

Fig. 2-16 Water Quality Change with Time in the two Model Rivers on Rainy Days

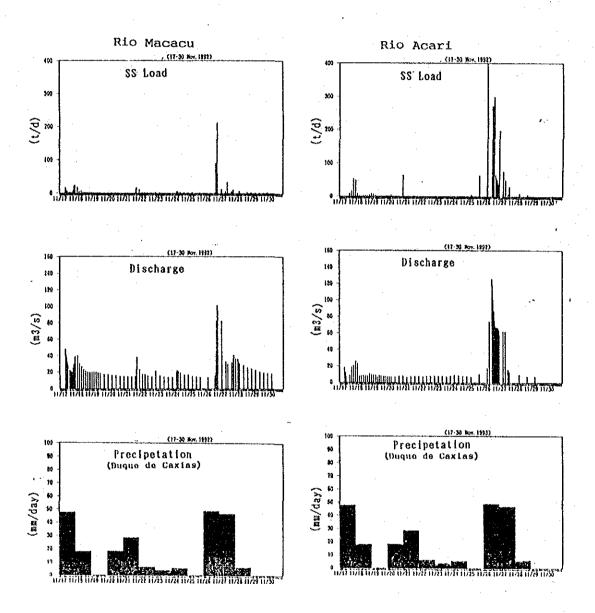


Fig: 2-17(1) Runoff Load Change with Time in the two Model Rivers in Freshet Time

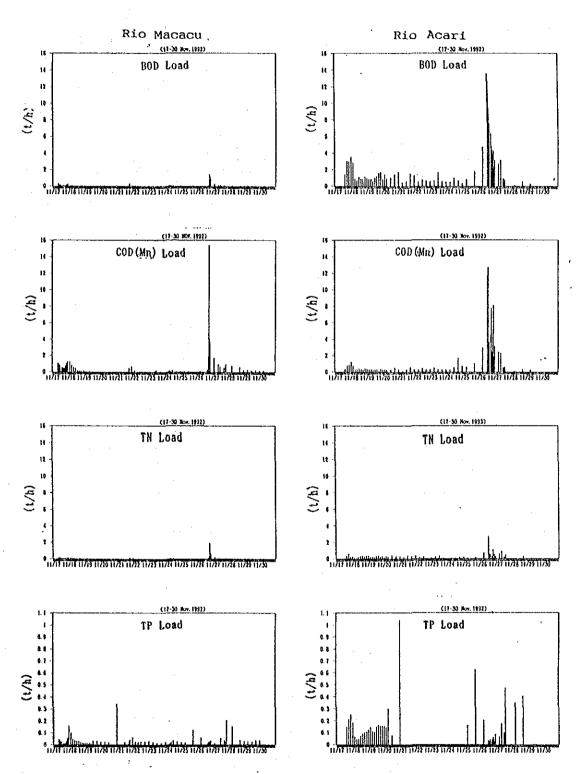
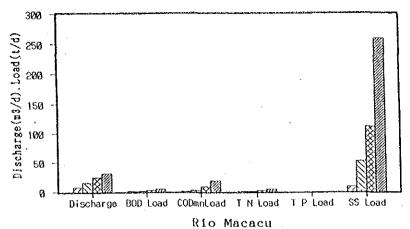
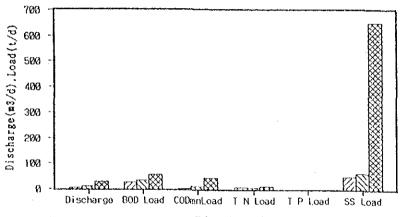


Fig. 2-17(2)Runoff Load Change with Time in the two Model Rivers in Freshet Time



☑ 0.2mm/day ☑ 14.28mm/day ☑ 16.75mm/day ☑ 24.88mm/day



Rio Acari ☑ 0.8mm/day ☑ 12.8mm/day ጪ 24.88mm/day

Fig. 2-18 Runoff Load Differences with Rain Intensity

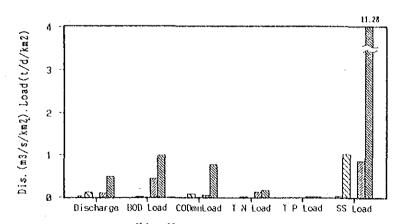


Fig.

Rio Macacu/Rio Acari

Rio Macacu(8.0mm/d) Rio Macacu(24mm/d) Rio Acari(8.0mm/d) Rio Acari(24mm/d)

2-19 Specific Runoff Load Differences with Rainfall
Intensity between the two Model Rivers

CHAPTER 3

ESTIMATE OF RUNOFF LOAD FROM THE BASIN

Chapter 3

Estimate of Runoff Load from the Basin

3.1 Need and Function for Estimation Model of Runoff Load

According to the observation data obtained in this survey, the discharge, water quality and runoff load characteristics of the main rivers in the basin were as described in the previous sections. However, in order to estimate the annual runoff load flowing into the bay from each sub-basin with accuracy, measurements should be carried out repeatedly under different conditions and a lengthy period and tremendous effort is needing in accumulating this data.

Accordingly, a runoff load estimation model including the various factors that restrict runoff load was formulated and designed to serve the following six purposes:

- (1) To estimate runoff load on rainy days (in the dry and rainy seasons),
- (2) To estimate the runoff discharge and runoff load of tidal rivers,
- (3) To estimate the runoff loads from uncovered areas of the observation station,
- (4) To estimate the future runoff loads according to changes in population,
- (5) To estimate the average runoff load over a long period of time,
- (6) To estimate an accurate runoff load with the least effort.

According to the pollution runoff mechanism chart (Fig.3-1), the generated pollution load from each source, and the runoff ratio estimated from the estimated effluent load and actual river runoff load will be used to estimate the runoff load in other basins. This method is called the generated pollution load method.

However, effluent load in the basin cannot be estimated due to insufficient point and non-point source data which is fundamental to such an estimation.

Accordingly, to estimate effluent load from both point and non-point sources, this report collected the basic data on runoff and water quality of model rivers in small basins, calculated their runoff load and used these as the generated pollution load for effluent loads of the larger basins.

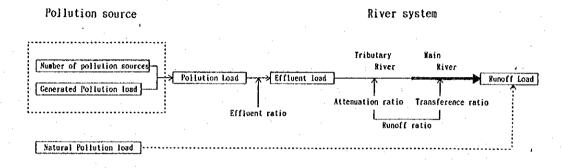


Fig. 3-1 Pollution Runoff Mechanism

3.2 Structure of the Estimation Model of Runoff Load

(1) Model Concept

The runoff load from the Guanabara Bay basin is known to be influenced by various factors. From the results of studies carried out on the aforementioned items, rainfall conditions, land use conditions, daily human activities, and industrial activities are the main influential factors.

As the population in the basin grows and the effects of human and industrial activities expand, the size of cities increase and land use conditions change. Therefore, land utilization, human and industrial activities are represented by population density.

Accordingly, runoff load was defined as a function of population density (Dp) and precipitation (Pr) in the basin, and the following equation was established:

L(Runoff load) = f(Dp, Pr)

(2) Estimation Model for Runoff Discharge

The estimation model comprises the runoff discharge model and runoff load model. The elements of each model and their relationship are represented as follows:

Runoff discharge (Q) = base runoff discharge (Qb) +
attained runoff volume of wastewater (Qw)
+ precipitation runoff discharge (Qp)

Runoff load (L) = runoff discharge (Q) x water quality (C) x runoff ratio (R) (Fig.3-2)

Runoff discharge on clear days is the value observed when the preceding period of clear days is five days or more. On the other hand, runoff discharge on rainy days is the value observed in other cases.

Runoff discharge on clear days (Qc) = Qb + Qw Runoff discharge on rainy days (Qr) = Qb + Qw + Qp (Fig. 3-3 and 3-4)

The basic runoff load in the rainy season is larger than that in the dry season because of rainfall. Therefore, calculations of runoff load should be carried out separately for the dry season and rainy season, using different basic runoff discharge values.

The descriptions of each element are as follows:

(1) Base runoff discharge (Qb)

A base runoff discharge is the constant discharge amount mainly originating from underground water. The base runoff discharge in the natural type river, Rio Macacu, was the lowest flow measured on consecutive clear days.

(2) Attained runoff volume of Waste water (Qw)

Attained runoff volume of waste water is defined, for convenience, as the wastewater amount from every point source reaching the observation stations. It is obtained by subtracting the basic runoff discharge amount (Qb) from the runoff discharge amount (Qc) on clear days.

In future, if the data given below has been obtained, the following equation can used to calculate attained runoff volume of wastewater (Qw):

- Qw = qi * a1 + qd * a2 + qt * a3 + q1 * a4 + qe * a5
 - qi: Runoff volume of wastewater per day from factories (If data is available from the factories, the value is used. In other cases, the calculation uses wastewater volume classified for industries.
 - qd: Runoff volume of domestic wastewater in areas without sewage works (population in areas without sewage works x discharge load per unit activity of source)
 - qt: Runoff volume of treated water discharge from existing sewage treatment plants

 (Calculated from measured values. Future values are estimated according to proposed sewage works.)
 - ql: Runoff volume of discharge by livestock (number of domestic animals per basin x generated pollution load per unit activity of source)
 - qe: Other runoff volume, that is, attained runoff volume of waste water = runoff volume from each source x runoff ratio (a1-5)

(3) Precipitation discharge amount (Qp)

The precipitation runoff discharge is the rain-affected amount of water discharged. It is precisely defined as the sum of the runoff discharge measured from the point where discharge increases after rainfall until the point where the runoff discharge returns to the normal level on a clear day. For convenience, the runoff discharge amount when the mean precipitation intensity exceeded 10mm/day was used.

The precipitation runoff discharge varies depending on the scale of rainfall, rainfall intensity, basin characteristics and number of preceding clear days; in actual estimation, relation of these elements to precipitation runoff discharge should be thoroughly analyzed.

In this survey, rainfall amount and the runoff discharges of the two model rivers (natural type and urban type) were used to analyze the relationship between rainfall intensity and precipitation runoff discharge. Precipitation intensity was classified by a notch of 10mm. Runoff discharge largely varies depending on rainfall intensity even if the volumes precipitated are the same. This factor was not represented in this model.

Runoff discharge differs as precipitation varies by area. Originally, the precipitation amount to be used for the model should be the amounts measured at several stations in consideration of rainfall distribution. However, such data was not obtained. The study was, therefore, left with no choice but to use the precipitation data obtained at only one observation station in Duque de Caxias, namely Petrobas, to estimate the precipitation runoff load.

(4) Estimation Model for Runoff Load

Runoff load is represented by the following equation:

Runoff load (L) = base runoff load (Lb) + attained runoff load of waste water (Lw) + precipitation runoff load (Lp)

- (a) Base runoff load (Lb) = load derived from said base runoff discharge
- (b) Attained runoff load of waste water (Lw) = Runoff load in clear days minus base runoff load; this value is equivalent to point source runoff load on clear days.
- (c) Precipitation runoff load (Lp) = surface runoff load according to rainfall, precipitation runoff load when rainfall intensity is 10mm/day or more.

Runoff load is obtained by using the empirical equation to represent relation between runoff discharge and runoff load. Runoff load varies to a great extent depending on the number of preceding clear days and rainfall intensity. This factor is not represented in this model because of deficiencies in the data.

(d) Runoff ratio

The process of the pollutants being discharged from their source and flowing into a river is defined as attenuation, the linear process of flowing downstream as transference, and the whole flow process from the source to the observation station as runoff.

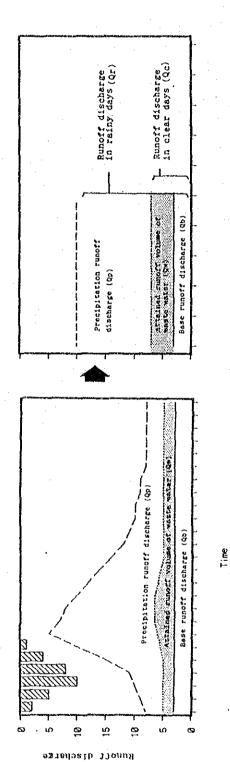


Fig. 3-2 Schematic Hydrograph and Constitution of Discharge

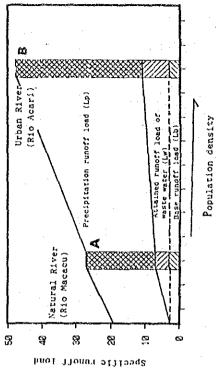


Fig. 3-4 Runoff Load Constitution of Natural Type . and Urban Type Rivers

Rainy d (25mm/d)

Attained runoff load of waste water (LW) Base runoff load (Lb)

Precipitation runoff load (Lp) Runoff Load Differences between Clear Days

and Rainy Days

3-3

Fig.

Rainy d (15mm/d)

Clear d (Grand)

8

X

8

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Specific runoff lond

63

Thus runoff ratio is the product of the attenuation ratio and transference ratio. Runoff ratio is the ratio of pollution load that reaches a reference point to all the total pollution load discharged in the basin.

Runoff ratio is influenced by the size of the basin, river bed conditions, runoff time and discharge. Of these, discharge most controls the runoff ratio thus influencing the ratio largely between clear days and rainy days. Survey results in Japan report that BOD runoff ratio is directly proportional to population density/(basin area)^{1/2}.

Here the relationship between the two is obtained assuming X=log(runoff ratio, %) and Y=log (population density/(basin acreage)^{1/2}) to calculate the discharge and runoff rate of BOD and COD(Mn). T-N, T-P and SS are assumed to be as soluble and runoff ratio is calculated by using the same equation as that for discharge.

Runoff ratio is calculated by using the observation data of a group of rivers. Runoff ratio is shown in Fig.3-5.

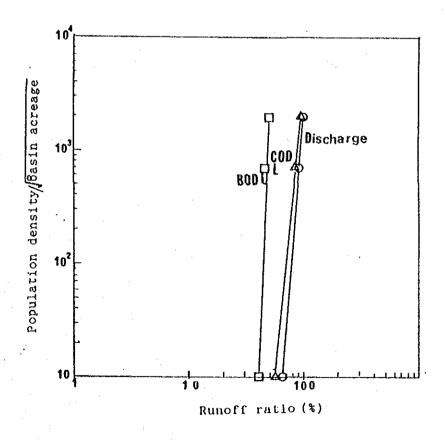


Fig. 3-5 Relationship between Runoff Ratio and Population Density/Basin Area

3.3 Procedure for Calculation of Runoff Discharge and Runoff Load

The annual runoff load was calculated in accordance with the operation flow chart (Fig.3-6).

Annual runoff load = runoff load on clear days + runoff load on rainy days

= runoff load in the dry season + runoff load in the rainy season.

The specific runoff volume and specific runoff load have a linear relation on log-log diagram for suspension solids, abundant at the initial stages of rainfall.

Therefore the runoff load; for water quality parameters with high runoff ratios in the initial stages (e.g., BOD, COD, TN, TP and SS) and are discharged as suspended solids, were calculated using the regression model.

Further, the specific load of each river was determined using population density; which strongly correlates to basin land utilization and generation load-factors that largely influence specific load; as a parameter.

Runoff load on clear and rainy days was calculated using the Separation Method 1, shown in Fig. 3-7.

Assumptions for the calculation of runoff load on clear days and runoff load on rainy days, to be carried out separately, are described below.

- (1) Runoff load on clear days = base runoff load + attained runoff load Base runoff load (discharge) = minimum value over 24-hour continuous observation (runoff load)
- (2) Calculation of the runoff ratio on clear days (re) Runoff ratio = runoff load (measured value)/effluent load(estimated value) Runoff ratio of each basin was calculated from population density, basin area and the measured runoff ratio of the model rivers.

Runoff ratio on rainy days was obtained from the precipitation per day and specific runoff discharge per day.

- (3) Runoff load per day on clear days = specific runoff load x basin area.
- (4) Calculation of rainy days by rainfall scales
 Annual precipitation is arranged as precipitation per one
 continuous rainfall and classified in scales of 10mm to
 calculate rainy days by months.
- (5) Calculation of specific runoff load of each basin by rainfall graphs. Rainfall exceeding 10mm is classified into scales of 10mm. The runoff load for the mean precipitation of rainfall scales was obtained using the regression model; then the value was multiplied by the number of rainfalls (number of rainy days) in order to calculate the runoff load of each rainfall scale.
- (6) Calculation of specific runoff load of each basin on rainy days
- (7) Runoff load per day on rainy days = specific runoff load per day x basin area.
- (8) Runoff load per month = runoff load on rainy days in each month + runoff load on clear days in each by month
- (9) Annual runoff load = runoff load in the dry season + runoff load in the rainy season
- (10) Runoff loads of unsurveyed areas, downstream of observation stations, were calculated for each basin assuming that the basins are homogeneous.

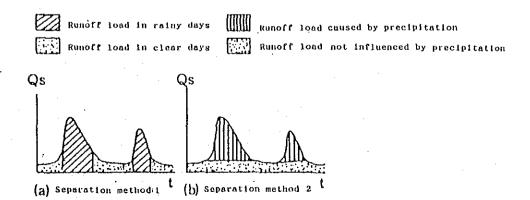


Fig. 3-7 Concept of Separation Methods

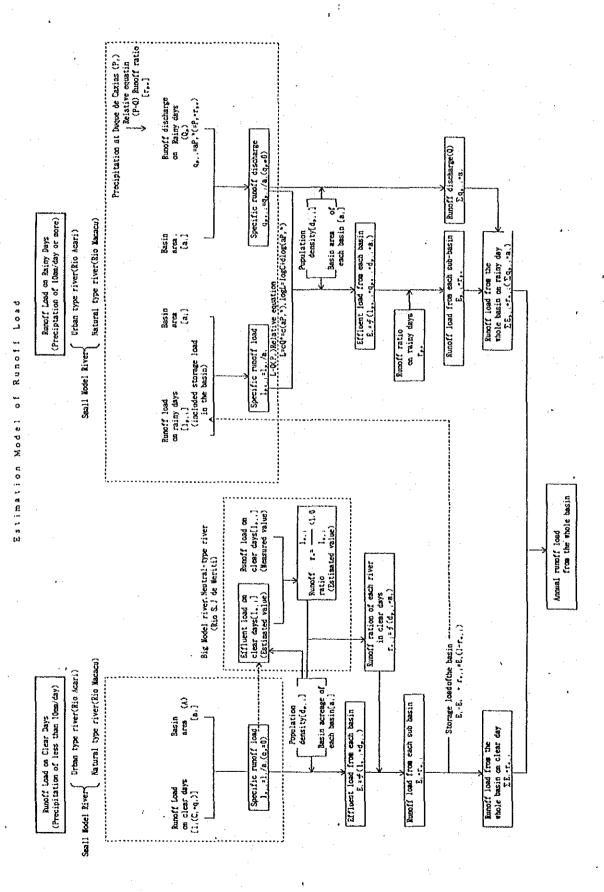


Fig. 3-8 Procedure for Calculation of Annual Runoff Load

Here each element is defined as follows:

- -Runoff load on clear days: Runoff load when precipitation per day is less than 10mm, or mean precipitation per day during the runoff period (one rainfall divided by runoff days) is less than 10mm even if precipitation per day is 10mm or more.
- -Runoff load on rainy days: Runoff load when precipitation per day during the runoff period is 10mm or more.
- -Number of rainy runoff days: Number of runoff days corresponding to one rainfall.
- -Number of clear runoff days: Number of days per year number of rainy runoff days.
- -Dry season: Period when precipitation is relatively small in a year. April to September for 1992.
- -Rainy season: Period when precipitation is relatively large in a year. October to March for 1992.

3.4 Setting of Parameters

(1) Runoff ratio (re)

The relationship between runoff ratio on clear days (X) and population density/(basin acreage) $^{1/2}$ (Y) can be calculated using the following equation: Y = $a*X^b$ (see Fig.3-5). Runoff ratio on rainy days is assumed to be 1.0, because it is included in the relationship between precipitation and runoff discharge.

Indicator	Equation	Coefficient of correlation
Discharge (TN, TP, SS) BOD	Y=3.382*X ^{1.4.73} Y=4*10^9*X ^{28.78}	0.995 0.999
CDD (Mn)	$Y=5.70*X^{11.97}$	0.995

(2) Runoff discharge (mean discharge)(Q)

The relationship between discharge (Q) and precipitation (mean precipitation (Pr)) in the model rivers can be obtained using the following equation:

$$Q = aPr^b$$

River Name	Equation	Coefficient of correlation
Rio Macacu	Q=0.00106R ^{1.625}	0.967
Rio Acari	Q=0.00279R ^{1.626}	0.991

(3) Runoff load (L)

The relationship between runoff discharge (Q) and runoff load(L) can be determined through the following equation:

Empirical equation: $L = cQ^{d}$

Therefore,

 $logL = log c+d*log(aPr^b)$

Indicator	Equation	Coefficient of correlation	Equation	Coefficient of correlation
BOD Load COD(Mn) Load TN Load TP Load SS Load	L= 0.045Q°. L= 10.998Q². L= 0.328Q¹. L= 0.040Q¹. L=190.957Q².	497 0.975 497 0.986 832 0.934	L= 1.463Q°.646 L= 3.170Q¹.647 L= 0.192Q°.266 L= 0.011Q°°.266 L=37.200Q¹.813	0.993 0.861 93 0.996

^{*} L:t/d/km², Q:m³/s/km²

(4) specific discharge (Qs) and specific runoff load (Ls) by Population Density (D)

The relationship between population density (Dp) and specific runoff load (L) can be obtained using the following equations:

Ls (Qs)
$$=$$
 e*Dp+f

Dp : population density (people/km2)

e,f: coefficients

Mean rainfall in runoff period; <10mm/day, 10-20mm/day, 20-30mm/day, 30-40mm/day

Established precipitation: One/day, USan/day, 25mm/day,

Indicator	Precipitation	Instituted Precipitati	on Dry Season	Rainy Season
Discharge	0-10am/day	<10mm/day	Qs=0.0105D+0.0251	Qs=0.0101D+0.0305
~	10-20mm/day	i5mm/day	Qs=0.0243D+0.0444	Qs=0.0239D+0.0471
	20-30mm/day	25mm/day	Qs=0.0584D+0.0817	Qs=0.0580D+0.0845
	30-40mm/day	35mm/day	Qs=0.1027D+0.1278	Qs=0.1023D+0.1306
BOD Load	0-10mm/day	<10mm/day	Ls=0.0641D+0.0022	Ls=0.0582D:0.0035
	10-20mm/day	15mm/day	l.s=0.0865D+0.0035	Ls=0.0865D+0.0040
	20-30mm/day	25mm/day	Ls*0.1357D+0.0044	Ls=0.1356D+0.0049
	30-40mm/day	35mm/day	Ls=0.1824D+0.0052	Ls=0.1823D+0.0057
COD(Mn) Load	0-10mm/day	<10mm/day	Ls=0.0072D+0.0012	Ls=0.0057D+0.0026
	10-20mm/day	15mm/day	Ls=0.0264D+0.0066	Ls=0.0264D+0.0073
	20-30mm/day	25mm/day	Ls*0.1209D+0.0432	Ls=0.1209D+0.0432
	30-40 ss/d ay	35mm/day	Ls=0.32750+0.1537	Ls=0.3274D+0.1543
TN Load	0-10mm/day	<10mm/day	Ls=0.0123D+0.0010	La=0.0152D+0.0010
	10-20mm/day	15mm/day	Ls=0.0167D+0.0028	Ls=0.0167D+0.0028
2	20-30mm/day	25mm/day	Ls=0.0202D+0.0088	Ls*0.02020*0.0088
•	30-40mm/day	35mm/day	Ls*0.0222D*0.0190	Ls*0.0222D+0.0190
TP Load	0-10mm/day	<10mm/day	Ls=0.0036D+0.0000	Ls=0.0026D+0.0000
	10-20mm/day	15mm/day	1.s=0.0021D+0.0002	Ls=0.0021D+0.0002
	20-30mm/day	25mm/day	La=0.0016D+0.0008	Ls=0.0016D+0.0008
÷	30-40mm/day	35mm/day	Ls=0.0012D+0.0019	LsO.0012D+0.0019
SS Load	0-10am/day	<10mm/day	l.s=0.0470D+0.0251	f.s=0.1107D+0.0231
	10-20mm/day	15ss/day	Ls=0.3241D+0.1026	Ls*0.3242D+0.1020
	20-30mm/day	25mm/day	Ls=1.4298D+0.6874	Ls=1.4298D+0.6868
	30-40am/day	35œm/day	Ls=3.7660D+2.5073	Ls=3.7661D+2.5067

- (5) Specific runoff load (Ls)
- (a) Base runoff (Qb) and base load (Lb)
 Base runoff and base load were obtained from the data
 (minimum runoff discharge) of the 24-hour observation conducted on the natural type river, Rio Macacu.

Discharge	BOD Load	COD(Mn) Load	TN Load	TP Load	SS Load
(m³/s/km²)	(t/d/km²)	(t/d/km²)	(t/d/km²)	(t/d/km²)	(t/d/km²)
0.031	0.005	0.002	0.001	0.000	0.020

(b) Specific runoff discharge per day (Qs) and specific runoff load per day (Ls) on clear days
Specific runoff discharge per day and specific runoff load per day were obtained from the data of the 24-hour observation conducted in Rio Macacu and Rio Acari on clear days.

River Name	Discharge	BOD Load	COD(Mn) Load	TN Load	TP Load	SS Load
	(m³/s/km²)	(t/d/km²)	t/d/km²)	(t/d/km²)	(t/d/km²)	(t/d/km²)
Rio Macacu Rio Acari	0.032	0.006 0.444	0.003 0.046	0.002 0.116	0.000 0.020	0.034

(c) Runoff discharge (Qp) and runoff load (Pr) on rainy days
The runoff discharge and runoff amount on rainy days were
obtained from the data of the observations conducted on the
two model rivers, on rainy days.

Natural type river (Rio Macacu)

Precipitation (mm/day)	Discharge (m³/s/km²)	BOD Load (t/d/km²)	COD(Mn) Load (t/d/km²)	TN Load (t/d/km²)	TP Load (t/d/km²)	SS Load (t/d/km²)
14.28	0.063	0.005	0.008	0.005	0.0005	0.203
16.75	0.094	0.008	0.035	0.008	0.001	0.432
24.08	0.119	0.016	0.074	0.016	0.001	1.014

Urban type river (Rio Acari)

Precipitation am/day	Discharge	BOD Load	COD(Mn) Load	TN Load	TP Load	SS Load
	(m³/s/km²)	(t/d/km²)	(t/d/km²)	(t/d/km²)	(t/d/km²)	(t/d/km²)
12.00	0.177	0.567	0.157	0.106	0.018	1.089
24.08	0.481	0.990	0.772	0.165	0.013	11.264

(6) Precipitation (Pr) (Data obtained from the Petrobras observation station at Duque de Caxias, 1992)

Mean precipitation in the runoff period was classified by rainfall scales, and number of rainy days by rainfall scales was obtained (see Table 3-1).

(7) Basin area (A) and Population density (D)

Basin area covered by observation stations, uncovered area, whole basin area and population density are shown in Table 3-2 and 3-3.

Table 3-1 Rainy Days during the Survey Period by Rainfall Scales (1992)

	Rain;	y seaso	n			Dry sea	son			Rain	y seaso	n
Precipetation	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP	OCT	YOY	DEC
<10mm/d	19	26	27	27	29	30	29	30	28	23	15	28
10-20mm/d	4	0	3	0	2	0	2	0	0	5	5	0
20-30mm/d	8	3	1	3	0	0	0	0	0	3	î	3
30mm< /d	0	0	. 0	0	0	0	0	0	3	0	3	0
	31	29	31	30	31	30	31	30	31	31	30	31

Table 3-2 Details of Basin Areas of the 25 Major Rivers Surveyed

NO NAME Basin Area Basin Basin Basin Basin Basin Area Basin Basin Basin Basin Basin Basin Basin			Racin Area	Racin Area	Covered	Uncovered	Population	Population
CANAL CANTO DO RIO CANAL DO RIO BMBA CANAL CANTO BMBA CANTO BMBA CANAL CA	NO	NAMP						
1 C1780	""	илми						
2 BM760 R10 BOMBA 5 26.20 3.40 22.80 183,099 6.99 3 1B810 R10 IMBOASSU 6 30.80 11.60 19.20 138,636 4.50 4 AM740 R10 ALCANTARA 8 144.60 58.50 68.80 470,420 3.25 5 MT820 R10 MUTONDO 8 5 5.50 6 GX720 R10 GUAXINDIBA 8 11.80 7 7 CC622 R10 CACEREBU 9 846.70 758.40 88.30 336,193 0.40 8 GP600 R10 GUAPIMIRIM 10 1253.10 1233.70 19.40 69,853 0.06 49 MC967 R10 MACACU 10-3 256.00 256.00 0.00 18.577 0.07 410 SB998 R10 SOBERBO 10-6 132.40 45.20 87.20 17.911 0.14 11 MG580 CANAL DE MAGE 11 18.30 4.60 13.70 8.458 0.46 12 RN560 R10 RONCADOR 12 111.40 107.00 4.40 36,370 0.33 13 1R540 R10 IRIRI 13 27.80 8.40 19.40 10.684 0.38 14 SR500 R10 SURVI 14 68.80 53.20 15.60 12.910 0.19 15 ES400 R10 ESTRELA 16 342.50 342.50 0.00 302,495 0.88 416 IN460 R10 INHOMIRIN 16-2 139.00 139.00 0.00 84.106 0.61 417 SC420 R10 SARACURUNA 16-3 186.00 186.00 0.00 194.173 1.04 18 1A260 R10 ICVACU 17-15 562.80 544.20 18.60 758,010 1.35 19 SP300 R10 SARACURUNA 16-3 186.00 186.00 0.00 194.173 1.04 18 1A260 R10 ICVACU 17-15 562.80 544.20 18.60 758,010 1.35 19 SP300 R10 SARACURUNA 16-3 186.00 186.00 0.00 194.173 1.04 18 1A260 R10 ICVACU 17-15 562.80 544.20 18.60 758,010 1.35 19 SP300 R10 SARACURUNA 16-3 186.00 186.00 0.00 194.173 1.04 18 1A260 R10 ICVACU 17-15 562.80 544.20 18.60 758,010 1.35 19 SP300 R10 SARACURUNA 16-3 186.00 186.00 0.00 194.173 1.04 20 SJ220 R10 S.J. DE MERITI 19 164.50 163.50 1.00 1.492.458 9.07 421 AC241 R10 ACARI 19-2 57.90 57.90 0.00 438.076 7.57 22 1J200 R10 IRAJA 20 35.70 27.30 8.40 500.276 14.01 23 PN180 CANAL DO PENHA 20 0.00 24 CN100 CANAL DO PENHA 20 0.00 24 CN100 CANAL DO PENHA 20 0.00 25 MN000 CANAL DO MANGUE 23 42.80 42.80 0.00 500.876 11.70 101AL 3912.50 3604.10 308.40 8.690,147 1.709.94	1 01780	CANAL CANTO DO RIO	7					5 64
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#:fributary river (Excluded from Total amount)	:Tributary river	(Excluded from Total	anount)	45.0.00			10,000,171	1

Table 3-3 Area, Population and Population Density by Sub-Basin

	Basin Area	Basin Area	Covered	Uncovered	Соуегед	Population	Population
Name	ł	(A) = (B) + (C)	Basin Area	Basin Area	Ratio(X)	1	Density
	NO.	(A)km2	(B) km2	(C) km2	(B/A+100)		(+10 3/km2
BCHARITAS	1	9, 40	0.00	9.40	0	53, 310	5. 67
CANAL CANTO DO RIO	2	7.40	7.40	0.00	100	41,745	5.64
BCATEDRAR	3	7.80	0.00	7.80	0	37, 458	4.80
BNORTE CENTRO	ŧ	7, 90	0.00	7,90	0	43,607	5.52
RIO BOWBA	5	26.20	3.40	22.80	13	183,099	6.99
RIO IMBOASSU	6	30.80	11.60	19.20	38	138,636	4.50
B ITAOCA	7	6.40	0.00	6.40	0	31,925	4.99
RIO ALCANTARA	8	144.60	75. 80	68.80	52	470, 420	3. 25
RIO CACEREBU	. 9	846.70	758.40	88, 30	90	336, 193	0.40
RIO GUAPINIRIN	10	1253.10	1233.70	19.40	98	69.853	0.0
CANAL DE MAGE	11	18.30	4.60	13.70	25	8,458	0.40
RIO RONCADOR	12	111.40	107.00	4.40	96	36, 370	0.3
RIO IRIRI	13	27.80	8.40	19.40	30	10,684	0.3
RIO SURUI	14	68.80	53. 20	15. 50	77	12,910	0.1
BMAUA	15	28.90	0.00	28.90	0	8,541	0.3
RIO ESTRELA	15	342.50	342.50	0.00	100	302, 495	0.8
RIO IGUACU	17-1-5	562.80	544.20	18.60	97	758,010	1.3
RIO SARAPUI	17-6	165.50	159.80	5.70	97	1,012,275	5. 1
BCABO DO BRITO	18	27.00	0.00	27.00	0	132.091	4.8
RIO S. J. DE MERITI	19	164.50	163.50	1.00	j 99	1, 492, 458	9.0
RIO IRAJA	. 20	35.70	27. 30	8.40	76	500, 276	14.0
CANAL DO CUNHA	21	63, 60	60.50	3, 10	95	815, 389	12.83
BS. CRISTOVAO	22	6.60	0.00	6.60	0	60,011	9.09
CANAL DO MANGUE	23	42.80	42.80	0.00	100	500,876	11.70
BBOTAFOGO	24	26,00	0.00	26,00	0	358, 622	13.7
I. DO GAYANADOR	25	36, 20	0.00	38.20	0	153,903	1.0
I. DO FUNDAO	26	5.40	0.00	5.40	0	5.277	0.98
I. DE PAQUETA	27	1.70	0.00	1.70	0	3, 254	1.9
1. DO ENGENHO	28	1, 30	0.00	1.30	0	11.034	8.4
1. DE S. CRUZ	29	1,40	0,00	1.40	0	4,851	3. 4
	Total	4080.50	3604, 10	476.40	88	7, 594, 031	

3.5 Calculation Results and its Validation

(1) Calculation results

The total runoff load of the 20 largest rivers (basin area covered: 3,604.1 km²), determined using the parameters defined in 3.4 and following the steps described in Fig.3-6, is shown in Fig. 3-8 and Table 3-4. The calculation was based on the precipitation data in 1992 and the population data in 1991. Fig.3-9 and Table 3-5 show the annual runoff loads on clear days and rainy days and in the rainy and dry seasons, and also the runoff load not influenced by precipitation and runoff load caused by precipitation.

The daily mean discharge of the 20 rivers directly flowing into the bay was estimated at $190.2 \text{ m}^3/\text{s}$, with a BOD load of 258.5 tons/day and a TN load of 91.9 tons/day.

40 to 50% of the annual discharge and BOD load, and 20% of the annual TN load were estimated to runoff from the basin during the rainy season (55 days in a year).

The runoff load ratio on rainy days was calculated based on the results of observations conducted after a short spell of rainfall of comparatively light intensity, hence, this ratio may be smaller than actuality. Yet the ratio of the runoff load in the rainy season to the annual load is significantly large.

By the way, the importance of the runoff load on rainy days was only recognized in Japan from the 1980s (the survey results are shown in **Table 3-6**).

(2) Comparison of the estimate value and measured value

Fig.3-10 compares of the estimate values and measured values. As far as this figure is concerned, the model used here closely reflects the measured values and the transition tendencies of the values. Accordingly, this model is effective in predicting runoff load from the basin.

(3) Estimation of runoff load from the entire basin area including uncovered basin area

Runoff loads for the rainy/dry season and annual runoff load in

1992 calculated by the estimation model are shown in Table 3-7.

(a) Annual runoff load

The mean runoff discharge per day from the entire basin (basin area covered: 4,080.5 km²) was estimated at 230.2 m³/s, and the BOD and TN loads in the runoff load are 330.6 tons/day and 116.2 tons/day respectively. Fig.3-11 shows the contribution ratio of each basin for various water quality items. BOD was observed to have been largely contributed by the Rio S.J.de Meriti (16.1%), Rio Sarapui (10.8%), Canal do Cunha (9.0%) and Rio Iguacu (7.8%). The runoff discharge and runoff load of each sub-basin are shown in Fig.3-12. (see Appendix IX)

(b) Runoff load during rainy and dry seasons

The ratio of the runoff loads in the rainy season and the dry season is of 6:4 in terms of BOD and TN. Yet, as aforementioned, the runoff discharge and runoff load during the rainy season; calculated based on the data from observations that were conducted (1) during a rainfall cycle preceded by a short period of clear days, (2) a light rainfall intensity of 25mm/day and (3) with a serious mistake of missing the measurement of the first flush; are likely to be smaller than the actual values.

On these grounds, the ratio of the rainy and dry seasons was revised to 7:3 in 1992.

TP load in the rainy and dry seasons had a ratio of 4:6, contrary to the other items, supposedly because data on water quality used in the calculation was obtained in the second rainfall observation, and the inorganic phosphorus concentration was not obtained as it was not detected during the first rainfall.

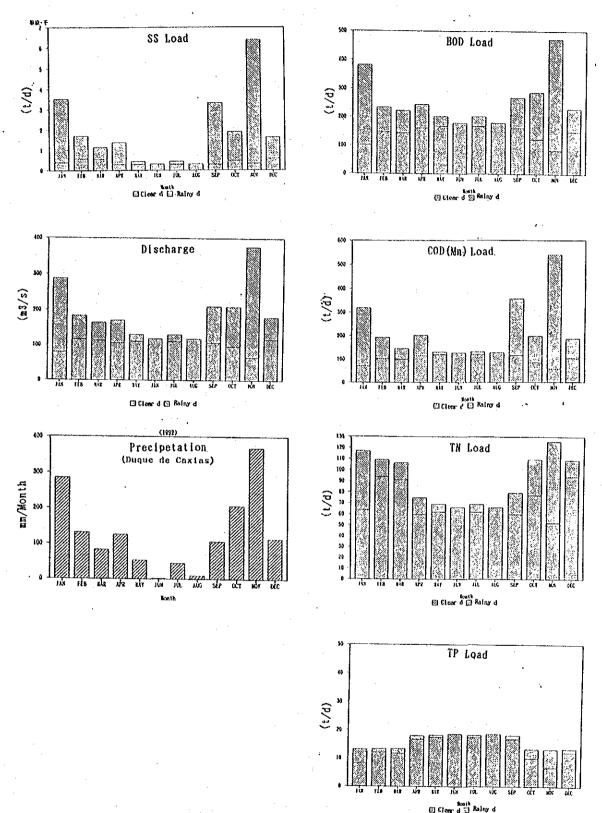
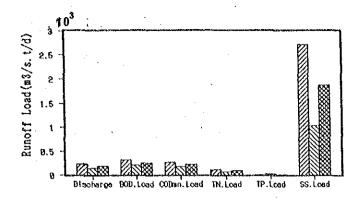


Fig. 3-8 Estimated Monthly Runoff Load from the 20 Rivers

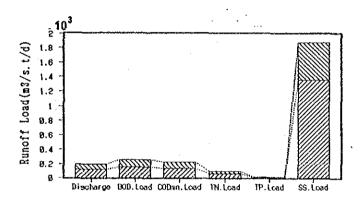
Table 3-4 Estimated Runoff Load from the 20 Rivers

			-						3	Calify Season					à	Dry sesson			_		fear	1 13 00			
		Covered		Sassa Area	Sasin AreaPopulationPopulationLand	Population	ķ	Dischar 200	š	COOmer, Load TK Load		TP. Loud SS	ğ	Discharge BOD.	ğ	CODER LOS TO	N. Lond TP.	S broi.	3	Discharge BOD.) Losed DOD-in.	ğ	The Loyd	77. Lond SS	D007
·¥	# .	Basin Area (Ka2)	Ą	3		(a/a)	Ĕ.	(2/5)	<u></u>		(2/8)	(6/2)	9 (9/3)	(8/8)	(2/4)	9/3	- G	(9/3)	(2/4)	(3/8)	- C	(2/2)	(2/8)	~~ \$3	9/3)
		-	-				_	_	_		_			-		-	_				-	 -	-	-	
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1 45740	NO ALCANTARA	3	463	144.60		ed KI	900	7	23	1,5	12.5	52	<u>.</u>	2	12	- -	۲ <u>،</u>	58	21			5.59	5	8	3
\$ 17220	RIO AUTORODO	S	43	_		73	E Carte	K	4	2	13	8	÷	13	18	3	다	8	2				12	8	12
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1 02920 1	NO CACHESTA	758.40	a,		35. 191	9	7.7	30. 93	5.05	20.07	9.69	33	23	2	92	20 20 20 20 20 20 20 20 20 20 20 20 20	29 2	F F	38			15.51	13 13	2	15. 31
_	RIO CUAPINIZIA	122. 73	2		59.853	98	K/A	유	νi Vi	17. 66	8. 52	=	*	-	12	\$		22	17 18	_		12 22	, 13	28	35. 55
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	RIG BONCHOOK	10, 30	12	111.40	35, 370	E)	K/X	H	23	2	ដ	23	23	22	*	9	0.46	8	k ::			7 8	28.55	8	23 23
	RIO IRIRI	2.40		8	5.5	88	K/A	0.36	6	E C	0 11	==	=	53	~	23	Š	0.0	8			2 38	8	5	7.75
	RIO SURCI	R L	7	55 55	12 910	61 0	*	2.2	\$ 0	1.05	0.51	3	<u>.</u>	z	• 2	З	 97	3	5 02		٠,	a. 79	H	8	*
	NO ESTREIA	342.85		347.85	35	6	5	17. 40	15.43	15.05	23	:2	g:	e.	9	<u>×</u>	i,	Si	벎	_		5 2	88.	22	11.83
25	RIO INDENDETA	139.00	16-2	8 55	3	0.6	5	2	₹ 39	4. 79	2 16		 Si	50:	50	2	8	23	25 25			23	33	53	S S
252	RIO SARACINENA	188, 30	£-91	186.00	Ľ,	3	*	10. 12	5, 89	9.31	ž	=	 =	23	7	۲. اور	2 13	3	: ES :			据 (+	= 4	c	67.97
. K. E. C.	RIO JOUGE	s ž	17-17-5	262 80	754,010	*1	×	12 23	36.23	22 22	짧	g,	52	=	_	28.28	. 9	s:	31.85		. "	26. 69	8 2	-	C IS
200	RIO SARATSI	159.20	7	165.50 th	. 012, 275	5.12	e de	24 21	6	37. 93	15.73	_	gg		23	22.22	17.2	5	139 52			 23	13.47	į,	25.25
2223	RIO S. J. DE MERITI	33	<u>.</u>	154.50	492, 453	6	e C	25 25	74. 90	86.38	3	50	<u>e</u>		en.	☆	17.31	3	207. 43			19. 23	20.47	88	386.36
ğ	RIO ACURT	8	15-2	8,13	35 036	:: ::	- Caro	23.01	7. El	N C	7, 17	21	i.			52	Si	5	25 25			53.7	R	8	25.52
200	RIO IRUJA	23	ន	21	500 276	14.03	Urban	S	3	5.65	6.36	=	·0	-		11.57	4. 32	1.22	5			12.31	5.59		100.01
7.18	CANAL BO PENIA	•	ន	•				8	8	8	90	2	9	2	9	8	8	8	8			8	8		8
2 038	CURAL DO COMIA	33	51	3	115, 389	23		23	38	30.35	12.62	gn	20			ដ	Si	33	08.48			25.43	5		201. 12
00	SAKE DO BANGUE	42.80	E,	42.80	300	11.70	F	23	۲, چ	19. 57	23		9	32		4. 82	5.17	83	70,46				. 23	. 1	130 91
5	TOTAL	3604.10		3912.56			Ξ.	32. 77	304. 43	264. 12	112 28	13. 31 1 27	712.55 1 1	46.61	. 9	79. 74	70. 33 1	18 37 1	29. 78			221. 93	5). 85 (15.54	371.16
TATOUTAL:	elimpotary myeryczelnoed frpm lotal amount	free lots.	uccust,										ı.												

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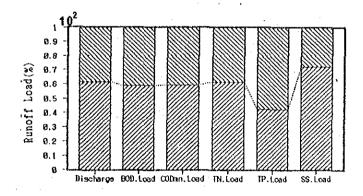


Fig. 3-9 Difference in Estimated Runoff Load between Rainy Season and Dry Season

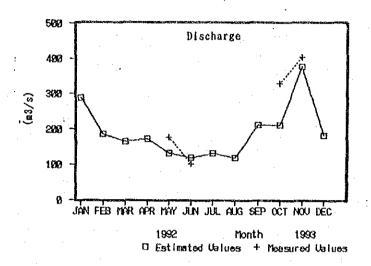
Table 3-5 Estimated Total Runoff Load from the 20 Rivers

(Clear days/ Rainy days, Rainy season/Dry sesason, Runoff Load not influenced by Precipitation/ Runoff Load caused by precipitation)

Clear day Rainy day							į
	Discharge	Deol God	COD (Nn) Load	TN Load	TP Load	bsoi SS	
	(#3/s)	\$ (1/Cay)	(1/day)	(1/day) T	((/day)	(1/dzy)	24
Clear day(<10ms/day):231days	105.18	55 1 147, 29	57 1 103, 51 1	11. 10. 11	77 (13.91	88 384.23	21
Rainy day(10mm <th>85.01</th> <th>45 111. 26</th> <th>43 118, 42 </th> <th>53 21.14</th> <th>23 1. 93</th> <th>12 1485.93</th> <th>79</th>	85.01	45 111. 26	43 118, 42	53 21.14	23 1. 93	12 1485.93	79
Mean Yalve	190,191	100 1 258, 54	1 100 1 221.93	100 91.85 i	100 15.84	100 1871, 164	100
Rainy sesson /Dry sesson							
	Discharge	SOD Load	COD (kn) Load	TN Load	TP Load	SS toad	
Season	(a3/s)	2 (t/day)	(t/day)	\$ (t/day)	% (1/day)	1 (1/day) l	ž
Rainy season (Oct-Mar. 1992)	133 77	61 : 304. 43	59 264.12	60 112.88	61 13.31	42 2712, 55	72
Dry season (Apr-Sep, 1992)	145, 51	39 212 212.66	41 179.74	40 70.83	39 18, 57	58 1029.78	28
Mean Value	190.191	100 1 258, 54	100 1 221.931	100 1 91,85 1	106 15.841	100 1871.16	100
Runoff Load not influenced by precipitation /8	unoff Load cause	precipitation / Runoff load caused by precipitation		-		;	
	Discharge	BOD Load	COD(Mn) Load	TH Load	TP Load	SS Load	
	(83/5)	X (1/day)	(t/day)	2 (t/day) 2	X (1/day)	% (t/day)	×
Runo'f Load not influenced by precipitation	116, 30 1	61 152.86	53 114.45	52 78, 19	85 15.38	97 : 424_86	23
	73,89	39 55, 68	37 107, 48	13.66	15 0.46	3 1445.31	77
	190,191	100 258.54	100 221.93	100 91.85	100 15.84	100 1871, 16	190

Table 3-6 Runoff Load Ratio on Clear Days and Rainy Days in Japan (Runoff Load (%))

	·						(1980-1989)
		OD (Mn)	Т	- P	T	- N	
River Name	Clear/ days	Rainy/ days	Clear/ days	Rainy/ days	Clear/ days	Rainy/ days	Runoff Load, Separation Method, etc.
R. Sandagawa 1985	(Urban 48.6	type riv	ver) 40.9	59.1	60.9	39.1	Runoff load tank model Separation method 2.
R. Ohtsu 1988	(Natur 29.7	al type 70.3	river) 34.3	65.7	36.4	63.6	Precipitation of 1 mm or less within 12 hours assumed a rainy day, using L-Q regression model equation.
R. Nogawa		type riv					75% of annual load value
1987	44.0	56.0		~~ **	- ~		was assumed as low flow
1988	51.4	48.6					and clear day.
1989	27.5	72.5					
R. Tamagawa		type ri	ver)				75% of annual load value
1985	44.8	55.2					was assumed as small water
1986	64.9	35.1		· 			amount and clear day.
1987	63.6	36.4					•
1988	28.2	71.8	47.4	52.6			
1989	46.9	53.1	59.5	40.5	-~		
R. Tomoegawa	(Urban	type ri	ver)				Clear days assumed no in-
1980/81	56.1	43.9	53.0	47.0	70.1	29.9	fluence of runoff by pre-
1981	55.1	44.9	52.8	47.2	69.7	30.3	cipitation. The following
1981/82	59.5	40.5	62.0	38.0	73.7	26.3	is the number of days deducted from the clear days.
1987	61.5	38.5	68.9	31.1	94.5	5.5	10 mm/day over +1 day 20 mm/day over +2 days
1988	40.4	59.6	35.9	64.1	63.1	36.9	30 mm/day over +3 days
1989	49.6	50.4			71.0	29.0	50 mm/day over +3 days
1303	49.0	əu.4	45.3	54.7	11.0	20.U	50 mm/day over +4 days
R. Sannougawa							Once per each season.
1979.80	18.4	81.6	32.7	67.3	61.5	38.5	4 time per year. L-Q regression model equation.



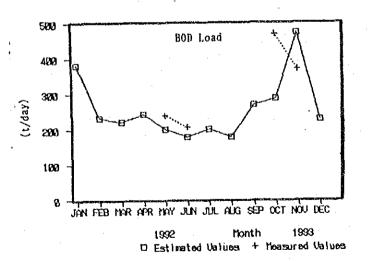


Fig. 3-10 Comparison of Estimated Runof Load with Measured One

Table 3-7 Estimated Runoff Loads from the Entire Basin (1991)

			-				-			Men. Volue			
;							l.			Mean value	,		
Basin No.	:	Correred	ein	asin Area	Sasin AreaPopulationPopulationLand	Population	use	Discharge	BOD Load	CODIED Load	Pod N	 bgd H	S Load
-	Nane	Basin Area	Š.	· ·	—	Density		,			:		
		(((Kn.²)		(10 3/km²)		(s/, #)	(t/d)	(t/d)	(t/d)	(1/g)	(1/4)
	BCHARITAS	9.40		9.40	53, 310		Urban	1.17	2.35	1.89	0.79	0.15	14.52
Eastern 2	CANAL CANTO DO RIO	7.40	~	7. 40	41, 745		Urb/S, T	0. 92	1.84	1.49	0.62	0.12	11.40
Basin	3 BCATEDRAR	7.80	ຕາ	7.80	37, 458		Urban	0.86	1.65	.33	0.56	0.11	10.33
7	BNORTE CENTRO	7.90	7	7. 90	43, 607		Urban	96 0	1.92		0.65	0.12	11.91
	S RIO BOMBA	26. 20	Ŋ	26. 20	183, 099		Urban	3, 75	8.00	6.33	2.65		48.90
<u> </u>	RIO IMBOASSU	30.80	မှ	30.80	138, 636		Urban	3. 14	6.02	4.81	2.00	0.38	38.02
	7 BITAOCA	6.40	ţ	6.40	31, 925		Urban	0.73	1.41	1.14	0.48	0.09	8.80
Northeastern 8	RIO ALCANTARA	144.60	60	144.60	470, 420		Urban	11.48	20.02	16.01	6.60	1.20	131.48
Basin	RIO CACEREBU	ശ്	თ	846. 70	336, 193		N/A	27.67	14.80	17. 28	2.08	0.80	174.97
)[RIO GUAPINIRIN	1253.10	21	1253. 10	69, 853		N/A	32. 36	4. 57	12. 97	5. 28	0. 26	156.07
	CANAL DE MAGE	18.30	Ξ	18, 30	8, 458		N/A	0.67	0.38	0.44	0. 18	0.05	4.16
77	RIO RONCADOR	111.40	12	111.40	36, 370		N/A	3.65	1.66	2.11	0.88	0.09	21.36
Ţ.	3 RIO IRIRI	27.80	13	27.80	10, 684		N/A	0.97	0.49	0.59	0. 25	0.03	5.77
14	RIO SURUI	68.80	7	68.80	12, 910		N/A	2.09	0.63	1.02	0.43	0.04	10.90
15		28.90	15	28. 90	8,541		N/A	0.96	0,40	0.53	0.22	0.02	5.36
Northwestern 16	S RIO ESTRELA	342.50	91	342, 50	302, 495		N/A	14. 10	12. 92	12.09	4.96	0. 72	111.33
Basin 17.1	S RIO IGUACU	562.80	17-175	562, 80	758, 010		N/A	27.01	31.97	27.58	11.26	1. 30	245.53
17.6	RIO SARAPUI	165, 50	17-6	165.50	1, 012, 275		Urban	20.61	43.40	33.72	13.94	2.66	268.38
18	BCABO DO BRITO	27.00	<u>8</u>	27.00	132, 091		Urban	2.93	5.75	4.58	 1.91	0.36	36.03
31	RIO S. J. DE MERITI	164.50	19	164.50	1, 492, 458	9. 07	Urban	28. 27	64.33	49.68	20. 59	4.01	388. 70
72	RIO IRAJA	35.70	20	35.70	500, 276		Urban	9.25	22.04	17.30	7.28	1.44	130, 49
Western 21	CANAL DO CUNBA	63. 60	21	63. 60	815, 389		Urban	15.04	35, 66	27.80	11. 62	2.30	212.01
Basin 22	BS. CRISTOVAO	6. 60	22	6.69	60, 011		Urban	1.21	2.67	2.14	0.90	0. 18	16.04
53	CANAL DO MANGUE	42.80	23	42.80	500, 876		Urb/S, T	9.40	21.96	17.20	7. 20	1, 42	130.91
24	BBOTAFOGO	26.00	24	26.00	358, 622		Urban	6. 68	15.84	12.48	5.24	1.04	93.81
	<u></u>	38. 20	22	38. 20	153, 903		Urban	3.59	6.66		2. 22	0.41	42.54
Island 26	1. DO FUNDAO	5.40	58	5.40	5, 277		N/4	0.25	0.23	0.22	0.09	0.01	1.93
. 21	三.	1.70	27	1.70	3, 254		N/A	0. 11	0.14	0.13	0.02	0.01	1.02
- 58	I. DO ENGENHO,	1.30	82	1.30	11.034		Urban	0.23	0.20	-0.41	0.17	0.03	93. 83.
25	I I. DE S. CRUZ	1.40	53	1.40	4,851		Ürban	0.13.	0.22	0.18	0.08	0.01	1.40
	Total			4080.50 F	7. 594. 031			230.16	330. 59	280.34	116.18	20.37	2337.07

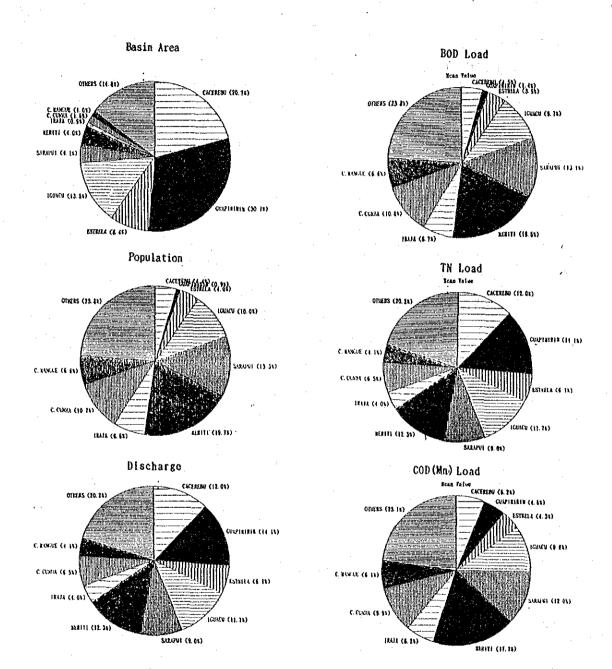


Fig. 3-11 Contribution Ratio of Estimated Runoff Load by Each Basin

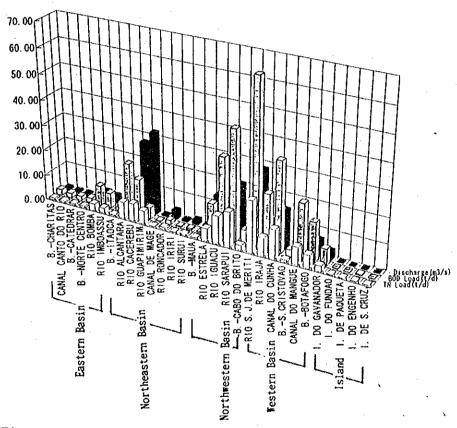


Fig. 3-12 Estimated Runoff Load from Each Sub-Basin

CHAPTER 4

DETAILS OF RUNOFF LOAD FROM THE BASIN

Chapter 4

Details of Runoff Load from the Basin

Pollutants flow into the bay through various routes, some of them via rivers or stormwater drains, and some directly from the pollution sources (factories, waste treatment plants, sewage disposal plants, etc.). Here, the total runoff load is calculated after obtaining the runoff load of each route.

4.1 Runoff Load Flowing into the Bay through Rivers and Stormwater Drains

Fig. 4-1 and Table 4-1 show the runoff load flowing into the bay from each area. According to the position of their mouths in the bay, rivers were classified into the following five groups: eastern basin (sub-basin Nos.1 to 6), northeastern basin (sub-basin Nos.7 to 14), Northwestern basin (sub-basin Nos. 15 to 18), western basin (sub-basin Nos.19 to 24) island basin (sub-basin Nos. 25 through 29). Due to differences in water quality items, 45 to 51% of the total runoff load comes from the western basin and about 30% from the northwestern basin. 35% of the total runoff discharge is contributed by the northeastern basin, and although this amount is larger than the western or northwestern basins, it only supplies 12 to 18% of the total runoff load.

Besides the rivers, stormwater drains also discharge runoff load into the bay. Separate sewers were installed in Rio de Janeiro a century ago. Yet since the construction of treatment plants they were never upgraded, rain water drains were used as sewers and there are 5 open outlets around Rio de Janeiro port, and another one north of Niteroi.

CEDAE estimated the BOD load flowing into the bay directly from the five outlets was about 36 tons/day (no data was available for RSD-02). However, since this figure was calculated based on pump capacity, runoff ratio was not considered. No data on the discharge outlet in northern Niteroi, outside of Jurujuba Bay, was obtained.

4.2 Runoff Loads Flowing into the Bay Directly from the Pollution Sources on the Coastal Areas

Among the various pollution sources along the coast of the bay, factories, that are located on the downstream side of the observation stations, discharge about 24 tons of BOD per day, sewage treatment plants (6 plants of Penha, ETEIG, ETEG, ETAR-AIRJ, ETAR-TECA and Icarai) discharge about 5.4 tons per day, and waste disposal plants (leachate), about 0.3 tons a day. The total BOD load totals approximately 30 tons a day.

Food factories (seafood processing factories among others) that are located between Niteroi and San Goncalo in the eastern area make up about 75% of the total load discharged by factories in the basin.

The total runoff load in terms of BOD load that flows into the bay was estimated as 360.53 tons a day by adding the runoff load of 330.59 tons/day from rivers and rain water drains to the runoff load of 29.94 tons/day from pollution sources on the coast.

Details are shown in Table 4-2 and Fig. 4-2.

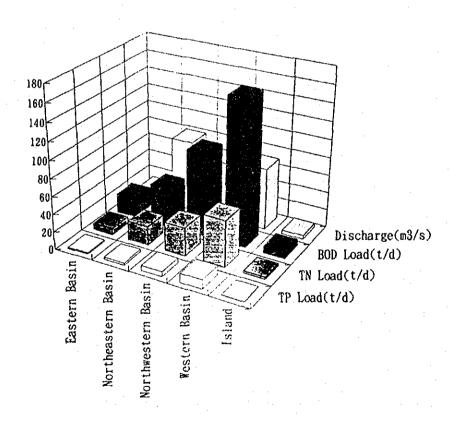


Fig. 4-1 Runoff Load from Each Area

Table 4-1 Details of BOD Load from Each Area

SN(1991)		
No.	1	1991 BOD. Load
	Каже	į į
		(t/d)
	BCHARITAS	2. 35
	CANAL CANTO DO RIO	1.84
	B. ~CATEDRAR	1.65
	BNORTE CENTRO	1.92 8.00
· · · · · · · · · · · · · · · · · · ·		6.02
River Sub Total		21.79
001 001	1	2. 13 6. 70
004	1	2.40
008	}	2. 10
009 Ind 027	•	1.94 0.80
034	1	0.66
044	1	0.51
047	!	0.48 0.38
113	ł .	0.38
Industry Sub Total		18. 32
WHTP SWDS		1.63
Smus Eastern Sub Total		41.736
í	BITAOCA	1.41
		20.07 14.80
River 10		4. 67
. 11	[0.38
17		1.66 0.49
14	1	0.63
River Sub Total		44.11
Ind Industry Sub Total	-	
WWTP		
SWDS Northeastern Sub Total		44. 112
northeastern sub rotar		0.40
16	1	12.92
River 171	1	31. 97 43. 40
18		5.75
River Sub Total		94.43
015 018		1. 32 1. 20
1nd 075	ľ	0.33
029		0.79
086		0.31 0.16
137 Industry Sub Total		4. 11
WWTP		
SWDS Northwestern Sub Total		0.30 98.841
ls	RIO S. J. DE MERITI	64. 33
20	RIO IRAJA	22.04
River 21 22		35. 66 2. 67
23	1 '	21.95
2.4	BBOTAFOGO	15.84
River Sub Total	 	162.50 0.72
Ind 042	i i	0.52
051 Industry Sub Total	ļ	0.45 1.69
industry Sub Iolai		2.45
SWDS		
Wester Sub Total 23	I. DO GAYANADOR	166, 641 6, 66
26		0. 23
Island 27	L. DE PAQUETA	0.14
28	[0.50 0.22
29 Island Sub Total	1. DE S. CRUZ	7, 75
Ind		
Industry Sub Total ##TP		0.28 1.16
SWDS	†	
Islands Sub Total		9. 195
	Total	360.53

Table 4-2 Details of BOD Load from the Basin

an and de research and with the state of the	Runoff toad from the basin(A)		Direct Runoff Load from Point Source(B)		
	River(t/d)	Industry(t/d)	PRTP(t/d)	SFDS(t/d)	(t/day)
Eastern Basin (1-6)	21.79	18. 32	1. 63	•	41. 74
Northeastern Basin (7–14)	44. 11	: :			44. 11
Northwestern Basin (15-19)	94.43	4.11		0. 30	98. 84
Vestern Basin (20-24)	162, 50 (RSD: 36, 00)	7	2. 45	- -	166. 64
Island (25-29)	7. 75	0. 28	1. 16	•	9. 19
Total	330. 59	24. 40	5. 24	0. 30	360. 53

Remarks

TTTP: Tastemater treatment plant STOS: Solid waste disposal site RSD: Raw sewage drain pipes

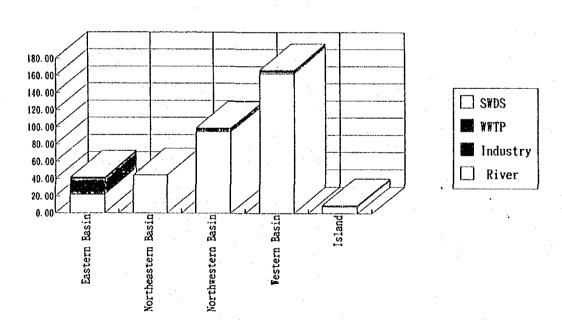


Fig. 4-2 Estimated Runoff Load (BOD) from Each Area

CHAPTER 5

THE FUTURE RUNOFF LOAD FROM THE BASIN

Chapter 5

The Future Runoff Load from the Basin

5.1 Calculation Method

In Chapter 12 of the Main Report, the future socio-economic conditions in the basin were forecast and population was estimated for two different scenarios using the Merkmal parameter. In this chapter, the future runoff loads from the Guanabara Bay basin will be estimated for 3 cases (2000, 2010-1, 2010-2) in accordance with the scenarios.

The estimated basin population in each scenario is based on the population by administrative unit shown in the Main Report, Table 12.3-1, and if every basin populations are determined using the weighted mean method, the basin population and population density are as shown in Table 5-1 to 5-2, Fig.5-1.

The future runoff load calculated in Chapter 3 (Potential Model of pollutant runoff load) will be used along with population density as parameters.

Finally, the present statistical relation of the BOD/capita in the estimation model is assumed to remain the same in the future.

5.2 Estimation Results and Evaluation

The runoff load calculations for 1991 and the two scenarios, 2000, 2010-1, 2010-2, are shown in **Table 5-3**. Further, **Fig.5-2** indicates the annual changes in runoff load for the worst case scenerio (2010-2). Along with population increase, the BOD load in 2010 was estimated to be 415 t/day, that is an 85 t/day increase from 1991.

Fig.5-3 shows the runoff loads for BOD and TN from each sub-basin.

Table 5-1 Future Population in the Basin (1991-2010)

		Γ		(1991)	(2000)	(2010-1)	(2010-2)
Basin No.	<u> </u>	Basin Area				,	
	Name		NO.	Population	Population	Population	Population
		(Km2)		(persons)	(persons)	(persons)	(persons)
				,			
1	BCHARITAS	9.40	· 1	53, 310	57,042	58, 641	59.707
Eastern 2	CANAL CANTO DO RIO	7.40	2	41,745	44,667	45, 920	46,754
Basin 3	BCATEDRAR	7.80	3	37, 458	40,080	41, 204	1, 41,953
4	BNORTE CENTRO	7.90	. 4	43,607	46, 659	47, 968	48,840
5	RIO BOMBA	26.20	5	183,099	227, 043	256, 339	258, 170
6	RIO IMBOASSU	30.80	6	138,636	180, 227	209.340	209, 340
7	BITAOCA	6.40	7	31.925	41,503	48, 207	48, 207
Northeaster 8	RIO ALCANTARA	144.60	8	470,420	592, 729	672,701	677, 405
Basin 9	RIO CACEREBU	846.70	9	336, 193	450, 499	534, 547	534,547
10	RIO GVAPIMIRIM	1253.10	10	69,853	83. 524	102.684	102,684
11	CANAL DE MAGE	18, 30	11	8,458	10, 150	12.349	12,349
12	RIO RONCADOR	111.40	. 12	36, 370	43,644	53, 100	53, 100
13	RIO IRIRI	27.89	13	10,684	12, 821	15.599	15,599
14	RIO SURUI	68.80	-14	12,910	15, 492	18,849	18,849
15	BMAUA	28.90	15	8,541	10, 249	12, 470	12, 470
Northwester 16.	RIO ESTRELA	342.50	16	302, 495	362, 994	423, 493	435, 593
Basin 17.1-5	RIO IGUACU	562.80	17-1-5	758,010	909, 612	1, 023, 314	1,076,374
17.6	RIO SARAPUI	165.50	17-6	1,012,275	1, 153, 994	1, 255, 221	1, 305, 835
18	BCABO DO BRITO	27.00	18	132,091	158, 509	178, 323	187, 569
19	RIO S. J. DE MERITI	164.50	19	1, 492, 458	1,611,855	1,671,553	1,716,327
20	RIO IRAJA	35.70	20	500, 276	535, 295	550, 304	560, 309
Western 21	CANAL DO CUNHA	63.60	21	815, 389	872, 466	895.928	913, 236
Basin 22	BS. CRISTOVAO	6. 60	22	60,011	54, 212	- 66,012	67, 212
23	CANAL DO MANGUE	42.80	23	500,876	535, 937	550, 964	560.981
24	BBOTAPOGO	26.00	24	358,622	383, 726	394, 484	401,657
25	1. DO GAVANADOR	38. 20	25	153, 903	164,676	169, 293	172, 371
Island 26	I. DO FUNDAO	5.40	26	5,277	5,646	5,805	5,910
27	L. DE PAQUETA	1.70	27	3, 254	3, 482	3,579	3,644
. 28	I. DO ENGENHO	1.30	28	11,034	11,806	12.137	12,358
29	1. DE S. CRUZ	1.40	29	4,851	5, 191	5,336	5, 433
	Total	4080.50		7, 594, 031	8,636,028	9, 336, 661	9, 564, 782

Table 5-2 Future Population Density in the Basin (1991-2010)

B • 31				(1991)	(2000)	(2010-1)	(2010-2)
Basin No.		Basin Area	No		D 1 . 4 4		h
	Name	/v a\	NO.	F			Population
	•	(Km2)		Density	Density	Density	Density
	h antatata			(10 ³ /km2)	(10 ³ /km2)	(10 ³ /km2)	(10 ³ /km ²)
1	BCHARITAS	9.40	i	5.67	6.07	6. 21	6.35
Eastern 2	CANAL CANTO DO RIO		2	5. 64	6.04	6. 21	6.32
Basin 3	BCATEDRAR	7.80	3	4.80	5.14	5. 28	5.38
4	BNORTE CENTRO	7.90	4	5. 52	5.91	6.07	6.18
5	RIO BOMBA	26.20	5	6.99	8.67	9.78	9.85
6.	RIO IMBOASSU	30.80	. 6	4.50	5.85	6.80	6.80
7	B ITAOCA	6.40	7	4.99	6.48	7. 53	7.53
Northeaster 8	RIO ALCANTARA	144.60	8	3.2\$	4.10	4.65	4.68
Hasin 9	RIO CACEREBU	846.70	9	0.40	0.53	0.63	0.63
- 10	RIO GUAPIMIRIM	1253 10	10	0.06	0.07	0.08	0.08
11	CANAL DE MAGE	18.30	11	0.46	0.55	0.67	0.67
12	RIO RONCADOR	111.40	12	0.33	0.39	0.48	0.48
13	RIO IRIRI	27.80	13	0.38	0.45	0.56	0.56
14	RIO SURVI	68.80	14	0.19	0.23	0.27	0.27
15	BMANA	28.90	15	0.30	0.35	0.43	0.43
Northwester 16	RIO ESTRELA	342.50	16	0.88	1.06	1. 24	1.27
Basin 17.1 5	RIO IGUACU	562.80	17-1-5	1.35	1.62	1.82	1.91
17.6	RIO SARAPUI	165.50	17-6	6.12	6, 97	7.58	7.89
18	BCABO DO BRITO	27 00	18	4.89	5, 87	6.60	6.95
19	RIO S. J. DE MERITI	164.50	19	9.07	9.80	10.16	10.43
20	RIO IRAJA	35. 70	20	14.01	14.99	15.41	15.69
Western 21	CANAL DO CUNHA	63, 60	21	12.82	13.72	. 14.10	14.36
Basin 22	BS. CRISTOVAO	6.60	22	9.09	9, 73	10.00	10, 18
23	CANAL DO MANGUE	42.80	23	11.70	12.52	12.87	13.11
24	BBOTAFOGO	26.00	24	13.79	14.76	15.17	15.45
25	1. DO GAVANADOR	38. 20	25	4.03	4, 31	4. 43	4.51
Island 26	I. DO FUNDAO	5. 40	26	0.98	1.05	1.07	1.09
27	I. DE PAQUETA	1.70	27	1.91	2.05	2.11	2.14
28	1. DO ENGENHO	1.30	28	8. 49	9.08	9.34	9.51
29	1. DE S. CRUZ	1, 40	29	3.47	3.71	3, 81	3.88
	lotal	4080.50		1.86	2.12	2, 29	2.34

population(persons)

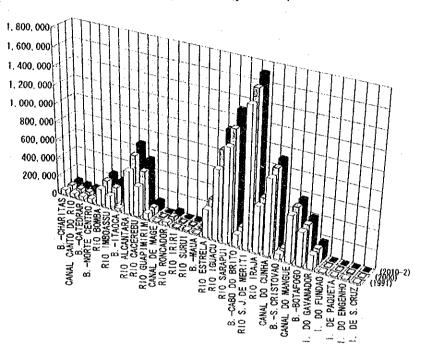
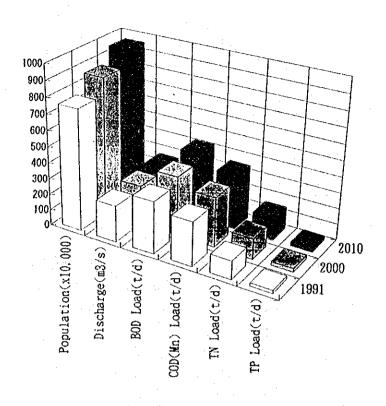


Fig. 5-1 The Future Population in the Basin (1991 - 2010)



Ludex	1991	2000	2010-1 (Scenario 1)	2010-2 (Scenario 2
Population				
(persons)	7, 594, 031	8,636,028	9, 336, 661	9, 564, 782
Baasin Area				
(Km2)	4,080.50	4,080,50	4,080.50	4, 080, 50
Discharge				
(m3/s)	230.16	247. 16	258.60	262.27
BOD Load				
(t/d)	330.59	375.40	405.42	415.33
COD(Mn) Load				
(t/d)	280.34	314.21	336.83	344.37
IN Load				
(t/d)	116.18	130.24	139.62	142,75
TP Load				
(t/d)	20.37	23.17	25.04	25.68
SS Load				
(t/d)	2, 337. 07	2,651.46	2,827.82	2,886.12

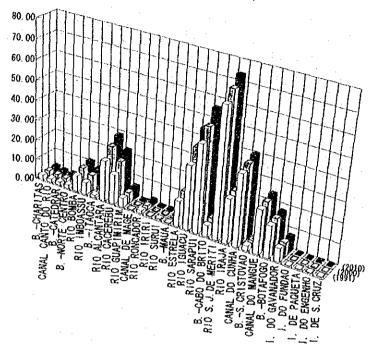
Fig. 5-2 Future Runoff Loads from the Basin (1991 - 2010)

Table 5-3 Estimation Runoff Load for BOD and TN from Each Sub-Basin

Basin No.			Basin Area		(1991)	(2000)	(2010-1)	(2010-2)
Basin NO.		Name	(Km2)	NO.	BOD Load (1/d)	BOD Load (t/d)	BOD Load (t/d)	BOD Load (t/d)
	1	BCHARITAS	9, 40	1	2. 35	2.51	2.59	2. 63
Eastern	2	CANAL CANTO DO RIC		2	1.84	1, 97	2.03	2. 01
Basin	3	BCATEDRAR	7.80	3 .	1.65	1.77	1.82	1.8
	. 4	BNORTE CENTRO	7. 90	4	1.92	. 2.06	2. 12	2. 1
	5	RIO BOMBA	26. 20	5	8.00	9, 96	11.26	11.34
	6	RIO IMBOASSU	30.80	6	6.02	7.85	9.13	9, 1
	7	BITAOCA	6.40	7	1.41	1.84	2.14	2. 1
Northeaster	8	RIO ALCANTARA	114.60	8	20.07	25.33	28.79	28.9
Basin	9	RIO CACEREBU	846.70	9	14.80	19.46	22, 90	22. 9
	10	RIO GUAPINIRIN	1253.10	10	4.67	5. 22	5.97	5.9
	11	CANAL DE MAGE	18, 30	11	0.38	0.45	0.55	0.5
	12	RIO RONCADOR	111.49	12	1.66	1.96	2.35	2.3
	13	RIO IRIRI	27.80	13	0.49	0.58	0.69	0.6
	14	RIO SURUI	68.80	14	0.63	0.74	. 0,88	0.8
	15	BMAUA	28.90	15	0.40	0.47	0.56	0.5
Northwester	16	RIO ESTRELA	342.50	16	12.92	- 15, 44	17.96	18.4
Basin	17.1 5	RIO IGUACU	562.80	17-1 5	31.97	38, 30	43.05	45. 2
	17.6	RIO SARAPUI	165.50	17-6	43.40	49.56	53.97	56.1
	18	BCABO DO BRITO	27.00	18	5.75	6.91	7.79	8.2
	19	RIO S. J. DE MERITI	164.50	19	64.33	69.56	72.17	74. 1
	20	RIO IRAJA	35.70	20	22.04	23.61	24. 28	24.7
Western	21	CANAL DO CUNHA	63.60	21	35.66	38. 20	39, 29	40.0
Basin	22	BS. CRISTOVAO	6.60	22	2. 67	2.86	2.94	2.9
	23	CANAL DO MANGUE	42.80	23	21.98	23.52	24.19	24.6
	24	BBOTAFOGO	26.00	24	15.84	16.97	17.45	17. 7
	25	1. DO GAVANADOR	38. 20	25	6.66	7.13	7. 33	7.4
Island	26	1. DO FUNDAO	5.40	26	0.23	0.25	0.26	0.2
	27	I. DE PAQUETA	1.70	27	0.14	0.15	0.16	0.1
	28	I. DO ENGENBO	1,30	28	0.50	0.53	0.55	0.5
	29	I. DE S. CRUZ	1.40	29	0.22	0.23	0. 24	0.2
		Total	4080.50		330.59	375.40	105.42	415.3

(IN)					1 4			1. /22.2.25
B 1 17			<u></u>		(1991)	(2000)	(2010-1)	(2010-2)
Basin No.		Name	Basin Area	NO.	TN Load	TN Load	TN Load	TN Load
		name	(17.2)	ĸυ.			•	(t/d)
	J		(Km2)		(t/d)	(t/d)	(t/d)	(1/4)
····	i	BCHARITAS	9.40	1	0.79	0.85	0.87	0.89
Eastern	2	CANAL CANTO DO RIO	7.40	Z	0.62	0.67	0.69	0.70
Basin	3	BCATEDRAR	7.80	3	0.56	0.60	0.δ2	0.63
	4	BNORTE CENTRO	7.90	4	0.65	0.70	0.72	0.73
	-5	RIO BOMBA	26. 20	5	2.65	3. 29	3.72	3.75
	6	RIO IMBOASSU	30.80	5	2.00	2.60	3.01	3.01
	7	BITAOCA	6.40	7	0.48	0.62	0.72	0.72
Northeaster	8	RIO ALCANTARA	144.60	8 .	6.60	8. 25	9.33	9.39
Basin	9	RIO CACEREBU	846.70	9	7.08	8.43	9.43	9.43
	10	RIO GUAPIMIRIM	1253.10	10	5. 28	5.46	5.70	5. 70
	11	CANAL DE MAGE	18.30	11	0.18	0.21	0.24	0. 24
	12	RIO RONCADOR	111.40	12	0.88	0.97	1.09	1.09
	13	RIO IRIRI	27.80	13	0.25	0.28	0.31	0.31
	14	RIO SURUI	68.80	14	0.43	0.46	0.51	. 0. 51
	15	BMAUA	28.90	15	0.22	0.25	0.28	0.28
Northwester	16	RIO ESTRELA	342.50	16	4.96	5.71	6.47	* 6.62
Basin	17.175	RIO IGUACU	562.80	17-175	11.26	13.14	14.56	15. 23
	17.6	RIO SARAPUI	165.50	17-6	13.94	15.88	17. 28	17.99
	18	BCABO DO BRITO	27.00	18	1.91	2. 29	2. 58	2. 71
	19	RIO S.J. DE MERITI	164.50	19	20.59	22. 26	23.10	23.73
	20	RIO IRAJA	35.70	20	7. 26	7.78	8.01	8.16
Western	21	CANAL DO CUNHA	63.60	21	11.62	12.45	12.81	13.05
Basin	22	BS. CRISTOVAO	6.60	22	0.90	0.97	0. 99	1.01
	23	CANAL DO MANGUE.	42.80	23	7. 20	. 7.71	7.94	8.08
	24	BBOTAFOGO	26.00	24	5. 24	5. 62	5.79	5.89
	25	I. DO GAVANADOR	38. 20	25	2. 22	2. 37	2. 43	2.48
Island	26	I. DO FUNDAO	5.40	26	0.09	0.10	0.10	0. 10
	27	I. DE PAQUETA	1.70	27	0.05	0.06	0.06	0.06
	28	I. DO ENGENHO	1.30	28	0.17	0.19	0.19	. 0.19
	29	I. DE S. CRUZ	1.40	29	0.08	0.08	0.08	0.09
	-	Total	4080.50		116.18	130.24	139.62	142.76





TN Load(t/d)

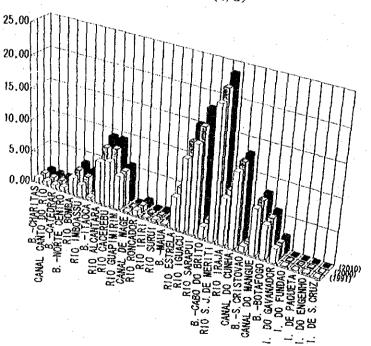


Fig. 5-3 Future Runoff Loads from Each Sub-Basin

CHAPTER 6

RUNOFF LOAD ESTIMATION RESULTS THROUGH INFLOW LOAD REDUCTION MEASURES

Chapter 6

Runoff load estimation results through inflow load reduction measures

6.1 Runoff load estimation results through reduction measures

Runoff load was estimated for the following 4 cases:

- (1) Runoff load in 2000 (primary treatment) and 2010 (secondary treatment) if the sewage treatment plants are completed under the IDB/OECF Program.
- (2) Runoff load in 2010 if the sewage treatment plant (primary treatment) are completed in urban areas outside the target area.
- (3) Runoff load in 2010 if the ocean outfall system is completed following the construction of the treatment plants (primary treatment) by 2000 under the IDB/OECF Program.

Ocean outfall (1) draft: To discharge the waste from the southern part of the Meriti basin and the southern part of the TOQUE-TOQUE sewage treatment district utside the bay.

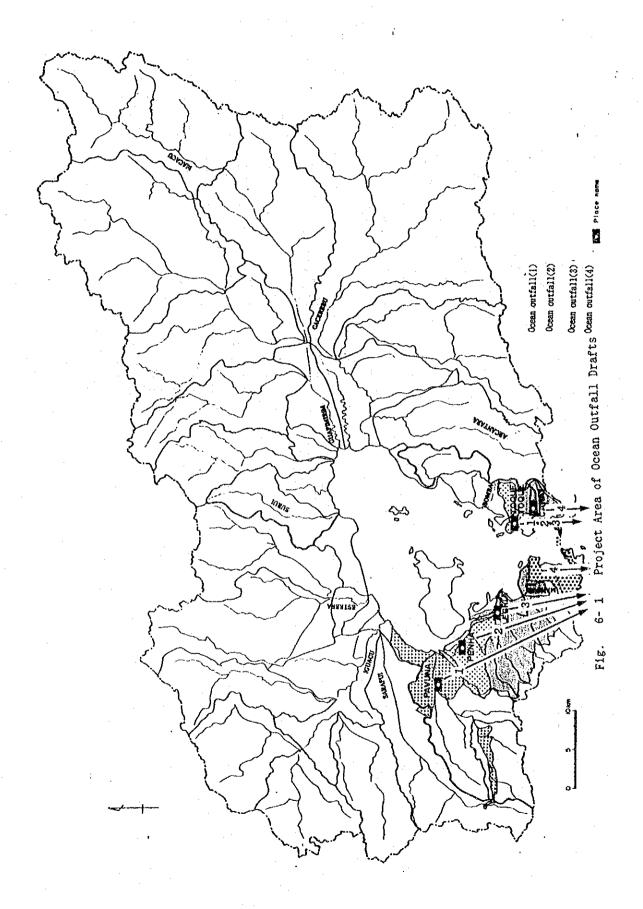
Ocean outfall (2) draft: To discharge the waste from the southern part of the PENHA sewage treatment district and the southern part of the TOQUETOQUE sewage treatment district outside the bay.

Ocean outfall (3) draft: To discharge waste from the southern part of the ALEGRIA and FUNDAO sewage treatment district and the eastern area of southern TOQUETOQUE sewage treatment district outside the bay.

Ocean outfall (4) draft: To discharge the waste from the southern part of the BOTAFOGO sewage treatment district and the southern part of the ICARAI sewage treatment district outside the bay.

Ocean outfall drafts are shown in Fig.6-1.

(4) Runoff load in 2010 with the construction of a retarding ponds



TTEM 1 2 3(B-1) 3(B-2) 3(B-3) 3(B-4) 4

Project Area(km²) 271.4 271.4 752.4 271.4 271.4 271.4 271.4 271.4

Measures

Primary Treatment x X X X X X

Secondary Treatment X X

Additional

Primary Treatment

X

Ocean Outfall

 $X \qquad X \qquad X \qquad X$

6.2 Points to be considered for the calculation

The runoff ratio in areas with sewers varies from that without sewers. Runoff loads are determined using the runoff ratio as a parameter, however the results of the calculations may largely differ according to the actual set up of the sewers.

The sum of the areas with sewers and the areas where sewers are to be constructed with sewers under the IDB/OECF Program will be, for calculation purposes, called the area with sewers, and the runoff load will be determined by multiplying the area with the runoff ratio of the area with sewers(Fig. 6-2).

This study assumed that population in the basin is evenly distributed. Given this assumption, the population in areas with sewers and therefore the evaluation of the load reduction could have been underestimated.

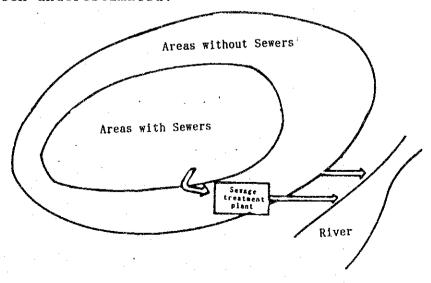


Fig. 6-2: Runoff ratio of the areas with and without sewers

6.3 Terms of Calculation

The parameters for the calculation were established as shown below:

(1) Runoff ratio of the areas with and without sewers

Using the WADA (1980) data, the runoff ratio in areas with sewers is as shown below, while the runoff ratio of the basin will be the weighted average of the areas with and without sewers.

In addition, the present runoff ratio of areas without sewers is assumed not to change.

Parameters	Areas with Se Primary Treatment	ewers Secondary Treatment	Areas without Sewers	
Discharge	100 %	100 %	100 %	
BOD	49	18	100	
TN	90	82	100	
TP	95	91	100	

The BOD value was determined by using the value (*1) obtained from the results of a study on the drainage system (combined sewer system) of Yabata river basin. The BOD value used is shown in Fig.6-3. TN and TP values were determined by using the BOD value actually measured.

*1 Source: Yasuhiko Wada: Non point source load quantity and its impact on the water environment, the 16th Health Engineering Research Institute Debate Article, Vol. 16, 1980.

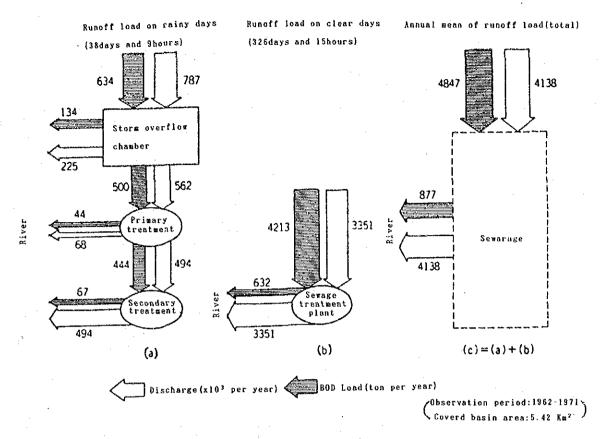


Fig. 6-3 Material Balance on the Sewer System in Yabata River Basin

(2) Waste Removal Ratio of the Sewage Treatment Plant

The waste removal ratio of the sewage treatment plant used to determine the runoff ratio of areas with sewers is as shown below:

Parameters	Areas with Ser Primary Treatment	wers Secondary Treatment	
Discharge	100 %	100 %	
BOD	50	90	
TN	10	20	
TP	-5	10	
4 - F			

(3) Ocean Outfall Runoff Ratio

The runoff ratio of areas with sewers to be adopted in the ocean outfall was based on *1 data and shown below. The remaining runoff load in areas with sewers is assumed to be discharged outside the bay through the ocean outfall.

Parameters	Areas with Sewers					
BOD	2.9 %					
TN	9.5					
TP	9.5					

TN and TP values are estimated to have a ratio similar to Discharge.

(4) Load Removal Ratio of the Retardation Ponds

The load removal ratio of the retardation ponds was established as shown below. The load removal ratio for the first flush of the retarding ponds of Matsuura (1987) (*2), which has a storage capacity of 200,000 m³, ratio of COD removed is 24%, 8% of TN, and 16% of TP. These values were used to determine the load removal ratio of the retardation ponds. The COD (Mn) ratio will also be used to also obtain the BOD ratio.

The maximum rainfall to be used for the calculation of the storage capacity of the retarding ponds will be set at 20mm/day for the effective use of the facilities.

Parameters	Areas with Sewers	· .
BOD	24 %	
TN	8	
ТР	16	
**	10	

^{*2} Source: Mushiake, ed.: Conservation and Recuperation of the water environment, Sankaido (October 1987)

6.4 Estimation Results and Evaluation

(1) Estimation Results

The runoff load estimation results with countour measurments are shown in Table 6-1 (Appendix X).

Fig.6-4 compares of runoff load of areas with sewage treatment and area without sewage treatment (1991-2010).

(2) Evaluation

The discharge load results based on the inflow load reduction countermeasures are shown in Fig.6-5, and Table 6-2. The effects of each reduction countermeasure for 2010 will be compared.

- Number (2) countermeasure will be used for BOD; the ocean outfall countermeasure (3) which will discharge treated sewage outside the bay will be used for TN and TP.
- Countermeasures that are highly attainable are (a) secondary treatment under the IDB/OECF Program, and (b) the ocean outfall (2) draft. The inflow load that can be removed through these countermeasures, in terms of BOD, is approximately the same at about 100 t/day.
- However, for the improvement of the bay water quality, countermeasure (3) is supposedly very effective, and costeffective in terms of maintenance and management.
- The previously mentioned countermeasure (1) alone will not be enough to attain the target bay water quality (to reduce the BOD in the load inflowing to the bay to 280 t/day). Hence, the primary treatment under the IDB/OECF Program intended for urban areas (614.8km2) outside the target area will be added to the secondary treatment under the same program. This is considered effective in possibly attaining the target bay water quality.
- The load reduction effects of the countermeasure involving the construction of retardation ponds is lower than the estimated value.

Even through the construction of a retarding pond totaling 9.4km² (water depth 2.0m), an annual mean load of only 10.4 tons/day (3%) can be effectively reduced.

(2010)

							and the second	
Cogstermeasure	method	reduced Volume (800 Load t/d	effect Impact extent)	direct effect	continuty	expenses construction expenses	O/M expenses	sabject tecknogical economic environmental
load reduction 1 2 3{B-1} 3{B-2} 3{B-3} 3{B-4} 4 River deciging Afforestation Release load sludge removal Froncts example circulation sidening of mayigational route		101 155 106 99 91 67 (13)			Δ Δ Θ Θ Θ Θ Ο Δ Δ Δ Δ	O A A A A A A	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	

O:Excellent

O:Good

△:Average

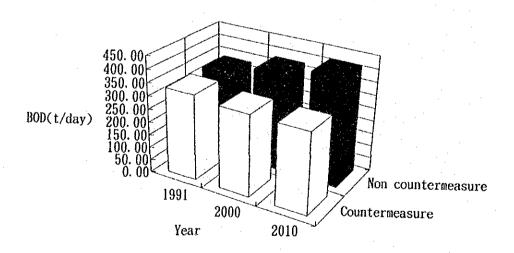


Fig. 6-4 Runoff Load of Area with Sewage Treatment and Area without Sewage Treatment

Table 6-1 Runoff Load with the IDB/OECF Program

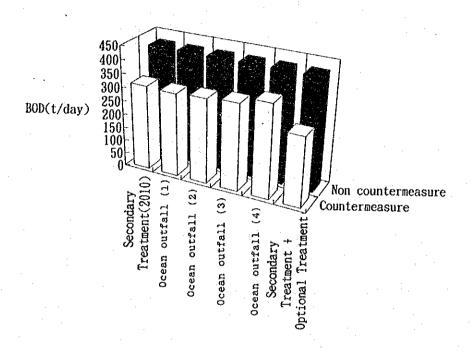
		Non-Severage	Primary Sewerage	Secondary Severage
Index		Treatment	Treatment	freatment
	<u> </u>	1991	2000	2010
Population	(persons)	7, 594, 031	8, 636, 028	9, 564, 782
	(%)	100	114	126
Discharge	(m3/s)	230. 16	247. 16	262.27
	(%)	100	107	114
BOD Load	(t/d)	330. 59	316.38	313. 87
1.0	(%)	100	96	95
TN Load	(t/d)	116.18	126. 45	135. 47
	(%)	100	109	117
TP Load	(t/d)	20. 37	22. 80	24. 96
	(%)	100	112	123

Table 6-2 Runoff Load with the Ocean Outfall Draft

Index	:	Non-Sewerage Treatment	Ocean outfall(1) with Sewerage	Ocean outfall(2) with Sewerage	Ocean outfall(3) with Sewerage	Ocean outfall(4) with Sewerage
		\	Treatment	Treatment	Treatment	Treatment
Population	(persons)	9. 564. 782	9, 564, 782	9, 564, 782	9, 564, 782	9, 564, 782
	(%)	100	100	100	100	100
Discharge	(m3/s)	262. 27	225. 78	231. 18	235. 70	253. 73
	(%)	100	86	88	90	97
BOD Load	(t/d)	415. 33	309. 58	316. 36	324. 02	348. 23
• .	(%)	100	75	76	78	84
TN Load	(t/d)	142. 76	114. 29	118.08	122. 49	136. 37
	(%)	100	80	83	86	96
TP Load	(t/d)	25. 68	20. 15	20. 94	21. 67	24. 42
	(%)	100	. 78	82	84	95

Table 6-3 Runoff Load with Retardation Pond Program

		Non-Severage	Retardation	Reduced
Index		Treatment	pond	Runoff Load
]	Program]
Population	(persons)	7, 594, 031	7, 594, 031	0
	(X)	100	100	0
Discharge	(m3/s)	230.15	_	-
	(%)	100	0	
BOD Load	(t/d)	330.59	320.16	10.43
	(%)	100	97	3
TN Load	(t/d)	116.18	116.07	0.11
	(%)	100	100	0
TP Load	(t/d)	20.37	-	-
•	(%)	100	0	



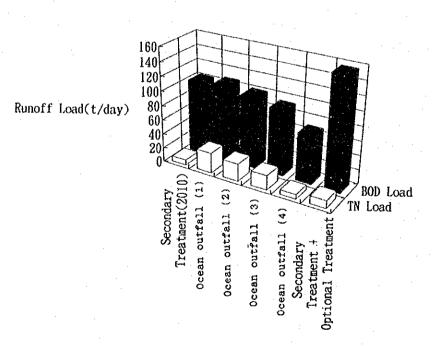


Fig. 6-5 BOD Load from the Basin with Countour Measurements and without Countour Measurements

Runoff Load from the Basin with Countour measures and without Countour measures Table 6-4

Name of Countermeasure	Year	Kunoff L	ad of Non-	Counterne Counterne	sure	Runoff Lo	ad by Coun	Runoff Load by Countermeasure		Keduction	Load by	ountermeas	ure
		Discharge	ischarge BOD Load IN Load IP LK	TN Load	TP Load	Discharge BOD Load	BOD Load	TN Load	TP Load	Discharge BOD Load TN Load TP	BOD Load	TN Load	TP Load
		(s/gu)	(£/d)	(t/d)	(t/d)	(s/gm)		(t/q)	(t/d)	(s/gm)	(t/d)	(t/d)	(t/d)
Non-Treatment(1991)	1991	230, 16	330.	116. 18	20.	230. 16		116.18	20.37	00 0	0.0	0.00	0.00
Primary Treatment(2000)	2000	247.16	375.40	130.24	23, 17	247.16	316.38	126.45	22.80	0.00	59.05	3.79	0.37
Secondary Treatment(2010)	2010	262. 27	415.	142. 76	53	262. 27	313.87	135. 47		6.99	101.46	7. 29	0. 72
Ocean outfall(1)	2010		415.		25				20, 15	36.49	105.75	28. 47	5.53
Ocean outfall(2)	2010	262. 27	415, 33	142. 76	ĸ	231.18		118.08	20, 94	31.09	98.97	24.68	4.74
Ocean outfall(3)	2010		415		25				21.67	26.57	91.31	20.27	4.01
Ocean outfall(4)	2010		415.				348. 23	136.37	24.45	8.54	67.10	6.39	1.26
Secondary Treatment + Optional Treatment2010	12010	262. 27	415, 33	142, 76	25.68	262. 27	260. 17	132, 00	24.63	0.00	155, 16	10.76	1.05

CHAPTER 7

THE WATER QUALITY OBSERVATION METHOD PROPOSED FOR TIDAL RIVERS

Chapter 7

The Water Quality Observation Method Proposed for Tidal Rivers

River water quality monitoring will be carried out based on the following two objectives: to maintain the river water quality standard and to determine the runoff load in Guanabara Bay.

The river observation stations established in this project are either located in non tidal or tidal zones. The observation stations of rivers discharging large water volumes and loads into Guanabara Bay are mainly located in tidal zones. The water quality and discharge of tidal rivers undergoes complex changes due to tidal effects, therefore, the establishment of monitoring stations within this area should be avoided.

In terms of accessibility, only a few stations, apart from those to be established, are considered as suitable monitoring sites. However, since water quality monitoring in tidal zones must be continuously carried out, the following observation methods are deemed appropriate.

7.1 Observation to Determine the Pollution Level in Rivers

The river water quality standard was established as the government's target to maintain the water quality appropriate for the use of the river.

It is, therefore, desirable to carry continuous observations on rainy days, once every month, 1 - 4 times a day.

The pollution level in tidal zones should is best measured by (1) selecting ebb tidal periods from the tidal level chart, (2) confirming that sea water influence is minimized a electric conductivity and salinity concentration meter should be used.

Although samples from non tidal rivers should be taken at a depth of 20% of the river depth from the water surface, it is necessary to take samples from several depths in tidal rivers in accordance with the degree of mixting of the water in the upper and lower layers.

For example, a total of 4 samples were taken from the upper and lower layers on the right and left banks of the Rio S.J.de Meriti, a comparatively deep and wide river, is very much desired to analyze water quality in terms of the degree the water from the upper and lower layers has mixed.

The proposed water quality measurement plan for each river (sampling frequency, sampling water level, lateral flow velocity)

7.2 Measurements to Determine Runoff Load in Clear Days

Due to the complex changes in the discharge and water quality in the tidal zones and the periodical use of the current, tidal influence can be removed by measuring the mean discharge and water quality in two tidal periods and in similar tidal phases, and by taking the mean of these two measurements.

Therefore, it is better to determine the load of tidal rivers through the quasi-constant analysis of the results of the 24 hour continuous measurement (continuous measurements carried out in the 2 tidal periods at an interval of 1 or 2 hours). There are specifically 3 methods used for analysis: the tidal prism method, the method using the mixing coefficient, and the method using the dispersion equation. Since these methods may not bring favorably accurate results, the numerical value of the non-constant dispersion equation method will be used for precise calculations.

As previously mentioned, the state of the water quality in the tidal zones, which varies due to the mixture of fresh and sea water, should be fully understood.

Generally, a weakly mixed type is indicated when the ratio of natural flow and tidal prism (difference between the water volume in the tidal zone during high tide and ebb tide) exceeds 0.7; a ratio ranging from 0.2-0.5 indicates a partially mixed-type, while less than 0.1 possibly indicates a well mixed type.

The calculation method that can be presently used is one for tidal rivers categorized under the well mixed type. Hence, the other mixture categories will either be treated like the well mixed type by using the mean concentration of salinity measured at different depths, or will be calculated as being made up of 2 layers which are divided by the boundary of salinity discontinuity.

The mean flow and load discharged into Guanabara Bay will be actually determined by calculating the flow and load of the 9 large tidal rivers, starting with the Rio S.J.de Meriti. The tidal level disparities of these rivers will be calculated by carrying out 24 hour measurements on clear days (continuous measurements in the two tidal periods at 1 or 2 hour intervals) in average periods, once in the rainy season and again in the dry season; the measured value should be subject to quasi-constant analysis. Recording all sampling times is, therefore, very important.

7.3 Measurements to Determine the Runoff Load on Rainy Days

The runoff load on rainy days will be determined using the runoff load estimation model, and the method to be used to determine

regression is simple and accurate (see Fig.7-1).

In this survey, the Rio Acari and Rio Macacu were selected as the urban and natural type model rivers, and the relation of the urban area ratio and runoff load was determined. In the future, the relation of the urban area ratio and runoff load should be further studied by carrying out observations on clear and rainy days in model rivers (non tidal rivers) whose land use conditions are neither urban nor natural, but somewhere in between.

It is also important to determine the runoff load in the first flush which occurs due to shift in seasons.

The following points should be heeded upon in the water quality analysis on rainy days:

- (1) The concentration of pollutants is well known to be high in the first stage of rainfall. Therefore, a 15 minute interval should be taken for observations in medium and small size rivers, 30 60 minute interval for large rivers; after the appearance of the peak flow volume, measurements in medium and small size rivers should be carried out at an interval of 30 60 minutes and 1 3 hours for large rivers.
- (2) As a rule, measurement on rainy days should be continuously carried out from the time directly prior to rainfall until the flow has returned to the amount it was before rainfall.

7.4 Measurements to Determine the Annual Runoff Load

Annual runoff load will be determined by summing the runoff load on clear days and rainy days.

The load discharged in Guanabara Bay should be measured monthly and seasonally as water quality in the bay is considered to also vary monthly and seasonally.

The Proposed Water Quality Measurement Plan for Each River Table 7-1

-			~	-		*****				-			_	-					-	-			_	-	-				
Sampling Depth		(No of Boints)	1								~	-							· -				2						
Discharge Measurement	Number of Measurement	lines for Velocity	5	dr I	2	2	-	بد	2	က	7	ന	-	2		T .	62	us.		2	63	2	വ	**	69			67	
	Regular Measurement	(A) = 0 = 0 = 0	Conce a month.	77	12	12	12	12	12	12	12	12	12	12	12	12.	12	12	12	12	12	12	12	12	12	21	12	12	
Frequency of Measurement	Hourly Change	Weasurement	(10 times Within & day)	1						2	2							7			2	2	2		2		2	2	
	NO.		ļ	~i	'n	9	œ	∞)	80)	6.	5	10-3	10-6		12	£.3	***	16	16-2	16-3	17-15	17-6	19	19-2	20	20	2.1	23	
Covered	Basin Area	(Km2)		7.40	3.40	11.60	58, 50	5.50	11.80	758.40	1233.70	256.00	45.20	4.60	107.00	8.40	53.20	342.50	139.00	186.00	544.20	159.80	163.50	57.90	27.30	1	60.50	42.80	3604.10
	Name		CANAL CANTO DO DIO	CARAL CARIO DO KIO		RIO IMBOASSU	RIO ALCANTARA	RIO MUTONDO	RIO GUAXINDIBA	RIO CACEREBU	RIO GUAPIMIRIM	RIO MACACU	RIO SOBERBO	CANAL DE MAGE	RIO RONCADOR	RIO IRIRI	RIO SURUI	RIO ESTRELA	RIO INHOMIRIM		RIO IGUACU	RIO SARAPUI	RIO S. J. DE MERITI	R10 ACARI	RIO IRAJA	CANAL DO PENHA	CANAL DO CUNHA	CANAL DO MANGUE	TOTAL
	No		401100	081.3	2 BM760	3 1B810	4 AN740	5 MT820	6 GX720	7 CC622	8 GP600					13 1R540	14 SR500	15 ES400	*16 IN460	*17 SC420	18 IA260			#21 AC241	22 11200	23 PN180	24 CN100	25 MN000	

Notes; twice a year (once during dry season, once during rainy season)

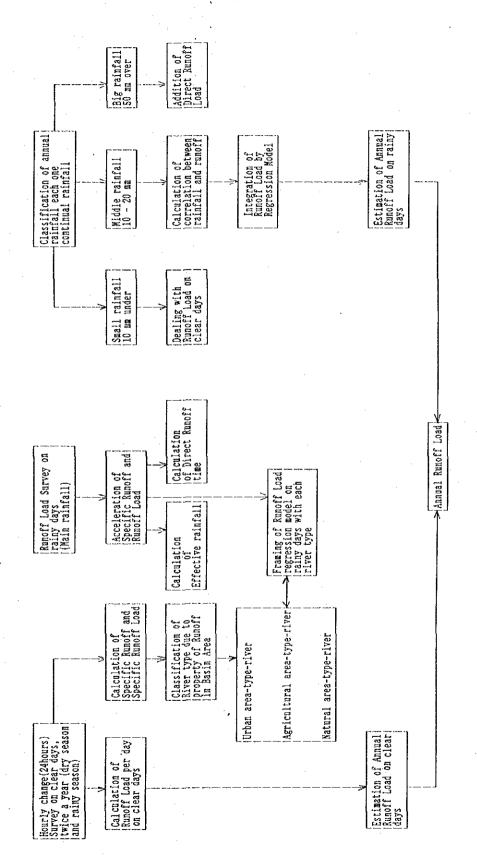


Fig. 7-1 The Proposed Runoff Load Estimation Model

APPENDIX

APPENDIX 1

RESULTS OF REGULAR SURVEY ON THE MAJOR 25 RIVERS ON CLEAR DAYS

Table APP. 1-1 Mean Water Quality of Regular Survey (May 1992 to Apr. 1993)

															(unit: mg/l)	
Covered Basin Area		Basin AreaPopulat	ig	PopulationLand use Density Type	and use	p-d	7		4	2	9	1	∞	5	Mean Value	Class
(Km2)	_	(Kn2)		(5/km2)				1992	١.,				1993			
					_	MAY	5	YUG	ES.	ठि	VOV	題	KAR	APR		
7.40	2	7.40	41.745	<u>z</u>	Urb/S. T	57	92	20	\$	94	4	40	99	18	26.0	
3.40	2	26.20	183, 099	66 g	Urban	8	105	40	40	100	48	130	8	200		
11.60		30.80	138, 636	4.50	Urban	12	2	4	2	∞	œ	ဖ	**	25		
58.50	<u>~</u>	144.60	470, 420	3.25	Urban	140	150	ı	150	380	140	*2900	096	8		
5.50			•	3, 25	Urban	40	116	30	40	06	22	20	45	70		
11.80	۰			3, 25	Urban	1.2	ch	20	ঘ	12		∞	18	20	_	
758. 40	6	846. 70	336, 193	0, 40	N/A	20	w	12	2	60	2	9	12	91		
233. 70	10	1253. 10	69, 853	0.06	. Y/N	00	6,5	<2.0	(2.0	7		(2.0	2	**		
256.00	10-3	256.00	18, 577	0.02	N/A	\$	25	0.0	65.0	<2.0	0	69	62.0	\$ 62.0		
45.20	9-01	132, 40	17, 911	0.14	N/K	96	160	120	13	8	2	S	13	12		
4.60	=	18.30	8, 458	0.46	N/A	42	20	S	16	12	8	40	12	30		9
107.00	12	111.40	36, 370	0.33	N/A	(2	*7	m	\$	(2.0	67	620	200	<2.0		
8.40	13	27.80	10,684	0.38	N/A	673	"	4	4	12	12	es	4	01		
53.20	4	68.80	12, 910	0.19	W/A	m	9	9	\$	65.0	2	r.	613	63		
342.50	16	342.50	302. 495	0.88 0.	K/A	9	12	40	4	ន	4	10	20	82		
139.00	16-2	139.00	84, 106	0.61	N/A	2	67	4	75	er)		2	74"	(2.0		
186.00	16-3	186.00	194, 173	1.04	N/A	13	rt.>	8	2	50	m	t ~	01	9		
544. 20	17-15	562, 80	758, 010	1.35	N/A	60	12	8	9	10	∞	ø	∞	4		
159.80	17-6	165.50	1, 012, 275	6. 12	Urban	24	KS KS	40	20	30	16	90	20	30		
163.50	19	164. 50	492. 158	9. 07	Urban	20	88	40	16	01	16	12	40	30		
57.90	19-2	57.90	438, 076	7.57	Urban	40	88	•		40	9	16	30	S		
27.30	8	35. 70	500, 276	14.01	Urban	06	83	30	20	99	25	91	40	69		
,	20					40	117	30	12	70	44	12	20	02		
60.50	21	63.60	815, 389	12.82	Groan	20	83	52	7	8	44	202	45	120		
42.80	:	42.80	500, 876	11. 70	Urb/S. T	2	88	20	14	30	92	30	30	65		
3604. 10	-	3912. SO B	1 FON 147		-											

Class 1: 3mg/l or less 2: 5mg/l or less 3: 10mg/l or less 4: 10mg/l more

Table APP. 1-1 Mean Water Quality of Regular Survey (May 1992 to Apr. 1993)

(COD(Cr)) · · · (1992 - 1993)

£	Name	Covered Basin Area	Š	Basin AreaP	opulationP tumber	0	nkand use Type		23	ന	¥	5	9	7	8	Ø)	Kean Walue
		(Kn2)		(Km2)		\sim				1992	\sim				1993		
-				****			<u>. </u>	MAY	35.	YEG	섫	ß	NOV	FEB	KAR	APR.	
1 CI 780	CANAL CANTO DO RIO	7.40	2	7.40		.v.	Urb/S. T	88	51	100	93	130	02	7.5	102	75	74.3
2 BK760		3.40	us	26.20	183, 099		Urban	8	119	110	140	360	210	320	8	280	191.0
3 18810	RIO INBOASSU		9	30.80			Urban			•	,		,		1440		1440.0
4 AN740	RIO ALCANTARA	58.50	∞	144.50			Urban	140	249	•	320	925	52				337.8
S XT820			00				Ürban	69	124	120	140	360	8	110	8	160	125.9
6 CX720	RIO GUAXINDIBA	11.80	œ		-		Urban	83	83	20	8	09	40	8	9	55	44.0
7 00622	RIO CACEREBU	758.40	ത	846.70	336, 193		N/A	25	91		40	30	40	ŝ	40	40	35. 2
8 GP600	RIO GUAPINIRIN	1233. 70	22	1253.10	69.853		N/A	G,	10	10	出	15	13	2	30	8	20.4
#9 MC967	RIO ENCACU	256.00	10-3	256.00	18 577		W/A	9	7	<10	12	010	12	0.0	2	010	10.0
*10 SB998	RIO SOBERBO	45.20	9-01	132.40	17. 911		N/A	210	230	022	8	53	20	8	53	30	114.4
11 MG580	CANAL DE MAGE	4.60	Π	18.30	8.458		N/A	081	73	S	8	22	70	8	40	120	83.2
12 RN560	RIO RONCADOR	107.00	12	111.40	36, 370	83	N/A	11	91	8	2	82	15	9	22	14	14.5
13 18540		8, 40	13	27.80	10.684	_	N/A	- 19	ន	1	8	65	25	S	35	40	45.5
14 SR500	RIO SURUI	53.20	14	68.80	12, 910	_	¥/¥	25	91	င္တ	53	20	13	8	15	16	23.2
15 5550	RIO ESTRELA	342.50	16	342.50	302, 495	·	N/A	88	-		-		30	20	40	40	33.6
*16 1N460	RIO INHOMIRIA	139.00	16-2	139.00	84, 106		K/A	13	တ	8	23	8	80	20	95	10	15.2
117 \$5420	RIO SARACURUNA	186.00	16-3	186.00	194 173		N/A	38	90	40	9	08	15	22	2	SS	27.1
18 1A260		54.20	17-1-5	562.80	758 010	_	N/A	45	51	•	9		40	89	22		37. 1
19 87300	RIO SARAPUI	159.80	17-6	165.50 1	. 012 275		Urban	38	6	100	75	75	99	70	20	140	2 89
20 \$1220	RIO S. J. DE MERITI	163.50	19	164.50	. 492, 458	_	Urban		43	430	ន	220	170	99	40	140	144.8
#21 AC241	RIO ACARI	57.90	2-61	57.90	438, 076		Urban	6.5	25			110	20	8	20	130	71.9
22 11200	RIO IRAJA	27.30	8	35. 70	500. 276	14.01	Urban	06	74	66	9	210	120 (20	40	190	104.9
23 PN180	CANAL DO PENHA	•	20	1				Z	126		8	320	06	ଛ	09	210	121.6
24 CN100	CANAL DO CUNHA	60,50	21	63.60	815, 389	12.82	Urban	88	22	100	23	180	90	22	45	220	98. 7
000		42.80	23	42.80	500, 876	2	Urb/S:T	20	18	130	90	185	160	130	.09	185	115.7
	TOTAL	3604. 10		3912.50 6	. 690, 147	•				l	. 		-				

Table APP. 1-1 Mean Water Quality of Regular Survey (May 1992 to Apr. 1993)

· (COD(Mn))

																	(MIT: EK/I)	_
,2	Name	Covered Basin Area	Š.	Basin Area	Population Number	Population Density	Land use Type	-	63	თ	.a.	ıC	မာ	۲	∞	თ	Kean Value	
		(Km2)		(Kn2)		(p/km2)	L			199	2				1993			
								KAY.	NS.	AUG	SEP	53	Ω	FEB	EAR	APR		
1 C1780	CANAL CANTO DO RIO	7.40	2	7.40	41,745	5,64	Urb/S. T	9.3	10.5	10.2	10.2	11.6	10.0	4.0	23.0	11.0	11,1	_
2 B¥760	RIO BOMBA	3.40	ر. دی	26. 20	183, 099	\$6 \$6	Urban	20.2	32.5	63	11.8	26.0	တ်	62.0	11.8	11.4	25.0	
3 IBS10	RIO IMBOASSU	11.60	G	88.88	138, 636	58.	Urban	11.3	တ်	10.6	12.6	10.0		9.6	9.4	23. 0	11.5	
4 AN740	RIO-ALCANTARA	58.50	ထ	144.60	470.420	33	Urban	21.5	23. 5	1	92.0	116.0	460.0	620.0	11.6	22. 8	170.9	
5 MT820	RIO MUTONDO	5.50	6			3.23	Urban	17.5	26.5	80	11.0	50.0	11.0	22.0	11.0	21.6	20.0	
6 GX720	RIO GUAXINDIBA	11.80	ø.			3.25	Urban	 ග්	co.	9.0	10.4	11.2	9.6	11.2	14.8	10.0	10.5	
7 00622	RIO CACEREBU	758, 40	თ	846.70	336	0.40	N/N	တ တ	6.5	& <u>.</u> ∞	10.4	10.0	9.4	9.0	12.2	10.4		
8 GP600	RIO GUAPINIRIM	1233. 70	10	1253. 10	69	0.06	N/A	6.9	n)	رن ص	9.6	7.2	ικί	3.4	7.2	7.4	 	
*9 MC967	RIO MACACU	256.00	10-3	256.00	<u>ജ</u>	0.07	N/A	2.1	1.9	1.8	4.00	3.0	1.8	2.2	ις 80	0.8	2.5	
30 SB998	RIO SOBERBO	45.20	10-6	132. 40		0. 14	N/A	32. 5	52.5	36.0	10.2	11.8	10.4	2.0	9.2	7.0	21.5	
11 46580	CANAL DE MAGE	4.60	=	18.30	8. 458	0.46	N/N	25. 5	24.5	10.2	22.0	10.6	10.2	11.4	80 61	21.0	16, 1	_
	RIO RONCADOR	107.00	2	111.40	33	رن دي	K/A	4.5	63 1	4.0	4.	6.6	ლ ლ	4.0	5.0	44	4.4	_
13 IR540	RIO IRIRI	8.40	55	27.80	10.	0.38	N/A	21.5	12.5	တ်	11.2	12.0	11.0	10.6	on C1	12.0	12.1	
	RIO SURUI	23. 20	14	88 88	15	0.19	N/A	 c>	મડે જ	5.6	4.6	% 0	7÷	5.2	80	5.4	다	_
	RIO ESTRELA	342, 50	91	342.50	302		N/A	7.9	9. S	14.0	9.2	-1	بن 4	0.6	7.6	2.4	 00	_
16 IN460	RIO INHOMIRIM	139.00	16-2	139,00	≈.	0.61	N/A	63	ල ල්	4.8	4.0	6.4	2.2	3.0	4.0	2.4	3.7	
17 8028	RIO SARACURUNA	186.00	16-3	186.00	194.	2	N/A	5.5	2.7	8.6	2.6	7.0	1.0	2.2	3.0	5.6	63	-
18 14260		544. 20	17-15	562.80	758.	.35	N/A	 ∞	7.7	16.0	10.6	0.6	 	7.2	7.0	5.6	∞;	
19 SP300	RIO SARAPUI	159.80	17-6	165.50	1.012.	6, 12	Urban	6.9	10.3	20.0	10.4	9	9.2	10.4	9.6	20.0	12.1	
20 \$7220		163, 50	13	164.50	1,492	9.07	Urban	1 -66	49.5	24.0	10.0	2.5	24.0	10.2	တ <u>ဲ</u> နှ	15. O	17.0	_
	RIO ACARI	57.90	19-2	57.90	438	7. 27	Urban	60	21.5	•		11.2	0:1	10.4	7.0	19.5	11.4	
	RIO IRAJA	27. 30	8	35. 70	500.	14.01	Urban	21.2	19.5		10.0	9.7	10.8	11.2		22.0	15.3	
	CANAL DO PENHA		82					10.0	28.5	28.0	10.4	9.0	10.4	10.6		22.0	15.7	
24 CN 100	8	60.50	53	63.60	815.	12, 82	Urban	-1 00	10.5		- ∞ σi	4.6	10.4	10.0		27.0	11.7	
25 KN000	CANAL DO MANGUE	42.80	23	42.80	500.876	11.70	Urb/S. T	10.1	25.5	14.0	ري دي	12.0	10.6		-1	18,4	13.3	
	TOTAL	3604. 10		3912.50	6.690.				-] 							_

Table APP. 1-1 Mean Water Quality of Regular Survey (May 1992 to Apr. 1993)

(1992 - 1993)

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		- 1						1									(unit : mg/l
Covered	· · · · · ·		Basin AreaP	_ 🕰 🤄		pulationPopulationLand	and use		64.	(1)	₩.	.	φ	r~	ø	ø	Mean Value
Name Basin Area NO. (Kn2)	MJ. (KAZ)	(K (K (K	Z	<u> </u>	i S	(5/kg2)	<u>8</u>		-{ 	7661	75				1993		
							<u>i</u>	MAY	JUN	YEC	SEP	동	20	E.	KAR	APR	_
\$	\$	\$	\$.s.	Urb/S. T	7. 65	17.11	20.05	10 51	22. 12		13.08	12.13		14.6
ន	ន	ន	ន			6.39	Urban	8. 02	40.05	24. 13	15. 45	28.07		34.05	17.21		23.4
RIO IMBOASSU 11.60 6 30.80 1	8	8	8	_,	38, 636	4.50	Urban	3, 16	9, 08	£	2.44	4,05		83.1	3, 10		4.0
99	99	99	99	-7"		3.25	Urban	5.50	22 08	70.14	11. 60	15.08		35,00	21.01		24.8
RIO MUTONDO S. 50 8	5.50 8	•				3.25	Urban	21. 20	30.07	20.17	13.40	23.06		23.02	9.30	15.01	20.2
					_	ĸ	Urban	6.09	15. 07	16.09	6.95	4.60		16.03	S. 50		, ç,
758.40 9 846.70	9 846. 70	ļ	ļ		336, 193	9	K/X	1.21	2.13	1.43	2 23	1.76		1.05	1.44		
10 1253.10	10 1253.10	_	_		69.853	8	N/A	0.90	0.36	0.75	96 0	1.36		0.46	1.06		ර ර
10-3 256.00	10-3 256.00	256.00			18, 577	3	N/A	0.75	0.51	0.75	0.55	6.3	:	0.41	1.30		
RBO 45. 20 10-6 132. 40	10-6 132.40	132.40			17. 911	Z	N/A	1.04	2. 15	:55	1. 42	1.31		0.52	0.81		1.2
JE 4.60 11 18.30	11 18.30				8, 458	46	N/A	12.03	3.82	18.04	7 15	8.09		10.00	7.00		60
ADOR 101.00 12 111.40	111.40				36, 370	83	N/A	0. 62	1. 12	. 53	0.81	0.71		0.81	0.32		8 0
IRIRI 8.40 13 27.80	13 27.80			_	0.684	88	N/A	1. 43	1. 91	1. 29	0.91	4.31		1.46	0.44		
14 68.80	14 68.80				12, 910	13	N/A	0.93	0.91	1.31	0.60	1. 46		1.01	0.37		60
342.50 16 342.50	16 342.50			Š	02, 495	88	N/A	2 2	4.09	2.	1. 32	2. 42			1.08		2.4
RIM 139.00 16-2 139.00	16-2 139.00	139.00		_	8, 166	19	N/A	1. 63	1.77	2.06	1.31	1.72		1.24	3.86		
16-3 186.00	16-3 186.00	186.00		<u>=</u> :	24. 173	ਤ	N/A	2. 48	2.30	2.27	1.41	3.01		1.63	4.25		23
544.20 17-175 562.80	17-1-5 562.80	562.80		<u></u>	58, 010	35	N/A	2. 43	4. 15	10.04	33	5.02		4.03	00.9		4.8
159.80 17-6 165.50 [1.	17-6 165.50 JJ.	165.50 [1.	ı.	-	12, 275	22	Urban	20.16	24: 03	10. 18	10.40	10.01		10.08	15.05		14.5
19 164. 50 [].	19 164. 50 [].	164. 50 [].			92, 458	5	Urban	20.07	17. 10	13.03	6.90	10.04		9.03	13.01		13.0
57. 90 19-2 57. 90	19-2 57.90	57.90		4	38, 076	7.57	Urban	9.03	18.21	,	ı	11.04		10.03	8.06		11.1
27.30 20 35.70	20 35. 70	35. 70		ĭñ	00, 276	14.01	Urban	24. 11	21.04	10.46	8.00	14.05		14.01	9.12		13.7
20	20		,					22, 10	19 56	10. 22	9.60	21.04		13.01	7.04		14.3
60.50 21 63.60	21 63. 60	63.60			815, 389	12.82	Urban	16.10	14.06	10.50	8.80	17.03		10.04	5.06		12.3
L DO MANGUE 42.80 23 42.80	23 42.80	42.80	42.80	_	500.876	5	Urb/S. T	19.09	12.51	8:41	8.35	12.06		10.40	13, 02		12.1
TOTAL 3604.10 : 3912.50 6.	1 3912, 50 6.	1 3912, 50 6.	3912, 50 6.	ø				-									

Table APP. 1-1 Mean Water Quality of Regular Survey (May 1992 to Apr. 1993)

Table APP. 1-1 Mean Water Quality of Regular Survey (May 1992 to Apr. 1993)

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<u></u>		1		·														,					, -						r
(unit : mg/1)	Kean Yalue																										9 0		
``	Ø		VPR.																								0.3		
	ø	1993	KAR																								0.2		
	r-		FEB	1.8	ဆ	4.3		1.5	0.6	2.6	(r)	43	1,4	9.0	တ က	 9	2.9	3.2	1.7	1.6	1,1	-0	2.2		6.0			1.2	
	w		YOY																								0.2		
	w		8																								o		
	ব	992	SEP											0.9										•			2.0	1.0	
	617	190	NG											0.4													0		
	67		101	65	1.8	2.0	63	0,5	0.4	ب. د.			1.3	0,	6.	5	1.7	O. 33	1.7	۲- دغ	() ()	9	c)	0.3	0,3	0	0,4	0.5	
Ì			KYX	5.5	2.5	2	4.5		0.3	0.4	c.;	5.	5	2.4	(1)	60 C	5.0		80 63		co co	cò	4,0	તે જ	es d	Ö	0.2	0.6	
	Land use Type		4	Urb/S. T	Urban	Urban	Urban	Urban	Urban	N/A	N/A	R/A	К/А	Y/*	N/A	N/A	W/A	N/A	N/A	N/A	N/A	Urban	Urban	Urban	Urban		Urben	Urb/S.T	
	8	(5/km2)	_	35	6. 99	4.50	3. 25	3. 25	3.25	0.40	0.06	0.07	0.14	0.46	0.33	88	0.19	0.88	0.61	1.04	1.35	6. 12	9.07	7, 57	14, 01		12.82		
			•				470, 420							8, 458													815, 389	8	9
	Basin Area	(Km2)		7.40	26, 20	30, 80	144.60			846. 70	1253, 10	256.00	132, 40	18.30	111.40	27.80	68, 80	342.50	139,00	186.00	562, 80	165.50	364. 50	57.90	35. 70		63. 60	42.80	3912, 50
	9			2	2	9	00	00	00	œ	91	10-3	9-01	=	12	23	7	9	7-91	16-3	17-1 5	17-6	57	19-2	83	8	77	23	
	Covered Basin Area	(KE2)		7, 40	3.40	11.60	58, 50	5.50	11,80	758.40	1233, 70	256.00	45, 20	4, 60	107.00	8.40	53, 20	342.50	139, 00	186.00	544, 20	159, 80	163.50	57, 90	27.30	1	60, 50	42.80	3604, 10
	Name		O.E.	CANAL CANTO DO RIO	RIO BOMBA	RIO IMBOASSU	RIO ALCANTARA	RIO MUTONDO	RIO GUAXINDIBA	RIO CACEREBU	RIO GUAPINIRIN	RIO MACACU	KIO SOBERBO	CANAL DE MAGE	RIO RONCADOR	RIO JRIRI	RIO SURUI	RIO ESTRELA	RIO INHOMIRIA	RIO SARACURUNA	RIO IGUACU	RIO SARAPUI	RIO S. J. DE MERITI	RIO ACARI	RIO IRAJA	CANAL DO PENHA	CANAL DO CUMBA	CANAL DO MANGUE	QUYT_
	,2			1 CI780	2 BM760	3 1B810	4 AN740	5 IIT820	6 CX720	7 00522	8 09600	#8 MC967	*10 SB998			_		15 55400	*16 IN460	*17 SC420		SP300	SJ220	AC241	17200	28:K	·	NN000	

Table APP. 1-1 Mean Water Quality of Regular Survey (May 1992 to Apr. 1993)

(Ka2) (Kaz) (Kaz) <th< th=""></th<>
S.T 300 1992 NOV FEB 44AR APR S.T 3000 5600 5600 5600 2400 2400 2400 R. S. T 5000 5600 5600 5600 2400 2400 300 R. S.
5. 64 Urb.N.S. T. XiV.Y. JUN AUG SEP OCT NOV FES AUR AUR 6. 59 Urban \$000 >160 9000 2400 9000 2400 9000 2400 9000 2400 9000 2400 9000 2400 9000 2400 9000 2400 9000 2400 9000 2400 9000 2400 9000 2400 9000 2400 9000 2400 9000 1700 3000 1700 <t< th=""></t<>
5. 64 UrbAn 5. 60 UrbAn
6. 99 Urban 56000 5000 >160 16000 2400 50000 2400 2400 50000 24000 24000 50000 24000 50000 1700 50000 1700 50000 1700
4. 50 Urban 800 90 240 500 600 900 500 500 500 500 500 500 500 500 500 500 500 500 500 170 500 500 500 500 170 500 170 500 500 500 170<
3. 25 Urban Urban 22000 13000 13000 50000 50000 50000 50000 50000 50000 50000 50000 50000 50000 50000 50000 17000
3. 25 Urban Urban 30000 13000 >160 9000 5000 30000 17000 2. 25 Urban 300 170 5 - 30 50 50 80 30000 170 22 23 13 22 13 13 23 23 13 13 23 23 13 13 23 23 13 13 23 23 13 13 23 23 13 23 23 13 23 23 13 23 23 13 23 23 13 23 23 13 23 23 13 23 13 23 13 23 23 23 24 50 20 24 50 20 20 20 20 23 23 23 23 23 23 23 240 240 240 240 240 240 240 240 240 240 240 240
3. 25 Urban 300 170 50 50 50 300 170 23 25 23 170 </td
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0.06 N/A 24 - 30 500 5 23 130 50 230 130 5 230 130 50 230 130 6 130 230 130 5 230 130 230 130 230 130 2200 <t< td=""></t<>
0.07 N/A 300 - 2400
0.14 N/A 24000 - >160 110 80 50 1600 2200 0.38 N/A 50000 240 >160 230 1600 16000 5000 50 1600
0. 46 N/A 90000 2400 >1600 16000 50000 C1600 0. 33 N/A 230 22 50 130 50 80 80 0. 38 N/A 30 1300 24 230 300 50 80 80 0. 18 N/A 30 130 24 240 300 80 30 80 0. 61 N/A 300 - 160 230 240 300 300 300 300 0. 61 N/A 300 - 160 230 240 300 300 300 300 1. 04 N/A 3000 - 110 50 2240 300
33 N/A 230 22 50 50 50 50 50 80 38 N/A 50 23 300 300 80 50 50 50 80 N/A 300 - 180 24 50 240 50 50 81 N/A 300 - 160 80 - 2400 300 300 61 N/A 230 - 160 80 - 130 220 240 300 M/A 230 110 50 130 - 130 220 300 M/A 230 - 180 220 2400 300 300 MA 3000 - 180 240 2400 300 300 MA 3000 - 180 240 2400 300 300 MA 3000 - 180 2400 300 18
0.38 N/A 50 23 300 300 80 30 <t< td=""></t<>
19 N/A 30 1330 24 250 240 50 88 N/A 230 - 166 23 - 240 300 61 N/A 230 - 166 23 - 130 230 100 55 N/A 230 - 160 230 - 130 230 110 55 N/A 3000 - 160 230 340 230 130 55 N/A 3000 - 160 2300 130 230 130 71 Urban 13000 - 160 2400 1300 1300 77 Urban 13000 - 160 2400 1300 1300 10 Urban 30000 - 160 2400 1600 3000 10 Urban 30000 - 16000 16000 21600 3000 10
88 N/A 300 - 160 80 - 240 300 800 64 N/A 230 - 160 230 - 130 800 800 94 N/A 230 110 50 110 50 230 110 12 Urban 50000 - >>160 8000 2400 2400 8000 57 Urban 50000 - >>160 80000 - 24000 8000 57 Urban 50000 - >>160 80000 - 24000 3000 57 Urban 50000 - >>160 80000 - 24000 3000 57 Urban 30000 - >>160 80000 61600 3000 10 Urban 30000 - >>160 23000 16000 3000 10 Urban 13000 - >>160 30000 61600 3000 10 Urban 13000 - >>160 2000
1. 04 N/A 230 2400 >160 230 - 130 800 300 300 300 300 300 300 300 300 3
1. 04 N/A 230 110 50 130 - 130 230 110 110 50 130 - 130 230 110 110 133 110 110 110 110 110 110 1
NA 3000 - 160 2300 2400 2300 2500 2500 2500 2500 2500 2500 2500 2500 2500 2500 25000
12 Urban 50000 - 160 8000 8000 1700 24000 13300 07 Urban 13000 - 160 80000 - 24000 8000 28000 28000 28000 28000 28000 28000 28000 28000 28000 28000 28000 28000 28000 28000 28000 28000 28000 30000 18000 18000 18000 28000 30000 30000 3000
9.07 Urban 13000 - >160 80000 - 24000 8000 28000 28000 2,000 14.01 Urban 50000 - >160 80000 1,0000 160000 160000 16000 1,00000 160000 160000 1,00000 160000 1,00000 160000 1,000000 1,0000000000
57 Urban 50000 - 23000 - 30000 616000 30000 616000 30000 9000 01 Urban 13000 - >160 30000 616000 9000
14. 01 Urban 30069 - >160 230000 160000 616000 9000 12. 82 Urban 13000 - >160 30000 9000 51600 9000 11. 70 Urb.S. T 3000 - >160 50000 61600 3000 11. 70 Urb.S. T 3000 - >160 50000 16000 1300
82 Urban 13000 - >160 30000 90000 016000 90000 3000
12.82 Urban 13000 - >160 13000 9000 30000 616000 3000 11.70 Urb/S.T 3000 - >160 50000 - 50000 16000 1300
11.70 Urb/S.T 3000 - >160 50000 - 50000 16000 1300

*:Irlbutary river(excluded Irom Total a N/A : Natural and Agricutural use Urban : Urban use Urb/S.T: Urban Use with Sewage Treatments

Table APP. 1-2 Mean Runoff Load of Regular Survey (May 1992 to Apr. 1993)

Discharge . (1992 - 1993)

Wean Value	ļ		1.0	0.1	60 60	0.1	0.2	0.1	35.2	53.5	∞ ∞å	بم سا	0.5	က တဲ	0.5	4.4	32.8	2.7	3.0	43.1	24.0	31.7	7.0	3.0	ij	တ	5.1	
		% 7.68	0.880	0.035	1. 292	0. 121	0.194	0.297	35, 595	55 268	8.561	2.020	502	6.028	0.142	3, 655	22, 322	2. 429	3, 935	27 403	14.461	33.887	7.496	1. 158	2 095	5.039	3. 620	213.76
eo	1993	MAR	1.007	0.060	6. 492	0. 121	0.110	0. 131	61.415	133.665	12.040	1. 990	0.439	10.930	. 385	7.504	40.166	2.940	2.430	36.044	26.463	28.905	5. 786	3.968	0.681	9.873	8.605	378. 48
2		Ē	0. 735	0.095	5.481	0.036	0 122	0.013	21. 251	31, 239	8. 137	0.832	0.542	4. 285	0.293	2 490	47. 778	1 762	2. 405	37.904	28. 230	9. 206	6, 955	2,343	0.880	3.852	3.717	201.99
ယ		NOV	0. 792	0. 162	5. 520	0.066	0.398	0.127	64. 627	76.334	17.154	2.651	0.608	18, 135	0.808	4. 138	27.846	4.566	3, 523	75. 754	43.830	53.595	6.367	3.546	0.921	17.692	8.686	403. 62
ıs		S	0. 590	0.129	3.557	0.081	0.159	0.063	22, 543	39.847	7. 330	1.054	0.747	11. 206	0. 605	5.418	54. 223	5. 772	3, 803	71.746	38. 994	56.067	6.661	3.062	1. 378	12, 927	5, 125	328. 47
\$	992	SE SE							-																			0.00
65	101	YIC													٠.												•	0.0
2) E	0.428	0.124	1.000	0.051	0.167	0.00	9.593	10.992	3.528	0. 721	0.440	2. 785	0.018	2.971	23.354	0.393	2 282	12, 163	9, 232	10.417	8. 504	5.367	0.682	8. 140	3.014	100.94
		XVX	2.378	0.109	2, 325	0.067	0.208	0.00	31. 146	27. 218	4. 605	1.019	0.166	5.043	0.062	4.943	14.065	0.936	2.514	40.871	6.919	29, 455	7.410	1.523	0.683	4.853	3.018	175, 05
Land use	S.	•	Urb/S. T	Urban	Urban	Urban	Urban	Urban	N/Y	N/A	W/A	N/A	. Y/N	N/A	N/A	N/A	N/A	N/A	N/A	N/A	Urban	Urban	Urban	Urban		Urban	Urb/S. T	
Population	(5/km2)		5. 64	6.93	ន	3.25	ĸ	3	10	90	62	4	46	83	88	5	88	5	2	铝	2	5.07	7.57	14.01		12.82		
AreaPopulationPopulationLand	TO ORDER				138, 636					59, 853																	500.876	
Basin Area	(KE2)				30.80					1253, 10																	42.80	
ş	2		7	'n	æ	∞	∞	e 0	ത	0.	-0-	9-01	Ξ	15	<u>.</u>	14	92	16-2	16-3	17-175	17-6	5:	19-2	8	2	21	23	
Covered	(Kn2)		7.40	3.40	11.60	SS. 50	5.50	11.80	758. 40	1233.70	256.60	45.20	4.60	107.00	8.40	53. 20	342, 50	139.00	186.00	544. 20	159.80	163, 50	57.90	27.30	•	60.50	42.80	3604.10
			CANAL CANTO DO RIO	RIO BOMBA	RIO IMBOASSU	RIO ALCANTARA	RIC MUTONDO	RIO GUAXINDIBA	RIO CACEREBU	RIO GUAPINIRIM	RIO MACACU	RIO SOBERBO	CANAL DE MAGE	RIO RONCADOR	RIO IRIRI	RIO SURUI	RIO ESTRELA	RIO INHOMIRIM	RIO SARACURUNA	RIO IGUACU	RIO SARAPUI	RIO S. J. DE MERITI	RIO ACARI	RIO IRAJA	CANAL DO PENHA		CANAL DO MANGUE	TOTAL
5	2		1 C1780	2 BK760	3 12810	4 AN740	5 MT820	6 GX720	7 00622	8 9960	#9 MC967	*10 SB998	11 MC580	12 RNS60	13 18540	14 SR500		*15 1N460	#17 85420	18 1A260	19 SP300	20 \$7220	±21 AC241	_			25 INV000	

Notes :: |) * Tributary river(excluded from Total amount)
2) AUG, SEP: Not measured of discharge
N/A : Natural and Agricutural use
Urban : Urban use
Urb.S. T: Urban Use with Sewage Treatments

Table APP. 1-2 Mean Runoff Load of Regular Survey (May 1992 to Apr. 1993)

00g) · .

sean Yalue			2.5	0.8	2.6	9;	1.0	0.1	29.0	12.2	1.2	4.4	0.1	1.4	0.3	1.2	40.6	0.6	2.4	30.1	47.3	57.9	22.7	14.4	2 's	44.6	22. 5					
S)		A.P.R	1.37	0.40	2.79	9 9	1. 17	0.51	49. 21	19. 10	1.48	2.02	0.54	1.04	0. 12	0.63	38. 57	0.42	2.18	9.47	37. 48	87.84	32, 38	9. 20	12.67	52.24	20.33	342, 94				
00	1993	KAR.	5.22	0.41	23	10.04	0.43	0.50	63.68	27. 72	2.08	2. 27	0.46	1.89	0.66	2.07	69.41	1.02	2. 10	24.91	45.73	99.90	15.00	13.71	5.32	38.39	22.30	432.30				
۲		EE:	2.55	1.07	3.58	9.02	0.74	0.01	5.14	5.40	1.41	3. 59	1.87	0.74	0.08	1.12	41.28	0.37	. 50	20.98	42.90	10.06	9.61	3.24	0.91	6. 99	5, 63	167.97				
£		Š	0.27	0.67	3.83	0.80	0.83	0.01	11. 17	5. 28	0.39	2.98	1,05	2.5	0.84	0.72	9.62	0, 47	0.91	52.33	60.28	74.09	3.30	19.61	2.83	67.26	57.04	371.15				
ທ		8	2.04	1.11	2.46	2.66	1. 24	0.07	15.58	6,39	1. 27	1.82	0.77	1.94	0. 63	0.94	93. 70	1. 60	6.57	61. 99	101.07	48.44	23.02	15.87	 33	89.35	13, 28	468, 36				
4	100	SEP																										0.8				
ന	198	32																										0.00				
81		Š	0.96	1. 12	0.88	0.66	1.67	00	4.14	1. 90	0.61	9. 97	1. 90	0.96	0.01	1.64	24. 21	0.07	99	12. 61	27. 92	34.20	49. 96	30.14	6.83	37. 27	16.93	206. 02				
		λΥX	4.93	0.75	2.4]	0.83	0.72	0.00	53.82	18.81	0.80	8. 45	0.0	0.87	0.05	1. 28	7, 29	0.19	2. 82	28. 25	14.35	20.90	25. 61	11.84	2.36	20.96	18, 25	239. 24				
Land use Type	<u>.</u>		Urb/S. T	Urban	Urban	Urban	Urben	Urban	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	Urban	Urban	Urban	Urban	~	Urban	Urb/S. T					
Population Density	(5/Km ²)		5.64	6.93	5.5	3, 25	33.	3.25	0.40	0.08	0.0	0.14	0.46	0.33	0.38	0.19	0.88	0.61	~	 	6. 12	9.07	7.57	14.01		12.82						
n AreaPopulationPopulationLand use					138, 636				336, 193	69.853	18, 577	17, 911	8, 458	36.370	10.684	12, 910	302, 495	84, 106	194, 173	758.010	1, 012, 275	1, 492, 458	438.076	500, 276	-	815, 389	500.876	6, 690, 147				
Basin Area	(Km2)				30,80				846.70	1253.10	256.00	132. 40	18.30	111.40	27.80	68.80	342. 50	139.00	186.00	562.80	165.50	164.50	57.90	35. 70	•		42.80		ount)			
Š	•		2	'n	φ.	00	- 60	60	o,	91	10-3	9-01	=	13	<u>::</u>	7	16	16-2	16-3	11-11-2	17-6	13	19-5	23	8	53	. 23		n Total am	arge		
Covered. Basin Area	(Km2)		7.40	3.40	11.60	58, 50	5.50	::8	758. 40	1233. 70	256.00	45.20	4.80	107.00	8.40	53. 20	342.50	139.00	186.00	544.20	159.80	163.50	57. 90	27.30	'	60.50	42.80	3604.10	cluded fro	ed of disch	nse	reatments
Nane			CANAL CANTO DO RIO	RIO BOMBA	RIO INBOASSU	RIO ALCANTARA	RIO MUTONDO	RIO GUAXINDIBA	RIO CACEREBU	RIO GUAPINIRIM	R10 MACACU	RIO SOBERBO	CANAL DE MAGE	RIO ROWCADOR	RIO IRIRI	RIO SURUI	RIO ESTRELA	RIO INHOMIRIM	RIO SARACURUNA	RIO IGUACU	RIO SARAPUI	RIO S. J. DE MERITI	RIO ACARI	RIO IRAJA	CANAL DO PENHA	CANAL DO CURRA	CANAL DO MANGUE	TOTAL	: 1) * Tributary river(excluded from Total amount)	2) AUG, SEP: Not measured of discharge	: natural and Agricutural use	Urb/S. T: Urban Use with Sewage Treatments
<u>%</u>			1 CI780	2 BW760	3 18810	4 AN740	5 MT820	6 GX720	7 00622	8 GP600	*9 MC967	*10 SB998		12 RN560	13 18540	14 SRS00	15 55/00	*16 1N460	*17 %428	18 1,250					23 PN180		25 KN000		Notes : 1)	20.17%	5	Urb/S. T: Ur

A1-9

Table APP. 1-2 Mean Runoff Load of Regular Survey (May 1992 to Apr. 1993)

(COD(Cr))

												_		_						۰									
Kean Value			5.4	2	138.4	1.2	2.0	0.4	112.9	99.7	2 %	10.0	8.2	11.1	2.1	ග්	59.6	0.4	6.5	114.0	132.3	350.2	44.4	26.5	15.3	80.9	55. 1		
σ.		APR.	5. 70	2.23	160.75	0.00	65 88 88	1.41	123.02	143, 25	7. 40	5.24	2.17	7. 29	0.49	5, 05	77. 14	2.10	11.90	71.03	174.92	409.90	84.20	19.01	38.01	108.84	57.86	1410.76	
တ	1993	MAR	60.03	0.52	807. 71	0.00	0.57	0.45	212.25	346.46	10.40	2 28	1.52	14, 17	5. 76	9, 73	138.81	2.34	2 10	77.86	45. 73	88	88	33	3.53	38.39	44. 61	1867.75	
~		FEB	4.79	2.63	8	00.00	1.16	0.03	91.80	26.99	7.03	5.75	2.81	3.70	1.26	6.45	82.56	35	5. 13	212.87	170.74	50.35	18.03	14.17	1.52	16.64	41.75	732. 19	
ဗ		NOV	4.79	Ж ;	0.0	0. 14	3.03	0.44	223. 35	85 83 83	22.23	11.45	3.68	23. 50	3, 49	5.36	72. 18	es 33	4.57	261.81	227. 21	787. 20	27.51	36. 76	13.15	137.57	120.08	2025. 68	
3		αt	6.63	4 01	0.00	6.47	3.57	0.33	58. 43	51.52	6.33	6.83	4.52	19.36	3.40	9.36	0.00	14.96	9.36	0.0	252. 68	1065.72	63.31	55.55	38. 10	201.09	81.92	1862.74	
ਧਾ	2	SEP		-,			-	• •	•••	-		:													_		_	0.00	
က	199	YIC				_															-		•					0.00	
62		JUN	1.89	1.27	8	1. 10	1. 79	0.00	13.26	60.		14, 33	2.98	2 39	8	4.00	0.00	0.31	3.47	15.76	31.91	38.70	61. 72	33	93	40.09	21.09	229. 23	
		MAY	7.81	0.75	900	0.81	1.24	6.00	58.35	21, 16	23	23.77	2.58	7.41	0.33	22, 21	46. 18	S	:S	158.91	22 72	0.00	31. 37	11.88	3	23. 48	18. 25	417.22	
and use	1		Grb/S.T	Urban	Urban	Urban	Urban	Orban	N/A	V/V	- V/N	N/4	Y/3	 V/V	 {}		 	W.	N/A	- Y/N	Urban	Urban	Urban	Urban		Srban	Urb/S. T		
Population Density I	(D/km22)						3. 25									_								_		12.82	70	~	
reaPopulationPopulationLand		_			138, 636				336, 193	69, 853	18, 577	17.911	8, 458	36, 370	10, 684	12,910	302, 495	34, 106	194, 173	758, 010	012.275	492, 458	438, 076	500, 276		815, 389	500, 876	690, 147	
	(KH2)				30.80							132.40								562.80					,	63. 60	42.80	3912, 50 B	(100
Ş.			2	u'>	တ	00	∞	ω	cr,	2	20-3	9-01	=	12	5	4.	91	16-2	16-3	17-1-5	17-6	13	19-2	8	8	23	23		Total emount
Covered Basin Area	(Km2)		7.40	2.40	11.60	58.50	5.50	11.80	758. 40	1233.70	256.00	45. 20	4. 60	107.00	8.40	53. 20	342. 50	139.00	386.00	544. 20	159.80	163.50	57.90	27.30	:	60.50	42, 80	3604.10	The from
Name	,		CANAL CANTO DO RIO	RIO BOMBA	RIO IMBOASSU	RIO ALCANTARA	RIO MUTONDO	RIO GUAXINDIBA	RIO CACEREBU	RIO CUAPINIRIN	RIO MACACU	RTO SOBERBO	CANAL DE MAGE	RIO ROMCADOR	RIO IRIRI	RIO SURUI	RIO ESTRELA	RIO INHOMIRIN	RIO SARACURUNA	RIO IGUACI	RIO SARAPUI	RIO S. J. DE MERITI	RIO ACARI	RIO IRAJA	CANAL DO PENHA	CANAL DO CUNRA	CANAL DO MANGUE	TOTAL	. 1) * Tributary river (over) uded from Total
·£			1 08210	2 BW760		4 AN740	5 km220	6 GX720	7 00622	8 GP600	*9 MC967	★10 SB998	11 #C580	RN560	13 18540	14 SR500	15 ES400	1N460	*17 \$5420	18 IA260	SF300	-	*21 AC241	22 11200	P. 180	CN100	25 MN000	Ţ	Mortoc

Notes : 1) * Tributary river(excluded from Total amount)
2) AUG.SEP: Not measured of discharge

A : Natural and Agricutural use
| Urban : Urban use
| Urb.S.T: Urban Use with Sewage Treatments

Table APP. 1-2 Mean Runoff Load of Regular Survey (May 1992 to Apr. 1993)

((cob()(lu))

																					_							
Kean Yalue			1.0	ස	69	60	0.4	0.1	30.6	30.0	2.0	2.0	0.5	ස	0.5	2	20.9	0.9	0.9	23. 3	21.9	38.3	7.3	ю 4	1.3	7.2	nş es	
6			Ö	ಧ		ರ	ci	0.28	÷	ဗ္ဌ	ರ	-:	oʻ	-:	Ġ		4	9	0	e	75	43	75	ત્રં	ന്	Ξ	r.	<u>78</u>
80	1993	MAR	2.00	90.0	5.27	0.12	0. 10	0 11	52.72	83. 15	83	1.58	0.37	4. 72	1.51	5.33	26.37	1.02	0.63	21.80	21.95	20.38	3.50	ς; 26	0.62	3.07	5.65	271.83
2		17.						0.01																				Г
9		NO.	0.68	0, 14	3.72	2. 52	0.38	0.11	52.49	38	2.57	2, 38	0.54	5.95	0, 77	1. 72	12. 99	0.87	0.30	11 78	z z	111.13	0.55	3.31	0.82	15.90	7. 95	306.09
5		EZ.	0.59	0.62	20.5	0.81	0.69	0.08	19, 48	24. 79	1.90	1.07	0.68	6.33	0.63	3.74	36.54	65	2.30	55. 79	32.34	15.50	6.45	0. 42	0.67	5.14	5.31	213.28
4	32	SEP																	•		•							0.00
ന	199	ACC		•							•																	0.00
2								8																				118.46
-		XYX	1.91	0.19	2.27	0.12	0.31	0.0	83 83	16.23	8	2.86	0.37	1.96	0.12	5 13 24	9.60	0.25	1.19	28 60	5.56	23. 16	5.83	2.76	0.59	4.07	2. 63	126.58
Land use			Urb/S. T	Urban	Urban	Urban	Urban	Urban	N/A	N/A	N/A	N/A	N/A	N/A	¥/¥	N/A	N/A	¥/N	N/A	N/A	Urban	Urban	Urban	Urban		Urban	Urb/S. T	
PopulationLand Density Type	(p/km2)							3, 25																		12.82		~
AreaPopulation Number					138, 636			•	336, 193	69, 853	18, 577	17, 911	8, 458	36, 370	10.684	12,910	302, 495	90.	194 173	758.010	1.012.275	1. 492. 458	438, 076	500, 276			500, 876	
	(Kn2)				30.80				- 4	1253. 10	-						ત્રં	oi.		તં							42.80	
Š.	_		2	w	æ	œ	∞	~	o	2	10-3	10-6	Ξ	음	23	14	16	16-2	16-3	17-17	17-6	<u></u>	19-2	8	8	77	23	
Covered Basin Area	(Km2)		7.40	3.40	11. 80	58.50	5.50	11.80	758.40	1233, 70	256.00	45.20	4.60	107.00	8.40	53.20	342.50	139.00	186.00	544. 20	159.80	163.50	57.90	27.30		60.50	42.80	3604. 10
			CANAL CANTO DO RIO		OASSU	ANTARA	OGNO	DAXINDIBA	CEREBU	UAPINIRIN	CACU	DBERBO	CANAL DE MAGE	RIO RONCADOR	RIE	SURUI	ESTRELA	NEOW I R I W	SARACURUMA	GUACU	SARAPUI	S. J. DE JERITI	CAR	SAJ.A	CANAL BO PENEA	S CONTA	DO MANGUE	
Name			CYNYT CY	RIO BOKBA	810 188	RIO ALC	RIO MU	R10 GU	R10 C	R10 6U	R10 I	R10 S	CANAL	RIOR	R10 1	810 S	R10 E2	830	R10 S	R10 1	RIOS	810 S	RIO ACAR	RIO IRAJ	CANEL	CARAL 30	SIN	į

Notes : 1) * Tributary river(excluded from Total amount)
2) AUG. SEP: Not measured of discharge
N/A : Natural and Agricutural use
Urban : Urban use
Urban Use with Sewage Treatments

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Table APP. 1-2 Mean Runoff Load of Regular Survey (May 1992 to Apr. 1993)

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		_																	<u> </u>										
Kean Yalue								0.0																					
ക		Æ.	1.07	0.16	0.45	0.19	0.25	0.13	5.07	6.24	0.30	0.09	0.15	0.37	0.01	0.19	2 88 82	0.16	0.43	7. 43	20.00	35.13	6.50	0.90	2.17	7. 41	4.07	95.30	
ସ	1953	KAR	1.06	0.09	1.74	0.52	0.03	0.06	25	12.22	1 36	0.14	0.27	0.30	0.02	0.24	3.75	0.88	88 0	81	34.41	32, 49	4.03	3. 13	0.41	4.32	9.68	130.87	
7		FEB	0.84	0. 28	1.02	0.11	0.24	0.05		1. 23	0.58	0.04	0.47	0.30	0.0	0. 22	E 9	0.19	80	13.20	24.58		6.03	2.84	0.99	 	5. 15	71.07	
9		NOV	0.73	0.32	5	0.23	0.67	0.09	8 65	3.57	1.33	8	1.08	0.53	6.05	0.11	 	0.30	0.37	3.16	46.33	42, 75	4. 71	4. 78	1.33	29. 28	9.54	152. 45	
		ઇ	1.13	0.31	1.24	0.13	0.40	0.83	65 45	-88	0.57	0.15	0.52	0.69	0.23	88	11.35	98	98	31.13	33	48.65	6.35	3. 72	2.51	19.05	, 33 33	168.86	
4	2	SEP			-							-		-		•						-						0.00	
က	1997	γΩC																										0.00	
2		JS.	0.63	0.43	0.3	0.10	0.43	0.00	1.77	37	0.15	0. 13	0.15	0.27	00.00	0.23	8.26	90.0	0.44	4, 37	19, 17	15.39	13.38	9. 76	I. 12	68.6	3.26	76.35	
		KAY	1.57	0.08	0.63	0.03	92	0.00	. 25 25	2. 12	9	0.09	0.17	0. 27	0.01	0.40	 53	0.13	0.54	88	12.05	51.88	5. 78	3, 17	1.30	6.75	4, 98	100.28	
and use			Urb/S. T	Urban	Urban	Urban	Urban	Urban	N/A	N/A	W/A	H/A	N/A	K/A	N/A	W/A	N/A	N/A	K/A	N/A	Urban	Urban	Urban	Orban		Urban	Urb/S. T	1	
orPopulationLand use Density Type	(p/km2)		5.64	·				3.25			_	_						٠.							•	12.82	2		
AreaPopulation					138, 636				336, 193	69, 853	18 577	17, 911	8, 458	36, 370	10.684	12,910	302, 495	%	194, 173	758, 010	. 012. 275	. 492. 458	438. 076	500. 276			500, 876		
Basin Area	(Кп2)				30.80															562.80					,	63. 60	42.80	c.	Inn't
Ş.	•/		2	ري د		∞	600	•	œ	2	10-3	9-01	=	12		75	9	16-2	16-3	17-175	17-6	σ.	19-2	20	20	77	23		Total amo
Covered Basin Area	(Kn2)		7.40	3.40	11.60	58, 50	5.50	11.80	758. 40	1233, 70	256.00	45.20	7. 60	107.00	8. 40	53.20	342, 50	139, 00	186.00	544. 20	159.80	163.50	57.90	27.30		60.50	42.80	3604.10	Dinded from
Name			CANAL CANTO DO RIO	RIO BOMBA	RIO IMBOASSU	RIO ALCANTARA	RIO MUTONDO	RIO GUAXINDIBA	RIO CACEREBU	RIO GUAPINIRIN	RIO MACACE	RIO SOBERBO	CANAL DE MAGE	RIO RONCADOR	RIO IRIRI	RIO SURUI	RIO ESTRELA	RIO INBOMINIM	RIO SARACURUNA	RIO IGUACU	RIO SARAPUI	RIO S. J. DE MERITI	RIO ACARI	RIO IRAJA	CANAL DO PENHA	CANAL DO CUMBA	CANAL DO MANGUE	IOTAL	Tributary river(excluded from Intal amount
8			1 C1780	2 BM760	3 15810	4 AN740	5 arr820	6 GX720	7 00622	8 09600	#9 MC967	*10 SE698	11 MG580	12 RN560	13 18540	14 SR500	15 25400	*16 IN460	*17 \$5420	18 1A260	19 SP300	20 \$1220	#21 AC241	22 1,200	23 PN 180	24 CN100	25 MX000		Motor (

Noies: 1) * Intubutary rivertexcluded from iotal ion.
2) AUG.SEP: Not measured of discharge
N/A : Matural and Agricutural use
Urban : Urban use
Urb/S.T: Urban Uso with Sewage Treatments

Table APP. 1-2 Mean Runoff Load of Regular Survey (May 1992 to Apr. 1993)

E

Rasi	Covered. Basin Area NO.	Basin Area	AreaPopulationPopulationLand use Number Density Type	opulation!	and use		63	ero	4	LD:	9	7	8	Gr .	Mean Value
(Kn2)		(Km2)		(p/km2)	L			1992	1 1				1993		
						¥ΑΥ	JÜN	AUG	SEP	r S	NOV	FEB	MAR	APR	
2.5	10 2				Urb/S. T	0.21	0.08				0.07	0.10	0.09	0.05	
က်	40 5				Urban	0.03	0.04				0.03	0.02	0.02	0.05	
≓	9 09	30.80	138, 636		Urban	0.12	0.09				0.23	0. 28	0.25	0.02	
2	50 8				Grban	0.01	0.05				0.17	0.07	0.03	0.03	
ഗ	50 8				Ürban	0.05	0.06				60 .0	S	0.01	0.09	
=	80.8				Urban	800	0.00				0.01	8	C. 02	0.02	
83	. 40	846. 70			N/A	0.54	88				2.51	0.28	3.18	0.92	
8		1253.10			N/.k	0.21	0,03				0.66	0.2	4.04	0.33	60
ķ		256.00			N/A	0.03	0.05	•			0, 12	90.0	0.26	0.04	
5	.20 10-6	132.40			. Y/N	0.02	0.05				0,02	0.01	0.03	0.01	
~		18.30	8, 458	0.46	N/A	0.03	0.11			0.0	0.08	0. 10	0.03	0.01	0.1
ò	7.00 12	Ξ.			N/A	0.04	0.05				0.13	0.83	0. 19	0.08	
		27.			N/A	0.00	0.00		•		0.03	0.0	0.05	0.00	0.0
ഗ		æ			R/A	0.06	90.0		<u>. </u>		0.04	0.04	0.16	0.03	
×		342			N/A	0.43	2.02				0.48	1.24	1.39	28 28	
m		139			N/A	0.02	0.0				0.03	0.02	0.04	0.05	
፟		186			N/A	00	0.02				6.03	0.03	0.01	0.02	
Ì.		282			N/A	1.77	. 28				23	3. 27	1.25	1.07	
ŝ		165.			Urban	1. 20	2.79				7. 57	7.56	4.80	2.75	
9	163.50 19	164.			Urban	5.09	1.80				9. 26	1.43	4.75	4. 25	
C.		 21			Urban	. 28	2 20				0.83	1.02	0.90	1. 52	
Ç.		33			Urban	0.39	1.39				0.61	0.45	0. 79	0. 18	
						0.18	0.18		•		0.16	0. 18	0. 16	0.36	
9	. 50 21			12.82	Urban	1.26	1.41	-			3.08	0.67	1.62	0.74	5.3
-7	42.80 23	42.80	500, 876	2	Urb/S. T	0.91	0.52				1.50	₹	.: %	0.55	
Š	3604.10	S				12, 53		00.00	0.00	24. 25	28, 95	16.68	24. 22	12.06	
L	Corplinged 6-or Total														

Notes: 1) * Tributary river(excluded from Total amount)
2) AUG. SEP: Not measured of discharge
N/A: Natural and Agricutural use
Urban: Urban use
Urban Use with Sewage Treatments

Table APP. 1-3 Monthly Runoff Load of Regular Survey (May 1992 to Dec. 1993)

		,		. (Nay. 1992)									
		1				Vater Qualit	y			,	Runoff Load		
	KO .	Rasin Area (Ku2)	Discharge (m3/s)	900 (mg/l)	(00(Cr) (eg/1)	000(Mn) (ng/l)	TN (mg/1)	TP (mg/l)	800 Load (t/day)	(t/day)	COD(in)Load (t/day)	TN Load (t/day)	TP Load (t/day)
1	C1780	7.40	2.378	24.0	38.0	9. 3	7.65	1.09	4. 93	7.81	1.91	1, 57	0. 2
ż	B11760	3.40	0.109	80.0	80.0	20. 2	8.02	3.00				0.08	0. (
3	18810	11.60		12.0		11.3	3. 16	0.60				0.63	Ō.
ā	AN740	58. 50	0.067	140.0	140.0	21. 5	5. 50	2. 50				0.03	0.
5	¥7820	5.50	0.208	40.0	69. 0	17. 5	21.20	3.00	0.72			0. 38	Q.
6	GX720	11.80	0.000	12.0	33.0	9.1	6.09	1.50	0.00		0.00	0.00	0.
7	CC622	758.40	31.146	20.0	25. 4	8. 9	1.21	0. 20	53.82		23. 95	3. 28	Ö.
8	GP600	1233.70		8.0	9. 0	6. 9	0.90	0.09	18.81			2. 12	0.
9	¥C967	256.00		2.0	6. 4	2. 1	0. 75	0.08	0.80		0.84	0. 30	0.
0	SB998	45. 20	1.019	96.0	270.0	32. 5	1.04	0.20	8.45	23.77		0.09	0.
l	¥6580	4.60	0.166	42.0	180. 0	25. 5	12.03	2, 00	0.60		0. 37	0. 17	0.
2	RN560	107.00	5.043	2.0	17. 0	4.5	0.62	0.10	0.87		1.96	0. 27	Ô.
3	1854D	8.40	0.062	3.0	61.0	21.5	1.43	0.35	0.02	0.33	0.12	0.01	Q.
4	S8500	53. 20	4.943	3.0	52.0	5. 1	0. 93	0.15	1.28	22. 21	2. 18	0.40	0.
5	ES 100	342. 50	14.065	6.0	38. 0	7. 9	2.81	0.35	7. 29	46. 18	9. 60	3. 45	0.
6	IN460	139.00	0.936	2.4	13.0	3. 1	1.63	0. 20	0.19	1.05		0.13	0.
7	SC420	186.00	2.514	13.0	38. 0	5. \$	2.48	0. 20	2.82		1. 19	. 0.54	· · · 0.
8	14260	544.20	40.871	8.0	45.0	8. 1	2. 43	0.50	28.25			8. 58	1.
9	SP300	159, 80	6.919	24.0	38. 0	9. 3	20. 16	2.00	14. 35			12.05	1.
0	\$1220	163. 50	29. 455	20.0	-	. 9.1	20. 07	2.00				51.08	5.
ŀ	AC241	57.90	7.410	40.0	49. Ù	9. 1	9. 03	2.00	25. 61			5.78	I.
2	11200	27:30		90.0	90. 0	21.0	24. 11	3.00	LI. 84			3. 17	0.
3	PN180	1	0.683	40.0	54:0	10.0	22. 10	3.00	2. 36			1. 30	0.
4	CX100	60.50	4.853	50.0	56. 0	9. 7	16.10	3.00	20.96		4. 07	6. 75	1.
5	#N000	42. 80	3.018	70.0	70.0	. 10.1	19.09	3.50	18. 25	18.25	2. 63	4. 98	0.
_	Total	3604.10							239. 24	417. 22	126. 58	100. 28	12.

		ļ	! !			Pater Qualit	у		1		Repost Load		
	KO	Basin Area (Km2)	Discharge (m3/s)	BOD (ng/1)	COD(Cr) (mg/l)	COO(Nn) (ng/1)	TN (eg/1)	TP (sg/l)	800 Load (t/day)	COD(Cr)Load (t/day)	OOD(IIn)Load	IN Load (t/day)	TP Load (t/day)
1	C1780	7. 19	0. 428	26.0	51.0	10.5	17.11	1.50	0.98	1.89	0. 39	0.63	0.0
2	B#760	3.40	0.124	105.0	119.0	32. 5	40.05	4.00	1.12		0.35	0.43	0.0
3	18810	11.60	1.000	10.0	-	9. 5	9.08	1.00	0.86		0. 82	0. 78	0.0
4	AN740	58.50	0.051	150.0	249. 0	23. 5	22. 08	4.00	0.66		0. 10	0.10	0.0
5	11820	5. 50	0.167	116.0	124.0	26. 5	30.07	4.00	1.67		0. 38	0.10	9.0
6	GX720	11.80	0.000	9.0	28.0	9. 3	15.07	2.00	0.00		0.00	0.00	
1	CC655	758. 40	9. 593	5.0	16.0	6. 5	2.13	0. 10	4.14	13. 26	5. 39		0.0
8	GP600	1233. 70	10.992	2. 0	9. 8	3.5	0.36	0.05	1. 90	9. 31	3. 32	1. 77 0. 34	0. 0
9	EC967	256.00	3. 528	2.0	3. 7	1.9	0.51	0. 05	0.61	1. 13	0.58	0. 34	0.0
10	SB998	45. 20	0.721	160. 0	230.0	52. 5	2. 15	0.10	9.97	14. 33	3. 27		0.0
ı	¥C580	4. 60	0.440	50.0	78. 5	24. 5	3. 82	3.00	1. 90	2.98	0. 93	0.13	0.0
12	RN560	107.00	2. 785	4. 0	9. 8	3.1	1. 12	0.09	0.96	2. 36	0. 75	0.15	0.1
3	18540	8.40	0.018	3. 4	23. 2	12.5	1.91	0. 20	0.01	0.01	0.13	0. 27	0.0
4	SR500	53. 20	2.971	6.4	15.6	5. 3	0.91	0. 25	1.64	4.00	1.36	0.00	0.0
5	ES 400	342.50	23. 354	12.0		9. 5	4.09	1.00	24. 21	0.00	19.17	0. 23	0.0
6	IN460	139.00	0. 393	2.0	9. 2	3. 3	1. 17	0. 20	0. 07	0.31	0.11	8. 26	2.0
7	SC 120	186.00	2. 292	5. 0	17.5	2.7	2. 20	0. 10	0. 99	3. 47	0. 11	0.06	0.0
В	14260	514. 20	12, 163	12. 0	15. 0	1,7	4.15	1.50	12.61	3. 41 15. 76		0.44	0.0
9	SP300	159.80	9. 232	35. 0	40.0	10.3	24 03	3.50	27. 92	31.91	8. 09 8. 22	4. 37	1.5
Ö	SJ220	163. 50	10.417	38. 0	43. 0	49. 5	17. 10	2.00	34.20			19. 17	2.1
i	AC241	57. 90	8. 504	68.0	81.0	21.5	18. 21	3.00	49. 96	38. 70	44. 55	15. 39	1.8
2	11200	27. 30	5. 367	65.0	74.0	19.5	21.01	3.00	30.14	61.72	15. 80	13. 38	2. 2
3	PN180	-	0. 682	117.0	159. 0	28.5	19.04	3.00	6.89	34. 31	9.04	9. 76	1. 3
4	CN100	60.50	8. 140	53.0	57.0	10.5	14.06	2.00	37. 27	9.37	1.68	1.12	0.1
5	#N000	12. 80	3.014	55.0	81.0	25. 0	12.51	2.00	16. 93	40. 09 21. 09	7. 38 6. 51	9. 89 3. 26	1. 4 0. 5

Table APP. 1-3 Monthly Runoff Load of Regular Survey (May 1992 to Dec. 1993)

		1				Vater Qualit	у			*	Runoff Load		
	Ю	Bosin Area (Ke2)	Discharge (m3/s)	800 (mg/l)	000(Cr) (#g/l)	COD(Ma) (mg/l)	TN (mg/1)	TP (mg/l)	BOD Load (t/day)	COD(Cr)Load (t/day)	(t/day)	TN Load (t/day)	TP Load (t/day)
1	C1780	7.40		20.0	100.0	10. 2	20.05	1.40					
2	B#760	3.40		40.0	110.0	9. 2	24. 13	3.00					
3	18810	11.60		4.0		10. 6	3.94	0.60					
4	AN740	58. 50		-	•	-	70. 14	62.00					
5	N1820	5.50	·	30.0	120.0	9.8	20. 17	3, 20					
6	GX720	11.80		20.0	70.0	9. 0	16.09	2, 50					
7	CC622	758. 40		12.0	- '	. 8.8	1. 43	0. 10					
8	GP600	1233. 70		2.0	10. 0	3. 8	0.75	0.04	i				
3	EC967	256.00		. 2.0	10.0	1.8	0. 75	0.01					
)	SB938	45. 20		120.0	220. 0	36. 0	1.55	0. 25					
ì	XG580	4.60		30.0	50. 0	10. 2	18.04	2. 50	1				
Ž	PN560	107.00		2.8	20. 0	4.0	1. 53	0.20					
3	1R540	8.40		4.0	-	8. 6	1. 29	0. 25					
ı	SR500	53. 20		5.6	30. 0	5. 6	1. 31	0.20					
5	ES400	342, 50		40.0		14.0	4.04	0.70					
3	18460	139, 00		4.0	20.0	4.8	2, 06	0.20					
!	SC420	186.00		20.0	40.0	6. 6	2. 27	0.10					
3	14260	544, 20		20.0	-	16.0	10.04	1. 40					
•	SP300	159. 80		40.0	100.0	20. 0	10. 18	1.60					
)	S1550	163. 50		40.0	430.0	24. 0	13. 03	2.00					
l	VC541	57. 90					7.2						
?	11200	27. 30		30.0	90. 0	25.0	10. 46	1.20	l				
3	PN180			30.0		28. 0	10. 22	1. 30					
ŧ	CN100	60. 50		25. 0.	100.0	20.0	10.50	1. 10					
5	#N000	42. 80		20.0	130. 0	14.0	8. 41	1.10					
	Total	3604.10	0.000						0.00	0.00	0.00	0.00	0.

		1 .	(·			Tater Qualit	·v		[Runoff Load		
	NO	Basin Area (Em2)	Discharge (m3/s)	BOD (Eg/l)	COD(Cr) (mg/l)	COD(In) (#g/I)	TH (mg/1)	TP (mg/l)	BOD Load (t/day)	COD(Cr)Load (t/day)	1 COD(Mn)Load (t/day)	TN Load (t/day)	TP Load (t/day)
ı	C1780	7.40		2. 0	60. 0	10. 2	10. 51	0.65					
2	B#760	3.40		40.0	140. 0	11.8	15, 45	2.70	ł				
3	IB810 -	11.60		2.0	-	12.6	2.44	0.50				7	
4	AN740	58.50		150. 0	350. 0	92.0	11.60	4. 30	i				
5	NT820	5.50	i i	40.0	140. 0	11.0	13. 40	2. 90					
6	GX720	11.80	i l	4.0	40.0	10. 4	6. 93	0. 60	l				
7	CC622	758.40	f l	2.0	40. 0	10.4	2. 23	0.40			•		
3	CP600	1233.70	i i	2.0	55. 0	9.6	0. 96	0.10					
9	KC987	256, 00	ł	2. 0	15. 0	3.4	0. \$5	0.07					
)	SB998	45. 20		13.0	60.0	10. 2	1. 42	0.10					
İ	NG580	4,60	l ·	16.0	80.0	22. 0	7. 15	1. 30					
2	RN560	107.00)	2.0	10.0	5.4	0. 81	0.08					
ł	18540	8.40		4.0	40.0	11.2	0. 91	0. 20					
ı	SR500	53. 20	. 1	2. 0	15.0	4.6	0. 60	0. 07				:	
,	ES400	342, 50		4. 0	-	9. 2	1. 32	0. 20					
,	IN460	139.60		2. 0	15.0	4. 0	1. 31	0.09					
	SC420	186.00		2.4	6. 0	2. 6	1.41	0.09					
3	14260	544.20		6.0	40.0	10.6	3. 39	0.60					
)	SP300	159. 80		20. 0	75. 0	10.4	10.40	1. 20					
)	\$1220	163.50		16.0	55. 0	10.0	9.90	0.80					
l	AC241	57.90			-	-	-	-					
2	11200	27. 30		20.0	60.0	10.0	8- 00	1.00					
,	PN180	-		12.0	£0. Q	10. 4	9. 60	0.80					
i.	CX100	60.50		14.0	60.0	. 9. 8	8, 80	0.70					
5	N000	12.80		14. 0	40.0	7. 8	8. 35	0. 70					
••	Total	3604.10	0.000						0.00	0.00	0.00	0.00	0.

Table APP. 1-3 Monthly Runoff Load of Regular Survey (May 1992 to Dec. 1993)

		····		(Oct. 1992	")	···			· 				
	NO.	Basin Area (Eu2)	Discharge (m3/s)	B00 (eg/l)	000(Cr) (eg/1)	Vater Qualiq (ODÇMn) (mg/1)	y TN (eg/1)	1P (eg/l)	800 Load (t/day)	COO(Cr)Load (1/day)	Runoff Load COD(In)Load (1/day)	TN Load (t/day)	TP Load (t/day)
			(#0/5/							(1/02/)	(170297		(1/045)
1	C1780	7.40	0.590	40.0	130. 0	11.5	22. 12	2.10	2.04	6. 63	0.59	1.13	0. 1
2	88760	3.40	0.129	100.0	350.0	56.0	28. 07	4. 20	1.11	4.01	0.62	0. 31	0.4
3	10810	11 60	3. 557	8.0	-	10.0	4. 05	0.70	2.46	0.00	3. 97	1. 24	0.
4	AN740	58.50	0.081	380. 0	925. 0	116.0	15.08	11.00	2.66	6.47	0.81	0.11	0.
5	41820	5.50	0, 159	90. 0	260.0	50. 0	29.05	4. 30	1. 24	3. 57	0. 69	0.40	0.
5	GX720	11.80	0.063	12.0	60. 0	11. 5	4.60	1. 20	0.07	0. 33	0.06	0.03	0.
7	CC655	758.40	22.543	8.0	30.0	10.0	1. 76	0.20	15. 58	59. 43	19 48	3. 13	Q.
8	GP600	1233.70	39.847	2.0	15. 0	7. 2	1. 36	0.15	6.89	51.64	24.79	4.66	0.
9	IC967	256.00	7. 330	2.0	10.0	3. 0	0.90	0.09	1. 27	6. 33	1. 90	0.57	0.
0	\$8998	45. 20	1.054	20.0	75.0	11.8	1.31	0. 20	1.82	6. 83		0.12	Ö.
ŧ	EG580	4.60	0.747	12.0	70.0	10. 8	8. 69	0.60	0.77	4.52	0.68	0.52	Ó.
2	RN560	107.00	11.206	2.0	20. 0	5. 6	0.71	0.06	1.94	19.36	S. 39	0. 69	Q.
3	18540	8.40	0. 605	12.0	65. 0	12. 0	· 4.3i	0.45	0.63	3. 40	0.63	0. 23	0.
4	SR500	53, 20	5.418	2.0	20.0	8. G	1.46	0.10	0.94	9. 36	3.74	0.68	Ó.
5	ES400	342.50	54. 223	20. 0	-	7.8	2. 42	0. 45	93. 70	0.00	36, 54	11.35	2.
6	1N460	139.00	5.772	3.2	30. 0	6.4	1: 72	0. 10	1-60	14.96	3 19	0.86	0.
7	SC420	186.00	3, 803	20.0	30.0	7.0	3.01	0.20	6.57	9. 86	2.30	0.99	0.
8	14260	511. 20	71.746	10.0		9.0	5. 02	0.75	61.99	0.00	55. 79	31. 13	4.
3	SP300	159.80	38, 991	30.0	75. 0	9.6	10.01	1.70	101.07	252, 68	32, 34	33. 73	5.
20	S1220	163.50	56.067	10. 0	220. 0	3. 2	10.04	1. 20	48. 44		15. 50	48. 65	5.
?1	YC541	57.90	6.661	40.0	110.0	11. 2	11.04	1.65	23.02	63. 31	6. 45	6. 35	0.
2	11500	27. 30	3.062	60.0	210.0	1.6	14.05	2. 15	15.87		0. 42	3. 72	0.
23	PN180		1.378	70.0	320.0	5.6	21.04	3. 45	8. 33			2.51	. 0.
4	CN100	60.50	12.927	80.0	180. Q	4. 6	17. 03	2.00	89. 35		5.14	19.02	2.
5	≅N000	42.80	5. 125	30.0	185. 0	12.0	12.06	2. 60	13. 28	81.92	5. 31	5. 34	1.
	Total	3604. 10	328. 467	_					468. 36	1862.74	213. 28	168. 86	24.

	NO .	Rusin Area	Discharge	BOD	(00(Cr)	Vater Qualit COD(Mn)	y TH	TP	800 Load	COD(Cr)Load	Runoff Load CDIX In Load	TN Load	TP Load
		(K=2)	(n3/s)	(ng/1)	(mg/l)	(rg/l)	(mg/l)	(mg/l)	(t/day)	(1/day)	(t/day)	(I/day)	(t/day)
1	C1780	7.40	0. 792	4.0	70.0	10.0	10.68	1.00	0.27		0. 68	0.73	0.0
2	BE160	3.40	0.162	48.0	210.0	9.8	23. 19	2. 60				0. 32	0. 0.
3	18810	11.60	5. 520	8.0	•	7.8	0. 85	G. 45				0. 41	0.2
4	AN740	58.50	0.065	140.0	25. 0	460.0	38, 50	30.00	0.80			0. 22	0. 1
5	#1820	5.50	0.398	24.0	90.0	11.0	19. 39	2.50				0. 67	0.0
6	GX720	11.89	0.127	1.0	40.0	9.6	5, 51	1, 90				0.06	0.0
7	CC655	758.40			40. 0	9. 4	0.72	0.45	11.17			3. 99	2. 5
8	GP600	1233.70	76. 334	0.8	15. 0	5.8	0.54	0. 10	5. 28			3, 57	0.0
9	WC967	256.00	17. 154	0.4	15. 0	1.8	0. 90	0.08	0. 59			1. 33	0. :
0	\$8998	45.20	2, 651	13.0	50.0	10. 4	0.39	0. 68	2.93			0.09	0.4
1	#G580	1.60	0.608	20.0	70.0	10. 2	20. 52	1.50				1.08	0.0
2	RN560	107.00	18, 135	1.6	15.0	3. 8	0. 34	0. 03	2.51			0. 53	Q. I
3 .	IRS 10	8.40	0.808	12.0	50. 0	11.0	0. 70	0.30	0.84			0.05	. 0.,1
4	SR500	53.20	4.138	2.0	15. 0	4. \$	0. 32	0.10	0, 72			0.11	. 0.
5	£\$400	342.50	27. 846	4.0	30. 0	5. 4	1. 47	0. 20				3.54	0.
6	IN460	139.00	4. 566	1.2	10. 0	2. 2	0.77	0.08	0.47			0. 30	0.
7	SC420	186.00	3. 523	3.0	15. 0	1.0	1. 21	0.08	0.91			0.37	0.
8	14260	544.20	75. 754	8.0	40.0	1.8	0.48	0. 35					7.
9	SP300	159. 80	43.830	16.0	60. Q	9. 2	12. 24	2.00	60, 59	227. 21	34. 81	46. 33	7.
0	\$1220	163.50	53. 595	16.0	170.0	24.0	9. 23	2.00	74.09	787. 20		12.75	9. :
1	AC241	57.90	6. 367	6.0	50.0	1.0	8. 55	1.50	3, 30	27.51	0.55	4. 21	0.
2	11200	27. 30	3.546	64.0	120.0	10.8	15, 60	2.00	19.61	36. 76	3. 31	4. 78	Ũ.
3	PN160	-	0.951	32.0	160. 0	10.0	16, 21	2.00	2.63	13.15	0. 82	1. 33	Q.
4	CN100	60.50	17.692	44.0	90.0	10.4	19. 15	2.00	67. 26	137.57	15. 90	29. 28	3.
5	N000	42. 80	8. 686	76.0	160. 0	10.6	12.71	2.00	\$7,04	120.08		9. 54	1.
	Total	3604.10	403.615						371, 15	2025. 68	306.03	152. 45	28.

Table APP. 1-3 Monthly Runoff Load of Regular Survey (May 1992 to Dec. 1993)

				(Feb. 1993)								
	МО	Basin Area	Discharge	B00 ·	000(Cr)	Tater Qualit COD(In)	y TN	TP	900 Load	COO(Cr)Load	Runoff Load	TN Load	TP Load
		(K±2)	(m3/s)	(mg/1)	(eg/l)	(ng/1)	(ng/1)	(eg/l)	(t/day)	(t/day)	(t/day)	(t/day)	(t/day)
1	C1780	7.40	0. 739	40.0	75. 0	4.0	13.08	1.60	2.55	4. 79	0. 26	0.84	O. 1
2	B#1760	3.40	0. 095	130-0	320. 0	62. 0	34. 05	6.00	1.07		0.51	0. 28	0.0
3	IB810	11.60	6. 481	6.4	•	9. 6	1.83	0.50	3.58	0.00	5. 38	1.02	0. 2
4	AN740	58.50	0. 036	2900.0	•	620. 0	35.00	22, 00	9.02	0.00	1. 93	0.11	0.0
5	#T820	5.50	0. 122	70.9	110.0	22. 0	23. 02	4.00	0.74	1.16	0. 23	0. 24	0.0
6	GX720	13.89	0.013	8.0	30.0	11. 2	16. 01	3. 20	0.01	0. 03	0.01	0.02	0.0
7	CC622	758.40	21, 251	. 2.8	50.0	9. 0	1.05	0. 15	5.14	91.80	16. 52	1.94	0. 2
8	GP800	1233.70	31, 239	2.0	10.0	3. 4	0. 16	0.09	5.40		9. 18	1. 23	0.2
9	MC987	256.00	8. 137	2.0	10.0	2. 2	0.41	0. 08	1 1.41	7.03	1.55	0.28	0.0
10	58998	45, 20	0.832	50.0	80.0	24. 0	0. 52	0. 20	3.59	5. 75	1.73	0.04	. 0.1
11	NG580	4.60	0.542	40.0	60.0	11.4	10.00	2. 20	1.87		0.53	0.47	0. i
12	RN560	107.00	4. 285	2.0	10.0	4.0	0.81	0. 07	0.74	3. 70	1.48	0.30	0. (
13	18540	8.40	0. 291	3.2	50.0	10.6	1.46	0.30	0.08	1.28	0. 27	0.04	0.1
14	SR500.	53. 20	2.490	5. 2	30.0	5. 2	1. 01	0.20	1.12	6.45	1, 12	0.22	0.
15	ES400	342.50	47. 778	10.0	20.0	9.0	1, 63	0. 30	41.28	82, 56	37. 15	6.71	1.
16	IN460	139.00	1. 762	2.4	20.0	3.0	1. 24	0.15	0.37		0.46	0.19	Ö.
17	SC420	186.00	2. 405	7.2	25. 0	2. 2	1.63	0.15	1.50	5. 19	0. 16	0.34	0.
18	14260	544.20	37. 904	6.4	65. 0	7. 2	4. 03	1.00	20.98	212.87	23. 58	13. 20	3.
19	SP300	159. 80	28, 230	18.0	70.0	10.4	10.08	3. 10	43.90	170.74	25. 37	24. 58	7.
20	\$1220	163, 50	9, 706	12.0	60. 0	10. 2	9. 03	1.70	10.06	50.32	8. 55	7.57	I.
21	AC241	57.90	6. 955	16.0	30. 0	10.4	10.03	1.70	9.61	18. 03	6. 25	6. 03	1.
22	11200	27.30	2. 343	16.0	70.0	- 11-2	14.01	2. 20			2. 27	2.84	0.
23	PN180	-	0.880	12.0	20. 0	10. 6	13.01	2. 40			0.81	0.99	Ó.
24 -	CN100	60.50	3. 852	20.0	50. 0	10.0	10.04	2.00		16.64	3. 33	3. 34	Ō.
25	¥N000	42.80	3.717	30. 0	130.0	10. 4	16, 03	2. 00			3. 34	5. 15	0.
	Total	3604.10	201. 994						167. 97	732. 19	141.81	71. 07	16.
		*Tributary r											

				(Xar. 1993)				·				
						Fater Qualit					Runoff Load		P
	NO	Rasin Area	Discharge	E00	000(Cr)	(COD(¥n)	TN .	TP (xg/1)	BOD Load		(t/day)	TN Load (1/day)	TP Load (t/day)
l		(Xe2)	(#3/s)	(eg/l)	(mg/l)	(eg/1)	(eg/})	(28/1)	(t/day)	(t/day)	(1/03/)	(1/02/)	(1/00//
ι	C1780	7.40	1.007	60.0	70.0	23. 0	12. 13	1.00	5. 22	6.09	2. 00	1.06	0.09
2	B¥760	3.40	0.060	80.0	100.0	11.8	17. 21	3. 20	0.41	0.52	0.06	0.09	0.02
3	18810	11.60	6.492	4.0	1440.0	9. 4	3. 10	0.45	2.24	807. 71	5. 27	1.74	0, 25
4	AN740	58. 50	0. 121	960. 0	-	11. 6	21.01	9.00	10.04	0.00	0.12	0. 22	0.09
5	¥1820	5, 50	0.110	45.0	60.0	11.0	9. 30	1. 20	0.43	0.57	0.10	0.09	0.01
6	GX720	11.80	0. 13!	18.0	40.0	14. 8	5, 50	1.40	0.20	0.45	0.17	0.06	0.02
7	CC622	758. 40	61.415	12.0	40.0	12. 2	1. 44	0.60	63.68	212, 25	64. 74	7. 64	3. 18
8	CP600	1233.70	133, 665	2.4	30. 0	7. 2	1.06	0. 35	27.72	346. 46	83. 15	12. 22	4. 04
9	MC967	256.00	12.040	2.0	10.0	5.8	1. 30	0. 25	2.08	10.40	6. 03	1.36	0. 28
10	S8998	45. 20	1. 990	13.2	15.0	9. 2	0. 81	0. 20	2.27	2. 58	1.58	0.14	0.03
11	¥G\$80	4, 60	0.439	12.0	40.0	9.8	7.00	0.80	0.46	1.52	0. 37	0. 27	0. 03
12	RN560	107.00	10. 930	2.0	15.0	5. 0	0. 32	0. 20	1.89	14. 17	4. 72	0.30	0. 19
13	TR540	8.40	1.905	4.0	35. 0	9. 2	0.44	0.30	0.66	5.76	1.51	. 0.07	0. 05
14	SR500	53. 20	7.501	. 3.2	15.0	8. 2	0.37	0. 25	2.07	9. 73	5. 32	0. 24	0. 16
15	ES100	342, 50	40. 166	20.0	40.0	7. 6	I. 08	0.40	69.41	138.81	26. 37	3.75	1. 39
16	IN460	139.00	2.940	4.0	10.0	4.0	3.86	9. 15	1.02		1. 02	0. 98	0.04
17	SC420	186.00	2. 430	10.0	10.0	3. 0	4. 25	0.06	2.10	2.10		0.89	0. 01
18	18260	544. 20	36.044	8.0	25. 0	7. 0	6.00	0.40	24.91		21. 80	18. 69	1. 25
19	SP300	159.80	26.463	20.0	20. 0	9. 6	15. 65	2. 10	45.73		21. 95	34. 41	4.80
20	S1220	163.50	28. 905	40.0	40.0	8. 4	13. 01	1.90	99.90		20. 98	32. 49	4,75
21	AC241	57.90	5. 786	39.0	50.0	7. 0	8.06	1.80	15.00		3.50	4. 03	0. 90
22	11200	27. 30	3.968	40.0	40. 0	11.2	9. 12	2. 30	13.71	13.71	3.84	3. 13	0. 79
23	PN180	l -	0.681	50.0	60.0	10.6	7. 04	2. 70	2.91		0. 62	0. 41	0. 16
24	CM100	60.50	9. 873	45.0	45. 0	3. 6	5. 06	1. 90	38. 39		3. 07	4. 32	1. 62
25	MN000	42.80	8, 605	30.0	60. 0	7. 6	13. 02	1. 80	22. 30	44. 61	5. 65	9. 68	1. 34
	Total	3604.10	378. 481						432.30	1867. 75	271.83	130. 87	24 22

Table APP. 1-3 Monthly Runoff Load of Regular Survey (May 1992 to Dec. 1993)

						Tster Qualit			ļ ·		Funoff Load		
	Ю	Basin Area (Ka2)	Discharge (m3/s)	BOD (eg/l)	COD(Cr) (ng/l)	000(lin) (eg/l)	TN (mg/l)	TP (eg/l)	BOD Load (t/day)	CCC(Cr)Load (t/day)	(OD(lin)Load (t/day)	TN Load (t/day)	TP Load (t/day)
1	C1780	7.40	0.880	. 18.0	75. 0	11.0	14.11	0. 30	1. 37	5.70	0.84	1. 07	0.0
2	BN760	3.40	0.092	\$0.0	280. 0	11.4	20. 02	2. 45	0.40	2. 23	0.09	0. 16	0.0
3	18310	11.60	1.292	25. 0	1440.0	12.0	4. 01	0.65	2.79	160.75	1.34	0. 45	0. (
ŧ	AN740	58.50	0.121	90.0	-	8.55	18.01	2. 70	0.94	0.00		0.19	0.0
5	#1820	5.50	0.194	70.0	160. 0	21.6	15.01	2.80	1.17	2.68	0.36	0.25	0. (
6	GX720	. 11-80	0. 297	20.0	55.0	10.0	5.10	0. 65	0.51	1.41	0. 26	0. 13	0.0
ī	CC622	758.40	35, 595	16.0	40.0	10. 4	1.65	0. 30	49.21			5. 07	0. 9
3	GP600	1233.70	\$5.268	4.0	30. 0	7. 4	1. 31	0. 07	19.10	143. 25	35. 34	6. 24	0.
9	NC967	256.00	8.551	2.0	10.0	0. 8	0.40	0.05	1.48			0.30	0.
0	SB998	45.20	2. 020	11.6	30.0	7.0	0.51	0.05	2.02		1, 22	0.09	0.
l	#G580	4.60	0. 209	30.0	120.0	21.0	8.01	0. 80	0.54			0. 15	0.
2	RN560	107, 00	6.028	2.0	14. 0	3. 4	0.71	0.15	1.04			0. 37	0.
3	18540	8.40	0.142	10.0	40.0	12.0	0.60	0. 20	0.12	0.49	0.15	0.01	0.1
•	SR500	53. 20	3, 655	2. 0	16. 0	5.4	0.60	0. 10	0.63			0. 19	Q.
5	ES400	342.50	22, 322	20.0	40.0	2.4	2.00	0. 30	38.57			3. 86	O.
5	18460	139,00	2, 129	2. 0	10.0	2.4	0.76	0.08	0.42	2.10	0.50	0.16	0.
١.	SC420	. 186.00	3.935	6.4	35. 0	2.6	1. 27	0. 07	2 18	11, 90	0.88	0 43	0.
3	1A260	514.20	27. 403	4.0	30.0	5. 6	3. 14	0.45	9.47	71.03	13.26	7 43	1.
}	SP300	159.80	14. 161	30. 0	140.0	20.0	16. Oi	2. 20	37.48	174. 92	24. 99	20.00	2.
)	SJ220	163.50	33.837	30.0	140.0	15. 0	12.02	1. 45	87.84	409.90	43. 92	35. 18	4.
İ	AC241	57.90	7, 496	50.0	130.0	19.5	10.03	2. 35	32.38	84.20	12.63	6.50	. 1.
?	11200	27. 30	1. 158	65.0	190. 0	22. 0	9.01	1. 80	6.50		2. 20	0.90	0.
3	081KG	-	2. 095	70. 0	210.0	22. 0	12.01	2.00	12.67		3. 98	2. 17	0.
Į	CN100	60.50	5. 039	120. 0	250. 0	27. 0	17. 01	1. 70	52. 24			7.41	0.
	NN000	42.80	3. 520	65. 0	185. 0	18. 4	13.03	1.75	20.33			4.07	0.
•	Total	3604.10	213. 758						342.91	1410.76	184. 93	95. 30	12.

		1		(Nay. 1993	D		· · · · · · · · · · · · · · · · · · ·						
	NO	Basin Area (Ko2)	Discharge (#3/s)	800 (mg/l)	COD(Cr) (ng/l)	Vater Qualii COO(Wn) (#g/l)	ly †* (*g/1)	IP (18/1) .	BOD Load (t/day)	000(Cr)Load (t/day)	Runoff Load COD(Vn)Load (t/day)	Th Load (t/day)	TP Lead
1	C1780	7. 10	0.875	72.0	250. 0	8. 4	18. 09	1. 56	5. 44	18. 90	0. 64		
2	B1760	3.40	0.121	310.0	530. 0	7.8	32.08	2. 74	3.55		0.08	1. 37	0.1
3	- 18810	11.60	0. 120	12.0		7.8	9.07	1.09	0.44		0.03	0.31	0.0
4	AN740	58. 50	0.064	160. Q	500.0	7. 6	16. 32	1. 97	0.11		0.01	0. 33	0.0
5	MT820	5.50	0.011	140. 0	300. 0	7.8	24. 09	2. 15	0.53		0.03	0.69	0.0
6	GX720	11.80	0.021	12.0	90.0	5, 4	19. 90	1.74	0. 53		0.03	0.09	0.0
7	CC655	758. 40	23.207	8. 0	20.0	3. 6	11.35	0.16	16.04		7. 22	0.04	0.0
8	GP600	1233. 70	21.908	4.0	40.0	2.0	0. 72	0.08	7.57	75.71	3.79	22. 78	0. 3
9	#C967	256.00	5. 459	0.4	6. 0	0.6	0.51	0.05	0.19	2. 83	0.28	1.35	0. 1
10	58998	45. 20	1.026	28, 0	30.0	5. 0	0.85	9.11	2. 48	2. 66	0.28	0.24	0. (
11	#G580	4. 60	0.268	50. 0	130.0	20.0	11.58	1. 65	1.16	3.01	0.46	0.08	0. (
12	RN560	107. 00	2. 922	2. 8	100.0	2. 0	1. 19	0. 09	0.71	25. 25	0. 10	0. 27	0.0
13	18540	8.49	0. 108	10.0	40.0	4. 8	0.86	0. 26	0.09	0.37	0.04	0.30 0.01	0.0
14	SR500	53. 20	2, 020	8.0	60.0	2.6	1. 29	0.17	1.40	10. 47	0.45		0. (
5	ES 100	342, 50	19. 037	6.0	- '	2.8	2.82	0.41	9. 87	0.00	1.61	0. 23 4. 64	0. 6
6	IN160 -	139.00	1.984	30.0	30.0	3.2	1.85	0. 13	5. 14	5.14	0.55	0.32	9.1
1	SC420	186.00	2.033	8.0	40.0	4. 8	2.00	0.06	1.41	7.03	0. 84	0.35	0.0
8	1Y560	514.20	15. 853	20. 0	60. Q	4.0	6. 05	0.80	27. 39	82.18	5. 48	8. 29	0. (
9	SP300	159. 80	6. 359	40. 0	80. Q	30.0	17. 19	1. 51	22.08	(4. 16	16.56	9. 49	1. 1
0	\$1550	163. 50	37. 404	50.0	110.0	40.0	16.31	1.43	161.59	355. 49	129. 27	52.71	0. 8
1	AC241	57. 90	6. 733	70.0	160. 0	26. 0	17. 22	1.62	40. 72	93. 08	15. 13	10.02	4.€
2	11200	27. 30	5. 321	80.0	200. 0	60. 0	24. 20	1, 80	36. 78	91.95	27. 58	11.13	0.9
3	PN180	-	0.663	52.0	110.0	5. 2	12.12	1. 38	2.98	6.30	0.30	0.71	0. d
4	CN100	60.50	11.092	50.0	140. 0	30.0	15.31	1.47	17. 92	134.17	28. 75	14.67	1.4
5	¥8000	42.80	7, 254	40.0	130. 0	20. 0	15. 29	1. 19	25. 07	81. 43	12.53	9.58	0.1
	Total	3604. 10	154. 991			*********			371.51	979. 15	238.63	138, 41	11. 0

Table APP. 1-3 Monthly Runoff Load of Regular Survey (May 1992 to Dec. 1993)

	NO.	1											
ŀ	MA					Tater Qualit	y				Runoff Load		
	NO	Basin Area (Km2)	Discharge (m3/s)	500 (ng/l)	COD(Cr) (eg/l)	000(fa) (eg/1)	1M (sg/1)	1P (bg/l)	BOO Load (t/day)	(COC(Cr)Load (t/day)	COD(Nn)Load (t/day)	TN Load (t/day)	TP Load (t/day)
1	C1780	7.40	0.608	13.0	93. 0	4. 6	11. 35	1, 05	0.68	4. 89	0. 24	0.60	0. 06
2	B#760	3.40	0.116	145.0	596. 0	5.8	28, 59	2. 28	1.45	5.97	0.06	0.29	0.02
3	18810	11.60	1.627	12.0		5. 6	5. 72	0.81	1.69	0.00	0.79	0.80	0.11
4	AN740	58.50	0.036	280.0	372.0	6.2	21. 37	2. 33	0.87	1.16	0.02	0.07	Q. O
5	#T820	5.50	0. 161	124.0	329. 0	6. 2	38. 28	2. 16	1.72	4.58	0.09	0.53	0. 0
6	GX720	11.80	0.065	8.0	62.0	4. 0	11.17	1.06	0.04		0. 02	0.05	0. 0
7	CC622	758. 40	32, 228	2.0	20.0	3.0	1.61	0.14	5.57		8. 35	4, 48	0.3
В	GP600	1233.70	27, 234	1.6	17.0	1.8	0.69	0.09	3.76		4.24	1.62	0.2
9	¥C967	256.00	5. 628	0.6	50.0	1.4	0.92	0.04	0.29		0. 68	0.45	0.0
10	SB998	45. 20	0. 933	23. 2	70.0	5.6	1.05	0.09	1.87		0.45	0.08	0.0
II	MG580	4.60	0.322	52.0	142.0	6.2	13.62	1. 49	1.45		0.17	0. 38	0.0
12	RN360	107.00	2, 756	1.2	9.0	2. 4	I. 10	0.08	0.29	2.14	0. 57	0.26	0.0
13	1R540	8.40	0. 101	2.0	28. 0	4.8	0. 81	0.18	0.02	0. 24	0.04	0.01	0.0
14	SR500	53. 20	2.891	1.6	16.0	1.8	. 1.11	0.11	0.40	4.00	0.45	0. 28	0.0
15	ES100	342.50	37. 980	12.0	- '	3.8	3.30	0. 32	39.38	0.00	12. 47	10.83	1.0
16	IN 160	139.00	2. 368	4.0	13.0	3.8	1.45	0.16	0.82		0.78	0.30	0.0
17	SC420	186.00	2.661	46.0	85. 0	7.4	2, 52	0.09	10.58	19.54	1.70	0.58	0.0
18	1A260	544.20	44.040	16.0	208. 0	3.4	27. 43	0.65	60.88	791.45	12.94	104.37	2. 4
19	SP300	159.80	11,680	30.0	80.0	4.8	10.03	1.50	30.27	80. 73	4. 84	10.12	3.5
20	SJ220	163, 50	16.614	40.0	210.0	5. 2	12.07	1.30	57, 42		7. 46	17. 33	1.8
21	AC241	57.90	6.716	30.0	100. 0	4. 2	10.05	1.50	17.41	58. 03	2, 44	5. 83	0.8
22	11200	27. 30	4.150	- 110.0	400.0	5.8	11.02	1.60	39.44	143. 42	2. 08	3.95	0. :
23	PN180	١ ٠	0.294	60.0	270.0	5.6	15.01	1.60	1.52		0.14	0.38	0. 0
24	CN100	60.50	9.190	25.0	250. 0	5. 2	8.04	1.30	19.85	198, 50	4. 13	6. 38	
25	NN000	42. 80	3. 759	40.0	200. 0	5. 2	10.09	1. 20	12.99		1.69	3. 28	
	Total	3604.10	195. 852						279.71	1710.34	60. 79	166. 02	9. 8

		· · · · · ·	·····	(Jul. 1993	3)								
			:			tater Quali	ly				Runoff Load		
	NO .	Rasin Area (K#2)	Discharge (#3/s)	800 (ng/l)	COD(Cr) (zg/l)	(OD(Nn) (ng/l)	TN (#g/l)	7P (ng/1)	ROD Load (t/day)	(t/day)	000(lin)Load (t/day)	1N load (t/day)	TP Load (t/day)
1	C1780	7.40	0. 643	18.0	106. 0	7. 8	12. 98	0. 89	1, 00	5. 89	0, 43	0. 72	0. (
2	84760	3.40	0. 243	70. 0	70. 0	7. 4	25. 74	1.89	1.47		0. 16	0. 12	0.0
3	18810	11.60	3. 268	12.0	10.0	5. 4	6. 65	0.86	3.39		1.52	1.88	0.
4	AN740	58.50	0.062	150.0	229. 0	7. 8	14. 82	1. 88	0.80		0.04	0.08	0.0
5	¥7820	5. 50	0.034	112.0	000.0	8. 0	12. 38	2.01	0.33		0. 02	0.04	0. (
6	GX720	11.80	0.053	20.0	77. 0	7. 4	15. 90	2. 10	0.00		0.02	0.04	0. (
7.	CC622	758.40	31.504	⟨2.0	18.0	3.4	2. 13	0.13	0.00		9. 25	5. 80	0.
8	GP600	1233.70	20. 461	3. 6	12. 0	1.0	0.91	0. 08	6.36		1. 77	1.61	0. 0.
9	RC967	256.00	3.856	1.4	20. 0	2.0	0. 57	0.01	0.47		0. 67	0.19	0.
0	SB938	45. 20	0.899	60.0	69. 0	8. 2	0. 98	0.16	4.66		0.61	0.08	0. 0.
t	16580	4.60	0. 227	52.0	132. 0	7.4	15. 91	1.69	1.02		0.15	0.08	0.
2	RNS60	107.00	1.604	3. 2	14.0	2.8	1. 38	0.12	0.44	1.94	0. 13	0. 19	0.1
3	18510	8.40	0.153	16. 0	51. 0	6.0	0. 95	0. 22	0.41	0. 67	0.08	0.13	V. 0.
4	\$8500	53. 20	2, 727	2.0	19.0	2. 4	0. 89	0.13	0. 27	4. 48	0. 57	0. 01	U. Q.
5	ES400	342.50	18.923	8. 0		2. 2	3. 55	0. 52	13.08	0.00	3. 60	5. 80	Q. O.
6	IN460	139.00	1.715	1.0	16.0	3. 2	1. 25	0. 28	0.59	2.37	0. 47	0.19	0. 0.
7	SC120	186.00	1.416		44.0	7. 2	1.46	0. 17	0.00	5. 38	0. 88	0.18	U. O.
8	14260	514.20	29. 428	16.0	68.0	5. 2	6. 32	1.01	40.68	172.90	13. 22	16.07	0. 2.
9	SP300	159.80	17. 475	49. 0	96.0	7.2	16.34	2.52	60. 39	144.91	10. 87	24. 67	2. 3.
0	SJ220	163.50	31. 209	68. 0	238. 0	7. 0	13. 67	1.51	183. 35	641.76	18.88	35. 86	J. 4.
1	AC241	57. 90	6. 521	38.0	62. 0	5. 8	12. 74	1. 23	21. 41	31. 93	3. 27	7. 18	9. 0.
2	11200	27. 30	1. 620	140.0	268. 0	7. 0	23. 61	2.11	19. 60	37.51	0.98	3. 30	0.
3	PN180		0.372	440.0	1427. 0	14.0	22.09	3. 87	14. 14	45. 86	0. 45	0.71	0.
4	CN100	60.50	2. 924	56.0	223. 0	6. 4	12. 96	1.31	14. 15	56. 34	1.62	3. 27	Ų. Q.
5	KN000	42.80	3.011	80.0	339. 0	7.0	10. 77	1. 15	20. 81	88. 19	1. 82	2.80	0. :
	Total	3604.10	165.941						381.80	1276. 33	65. 84	101.96	13.

Table APP. 1-3 Monthly Runoff Load of Regular Survey (May 1992 to Dec. 1993)

					,	Vater Qualit	Υ .			7	Runoff Load		
	Ю	Basin Area	Discharge	500	COD(Cr)	COD(Kn)	TN	TP	BOD Load	000(Cr)Load	COD(Nn)Load	TN Load	TP Load
		(Ks2)	(u3/s)	(mg/1)	(rg/1)	(mg/l)	(eg/1)	(eg/l)	(t/day)	(t/day)	(t/day)	(t/day)	(t/day)
i	C1780	7.40	0.557	36. 0	103. 0	2.0	11.47	1.21	1.73	4. 96	0.10	0.55	0.0
2	B#760	3.40	0.152	144.0	385. 0	6.2	26. 09	2.44	1.89	5.08	0.08	0.34	. 0.0
3	18810	11.60	2.579	6. 0	100	5.0	4. 88	0. 72	1.34	0.00	1.11	1.09	0.1
4	AN740	58.50	0.037	250. 0	412, 0	6. 2	18.94	2. 27	0.80			0.06	0.0
5	ET820	5.50	0.092	100. 0	180. 0	6.0	23.95	2. 38	0.79	1.43	0. 05	0.19	0. 0
6	GX720	11.80	0.050	28. 0	94. 0	5. 4	18. 53	2. 23	0.12	0.41		0.08	0.0
7	CC622	758.40	32.806	6.0	28. 0	2.2	1.69	0.14	17.01			4, 79	0.4
8	GP600	1233.70	20.858	(2	5.0	1.0	0.83	0.07	0.00	10. 81	1.80	1.50	0.1
9	HC967	256.00	3.716	1.4	2.0	0.4	0.53	0.06	0.45			0. 17	0.0
0	SB998	45.20	0.225	60.0	82.0	6.0	1. 22	0.10	1.17	1.59		0.02	0.0
1	#G580	4.60	0.190	44.0	250.0	6.0	18. 23	1.72	0.72			0.30	0.0
2	RN560	107.60	0.909	3. 2	10.0	2. 2	0. 91	0.14	0.25	0.79	0. 17	0.07	0.
3	1R540	8.40	0.070	6. 8	310.0	6. 2	1.65	0.40	0.04	1. 87	0.04	0.01	Ç. (
4	SR500.	53. 20	2.053	3. 0	27. 0	1.4	0.80	0. 16	0.53	4. 79		0.14	0.
5	ES400	342.50	49.622	8. 0		2.4	1.77	0.37	34.30			7. 59	1.
6	1N460	139.00	1.163	2. 4	12.0	1.8	1.67	0. 21	0.24			0.17	0.
7	SC420	186 00	0.420	3. 2	14:0	3.2	1. 36	0.03	0.12	0. 51		0.05	0.
8	17560	544.20	16.330	10.0	74. 0	4.6	3.04	1.31	14.11		6. 49	4.29	1.4
9	SP300	159.80	24.069	22.0	81.0	6. 6	12.86	2. 32	45.75	168. 44		28. 74	4.
ð	SJ220	163.50	22, 126	50.0	181.0	8.0	11.26	1. 89	95.58			21.53	3. (
L	AC241	57.90	5. 316	48.0	180. 0	5. 6	7. 68	1.51	22. 17	83. 14		3.55	0.
2	13200	27. 30	1.924	150.0	456.0	6. 4	29. 40	1.91	24.91	75.80		4.89	0.
3	PN180		0.416	360.0	860. 0	13.5	45. 47	2. 52	12.94			1.63	0.
4	CN100	60.50	3, 706	56.0	146.0	6.0	12.85	1.41	17.93			4.11	0.
5	NNOOO	42.80	2. 933	110.0	361.0		18. 40	I. 54	27. 88			4. 66	0.
	Total	3604.10	181. 479						293. 65	978.71	61. 12	84. 57	14.

		,		(OCT, 1993)									
		1 . 1				Water Qual			2.2	Runoff Lo			
	XO.	Basin Area		BOD	COD(C1)	COD (Mn)	î-X	7-P			CODani,OAD	T-N LOAD	T-P LOAD
		(Xm2)	(#3/s)	(mg/l)	(mg/l)	(mg/l)	(ng/1)	(mg/1)	(t/day)	(t/day)	(t/day)	(t/day)	(t/day)
1	C1780	7.40	0, 857	90.0	50.0	5.4	18.87	1. 70	5.11	5. 11	0.35	1.07	0.10
2	BM760	S. 40	0.164	160.0	250.0	7. 2	32.85	2.56	2. 27	3, 54	0.10	0.47	0.04
3	18810	11.60	2.303	12.0	_	5.0	3.14	0.53	2.39		G. 99	0.62	0.10
4	AX740	58.50	0.054	120.0	\$08.0	14.0	26.43	3, 30	0.56	2.37	0. 07	0.12	0.02
5	MT8 20	5. 50	0.101	-	127.0	4. 2	24. 29	2.58		1.11	0.04	0.21	0.02
5	GX726	11.80	0.038	20.0	36.0	6.4	13.65	2. 32	0.07	0.12	0.02	0.04	0.01
7	CC622	758.40	14.669	8.0	75.0	8.3	1.09	0, 15	10.14	96. 32	6.08	1.39	0.19
8	G2600	1233.70	15. 110	2.0	13.0	1.6	1.08	0.08	2.61	16.97	2.09	1.41	0.10
g	MC967	256.00	6. 173	2.0	\$5.0	1.0	0.34	0.05	1.07	18.57	0.53	0.18	0.03
10	SB980	45, 20	0.528	40.0	35.0	7. 2	1.40	0. 23	2.17	1. 90	0.39	0.08	0.01
11	MG580	4, 60	0.051	30.0	145.0	6.0	13.48	1.76	0, 13	0. 64	0.03	0.06	0.01
12	RN560	107.00	0.677	1.8	28.0	2.0	1.84	0.13	0.09	1.54	0.12	0.11	0.:01
13	1R540	8.40	0.099	4.0	_	6.4	1.22	0.23	0,03		0.05	0.01	0.00
14	SR300	53.20	1.763	16.0	-	3, 8	2.17	0.29	2.44		0.58	0.33	0.04
15	ES400	342.50	46, 375	20.6	-	3.6	2.05	0.46	80.14		14.42	8.21	1.85
16	18460	139.00	0.361	5.6	8.0	3.0	2.08	0,46	0.18	0. 26	0.10	0.07	0.61
17	SC420	186. DO	0.490	<2	11.0	0.8	1.98	0.19	0.08	0.47	0.03	0.08	0.01
18	1A250	544.20	44.241	6.0	83.0	2.8	8.14	1.29	22.93	317. 26	10.70	23, 48	4. 92
19	SP300	159.80	18, 287	20.0	225.0	5.4	14.05	2,06	31.60	355.50	8. 53	22.20	3.25
20	S1220	163.50	25.889	20.0	378.0	5.6	12.04	1.68	44.74	845. 51	12.53	26.94	3.76
21	AC241	57.90	5. 331	80.0	92.0	4.4	13. \$4	1. 35	36.85	42.38	2.03	6.15	0.62
22	11200	27. 30	1.228	20.0	252.0	1.4	8.92	1. 07	2.12	26.74		0.95	0.11
23	PK180	- 1	1. 370	60.0	396.0	6.7	9.45	1. 78	7.10	45.87	0.79	1.12	0.21
24	CN100	60.50	6, 965	40.0	50.0	6.6	8.21	1.58	24,07	30.09	3.97	4.94	0.95
25	MNOOO	12.80	3. 283	10.0	\$5.0	4.8	7.40	1. 10	2.84	15. 60	1.38	2.10	0.31
	TOTAL	3604.10	183. 324						241.37	1765.39	62. 99	95, 78	16.01

Table APP. 1-3 Monthly Runoff Load of Regular Survey (May 1992 to Dec. 1993)

		· · · · · · · · · · · · · · · · · · ·	(NOV. 1993		Water Qual	ity		T	Runoff Loa			
NO	Basin Area (Km2)	Discharge (m3/s)	BOD (mg/1)	COD(Cr)	COD(Mn) (mg/l)	T-N (ng/1)	T-P (mg/1)	80D LOAD (1/day)		CODanLOAD (t/day)	T-N LOAD (t∕day)	
1 C1780	7.40	0.705	40.0	102.0	6. 6			2.44	6.21	0.40		
2 BW760	3.40	0.078	72.0	187.0	5.5			0.49	1.26	0.04		
3 18810	11.60	6. 339	2.0	-	4.4			1.10		2.41		
4 ANT40	58,50	0.029	3100.0	4460.0	5.4			7.77	11. 17	0.02		
5 MT820	5, 50	0.123	80.0	187.0	5.2			0.64	1.99	0.06		
6 GX120	11.30	0.014	40.0	125.0	5. 5			0.05	0.15	0.01		
7 CC622	158.40	24, 121	8.0	-	0.0			16.67		0.00		
8 GP600	1233.70	27.334	4.0	17.0	2.4			9.45	40.18	5.67		
9 MC967	256.00	5, 824	1.6	<10	5.8		•	0.81	5,03	2. 92		
10 SB980	45. 20	0.913	5. 0	34.0	5. 6			0.39	2.68	0.44		
11 MG580	4.60	0.415	40.0	544.0	6.6			1.43	19.51	0.24		
12 RN560		4, 759	4.0	15.0	2.8			1.64		1.15		
13 IR540	8, 40	0.627	10.0		5.6			0.54		Ú. 36		
14 - SR500	\$3,20	8.727	22.0	-	5.0			16.59		3.77		
15 ES400		66.412	6.0	-	4.0			34.43		22. 95		
15 13460	139.00	0.567	· <u>-</u>		-			1				
17 SC420	186.00	1, 208	-	-	_							
18 [A260	544. 20	18, 945	30.0	577.0	3.6			49.11		5.89		
19 SP300	159.80	13.967	20.0	141.0	5.8			24.13	170.15	7.00		
20 51220	163.50	44.353	50.0	326.0	6.2			191.60	1249. 28	23. 76		
21 AC241	57, 90	5,609	-	-	-			1				
22 13200	27, 30	1.432	\$0.0	266.0	5. 4			6.19		0. 67		
23 PN180	_	1.023	50.0	237.0	6.4			1.42		0.57		
24 CX100	60, 50	2,708	44.0	170.0	5.8			10.29		1.36		
25 MN000	42.80	1.761	20.0	163.0	4. 4			3.04	24.80	0.67		
TOTAL	3604.10	223.872		~				382.02	2569.33	75. 97		

		*		(DEC. 1993									
			1			Asfet Gas				Runoff Lo		7 V 1010	# D 1015
	NO	Basin Area (Ka2)	Discharge (#3/s)	809 (mg/l)	COD(Cr) (mg/l)	COD(Mn) (mg/l)	f-X (BE/I)	T-P (mg/l)	(t/day)		CODanLOAD (t/day)	(t/day)	T-P LOAD (t/day)
1	C1780	7.40	0.657	96.0	250.0	7. 4			5.45	14.19	0. 42		
2	BW760	3. 40	0.164	60.0	155.0	7. 4			0.85	2. 20	0.10		
3	18810	11.60	2.303	12.0	-	6.8			2.39		1. 35		
4	AS740	58.50	0.054	1600.0	2130.0	33.0			7.45	9.94	0.18		
5	MT820	5.50	0.101	44.0	194.0	6.4			0.38	1.63	0.06		
. 5	GX720	11.80	0.038	8.0	126.0	5.2			0.03	0.41	0.02		
7	CC622	758.40	14.669	7. 0	-	5.8			8.87		7. 35		
. 8	GP600	1233.70	15. 110	10.0	20.0	3. 0			13.06	25.11	3.92		
9	MC967	255.00	6.173	-	-	-			1				
10	SB980	45.20	0.628	_	-	-							
11	VG580	4.60	0.051	50.0	100.0	7. 2			0. 22	0.44	0.03		
12	RN560	107.00	0.677	12.0	17.0	4. 2			0.70	0.99	0.25		
13	12540	8.40	0.099	20.0	-	7.8			0.17		0.07		
14	SR\$00	53.20	1.763	12.0	**	5.8			1.83		0.88		
15	E\$400	342.50	46,375	12.0	-	4.4			48.08		17. 53		
18	IN460	139,00	0.381	10.0	30.0	3.4			0.33	0.99	0.11		
17	SC420	186.00	0.490	30.0	30.0	3. 2			1.27	1.27	0.14		
18	14250	544.20	44.241	-	-	-							
19	SP300	159.80	18.287	· -	-	-							
20	51220	163.50	25.889		-	-							
21	AC241	\$7.90	5. 331	44.0	85.0	7. 0			20.27	40.53	3. 22		
22	1,200	27. 30	1. 228		-	-			!				
23	PN180	-	1.370	-	-	-							
24	CNIGO	60.50	6.965		-	-							
25	MN000	42.80	3. 283	-	-	·							
	TOTAL	3504, 10	183.324						89.49	55.98	32. 26		

Table APP. 1-4 Results of River Water Quality Analysis of Regular Survey

f 5 6 8 f 5 7 2 7 6 6 2 2 f 7 2 6 6 7 7 7 7		-	2	က	7	ь	g	t-	φs	O.	0	Ξ	12	13	7.7	15	9.
DATE OF SARPLING	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	KAY OF	71 150	AUC 26	SEF 23	61 130	81 AOX	FEB 11	MAR 11	APR 12 M	MAY 24	JUN 22.	JGL 20	AUG 18	0CT19	NOVIS	DECIS
General number of the laboratory	boratory	1609	8908	8016	10921	11969	13419	1774	2756	4777	7762	9089	10939	12576	15511	16815	18535
CODE PARAMETER	YLLIND																
Time	æ	10.25	8	8. 10	8.55	9.15	9.45	9.00	10.00	9, 45	10.05	8.20	8. 10	. 15 75	8.35	8.10	8, 2
	υ·	29.00	22	19.00	S S2	30.00	25. 50	32, 00	25, 00	30.00	26.50	20.00	22.00	22	24.00	26.00	27.00
02061F Vater temperature	re C	26. 48	8 8 8	22	33	26. 66 26. 66	25. E.	30.00	20, 30	26.30	24. 58	22. 61	23.55	23, 29	27, 70	28.43	28. 5
	8 %	o o	C .27			n n		ວ່ ູ	3 c	⇒ ×i	o ° °	2.0) .	, c]; c	 	es c
		SO.	200	707	968	0,00	5	5	. S	404	9	3	3	5		2, 20	j
17300F Salinity (field)		28	3 8	35	2 6	9 g	6.51			3							
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200001 NO CELLANY	7	2 4	- 6	3 6	2.5	3 6	2		9.0) t	5	;		9 4		3	ċ
	7 7 7	2.5	- 6	9 5	9 6	7 6	7. 0. 1.	7.0	7 0	2.0	0.00	7 6 6	200	 	9 0	,	•
00000 Spanice 000001 BOD (10101)		3	;	30.00	? ?	212.5	ora in	070.0	0.00	3 0	5	20.5	3 E	3 4	5	ę	
	1/20	5 8	3	3 0	9 6	?	-	7	8	2		3	9	9	8	2	•
	70%	3 5	ũ	? ?	;	130		7,5	102	£.	950	Ö	105	103	08	102	6
08303L bissolved COO	7/20		5	8	3 8	3	2	2	2	2	Ì,	3	?	2	:	1	•
	170	0.00	0.003	00.00	500	<0.003	.0.001	0,004	0.002	0.005	0.004	0.006	0.008	0.011	0.006	0.001	0.00
		00	25	1.40	0 65	2 10	. B	- G	100	0.30	5.5	5	0.89	1.21	1. 70		-65
_		88	0, 40	0.10	02.0	<u>:</u>	3	;		;	; ; .	:	3		:	•	
		0.65	0.25	0.05	0.50	0.20				0, 15	0, 45	0.06	6	0.18	0.22	0.22	0.08
		0.35	2	 	6.55	8			ı	13.		0.99	0.77	1.03	1.48	; ;	
07556L Amonia Nitrogen		2 80	3.40	12.00	3.20	15.00		12, 00	3, 80	5.00	6, 49	38	63	7.3	3 82	4.58	2.5
		60	0.10	0	0.45	0		9.0	0.08	0.0	0.077	0.198	0.084	0.086	0.010	0.042	0
	20 E2 W/1	0.060	0.010	0.085	0.050	0.030		0,080	0.050	0.040	0.008	0.010	0.020	0.014	0.001	0.014	0.0
		5	17,00	20.00	100	22.00	10.50	13.00	12.00	14.00	18.00	11.14	12, 86	11.37	18, 79		*
		7.00	8.8	12.00	8		:	!			•						
		4.70	13.50	8	8 8	7.00		3, 00	8, 40	9.00	11.60	7. 18	65	9.64	14.97		
		4.20	2.60	8	8	:					•						
07801L Total Nitrogen	1 /N 3	7. 650	17, 110	20.045	10.510	22. 120		13,080	12, 130	14, 110	18.09	11.35	12.98	11.47	18.87		
		7, 150	6, 110	12.045	9.510						•						
	ne ng/l	တ	10.5	10.2	10.2	9	10.0	4.0	23.0	11.0	တ်	4.6	7.8	ις ∞	9.4	ð. 6	۲-
	ie mg/1			7.4	o;						•						
03101L DO	mg/1	3	3.5			7					ı	: "					
		হ	\$	∞	*	4	á.	₹:		. '						٠.	
		83	45	70	S	5	40	46	80	55	8	210	53	67	110	38	215
	1/80	*									S S	<u>~</u>	=	83	7.1	e0	ינא
					:						. ;						
5111L Fecal Coli x1000	10 MPN/100m1	es.		1386.88	3.69	800 30	3000.00	1100	2300	2000	1:000	1300	1700				116000
		~		9000,00	8	9000.00	5000.00	3000	2300		20000	1700	17000			300	116000
48004L Cadmium	mg Cd/1	0.000	0.00	<0.002	c0. 00 5	0.005	0.005	0.003	0.005	<0.002 <	<0.002	0.005	co. 002	<0.005	CO. 002	<0,002	9 9
	98 Pb/1	<0.02	20	9	ය ප	000	0.05	<0.02	0.02		20	25	25			CO. 02	0 0
29005L Copper	mg Cu/l	.0.005	0.010	0.002	0.020	0.020	0.040	0.010	0.015							0.005	0
	ng Cu/l	<0.01	0. 0.	6.0	6.01	.0.0			٠		•						
	mg Cr/1	c0. 01	0. 0.	6,01	÷	(0.0)	0.0	ć0.03	0.0		٠.		. 10.03	<0.01	<0.01	<0.01	0,0
80013L Mercury	ug Hg/l	0.10	0. 15	0.30	ć). 10	0.10	0.10	0.25	0.40	0. 10	0.15	0	<0, 10	 O	0.40	<0,10	
	mg 2n/1	0.050	0,060	0.100	0.030	0.120	0.015	0.002	0.020	0. 200	0.060	0.040	0.050	0.030	0, 100	0,040	0.30
	1/8n	<0.001		(0.001	(0.001						•						
	1/20	<0,001		100.0	0,001												
	1/30	0.00		0.00	00.00												
	ug/!	(0.001		0.00	0.00												
8170L PCB's	17																

Table APP. 1-4 Results of River Water Quality Analysis of Regular Survey

Control Cont				7	က	₹.	νs	ယ	۲-	o o	. 6	113	11	21	13	7	15	16
Paralette of the blackware September of the september of the blackware September of the blackware September of the september	ATE OF SAMPLING		MAY 04 J	11 N	25	£3	61	55	=	: =	2		16N 22	20	<u> </u>	0CT19	NOV18	DEC15
Attribution of the constant of	eneral number of the la	boratory	6092	6910	6016	10922	11970	13420	1775	2757	4778	7763	0606	10940	12577	15512	16816	18535
This responsible Control (Field Section		WITY				 				t t t t t t t t t t t t t t t t t t t		t ; ; ;	1					
The respective C 55.0 15.0 25.0 11.0		=======================================	11.40	10, 15		9.15	10.00	10.30	9.30	10.50	10.40	10.50	9, 10	9,05	9.00	9.25	8.50	9.00
Name of the control			31.00	25.00		25.00	30.00	27.00	32.00	26.50	27.00	27.00	21.00	23, 00	24.00	27.00	27.00	30.00
The control of the co		re C	26.30	22.50		22. 70	27. 79	26.84	30.00	23.30	26.30	23. 76	20.84	22. 20	21 81	27. 47	28. 23	28.47
Conditionary Cond			رب دی	7.0			, y	ر د د د	0.	7.0	7.0	2,5	4. 0.	9.5		1.0	2,5	2.0
Manufacticitation (c. 1970) 1970 1970 1970 1970 1970 1970 1970 1970					Ş	i	0.52	0. 62	0.59	0.58	;	: :	0, 42	6. 13	:3 :3	9	1, 11	70
Milling (Titled) X			610	3	193	8		• ;			687							
Discopled to Ref. 1		yi ⊊	න් රේ ර	0.28	ල : ල :	8	0.29	0.5						;	;	,		
Control Cont			g '	17.7	5. 44	9.0	7. 53	. 65	ı	6.20	6.4	S. 43		6.84	9 9	. 6	5, 42	ò
Deciron Second		mg/1	2.2	× 6	9 5	0 6	00 q 		& 	9		- 0	5	× 5	9 9		ı	١
Discripted May 18, 10 of		15 CA/1	0. 020 0.00	070.0	U: U40	(c). 010	010.00	0.0	0.030	0.040	0.030	0.038	0.070	0.025	0.033	201.0		•
Partial Processors Partial		1 /8	9 5	201	÷ 5	9 5	20.	9	130	Ş	25	3.5	C:	2	757	2	7	3
Paralle Mosphores ag 11 1 2 0.00 0.00 0.00 0.00 0.00 0.00 0.		7,5	2	1.0	2 =	9 05	360	016	600	5	900	6.20	202		195	250		15.5
Principle Series		7 (Y)	3 ,	7	9	9 6	200	13	920	2	007	Dec.	000	2	360	3	<u>.</u>	•
Total Massbores et 91 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		7 / Se	0.00	810	0.00	000	0 640	0.00	200	0000	020	0.00	000	0.036	0.030	0.030	0.029	0.032
DESS. Procedures 6 77 1 150 1 10 0 2 10 1 10 0 2 10 1 10 0 2 10 1 10 0 1			35	200	25.	0.00	250	200	000	0.320	200.0	0.056	0.000	200		2 56		*
Contributional National Nati			9.6	38	3 5	⊇ Ç	4. 50	3	<u>ک</u> ک	3.20	Ç. 72		6. 60	20	7	;		
Number Property 150 10			3 S	9 8	3 6	9 9	2,54			:	1 20		1 97		46	2, 43	2, 34	1.8
Number N				8 =	6	⊋ ⊊ 	; c				2 -		. 6	200	9	0.13		i
Hittrie Nitrogen ag Ni		17 N DE 0.	88	3 2	5 6		, S		30 00	9	3 8	. X	3 6		9	6.43	6, 97	100
Hittire Nitrogen ag Ni		'n ac N/]	6	9	2	. K	200		200	3 6	3 6	0.00	0 0 0	000	0 024	0,040	0.027	0.0
Figle days Nitrogen ag N1 S 00 G 00 E 0 G 00 E 0 G 00 E 0 G 00 E 0 G E 0 E		in as N/1	0.00	0.003	000	200	0 000		000	07.0	500	000	0.022	0.003	0.004	0.003	0.005	0,003
Diss. Kjeldahi N. av N.1 7.00 20.00 15.00		-	8,00	40.00	24.00	15.00	28.00	23,00	34 00	17.00	20.00	32.00	28.55	25. 71	26.06	32, 80		*
Dissolved Total College ag N1 5.00 36.50 11.00 11.30 8.00 14.00 11.00 14.00 23.20 21.22 19.99 18.00 24.37 Dissolved Total College ag N1 4.00 16.50 20.00		, ng N/1	7.00	20.00	20.00	15.00		3	-	3	S	,			;			
Diss.Organic M. mg X/1			5.00	36.50	11.00	11.30	8.00		14.00	11.00	14, 00	23, 20	21.22	19, 99	18.00	24.37		
Diss. Nitrogen mg N/1 8 020 24,130 15,450 28.072 34,049 17,209 20,020 32,08 28,59 25,74 26,09 32,08 28,59 25,74 26,09 32,08 28,50 20,210 20,210 20,211,8 26,09 21,13 26,09 20,020			4.00	16.50	7.00	38	;		:	:	:							
Diss. Nitrogen	_		8.020	40.051	24, 130	15. 450	28.072		34 049	17. 209	20.020	32.08		25. 74	26.09	32.85		
		пg №1	7. 200	20.051	20. 130	15.450												•
Diss. OC alcaline mg/l		ine ag/1	20.5	3 3 3 3 3	9.5		26.0	တ တာ	62.0	 ::	11.4		ις; ∞	7.4		7:		-
Page Page			•		7.4	10.6												
Suspended Solids Right 14 27 44 5 5 150 185 50 180 50 5 5 5 5 5 5 5 5		1/80	9:	α į			. 2											
Suspended Solids mg/1		ole mg/l	-	12	₹	<u>ب</u>	on ;	ੲ ;		;	1		,	•	:	-	101	
15.85c1 ved TOC REV. # 15.85c1 ved TOC REV. # 15.85c1 ved TOC REV. # 15.85c1 ved TOC REV. # 15.85c1 ved TOC REV. # 15.85c1 ved TOC REV. # 15.85c1 ved TOC REV. # 15.85c1 ved TOC REV. # 15.85c1 ved TOC REV. # 15.85c1 ved TOC REV. # 15.85c1 ved TOC REV.			560	8	5	120	33	20	180	2	20	320	÷ ;	184	4.	200	. C	
Discourse (100) IPP/100n1 24000. B		1/85	* 1									2	5	0	?	3	2	
Perial Cali x1000 NFX/100m S4000			- 2			•		:		•			000	000	0000	000003	00000	1600
Contail Coli X1000 MPA/11001 50000.00 50000.00 5160 160000.00 500000 50000 50000 50000 50000 50000			4		5000.00	2091		1000.00	30000	20000	13000	30000	00067	20000	13000	20000	10000	004
Capper mg Ca/1 (1.009 0.094 0.002 0.			ನ		5000.00	>160		4000.00	20000	00006	24000	1600000	30000	2000	24000	00000	600.07	
Copper May 7571 (U.02 0.02 (U.02 0.02 0.02 0.02 0.02 0.02 0.02 0.02		7/S	0. no.	0.00 0.004	20.00	CO. 002		CO. 00Z	<0.002	.0. 00Z	<0.00Z	0.005	0,002	50. 005	200.00	700	1000	
Chromium IV mg Cu/1 (0.01) 0.030 (0.005 0.000 (0.002 0.015 0		8 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	CO. 02	₽ .; .;	. 6. 02 	<u>0</u>		(0.05	0.02	0.02	<0.02	CO: 02	<0.02	20.02	20.03		40.0	
Chromius IV mg Cu/1 (0.01 (0.01 (0.01 (0.01 (0.01 (0.01) (0.01 (0.01) (0.01 (0	Copper	1,50 E	ć. 005	0.030	00.00	0.020		(0 00 5	0.020	0.015			0.015	6.005	<0.005	0.00		3
University mg Cr71 (0.01 0.01 0.03 (0.01 (Chromium	1/n2 8m	0.01 0.01	10 to	5	0. 0.								9	4	30 07	10 0	0 07
##EFERTY UR #87.1 (0.08) 0.120 (0.10) (0.10) 0.50 (0.10) (1/17 28	70.0	5 6	90 c	5 6		0.010	0.01	.0.01	9.0	9		9.0	1 5 2 6			6
2.10c mg 2.7/1 0.089 0.120 0.030 0.040 0.050 0.010 0.040 0.050 0.040 0.050 0.0		1/2 H 2/1	10.01	3	G;	0.10		0. 1.	0.50	0. 10	00.10	(0. 10		01.00	0.10	200	21.5	0.0
pp D0: ug/1 <0.001 <0.001 pp D0: ug/1 <0.001 <0.001 pp D0: ug/1 <0.001 <0.001		1/62 85	0.080	0.120	0.030	0.040		0.010	0.010	0.040	0.040	0.060		0.060	Oct O	0.030	0.000	6.0
100 00 100 100 100 1/8n 200 dod		1/80	6.003		9	00.0												
pp DD6 ug/1 <0.001 (0.001 pp DD9 ug/1 <0.001 (0.001 pp DD9 ug/1 co.001 co.001 pp DD9 ug/1		1/80	<0.001		6.00	(0.00)												
pp DDD ug/i <0.001 <0.001		1/80	<0.001		0.00	:0.001												
		78n	<0.001		(0.901	O. 001												

Table APP. 1-4 Results of River Water Quality Analysis of Regular Survey

DATE OF SAMPLING General number o				,	,			1			:	5						
Coneral nux				,	·9	-5T	n	5	~	•	3>	2	=	91	2	5	61	16
General nua	PLING		MAY 05	י 11 אטר	AUG 26	SEP 29 C	2CT 225	NOV 24 F	EB 11 83.	(AR 11 A	IPR 12	8AY 24	JEN 22	JUL 20 A	NUC 18 (0CT19	WOY18	DECIS
	General number of the laboratory	ratory	6159	1169	9110	10923	22	13647	1776	2758	4779	7764	1606	10941	12578	15513	16817	18537
	PARAMETER	YELKI								1								
Ŀ	- inc	ine	10, 10	11.00	00 %	9.35	17.40	9.00	00 07	11.10	11.30	11:30	9.45	9.45	9.30	10, 10	9.10	9.25
	Air temperature	U	28.00	24.00	21.00	25.00	21.00	28.00	33.00	2. 00	31.00	26.00	23.00	24.00	25.00	29.00	28 00	30.00
02061F Ta	fater temperature	0	25.58	22.00	19.20	21.90	23. 70	25.45	30.00	26.40	25.90	22.54	13, 43	21.39	20.93	28, 28	29. 17	29. 23
	Transp. (tube)	8	o oi	17.0			o; ⊙	ල න්	5.0	တ်	⊗	5.0		10.0	6.00	3.0	s. 0	5.0
	Conduct. (field)	5/Si					31.67	32, 32	3. 73	•		23.85	30.4	33, 73	0.23	43.39	46, 79	33, 16
	nduct. (field)	15/SI	30560	34910	37500	26900					18960							
	Salinity (field)	×	18, 71	23, 30	23, 10	15.90	22. 41	19.84										
	(field)		7.04	ţ-	9.33	6, 7	38	55		8	6.67	50.00	6.64	6.54	6, 83	7.00	7.10	7.12
	DO (field)	1/20	2. 10	20	6-5	2.7		2.2	**	, ,	<u>-</u>	د. اد	2	œ	(-)		1	1
		Eg CN/1	<0.010	60.010	<0.010 0.010	c0.010	¢0.010	00.00	00.010	0.010	0.010	0.025	0.015	0.003	0.018	0.00\$		44
		2g/ 1	12	2	7	C-3	99	ຕາ	49		23	21	13	12	40	12	£-3	12
		ng/1	60		-1	6			:						٠			
	COD (total)	18/1					,	•		1440		٠.					ı	t
		1/2=			,										•			
		1/20	0.001	9.69	·0· 001	.0 001	0.005	:0.001	0.00	0.004	0.008	0.006	0.002	0.005	0.003		0.010	00.00
	t/h	22 P/1	6 6	3:	3	0.50	0. 70	0.45	0. 20	0.45	0.65	- 69		0.86	0. 72			*
	š	1/6 86	ද ද	0.5	0 0 0	0. 10												
		1/d 29	5.5	S S	0	9 6	0.50				0.20	= ;	0.24	e 6	O 0	5 6	0. 2.1	0. 21
20	Urganic Phospi	7.2	÷ 6)))	٠ د د د	j .	٠. د د		3		e. 45	% 6 6 6	200	20 c 20 -	87 -		6	0
	Without Miroden	7 2 2) 	2	25	2 H	6 c		5.45	200	07:0	08.7	9 9	1. 32	0.00	3 K	0.0	3 6
	Withits Mirroren	1/5/50	0.10	000	. O	9 6	5 6		10.0	, c	000	3 6	500	5 6	0.00	200	0.00	2 2 2
	Wittle Altrogen	7/8/20	200	3 6	20.0	25.0) i	00.0	0. 030	0.030	200 200 200 200 200 200 200 200 200 200	0.022	60.03 0.03	0.010	0.003		6.04	. *
	Disc Kiplich N	N 20	88	88	3 6	3 2	3	6.	6	20.00	3	e é	3	5	î			•
	ganic Aircogn	N/1	2 2		22.0		2 45		56	05 6	9.80	9	33	4.67	3.08	2, 58		
	Diss. Organic N.	N X	9	2 3	9	0.30	?		3	ì	i	3	;		:			
	Total Mitrogen	1/K 3m	3, 160	9.080	3.96	2.440	4.050		1.830	3.100	4.008	9.07	5.72	6.65	4.88	3, 14		
	Diss. Nitrogen	1/N ZE	2.160	5.080	3,340	2, 240		-										
	Total OC alcaline	1/20		හ හ්	10.6	12.6	10.0	65 1-1	တ် မာ	6	23.0	~ 3	s,	s;	0	o,	4.4	6.8
	Diss. OC alcaline	1/80		•	တ တဲ	9	•		•									
		1/2		,	4	;	2) (
		1/80	÷ :	5 8	6	T [Ş	22 6		ć		č	ę	ć	-	90.	-
10401L SUST	ended Solids	128/1	65. •	₹	3	3	07	9	2)	⊋	D.	g Y	G =	\$	90	<u> </u>	9	78
	TUL	16/1	٠.									3	Ξ.	-		:		ì
	sal Coli x1000	MPW/100ml	505		5	76	20	920	œ	400	170	230	170	05		800	80	130
	Total Coli x1000	NPA/100m1	200		8 8	5 a	240	13 E	2	1300	300	200	000	88		800	130	130
-		ag Cd/1	0.030	0.085		000	0.00	CO. 002	c0. 005	¢0.002	<0.002	(0.002	(n. 002	<0.002	(0.002	<0.002	<0,092	0,075
		ng Pb/1	0.10	8	(0.02	200	9	0.02	CO. 02	¢0.05	(0.02	0.05	<0.02	<0.02		<0.02	<0.03	0.14
_	Ļ	ag Cu/1	0.090	0.040	0.00	0.050	9.100	c0.005	<0.00	0.040	:	:	<0.005	c0. 005	=	<0.005	<0,005	0.040
-	Va IV	mg Cu/1	0.0	10.0	¢0.0	40.0	0.0											
		AR Cr/1	0.0	0.03	(0.0)	0.0	0.0	:0.01	0.0	<0.01	. <0, 01	<0.01	0.01	0.01	0.01	<0.01	<0.01	<0,01
	ŗ	ug Hg/1	0. 10	· <0.10	6.10	ćo. 10	0. 10	ි. 10	0.35	0.03	01.0	<0.10	<0.10	<0.10	0. 10	0. 10	<0, 10	0, 20
		ng 2n/1	0.020	0.070	0.020	0.020	0.030	© 002	(0.005	0.015	0.080	0.008	0.010	c0, 005	<0.005	0.015	0.002	0.060
		1/80	<0.001		60.00	<0.00I												
	300,40	[/2n	00 00		100 0	:0.001												
		ug/1	00.0		.00 OOI	·0.001							-					
18013L pp.	000	ug/1	<0.00 o		:0.00	<0.001												
	Se. s	ng/ i	-0.01		0.0	0.01												

Table APP. 1-4 Results of River Water Quality Analysis of Regular Survey

Control December				7	6.3	4	ις	9	7	∞	5	10	=	13	13	14	15	16
Production Control C	DATE OF SAMPLING			:		•	OCT 13	6	=	=	1	24	22		AUG 18	0CT19	NOVIB	DEC15
Properties 115 22	General number of	the laboratory	6093	6914	9112	10925	11972	13422	1778	2760	4781	7765	2606	10942	12579	15514	15818	18538
The respective (1 a) 155 12 9 46 12 12 12 12 12 12 12 12 12 12 12 12 12		YLIND		,				1			1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		1 1 1 1 1	; ; ; ;	!		1	
The controlled Control of Control	٠.	=	13.5	12. 20	9, 40	10, 15	11.21	11.30	11, 00	12, 15	12.25	12.10	10.50	10.55	10.15	10, 35	9.30	9.55
The contribution of the co		ature C	32.00	26.00	22.50	24.00	32, 00	35.38	36, 00	23. 50	32,00	26.00	27.00	28.00	26.00	30.00	30.00	31.50
Transactional Care at State 2 to 1 to 1 to 1 to 1 to 1 to 1 to 2 to 2		erature C	26.87	S. S.	23.00	23.00	29.06	26.98	32.00	28.80	26.90	24, 40	23, 37	24.06	23. 40	28, 88	31.08	30.04
Control Cont			(S)	9.6			5.5	ر ان	ij. 1	2.0	2,5	2.0	2.5	49 44	5.5	1.0	. S	2.0
			007	ć,		Š	9	0. 95	- C	0.68		0.47	0.39	0.25	0. 47	0.24	0.33	0.31
Colingian Coli			480	2 6	1553	385	č	,			409							
December December		7 (717.1	36	3 -	3 5	3 5	77.0	2 6		•	;	•			;	;	i	•
Control Cont			20.0	2 6		و د د	p. 97	6. 88		5.40	6.31	5. 43	60 60 60	6.96	6.58	6. 50	5.84	9. 27
Second Continue Co				0.00	7.0	5 o	- c	00 c	- :		. 2	6.3	4.3	urs e	1.7		•	•
Dissolved Cop Barria 100 249 250					200.	910.5	30.0	200	cu. 010	0.030	020.0	5.00	0.083	200	0.023	0.772		* .
Discretation Disc		÷	98	3		2 5	2000	140	0067	95	3	n a :	0.82	120	002	021	nn r	Ten.
Paresident Office and Paresident Office and			140	249		320	925	55		,		602	379	900	617	805	4460	2130
Particular Brosshorus as \$71						2	!	3				3	,	14.	:	;	•	
Destrict Proposed State 2.00 4.00 4.00 4.00 1.00 30.09 22.05 3.70 1.97 2.35 1.88 2.27 3.19 45.00 4.00			0.010	0.007	0.003	0.003	0.009	6.020	0.100	0.010	0.00	0.000	0.027	0.021	0.017	0.010	0.026	0.02
Pursentations at \$71 2.0 2.0 53.0 0 2.0 0 10.0 0 1.0 0			2.50	4.00	62.00	4 30	11.00	30.00	22.00	9.00	2. 70	1.97	2.33		2.27	3,30		*
Organic Microgen Reg 7/1 2.0 2.0 1.0 1.2 1.2 1.2 1.2 1.2 1.2 1.2 1.2 1.2 0.7 0.0 0.0 2.0 1.0 0.0 2.0 1.0 0.0 2.0 1.0 0.0 2.0 1.0 0.0 2.0 1.0 0.0 2.0 1.0 0.0 2.0 1.0 0.0 2.0 1.0 0.0			2.00	5.00	25. 25.	2.50												
National Continues Nationa			0.10	5.00	53.00	2 30	10.00				1.25	1. 23	1. 90	1.09	1. 67	3.07	45.00	15.9
Nitrate Nitrogen as Ni 1 2.0 2.0 10 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.			2.40	20.2	8.	2	1.00				1.45	0.74	0.43	0.79	0. 60	0. 23		
No. No.			2.5	2 5	96.5	S 5	0.0		9.00	2.10	1.70	2, 90	2.37	3, 17	3.71	4, 53	6,80	2
Figuratian Figuration Fig			0000	0.07	71.7	0.50	90.0		0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	0.01	0.01	0. 28 1	0. 285	0.017	2. 884	0.126	2, 326	0 .
Diss. Kjeldahl X R 4, 50 6,00 8,00 15,00 25,00		itrogen mg N/1	5.50	32 22	20.07	3 6	0.00	40.00	0. 004 0. 004	5.628	0.010	6.638	6.63	S. C.O.	1.020	007.00	77.7	Ä. 1
Occanic Nitrogen as N.1 3.9 19. 30 19. 30 59. 00 8.70 5. 00 18. 90 18. 10 18. 67 11. 63 11. 33 21. 57 Diss. Nitrogen as Nitroge		ahl N. mg N/l	4, 50	8.00	90 09	8 8	2	00.00	90.00	77.00	.00	20.01	to -1.7	74.00	5.0	70.00		٠
Diss. Organization with 2 to 20 2 to 30 3 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0			3.30	19, 30	26.00	8, 70	5.00		26.00	18.90	16, 30	13, 10		11.63	11, 33	21.57		
Diss. Order lattices m. R. M. 1 5.500 22.08 70.140 11.660 15.081 85.004 21.008 18.010 16.32 21.37 14.82 18.94 26.43 Total Ocal Calculus mg/l 21.5 22.8 7.140 11.660 15.001 11.6 62.0 11.6 62.2 8 7.6 6.2 7.8 6.2 14.0 6.4 Total Ocal Calculus mg/l 21.5 22.8 7.140 11.60 15.00 11.6 12.8 7.14 6.2 7.8 6.2 14.0 6.4 Diss. Ocal Calculus mg/l 21.5 22.8 7.140 11.6 22.8 7.6 6.2 7.8 6.2 14.0 6.4 Diss. Ocal Calculus mg/l 21.5 22.8 7.140 11.6 22.8 7.6 6.2 7.8 6.2 14.0 6.4 Diss. Ocal Calculus mg/l 22.00 25.00 25.00 11.6 2.2 8 7.6 6.2 7.8 6.2 14.0 6.4 Diss. Ocal Calculus mg/l 22.00 25.00 2			2,30	3.30	49.00	6. 70												
Total Continue mg/1 2.1.5 23.5 - 92.0 116.0 460.0 620.0 11.6. 22.8 7.6 6.2 7.8 6.2 14.0 6.4 6.4 6.5 C stealine mg/1 2.1.5 23.5 - 92.0 116.0 460.0 620.0 11.6. 22.8 7.6 6.2 7.8 6.2 14.0 6.4 6.4 C biss. Contractable mg/1 3.8 3.2 8 4.4 6.0 6.0 0.8 6.0 0.8 6.0 11.6 0.1 11.6 22.8 7.6 6.2 7.8 6.2 14.0 6.4 11.0 0.1		-	5.500	22. 080	.0. 140	11,600	15.081		35.004	21.003	18,010	16.32	21.37	14.82		26, 43		
Second Solids		Sen m8 1/7 1 calino m6/1	3.500	9.080 1.080 1.080	50.140	9. 600 9.					1			,	:	:		2
Hexan extractable mg/1 3.8 3.2		741ine mo/1	6.1.2	0.00	. ,	= c	116.0	99	620.0	9	22.8	7.6		 	2.9	14.0	o,	, ,
Heyan cxtractable mg/1 45 23 8 44 64 5 61 61 61 61 61 61 61			60	6		36.0	0		-									
Suspended Solids mg/1 200 35 340 180 85 200 420 420 80 95 153 122 151 357 421 TOC mg/1 * * * * * * * * * * * * * * * * * * *		• •	55	88	00	4)	3	4	÷ &									
TOTAL ANGLO MPN/100m1 CONTROL MPN/100m1 14000 S000 S000 S000 S000 S000 S000 S0			200	33.	340	81	. &	200	45.5	420	08	56	153	122	151	357	421	150
Descrived 10.0 mg/l # # # # # # # # # # # # # # # # # # #			*									21	209	er.	98	215	148	150
Total Cali x1000 MPV/10m1 2200 2000 16000 1000 1000 1000 1000 22000 3000 160000 22000 3000 160000 22000 3000 160000 22000 3000 160000 22000 3000 160000 22000 3000 160000 22000 3000 160000 200000 20000 20000 20000 20000 20000 20000 20000 20000 20000 20000 20000 20000 20000 200					:													
Cadmium March 10044					8	9	24000	11000	9000	20000		22000		30000	16000		0000006	200
Lead mg Poly (1,002 d. 0.002 d				900	052	91.69	30000	20006	16000	00006		28000		30000	160000		0000008	3,6
Copper mg Co./1 (3.05 0.05) 0.02 (3.02 0.05) 0.02 (3.02 0.05) 0.02 (3.02 0.05) 0.02 (3.02 0.05) 0.05 (3.02 0.05) 0.05 (3.05) 0.010 0		7 7 7 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	50.005	6.00	(U. 002	€. 002 9.002	¢0.002	0.005	c0.005	0.005		<0.005	ö	<0: 005	<0.002		200,05	00'00'
Chronium vs Cv. 1 (0.01 (0.01)		1/10/19	20.00	500	20.05	70 °C	20.05	0.02	0.02	0.02		<0.02	_	0.05	0.05		20.00	70,0
Chronium mg Cr/1 (0.01 (Chromium		.0.00	0.020	. o. c.	U. U33	G. 603	07.0	0.020	0.015			~	<0.005	0.010		0. 010	. n
Recent WE High! (0.10 0.50 0.70 0.10 0.10 0.10 0.10 0.10 0.10 0.1	Chronium		5 5	5 5	, , , ,	5 5 7 9	3 6 3 6	5	,	2		,	•	,			10 07	
Zinc mk Zh/1 0.050 0.060 0.300 0.090 0.100 0.030 0.060 0.050 0.100 0.050 0.140 0.050		1/8H 8n	co. 10	0.50	0.30	0 0	5 0	5 C	, c. d.	, c	2 5	5 5	0.00	5 5	9 0	0.10	40.10	0.10
pp bbT ug/1 (0.001 (0.001 (0.001 a) 0.001 (0.001 b) 0.001 (0.001 a) 0.001 (0.0		mg 2n/1	0.050	0.060	0.300	0.090	2 6	0.030	0.060	0.050	0 050	001	21	080	030	060.0	0,140	0.150
op' DDS ug/1 (0.001 (0.001 (0.001 pp' DDS ug/1 (0.001 (0.0		ng/I	<0.001		00.00	(0.001	3	3	3	5	6	3			ò			
pp 205 ug/1 0.01 <0.001		0g/1	<0.001		<0.001	(0.001)												
pp 000 ug/1 <0.001 <0.001		ng/]	0.03		<0.00	(0.00]												
		ng/ I	<0.001		·0.001	(0, 001												

Table APP. 1-4 Results of River Water Quality Analysis of Regular Survey

TO OF SAMPLE NO			2	es	47	w	9	2	69	6	2	Ξ	12	13	14	15	16
DATE OF SAMPLING		MAY 0.5	30% 17	92 90V	62 43S	ØT 19	XOV 19	FEB 11 3	MAR 11 A	APR 12 M	MAY 24 5	JUN 22 J	JEL 20	400 18	0CT19	NOVIB	DEC15
General number of the laboratory	the laboratory	919	5913	9111	10924	11911	13421	1771	2759	4780	7766	9083	10943	12580	15515	15819	18539
CODE PARAMETER	±183																±
Time	æ	10.55				10.55	11. 15	10.30	11.45	12.05	12.30	10.25	10.35	10.00	10.20	9.30	9.45
٠.	stature C	8 8 8	27.00	22.00	25.00	32.00	30,00	34.00	28. 20	31.50	24.00	24.00	27.00	27.00	29.00	30.00	32.00
	later temperature C	26.63				8 8	8 8	32.00	28. 70	28. 20	24. 73	22. 19	23. 93	23. 38	28.17	28.46	28, 90
		10.01					. F	ස ස්	ر د د	တ်	ر. در	3,5	5.5	, i	<u>:</u>	5.5	~
02043F Conduct (Tield)		010	063	000	5	n. 6/		0, 52	0.49	. E	7.	0.24	0. 44	Ç. 4	0.3	60	0 18
72042F Carabat. (11814)	(fight) *	010	25		200	0.00	5			939							
	(11010) A) č	60		9 t	3 8	3 T				0	6	6	10	ě		
		5	- u		5		. ·	· •	, ,	ور ا ا	ő (0.0	6) · ·	5.0	0.02
OSEGUI Compide	72.22		96		010 69) (010	0 6	36	9 6	0 % 0	0.10	10				١
			116		Ģ	6				3 5	140	194	6	100	, i	c	
		28			22	;	i	2	?	:	:	;		:		3	Ş
		69	124		140	260	65	110	9	160	300	329		180	127	187	194
8303L Dissolved COD		•			99												
		,	0.043		0.010	0.040	0.020	0.030	0.010	0.006	0.027	0.090	0.040	0.037	0.030	0.021	0.020
15408L Total Phosphorus			8		2 90	4.30	2.50	4.00	1. 20	2.80	2, 15	2.16	2.01	2.38	2.58		*
	S.		e 8		1.30							-					
15252L Orthophoshate			8		9	3.				1.30	38	 85	 	1.90	1.87	2.32	1.36
			3		28:	9.39				1. 20	0. 78	0. 13	<u>.</u>	0. 78	0, 71		
07556L Ammonia Nitrogen		G. 65	3.30	11.00	3.40	17.88		11.00		200	7.60		5.52	7. 24	6.33	7. 22	5. 11
			2 5		0.20	9.09		0.00	B 9	0.0	0.088	0.014	0.038	6.020	0.016	0,033	
07209L Nitrite Nitrogen	Altrogen ag N/1		200		200	200.0		0: 050	000	0.030	0.002	0.00	0.002	0.053	0.004	0.010	0 003
	Ajeldani Mitrogen az 3/1	20.02	3 6		3 6	67 67	30	23.00	7. 00	90.61	74.00	29. 20	12. 34	23.00	74.21		
		9.00	20.00		3 6	10.00		90	1.70	10 00	16.40	20 90	i o	18.84	17 24		
	=	3 6	25.55		8 8	3		3	÷	3	2	3	5	5			
07801L Total Mitrogen	rogen ag N/1	21. 200	30.065		13, 400	29, 062		23, 020	9, 300	15.010	24.09	38. 28	12.38	23, 95	24, 29		
	rogen ng N/1	19. 200	18.065		7. 400								:		٠		
		17.5	26.5		11.0	50.0	11.0	22.0	11.0	21.6	 8	6.2	% 0 8	9	4.2	5.2	9
			•	7.6	10.6												
	1/88	ξ		=	7	<u>ه</u> ب	•	- • ⇒									
10101 Succeeded Solids	45 -	200	2 5	130	7 =	<u>.</u>	5 5	<u>د</u>	300	140	C.	190	999	94	6.8	666	
:	I CALLES INC. 1	*	3	3	2	3	2	2	3	<u>.</u>	2 2	3	3 52	3 53	9 90	2 67	202
_		*									:		}	•	: :		į
				3000	× 160	1400	30000	8000	30000	7000	17000	13000 7	24000000	30000	80000	24000	80
		<u></u> +		13000	× 160	9000	30000	20000	30000	17000	00006	20000	24000000	00006	130000	30000	
48004L Cadmium	(/p) %u		•	<0.002	0.005	0.005	<0.002	<0.002	<0.002	<0.002	<0.002	<0.002	<0.002	<0.002	<0.002	<0,002	0.025
	ng Pb/1	30.02	98	9.6	<0.02 0.02	<0.02	0.05	6.02	<0 05 <0 05 <0 05	0.02	.0. 0 <u>2</u>	CO. 05	<0.02	6.02	0.20	20.05	91.
ZSUMPL Copper				50.00	9 6	070.0	U. U12	0.010	0.030			0.020	200	÷. 003	0.020	0.013	0.040
AIGIL Chromium IV				5 6	∃ 5 5 €	5 5	6	10 0	. 10 67	5			10 01	ç	50	,	.00
SOULT Vergery	1/20 Sim			, c	9 9	30	3 9	, u	200	5 0	01 (P)	5 5	, c	; C	000	07 0	0.01
				0,040	0.040	0.080	020	<0.005	0.080	0.050	000	0.070	050	060	0.00	0.080	0.020
	ng/	_		0,001	0.00		:										
	1/80	<0.001		<0.001	(0.001					-							
18023L pp. DDE	1/20	. 0.001		(0.001	.0.001												
	[/Sn	<0.001		(0, 001	<0.001												