				
139	.00	2.655	2.508	18.28	229.0	055	91.67	18.28				
140	00	2.642	2.388	19.54	209.5	042	83.85	19.54				
	.00	2.627		21.63	187.9	.027	75.19	21.63				
141												
142	.00	2.613	2.005	21.11	166.7	.013	66.74	21.11				
143	.00	2.599	2.002	20.82	145.9	.000	68 . 41	20.82				
144	.00	2.585	2.002	20.53	125.4	.000	50.19	20.53				

***	INUNDATION	ANALYSIS	BLOCK No. 3 ***	CASE	PLAN

*****	RUNOFF	INSIDE	OUTSIDE	DRAINAGE	I NUN	DATION	* ************************************
TIME	DISCHARGE	W.L.	W.L.		PONDAGE DEPTH		DRAINAGE DISCHARGE of FACILITIES (1000m3)
(hr)	$(1000m^3)$	(EL.m)	(EL.m)		1000*cum) (m)		2 3 4 7 9
*****			******	,	er and the second of the secon	*******	
1	.00	3.322	2.501	129.55	540.3 000	213.27	83.40 .00 .00, 8.25 54.40
2 3	.00	3.247	2.501	123.97	416.3 .000	164.33	77.59 .00 .00 8.01 54.40
4	.02	3.184 3.134	2.483	104.91 83.29	311.4 .000 228.1 .000	122.93 90.06	72.32 .00 14.03 7.78 54.40 68.28 .00 31.82 7.56 54.40
5	.05	3.089	2.413	75.13	153.0 .000	60.42	64.89 .00 37.35 6.81 54.40
6	.09	3.049	2.369	67.33	85.8 .000	33.89	61.82 3.28 40.11 5.50 54.40
7	.18	3.021	2.322	46.18	39.8 .000	15.73	59.47 22.86 40.31 4.51 54.40
8		2.927	2.271	35.83	4.3 000	1.71	57.44 32.63 39.63 3.74 54.40
9		2.900	2.202	85	4.0 .000	1.60	51.12 55.78 48.53 3.37 54.40
10	1.04	2.882	2.078	1.21	3.8 .000	1.53	49.86 57.21 45.60 3.37 54.40
11 12	1.94 3.27	2.841	2.005	2.36	3.4 000	1.36	48.47 59.05 41.07 3.37 54.40
13	4.77	2.708	2.004	3.96 5.40	2.7 000	1.08 .83	44.48 67.84 26.59 3.37 54.40 40.36 69.02 19.79 3.37 54.40
14	6.08	2.655	2.004	6.61	1.5 .000	. 62	36.92 66.34 17.77 3.37 54.40
15	6.98	2.606	2.003	7.48	1.1 .000	.42	33.95 63.13 17.11 3.37 54.40
16	7.56	2.556	2.003	8.06	. 6 . 000	.22	31.30 59.75 16.66 3.37 54.40
17	7.93	2.500	2.003	11.89	0 000	.00	30.08 50.63 12.62 3.37 54.40
18	8.14	2.500	2.003	17.40	.0 .000	.00	29.01 44.29 11.52 3.37 54.40
19	8.20	2.500	2.002	15.03	.0 .000	.00	27.92 39.61 13.78 3.37 54.40
20 21	8.13 7.98	2.500	2.001	13.05	.0 .000	.00	26.82 35.49 13.62 3.37 54.40
22	7.80	2.500	2.001	25.82 35.06	.0 .000	.00	26.75 25.62 10.13 3.37 54.40 26.75 16.92 9.38 3.37 54.40
23	7.68	2.500	2.001	40.91	.0 000	.00	26.75 16.92 9.38 3.37 54.40 26.75 10.99 9.28 3.37 54.40
24	7.75	2.500	2.001	43.22	.0 000	.00	26.75 8.45 9.48 3.37 54.40
25	8.08	2.500	2.001	43.36	.0 000	.00	26.75 8.02 9.97 3.37 54.40
26	8.58	2.500	2.001	42.76	.0 .000	.00	26.75 8.30 10.68 3.37 54.40
27	9.27	2.500	2.001	41.90	.0 .000	.00	26.75 8.82 11.57 3.37 54.40
28	10.10	2.500	2.001	40.90	.0 .000	.00	26.75 9.47 12.59 3.37 54.40
29	11.14	2.500	2.001	39.83	.0 000	.00	26.75 10.22 13.75 3.37 54.40
30	12.45	2.500	2.002	38.68	.0 .000	.00	26.75 11.07 15.15 3.37 54.40
31 32	14.12 16.27	2.500	2.002	37.29	.0 .000	.00	26.75 12.10 16.90 3.37 54.40
33	19.94	2.500	2.002	35.42 32.58	.0 .000 .000.	.00	26.75 13.46 19.20 3.37 54.40 26.75 15.52 22.99 3.38 54.40
34	27.48	2.500	2.004	30.37	.0 .000	.00	26.75 15.52 22.99 3.38 54.40 26.98 16.71 29.84 3.52 54.40
35	39.62	2.635	2.005	38.29	1.3 000	54	29.82 4.23 37.75 3.94 54.40
36	52.85	2.859	2.006	50.61	3.6 .000	1.43	40.70 .00 39.96 4.52 54.40
37	62.58	2.937	2.007	61.79	4.4 .000	1.75	50.88 .00 38.32 5.16 54.40
38	67.34	2.973	2.017	66.98	4.7 .000	1.89	54.04 .00 35.65 5.81 54.40
39 40	68.49 67.77	2.999	2.040	68.23	5.0 .000	1.99	56.12 .00 35.85 6.43 54.40
41	66.36	3.001 3.004	2.072	65.63 61.31	7.1 .000 12.2 .000	2.84 4.83	57.12 .00 38.85 7.04 54.40 57.28 .00 42.91 7.45 54.40
42	64.95	3.008	2.152	58.74	18.4 .000	7.28	57.28 .00 42.91 7.45 54.40 57.51 .00 45.63 7.53 54.40
43	63.81	3.014	2.201	53.97	28.2 .000	11.16	57.81 3.08 47.53 7.62 54.40
44	62.95	3.023	2.255	48.72	42.4 ,000	16.78	58.33 7.74 48.56 7.70 54.40
45	62.44	3.031	2.315	47.87	57.0 .000	22.53	58.93 8.44 49.22 7.80 54.40
46	62.23	3.040	2.375	47.83	71.4 000	28.21	59.53 8.47 49.72 7.90 54.40
47	62.05	3.048	2.430	48.06	85.4 .000	33.74	60.12 8.30 50.16 8.00 54.40
48 49	61.64 60.99	3.056	2.477	48.25	98.8 .000	39.02	60.69 8.19 50.54 8.10 54.40
50	60.21	3.071	2.501	48.30 48.21	111.5 .000 123.5 .000	44.04	61.24 8.25 50.89 8.20 54.40 61.75 8.46 51.19 8.30 54.40
51	59.43	3.078	2.508	48.03	123.5 .000 134.9 .000	48.77 53.27	61.75 8.46 51.19 8.30 54.40 62.25 8.77 51.44 8.40 54.40
52	58.71	3.085	2.512	47.83	145.8 .000	57.57	62.72 9.14 51.65 8.49 54.40
53	58.07	3.091	2.516	47.62	156.3 .000	61.70	63.17 9.54 51.83 8.58 54.40
54	57.52	3.097	2.520	47.42	166.3 .000	65.68	63.60 9.93 51.97 8.68 54.40
55	57.05	3.103	2.525	47.25	176.1 .000	69.55	64.02 10.31 52.09 8.77 54.40
56 57	56.67 56.49	3.109	2.530	47.11	185.7 .000	73.32	64.44 10.67 52.19 8.86 54.40
58	56.65	3.114 3.120	2.535 2.540	47.02	195.2 .000	77.07	64.84 10.98 52.29 8.95 54.40
59	57.08	3.126	2.546	47.02 47.13	204.8 .000 214.8 .000	80.87	65.25 11.19 52.41 9.04 54.40
60	57.50	3.132	2.552	47.13	214.8 .000 224.9 .000	84.80 88.81	65.67 11.27 52.54 9.12 54.40 66.10 11.27 52.70 9.20 54.40
61	57.70	3.138	2.558	47.53	235.1 .000	92.83	66.54 11.27 52.87 9.27 54.40
62	57.63	3.144	2.565	47.66	245.1 .000	96.76	66.97 11.35 53.03 9.32 54.40
63	57.38	3.150	2.571	47.70	254.8 ,000	100.58	67.40 11.54 53.18 9.37 54.40
64	57.03	3.156	2.577	47.67	264.1 000	104.28	67.81 11.84 53.29 9.41 54.40
65	56.67	3,161	2.684	47.61	273.2 .000	107.85	68.21 12.22 53.34 9.43 54.40
66	56.32	3.167	2.590	47.57	281.9 .000	111.31	68.59 12.66 53.32 9.45 54.40
67 68	55.99 55.66	$3.172 \\ 3.177$	2.597 2.604	47.54 47.55	290.4 .000	114.64	68.97 13.13 53.23 9.46 54.40
69	55.35	3.181	2.610	47.55	298.5 .000 306.2 .000	117.84 120.90	69.33 13.64 53.08 9.46 54.40 69.68 14.16 52.86 9.45 54.40
70	55.03	3.186	2.617	47.68	313.6 .000	120.90	70.01 14.68 52.60 9.44 54.40
71	54.70	3.190	2.624	47.78	320.5 .000	126.54	70.32 15.22 52.30 9.42 54.40
72	54.34	3.194	2.630	47.88	327.0 .000	129.09	70.61 15.78 51.97 9.38 54.40

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	RUNOFF		OUTSIDE	DRAINAGE		MDATION	DDATMACE DISCH	SADOR - E PAGILI	mrne (1000=3)
	DISCHARGE	W.L.	W.L.		PONDAGE DEPTI			ARGE of FACILI	1152 (1000m.)
(hr)	(1000m ³)	(EL.m)	(EL.m)	(1000m ³) ((1000*cum) (m)	(ha)	2 3 4	7 9	
	*********	******					* ************************************	************	******
73	53.96	3.197	2.637	47.99	332.9 .000			9.34 54.40	*
74	53.57	3.201	2.643	48.09	338.4 .000		71.14 16.95 51.21	9.29 54.40	
75	53.20	3.204	2.650	48.22	343.4 .000		71.37 17.56 50.75	9.24 54.40	
76	52.84	3.206	2.656	48.38	347.9 .000		71.58.18.20 50.22	9.17 54.40	
77		3.209	2.662	48.55	351.8 .000	138.89	71.77 18.87 49.65	9.10 54.40	
78	52.19	3.211	2.668	48.71	355.3 .000	140.26	71.93 19.54 49.05	9.02 54.40	
79	51.89	3.213	2.674	48.88	358.3 .000		72.07 20.22 48.43	8.93 54.40	•
		3.214	2.680	49.07	360.8 .000		72.20 20.89 47.79	8.84 54.40	
80	51.58				362.8 .000		72.29 21.55 47.12	8.75 54.40	
81	51.22	3.215	2.686				72.37 22.21 46.42	8.65 54.40	
82	50.79	3.216	2.691	49.49	364.1 .000				
83	50.30	3.216	2.697	49.68	364.7 .000		72.41 22.89 45.69	8.54 54.40	
84	49.78	3.216	2.702	49.86	364.6 .000		72.42 23.58 44.94	8.44 54.40	
85	49 25	3.216	2,707	50.00	363.8 .000		72.40 24.29 44.18	8.33 54.40	
86	48.73	3.215	2.712	50.13	362.4 .000	143.08	72.36 25.01 43.40	8.22 54.40	
87	48.21	3.214	2.717	50.23	360.4 .000	142.29	72.28 25.72 42.62	8.11 54.40	
88	47.68	3.212	2.722		357.8 .000	141.25	72.18 26.43 41.84	8.00 54.40	
		3.210	2.726	50.32	354.6 .000		72.05 27.13 41.10	7.90 54.40	
89	47.13			50.30	350.9 .000		71.90 27.83 40.38	7.79 54.40	
90	46.55	3.208	2.730				71.72 28.52 39.70	7.68 54.40	
91	45.91	3.206	2.734	50.22				7.57 54.40	
92	45.21	3.203	2.738	50.07	341.7 .000		71.52 29.21 39.07		
93	44.47	3.199	2.741	49.91	336.3 .000		71.29 29.89 38.47	7.42 54.40	
94	43.73	3.195	2.744	50.10	329.9 .000		71.04 30.62 37.93	6.79 54.40	
95	42.99	3.191	2.747	50.14	322.7 .000		70.74 31.42 37.46	6.12 54.40	
96	42.25	3.187	2.750	50.00	315.0 .000	124.35	70.41 32.22 37.02	5.57 54.40	
97	41.52	3.182	2.752	49.76	306.7 .000	121.10	70.06 33.01 36.58	5,11 54,40	
98	40.78	3.176	2.755	49.47	298.1 .000		69.69 33.78 36.12	4.72 54.40	
		3.171	2.757	49.15	289.0 .000		69.30 34.50 35.66	4.38 54.40	
99	40.06			48.82	279.5 .000		68.90 35.20 35.19	4.09 54.40	
100	39.36	3.165	2.759				68.48 35.86 34.71	3.82 54.40	
101	38.66	3.159	2.760		269.7 .000				
102	37.95	3.153	2.761	48.13	259.5 .000		68.04 36.50 34.22	3.59 54.40	
. 103	37.24	3.146	2.763	48.26	248.5 .000		67.59 37.14 33.74	3.40 54.40	
104	36.53	3.140	2.763	48.08	236.9 .000	93.54	67.10 37.87 33.35	3.37 54.40	
105	35.82	3.133	2.764	47.43	225.3 .000	88.96	66.60 38.53 32.91	3,37 54.40	
106	35.12	3.125	2,764	46.88	213.5 .000		66,10 39.12 32.43	3.37 54.40	
107	34.44	3.118	2.764	46.39	201.6 .000		65.59 39.67 31.93	3.37 54.40	
108	33.76	3.111	2.764	45.92	189 4 .000		65.07 40.18 31.43	3.37 54.40	
				45.46	177.1 .000		64.55 40.66 30.95	3.37 54.40	
109	33.10	3.104	2.764				64.02 41.11 30.49	3.37 54.40	
110	32.44	3.096	2.763	45.00	164.5				
111	31.79	3.088	2.762	44.54	151.8 .000		63.48 41.54 30.05	3.37 54.40	
112	31.16	3.081	2.761	44.07	138.8 .000		62.94 41.94 29.63	3.37 54.40	
113	30.55	3.073	2.759	43.61	125.8 .000		62.39 42.31 29.23	3.37 54.40	
114	29 95	3.065	2.758	43.20	112.5 .000	44.44	61.84 42.64 28.81	3.37 54.40	
116	29.35	3.056	2.756	42.99	98,.9 .000	39.06	61.28 42.90 28.23	3.37 54.40	
116	28.55	3.048	2.754	43.13	84.3 .000		60,69 43,12 27,34	3.37 54.40	
117	27.22	3.038	2.752	43.41	68.1 .000		60.05 43.38 26.20	3.37 54.40	
118	25.36	3.027	2.749		50.2 .000		59,34 43.80 25.19	3.37 54.40	
							58.57 44.40 24.47	3.37 54.40	
119	23.56	3.016	2.746	42.73	31.0 .000 11.5 .000		67.78 45.05 23.95	3.37 54.40	
120	22.30	3.004	2.743	41.83					
121	21.60	2.954	2.740	28.53			56.69 51.42 29.85	3.37 54.40	
122	21.17	2.939	2.736	21.32	4.4 .000		55.38 55.19 32.00	3 37 54 40	
123	20.82	2.964	2.731	20.57	4.6 .000		51.11 69.16 24.55	3.37 54.40	
124	20 42	2.918	2.724	20.88	*		45.55 62.53 15.34	3.37 54.40	
125	19.87	2.885	2.716	20.20	3.8 .000		43.15 64.96 11.22	3.37 54.40	
126	19.20	2.864	2.707	19.40	3.6 .000	1.46	41.30 65.97 9.18	3.37 54.40	
127	18.60	2.849	2.698	18.75	3.5 .000		40.03 66.13 8.43	3.37 54.40	
128	18.09	2.847	2.688	18.10	3.5 .000				
129	17.38	2.870	2.676	17.15	3.7000		28.92 50.27 14.84	3.37 54.40	
		2.812	2.663	16.93	3.1 .000		23.50 45.45 14.49	3.37 54.40	
130	16.35						20.09 45.35 12.52	3.37 54.40	
131	15.07	2.758	2.649	15.61	2.6 .000		16.62 44.33 11.53	3.37 54.40	
132	13.68	2.707	2.634	14.19	2.1 .000				
133	12.36	2.659	2.618	12.85	1.6 .000		12.93 42.74 10.79	3.37 54.40	
134	11.32	2.610	2.601	11.80	1.1 .000		8.52 40.44 9.74	3.37 54.40	
135	10.58	2.574	2.583	10.95	7 .000		.74 37.15 6.13	3.37 54.40	
136	10.10	2.523	2.564	12.42	.2 .000		.00 33.29 7.80	3.37 54.40	
137	9.84	2.500	2.545	17.22	0 .000		00 29.90 6.48	3.37 54.40	
138	9.74	2.500	2.526	33.47	.0 .000		.00 17.32 3.04	3.37 54.40	
139	9.67	2.500	2.508	47.28	.0 .000		.00 4.08 2.78	3.37 54.40	
140	9.53	2.500	2.388	56.92	0 .000		10.71 .14 2.57	3.37 54.40	
							26.89 .05 2.45	3.37 54.40	*
141	9.14 7.94	2.500	2.199	61.06	0 .000			3.37 54.40	
142		2.500	2.005	59.98	0 .000			3.37 54.40	
143	5.49	2.500	2.002	58.07	.0 .000		26.75 .03 1.80		
144	1.99	2.500	2.002	55.67	.0 .000	.00	26.75 .02 .72	3.37 54.40	

***	INUNDATION	ANALYSIS	BLOCK No.	4	***	CASE	. 6

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	RUNOFF	INSIDE	OUTSIDE	DRAINAGE	Ii	NOITADNU	
TIME	DISCHARGE		. W.L		PONDAGE DEP	TH AREA	DRAINAGE DISCHARGE of FACILITIES (1000m3)
(hr)	(1000m³)	(EL.m)	(EL.m)	(1000m ³) ((1000*cum) (m	(ha)	3
****	********	******	*******	********	*********	*********	
1	.00	3.060	3.322	.00	699.0 .00		
2	.00	3.060	3.247	.00	699.0 .00		.00
3	.00	3.060	3.184	.00	699.0 .00		,00
4	.01	3.060	3.134	.00	699.0 .00		.00
5	.07	3.060	3.089	.00	699.1 .00		.00
6	. 17	3.059	3.049	3.28	696.0 .00		3.28
7.	. 27	3.054	3.021	22.86	673.4 .00		22.86
8	.42	3.047	2.927	32.63	641 2 .00		32.63
9	. 64	3.035	2.900	55.78	586.0 .00		55.78
10	1.12	3.022	2.882	57.21	529.9 .00		57.21
11	2.19	3.010	2.841	59.05	473.1 .00		59.05
12	3.66	2.977	2.771	67.84	408.9 .00	0 .162.98	67.84
13	4.86	2.902	2.708	69.02	344.7 .00	00 137.41	69.02
14	5.60	2.831	2.655	66.34	284.0 .00	0 113.19	66.34
15	6.12	2.765	2.606	63.13	227.0 .00	90.47	63.13
16	6.55	2 703	2.556	59.75	173.8 .00	69.27	59.75
17	6.93	2.652	2.500	50.63	130.1 .00		50.63
18	7.29	2.608	2.500	44.29	93.1 .00		44.29
19	7.60	2.571	2.500	39.61	61.1 .00		39.61
	7.79	2.539	2.500	35.49	33.4 .00		35.49
20				25.62	16.6 .00		25.62
21	7.82	2.518	2.500				16.92
22	7.72	2.507	2.500	16.92			
23	7.65	2.504	2.500	10.99	3 0 .00		10.99
24	7.75	2.503	2.600	8.45	2.3 .00		8.45
25	8.06	2.503	2.500	8.02	2.4 .00		8.02
26	8.56	2.503	2.500	8.30	2.6 .00		8.30
27	9.20	2.504	2.500	8.82	3.0 .00		8.82
28	9.96	2.504	2.500	9.47	3.5 .00		9.47
29	10.81	2.505	2.500	10.22	4.1 .00		10.22
30	11.81	2 506	2.500	11.07	4.8 .0		11.07
31	13.13	2.507	2.500	12.10	5.8 .0		12.10
32	15.02	2.509	2.500	13.46	7.4 .0		13.46
33	18.59	2.512	2.500	15.52	10.6 .0	00 4.18	15.52
34	26.76	2.524	2.500	16.71	20.5 .0		16.71
35	39.78	2.565	2.635	4.23	56.1 .0	00 22.35	4.23
36	51.40	2.625	2.859	.00	107.5 .0	00 42.84	.00
37	57.34	2.692	2.937	.00	164.8 .0	00 65.70	.00
38	59.34	2.761	2.973	.00	224.2 .0	00 89.35	.00
39	59.41	2.831	2,999	.00	283.6 .0	00 113.03	.00
40	58.22	2.898	3.001	.00	341.8 .0		.00
41	56.18	2.964	3.004	.00	398.0 .0		.00
42	53.95	3.005	3.008	.00	451.9 .0		.00
43	52.00	3.016	3.014	3.08	500.8 .0		3.08
44	50.28	3.025	3.023	7.74	543.4 .0		7.74
45	48.61	3.034	3.031	8.44	583.6 .0		8.44
46	47.04	3.043	3,040	8.47	622.1 .0		8,47
47	45.68	3.051	3.048	8.30	659.5 .0		8.30
48	44.37	3.059	3.056	8.19	695.7 .0		8.19
	43.06	3.067	3.064	8.25	730.5 .0		8.25
49							8.46
50	41.85 40.81	3.074	3.071	8.46 8.77	763.9 .0 795.9 .0		8.77
51		3.082	3.078				9.14
52	39.96	3.088	3.085	9.14			
53	39.27	3.095	3.091	9.54	856.5 .0		9.54
54	38.72	3.101	3.097	9.93	885.3 .0		9.93
55	38.28	3.108	3.103	10.31		48 291.09	10.31
56	37.94	3.114	3.109	10.67	940.5 .0		10.01
57	37.75	3.120	3.114	10.98	967.3 .0		
58	37.86	3.126	3.120	11.19	994.0 .0		11.19
59	38.32	3.132	3.126	11.27	1021.0 .0		11.27
60	38.76	3.138	3.132	11.27	1048.5 .0		11.27
61	38.94	3.144	3.138	11.27	1076.2 .0		11.27
62	38.93	3.150	3.144	11.35	1103.8 .0		11.35
63	38.85	3.156	3.150	11.54	1131.1 .0		11.54
64	38.75	3.162	3.156	11.84	1158.0 .1		11.84
65	38.65	3.168	3.161	12.22	1184 4 .1		12.22
66	38.55	3.174	3.167	12.66	1210.3 .1		12.66
67	38.45	3.179	3.172	13.13	1235.6 .1		13,13
68.	38.26	3.185	3.177	13.64	1260.2 .1		13.64
69	37.95	3.190	3.181	14.16	1284.0 .1		14.16
70	37.55	3.195	3.186	14.68		35 388.72	14.68
71	37.11	3.200	3.190	15.22	1328.8 .1		15.22
72	36.64	3.205	3.194	15.78	1349.7 .1		15.78
				-			

CASE PLAN

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	RUNOFF	INSIDE	OUTSIDE	DRAINAGE			WOITAC								
TIME	DISCHARGE	W.L.	W.L.	DISCHARGE	PONDAGE	DEPTH	AREA		INAGE	DISCH	ARGE o	î FACIL	ITIES	(1000m³)
(hr)	(1000m ³)	(EL.m)	(EL.m)	(1000m ³)	(1000*cum)) (m)	(ha)	3							
*****		******		******			********* 404.23	***** 16.35	*****	****	****	******	******	*****	* *
73	36.15	3.209	3.197			. 149		16.95							
74	35,68	3.213	3.201	16.95	1388.2	. 163	408.88	17.56							
75	35.22	3.217	3.204	17.56	1405.8	.157	413.26								
76	34.79	3.221	3.206	18.20	1422.4	.161	417.37	18.20							
77	34.38	3.224	3 209	18.87	1437.9	.164	421.21	18.87							
78	33.98	3.227	3.211	19.54	1452.4	.167	424.79	19.54							
79	33.51	3.230	3.213	20.22	1465.7	.170	428.09	20.22							
80	32.91	3.233	3.214	20.89	1477.7	.173	431.07	20.89							
81	32.17	3.235	3.215	21.55	1488.3	.175	433.71	21.55							
82	31.36	3.237	3.216	22.21	1497.5	.177	435.98	22.21	•						
83	30.51	3.239	3.216	22.89	1505.1	. 179	437.87	22.89							
84	29.66	3.240	3.216	23.58	1511.2	, 180	439.38	23.58					1		
85	28.82	3.241	3.216	24 29	1515.7	. 181	440.50	24.29							
86	28.00	3.242	3.215	25.01	1518.7	.182	441.24	25.01							
87	27.19	3.242	3.214	25.72	1520.2	,182	441.61	25.72							
88	26.41	3.242	3.212	26.43	1520.1	.182	441.60	26.43							
89	25.67	3.242	3.210	27.13	1518.7	.182	441.24	27.13							
90	24.97	3.242	3.208	27.83	1515.8	.182	440.53	27.83							
91	24.29	3.241	3.206	28.52	1511.6	.181	439.48	28.52					•		
92	23.63	3.239	3.203	29.21	1506.0	.179	438.10	29.21							
93	22.98	3.238	3.199	29.89	1499.1	.178	436.38	29.89							
94	22.34	3.236	3.195	30.62	1490.8	.176	434.33	30.62							
95	21.69	3.234	3.191	31.42	1481.1	.174	431.92	31.42							
96	21.04	3.231	3.187	32.22	1469.9	.171	429.15	32.22							
97	20.39	3,229	3.182	33.01	1457.3	. 169	426.02	33.01							
98	19.77	3.225	3.176	33.78	1443.3	. 165	422.54	33.78							
99	19.18	3.222	3.171	34.50	1428.0	.162	418.74	34.50							
100	18.64	3.218	3.165	35.20	1411.4	.158	414.64	35.20							
101	18.12	3.214	3,159	35.86	1393.7	.154	410.23	35.86							
102	17.60	3.210	3.153	36.50	1374.8	.150	405.55	36.50							
103	17.11	3.206	3.146	37.14	1354.7	.146	400.58	37.14							
104	16.62	3.201	3.140	37.87	1333.5	.141	395.31	37.87							
105	16.14	3.196	3.133	38.53	1311.1	136	389.76	38.53							
106	15.68		3.125	39.12	1287.6	131	383.94	39.12							
107	15.22	3.185	3.118	39.67	1263.2	.125	377.88	39.67							
108	14.78	3.180	3.111	40.18	1237.8	.120	371.58	40.18							
109	14.34	3.174	3.104	40.66	1211.5	.114	365.05	40.66							
110	13.92	3.168	3.096	41.11	1184.3	108	358.31	41.11							
111	13.50	3.162	3.088	41.54	1156.2	.102	351.36	41.54							
112	13.10	3.155	3.081	41.94	1127.4	. 095	344.21	41.94							
113	12.62	3.149	3.073	42.31	1097.7	.089	336.84	42.31							
114	11.61	3.142	3.065	42.64	1066.7	.082	329.15	42.64							
115	9.65	3.134	3.056	42.90	1033.4	074	320.90	42.90							
116	7.14	3.126	3.048	43.12	997.5	.066	311.98	43.12							
117	5.04	3.118	3.038	43.38	959.1	058	302.48	43.38							
118	3.86	3.109	3.027	43.80	919.2	.049	292.57	43.80							
119	3.38	3.100	3.016	44 40	878.2	.040	282.40	44.40							
120	3.19	3.091	3.004	45.05	836.3	.031	272.01	45.05							
121	3.06	3.080	2.954	51.42	788.0	.020	260.02	51.42							
122	2.94	3.068	2,939	55.19	735.7	.008	247.06	55.19							
123	2.83	3.056	2.964	59.16	679.4	.000	233.10	59.16							
124	2.75	3.042	2.918	62.53	619.6	.000	218.27	62.53							
125	2.68	3.029	2.885	64.96	557.3	.000	202.83	64.96							
126	2.62	3.014	2.864	65.97	494.0	.000	187.12	65.97							
127	2.56	3.000	2.849	66.13	430.4	. 000	171.35	66.13							
128	2.50	2.932	2.847	62.02	370.9	.000	147.84	62.02							
129	2.44	2.877	2.870	50.27	323.1	.000	128.77	50.27							
130	2.38	2.826	2.812	45.45	280.0	.000	111.60	45.45							
131	2.32	2.776	2.758	45.35	236.9	.000	94.45	45.35							
132	2.26	2.727	2.707	44.33	194.9	.000	77.68	44.33							
133	2.19	2.680	2.659	42.74	154.3	.000	61.52	42.74							
134	2.13	2.635	2.610	40.44	. 116.0	.000	46.24	40.44							
135	2.00	2:594	2.574	37.15	80.9	.000	32.23	37.15							
136	1.69	2.557	2,523	33.29	49.3	.000	.19.64	33.29							
137	1.17	2.524	2.500	29.90	20.5	.000	8.19	29.90							
138	.60	2.504	2.500	17.32	3.8	. 000	1.52	17.32							
139	. 26	2.500	2.500	4.08	0	.000	.00	4.08		-					
140	.11	2.500	2.500	.11	0	.000	.00	. 14							
141	.04	2.500	2,500	.03	. 0	.000	.00	. 05							
142	.02	2.500	2.500	.03	0	000	.00	, 05							
143	.02	2.500	2.500	.02	, , 0	.000	.00	.03							
144	. 01	2,500	2.500	.01	. 0	000	.00	.02							
				and the second of the											

*****	********	******	*******	*******	**********	********	**********	***********
TIME	RUNOFF DISCHARGE	INSIDE W.L.	OUTSIDE W.L.	DRAINAGE		UNDATION	100	DISCHARGE of FACILITIES (1000m3)
(hr)	(1000m ³)	(EL m)	(EL.m)	(1000m ³)	(1000*cum) (m)	(ha)	4 5	or therefore (1000m°)
	*******			*******	********	*********	*	******************
1 2	.00 .01	3.198 3.224	3.322	-48.40 -26.37	288.3 .04		.00 48.40	
3		3.218	3.184	6.15	314.7 .07 308.6 .06		.00 26.37 14.03 7.88	
4	.11	3.194	3.134	24.21	284 5 .04		31.82 7.61	
5	.19	3.167	3.089	28.00	256.7 .01		37.35 9.35	
6	. 30	3.137	3.049	29.82	227.1 .00		40.11 10.29	
. 7	.43	3.108	3.021	29.66	197.9 .00		40.31 10.65	
: 8	.63 1.05	3.080	2 927	29.18	169.4 .000	_	39.63 10.45	
10	2.06	3.013	2.900	36.65 33.39	133.8 .000 102.4 .000		48.53 11:88	
11	3.48	2.922	2.841	30.75	75.2 .000		45.60 12.21 41.07 16.26	
12	4.66	2.808	2.771	25.00	54.8 .000		26.59 39.96	
13	5.40	2.738	2.708	17.84	42.4 .000		19.79 51.36	
14	5.90	2.684	2.655	15.60	32.7 .000		17.77 57.91	
15 16	6.25 6.48	2.636 2.593	2.606	14.76	24.2 .000		17.11 62.80	
17	6.64	2.574	2.556 2.500	14.16 9.98	16.5 .000 13.2 .000		16.66 66.92	
18	6.71	2.563	2 500	8.74	11.1 .000		12.62 69.67 11.52 70.92	
19	6.68	2.539	2.500	10.89	6.9 .000		13.78 72.07	
20	6.51	2.516	2.500	10.61	2.8 .000		13.62 74.23	
21	6.27	2.512	2.500	7.02	2.1 .000		10.13 75.19	
22	6.04	2.511	2.500	6.16	2 0 .000		9.38 75.34	$\mathcal{L}_{i}(\mathcal{L}_{i}) = \mathcal{L}_{i}(\mathcal{L}_{i}) + \mathcal{L}_{i}(\mathcal{L}_{i}) = \mathcal{L}_{i}(\mathcal{L}_{i})$ (4.1)
23 24	5.93 5.97	$2.511 \\ 2.512$	2.500	5.90	2.0 .000		9.28 75.36	
25	6.19	2.514	2.500 2.500	5.82 5.91	2.1 .000 2.4 .000		9.48 75.32	
26	6.56	2.516	2.500	6.16	2.8 .000		9.97 75.23 10.68 75.08	
27	7.05	2.519	2.500	6.54	3.3 .000		11.57 74.89	
28	7.61	2.522	2.500	6.98	4.0 .000		12.59 74.65	
29	8.28	2.527	2.500	7.49	4.8 .000		13.75 74.34	and the second second second second second
30 31	9.19 10.44	2.533 2.541	2.500 2.500	8.10	5.8 ,000		15.15 73.94	
32	12.45	2.555	2.500	8.91 10.03	7.4 .000 9.8 .000		16.90 73.37	
33	17.55	2.587	2.500	11.85	15.5 .000		19.20 72.53 22.99 70.85	
34	27.64	2.662	2.500	14.20	28.9 .000		29.84 66.63	
35	38.39	2.793	2.635	15.24	52.1 .000		37.75 56.89	and the second of the second o
36 37	44.40	2.970	2.859	12.84	83.6 .000		39.96 37.02	
38	45.71 44.38	3.018 3.047	2.937	22.12 15.74	107.2 .000		38.32 16.20	and the second of the second o
39	41.64	3.075	2.999	13.50	135.9 .000 164.0 .000		35.65 19.91 35.85 22.34	
40	38.16	3.098	3.001	14.40	187.8 .000		38.85 24.45	
41	34.52	3.117	3.004	16.05	206.2 .000		42.91 26.86	
42	31.28	3.131	3.008	16.34	221.2 .000		45.63 29.29	
43 44	28.62	3.144	3.014	16.00	233.8 .000		47.53 31.53	
45	26.32 24.27	3.155 3.165	3.023 3.031	15.04 13.94	245.1 .005		48.56 33.52	
46	22.57	3.176	3.040	12.88	255.4 .015 265.1 .025		49.22 35.27 49.72 36.84	
47	21.13	3.184	3.048	11.92	274.3 .034		50.16 38.23	
48	19.79	3.193	3.056	11.12	283.0 .043		50.54 39.43	
49	18.55	3.201	3.064	10.48	291.0 .051		50.89 40.41	
50	17.45	3.208	3.071	9.98	298.5 .058		51.19 41.21	
51 52	$16.52 \\ 15.73$	3.215 3.222	3.078 3.085	9.58	305.4 .066		51.44 41.86	
53	15.06	3.228	3.091	9.25 8.98	311.9 .072 318.0 .078		51.65 42.40	
54	14.49	3.233	3.097	8.73	323.8 .083		51.83 42.85 51.97 43.24	
55	14.00	3.239	3.103	8.51	329 3 089		52.09 43.58	
56	13.62	3.244	3.109	8.30	334.6 .094		52.19 43.90	
57	13.42	3.249	3.114	8,10	339.9 .099	100.34	52.29 44.19	
58 59	13.42 13.47	$3.255 \\ 3.261$	3.120	7.91	345.4 .105		52.41 44.49	
60	13.41	3.267	3.132	7.68 7.42	351.2 .111 357.2 .117		52.54 44.86	
61	13.23	3.273	3.138	7.21	357.2 .117 363.2 .123		52.70 45.28 52.87 45.66	
62	12.99	3.278	3.144	7.12	369.1 .128		53.03 45.91	
63	12.74	3.284	3.150	7.18	374.6 .134	109.25	53.18 46.01	
64	12.49	3.289	3,156	7.34	379.8 .139	110.57	53.29 45.95	
65 66	12.25 12.04	3,294 3,298	$3.161 \\ 3.167$	7.58 7.85	384.4 .144		53.34 45.76	
67	11.83	3.302	3.172	$7.85 \\ 8.12$	388.6 .148 392.3 .152		53.32 45.47	
68	11.63	3.305	3.177	8.38	395.6 .155		53.23 45.11 53.08 44.69	
69	11.43	3.308	3.181	8.61	398.4 .158		52.86 44.25	
70	11.23	3.310	3.186	8.81	400.8 .160	115.97	52.60 43.79	
$\begin{array}{c} 71 \\ 72 \end{array}$	11.04 10.83	3.312 3.314	3.190	8.97	402.9 .162	116.50	52.30 43.33	
	10.00	0.014	3.194	9.12	404.6 .164	116.94	51.97 42.84	

*****	*******	******	********		******		ATION	*****	****	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	
	RUNOFF	INSIDE		DRAINAGE	DOUBLOD			DDAIMAGE	DISCHARGE of	የ ልሮ፤፤ ፤ ተገ	res (1000m ³)
	DISCHARGE	W.L.	W.L.	DISCHARGE	PUNDAGE	/ml	AREA (ha)	4 5	DIOGRAMON OF		
(hr)	$(1000m^3)$	(EL.m)	(EL.m)) ("MUUUI) **********	(1000*cum)		(11CL) *******		********	******	*********
*****	10 03	3.315	3.197	9.27	406.0	.165	117.29	51.60 42.33			
73	10.63		3.201	9.50	406.9	.166	117.53	51.21 41.71			
74	10.43	3.316		9.90	407.2	.166	117.62	50.75 40.84			
75	10.25	3.316	3.204		407.1	.166	117.59	50.22 40.03			
76	10.08	3.316	3.206	10.19	406.6	.166	117.46	49.65 39.25			
77	9.92	3.316	3.209	10.40			117.26	49.05 38.49			
78	9.77	3.315	3.211	10.56	405.8	.165					
79	9.63	3.314	3.213	10.72	404.8	164	116.98	48.43 37.72			
80	9.49	3.312	3 214	10.90	403.3	.162	116.62	47.79 36.89			
81	9.33	3.311	3.215	11.12	401.5	. 161	116.16	47.12 36.00			
82	9.14	3.308	3.216	11.35	399.3	.158	115.59	46.42 35.07			
83	8.94	3.306	3.216	11.58	396.7	.156	114.91	45.69 34.12			
84	8.74	3.303	3.216	11.78	393.7	.153	114.13	44.94 33.16			
85	8.54	3.299	3.216	11.95	390.3	.149	113.26	44.18 32.22			
86	8.35	3.296	3.215	12.09	386.5	. 146	112.30	43.40 31.30			
87	8.17	3.292	3.214	12.19	382.5	.142	111.27	42.62 30.42			
88	7.99	3.288	3.212	12.24	378.2	. 138	110.18	41.84 29.60			
89	7.83	3.283	3.210	12.25	373.8	.133	109.04	41.10 28.85			
90	7.67	3.279	3.208	12.23	369.3	.129	107.87	40.38 28.15			
91	7.52	3.274	3.206	12.22	364.5	.124	106.67	39.70 27.48			
92	7.37	3.269	3.203	12.23	359.7	.119	105.42	39.07 26.84			
93	7.23	3.264	3.199	12.26	354.7	.114	104.13	38.47 26.21			
94	7.09	3.259	3.195	12.33	349.4	.109	102.79	37.93 25.60	•		
95	6.95	3.253	3.191	12 47	343.9	.103	101.37	37.46 24.99			
96	6.81	3.248	3.187	12.63	338.1	.098	99.88	37.02 24.39			
97	6.66	3.242	3.182	12.78	332.0	.092	98.31	36.58 23.80	•		
98	6.52	3.235	3.176	12.92	325.6	.085	96.67	36.12 23.21			
. 99	6.39	3.229	3.171	13.05	318.9	.079	94.96	35.66 22.62			
100	6.26	3.222	3 165	13.16	312.0	.072	93.19	35.19 22.02			
	6.14	3.215	3.159	13.27	304.9	.065	91.37	34.71 21.43			
101	6.03	3.207	3 153	13.37	297.5	.057	89.48	34.22 20.85			
102	5.92	3.200	3.146	13.48	290.0	.050	87.54	33.74 20.26			
103		3.192	3.140	13.66	282.1	.042	85.53	33.35 19.69			
104	5.81 5.70	3.184	3.133	13.78	274.1	.034	83.46	32.91 19.13			
105	5.60	3.176	3.125	13.84	265.8	.026	81.35	32.43 18.58			
106 107	5.50	3.167	3.118	13.87	257.4	.017	79.20	31.93 18.06			
108	5:41	3.159	3.111	13.85	249.0	.009	77.03	31.43 17.58			
109	5.31	3.151	3.104	13.81	240.5	.001	74.85	30.95 17.14			
110	5.23	3.142	3.096	13.76	232.0	.000	72.67	30.49 16.73			
111	5.14	3.134	3.088	13.72	223.4	.000	70.47	30.05 16.33			
112	5.06	3.125	3.081	13.68	214.8	.000	68.26	29.63 15.96			
113	4.98	3.116	3.073	13.67	206.1	.000	68.03	29.23 15.56			
114	4.91	3.107	3.065	13.90	. 197.1	.000	63.72	28.81 14.90			
115	4.83	3.098	3.056	14.79	187.1	.000	61.17	28.23 13.45			
116	4.66	3.086	3.048		175.6	.000	58.22	27.34 11.19			
117	4.21	3.074	3.038	16.78	163.1	.000	55.00	26.20 9.42			
118	3.55	3.061	3.027	16.30	150.3	.000	51.73	25.19 8.89			
119	3.04	3.048	3.016	15.76	137.6	.000	48.46	24.47 8.71			
120	2.83	3.036	3.004	15.40	125.0	.000	45.24	23.95 8.55			
121	2.79	3.018	2.954	20.66	. 107.2	000	40.66	29.85 9.19	9		
122	2.78	2.997	2.939	21.55	88.4	.000	35.76	32.00 10.45			
123	2.77	2.912	2.964	17.88	73.3	.000	29.64	24.55 22.32			
124	2.76	2.861	2.918	11.79	64.3	.000	25.99	15.34 36.29			
125	2.75	2.833	2.885	7.77	59.2	.000	23.96	11.22 42.17			
126	2.74	2.815	2.864	5.83	56.1	.000	22.70	9.18 45.15	•		
127	2.73	2.802	2.849	5,17	53.7	.000	21.72	8.43 47.08			
128	2.72	2.776	2.847	7.29	49.1	.000	19.87	10.47 49.15			
129	2.71	2.725	2.870	11.73	40.1	.000	16.21	14.84 53.27			
130	2.70	2.676	2.812	11.46	31.3	.000	12.67	14.49 58.88			
131	2.69	2.637	2.758	9.56	24.5	.000	9.89	12.52 63.03			
132	2.68	2.604	2.707	8.64	18.5	.000	7.48	11.53 66.27			
133	2.67	2.574	2.659	7.98	13.2	.000	5.33	10.79 68.99			
134	2.66	2.549	2.610	7.07	8.8	.000	3.55	9.74 71.29			
135	2.65	2.542	2.574	3.94	7.5	.000	3.03	6.13 72.42			
	2.63	2.520	2.523	6.55	3.6	.000	1.44	7.80 73.76			
136	2.63	2.501	2.500	5.93	.3	.000	.11	6.48 75.50			
137	2.60	2.501	2.500	2.68	.2	.000	08	3.04 76.10			
138	2.55	2.501	2.500	2.58	. 2	.000	07	2.78 76.11			
139			2.500	2.51	.1	.000	.06	2.57 76.12			
140	2.48	2,501	2.500	2.45	.1	.000	.05	2.45 76.13			
141	2 44	2.501	2.500	2.32	.1	.000	.04	2.32 76.13			
142	2.29	2.501	2.500	1.80	.ô	.000	.02	1.80 76.15			•
143	1.74	2.500 2.500		.70 .	.0	.000	.00	.72 76.18			
144	.66	2.000	2.000								

****	********	*******	******** *	*****	The second	10 to	- 10 miles
***	INUNDATION	ANALYSIS	BLOCK No.	6 ***	CAS	E	PLAN

RUNOFF INSIDE OUTSIDE W.L. DRAINAGE --INUNDATION--DISCHARGE PONDAGE DEPTH (1000m³) (1000*cum) (m) TIME DISCHARGE W.L. AREA (ha) ****** DRAINAGE DISCHARGE of FACILITIES $(1000m^3)$ (EL m) (EL.m) (hr) 5 6 ******* ***** ***** 3.316 3.198 .00 48.40 48.40 22.79 26.37 22.79 7.88 22.79 1 114.5 . 000 45.55 .00 3.224 26.37 88.1 .000 35.06 .01 3.222 3.201 3.218 7.87 7.59 80.3 .000 31.93 000 3.194 72.7 28.92 7.61 22.79 .03 3.175 3.167 9.32 63.4 9.35 22.79 .000 25.22 6 7 104 3.147 3.137 10.23 10.29 22.79 53.2 .000 21.17 3.118 42.7 .000 10.65 22.79 10.45 22.79 11.88 22.79 .06 3.108 10.56 16.99 .08 3.090 3.080 10.33 .000 12.91 q .13 3.058 3.045 11.69 .000 8.31 10 3.026 . 25 3.013 11:87 9.3 .000 3.69 12.21 22.79 2.922 . 44 3.000 9.68 .0 :000 16.26 22.79 .01 12 . 61 3.000 2.808 60 .000 .02 39.96 22.79 13 .71 3.000 2.738 .71 .78 . 0 .000 .02 51.36 22.79 2.684 3.000 . 0 .000 .02 57.91 22.79 15 . 84 3.000 2.636 .84 .000 .02 62.80 22.79 16 .90 3:000 2.593 .89 . 1 ann. 0.2 66.92 22.79 2.574 . 94 3.000 .94 .000 . 1 . 02 69.67 22.79 , 99 18 3.000 2.563 .99 .000 .02 70.92 22.79 19 1.03 72.07 22.79 3.000 2.539 1.03 .000 .03 3.000 1.07 2.516 1.07 .000 .03 1.11 21 3.000 2.512 1.11 .000 .03 75.19 22.79 75.34 22.79 75.36 22.79 3.000 2.511 1.14 -0000. 03 23 1.21 2.511 1 20 3.000 .000 . 1 .03 24 1.32 3.000 2.512 1.31 .000 .03 75.32 22.79 25 . 000 1.46 3.000 2.514 1.45 .04 75.23 22.79 2.516 1.63 3.000 1.62 75.08 22.79 74.89 22.79 .000 . 1 .04 27 1.81 3.000 2.519 1.80 .000 .05 28 2.01 3.000 2.522 2.00 . იიი .05 74.65 22.79 2.25 2.527 3.000 2.23 . 000 74.34 22.79 73.94 22.79 .06 30 3.000 2:533 2.51 .000 .06 31 2.87 3.001 2.541 2.84 .000 . 07 73.37 22.79 3.30 2.555 3.27 3.001 .000 72.53 22.79 . 08 3.98 5.77 33 3.001 2.587 3.92 .000 70.85 22.79 .11 34 2.662 5.64 8.27 3.001 ann. . 16 66.63 22.79 8.42 2.793 3.002 . 6 .000 .22 56.89 22.79 10.31 11.31 36 3,005 2.970 9.12 .000 .69 37.02 22.79 37 3.018 17.1 3.047 -4.05 .000 6.80 16.20 22.79 38 12.05 3.047 31.0 3.086 -1.83 .000 12.32 19,91 22,79 3.122 3.155 39 12.68 3,075 -.41 44.1 .000 17.53 22.34 22.82 13.24 3.098 1.24 40 56.1 .000 22.31 24.45 23.21 41 13.74 3.185 3.117 66.9 .000 2.94 26.60 26.86 23.92 3.131 42 14.21 3.211 4.55 76.5 .000 30.45 29.29 24.75 43 14.65 3.235 5.95 85.2 .000 33.91 31.53 25,59 15.06 3.257 3.155 7.12 93.2 .000 37.07 33.52 26.40 45 15.45 15.80 3.278 3.165 8.09 100.5 .000 40.00 35.27 27.18 3.297 46 42.74 45.28 8:92 107.4 .000 36.84 27.93 47 16.04 3.314 3.184 9.65 113.8 .000 38,23 28,59 48 16.13 3.330 3.193 10.35 119.6 .000 47.58 39.43 29.08 40.41 29.37 49 16.17 3.345 3.201 11.04 124.7 .000 49.62 16.20 3.357 3.208 11.67 129.3 . 000 51.42 41.21 29.53 41.86 29.64 12.22 16.24 3.368 3.215 53.02 133.3 .000 52 16.29 3.378 3.222 12.66 136.9 .000 42.40 54.46 29.73 3.228 16.33 3.387 13.03 140.2 .000 55.78 56.99 42.85 29.82 13.33 54 16.39 3.396 3.233 143.3 43.24 29.91 .000 16.45 3.239 55 3.404 13.58 146.1 .000 58.13 43.58 30.00 3.411 13.79 16.53 3.244 43.90 30.11 44.19 30.26 148.9 .000 59.22 3.249 3.255 151.6 57 16.65 3.419 13.94 .000 60.30 58 14.02 14.02 16.91 3.427 154.5 .000 44.49 30.48 17.22 3.436 3.261 157.7 .000 62.73 44.86 30.84 17.44 17.55 60 3.445 3.267 14.10 161.0 .000 64.05 45.28 31.18 164.1 61 3,453 3.273 14.45 65.29 45.66 .003 17.63 3.278 3.461 15.02 45.91 30.90 46.01 30.35 166.7 .011 66.32 17.69 17.75 63 3.466 3.284 15.66 168.8 .016 67.13 29.67 64 3.470 3.289 16.27 170.2 .020 67.72 45.95 17.80 3.473 16.81 65 3.294 171.2 45.76 28.94 45.47 28.20 .023 68.11 17.27 17.65 66 17.84 3.475 3.298 171.8 .025 68.33 17.89 67 3.475 3.302 172.0 .025 68.43 45.11 3.305 17.92 3.475 17,96 172.0 68 26.74 26.06 .025 68.42 44.69 69 17.96 3.474 3.308 18.19 171.8 .024 68.32 44.25 70 71 3.473 3.310 3.312 18.00 18.39 171.4 .023 68.17 43.79 25.41 18.56 18.02 170.8 .02267.95 43.33 24.77 3,314 67.67 170.1 .020 42.84 24.12

CASE PLAN

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	RUNOFF	INSIDE	OUTSIDE	DRAINAGE			WOITAC				. 1000 3
TIME	DISCHARGE	W.L.	W.L.	DISCHARGE	PONDAGE	DEPTH	AREA	DRAINAGE	DISCHARGE of	FACILITIE	ES (1000m³)
(hr)	(1000m³)	(EL.m)	(EL m)	$(1000m^3)$ ((ha)	5 6			
*****	******	******	******	*******	******		*******		*********	*****	*****
73	18.02	3.467	3.315	18.93	169.2	.017	67.31	42.33 23.41			
74	18.02	3.463	3.316	19.77	167.5	.013	66.62	41.71 22.83			
75	18.02	3.457	3.316	20.04	165.4	.007	65.81	40.84 22.79			
76	18.02	3.451	3.316	20.04	163.4	.001	65.01	40.03 22.79			
77	18.02	3.446	3.316	20.07	161.4	.000	64.20	39.25 22.79			
78	17.99	3.440	3.315	20.10	159.3	.000	63.36	38.49 22.79			
79	17.83	3.434	3.314	20.09	157.0	.000	62.46	37.72 22.79			
80	17.63	3.427	3.312	20.02	154.5	.000	61.47	36.89 22.79			
81	17.15	3.419	3.311	19.85	151.8	.000	60.39	36.00 22.79			
82	16.75	3.411	3.308	19.61	149.0	.000	59.25	35.07 22.79			
83	16.35	3.403	3.306	19.33	146.0	.000	58.07	34.12 22.79			
84	15.97	3.395	3.303	19.02	142.9	.000	56.85	33.16 22.79			
85	15.59	3.386	3.299	18.69	139.8	.000	55.62	32.22 22.79			
86	15.21	3.378	3.296	18.32	136.7	.000	54.38	31.30 22.79			
87	- 14.84	3.369	3.292	17.91	133.7	.000	53.16	30.42 22.79			
88	14.49	3.361	. 3.288	17.47	130.7	.000	51.98	29.60 22.79			
89	14.17	3.353	3.283	17.06	127.8	.000	50.83	28.85 22.79			
90	13.86	3.345	3.279	16.70	124.9	000	49.70	28.15 22.79			
91	13.56	3.337	3.274	16.37	122.1	.000	48.58	27.48 22.79			
92	13.26	3.330	3.269	16.06	119.3	.000	47.47	26.84 22.79			
93	12.96	3.322	3.264	15.78	116.5	.000	46.34	26.21 22.79			
94	12.64	3.314	3.259	15.50	113.6	.000	45.21	25.60 22.79			
95	12.31	3.306	3.253	15.23	110.7	.000	44.04	24.99 22.79			
96	11.97	3.298	3.248	14.97	107.7	.000	42.85	24.39 22.79			
97	11.64	3.289	3.242	14.72	104.6	.000	41.62	23.80 22.79			•
98	11.31	3.280	3.235	14.46	101.5	,000	40.37	23.21 22.79			
99	10.98	3.271	3.229	14.21	98.3	.000	39.08	22.62 22.79			
100	10.66	3.262	3.222	13.96	95.0	.000	37.77	22.02 22.79			
101	10.35	3.253	3.216	13.70	91.6	.000	36.44	21.43 22.79			
102	10.05	3.244	3.207	13.45	88.2	.000	35.09	20.85 22.79			
103	9.75	3.234	3.200	13.20	84.7	.000	33.71	20.26 22.79			•
104	9,46	3.224	3.192	12.97	81.2	.000	32.31	19.69 22.79			
105	9.18	3.215	3.184	12.75	77.7	.000	30.89	19.13 22.79			
	8.90	3.205	3.176	12.50	74 1	.000	29.46	18.58 22.79			
106 107	8.64	3.195	3.167	12.22	70.5	.000	28.04	18.06 22.79			
	8.38	3.185	3.159	11.92	67.0	.000	26.63	17.58 22.79			
108 109	8.13	3.175	3.161	11.64	63.4	.000	25.24	17.14 22.79			
110	7.89	3.166	3.142	11.38	60.0	.000	23.85	16.73 22.79			
111	7.66	3.156	3.134	11.14	56.5	.000	22.47	16.33 22.79			•
112	7.44	3.146	3.125	10.91	53.0	.000	21.09	15.96 22.79			
113	7.10	3.137	3.116	10.66	49.4	.000	19.67	15.56 22.79		*	
114	6.01	3.125	3.107	10.16	45.3	.000	18.02	14.90 22.79			
115	3.60	3.111	3.098	8.86	40.0	.000	15.93	13.45 22.79			
116	1.04	3.095	3.086	6.75	34.3	.000	13.65	11.19 22.79			
117	.00	3.081	3.074	5.13	29.2	.000	11.62	9.42 22.79			
118	.00	3.068	3.061	4.73	24.5	.000	9.73	8.89 22.79			
119	.00	3.055	3.048	4.66	19.8	.000	7.88	8.71 22.79			
120	.00	3.042	3.036	4.61	15.2	.000	6.06	8.55 22.79			
121	.00	3.027	3.018	5.34	9.9	.000	3.92	9.19 22.79			
122	.00	3.009	2.997	6.71	3.2	.000	1.25	10.45 22.79			
123	.00	3.000	2.912	3.03	1	.000	. 05	22.32 22.79			
124	.00	3.000	2.861	.00	ĩ	.000	.05	36.29 22.79			
125	.00	3.000	2.833	00	1	.000	.05	42.17 22.79			
126	, 00	3.000	2.815	.00	i	.000	. 04	45.15 22.79			
127	.00	3.000	2.802	.00	1	.000	. 04	47.08 22.79			
128	.00	3.000	2.776	.00	i	.000	.04	49.15 22.79			
129	.00	3.000	2.725	.00	î	.000	. 04	53.27 22.79			
	.00	3,000	2.676	.00	î	.000	.04	58.88 22.79			
130							.04	63.03 22.79			
131	.00	3.000	2.637 2.604	.00	. 1	.000	.04	66.27 22.79			
132		3.000	2.574	.00	. 1	.000	.04	68.99 22.79			
133	.00					.000	.03	71.29 22.79			
134	.00	3.000	2.549	.01	. 1	.000	.02	72.42 22.79			
135	.00	3.000	2.542	.03	1	.000	.01	73.76 22.79			
136	.00	3.000	2,520	04	0	.000	.01	75.50 22.79			•
137	.00	3.000	2.501	.01	0	,000	.00	76.10 22.79		* *	
138	,00	3.000	2.501	.01	0	.000	.00	76.10 22.79			
139	.00	3.000	2.501	.01	0	,000	.00	76.12 22.79			
140	.00	3.000	2.501	.00	0	.000	.00	76.13 22.79			
141	.00	3.000	2.501	.00	0	.000	.00	76.13 22.79			
142	.00	3.000	2.501 2.500	.00	.0	.000	.00	76.15 22.79			
143	.00			.00	.0	.000	.00	76.18 22.79			
144	.00	3.000	2.500	.00	. 0	. 500		.0.10 66.70			

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***	TAHIADATI	ON ANATVETE	RIDCY, No. 7 ***	

CASE PLA

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	RUNOFF	INSIDE	OUTSIDE	DRAINAGE		INUN	DATION	100					
TIME	DISCHARGE		W.L.	DISCHARGE	PONDAGE	DEPTH	AREA	DRA I	NAGE DI	SCHARGE 4	of FACIL	TIES (1000	/m³)
(hr)	(1000m³)	(EL.m)	(EL.m)	(1000m ³)	(1000*cuin) (m)·	: (ha) .	6				- 1 T	
	*******			*******	******	*****	*******	*****	*****	******	*******	********	****
1	.00	4.000	3.316	.00	. 0	.000	. 00	22.79			4.00		
2	.00	4.000	3.243	.00	.0	.000	.00	22.79					
3	01	4.000	3.222	.01	.0		.00	22.79			100	100	
4 .	.02	4.000	3.201	.02	. 0	000	.00	22.79	•	2.0			
5	.03	4.000	3.175	.03	.0	.000	.00	22.79		•		1.	
6	.06	4.000	3.147	.06	.0	000	.00	22.79	- "				
7		4.000	3.118	.08	.0	.000		22 79					
8	. 12	4.000	3.090	.12	. 0	.000	.00	22.79					2
9	. 19	4.000	3.058	. 19	.0	.000	.00	22.79				and the second	
10	. 34	4.000	3.026	.34	.0	.000	.00	22.79					
11	. 64	4.000	3.000	.64	.0	.000	00	22.79					
12	. 99	4.000	3.000	. 99	0	.000	. 00	22.79		i.		• •	
13	1.24	4.000	- 3.000	1.24	. 0	000	.00	22.7 9			2		
· 14	1.39	4.000	3.000	1.39	.0	000	00	22.79					
15	1.51	4.000	3.000	1.51	.0	.000	.00	22.79			٠,		
16	1.61	4.000	3.000	1.61	.0	000	.00	22.79			100	Annual Stage	
17	1.70	4.000	3.000	1.70	.0	000	00	22.79					
18	1.79	4 000	3.000	1.79	. 0	.000	.00	22.79					
19	1.87	4 000	3.000	1.87	.0	.000	.00	22.79			100		
20	1.94	4.000	3.000	1.94	.0	.000	00	22.79					
21	2.01	4.000	3.000	2.01	. 0	.000	.00	22.79		1.	•	:	
22	2.08	4.000	3.000	2.08	.0	.000	.00	22.79					
23	2.18	4.000	3.000	2.18	. 0	.000	00	22.79				e de la companya de	
24	2.35	4 000	3.000	2.35	.0	.000	.00	22.79			1. 1	\$ 100	
25	2.60	4.000	3.000	2.60	. 0	.000	.00	22.79				1.6	
26	2.90	4.000	3.000	2.90	.0		.00	22.79					
27	3.24	4.000	3.000	3.24	.0	.000	.00	22.79				No.	
28	3.61	4.000	3.000	3.61	. 0	.000	.00	22.79				the first of the second	
29	4.04	4.000	3.000	4.04	.0.	.000	.00	22.79			200		
30	4.54	4.000	3.000	4.54	. 0	.000	.00	22.79			100		
31	5.14	4.000	3,001	5.14	. 0	.000	.00	22.79		7.4			
32	5.91	4.000	3.001	5.91	. 0	.000	.00	22.79				•	
33	7.21	4.000	3.001	7.21	. 0	.000	.00	22.79	.*	5.5		4,	
34	10.00	4.000	3.001	10.00	. 0	.000	.00	22.79					
35	14.25	4.000	3.002	14.25	. 0	.000	.00	22.79	- 2				
36	18.00	4.000	3.005	18.00	.0	.000	.00	22.79				5.	
37	20.26	4.000	3.047	20.26	.0	.000	.00	22.79					
38	21.74	4.000	3.086	21.74	. 0	.000	.00	22.79					
39	22.95	4.007	3.122	22.75	. 2	.007	.09	22.82					
40	24.00	4.035	3.155	23.21	1.0	035	. 42	23.21		1			
41	24.95	4.072	3.185	23.92	2.0	.072	.86	23.92					
42	25.82	4.110	3.211	24.75	3.1	.110	1.32	24.75				*	
43	26.63	4.147	3.235	25,59	4.1	.147	1.77	25.59				4	
44	27.40	4.183	3.257	26.40	5.1	.183	2.20	26.40			+ * * + * *		
45	28.13	4.217	3.278	27.18	6.1	.217	2.60	27.18					
46	28.80	4.248	3.297	27.93	6.9	.248	2.98	27.93					100
47	29.30	4.273	3.314	28.59	7.7	.273	3.28	28.59		•	100	1.4	
48	29.54	4.290	3.330	29.08	8.1	290	3.48	29.08					
49	29.62	4.299	3.345	29.37	8.4	299	3.59	29.37			•		
50	29.65	4.304	3.357	29.53	8.5	. 304	3.65	29.53					
51	29.76	4.309	3.368	29.64	8.6	.309	3.70	29.64					
52	29.84	4.312	3.378	29.73	8.7	.312	3.75	29.73				100	
53	29.92	4.316	3.387	29.82	8.8	.316	3.79	29.82				<i>e</i>	
54	30.01	4.320	3.396	29.91	9.0	.320	3.84	29.91					
55	30.12	4 324	3.404	30.00	9.1	.324	3.89	30.00					
56	30.25	4.329	3.411	30.11	9.2	.329		30.11					
57	30.45	4.336	3.419	30.25	9.4	. 336	3.95		100				
58	30.84	4.349	3.427	30.48			4.04	30.25					
59	31.31	4.366			9.8	.349	4.19	30.48			•		
60	31.42	4.375	3.436 3.445	30.84	10.3	. 366	4.40	30.84					
61	31.02	4.368	3.453	31.18 31.21	$10.5 \\ 10.3$.375	4.50	31.18			*		
62	30,36					. 368	4.42	31.21			2.		
63	29.60	4.349	3.461	30.90	9.8	. 349	4.19	30.90				•	
64	28.82	4.322	3.466	30.35	9.0	. 322	3.87	30.35					
65		4.292	3.470	29.67	8.2	.292	3.50	29.67		٠.	100		
66	28.04	4.260	3.473	28.94	7.3	.260	3.12	28.94					
67	27.28 26.55	4.227	3.475	28.20	6.4	227	2.73	28.20					
		4,195	3.475	27.46	5.5	.195	2.34	27.46		•	٠		٠.
68 69	25.87	4.164	3.475	26.74	4.6	.164	1.97	26.74					- 1
70	25.23	4.134	3.474	26.06	3.8	134	1.61	26.06					
71	24.60 23.95	4 105	3.473	25.41	3.0	105	1 27	25.41				200	
72		4 076	3.472	24.77	2.1	.076	92	24.77					
, 4	23.24	4.045	3.470	24.12	1.3	.045	54	24.12					

CASE PLAN

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	RUNOFF	INSIDE	OUTSIDE	DRAINAGE			DATION		
TIME	DISCHARGE		W.L.	DISCHARGE	PONDAGE	DEPTH	AREA		INAGE DISCHARGE of FACILITIES (1000m3)
(hr)	$(1000m^3)$	(EL.m)	(EL.m)	$(1000m^3)$ (1000*cum) (m)	(ha)	6	
****	********	*****	*******	*********			*******		*************
73	22.45	4.011	3.467	23.41	. 3	.011	. 13	23.41	
74	21.63	4.000	3.463	21.93	0	.000	.00	22.83	
75	20.80	4.000	3.457	20.80	.0	.000	.00	22.79 22.79	•
76	19.98	4.000	3.451	19.98	.0	.000	.00		•
77	19.18	4.000	3.446	19.18	.0	.000	00	22.79	
78	18.39	4.000	3.440	18,39	. 0	.000	.00	22.79	
79	17.62	4.000	3.434	17.62	.0	.000	.00	22.79	
80	16.88	4.000	3.427	16.88	.0	.000	.00	22.79	
81	16.15	4.000	3.419	16.15	. 0	.000	.00	22.79	
82	15.45	4.000	3.411	15.45	.0	.000	00	22.79	
83	14.78	4.000	3.403	14.78	. 0	.000	.00	22.79	
84	14.14	4.000	3.395	14.14	. 0	.000	.00	22.79	
85	13.54	4.000	3.386	13.54	. 0	.000	.00	22.79	
86	12.98	4.000	3.378	12.98	.0	.000	.00	22.79	
87	12.52	4.000	3.369	12.52	.0	.000	.00	22.79	
88	12.13	4.000	3.361	12.13	.0	.000	.00	22.79	
89	11.79	4.000	3.353	11.79	.0	.000	.00	22.79	
90	11.45	4.000	3.345	11.45	. 0	.000	.00	22.79	i i
91	11.11	4.000	3.337	11.11	.0	.000	.00	22.79	
92	10.77	4.000	3.330	10.77	.0	.000	.00	22.79	
93	10.43	4.000	3.322	10.43	.0	.000	.00	22.79	
94	10.10	4.000	3.314	10.10	. 0	.000	.00	22.79	
95	9.76	4.000	3.306	9.76	. 0	.000	.00	22.79	
96	9.42	4 000	3.298	9.42	.0	.000	.00	22.79	
97	9.08	4.000	3.289	9.08	. 0	.000	.00	22.79	
98	8.74	4.000	3.280	8.74	.0	.000	.00	22.79	
99	8.41	4.000	3.271	8.41	. 0	.000	.00	22.79	•
100	8.07	4.000	3.262	8.07	. 0	.000	.00	22.79	
101	7.73	4.000	3.253	7.73	. 0	.000	.00	22.79	
102	7.39	4 000	3.244	7.39	. 0	.000	.00	22.79	
102	7.06	4 000	3.234	7.06	.0	.000	.00	22.79	
103	6.72	4.000	3.224	6.72	. 0	.000	.00	22.79	
105	6.39	4.000	3.215	6.39	. 0	.000	.00	22.79	
106	6.08	4.000	3.205	6.08	.0	.000	.00	22.79	·
	5.84	4.000	3.195	5.84	0	.000	.00	22.79	
107		4.000	3.185	5.66	. 0	.000	.00	22.79	
108	5.66	4.000	3.175	5.50	. o	.000	.00	22.79	
109	5.50		3.166	5.35	ŏ	.000	.00	22.79	
110	5.35	4.000	3.156	5.19	. 0	.000	.00	22.79	•
111	5.19	4.000		5.04	.0	.000	.00	22.79	
112	5.04	4.000	3.146 3.137	4.89	.0	.000	.00	22.79	
113	4.89	4.000	3.125	4.74	. 0	.000	.00	22.79	
114	4.74	4.000	3.111	4.59	. 0	.000	.00	22.79	
115	4.59	4.000		4.44	.0	.000	.00	22.79	
116	4.44	4.000	3.095	4.29	.0	.000	.00	22.79	· ·
117	4.29	4.000	3.081		.0	.000	, ŏŏ	22.79	
118	4.16	4.000	3.068	4.16 4.05	. 0	.000	.00	22.79	
119	4.05	4.000	3.055	3.94	.0	.000	.00	22.79	·
120	3.94	4.000	3.042		. 0	.000	.00	22.79	
121	3.84	4.000	3.027	3.84	.0	.000	.00	22.79	
122	3.74	4.000	3.009	3.74		.000	.00	22.79	
123	3.64	4.000	3.000	3.64	0	.000	.00	22.79	•
124	3.54	4.000	3.000	3.54	.0		.00	22.79	
125	3.44	4.000	3.000	3.44	.0	.000	.00	22.79	
126	3.35	4.000	3.000	3.35	. 0	.000	.00	22.79	
127	3.26	4,000	3.000	3.26	. 0	.000			
128	3.18	4.000	3.000	3.18	.0	.000	.00	22.79 22.79	
129	3.10	4.000	3.000	3.10	.0	.000	.00	22.79	
130	3.03	4.000	3.000	3.03	.0	.000	.00	22.79	
131		4.000	3.000	2.95	.0	.000	.00		
132	2.88	4.000	3.000	2.88	. 0	.000	.00	22.79	
133	2.81	4.000	3.000	2.81	. 0	.000	.00	22.79	
134	2.66	4.000	3.000	2.66	. 0	.000	.00	22.79	
135	2.16	4.000	3.000	2.16	.0	.000	.00	22.79	
136	1.21	4.000	3.000	1.21	.0	.000	.00	22.79	
137	.55	4.000	3.000	. 55	.0	.000	.00	22.79	
138	. 36	4.000	3.000		. 0	.000	.00	22.79	·
139	.20	4.000	3.000	. 20	.0	000	.00	22.79	
140	.06	4.000	3.000	.06	.0	.000	.00	22.79	
141	.00	4:000	3.000	. 00	.0	.000	.00	22.79	
142	đo.	4.000	3.000	.00	0	.000	.00	22.79	
143		4.000	3.000	.00	. 0	.000	.00	22.79	
144	.00	4.000	3.000	.00	. 0	.000	.00	22.79	

	*******	1.	*******	*******	******	*****	******	**************
	RUNOFF	INSIDE	OUTSIDE	DRAINAGE		I NUእ	WOITAD	
	DISCHARGE	W L	W.L.	DISCHARGE			AREA	DRAINAGE DISCHARGE of FACILITIES (1000m ³)
(hr)	(1000m ³ .)	(ELm)	(EL.m)	(1000m³) (1000*cum)	(m)	(ha)	. 7
******	*******	*******	*******	********	******	*****	*******	
1	. 00	4.574	3.322	8 25	44.0	. 000	15.29	8.25
2	.00	4.550	3.247	8.01	35.9	.000	12.85	8.01
3	.01	4.525	3.184	7.78	28.2	.000	10.49	7.78
4	01	4.502	3 134	7.56	20.6	.000	8.19	7.56
5 -	. 02	4.346	3.089	6.81	13.8	.000	5.53	6.81
- 6	.03	4.209	3.049	5.50	8.4	.000	3.35	5.50
7	.05	4.098	3.021	4.51	3.9	.000	1.56	4.51
8	.07	4.006	2.927	3.74	. 2	.000	.09	3.74
9	.12	4.000	2.900	. 35	. 0	.000	.00	3.37
10	.24	4.000	2.882	. 24	. 0	.000	.00	3.37
11	.38	4.000	2.841	38	. 0	.000	.00	3.37
12	.48	4.000	2.771	48	, 0	.000	.00	3.37
13	.54	4.000	2 708	. 54	. ŏ	.000	.00	3.37
14	.58	4.000	2.655	58	.ŏ	.000	.00	3.37
15	.62	4.000	2.606	.62	.0	.000	.00	3,37
16	.66	4.000	2.556	.66	.0		.00	
17	.69	4.000				.000		3.37
	.72		2.500	69	.0	.000	.00	3.37
18		4.000	2.500	.72	.0	.000	.00	3.37
19	.75	4.000	2.500	75	. 0	.000	.00	3.37
20	. 78	4.000	2.500	.78	. 0	.000	.00	3.37
21	. 81	4.000	2.500	.81	0	.000	.00	3.37
22	. 84	4.000	2.500	.84	. 0	.000	.00	3.37
23	.90	4.000	2.500	90	.0	.000	.00	3.37
24	1.00	4.000	2.500	1.00	. 0	.000	.00	3.37
25	1.11	4.000	2.500	1.11	. 0	.000		3.37
26	1.24	4.000	2.500	1.24	.0	.000	.00	3.37
27	1.38	4.000	2.500	1.38	.0	,000	.00	3.37
28	1.54	4.000	2.500	1.54	0	.000	.00	3.37
29	1.73	4.000	2.500	1.73	.0	.000	.00	3.37
30	1.95	4.000	2.500	1.95	.0	.000	.00	3.37
31	2.22	4.000	2.500	2.22	. 0	.000	.00	3.37
32	2.59	4.000	2.500	2.59	0	.000	.00	3.37
33	3.48	4.006	2.500	3.26	. 2	.000	.09	3.38
34	5.15	4.046	2.500	3.52	18	.000	.74	3.52
35	6.77	4.117	2.635	3.94	4.7	.000	1.87	3.94
36	7.74	4.197	2.859	4.52	7.9	.000	3.16	4.52
37	8.35	4.277	2.937	5.16	11 1	.000	4.44	5.16
38	8.84	4.353	2.973	5.81	14.1	.000	5.65	5.81
39	9.26	4.424	2.999	6.43	17.0		6.78	6.43
40	9.64	4.489	3.001	7.04	19.6	.000	7.82	7.04
41	9.99	4.507	3.004	7.45	22.1	.000	8.64	7.45
42	10.31	4.515	3.008	7.53	24.9	.000	9.48	7.53
43	10.61	4.524	3.014	7.62	27.9	.000	10.40	7.62
44	10.01	4.534	3.023	7.70				
	11.18				31.1	.000	11.37	7.70
45		4.545	3.031	7.80	34.5	.000	12.40	7.80
46	11.40	4.556	3.040	7.90	37.9	.000	13.46	7.90
47	11.51	4.567	3.048	8.00	41.5	.000	14.53	8.00
48	11.55	4.577	3.056	8.10	44.9	.000	15.58	8.10
49	11.58	4.588	3.064	8.20	48.3	.000	16.61	8.20
50	11.60	4.598	3.071	8.30	51.6	.000	17.61	8.30
51	11.63	4.608	3.078	8.40	54.8	.008	18.60	8.40
52	11.66	4.618	3.085	8.49	58.0	.018	19.56	8.49
53	11.70	4.628	3.091	8.58	61 1	.028	20.51	8.58
54	11.74	4.637	3.097	8.68	64.2	.037	21.44	8.68
55	11.79	4.647	3.103	8.77	67.2	.047	22.36	8.77
56	11.85	4.656	3.109	8.86	70.2	.056	23.27	8.86
57	11.91	4.665	3.114	8.95	73.1	.065	24.17	8.95
58	11.93	4.674	3.120	9.04	76 0	.074	25.05	9.04
59	11.82	4.682	3.126	9.12	78 7	.082	25.87	9.12
60	11.58	4.690	3.132	9.20	81.1	.090	26.60	9.20
61	11.29	4.696	3.138	9.27	83 1	.096	27.22	9.27
62	11.00	4.701	3.144	9.32	84 8	.101	27.73	9.32
63	10.70	4.705	3.150	9.37	86 1	.105	28.13	9.37
64	10.41	4.709	3.156	9.41	87.1	.109	28.43	9.41
65	10.13	4.711	3.161	9.43	87.8	.111	28.65	9.43
66	9.88	4.712	3.167	9.45	88 3	.112	28.78	9.45
67	9.63	4.713	3.172	9.46		.113	28.83	9.46
68	9.39	4.712	3.177	9.46	88.4	,112	28.81	9.46
69	9.12	4.711	3.181	9.45	88 0	.111	28.71	9.45
70	8.83	4.709	3.186	9.44	87 4	.109	28.52	9.43
71	8.51	4.707	3.190	9.42	86,5		28.25	
72	8.18	4.703	3.194	9.38	85.3	.107		9.42
	0.10	4.,00	0.104	0.00	00.0	.103	27.88	9.38

*** INUNDATION ANALYSIS BLOCK No. 8 *** CASE PLAN

****	********	******	OUTSIDE	********** DRAINAGE	********		AT'ION	*********	***********
TIME	RUNOFF DISCHARGE	INSIDE W.L.	W.L.	DISCHARGE	PONDAGE	DEPTH	AREA	DRAINAGE DI	SCHARGE of FACILITIES (1000m3)
(hr)		(EL.m)	(EL.m)	(1000m³) (1000*cum	(m)	(ha)	7	Schmids of Therefiles (1000m)
****	*****	*****		******					**********
73	7.85	4.698	3.197	9.34	83.8	.098	27.43	9.34	
74	7.52	4.693	3.201	9.29	82.1	.093	26.89	9.29	•
75	7.20	4.686	3.204	9.24	80.0	.086	26.27	9.24	
76	6.89	4.679	3.206	9.17	77.7	.079	25.57	9.17	'
77	6.59	4.672	3.209	9.10	75.2	.072	24.81	9.10	
78	6.29	4.663	3.211	9.02	72.5	.063	23.98	9.02	
79	6.01	4.654	3.213	8.93	69.6	. 054	23.09	8.93	
80	5.73	4.644	3.214	8.84	66.5	.044	22.14	8.84	
81	5.48	4.634	3.215	8.75	63.2	.034	21.15	8.75	
82	5.23	4 624	3.216	8.65	59.8	.024	20.11	8.65	
83 84	5.01	4.613	3.216 3.216	8.54 8.44	56.2 52.6	.013	19.03 17.93	8.54	
85	4.83 4.68	4.590	3.216	8.33	49.0	.000	16.82	8.44 8.33	
86	4.55	4.579	3.215	8.22	45.3	.000	15.70	8.22	
87	4.41	4.567	3.214	8.11	41.6	.000	14.58	8.11	
88		4.556	3.212	8.00	37.9	.000	13.44	8.00	
89	4.13	4.544	3.210	7.90	34.1	.000	12.29	7.90	
90	3.99	4.532	3.208	7.79	30.3	.000	11.14	7.79	
91	3.85	4.520	3.206	7.68	26.5	.000	9.97	7.68	
92	3.71	4.508	3.203	7.57	22.6	.000	8.80	7.57	
93	3.58	4.470	3.199	7.42	18.8	.000	7.52	7.42	
94	3.44	4.386	3.195	6.79	15.4	.000	6.18	6.79	
95	3.30	4.316	3.191	6.12	12.6	.000	5.05	6.12	
96	3.16	4.255	3.187	5.57	10.2	.000	4.09	5.57	
97	3.02	4.203	3.182	5.11	8.1	.000	3.25	5.11	
98	2.88	4.158	3.176	4.72	6.3	.000	2.52	4.72	
99	2.75	4.117	3.171	4.38	4.7	.000	1.87	4.38	
100	2.61	4.080	3.165	4.09	3.2	.000	1.27	4.09	
101	2.47	4.046	3.159	3.82	1.8	.000	.73	3.82	
102	2.35	4.015	3.153	3.59	.6	.000	. 24	3.59	
103	2.25	4.000	3.146	2.85	. 0	.000	.00	3.40	
104	2.19	4.000	3.140	2.19	.0	.000	,00	3.37	
105	2.12	4.000	3.133	2.12	.0	.000	.00	3.37	•
106 107	2.06	4.000	$3.125 \\ 3.118$	2.06 2.00	.0 .0	.000	.00	3.37 3.37	
108	1.94	4.000	3.111	1.94	.0	.000	.00	3.37	•
109	1.88	4.000	3.104	1.88	.0	.000	.00	3.37	•
110	1.81	4.000	3.096	1.81	.0	.000	.00	3.37	
111	1.75	4.000	3.088	1.75	.0	.000	.00	3.37	•
112	1.69	4.000	3.081	1.69	.0	.000	,00	3.37	
113	1.64	4.000	3.073	1.64	.0	.000	.00	3.37	
114	1.59	4.000	3.065	1.59	. 0	.000	.00	3.37	
115	1.54	4.000	3.056	1.54	. 0	.000	. 00	3.37	
116	1.50	4.000	3.048	1.50	.0	.000	.00	3.37	
117	1.46	4.000	3.038	1.46	. , 0	.000	.00	3.37	
118	1.42	4.000	3.027	1.42	.0	.000	.00	3.37	
119	1.38	4.000	3.016	1.38	. 0	.000	.00	3.37	
120	1.34	4.000	3.004	1.34	.0	.000	.00	3.37	
121	1.30	4.000	2.954	1.30	.0	.000	00	3.37	
122	1.26	4.000	2.939	1.26	.0	.000	.00	3.37	
123	1.23	4.000	2.964	1.23	.0	.000	.00	3.37	
124	1.20	4.000	2.918	1.20	.0	.000	.00	3.37	
125	1.17	4.000	2.885	1.17	.0	.000	.00	3.37	
126	1.14	4.000	2.864	1.14	0	.000	.00	3.37	
127	1.11	4.000	2.849 2.847	1.11	0	.000	. 00 . 00	3.37 3.37	
128	1.08	4.000	2.870	1.05	.0	.000	.00	3.37	
129 130	1.03	4.000	2.812	1.02	.0	.000	.00	3.37	
131	1.00	4.000	2.758	1.00	.0	.000	.00	3.37	
132	.98	4.000	2.707	.98	0	.000	.00	3.37	
133	. 95	4,000	2.659	.95	Ō	.000	.00	3.37	
134	.93	4.000	2.610	.93	. ŏ	.000	.00	3.37	
135	,91	4.000	2.574	.91	. 0	.000	.00	3.37	
136	.89	4.000	2.523	.89	.0	.000	.00	3.37	
137	.80	4.000	2.500	.80	.0	.000	.00	3.37	
138	.56	4.000	2.500	.56	.0	.000	.00	3.37	
139	. 25	4.000	2.500	.25	. 0	.000	.00	3.37	
140	. 05	4.000	2.500	.05	0	.000	.00	3.37	
141	.00	4.000	2.500	.00	. 0	.000	.00	3.37	
142	.00	4.000	2.500	.00	. 0	.000	.00	3.37	
143	.00	4.000	2.500	.00	.0	.000	.00	3.37	
144	.00	4.000	2.500	.00	, . 0	.000	.00	3.37	

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APPENDIX C

SOIL AND LAND USE

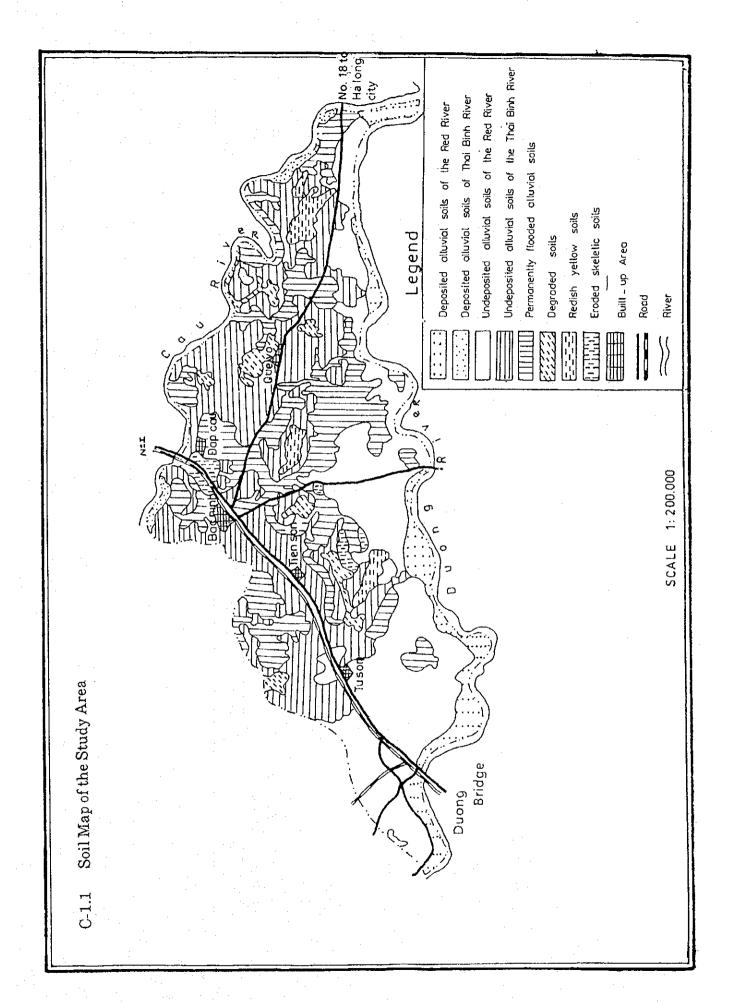
APPENDIX C

SOIL AND LAND USE

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C-1.2 Major Soil Unit in the Study Area (Source: NIAPP, Hanoi)

	7 17		Ha	Ha Bac		Ha	Hanoi
Soil Units	Area (na)	Tien Son	One No	Bac Ninh	Yen Phong	Gia Lam	Dong Anh
Fluvisols	7,126	2,920	1,096	398	500	1,848	655
Gleysols	13,710	5,158	6,567	754	465	989	80
Cambisols	5,998	1,380	3,978	420	220	1	1
Plinthosols	2,291	1,204	870	<i>L</i> 9	150	l	ı
Acrisols	930	l	930	I		1	1
Leptosols	816	678	12	126		 	
Sub-total	30,871	11,340	13,453	1,765	1,044	2,534	735
Residential area + Specialized uses	9,149	3,510	3,227	775	376	1,036	225
Total	40,020	14,850	16,680	2,540	1,420	3,570	960

C-1.3 Characteristics of Soil in the Study Area (Source: NIAPP - HANOI)

			Organic		Total (%)	Г	Available		Exchans	Exchangeable Cation	tion	CEC
Soil units	Depth	На	matter	Z	P205	K20	P2O5 K2O mg/100g soil.)	soil.)	(meq/10	(meq/100 g soil)		(meq/100
	(cm)	KCL	(%)			•	P205	K20	Cart	Mg++	H+	g soil.)
- Deposited alluvial soils of the Red river	0-18	7,1	1,2	0,12	0,1	1,86	29,2	35,1	14	8	0,05	23
(Gia lam district)	18 - 53	7,6	9,0	0,09	0,09	1,98	24,5	17,4	13	4	0,1	19
	53 - 85	7,8	0,4	0,00	0,09	2,07	22,7	15,7	18	4	0,1	26
	85 - 110	7,8	0,4	0,04	0,08	1,91	22,5	10	18	7	0,1	. 23
- Undeposited alluvial soils of the Red	0-19	7,5	1,	0,13	0,12	1,54	38,2	18,1	18	10	0,1	31
river (Gia lam district)	19 - 40	7,7	8,0	0,09	0,12	1,59	35,2	18,4	18	10	0,05	30
	40 - 51	7,7	9,0	0,06	0,1	1,86	32,7	10,9	18	11	0,16	30
	51 - 120	7,8	0,3	0,04	60,0	1,82	28,8	10,9	19	11	0,1	32
-Gleyic undeposited alluvial soils of the	0 - 20	4,7	2,7	0,2	90,0	2,24	18,8	∞	7	0,18	0,15	12
Red river (Gia lam district)	20 - 30	6,3	7	0,17	0,04	2,26	18,8	8,5		0,47	0,1	13
	30 - 80	5,7	1,4	0,13	0,03	2,31	4,8	11,5	10	1,23	0,1	13
	80 - 120	5,8	_	0,11	0,03	2,37	4,6	10,4	11	1,62	0,1	15
- Undeposited alluvial soils of the Red	0-20	4,8	1,1	0,12	0,03*	19,0	3,4*	13,2	7	96'0	0,16	9
river (Dong anh district)	20 - 37	5,4	0,2	0,03	0,02	0,64	1,2	6,1	æ	1,34	0,16	7
	37 - 105	3,4	0,1	0,03	0,02	1,42	6,0	5,7	m	1,44	6,05	19
- Permanently flooded alluvial soils of	0-20	6,5	1,9	0,19	90,0	1,4	frace *	10	φ.	2,6	0,56	
the Red river (Que vo district)	20 - 37	6,5	1,7	0,19	0,05	1,35	frace	10	10	1,2	0,56	
	37 - 100	4	1,7	0,25	0,03	0,67	trace	10	4	-	3,5	
- Undeposited alluvial soils of the Red	0-20	4,6	1,6	0,16			15,9	17,6				
river (Viet doan commune, Tien son												
district)				·								
- Undeposited alluvial soils of the Thai	0-20	4,3	2,4	0,21			7,5*	6,8				
binh river (Dai xuan commune,												
Que vo district)												

Note*: Very poor