regulations and standards at national level, is the logical follow-up activity of the recently promulgated national environmental law.

2.5 Available Topographic Map

The topographic maps that have been collected for the study are listed below:

(1)	1:2,000	prepared SECC	PT in	1991,	covering	the	mountain	area of the
		study area,	: •	5. j				· · ·

- (2) 1:10,000 prepared in 1970s, covering the valley floor of the Sula Valley,
- (3) 1:50,000 prepared in 1963-1980, covering the whole country.

The orientation maps of these topographic maps are shown in Figs. 2.3 (1)~(3).

2.6 Available Aerial Photograph

The aerial photographs collected are listed below:

(1)	1:60,000	taken in 1954, covering the whole study area,
(2)	1:10,000	color photos, taken soon after the hurricane Fifi of 1974, covering only a part of the northern lowland area of Choloma,
(3)	1:20,000	color photos, taken soon after the hurricane fifi, covering only a part of the lowland and mountain area,
(4)	1:20,000	taken in 1975, covering a part of the lowland and mountain,
(5)	1:40,000	taken in 1977, covering the whole study area,
(6)	1:10,000	taken in 1989, covering the whole study area,
(7)	1:40,000	taken in 1992, covering almost the whole study area.

The orientation maps of these aerial photographs are shown in Figs. 2.4 (1)~(7).

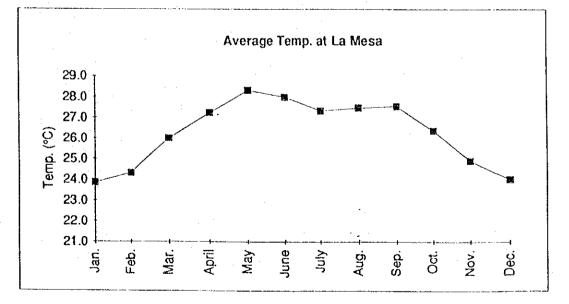
TABLES

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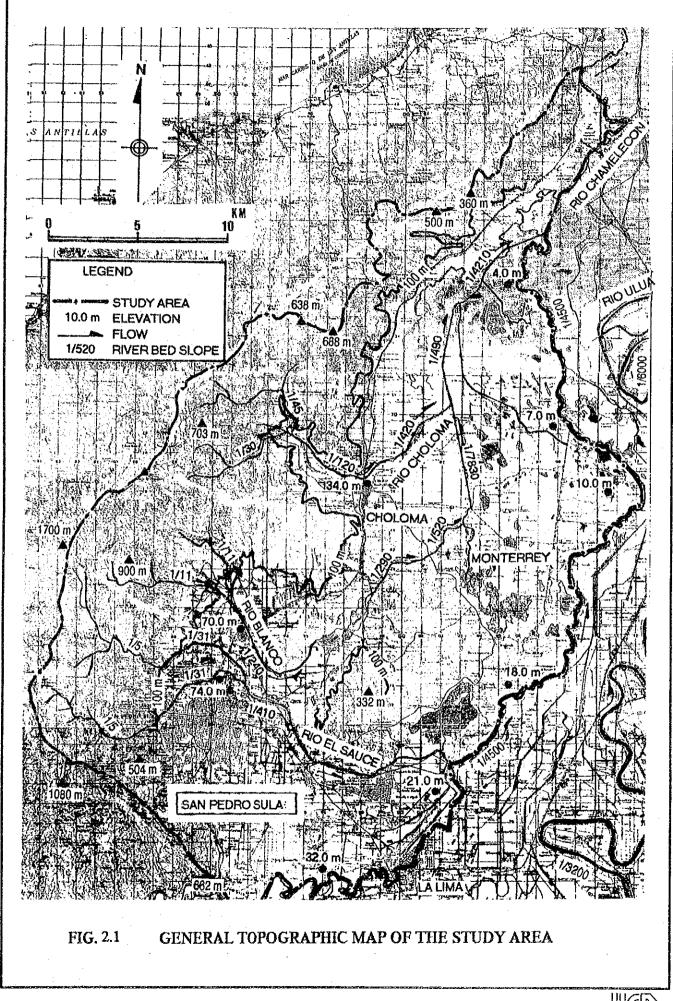
TABLE	2.1	
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MONTHLY AVERAGE TEMPERATURE (LA MESA)

YEAR	Jan.	Feb.	Mar.	April	May	June	July	Aug.	Sep.	Oct.	Nov.	Dec.
1965	22.9	25.1	26.0	27.5	27.8	28.0	27.0	27.2	27.6	26.2	25.3	24.5
1966	23.0	23.5	23,8	26.8	27.0	26.6	26.6	26.7	26.5	25.7	22.7	22.3
1967	22.8	23.4	24.4	26.0	26.5	26.7	-26.1	26.7	26.8	25.4	23.8	23.9
1968	22.8	22.5	23.3	24.9	26.7	26.7	26.1	26.0	26.3	25.4	23.8	23.4
1969	23.6	24.1	25.3	27.0	27.7	27.5	26.6	27.8	28.7	26.6	23.9	23.2
1970	23.9	22.4	25.4	27.2	26.3	27.1	26.7	26.8	26.5	26.2	22.6	23.3
1971	23.2	23.9	24.6	25.1	26.9	26.9	26.6	26.8	27.0	26.2	25.0	24.0
1972	23.9	23.6	25.2	26.9	27.5	27.6	26.5	25.4	26.7	25.1	25.3	22.9
1973	24.6	23.4	27.3	28.3	28.5	28.0	27.3	27.5	27.8	26.5	25.4	22.3
1974	24.5	23.9	26.0	27.3	28.8	28.3	26.9	26.9	27.1	24.9	24.0	23.7
1975	23.9	24.6	26.5	27.1	28.8	28.3	27.6	27.3	26.9	25.6	23.4	22.4
1976	21.8	22.1	25.3	25.6	27.5	26.7	26.6	26.9	27.2	26.0	24.1	23.6
1977	23.1	24.9	26.2	25.6	26.9	26.8	26.8	27.7	27.5	26.3	25.1	24.4
1978	23.3	22.6	24.9	26.8	29.6	27.3	26.5	27.0	27.0	25.9	25.2	24.4
1979	23.8	23.6	26.1	27.8	28.3	27.4	27.8	26.8	26.9	26.3	24.3	23.5
1980	24.1	24.2	26.0	26.1	28.4	26.6	26.8	27.3	27.5	26.3	24.6	22.4
1981	22.0	23.8	26.4	26.5	28.2	27.0	27.0	27.3	27.0	26.3	24.0	23.9
1982	24.6	25.2	26.0	27.5	27.8	28.6	26.5	26.7	27.0	26.8	24.6	24.3
1983	23.8	24.8	26.8	28.0	28.9	29.2	27.0	27.7	27.6	26.6	25.9	25.1
1984	23.1	24.3	26.1	28.0	28.4	27.4	28.1	28.2	27.8	27.9	24.8	24.9
1985	24.2	25.4	27.5	28.2	28.8	29.0	28.3	28.6	28.4	28.2	26.9	25.4
1986	23.6	26.3	26.1	27.6	29.3	29.2	28.0	28.6	28.0	27.4	27.0	25.8
1987	24.1	26.2	28.4	26.8	29.1	30.3	28.5	28.8	29.7	26.5	25.9	26.0
1988	25.0	25.4	26.8	29.2	30.1	30.5	29.0	29:2	29.1	25.9	27.2	24.4
1989	25.3	24.8	26.1	28.3	29.3	29.0	28.7	28.7	28.3	27.0	28.0	24.2
1990	26.9	27.0	27.4	29.5	31.2	29.7	29.9	29.3	29.1	27.3	24.6	25.1
1991	25.8	25.5	27.7	29.5	30.0	29.3	28.1	27.4	27.9	27.5	25.0	25.0
	23.8	24.3	26.0	27.2	28.3	28.0	27.3	27.5	27.6	26.4	24.9	24.0



FIGURES

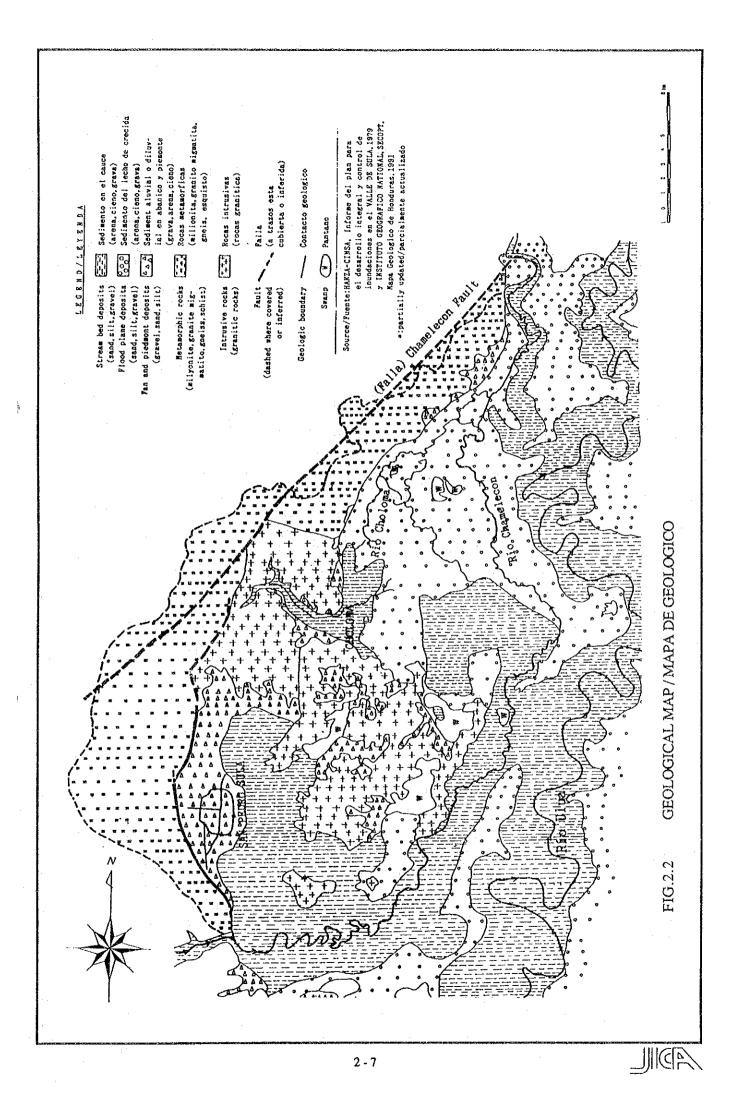


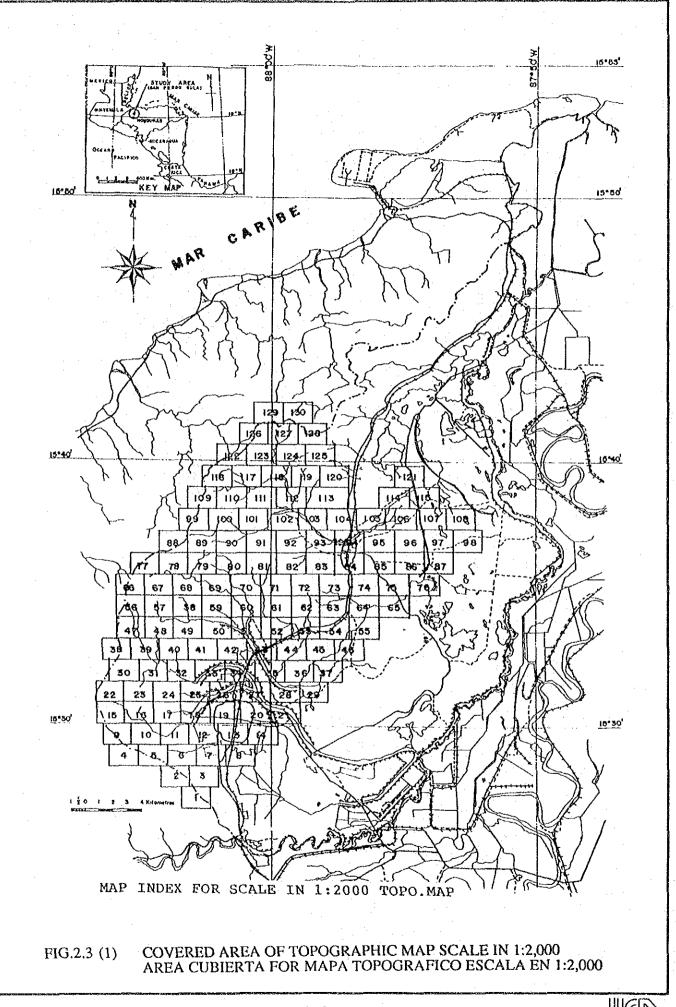
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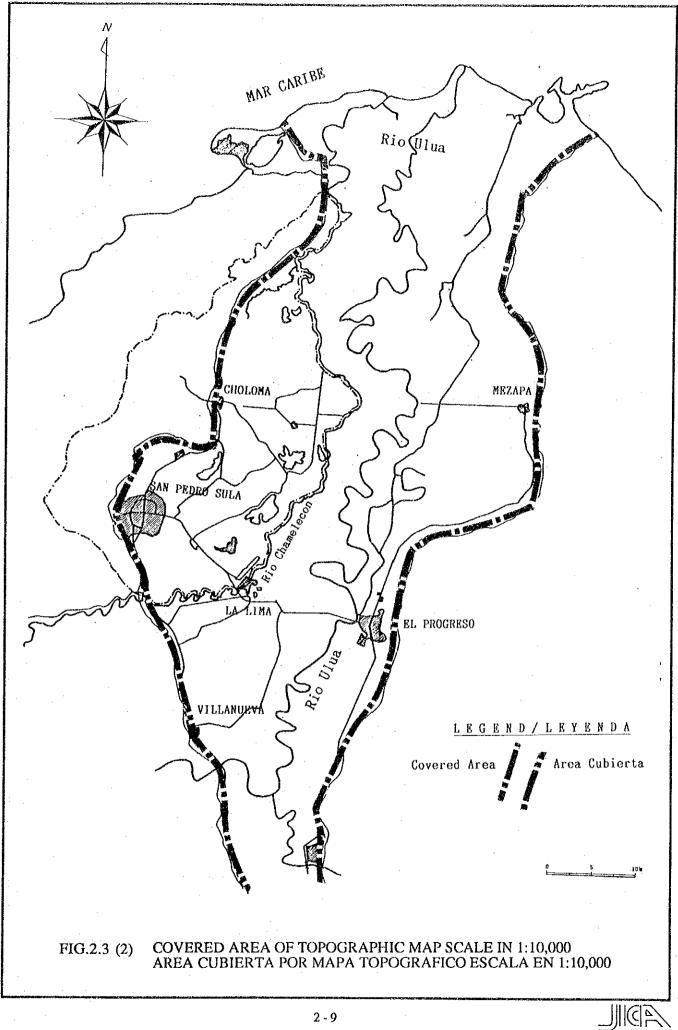
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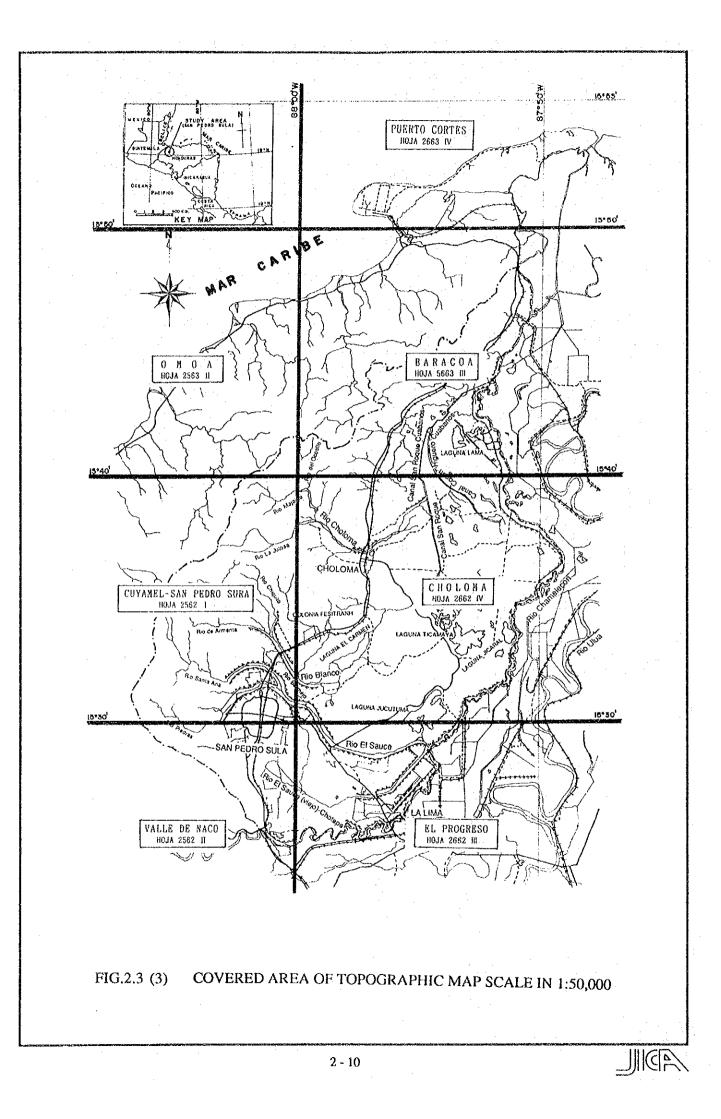
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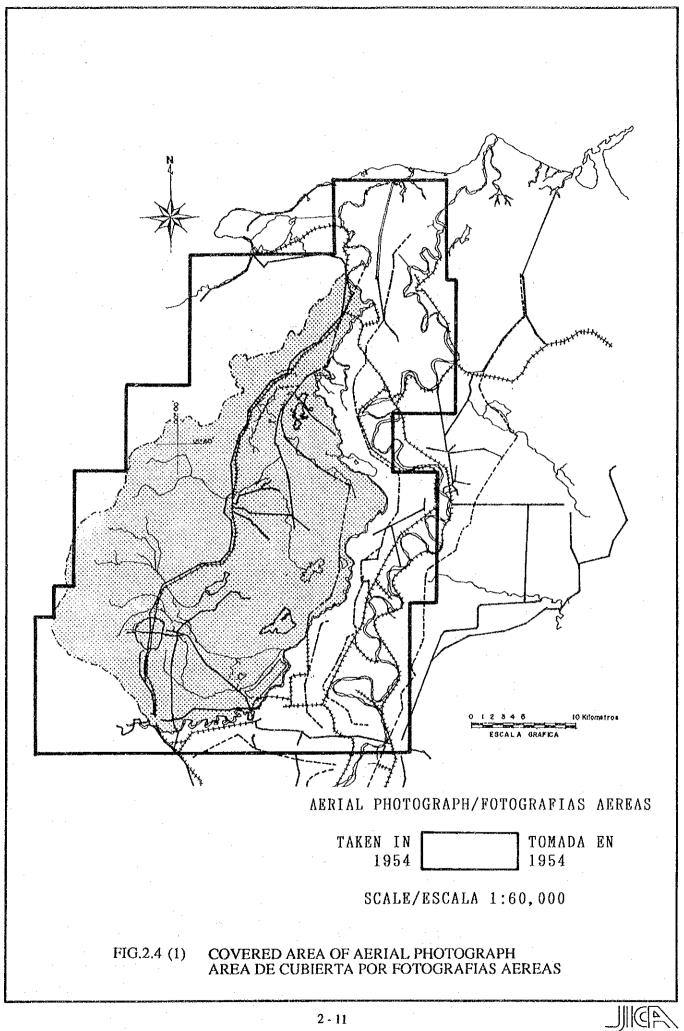


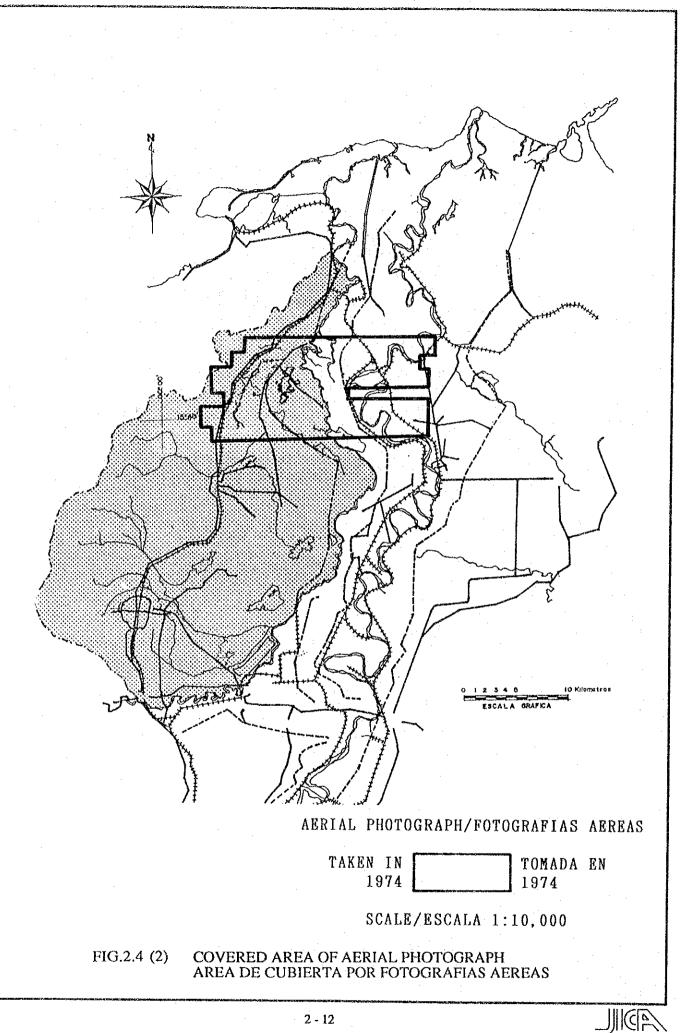


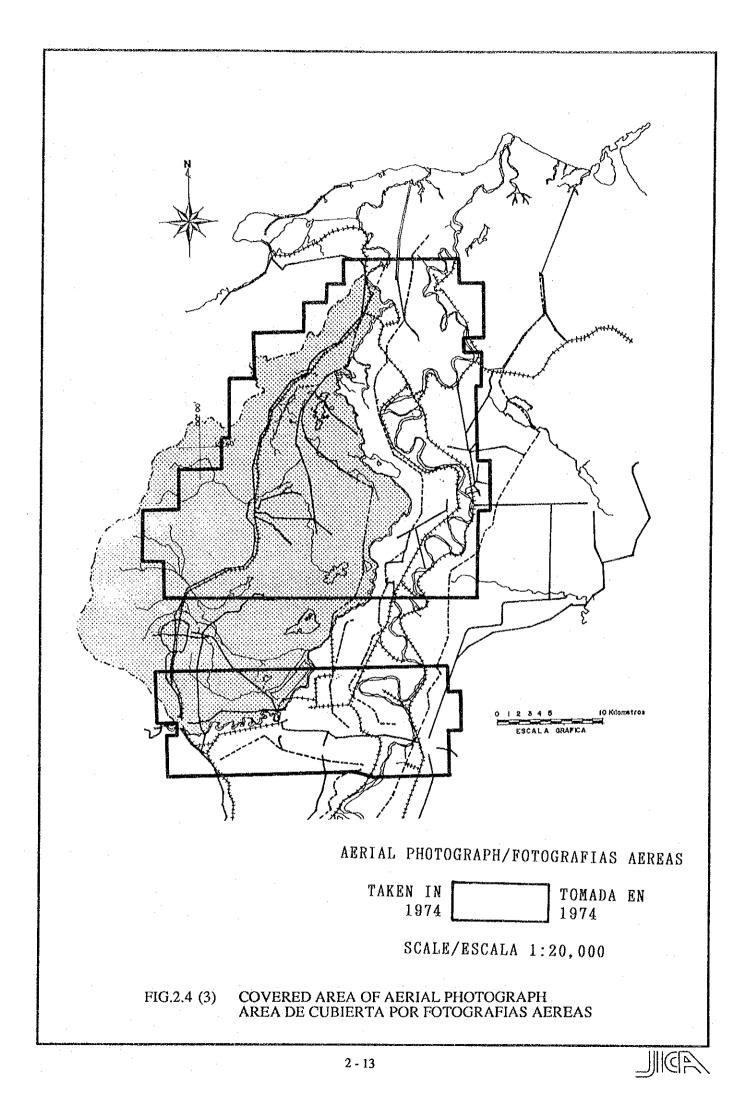
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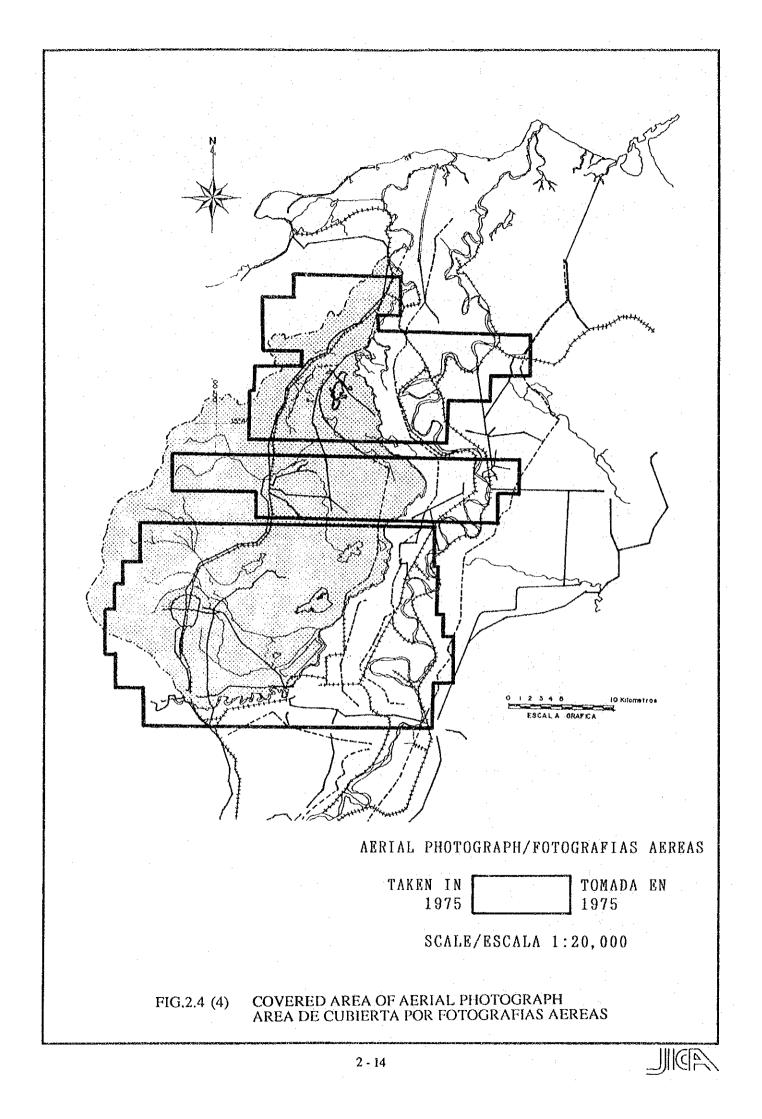


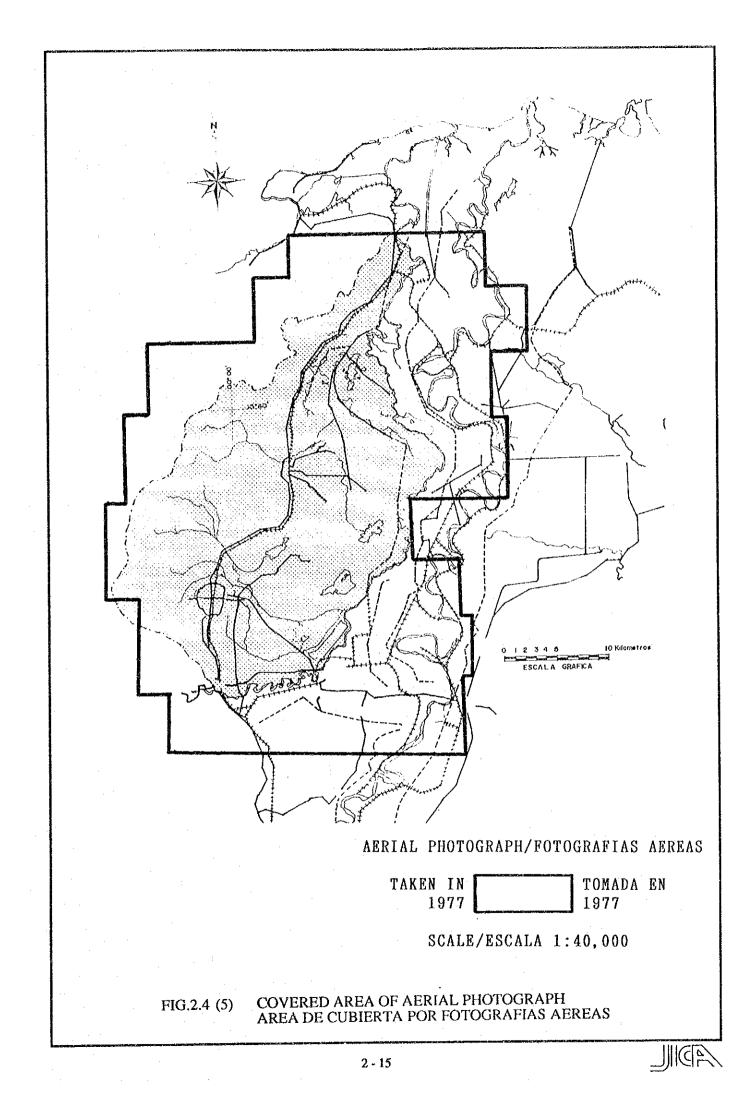


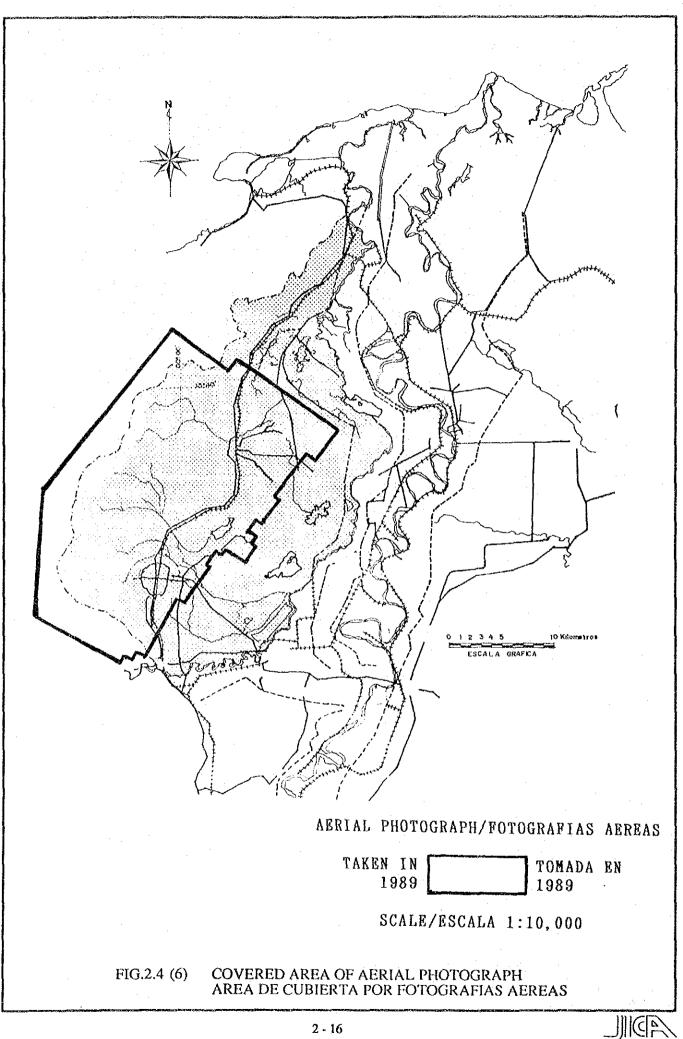


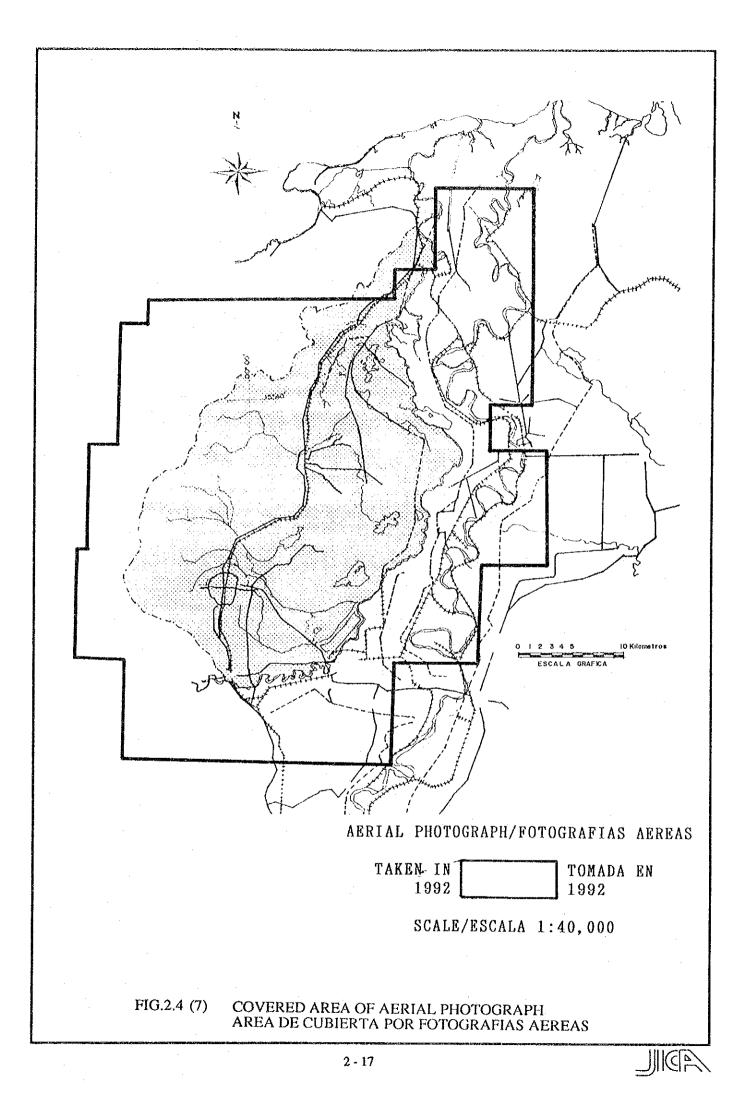














RAINFALL GAUGING STATION INSTALLED BY JICA AT EL ZAPOTAL NORTE (1993)

CHAPTER 3 HYDROLOGICAL STUDY

CHAPTER 3 HYDROLOGICAL STUDY

3.1 General

In order to formulate a Master Plan on erosion, sediment and flood control for the study area, the rainfall and runoff characteristics were studied and assessed based on the collected data. The hurricane Fifi of 1974 and the other storms were assessed from hydrological aspects.

3.2 Rainfall and River Stage Data

3.2.1 Rainfall Data

The rainfall data of thirty four (34) gauging stations in and around the study area have been collected from the National Meteorological Division (Servicio Meteorologia Nacional: SMN), Ministry of Natural Resources (Ministerio de Recursos Naturales: MRN) and Tela Railroad Co (TRRC). Nine (9) rain gauges of them belong to SMN / MRN, and the other twenty five (25) rain gauges belong to TRRC. Those rain gauges and their observation periods are listed in *Tables* 3.1 (1) and (2). The distribution of the rain gauges located in and around the study area is shown in *Fig.* 3.1.

Among those rain gauges the station at La Mesa is only one that has comparatively a long observation period from 1944 to 1991 and also located in the study area.

3.2.2 River Stage and Discharge Data

River water-level and discharge data are available for the Rio Ulua and the Rio Chamelecon, but their tributaries in the study area has no gauging station. The annual maximum water levels and discharges at the Rio Ulua and at the Rio Chamelecon are shown in *Table 3.2*. The gauging stations and their duration of observation are listed as follows.

Gauging Station	Duration	Data		
(Rio Ulua)				
Puente Pimienta	1966~1988	discharge and river stage		
1991~1992				
Santiago	1992	river stage		
Guanacastales	1992	river stage		

Table The Gauging Station and Duration of Observation

(Rio Chamelecon)		
Puente Chamelecon	1966~1973	discharge and river stage
	1980~1989	
Pacmar	1992	river stage

Note: The river stage data of the Puente Pimienta and the Puente Chamelecon are unable to be used for estimation of any flood stage, because of no indication of their datum elevations.

3.3 Rainfall Analysis

3.3.1 Average Rainfall

The annual and monthly rainfall amounts are calculated for La Mesa, El Modelo, Puerto Cortes and Omoa, and shown in *Tables* 3.3~3.6. The tables show that the rainfall amounts at Puerto Cortes and Omoa which are located in the coastal area, are twice as much as those at La Mesa and El Modelo which are located in the inland area.

3.3.2 Maximum Rainfall

Annual and monthly maximum daily rainfalls (from 6:00 AM to 6:00 AM) at both La Mesa and Puerto Cortes are selected and shown in *Tables* 3.7(1)~3.9(2). The tables show that the maximum daily rainfall amounts at both La Mesa and Puerto Cortes are 340 mm and 283.2 mm which are recorded in the hurricane Fifi, but the maximum 6-hour rainfall at La Mesa is 154.4 mm recorded on June 10, 1991. The maximum 6-hour rainfall in the hurricane Fifi is 140 mm at La Mesa.

The hourly rainfall data were also collected, but rainfall recording charts of La Mesa are only available for fifteen (15) years. The maximum hourly and 2-hour rainfalls, based on the rainfall recording charts, are shown in *Table* 3.10.

3.3.3 Frequency Analysis

The frequency analysis of daily rainfalls was conducted for both La Mesa and Puerto Cortes, but the frequency analysis of 6-hour, 2 hour and hourly rainfalls was done only for La Mesa. For frequency analysis the methods of Iwai, Thomas, Hazen, Ishihara/Takase and Gumbel are applied. The results are summarized and shown in Tables 3.11-3.13 and Figs. 3.2-3.4.

3.3.4 Rainfall Pattern of the Hurricane Fifi

The rainfall recording sheet during the hurricane Fifi was collected only for the station at Tela. The hourly rainfall distribution of Tela during the hurricane Fifi is shown in Fig. 3.5.

The hourly rainfall distribution of La Mesa was estimated based on that of Tela and shown in *Figs*. 3.6 and 3.7. Though the total rainfall amounts of the two stations are different but their rainfall distribution during the hurricane Fifi seemed to have a similar hourly distribution.

3.3.5 Rainfall Patterns in the Study Area

The rainfall pattern in the study area is studied based on the collected data from the two stations (La Mesa and El Modelo). The two stations are located nearby each other and their annual average rainfall amounts are similar.

The total rainfalls over 70 mm were selected and shown in Figs 3.8 and 3.9. The relationship between the cumulative rainfall depth and the rainfall duration of each rainfall is shown in Fig. 3.10. The figure shows that there are two rainfall patterns in the study area, i.e., a long duration with a low rainfall intensity and a short duration with a high rainfall intensity.

3.3.6 Rainfall Intensity and Duration

The relation between probable rainfall depths and duration calculated for the rainfall station at La Mesa are shown in the following table.

Table Pro		unit : mm				
Duration (minute)						
	100	50	30	10	5	2
60	81.3	73.2	67.1	53.9	45.2	32.0
120	104.7	93.5	85.3	67.1	53.2	37.1
360	160.4	143.7	131.3	104.2	86.2	59.1
1440	251.1	222.7	201.6	155.5	124.9	78.8

A relation between the rainfall intensity and the duration of a rainfall was decided by the following formula.

 $r = a/(t^{n}+b)$

where,

r: rainfall intensity (mm/hr) t: time duration a, b and n: constant

The constants "a" and "b" for each return period were calculated using n=2/3. The results are shown in the following table.

	Table Constant.			
Return period	a	b	n	
1/100	1365	1.49	2/3	
1/50	1212	1.26	2/3	
1/30	1099	1.07	2/3	
1/10	850	0.48	2/3	
1/5	681	-0.06	2/3	
1/2	439	-1.63	2/3	

Fig. 3.11 shows the rainfall intensity and time duration curves calculated.

3.3.7 Evaluation of Hurricane Fifi

On the basis of collected rainfall data, the hurricane Fifi is assessed and summarized as follows:

- The maximum daily rainfall of 340 mm at La Mesa is estimated to be a storm larger than once in 200-year frequency, however the rainfall amount of 280 mm at Puerto Cortes is estimated to be a storm of once in 20~30-year frequency,.
- The maximum six (6)-hour rainfall of 140 mm at La Mesa, is estimated to be a scale of once in 50~70-year frequency.
- Though the hourly rainfall data have not collected, the maximum hourly rainfall intensity is estimated to be 66 mm at La Mesa. The rainfall amount is likely a storm once in 30-year frequency.

3.4 Flood Run-off Analysis

3.4.1 Run-off Analysis Method

For flood runoff analysis purposes, the Rational Formula, the Unit Hydrograph Method and the Storage Function Method have been studied. The methods for this study are decided to apply the Rational Formula and the Nakayasu's Unit Hydrograph Method that was developed in Japan because of the following reasons:

- The Rational Formula is proper to estimate a peak discharge from a comparatively small basin of less than 200 square km. The method is applied for estimating a peak discharge in designing the erosion and debris control works.
- The Nakayasu's Unit Hydrograph Method is widely used in Japan for planning river works. The method is applied for the flood runoff analysis of the pilot river basins that have a similar topographic condition as those in Japan. By using this method, it becomes possible to estimate not only a peak discharge, but also a shape of flood hydrograph,
- The storage function method is also widely used in Japan for planning river works. The method is applied for estimation of a peak discharge and a shape of hydrograph, but it requires discharge data in order to calibrate the shape of hydrograph estimated by the method. The method is not able to be applied properly for this study, because no required discharge data are available in the pilot river basins.

For this analysis, the study area was divided into twenty-two (22) sub-drainage basins. They are summarized in *Table* 3.14. River system models for both the existing river systems and the proposed alternative river systems are shown in *Figs.* 3.12~3.14 respectively.

In order to select an optimum design rainfall pattern, the following three rainfall patterns were studied and the probable discharge of pattern C was decided to be the design rainfall pattern which gives the maximum discharge in general.

-Pattern A : Maximum rainfall intensity occurs at the initial stage of rainfall,

-Pattern B : Maximum rainfall intensity occurs at the middle stage of rainfall,

-Pattern C : Maximum rainfall intensity occurs at the final stage of rainfall.

The rainfall patterns of A,B,and C used for run-off simulation, are shown in *Fig.*3.15. And the probable discharges by those rainfall patterns are summarized in *Tables* 3.15~3.20. The probable peak discharges by the rainfall pattern C are summarized in *Table* 3.21 and 3.22, and *Figs.* 3.16 and 3.17 respectively.

The shapes of hydrograph were calculated for different rainfall patterns both at the junction of the Rio Choloma with the Canal San Roque and at the river mouth of the Rio El Sauce, and they are shown in *Figs* $3.18 \sim 3.20$.

3.4.2 Simulation of the Hurricane Fifi's Flood

The flood discharge caused by the hurricane Fifi was assessed for the Rio Choloma based on the estimated rainfall pattern with total rainfall of 376 mm. The hydrograph and the peak discharges at major points are shown in *Table 3.23* and *Fig. 3.21* respectively.

The peak discharges by the simulation on the hurricane Fifi are assessed as follows:

- At the most downstream of each basin, the peak discharge by the simulation on the hurricane Fifi, is larger than the estimated peak discharge based on a daily rainfall once in 50-year return period,
- At the mid reach of each basin, however, the peak discharge by the simulation on the hurricane Fifi at the mid reach is similar to the estimated value based on a daily rainfall once in 30~50-year return period,
- At the upper reach of each basin the peak discharge by the simulation on the hurricane Fifi is likely less than the estimated peak discharge based on a daily rainfall once in 30-year return period.
- The Fifi's flood was assessed at the same scale of the flood that was calculated by the daily rainfall once in 50-year return period for the whole basin.

3.4.3 Probable Discharge Distribution

The rainfall once in 50-year return period with the rainfall pattern C was selected for the design rainfall of the master plan. The peak discharges estimated are shown in *Figs.* 3.22 and 3.23.

TABLES

TABLE 3.1 (1) AVAILABLE DAILY RAINFALL DATA

Station Name	Available Period	Managed By
La Mesa	1994 - 1991	SMN
El Modelo	1975 - 1990	MRN
Puerto Cortes	1945 - 1950 1962 - 1980	SMN
Omoa	1987 - 1991	SMN
Guaymas	1978-1990	MRN
Peña Blanca	1956-1977	SMN
Morazan	1966-1981	MRN
Quimistan	1968-1981 1986-1990	MRN
Finca 3	1969-1974	TRRC.
Santiago	1969-1974	TRRC.
Bejuco	1979-1981	TRRC.
Barranco	1969-1980	TRRC.
Oliva	1969-1980	TRRC.
Llano	1969-1980	TRRC.
Higuerito Central	1969-1977	TRRC.
Garroba	1975-1980	TRRC.
Blanco	1969-1980	TRRC.
Progreso	1969-1978	TRRC.
Buena Vista	1969-1988	TRRC.
Cobb	1969-1988	TRRC.
Las Flores	1969-1988	TRRC.
Naranjo Chino	1969-1988	TRRC.
Los Indios	1969-1988	TRRC.
Monterrey	1969-1988	TRRC.
Breck	1969-1988	TRRC.
Palomas	1969-1988	TRRC.
Birichiche	1979-1981	TRRC.
La Fragua	1969-1988	TRRC.
Guanacastales	1969-1972	TRRC.
La Lima	1969-1980	TRRC.
Guarumas	1974-1988	TRRC.
Omonita	1975-1988	TRRC.

Station Name	Available Period	Managed By
San Juan	1969-1988	TRRC.
La Curva	1970-1988	TRRC.
Tacamiche	1969-1988	TRRC.
Copen	1969-1988	TRRC.
Corozal	1969-1988	TRRC.
Mopala	1969-1988	TRRC.
Santa Rosa	1969-1988	TRRC.
Ceibita	1969-1988	TRRC.
Indiana	1969-1988	TRRC.
Caimito	1969-1988	TRRC.
Limones	1969-1988	TRRC.
Laurel	1969-1988	TRRC.
Lupo	1969-1988	TRRC.
Mercedes	1969-1982	TRRC.
Tibombo	1969-1988	TRRC.

TABLE 3.1 (2) AVAILABLE DAILY RAINFALL DATA

TABLE 3.2

ANNUAL MAXIMUM WATER LEVEL AND DISCHARGE (RIO ULUA AND RIO CHAMELECON)

Rio Ulua at Puente Pimienta											
			Perio	d		Month	Date	Water Level (m)	Discharge (qu. m/sec)		
1.	May	1966	1 -	April	1967	Sep.	15	4.58	1,042.00		
2	May	1967	-	April	1968	Oct.	12	3.36	981.20		
З	May	1968	- 1	April	1969	Sep.	21	5.00	1,200.00		
4	May	1969	-	April	1970	Sep.	3	8.50	2,735.00		
5	May	1970	-	April	1971	Aug.	28	4.60	998.00		
6	May	1971	-	April	1972	Oct.	9	4.46	943.00		
7	May	1972	· •	April	1973	Aug.	29	4.16	845.00		
8	May	1973	-	April	1974	Aug.	29	5.58	1,370.00		
9	May	1974	-	April	1975	Sep.	19	7.30	2,170.00		
10	May	1975	-	April	1976	Sep.	24	5.94	1,550.00		
11	May	1976	-	April	1977	June	13	6.92	2,269.00		
12	May	1977	-	April	1978	June	1	4.64	1,400.00		
13	May.	1978	-	April	1979	Sep.	6	6.13	2,104.20		
14	May	1979		April	1980	June	9	6.52	1,756.00		
15	May	1980	-	April	1981	Sep.	13	7.50	2,681.00		
16	May	1981		April	1982	Sep.	25	7.36	1,687.22		
17	May	1982	· •	April	1983	June	13	6.04	1,545.94		
18	May	1983	· •	April	1984	Sep.	29	6.62	1,827.15		
19	Мау	1984	-	April	1985	Sep.	3	6.18	1,653.26		
20	May	1985	-	April	1986	Sep.	28	4.60	1,043.43		
21	May	1986	-	April	1987	Sep.	13	4.82	982.84		
22	May	1987	-	April	1988	Sep.	24	4.90	1,495.17		

Rio Ulua at Puente Pimienta

Max. 8.50 2,735.00

Rio Chamelecon at Puente Chamelecon

		1	Period	t ·		Month	Date	Water Level (m)	Discharge (qu. m/sec)
1	May	1966		April	1967	Nov.	21	4.94	341.60
2	May	1967	-	April	1968	Oct.	19	5.66	561.50
3	May	1968		April	1969	Sep.	25	4.60	552.00
4	May	1969	<u>ب</u>	April	1970	Sep.	3	6.20	833.00
5	May	1970	•	April	1971	Sep	24	4.10	465.00
6	May	1971	•	April	1972	Nov.	21	3.32	322.00
7	May	1972	•	April	1973	July	28	1.58	62.70
8	May	1980	-	April	1981	Oct.	7	5.00	483.00
9	May	1981		April	1982	June	23	5.44	902.71
10	May	1982	-	April	1983	Sep.	24	4.78	701.69
11	May	1983	-	April	1984	Nov.	16	5.06	821.66
12	May	1984	-	April	1985	Sep.	3	5.00	429.28
13	May	1985	•	April	1986	Sep.	27	3.04	205.48
14	May	1986	- :	April	1987	Sep.	29	3.35	349.45
15	May	1987	-	April	1988	Sep.	21	2.74	218.18
							Max.	6.20	902.71

Note: Water level is no an elevation at the station.

TABLE 3.3.

AVERAGE MONTHLY RAINFALL AT LA MESA

	Jan.	Feb.	Mar.	April	May	June	July	Aug,	Sep.	િ ઉત	Nov.	Dec.	Total
	Jan.	Feu.	27.2	30.0	35.1	A DESCRIPTION OF TAXABLE PARTY.	148.6	177.8	150.6	275.3	69.1	131.6	
1944 1945	77.5	35.3	10.0	21.1	70,1	158.2 99.3	111.5	177.8	100.0	146.6	284.2	54.4	
	- 11.5						72.4	31.0	43.7	21.3	28.4	15.0	357.0
1946	8.9	73.2	3.0	38.1	12.7	9.9 74.7	261.1	175.5	152.4	40.1	64.3	69.6	1,008.
1947	44.7		68.1	7.1	17.0	96.5	193.3	1156	217.2	60,7	98.3	80.8	1,352.3
1948	96.5	189.2	10.9	16.5	176.8		95.5	87.4	161.3	75.9	137.9	228.6	975.0
1949	79.2	4.3	19.1	14.0	2.0	70.4	95.5 96.0	66.8	85.1	172.7	135.9	79.0	
1950 1951	161.3 23.1	68.8	85.1 0.0	0.0	101.9	233.7 136.1	113.3	96.5	49.0	1/2./	6.1	10.9	<u>-</u>
		19.1	6.1	73.7	39.9	215.4	162.8	90.0	232.2	159.0	153.7	233.9	
1952	38.6		30.7		213.1	191.3	152.4	140.7	106.7	178.6	210.1	97.3	1,466.
1953	92.2	48.8		4.6	55.4	242.6	108.0	122.9	312.2	263.4	145.8	92.2	1,622
1954	128.5	22.9	47.5	81.0				136,1	222.0	186.7	140.0	206.0	1,261.
1955	65.6	68.1	74	6.9	2.3	36.6	172.5	167.1	222.0	293.4	269.7	218.2	1,717.9
1956	43.4	20.8	30.7	48.0	132.8	181.4	81.0	213.4	85.3	78.7	60.5	138.2	1,384.
1957	126.5	31.5	131.8	3.0	99.1	139.2	276.9 377.2	175.8	137.4	175.8	68.3	46.5	1,504.
1958	47.5	16.5	98.3	3.6	65.5	299.5					161.0	63.8	1.326.
1959	57.4	6.9	37.1	92.7	28.4	305.3	88.4	77.7	169.9	237.7		121.9	1,481.
1960	53.8	41.7	75.7	80.8	38.1	249.9	143.8	86.1	236.2	147.3	205.7	74.4	1,157.
1961	181.1	74.7	45.7	17.5	23.6	63.8	251.7	64.3 87.6	108.2	148.6 297.9	103.4	55.9	1,339.
1962	71.4	25.6	84.1	117.3	45.5	218.4	127.5	141.5	271.8	141.0	114.8	118.6	1,242.
1963	34.3	133.1	112.5	4.8	33.5	69.8	66.5 83.8			127.2	88.9	245.6	
1964	31.0	61.5	2.0	34.5	51.6	263.4		59.9	160.0	127.2		127.3	1,209.4
1965	52.6	-	17.5	8.1	15.7	49.0	329.4	125.5	49.0	63.8 143.0	318.5	81.0	1,450.
1966	51.3	172.7	77.5	48.0	62.0	306.3	88.1	145.8	145.0		129.5		
1967	136.4	98.6	14.7	20.6	20.8	205.7	50.8	83.8	156.7	247.4	222.5	122.9	1,380.
1968	34.8	32.3	44.5	2.5	262.1	123.2	118.6	86.1	221.7	158.5 11.4	146.1 274.1	150.6 66.6	1,381.0
1969	47.5	9.4	52.8	7.6	152.2	84.8	154.4	133.6	174.8			160.5	969.1
1970	47.2	54.6	1.5	0.3	30.7	61.7	121.7	96.0	201.9	69.1	124.5		
1971	83.8	25.4	24.9	27.4	45.7	35.0	52.4	37.8	74.2	46.3	184.6	48.4	685.9 765.
1972	34.8	149.4	30.0	3.5	12.4	151.3	65.8	112.6	103.6	27.5	15.7 108.6	59.1 15.0	765.
1973	13.6	25.6	4.2	77.1	83.0	51.6	119.5	145.8	65.4 549.2	37.6 430.5	90.8	62.4	1,629.0
1974	17.8	22.4	5.9	5.5	17.0	314.5	41.6	71.4				92.8	526
1975	61	1.4	0,0	0.0	22.4	11.9		105.0	53.9	106.7	119.9		1,176.9
1976	266.5	24.6	0.5	76.8	62.4	159.0	60.2	16.3	24.7	155.8	189.1	141.0 112.6	941.
1977	42.8	53.8	13.3	55.9	79.8	176.9	67.3	88.6	58.5	100.2	92.1,	138.4	1,379.
1978	113.2	31.8	78.4	0,3	199.5	145.8 110.1	<u>112.7</u> 90.0	113.8 165.5	140.9	134.6 164.0	170.5 257.7	138.4	1,476
1979	51.2	88.5	15.1	44.7	66.3			71.4	245.5	242.8	181.0	92.6	1,470.
1980	22.4	61,9	24	149.9	41.6	201.5	79.9		309.6			216.5	1,457.
1981	68.4	219.1	28.6	9.9	26.0	243.7	121.6	156.3	146.6	253.6	53.6 92.9	68.7	1,094.
1982 1983	103.2	98.4 9.6	44.6 13.7	9.2 91.1	68,9 13.5	93.8 118.5	94.1 93.1	90,6 152.9	170.7 85.3	159.1	92.9	175.2	966.
1983	82.5	34.6	46.6	3.3	57.0	169.3	204.1	172.4	145.6	65.2	105.0	105.2	1,190
1984	82.5 45.0	34.6	46.6	3.3 46.9	57.0 78.9	109.0	138.9	99.5	97.5	38.7	28.1	72.8	785.
1985	<u>45.0</u> 81.1			46.9	67.7	84.8	195.9	99.5	109.6	181.1	46.7	86.0	1,007.
1986		10.5	45.9 65.7	4.5	0.0	64.6	245.0		93.8	68.3	231.9	221.8	1,184.
1987	74.9	10.8			0.0 79.9			103.6 211.5			36.7	253.2	1,104.5
	138.2	93.6	34.5	25.0		146.4	189.6		152.1	327.0		129.3	
1989	86.6	70.9	6.7	12.4	49.4	88.6	84.4	78.0	184.2	137.8	128.2		955.
1990	44.1	16.0	51.4	1.3	52.6	232.2	35.2		89.7	75.5	157.2	93.1	
1991	60.0	46.4	44.7	48.3	63.2	68.5	53.9	114.3	101.5	59.0	424.7	67.9	1,152.
Ave.	71.2	54.8	35.8	31.4	63.3	145.1	129.1	113.2	153.6	144.0	138.7	115.0	1,185.

(unit : mm)

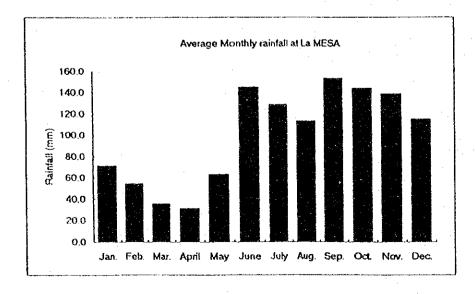


TABLE 3.4

AVERAGE MONTHLY RAINFALL AT EL MODELO

	Jan.	Feb.	Mar.	April	May	June	July	Aug.	Sep.	Oct.	Nov.	Dec.	Total
1975	82.4	7.6	57.7	2.7	2.9	42.2	48.1	76.1	87.4	76.3	102.6	72.5	-
1976	199.3	21.4	2.1	40.3	160.5	138.5	118.8	92.8	90.3	196.4	198.5	136.6	1.395.5
1977	50.6	55.7	13.7	63.2	107.5	395.6	74.5	157.7	÷.	42.2	119.1	82.6	_
1978	75.0	134.7	86.5	3.2	165.4	135.6	174.7	101.0	203.0	-	178.0	141.3	1.398.4
1979	53.7	80.5	26.6	17.6	-	-	-	-	-	-	-		-
1982	137.4	132.5	35.7	61.6	84.3	120.0	61.3	-	-	-	72.9	58.0	-
1984	77.3	30.4		-	108.9	-	258.4	179.0		65.7	69.2	103.4	- 1
1985	55.9	28.7	12.9	50.0	53.9	71.4	209.7	100.8	171.7	24.9	46.0	53.4	879.3
1986	89.2	11.8	36.0	0.7	78.4	100.2	201.7	123.2	122.4	190.0	58.7	78.9	1,091.2
1987	82.4	7.6	57.7	2.7	2.3	87.3	213.1	153.9	110.7	62.0	210.2	209.6	1,199.5
1988	107.3	94.8	40.5	29.5	63.5	144.4	217.6	118.4	155.2	267.3	47.0	209.0	1,494.5
1989	87.9	56.5	7.3	4.9	59.3	91.8	46.0	151.3	162.1	112.7	-	-	-
1990	-	-	ш I	-	-	-	-	132.0	104.8	20.1	173.9	24.4	-
Ave.	91.5	55.2	34.2	25.1	80.6	132.7	147.6	126.0	134.2	105.8	116.0	106.3	1,243.1

(unit : mm)

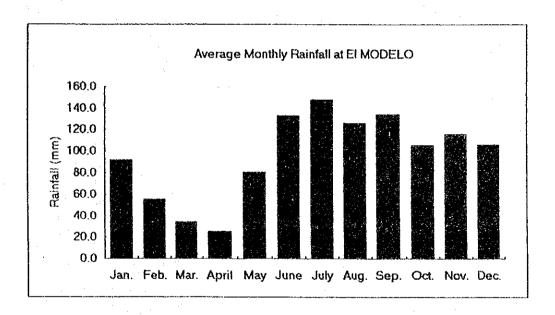
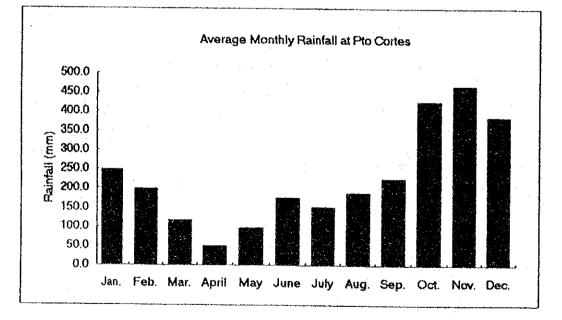


TABLE 3,5

AVERAGE MONTHLY RAINFALL AT PUERTO CORTES

	Jan.	Feb.	Mar.	April	May	June	July	Aug.	Sep.	Oct.	Nov.	Dec.	Tota
1945	+	23.9	8.9	0.0	204.2	90.7	188.5	380.0	602.7		1404	L Dec.	
1946	225.3	92.5	78.0	25.4	9.4	138.4	117.1	103.9	109.5	430.3	589.0	390.1	2.308.
1947	110.2	295.1	201.7	92.7	34.5	99.8		100.3	128.5	-216.4	284.7	577.6	2,000.
1948	660.4	165.9	35.3	50.5	104.1	153.2	147.8	117.3	172.2	450.1	235.5	237.0	2.529.
1949	162.6	19.3	24.4	48.0	18.0	47.2	115.6	125.5	76.7	246.9	447.5	570.7	1.902
1950	144.8	170.4	21.3	27.2	1.3	252.2	256.5	169.9	110.0	777.5		1 3/0.7	1,502.
1962	-	-	25.1		68.6		293.6	117.1	309.6	427.8	390.9	253.0	
1963	146.3	185.7	322.6	49.3	72.9	54,4	62.5	137.4	241.8	591.1	528.6	f	2,779.
1964	142.7	191.5		5.1	23.6	243.8	117.1	286.8	163.8	408.7	409.4	503.7	2.119.
1965	374.4	188.0	68.1	62.0	34.3	169.9	178.3	211.3	158.2	646.4	509.8	501.9	3,102
1966	467.4	574.3	243.8	28.2	91.2	454.7	176.5	94.7	94.2	631.7		301.3	0,10Z.
1967	387.1	301.2	79.2	121.4	127.3	243.8	193.3	157.7	221.7	467.4	525.5	156.7	2,982.
1968	195.1	196.1	113.7	22.6	100.3	82.8	143.0	278.6	138.7	413.5	457.2		2,729
1969	254.0	72.1	327.9	10.4	242.3	244.9	111.8	78.0	507.0	340.9	807.7	312.4	3,309
1970	294.9	242.6	50.3	25.4	115.1	223.3	134.1	87.4	258.1	252.7	385.3		2,501.
1971	244.1	179.8	131.8	25.4	35.3	96.5	86.6	244.4	146.3	112.0	498.6		2,113.
1972	211.3	459.0	47.8	46.5	95.0	242.8	193.3	209.6	268.0	181.4	111.0		2,386.
1973	77.0	232.9	131.8		242.3			160.7	242.0	178.7	263.4	211.3	2,000.
1974	83.9	89.1	32.0	25.0	85.8	155.3	120.0	153.7	578.5	697.9	210.8		2,403.0
1975	170.7	34.8	0.0	1.8	35.3	78.5	67.4	159.9	182.6	579.8	547.3		2,297.0
1976	458.5	130.9	8.1	156.5	105.9	242.8	185,7	217.6	155.6	656.6	742.8		3,531.2
1977	121.5	210.2	50.5	210.6	171.2	155.8	170.6	131.6	77.3	251.1	249.8		2,111.
1978	349.0	153.8	543.6	20,4	78.0	128.5	150.4	301.0	171.8	316.6	492.7		3,329.1
1979	220.9	374.1	93.9	52.8	247.0	261.6	137.6	403.1	288.0	539.7	1,118.4	327.3	4,064.4
1980	189.7	178.2	162.6	- -	•	-				-		- ULT.U	7,004.
Ave.	247.5	198.4	116.8	50.3	97.6	175.5	152.2	188.1	225.1	426.7	466.9	385.6	2,728.3



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AVERAGE MONTHLY RAINFALL AT OMOA

	Jan.	Feb.	Mar.	April	May	June	July	Aug.	Sep.	Oct.	Nov.	Dec.	Total
1987	140.6	140.9	126.7	55.7	2.7	37.7	214.1	232.1	210.7	216.6	505.1	429.2	2,312.1
1988	459.6	511.6	144.0	116.3	9.4	179.8	253.7	183.5	107.0	1,008.4	180.7	559.8	3,713.8
1989	252.4	136.3	14.5	11.0	15.9	16.9	142.5	100.0	156.3	492.0	706.7	428.2	2,472.7
1990	277.6	241.3	244.8	-	82.8	144.4	254.0	406.5	564.9	226.5	1,031.0	238.8	-
1991	147.7	14.7	94.1	9.9	267.5	91.5	147.4	167.1	257.6	163.1	693.9	189.7	2,244.2
Ave.	255.6	209.0	124.8	48.2	75.7	94.1	202.3	217.8	259.3	421.3	623.5	369.1	2,685.7

(unit : mm)

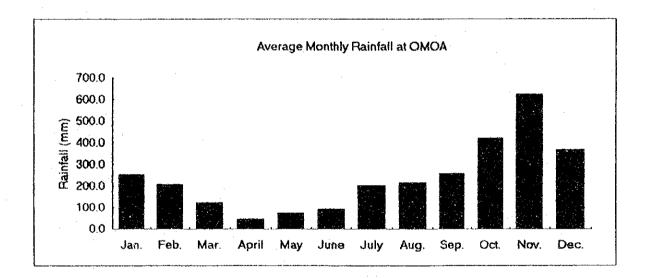


TABLE 3.7 (1)

MONTHLY MAXIMUM ONE DAY RAINFALL AT LA MESA

- Contactor - Cont	Tradition and the State	Jan.	Feb.	Mar.	April	May	Júne	July	Aug	Sep.	Oct.	Nov.	Dec.	Max.
1944	Date		-	9	3	31	17	27	25/27	3/21	25	9	1	
	Rainfall		†	10.9	30.0	32.0	52.1	21.1	33.0	52.1	53.1	17.0	30.0	53.1
1945	Date	2	25	10	6	21	17	17		52.1	27	23	10	35.1
	Rainfall	11.9	8.9	9.9	9.9	23.1	52.1	24.4			37.1	48.0	20.1	52.1
1946	Date	24	15	9/10	19	5	16/17/26/20	28	12	20	3	27	4	JZ.1
	Rainfall	3.0	45.0	10.2	25.9	8.9	2.0	58.9	23.9	14.0	4.1	14.0		50.0
1947	Date	9	14	16	27	9	16	8	20.9	6	5	14.0	9.9 6	58.9
	Rainfall	13.0	8.1	52.1	4.1	11.9	11.9	97.0	24.9	32.0	22.1	35.1		070
1948	Date	9	2/3	13	3	25	10/21	18	3	24	3	11	20.1 30	97.0
	Rainfall	27.9	52.1	10.9	16.0	55.9	21.1	42.9	22.1	52.1	13.0	35.1		EE O
1949	Date	11	10	20	6	22	22	25	12		5		52.1	55.9
	Rainfall	17.3	2.3	9.9	14.0	2.0	23.1	34.0	12	26	11	13	25	AP A
1950	Date	21	2.5	30	14.0	2.0	28		and and an inclusion of the second	45.0	30.0	43.9	34.0	45.0
1000	Rainfall	36.1	11.9	48.0				16	29	14	25	13	11	
1951	Date	25	3	<u></u>		28	50.0	23.4	50.0	39.9	56.9	19.3	22.1	56.9
	Rainfall	9.1	6.4			28 53.3	23	23	23	9		18	17/27	07.0
1952	Date	9.1 13	27	7	27	<u>53.3</u> 12	97.3	26.4	40.6	21.1		2.8	2.0	97.3
	Rainfall	10.4	17.0	6.1	34.5	15.2	7	19		30	8	27	11	
1953	Date	24	18	27	22	27	42.4	57.4		48.3	31.0	23.9	45.2	57.4
1350	Rainfall	53.3	13.0	12.7	3.3		23	26/27	15	25	30	9	31	
1954	Date	7	13.0		22	61.0	48.3	26.7	12.7	24.6	88.9	61.0	31.8	88.9
1004	Rainfall	32.0	10	4		12	14	18	15	27	3	3	15	
1955	Date	31	10.4	25.7	23.9	15.5	45.0	51.3	30.0	148.6	55.4	61.7	17.5	148.6
1333	Rainfall	33.0	13.5	28	16	31	21	3	30	2	31	10	16	
1956	Date		29	2.5	2.5	1.3	10.9	25.7	39.6	56.4	51.6	56.4	68.1	68.1
1300	Rainfall	4 17.5		25	11	25	.10	6	24	15	27	10	27	• • • <u>-</u>
1957			14.0	14.2	24.6	26.7	33.8	16.5	36.1	50.3	137.2	64.8	49.0	137.2
1957	Date	18	11	26	15	7	_21	29	15	11	25	9	9	·
1958	Rainfall	23.6	10.7	71.9	1.8	45.7	34.3	49.8	52.3	38.6	37.8	22.4	36.1	71.9
1906	Date	4/16	3	15	13	16	12	11	10	27	20	- 3	21	·
1959	Rainfall	10.7	7.6	70.4	3.3	33.8	80.5	37.1	48.8	36.1	43.7	28.4	16.5	80.5
1909	Date Rainfall	16	2	9	15	31	20	19	6	22	25	28	24	
1960		12.7	4.8	18.8	31.5	17.8	58.2	13.5	32.8	34.3	62.0	28.4	36.3	62.0
1900	Date	8	26	5	11	23	9	9	28	10	23	25	13	
1961	Rainfall Date	13.0 17	22.6	29.5	41.9	16.0	77,7	31.0	48.0	36.8	31.2	55.4	35.1	77.7
1301	Rainfall		4	9	20	25	18	23	14	4	16	5	30	•
1962		56.9	29.2	21.6	9.9	11.7	16.3	102.9	17.5	29.5	34.3	28.4	14.7	102.9
1902		17	12	6	17	2	11	5	14	27	4	25	30	
1062	Rainfall Date		12.2	51.3	49.0	14.0	52.3	18.5	12.4	39.1	94.7	23.1	15.0	94.7
1300	Rainfall	31	20	23	15	20	.12	28	17	24	11	2	17	
1064	Date	8.9	81.5	42.7	. 3.0	9.1	33.5	20.6	47.0	66.8	23.4	30.2	21.6	81.5
1304	Contractor and the second	10	29	31	17	31	6	29	13	13	20	7.	7	
1065	Rainfall	16.5	25.9	2.0	15.0	30.5	47.2	17.3	14.7	41.1	37.3	38,1	143.3	143.3
1965			25	5	29	1	10	17	15	21	30	5	9	
1000	Rainfall	- 20	17.5	8.1	9.4	31.8	48.8	42.2	13.2	12.7	120.4	39.9	43.9	120.4
0061	Date	26	4	14	10	30	4	6	17	30	25	20	11	
1067	Rainfall	15.5	75.4	29.0	18.8	26.9	80.0	21.1	40.9	20.3	54.1	34.0	25.9	8 0.0
1907	Date	29	24	10	4	24	19	16	30	26	19	6	.23	
	Rainfall	34.3	35.3	6.4	12.2	10.7	46.0	9.1	39.4	36.3	75.4	51.8	61.0	75.4

 TABLE 3.7 (2)
 MONTHLY MAXIMUM ONE DAY RAINFALL AT LA MESA

		Jan.	Feb.	Mar.	April	May	June	July	Aug.	Sep.	Oct.	Nov.	Dec.	Max.
1968	Date	13	19	17	11	25	6	31	12	24	18	19	23	
1300	Rainfall	18.3	15.7	17.8	1.8	101.3	39.9	33.8	·11.9	95.5	42.9	50.8	42.7	101.3
1969	Date	5	3	31	1.0	101.3	-5 -5	12	28	95.5 2	42.9	<u> </u>	42.7 11	101.5
1905	Rainfall	22.1	6.1	17.8		48.5	19.8	27.9	27.9	60.2	.13.2	62.2	14.5	62.2
1970		6	21	14	11		······································		27.9	21				02.2
1910	Date Rainfall	11.4	10.7			16	. 10	.13			30	23	6	
1971		20	9	<u>1.0</u> 4	0.3	21.3	16.8	35.3	36.6	71.1	26.2	22.6	38.6	71.1
1971	Date	34.3	12.4	19.8	6		11	28	30	10	21	<u>11</u>	23	
1972	Rainfall	16	4	19.0	19.1	20.3 19	11.0	12.7	26.2	33.7	16.5	62.1	30.0	62.1
1912	Date Rainfall	14.9	4 54.4		2		20	8/19	12	2	10	8	16	
1072		14.9		13.2	3.3	6.6	31.0	14.0	23.6	45.7	11.4	4.4	29.7	54.4
1973	Date	2.6	4	17	28	14	20	18	31	18	11	6	7	50.0
1074	Rainfall		9.3	2.2	52.9	33.8	23.1	16.5	28.2	14.2	10.6	28.6	5.5	52.9
1974	Date	3	26	8	17	20	25	26	25/31	18	22	12	1	<u> </u>
1075	Rainfall	11.0	18.6	3.3	2.8	12.3	113.4	8.0	23.0	340.0	80.4	22.1	31.7	340.0
<u>1975</u>	Date	20	13/24			23	13	22	13	11	31	23	7	
1070	Rainfall	3.0	0.5			11.0	5.0	2.9	22.1	12.0	37.9	43.0	26.7	43.0
1976	Date	9	1	17	26	17	3	5	1	3	10	7	16	
	Rainfall	95.9	6.1	0.5	49.0	18.8	37.4	20.9	5.2	8.0	46.0	46.1	31.3	95,9
1977	Date	17	16	1	23	3	6	13	21	22	13	11	21	· · · · · · · · · · · · · · · · · · ·
	Rainfall	24.1	36.0	4.2	31.4	30.9	32.1	20.0	28.0	23.2	44.9	50.1	32.1	50.1
1978	Date	28	21	27	14	28	22	25	24	18	19	19	13	*·-··
	Rainfall	50.9	16.8	21.5	0.3	100.3	20.3	33.6	50.0	26.9	46.8	73.0	56.3	100.3
1979	Date	9	8	15	17	7	23	22	29	3	11	297	25	
	Rainfall	12.1	23.5	6.9	38.0	22.2	24.0	30.5	36.5	42.5	34.6	83.2	50.6	83.2
1980	Date	4	3	15	17	22	15	_27	3	13	5	5	13	
	Rainfall	10.8	14.3	1.4	93.0	39.4	34.8	21.5	18.2	96.4	78.7	44.2	25.0	96.4
1981	Date	10	14	17	18	4	1	12	17	9	18	2	5	· · · · · · · · · · · · · · · · · · ·
	Rainfall	13.8	56.2	16.4	6.3	16.9	37.2	38.8	.43.1	35.4	102.6	28.4	62.8	102.6
1982	Date	11	26	1	30	28	13	4	- 28	14	4	2	2	
	Rainfall	36.9	33.8	28.6	28.1	12.8	42.0	32.7	19.8	44.0	65.1	43.7	47.4	65.1
1983	Date	11	26	11	16	29	. 14	31	26	15	25	16	25	
	Rainfall	30.6	3.8	10.3	44.5	13.5	31.3	11.1	27.4	21.2	7.5	61.8	65.6	65.6
1984	Date	15	5	20	5	30	15	29	25	10	26	21	·7	
	Rainfall	35.4	24.2	24.7	2.1	14.7	57.5	51.9	33.9	28.6	27.8	33.2	46.0	57.5
1985	Date	22	12	6	2	6	13	4	17	22	16	23	25	
	Rainfall	22.5	12.6	5.8	41.2	46.2	40.0	35.4	37.7	20.0	13.6	12.2	14.8	46.2
1986	Date	19	13	21	21	25	7	19	22	11	28	14	6	
	Rainfall	28.3	3.6	18.9	1.8	17.6	17.8	40.3	17.3	41.2	50.6	10.5	23.2	50.6
1987	Date	11	7	4	27	İ	9	10	25	1	29	11	30	
	Rainfall	36.9	7.8	21.8	1.7		23.5	31.4	32.9	31.0	22.6	69.3	50.5	69.3
1988	Date	11	21	15	11	30	20	29	24	29	12	22	2	
	Rainfall	34.4	39.8	20.7	14.0	67.6	69.6	48.0	65.5	38.1	54.1	14.6	146.0	146.0
1989	Date	21	22	7	18	12	19	23	20	11	11	30	13	
	Rainfall	26.4	19.5	3.3	4.8	30.4	17.4	46.7	20.3	55.0	53.8	31.2	43.7	55.0
1990	Date	25	12	20	27	23	17	30	11	27	25	29	4	
	Rainfall	23.9	31.8	32.4	25.6	14.2	20.0	13.5	15.0	36.4	34.4	104.5		104.5
1991	Date	7	15	10	1	26	9	4	6	20	19	25	14	· · · · · · ·
-	Rainfall	18.1	6.5	37.2	1.2		154.4	· · · · · · · · · · · · · · · · · · ·		20.4	19.4	37.5	20.3	154.4

TABLE 3,8

MONTHLY MAXIMUM ONE DAY RAINFALL AT PUERTO CORTES

				and the second										a a substantia a su
	Classes distantice cares	Jan.	Feb.	Mar.	April	May	June	July	Aug.	Sep.	Oct.	Nov.	Dec.	Max.
1945	Date		3	22		25	20	2	31	28	-	-	-	
	Rainfall		12.7	3.6		66.0	43.4	63.0	154.9	160.5				160.5
1946	Date	25	5	5	28	5	11	.18	21	2	30	28	5	
	Rainfali	86.4	21.6	21.6	11.4	8,4	54.9	21.3	35.8	18.0	154.2	180.8	121.9	180.8
1947	Date	24	26	17	17	9	13	-	-	13	21	28	23	
	Rainfall	38.1	76.2	70.4	48.3	24.1	34.8	·.		42.7	80.0	85.1	123.2	123.2
1948	Date	31	3	12	18	21	25	8	19	17	20	12	30	
	Rainfall	120.9	64.8	23.1	28.2	35.1	76.2	31.2	48.5	41.9	97.8	94.7	100.3	120.9
1949	Date	12	2	1	6	23	20	30	31	6	15	1	25	
	Rainfail	47.8	5.3	12.7	22.4	16.3	17.0	40.9	41.9	22.9	15.2	70.9	153.2	153.2
1950	Date	22	17	31	7	8	9	11	12	22	25			
	Rainfall	30.0	44.2	17.8	25.1	1.0	56.9	73.2	37.3	36.8	163.1	43.9	34.0	163.1
1962	Date			17	-	6		17	2	21	3	11	14	
	Rainfall			17.0	·····	18.5		41.9	48.3	178.8	154.9		118.4	178.8
1963	Date	21	14	23	4	30	24	14	16	24	5	18	17	170.0
	Rainfall	33.0	70.1	118.6	24.1	36.3	29.0	16.0	55.6	40.6	77.2	214,9	103.4	214.9
1964	Date	10	28	110.0	16	15	21	20	22	16	20	8	100.4	614.3
1001	Rainfall	40.6		69.9	5.1	7.9	51.1	37.1	52.1	38.1	85.1	76.2	150.6	150.6
1965	Date	17	25	4	30	2	9	13	30	14	30	6	8	100,0
1305	Rainfall	125.0	80.0	31.8	30.5	15.2	64.0	57.7	30.7	31.8	143.5	and in the second	83.3	143.5
1966	Date	6	4	30	6	10	4	8	1	9	29		00.0	140.0
1300	Rainfall	171.2	157.7	71.9	17.3	27.7	207.0	74.9	22.9	34.8	175.3	<u> </u>		207.0
1967	Date	29	12	31	5	23	207.0	24	20	-34.8 	9	5	- 22	207.0
1501	Rainfall	101.6	71.1		100.1	and a sector of the sector of				and a second second		83.8	23	101 0
1968	Date	16	5	34.3 22		71.1 5	51.1	34.3 5	63.2 12	73.4	80.0 24	03.0 19	43.2	101.6
1900	Rainfall	22.4	74,4	44.5	6 14.7		20			24			7	100 7
1969	Date	19	4.4	44.5		127.0	25.4	50.8	119.9	34.3	130.8	127.0	139.7	139,7
1909	Rainfall	82.6			2	28	28	19	26	2	24	19	12	
1970		82.0 9	53.3 1	127.3	7.9	81.3	81.3	20.1	29.2	144.8	92.7	157.7	88.9	157.7
1970	Date			13	3	16	8	11	22	7	20	14	5	
1071	Rainfall	74.9	34.3	16.8	24.6	50.8	62.0	35.3	12.7	97.0	33.3	81.3	46.2	97.0
1971	Date	16	9	.4	3	15	5	5	29	11	13	20	12	
1070	Rainfall	49.5	66.0	43.2	24.6	18.8	50.8	13.0	50.8	50.8	50.8	102.6	88,9	102.6
1972	Date	17	3	10	2	31	11	19	15	27	3	8	17	
1070	Rainfall	59.7	114.8	25.9	26.7	59.7	74.9	33.3	39.6	45.7	62.0	33.5	123.2	123.2
1973	Date	28	4	3		28		-	8/9	2	25	29	7 :	:
	Rainfall	24.6	50.8	45.0		81.3			40.1	49.5	43.4	48.3	113.5	113,5
1974	Date	8	28	13	5	22	28	9	31	18	10	12	17	
	Rainfall	12.7	52.1	19.3	7.1	71.6	43.2	55.9	45.2	283.2	136.7	59.7	87.9	283.2
1975	Date	_27	13		4	13	28	1	17	29	28	23	- 26	
	Rainfall	39.1	11.2		1.0	25.7	46.2	41.9	35.3	46.0	107.2		91.2	107.2
1976	Date	9	23	5	25	18	4	23	6	12	20	23	23	. <u>.</u>
	Rainfall	131.3	44.7	4.3	94.0	49.8	78.7	88.9	27.2	33.0	165.9		101.6	231.1
1977	Date	18	16	23	23	31	1	2	7	17	14	25	21	
	Rainfall	40.6	68.8	36.6	50.8	78.2	58.4	37.1	21.1	17.8	83.1	62.2	132.1	132.1
1978	Date	29	4	4	14	19	16	20	6	4	31	19	11	
	Rainfall	113.0	53.8	108.0	6.4	43.2	20.3	26.4	43.7	41.9	58.9	152.4	209.8	209.8
1979	Date	2	11	26	15	27	- 30	22	7	24	2	29	25	
	Rainfall	120.9	148.6	24.1	28.4	165.6	51.8	37.3	83.8	50.8	83.3	229.9	95.3	229.9
1980	Date	23	3	2	-	-	•		<u> </u>	-	-	-		a di serencia
	Rainfall	115.8	57.2	6.3			[[[[ti	115.8

 TABLE 3.9 (1)
 MONTHLY MAXIMUM 6 HOURS RAINFALL AT LA MESA

		Jan.	Feb	Mar:	April	May	June	July	Aug	Sep.	Oct.	Nov.	Dec.	Max.
956	Date	4	29	25	26	25	10	6	24	15	27	10	-27	
	Rainfall	8.9	14.0	10.9	16.0	26.7	33.8	16.3	36.1	48.8	66.0	35.6	22.9	66.0
957	Date	11	11	26	15	7	21	29	15	11	25	9	9	
	Rainfall	18.8	10.7	27.7	1.8	45.2	26.2	49.8	50.3	38.6	36.8	22.4	32.5	50.3
1958	Date	29	3	15	13	16	12	11	10.	27	20	3	21	
	Rainfall	9.1	5.1	66.0	3.3	33.5	80.5	36.3	47.2	36.1	42.7	28.4	6.4	80.5
959	Date	16	2	9	15	31	20	19	6	22	25	6	24	
	Rainfall	12.7	4.8	17.8	27.9	17.0	58.2	12.7	30.7	33.0	38.9	23.1	30.5	58.2
960	Date	9	26	5	11	23	9	9	28	10	24	25	13	
	Rainfall	6.6	22.6	13.7	17.0	16.0	52.8	29.7	45.5	36.8	17.8	44.5	17.8	52.8
961	Date	17	4	9	2	25	18	23	14	4	16	5	3	* ************************************
	Rainfall	23.6	25.1	21.6	6.4	7.1	16.3	40.6	15.7	22.4	26.2	27.7	9.4	40.6
962	Date	23	12	6	16	2	11	5	14	27	4	25	30	
	Rainfall	11.2	8.9	50.0	30.5	14.0	35.6	18.5	12.4	20.8	64.3	17.5	7.6	64.3
1963	Date	31	20	23	15	23	12	28	17	24	11	2	17	
	Rainfall	6.6	28.2	25.7	3.0	9.1	33.5	20.3	45.7	66.8	22.9	23.6	20.3	66.8
964	Date	10	29	31	17	31	6	29	10/13	22	21	7	7	
	Rainfall	15.0	15.0	1.3	12.7	30.5	47.2	15.0	13.5	32.3	20.6	33.0	53.3	53.3
965	Date	14	25	5	29	1	10	. 17	17	21/25	30	5	9	
	Rainfall	10.2	8.4	3.8	9.4	14.7	48.3	37.8	8.9	10.7	82.8	24.9	16.8	82.8
966	Date	26	4	14	10	30	5	6	17	30	25	29	. 11	
	Rainfall	10.4	35.6	25.7	18.8	26.9	44.7	21.1	38.4	20.3	51.6	15.2	22.4	51.6
967	Date	20	12	10	4	24	17	2	30	26	19	10	23	
	Rainfall	22.1	24.9	6.4	12.2	8.9	51.8	7.4	31.0	35.8	66.0	46.5	35.6	66.0
968	Date	14	19	17	11	25	6	31	25	24	18	30	24	
	Rainfall	10.7	15.2	15.0	1.8	98.6	30.7	33.8	8.6	84.8	37.8	25.1	23.9	98.6
969	Date	5	3	4	1	17	5	12	28	15	4	20	11	
	Rainfall	20.8	5.8	10.2	7.6	48.5	19.8	27.9	27.9	34.8	3.0	33.0	12.4	48.5
970	Date	7	22	14	11	17	10	13	3	21	30	28	6	
	Rainfall	11.4	5.8	1.0	<u>.</u>	15.0	16.8	33.0	29.0	71.1	24.9	20.6	20.3	71.1
971	Date	20	9	5	6	15	10	28	31	10	14	11	23	
~~~~	Rainfall	19.6	5.8	19.8	8.9	20.3	10.2	12.7	26.2	21.3	11.4	25.4	26.2	26.2
972	Date	17	4	11	2	20	20	8/19	12	2	10	30	16	
	Rainfall	14.7	21.1	12.7	3.3	6.6	31.0	14.0	23.6	45.7	11.4	3.8	13.5	45.7
973	Date	12	28	. 1.	28	15	20	19	31	-17	17	6	8	
	Rainfall	3.0	3.3	2.0	26.2	33.5	21.6	16.4	19.1	11.2	10.4	27.0	5.5	33.5
974	Date	3	26	8	6/17	21	25	26	25	19	23	12	1	
	Rainfall	11.0	12.0	3.3	2.6	11.9	110.4	8.0	21.0	140.0	80.0	22.0	23.3	140.0
975	Date	21	24			23	13	7	13	12	31	23	7	
· .	Rainfall	3.0	0.5			11.0	5.0	2.0	22.1	9.1	36.0	21.2	16.4	36.0
1976	Date	9	2	17	26	17	3	25	1	3	10	7	15	
	Rainfall	46.0	6,1	0.5	37.0	18.8	30.0	18.0	5.2	8.0	36.0	22.7	26.0	46.0
1977	Date	17	16	1	24	4	18	14	27	23	13	11	21	
	Rainfall	16.0	16.1	5.0	29.0	26.7	23.2	20.0	14.7	23.2	25.3	30.0	27.5	30.0
1978	Date	28	21	5	15	28	13	25	25	19	6	20	13	
	Rainfall	32.6	12.4	15.4	0.3	80.0	18.8	33.6	30.0	20.8	23.4	34.4	50.0	80.0
1979		9	19	7	18	7	23	22	29	20	5	14	25	
	Rainfall	11.8	17.0	4.3	38.0	22.2	21.3	30.3	36.5	34.0	22.0	28.1	27.8	38.0

		· · · ·				1						a la dist		· ·
		Jan.	Feb.	Mar.	April	May	June	July	Aug	Sep.	Oct.	Nov.	Dec.	Max.
1960	Date	4	3	15	17	22	15	27	3	13	6	5	13	
	Rainfall	6.2	12.4	1.4	30.0	33.2	34.6	13.1	16.0	96.4	37.2	40.5	12.2	96.4
1981	Date	11	13	17	18	4	16	12	17	9	19	2	5	
	Rainfall	9.3	33.0	14.2	6.3	16.9	36.1	25.8	42.9	34.4	62.0	20.5	24.0	62.0
1982	Date	9	27	2	12	1	13	4	- 28	22	4	2	13	
	Rainfall	25.5	33.8	25.6	9.0	27.9	39.0	30.2	19.7	41.9	64.2	43.5	22.4	64.2
1983	Date	12	26	11	16	30	14	31	26	15	14	16	30	
	Rainfall	30,6	1.9	8.4	37.5	12.1	24.0	9.4	21.0	19.0	5.3	35.0	60.5	60.5
1984	Date	15	5	21	6	28	16	29	2/25	10	26	22	7.	
	Rainfall	15.2	10.0	17.5	1.2	11.6	57.5	51.2	23.6	25.2	22.3	28.5	25.3	57.5
1985	Date	22	12	6	2	: 7	13	4	17	22	16	23	14	
	Rainfall	10.0	12.1	5.1	38.0	46.2	40.0	35.4	33.7	20.0	10.0	12.2	8.2	46.2
1986	Date	20	3	21	21	28	7	23	18	12	28	14	3	
	Rainfall	13.0	3.5	14.9	1.8	14.5	17.5	31.5	16.0	41.0	47.6	7.4	22.1	47.6
1987	Date	12	7	13	27		9	10	25	1	30	.11	17	
а. — н	Rainfall	34.7	3.6	16.0	1.6		23.5	28.1	23.2	30.0	10.0	60.0	27.0	60.0
1988	Date	22	21	15	11	30	20	27	- 24	:29	12	- 22	2	****
	Rainfall	22.1	20.2	9.6	9.0	67.2	43.8	38.1	43.0	35.6	30.4	11.9	60.2	67.2
1989	Date	21	22	7	12	12	19	23	20	12	11	30	14	
	Rainfall	15.9	14.6	2.2	3.4	30.0	17.3	46.1	20.3	36.0	51.9	27.7	32.2	51.9
1990	Date	26	12	21	28	27	17	31	11	28	25	29	5	
	Rainfall	23.9	19.7	20.2	25.6	13.3	20.0	13.5	12.0	24.4	28.5	50.8	38.1	50.8
1991	Date	7	16	10	2	27	10	4	6	2	19	25	4	
	Rainfall	18.0	4.1	29.0	0.9	17.0	154.4	14.2	26.3	26.8	11.8	16.2'	15.5	154.4

 TABLE 3.9 (2)
 MONTHLY MAXIMUM 6 HOURS RAINFALL AT LA MESA

# YEARLY MAXIMUM ONE HOUR AND 2 HOURS RAINFALL AT LA MESA

Year	1 hour	2 hour
1966	38.1	40.6
1967	41.9	55.9
1968	· • •	-
1969	35.6	36.8
1970	27.9	31.8
1971	14.0	19.1
1972	25.4	26.7
1973	22.9	26.7
1974	-	-
1975	21.2	25.0
1976	-	-
1977		-
1978	-	-
1979	-	·-
1980	54.0	82.1
1981	31.5	36.0
1982	52.5	57.0
1983	33.0	38.0
1984	43.0	49.5
1985	46.2	46.2
1986	l	- '
1987	-	-
1988	-	-
1989	-	-
1990	-	-
1991	18.9	20.0

(Unit : mm)

Note : Record sheets are partially lacked.

## RESULT OF FREQUENCY ANALYSIS (ONE DAY RAINFALL AT LA MESA AND PUERTO CORTES)

	STATION NAME:	La Mesa		1 day Rainfall
RETURN	IWAI	THOMAS	HAZEN	GUMBEL
PERIOD	METHOD	METHOD	METHOD	METHOD
2	75.4	78.4	78.4	78.8
5	107.6	110.9	108.8	124.9
10	132.1	132.9	129.1	155.5
20	157.9	154.4	148.7	184.8
30	173.8	166.9	160.0	201.6
50	194.6	182.7	174.3	222.7
70	209.0	193.2	183.8	236.5
100	224.7	204.4	193.8	251.1
150	243.2	217.3	205.3	267.6
200	256.9	226.5	213.6	279.4
500	302.9	256.5	240.2	316.7
1000	340.7	279.9	260.9	344.9

Unit : mm

Ş	STATION NAME:	1 day Rainfal		
RETURN PERIOD	IWAI METHOD	THOMAS METHOD	HAZEN METHOD	GUMBEL METHOD
2	149.3	151.8	151.8	152.2
5	199.1	206.1	199.1	209.1
10	233.5	241.8	229.4	246.8
20	267.3	276.0	257.9	283.0
30	287.1	295.6	274.1	303.8
50	312.2	320.2	294.3	329.8
70	329.1	336.4	307.4	346.8
100	347.1	353.6	321.3	364.8
150	367.8	373.2	337.1	385.3
200	382.8	387.2	348.3	399.8
500	431.5	432.1	383.9	445.8
1000	469.8	466.7	411.0	480.7
	· · · · · · · · · · · · · · · · · · ·	·····		

Unit : mm

Note : To calculate the probable rainfall, 1962 - 1980 rainfall data are used.

## RESULT OF FREQUENCY ANALYSIS (6 HOURS RAINFALL AT LA MESA)

	STATION NAME:	La Mesa		6 hous Rainfal
RETURN PERIOD	IWAI METHOD	THOMAS METHOD	HAZEN METHOD	GUMBEL METHOD
2	58.4	58.7	58.7	59.1
5	. 81.1	82.9	81.0	86.2
10	96.6	99.2	95.9	104.2
20	111.6	115.2	110.2	121.4
30	120.4	124.5	118.5	131.3
50	131.5	136.2	128.9	143.7
70	138.9	144.0	135.8	151.8
100	146.8	152.3	143.1	160.4
150	155.9	161.9	151.4	170.1
200	162.4	168.7	157.4	177.0
500	183.5	190.9	176.7	199.0
1000	200.0	208.3	191.7	215.6

Unit : mm

	STATION NAME:	La Mesa	e de la companya de l	t hour Rainfa
	STATION NAME.	La mesa		
RETURN	IWAI	THOMAS	HAZEN	GUMBEL
PERIOD	METHOD	METHOD	METHOD	METHOD
2	32.5	31.5	31.5	32.0
5	43.7	46.0	43.8	45.2
10	50.4	56.0	52.0	53,9
20	56.4	65.8	59.9	62.3
30	59.7	71.6	64.5	67.1
50	63.7	79.0	70.3	73.2
70	66.2	84.0	74.1	77.1
100	68.9	89.3	78.2	81.3
150	71.9	95.4	82.9	86.1
200	74.0	99.8	86.2	89.4
500	80.5	114.3	97.0	100.1
1000	85.3	125.7	105.4	108.2

Unit : mm

	STATION NAME:	La Mesa		2 hours rainfall
RETURN PERIOD	IWAI METHOD	THOMAS METHOD	HAZEN METHOD	GUMBEL METHOD
2	37.4	36.5	36.4	37.1
5	51.8	54.0	51.3	55.2
10	60.7	66.2	61.4	67.1
20	69.0	78.5	71.2	78.6
30	73.6	85.7	76.9	85.3
50	79.2	94.9	84.2	93.5
70	82.9	101.1	88.9	98.9
100	86.7	107.8	94.1	104.7
150	91.1	115.5	99.9	111.2
200	94.1	121.1	104.1	115.8
500	103.7	139.4	117.8	130.4
1000	111.0	153.9	128.4	141.5

Unit : mm

## TABLE 3.14 CATCHMENT AREA OF SUB-BASIN

		Catchme	nt/River
		C.A.	L (km)
		(sq. km)	(((1))
1	Rio Cholome, Rio Blanco, Canal San Roque, Canal S. R C. and Canal C	H C. Basin	
C-1	River Mouth of the Basin	420.15	48.4
C-2	Choloma, Blanco, San Roque, Canal San Roque - Cuabanos, C-H-C Basin	366.45	42.1
	I Canal Copen-Higuero-Cuabanos	33.43	9.6
	ii Choloma, Blanco and San Roque	333.02	42,1
C-3	Rio Choloma, Rio Blanco and S.R. Basin	297.13	37.4
RC-1	Rio Choloma Basin	106.89	20.7
RC-2	1 at Choloma Bridge	71.64	13.6
RC-3	II at Jutosa (Junction of Rio La Jutosa)	55.02	9.4
• C-4	Rio Blanco - Canal San Roque Basin	190.24	37.4
RB-1	Rio Blanco Basin	137.98	31.0
R8-2	i Outlet of Laguna El Carmen	107.41	22.7
RB-3	ii Inlet of Laguna El Carmen	83.72	19.2
RB-4	ili Prop. Diversion Point	71.35	15.7
RB-5	iv Rio del Zapotal and Rio de Armenta	43.90	12.2
	Rio El Sauce and Rio El Sauce (viejo) - Chotepe Basin	<b>_</b>	· · · · · · · · · · · · · · · · · · ·
S-1	River Mouth of Rio El Sauce	215.70	29.9
RS-1	Rio El Sauce Basin	118.33	29.7
<b>RS-2</b>	i Mid. of Rio El Sauce	79.98	21.8
RS-3	ii Jct. of Prop. Diversion	75.33	18.1
<b>RS-4</b>	iii Rio Santa Ana and Rio Piedras	72.16	15.4
RSB-1		37.63	13.4
RSP-1	v Rio Piedras Basin (at National Road)	30.87	12.6
RSV-1	Rio El Sauce (viejo) - Chotepe Basin	97.37	22.9

Note: Retention effect of Laguna El Carmen is not considered in this calculation

C.A. : Catchment Area of the Basin (sq. km) L. : Maximum River S.R. : Canal San Roque S.R.-C. : Canal San Roque Cuabanos L: Maximum River Length of the Basin (km)

C-H-C : Canal Copen-Higuero-Cuabanos

		Catchment	River	Peak Disch	infall Pattern	m / sec.)
		C.A. (sq. km)	(km)	A	B	C
 I	Rio Choloma, Rio Blanco, Canal San Roque, Ca				K	
			· · · ·	: .		
C-1	River Mouth of the Basin	420.15	48.4	1023.6	1,793.4	2,285.9
C-2	Choloma, Blanco, S.R. ,S.RC ,C-H-C Basin	366.45	42.1	896.0	1,612.4	2,114.
	i Canal Copen-Higuero-Cuabanos	33.43	9.6	126.3	303.1	387.3
	ii Choloma, Blanco and San Roque	333.02	42.1	822.5	1,487.4	1,949.6
C-3	Rio Choloma, Rio Blanco and S.R. Basin	297.13	37.4	764.7	1,394.5	1,826.7
RC-1	Rio Choloma Basin	106.89	20.7	330.2	678.3	888.3
RC-2	i at Choloma Bridge	71.64	13.6	250.1	543.5	688.1
RC-3	li at Jutosa (Junction of Rio La Jutosa)	55.02	9.4	198.6	458.1	596.7
C-4	Rio Blanco - Canal San Roque Basin	190.24	37.4	517.1	964.6	1,250.1
RB-1	Rio Blanco Basin	137,98	31.0	383.1	741,9	998.7
<b>RB-2</b>	i Outlet of Laguna El Carmen	107.41	22.7	325.9	663.4	872.4
RB-3	ii Inlet of Laguna El Carmen	83.72	19.2	270.1	568.1	733.9
RB-4	iii Prop. Diversion Point	71.35	15.7	244.9	527.2	671.2
R8-5	iv Rio del Zapotal and Rio de Armenta	43.90	12.2	165.4	375.3	463.7
11	Rio El Sauce and Rio El Sauce (viejo) - Chotepo	e Basin				
S-1	River Mouth of Rio El Sauce	215.70	29.9	575.0	1,087.7	1,474.1
RS-1	Rio El Sauce Basin	118.33	29.7	336.8	663.3	885.
RS-2	i Mid. of Rio El Sauce	79.98	21.8	252.3	528.1	684.0
RS-3		75.33	18.1	249.4	530.0	680.
RS-4		72.16	15.4	248.3	534.8	680.
RSB-1	iv Rio Santa Ana Basin (at National Road)	37.63	13.4	141.4	322.4	397.4
RSP-1	v Rio Piedras Basin (at National Road)	30.87	12.6	119.8	278.7	339.5
RSV-1	Rio El Sauce (viejo) - Chotepe Basin	97.37	22.9	297.6	611.1	800.

### RESULT OF RUN-OFF SIMULATION BY UNIT HYDROGRAPH METHOD (100-YEAR RETURN PERIOD)

Note: Retention effect of Laguna El Carmen is not considered in this calculation

C.A. : Catchment Area of the Basin (sq. km) L : Maximum River Length of the Basin (km) S.R. : Canal San Roque S.R.-C : Canal San Roque Cuabanos

C-H-C : Canal Copen-Higuero-Cuabanos

**Rainfall Pattern** 

**TABLE 3.15** 

A : Maximum intensity of rainfall occures at the beginning of the rain.

B : Maximum intensity of rainfall occures at the midlle of the rain.

C : Maximum intensity of rainfall occures at the end of the rain.

#### Runoff(100)

#### RESULT OF RUN-OFF SIMULATION BY UNIT HYDROGRAPH METHOD (50-YEAR RETURN PERIOD)

ſ	2) Mandardo Maine (Arybide) aprime page approximate). And Article Testinal Ministry of page	Catchmen	VRiver	Peak Disc		m / sec.)
		C.A.	Ĺ.		Pattern	
		(sq. km)	(km)	<u>A</u>	В	С
1	Rio Choloma, Rio Blanco, Canal San Roque, C	anal S.RC an	d Canal C-H	•C Basin		
_C-1	River Mouth of the Basin	420.15	48.4	851.9	1,514.6	2,010.9
C-2	Choloma, Blanco, S.R. ,S.RC ,C-H-C Basin	366.45	42.1	744.6	1,360.5	1,862.1
	i Canal Copen-Higuero-Cuabanos	33.43	9.6	100.3	252.4	345.6
	li Choloma, Blanco and San Roque	333.02	42.1	683.9	1,255.3	1,717.6
C-3	Rio Choloma, Rio Blanco and S.R. Basin	297.13	37.4	634.3	1,177.5	1,610.7
RC-1	Rio Choloma Basin	106.89	20.7	271.3	564.8	787.7
RC-2	i at Choloma Bridge	71.64	13.6	202.6	453.2	611.9
RC-3		55.02	9.4	159.6	381.2	531.6
C-4	Rio Blanco - Canal San Roque Basin	190.24	37.4	424.7	806.7	1,104.1
RB-1	Rio Blanco Basin	137.98	31.0	313.6	620.6	883.7
RB-2	i Outlet of Laguna El Carmen	107.41	22.7	267.6	552.3	773.4
R8-3	ii Inlet of Laguna El Carmen	83.72	19.2	220.8	473.3	651.5
RB-4	iii Prop. Diversion Point	71.35	15.7	198.8	439.5	596.6
RB-5	iv Rio del Zapotal and Rio de Armenta	43.90	12.2	132.4	313.3	413.1
* <b>1</b> 1	Rio El Sauce and Rio El Sauce (viejo) - Chotepo	e Basin	, <b>I</b> ,			
S-1	River Mouth of Rio El Sauce	215.70	29.9	472.7	918.0	1,302.9
RS-1	Rio El Sauce Basin	118.33	29.7	276.0	551.9	784.2
RS-2	i Mid. of Rio El Sauce	79.98	21.8	206.7	439.9	607.6
RS-3	ii Jct. of Prop. Diversion	75.33	18.1	203.2	441.7	604.2
RS-4	iii Rio Santa Ana and Rio Piedras	72.16	15.4	201.6	445.8	605.0
RSB-1		37.63	13.4	113.0	269.2	354.1
RSP-1	v Rio Piedras Basin (at National Road)	30.87	12.6	95.8	232.8	303.0
RSV-1	Rio El Sauce (viejo) - Chotepe Basin	97.37	22.9	244.6	508.8	709.9

Note: Retention effect of Laguna El Carmen is not considered in this calculation

C.A. : Catchment Area of the Basin (sq. km) L. : Maximum River Length of the Basin (km) S.R. : Canal San Roque S.R.-C : Canal San Roque Cuabanos C-H-C : Canal Copen-Higuero-Cuabanos

**Rainfall Pattern** 

A : Maximum intensity of rainfall occures at the beginning of the rain.

B : Maximum intensity of rainfall occures at the midlle of the rain.

C : Maximum intensity of rainfall occures at the end of the rain.

RUNOFF(1/50)

#### **RESULT OF RUN-OFF SIMULATION BY UNIT HYDROGRAPH METHOD** (30-YEAR RETURN PERIOD)

		Catchmen	/River	Peak Disch		n / sec.)
•		C.A.			Pattern	· · · · · · · · · · · · · · · · · · ·
		(sq. km)	(km)	A	B	<u> </u>
	Rio Choloma, Rio Blanco, Canal San Roque, Ca	nal S.RC and	d Canal C-H	-C Basin		
C-1	River Mouth of the Basin	420.15	48.4	724.7	1,305.3	1,800.9
C-2	Choloma, Blanco, S.R. ,S.RC ,C-H-C Basin	366.45	42.1	631.6	1,171.2	1,669.
	i Canal Copen-Higuero-Cuabanos	33.43	9.6	83.7	213.5	313.9
	ii Choloma, Blanco and San Roque	333.02	42.1	579.1	1,080.8	1,540.5
C-3	Rio Choloma, Rio Blanco and S.R. Basin	297.13	37.4	536.7	1,014.2	1,445.8
RC-1	Rio Choloma Basin	106.89	20.7	227.3	478.4	711.2
RC-2	i at Choloma Bridge	71.64	13.6	168.7	384.2	553.9
RC-3	ii at Jutosa (Junction of Rio La Jutosa)	55.02	9.4	132.8	322.3	482.2
C-4	Rio Blanco - Canal San Roque Basin	190.24	37.4	360.2	695.4	992.
<b>RB-1</b>	Rio Blanco Basin	137.98	31.0	265.0	534.5	796.0
<b>RB-2</b>	i Outlet of Laguna El Carmen	107.41	22.7	224.5	467.8	698.
R8-3	ii Inlet of Laguna El Carmen	83.72	19.2	184.1	401.0	588.
RB-4	iii Prop. Diversion Point	71.35	15.7	165.5	372.5	539.
RB-5	iv Rio del Zapotal and Rio de Armenta	43.90	12.2	110.7	265.8	374.
11	Rio El Sauce and Rio El Sauce (viejo) - Chotepe	Basin	I_	L_		
S-1	River Mouth of Rio El Sauce	215.70	29.9	399.7	790.2	1,172.0
RS-1	Rio El Sauce Basin	118.33	29.7	232.6	473.5	706.
<b>RS-2</b>	i Mid. of Rio El Sauce	79.98	21.8	172.6	372.7	549.
RS-3	ii Jct. of Prop. Diversion	75.33	18.1	169.0	374.3	546.
RS-4	iii Rio Santa Ana and Rio Piedras	72.16	15.4	167.8	377.9	547.
RSB-1		37.63	13.4	94.5	228.4	321.
RSP-1	v Rio Piedras Basin (at National Road)	30.87	12.6	79.8	197.5	275.
RSV-1	Rio El Sauce (viejo) - Chotepe Basin	97.37	22.9	204.9	430.9	640,

Note: Retention effect of Laguna El Carmen is not considered in this calculation

C.A. : Catchment Area of the Basin (sq. km) L. : Maximum River Length of the Basin (km) S.R. : Canal San Roque S.R.-C : Canal San Roque Cuabanos C-H-C : Canal Copen-Higuero-Cuabanos

Rainfall Pattern

A : Maximum intensity of rainfall occures at the beginning of the rain.

B : Maximum intensity of rainfall occures at the midlle of the rain.

C : Maximum intensity of rainfall occures at the end of the rain.

RUNOFF(1/30)

TABLE	3.18
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# RESULT OF RUN-OFF SIMULATION BY UNIT HYDROGRAPH METHOD

and the constraint date	<b>ĸĸĊŦĊĬĊĸĊĸĊŢĊŎĊĊŢĊŎĊĬĊŎĊĹĊŢĊĊĬĊĬŎŎĊŎŎŎŎŎŎŎŎŎŎŎŎŎŎŎ</b>	Catchmen	l/River	Peak Disch		n / sec.)
		C.A.	L  _		Pattern	
		(sq. km)	(km)	A	B	C
1	Rio Choloma, Rio Blanco, Canal San Roque, C	anal S.RC an	d Canal C-H	-C Basin		
C-1	River Mouth of the Basin	420.15	48.4	454.5	836.1	1,304.4
C-2	Choloma, Blanco, S.R. ,S.RC ,C-H-C Basin	366.45	42,1	394.6	746.4	1,213.0
02	i Canal Copen-Higuero-Cuabanos	33.43	9.6	46.5	124.1	238.9
	ii Choloma, Blanco and San Roque	333.02	42.1	361.3	689.1	1,120.5
C-3	Rio Choloma, Rio Blanco and S.R. Basin	297.13	37.4	333.3	647.4	1,054.5
RC-1	Rio Choloma Basin	106.89	20.7	135.2	302.7	528.9
RC-2	i at Choloma Bridge	71.64	13.6	97.9	232.6	416.0
RC-3	ii at Jutosa (Junction of Rio La Jutosa)	55.02	9.4	75.2	190.8	364.9
C-4	Rio Blanco - Canal San Roque Basin	190.24	37.4	221.1	445.0	727.8
RB-1	Rio Blanco Basin	137.98	31.0	160.8	340.6	587.2
RB-2	i Outlet of Laguna El Carmen	107.41	22.7	133.8	297.4	518.4
RB-3	ii Inlet of Laguna El Carmen	83.72	19.2	108.8	249.6	439.6
RB-4	iii Prop. Diversion Point	71.35	15.7	96.6	227.3	404
RB-5	iv Rio del Zapotal and Rio de Armenta	43.90	12.2	62.6	157.1	283.4
	Rio El Sauce and Rio El Sauce (vlejo) - Choter	e Basin	<b>I</b>			
S-1	River Mouth of Rio El Sauce	215.70	29.9	245.6	502.3	860.
RS-1	Rio El Sauce Basin	118,33	29.7	140.2	302.1	522.
RS-2		79.98	21.8	102.2	233.0	409.
RS-3		75.33	18.1	99.5	230.9	408.
RS-4	iii Rio Santa Ana and Rio Piedras	72.16	15.4	97.9	230.5	410.
RSB-1		37.63	13.4	53.3	135.0	243.
RSP-1		30.87	12.6	44.5	116.8	208.
RSV-	Rio El Sauce (viejo) - Chotepe Basin	97.37	22.9	121.9	272.8	476.

Note: Retention effect of Laguna El Carmen is not considered in this calculation

C.A. : Catchment Area of the Basin (sq. km) L : Maximum River Length of the Basin (km) S.R. : Canal San Roque S.R.-C : Canal San Roque Cuabanos C-H-C : Canal Copen-Higuero-Cuabanos

**Rainfall Pattern** 

A : Maximum intensity of rainfall occures at the beginning of the rain.

B : Maximum intensity of rainfall occures at the midlle of the rain.

C : Maximum intensity of rainfall occures at the end of the rain.

#### RUNOFF(1/10)

#### **TABLE 3.19 RESULT OF RUN-OFF SIMULATION BY UNIT HYDROGRAPH METHOD** (5-YEAR RETURN PERIOD)

NUMBER OF THE OWNER.	na managana kata kata kata kata kata kata kata k	Catchment	/River	Peak Disch		n / sec.)
		C.A	L	· · · ·	Pattern	
		(sq. km)	(km)	A	<u>B</u>	C
1	Rio Choloma, Rio Blanco, Canal San Roque, C	anal S.R. C and	l Canal C-H	-C Basin	:	
C-1	River Mouth of the Basin	420.15	48.4	284.4	505.7	916.1
C-2	Choloma, Blanco, S.R. ,S.RC ,C-H-C Basin	366.45	42.1	244.9	447.3	853.6
1 - E	i Canal Copen-Higuero-Cuabanos	33.43	9.6	26.4	74.0	173.8
	ii Choloma, Blanco and San Roque	333.02	42.1	223,6	412.6	789.1
C-3	Rio Choloma, Rio Blanco and S.R. Basin	297.13	37.4	204.5	387.7	743.9
RC-1	Rio Choloma Basin	106.89	20.7	78.7	179.8	378.0
RC-2	i at Choloma Bridge	71.64	13.6	55.7	138.2	299.5
RC-3	ii at Jutosa (Junction of Rio La Jutosa)	55.02	9.4	42.7	112.6	264.2
C-4	Rio Blanco - Canal San Roque Basin	190.24	37.4	133.6	266.6	515.3
RB-1	Rio Blanco Basin	137.98	31.0	95.7	202.4	417.4
R8-2	i Outlet of Laguna El Carmen	107.41	22.7	78.2	176.7	370.3
R8-3	ii Inlet of Laguna El Carmen	83.72	19.2	62.6	148.3	315.2
RB-4	iii Prop. Diversion Point	71.35	15.7	55.0	135.1	291.0
RB-5	iv Rio del Zapotal and Rio de Armenta	43.90	12.2	35.6	93.5	205.0
11	Rio El Sauce and Rio El Sauce (viejo) - Chotep	e Basin	<b></b>			
S-1	River Mouth of Rio El Sauce	215.70	29.9	148.1	298.3	609.5
RS-1	Rio El Sauce Basin	118.33	29.7	82.9	179.5	372.1
RS-2	i Mid. of Rio El Sauce	79.98	21.8	59.0	138.4	293.3
RS-3	ii Jct. of Prop. Diversion	75.33	18.1	56.9	137,2	293.4
RS-4	iii Rio Santa Ana and Rio Piedras	72.16	15.4	55.7	137.0	295.2
RSB-1	iv Rio Santa Ana Basin (at National Road)	37.63	13.4	30,4	80.3	176.1
RSP-1	v Rio Piedras Basin (at National Road)	30.87	12.6	25.4	69.5	151.7
RSV-1	Rio El Sauce (viejo) - Chotepe Basin	97.37	22.9	71.0	162.1	340.6

Note: Retention effect of Laguna El Carmen is not considered in this calculation

C.A. : Catchment Area of the Basin (sq. km) L : Maximum Rive S.R. : Canal San Roque S.R.-C : Canal San Roque Cuabanos L : Maximum River Length of the Basin (km)

C-H-C : Canal Copen-Higuero-Cuabanos

**Rainfall Pattern** 

A : Maximum intensity of rainfall occures at the beginning of the rain.

B : Maximum intensity of rainfall occures at the midlle of the rain.

C : Maximum intensity of rainfall occures at the end of the rain,

RUNOFF(1/5)

# RESULT OF RUN-OFF SIMULATION BY UNIT HYDROGRAPH METHOD

		Catchmen	t/River	Peak Disch		/ sec.)
		C.A.			Pattern	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~
		(sq. km)	(km)	<u>A</u>	<u>B</u>	C
- 1	Rio Choloma, Rio Blanco, Canal San Roque, Ca	anal S.RC an	d Canal C-H	-C Basin		
C-1	River Mouth of the Basin	420.15	48.4	95.2	177.5	338.7
C-2	Choloma, Blanco, S.R. ,S.RC ,C-H-C Basin	366.45	42.1	81.8	157.7	316.0
	i Canal Copen-Higuero-Cuabanos	33.43	9.6	9.2	28.1	65.8
	ii Choloma, Blanco and San Roque	333.02	42.1	74.6	145.6	292.3
C-3	Rio Choloma, Rio Blanco and S.R. Basin	297.13	37.4	68.2	136.7	275.9
RC-1	Rio Choloma Basin	106.89	20.7	26.7	63.5	141.5
RC-2	i at Choloma Bridge	71.64	13.6	19.2	50.5	112.7
RC-3	ii at Jutosa (Junction of Rio La Jutosa)	55.02	9.4	14.8	42.3	99.8
C-4	Rio Blanco - Canal San Roque Basin	190.24	37.4	44.6	93.9	191.5
RB-1	Rio Blanco Basin	137.98	31.0	32.0	71.6	155.6
R8-2	i Outlet of Laguna El Carmen	107.41	22.7	26.4	62.4	138.5
<b>RB-3</b>	ii Inlet of Laguna El Carmen	83.72	19.2	21.4	52.6	118.2
R8-4	iii Prop. Diversion Point	71.35	15.7	18.9	48.9	109.4
RB-5	iv Rio del Zapotal and Rio de Armenta	43.90	12.2	12.3	35.0	77.4
11	RIo El Sauce and Rio El Sauce (viejo) - Chotepo	e Basin	•	1	I	
S-1	River Mouth of Rio El Sauce	215.70	29.9	49.4	105.6	226.7
RS-1	Rio El Sauce Basin	118.33	29.7	27.8	63.4	138.9
<b>RS-2</b>	i Mid. of Rio El Sauce	79.98	21.8	20.1	48.9	109.9
RS-3	ii Jct. of Prop. Diversion	75.33	18:1	19.5	49.1	110.1
RS-4	iii Rio Santa Ana and Rio Piedras	72.16	15.4	19.2	49.7	110.9
RSB-1		37.63	13.4	10.5	30.1	66.5
RSP-1	v Rio Piedras Basin (at National Road)	30.87	12.6	8.8	26.1	57,4
RSV-1	Rio El Sauce (viejo) - Chotepe Basin	97.37	22.9	24.1	57.2	127.5

Note: Retention effect of Laguna El Carmen is not considered in this calculation

C.A. : Catchment Area of the Basin (sq. km) L : Maximum River Length of the Basin (km) S.R. : Canal San Roque S.R.-C : Canal San Roque Cuabanos

C-H-C : Canal Copen-Higuero-Cuabanos

Rainfall Pattern

A : Maximum intensity of rainfall occures at the beginning of the rain.

- B : Maximum intensity of rainfall occures at the midlle of the rain.
- C : Maximum intensity of rainfall occures at the end of the rain.

#### RUNOFF(1/2)

PROBABLE FLOOD PEAK DISCHARGE OF RAINFALL PATTERN C (PRESENT RIVER SYSTEM)

		Catchment/River	River		Peak Discharge	charge (qu.	m / sec.)			
		C.A.	^^			Return Period				
		(sq. km)	(Km)	2-year	5-year	10-year	30-year	50-year	100-year	•
	Rio Choloma, Rio Bianco, Canai San Roque, Canal	nal San Roqu	e-Cuabano	San Roque-Cuabanos and Canal Copen-Higuero-Cuabanos	Copen-Higu	ero-Cuabano	os Basin			
5	River Mouth of the Basin	420.15	48.4	338.7	916.1	1,304.1	1,800.9	2,010.9	2,285.9	1.
0-2	Choloma, Blanco, S.R. ,S.RC ,C-H-C Basin	366.45	42.1	316.0	853.6 173 8	1,213.0	1,669.5	1,862.1	2,114.5	
	i Choloma, Blanco and San Roque	333.02	42.1	292.3	789.1	1,120.5		1,717.6	1,949.6	
မ္မ	Rio Choloma, Rio Blanco and S.R. Basin	297.13	37.4	275.9	743.9	1,054.5	1,445.8	1,610.7	1,826.7	e Se
но 1-0-	Rio Choloma Basin	105.89	20.7	141.5	378.0	528.9	711.2	7.87.7	888.3	•
HC-2 C-2 C-2	• :	71,64	13.6	112.7	299.5	416.0	553.9	611.9	688.1	
	II at Jurosa (Junction of hig La Jurosa)	20.00		0	204.4	104.400	402.2	0.12	7.920	
0-4	Rio Blanco - Canal San Roque Basin	190.24	37.4	191.5	515.3	727.8	992.8	1,104.1	1,250.1	
RB-1	in Second	137.98	31.0	155.6	417.4	587.2	796.0	883.7	998.7	
ο Ω Ω Ω Ω Ω	· ;:	107.41	22.7	138.5	370.3	518.4	698.0	773.4	872.4	
	II Inter of Laguna El Carmen	71.25	2.5	100.4	0 100	408.0	5.00°	501.0 201.0	5.00 571 0	
29 29 29 20 20	≣.≥	43,90	12.2	77.4	205.1	283.4	374.6	413.1	463.7	
=	Rio El Sauce and Rio El Sauca (viejo) - Chotepe E	epe Basin								- - -
ۍ ۲	River Mouth of Rio El Sauce	215.70	29.9	226.7	609.5	860.2	1,172.0	1,302.9	1,474.7	•
RS-1	Rio El Sauce Basin	118.33	29.7	138.9	372.1	522.7	706.9	784.2	885.7	
RS-2 SS-2	:	79,98	21.8	109.9	293.3	409.5	549.0	607.6	634.6 600.0	
5 4 4 7 4 7 4	II JCL. OF Frop. LIVERSION III Rio Santa Ana and Rio Piedras	72.16	15.4	+ 10.9	295.2	400.7	547.5	605.0	680.7	
RSB-1	≤.∶	37,63	13.4	66.5	176.1	243.1	321.2	354.1	397.4	
RSP-1	>	30,87	12.6	57.4	151.7	208.9	275.1	303.0	339.8	
RSV-1	RSV-1 Rio El Sauce (viejo) - Chotepe Basin	97.37	22.9	127.5	340.6	476.5	640.9	709.9	800.5	
	Note: Retention effect of Laguna El Carmen is not considered in this calculation	nsidered in th	is calculatio	c						

C.A. : Catchment Area of the Basin (sq. km) L : Maximum River Length of the Basin (km) S.R. : Canal San Roque S.R.-C : Canal San Roque Cuabanos C-H-C : Canal Copen-Higuero-Cuabanos

PROBABLE FLOOD PEAK DISCHARGE OF RAINFALL PATTERN C (ALTERNATIVE RIVER SYSTEM)

		Catchment/River	River		Peak Discharge	charge (qu. 1	m / sec.)		
		(m) (m)		1000	2 1001	Return Pariod		EO LOOK	1000
	Rio Choloma, Rio Blanco, Canal San Roque, Canal	San San	e-Cuabanc	2-year s and Canal	o-year Copen-Higu	Roque-Cuabanos and Canai Copen-Higuero-Cuabanos	os Basin	ou-year	100-year
5	River Mouth of the Basin	348.80	32.7	326.9	881.5	1,249.8	1,714.1	1,909.7	2,166.0
0-2	Choloma, Blanco, S.R., S.RC. C-H-C Basin	295.10	26.4	303.0 6	815.5	1,152.8	1,574.1	1,751.2	
	i Canal Copen-Higuero-Cuapanos ii Choloma, Blanco and San Roque	261,67	26.4 26.4	274.8	739.1		1,423.2	345.0 1,582.6	387.3 1,791.7
က ပ	Rio Choloma, Rio Blanco and S.R. Basin	225.78	21.7	257.1	690.1	971.9	1,319.2	1,465.7	1,657.3
н С-	н Ц	106.89	20.7	141.5	378.0		711.2	787.7	838.3
но 2 1 2 2 2 1 2 2 2 2 2 2 2 2 2 2 2 2 2	i at Choloma Bridge ii at Jutosa (Junction of Rio La Jutosa)	71.64 55.02	0 9 9 9	112.7 99.8	299.5 264.2	416.0 364.9	553.9 482.2	611.9 531.6	688.1 596.7
0-4-0	Rio Blanco - Canal San Roque Basin	118.89	21.7	152.4	407.6	570.9	769.2	852.5	961.8
RB-1	Rio Blanco Basin	66.63	15.3	104.1	276.8	384.7	512.5	566.2	636.8
IJ	Rio El Sauce and Rio El Sauce (viejo) - Chotepe Basin	e Basin							
\$-1	River Mouth of Rio El Sauce	287.05	30.1	285.6	769.1	1,088.1	1,487.6	1,655.7	1,876.0
RS-1		189.68	29.9	204.1	548.3	773.1	1,051.6	1,168.5	1,321.9
H V K	i Mid. or Hio El Sauce ii Ubstream of Jct. of Prop. Diversion	75.33	18.1	1	293.4		546.3	604.2	·'
RS-4	iii Rio Santa Ana and Rio Piedras	72.16	15.4	110.9	295.2		547.5	605.0	680.7
RSB-1 RSP-1	iv Rio Santa Ana Basin (at National Road) v Rio Piedras Basin (at National Road)	37.63 30.87	13.4 12.6	66.5 57.4	176.1 151.7	243.1 208.9	321.2 275.1	354.1 303.0	397.4 339.8
RB-4 RB-5	iii Prop. Diversion Point iv Rio del Zapotal and Rio de Armenta	71.35 43.90	15.7	109.4 77.4	291.0 205.1	404.7 283.4	539.8 374.6	596.6 413.1	671.2 463.7
5V-1	RSV-1 Rio El Sauce (viejo) - Chotepe Basin	97.37	22.9	127.5	340.6	476.5	640.9	709.9	800.5

Note: Retention effect of Laguna El Carmen is not considered in this calculation

C.A. : Catchment Area of the Basin (sq. km) L : Maximum River Length of the Basin (km) S.R. : Canal San Roque S.R.-C : Canal San Roque Cuabanos C-H-C : Canal Copen-Higuero-Cuabanos

## SIMULATED PEAK DISCHARGE OF HURRICANE FIFI

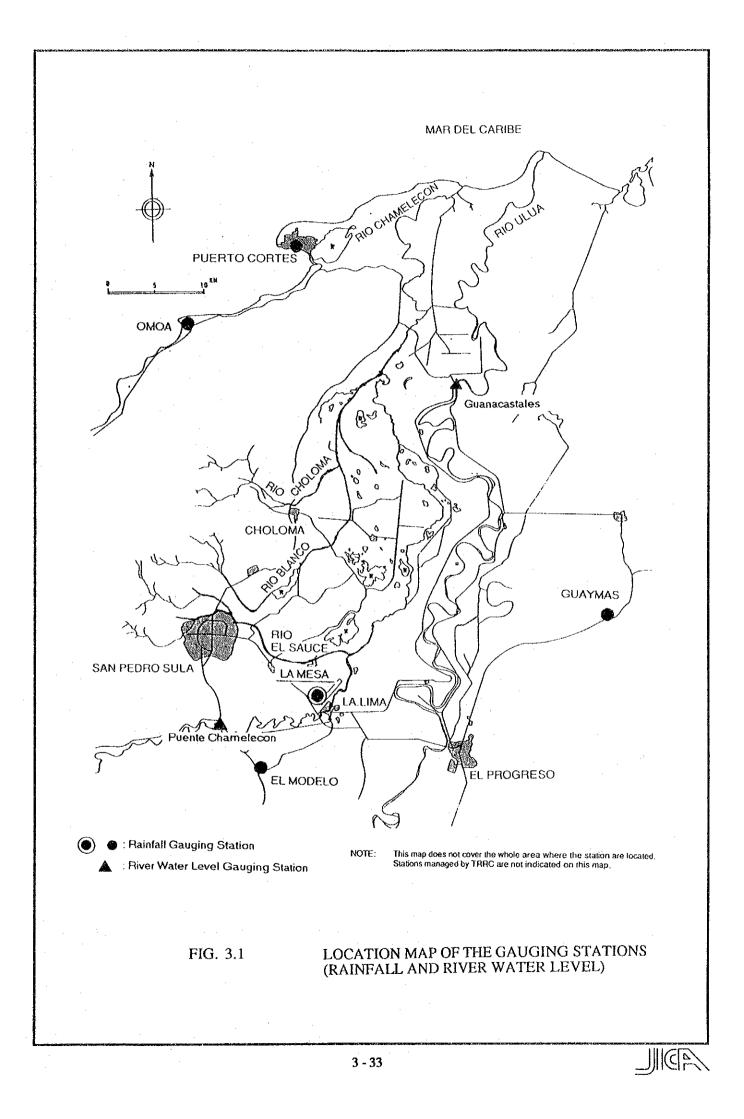
THE REPAIR AND	₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩	Catch	ment/River	Discharge
		C.A.	L	1 . Q
		(sq. km)	(km)	(qu.n/sec)
- 1	Rio Choloma, Rio Blanco, Canal San Roque, Canal S.R.	C and Canal C-	H-C Basin	
C-1	River Mouth of the Basin	420.15	48.4	2,079.0
C-2	Choloma, Blanco, San Roque, Canal S.RC ,C-H-C Basin	366.45	42,1	1,838.3
	i Canal Copen-Higuero-Cuabanos	33.43	9.6	297.4
	il Choloma, Blanco and San Roque	333.02	42.1	1,686.0
C-3	Rio Choloma, Rio Blanco and S.R. Basin	297.13	37.4	1,559.0
RC-1	Rio Choloma Basin	106.89	20.7	668.3
RÇ-2	i at Choloma Bridge	71.64	13.6	514.7
RC-3	ii at Jutosa (Junction of Rio La Jutosa)	55.02	9.4	453.1
C-4	Rio Blanco - Canal San Roque Basin	190.24	37.4	1,039.7
RB-1	Rio Blanco Basin	137.98	31.0	767.7
RB-2	i Outlet of Laguna El Carmen	107.41	22.7	658.3
<b>RB-3</b>	ii Inlet of Laguna El Carmen	83.72	19.2	546.9
RB-4	ili Prop. Diversion Point	71.35	15.7	500.3
RB-5	iv Rio del Zapotal and Rio de Armenta	43.90	12.2	351.8
11	Rio El Sauce and Rio El Sauce (viejo) - Chotepe Basin		·	l
S-1	River Mouth of Rio El Sauce	215.70	29.9	1,159.4
RS-1	Rio El Sauce Basin	118.33	29.7	676.3
RS-2	i Mid. of Rio El Sauce	79.98	21.8	510.8
RS-3	ii Jct. of Prop. Diversion	75.33	18.1	504.6
RS-4	lii Rio Santa Ana and Rio Piedras	72.16	15.4	507.5
RSB-1	iv Rio Santa Ana Basin (at National Road)	37.63	13.4	301.9
RSP-1	v Rio Piedras Basin (at National Road)	30.87	12.6	259.8
RSV-1	Rio El Sauce (viejo) - Chotepe Basin	97.37	22.9	602.4

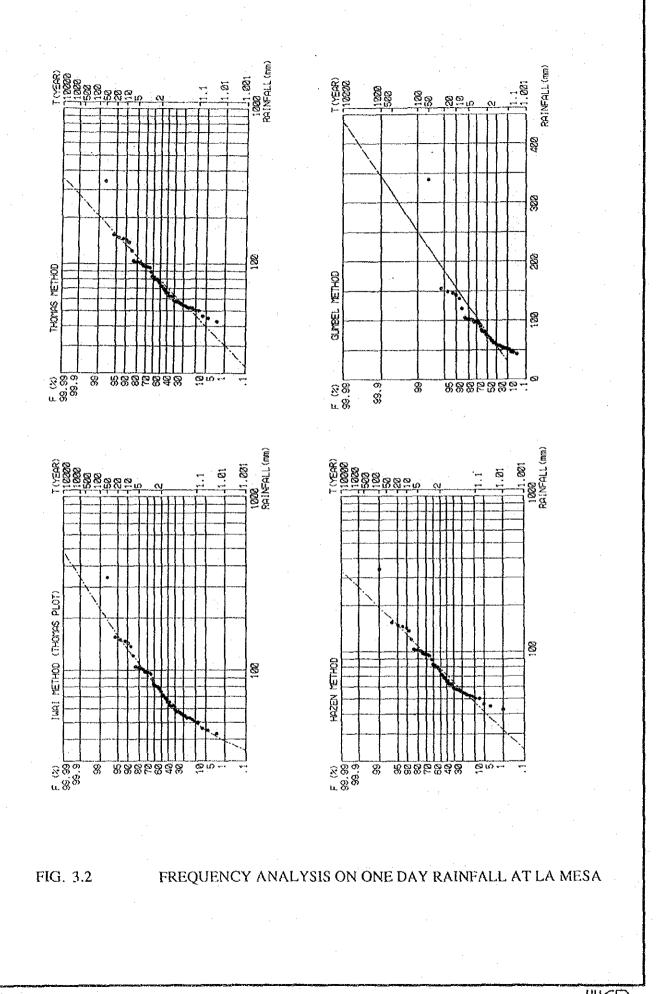
Note: Retention effect of Laguna El Carmen is not considered in this calculation

C.A. : Catchment Area of the Basin (sq. km) L : Maximum Rive S.R. : Canal San Roque S.R.-C : Canal San Roque Cuabanos C-H-C : Canal Copen-Higuero-Cuabanos L : Maximum River Length of the Basin (km)

## FIGURES

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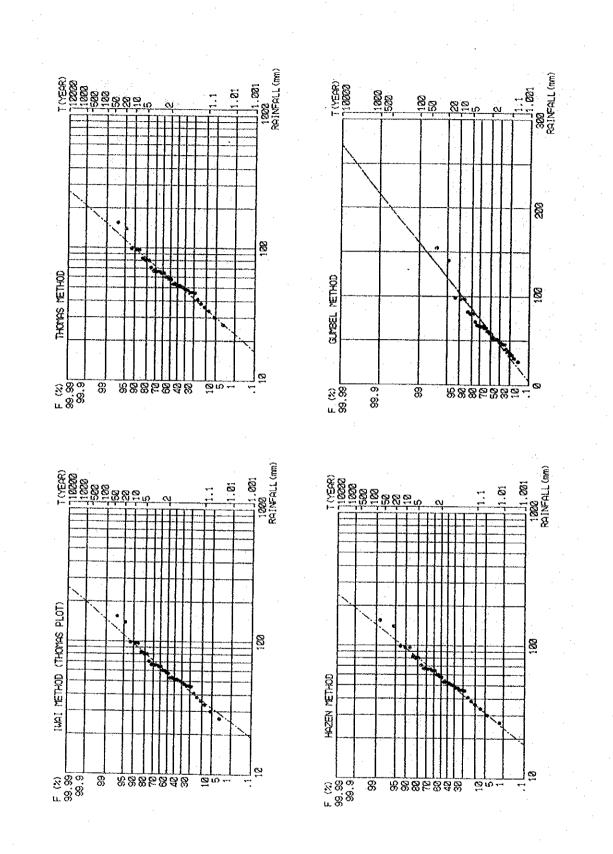
3 - 34

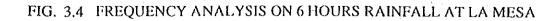
ADIL

1203 RAINFALL (mm) 6202 RAINFALL (mm) . 061 (KE2R) 1,000 6 លដ្ឋន្ល 520 600 88 THOMAS, METHOD METHOD 88 ន្ទ GUNBEL 8 т 80 80.00 90.00 10.00 3 8 ចាល⊶ -<u>8</u>.9 т (%) 80.80 80.80 8 88888882. 1000 RAINFALL (mm) 1826 RAINFALL (mm) (YEAR) 1000 1000 1000 100 100. ;00 ; 0 0 METHOD (THOMAS PLOT) HAZEN NETHOD 122 14MI 8 (*) 86.96 96.96 an Bro н 89, 99 99, 99 19 8 8 888888888 ក្ត្មីក --

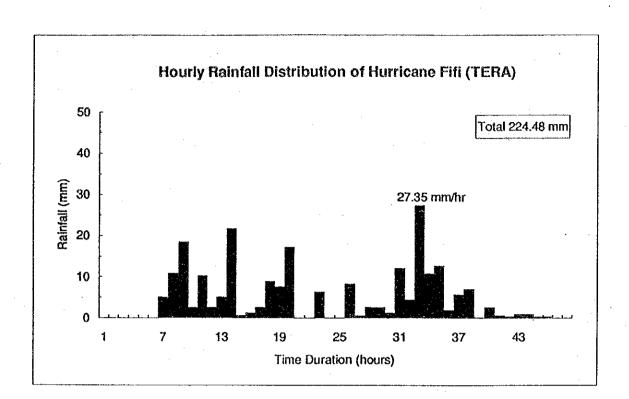
FIG. 3.3 FREQUENCY ANALYSIS ON ONE DAY RAINFALL AT PUERTO CORTES

3 - 35



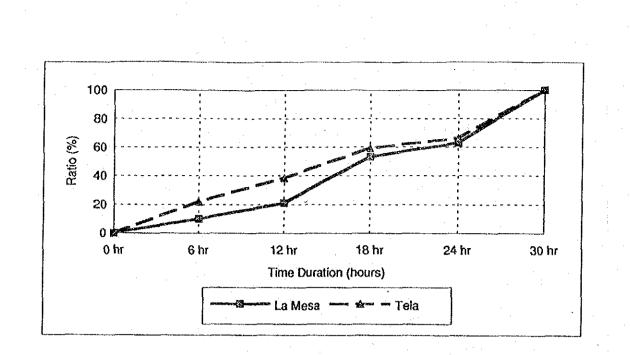


3 - 36



- hr	Rain (mm)	hr	Rain (mm)	hr	Rain (mm)	hr	Rain (mm)
. 1	0.00	13	5.09	25	0.00	37	5.73
2	0.00	14	21.63	26	8.27	38	7.00
3	0.00	15	0.64	27	0.64	39	0.13
4	0.00	16	1.27	28	2.55	40	2.55
5	0.00	17	2.55	29	2.55	41	0.64
6	0.00	18	8.91	30	1.27	42	0.32
7	5.09	19	7.64	31	12.09	43	0.96
8	10.82	20	17.18	32	4.46	44	0.96
9	18.45	21	0.00	33	27.35	45	0.32
10	2.55	22	0.00	34	10.82	46	0.32
11	10.18	23	6.36	35	12.73	47	0.00
12	2.55	24	0.00	36	1.91	_ 48	0.00

FIG. 3.5 HOURLY RAINFALL DISTRIBUTION OF THE HURRICANE FIFI AT TELA



Accumulative Rainfall Ratio

	6 hr	12 hr	18 hr	24 hr	30 hr
La Mesa	9.6	20.2	52.1	62.8	100.0
Tela	23.1	39.7	59.4	66.3	100.0

(Unit : %)

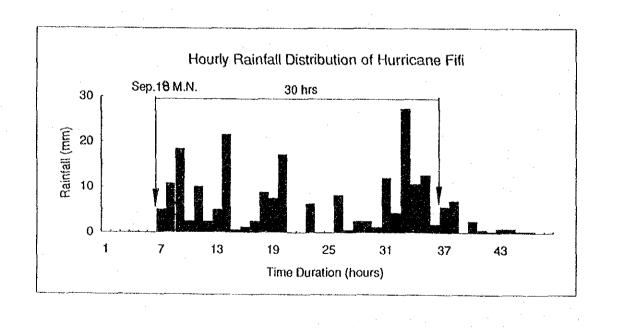
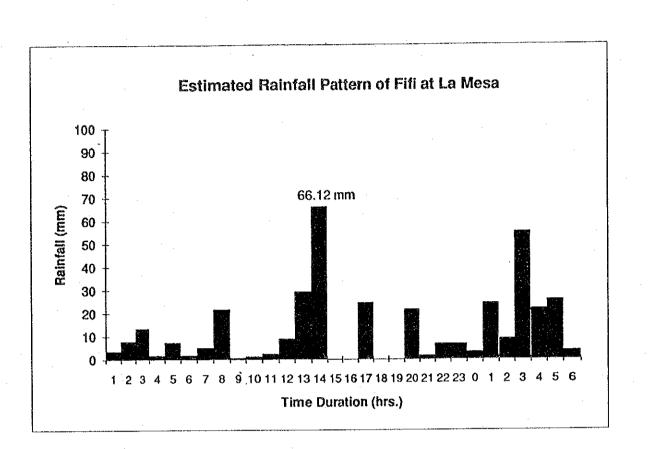


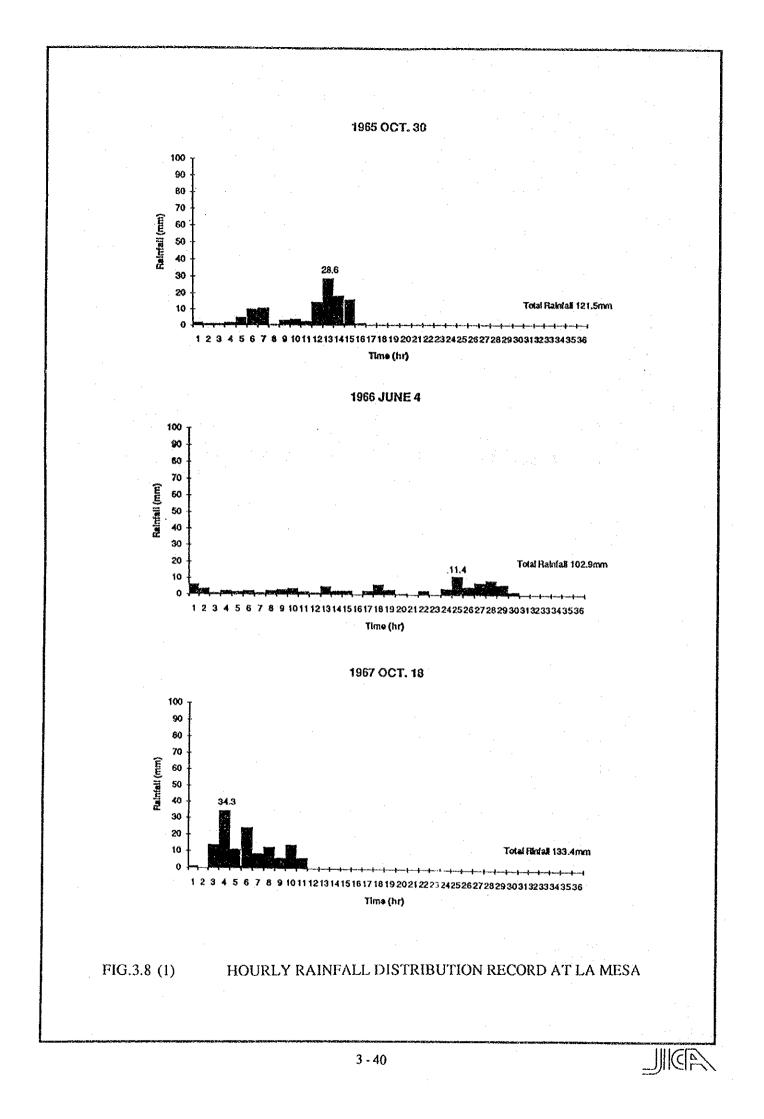
FIG. 3.6 ACCUMULATIVE RAINFALL RATIO OF THE HURRICANE FIFI AT LA MESA AND TELA

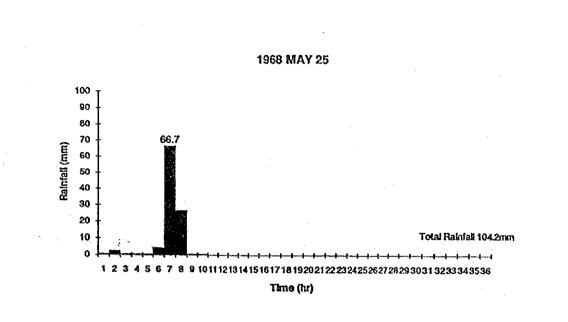


		19-Sept.			
	0 to 6	6 to12	12 to 18	18 to 24	0 to 6
1	3.69	5.08	29.40	0.00	24.40
2	7.85	21.58	66.12	21.65	9.00
. 3	13.38	0.64	0.00	1.67	55.21
4	1.85	1.27	0.00	6.68	21.84
5	7.38	2.54	24.48	6.68	25.69
6	1.85	8.89	0.00	3.32	3.86
Total	36.00	40.00	120.00	40.00	140.00

FIG. 3.7 ESTIMATED HOURLY RAINFALL DISTRIBUTION OF THE HURRICANE FIFI AT LA MESA

(C)





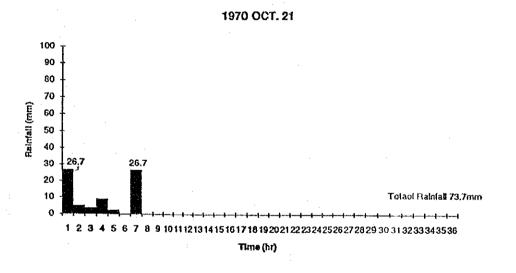
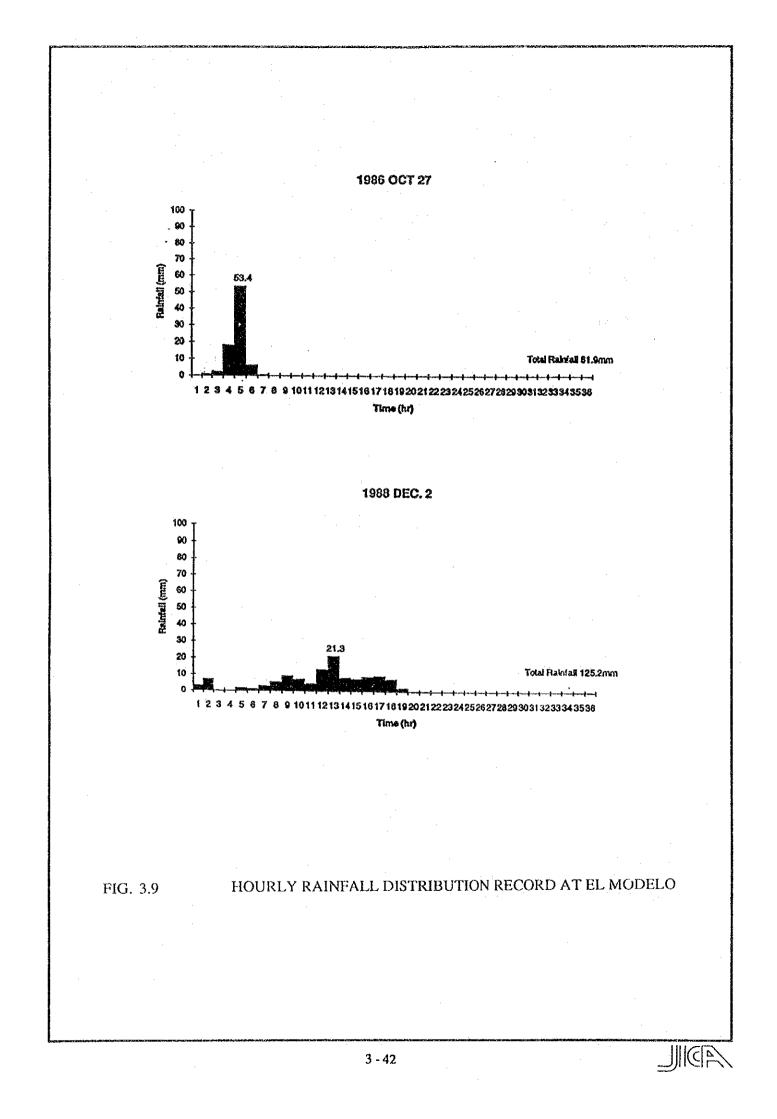
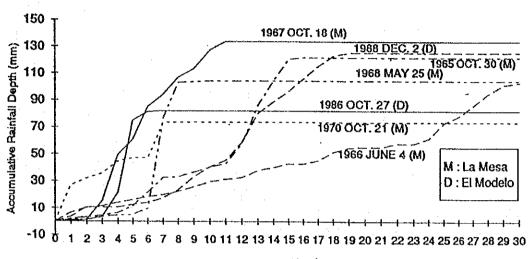


FIG. 3.8 (2)

# HOURLY RAINFALL DISTRIBUTION RECORD AT LA MESA

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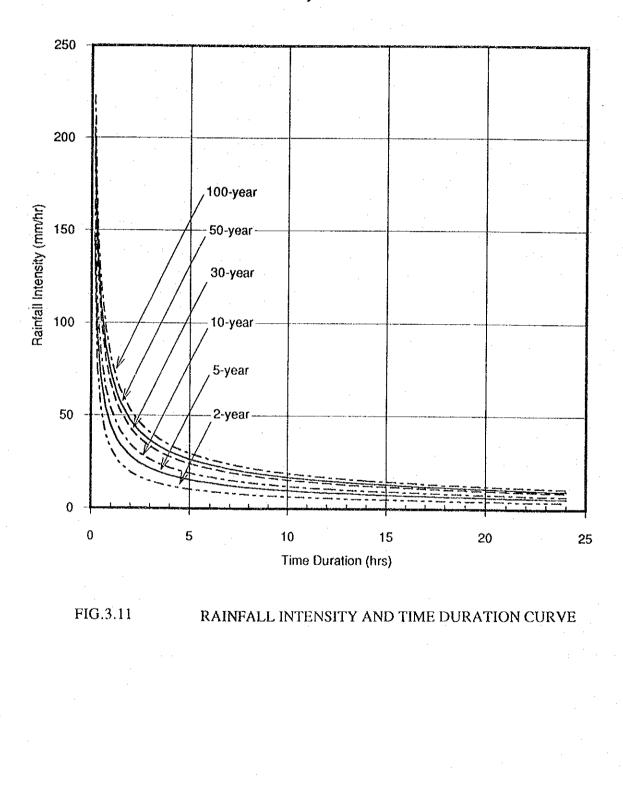




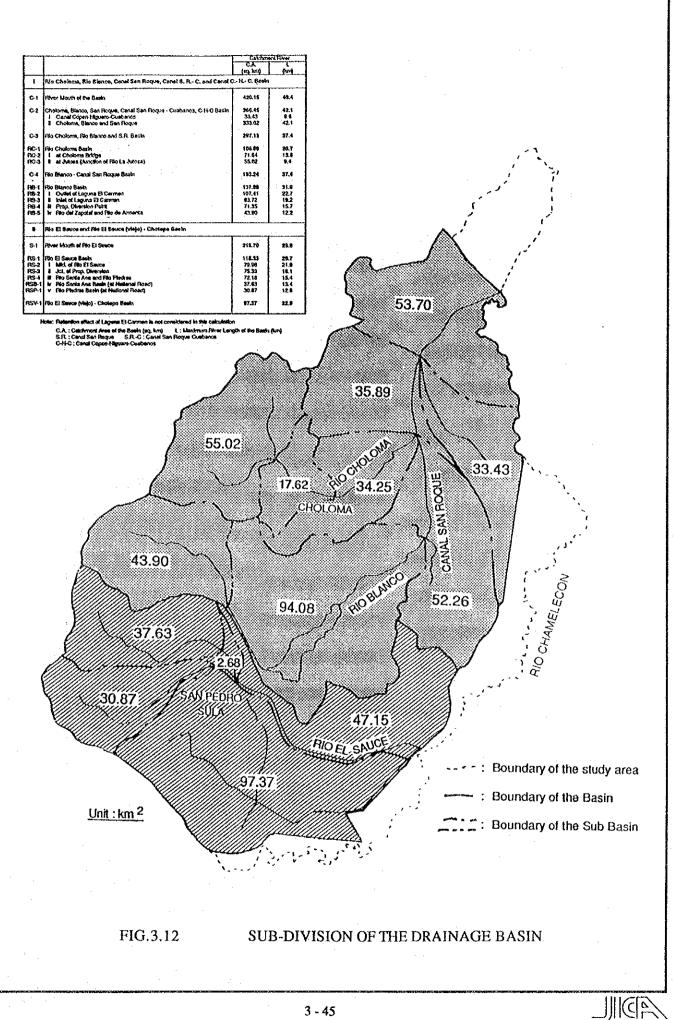
Time (Hour)

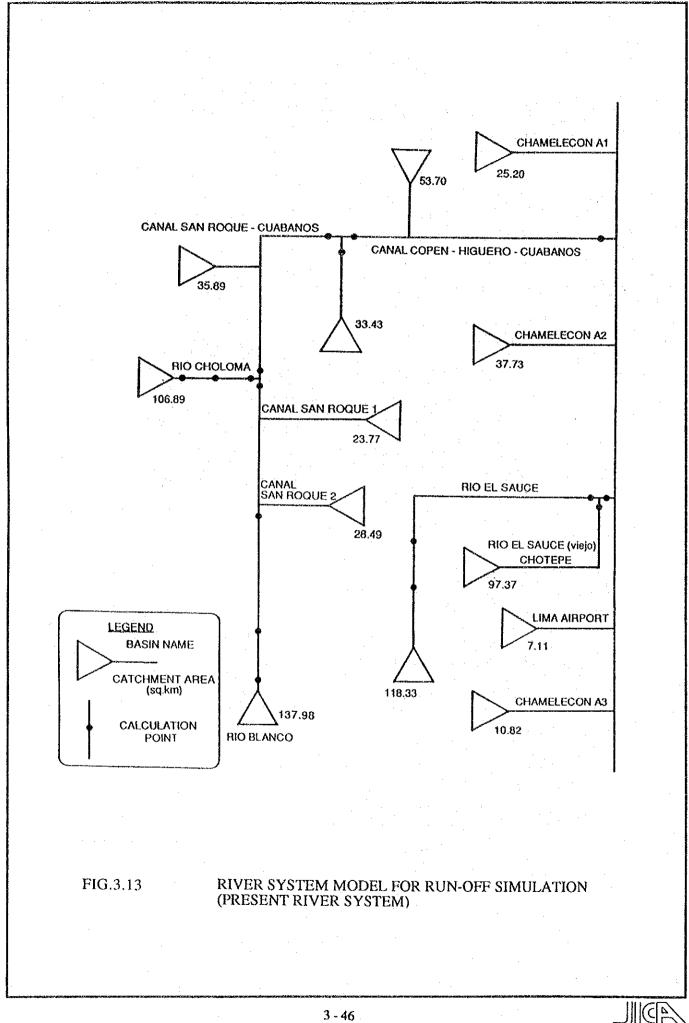
FIG.3.10

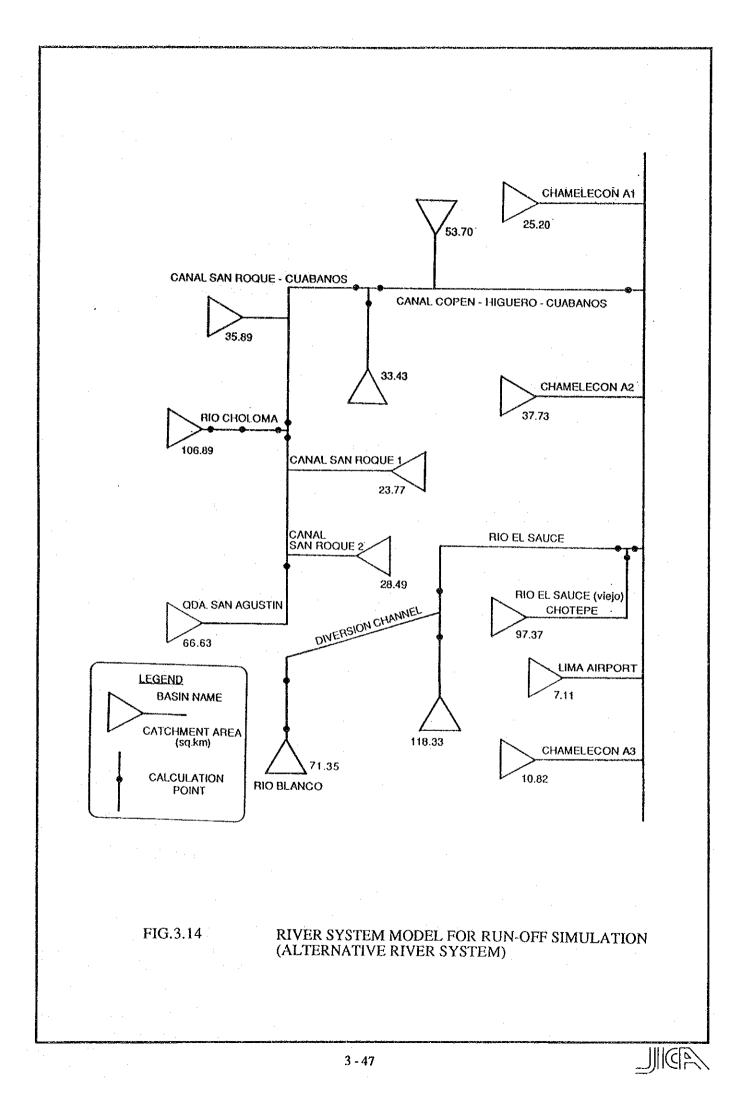
# ACCUMULATIVE RAINFALL DEPTH AND DURATION CURVE



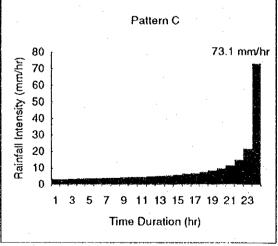
## Rainfall Intensity and Time Duration Curve







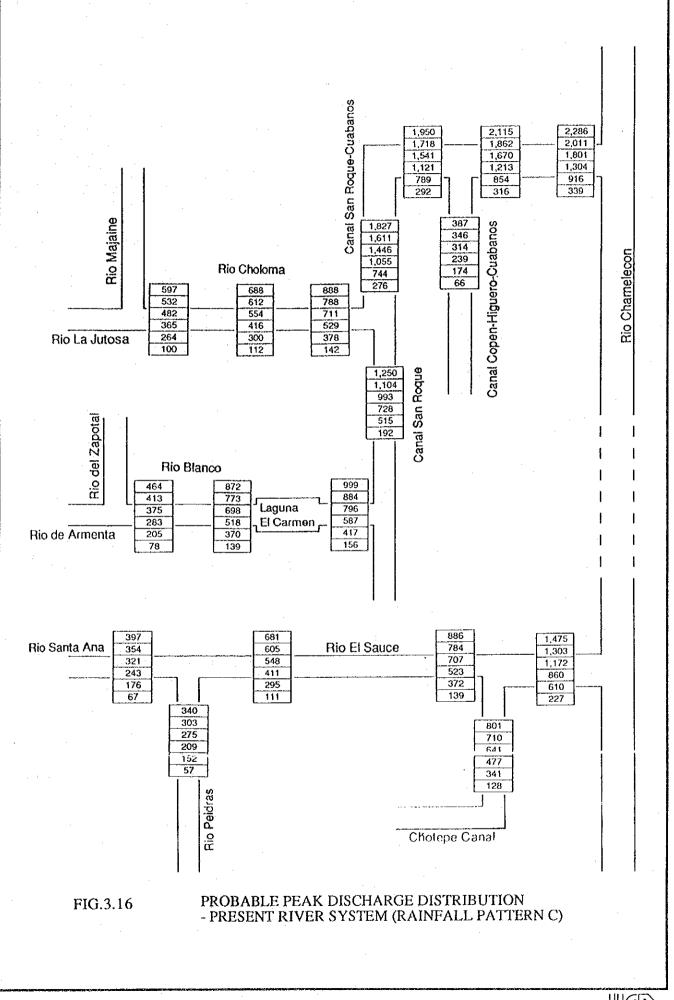
		· · ·	
	Rainfall Intensity (mm/hr)		
T (hr)	Pattern A	Pattern B	Pattern C
1	73.1	3.3	3.2
2.	21.7	3.6	3.3
3	15.0	3.8	3.4
4	11.8	4.1	3.6
5	10.0	4.5	3.7
· 6	8.7	5.0	3.8
7	7.7	5.6	4.0
8	7.0	6.5	4.1
9	6.5	7.7	4.3
10	6.0	10.0	4.5
11	5.6	15.0	4.7
12	5.3	73.1	5.0
13	5.0	21.7	5.3
14	4.7	11.8	5.6
15	4.5	8.7	6.0
16	4.3	7.0	6.5
17	4.1	6.0	7.0
18	4.0	5.3	7.7
19	3.8	4.7	8.7
20	3.7	4.3	10.0
21	3.6	4.0	11.8
22	3.4	3.7	15.0
23	3.3	3.4	21.7
24	3.2	3.2	73.1

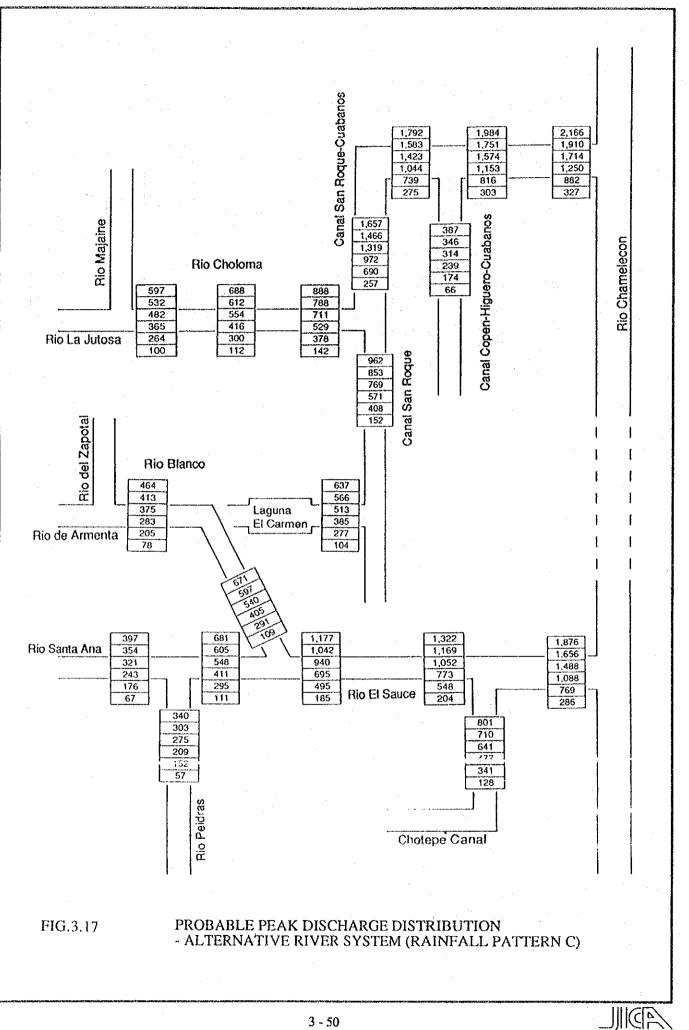


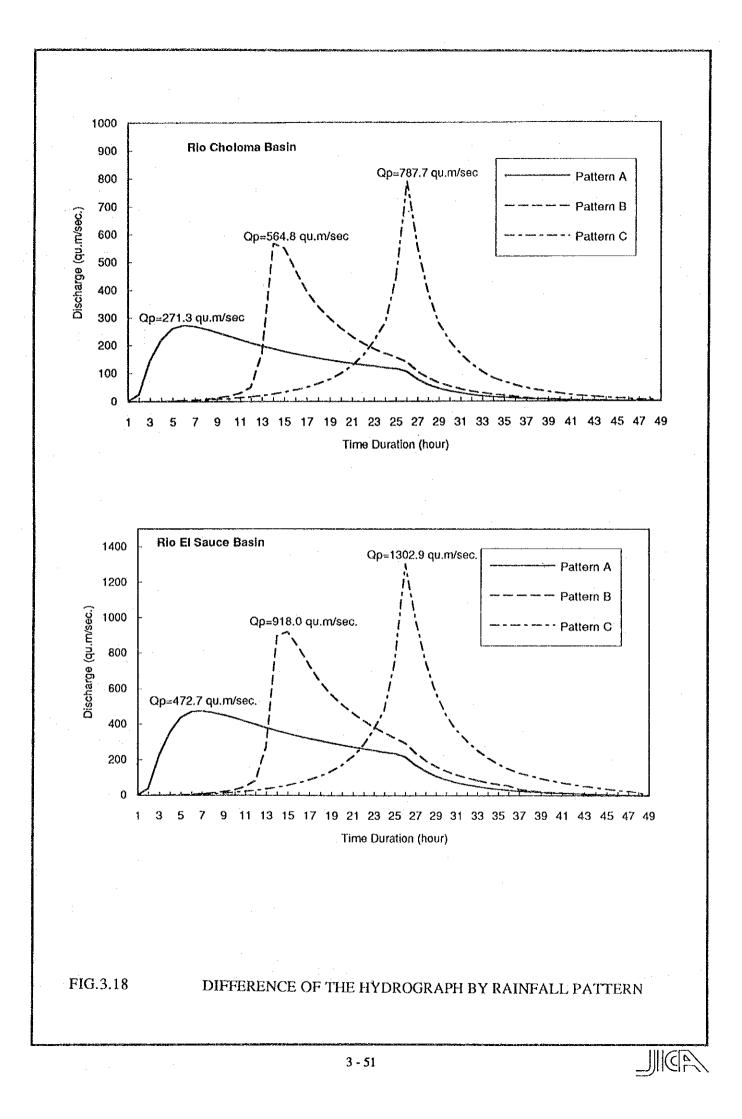
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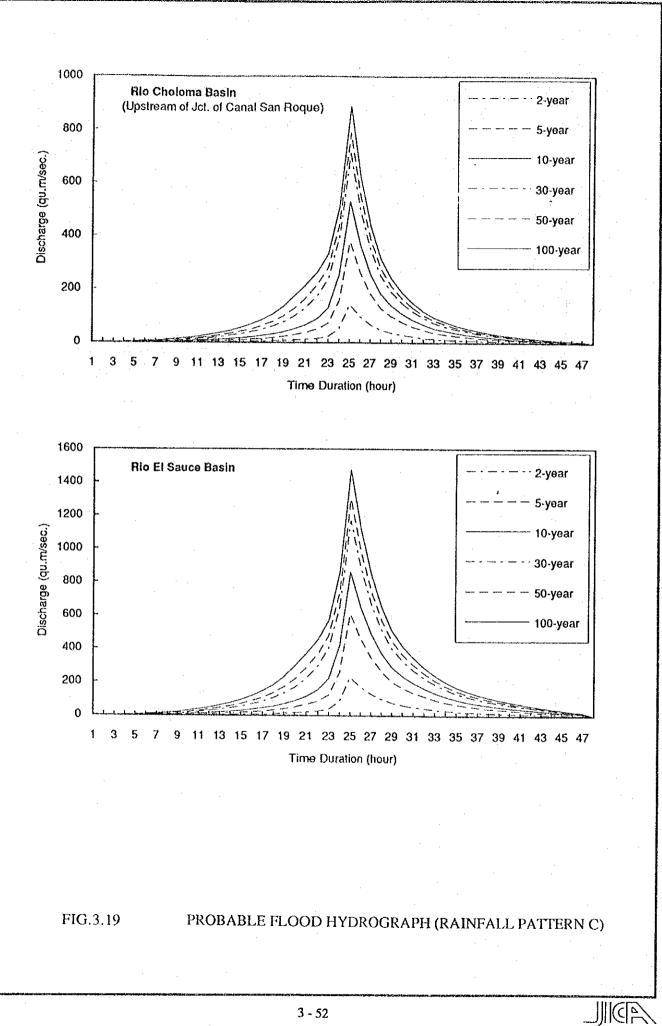
FIG.3.15

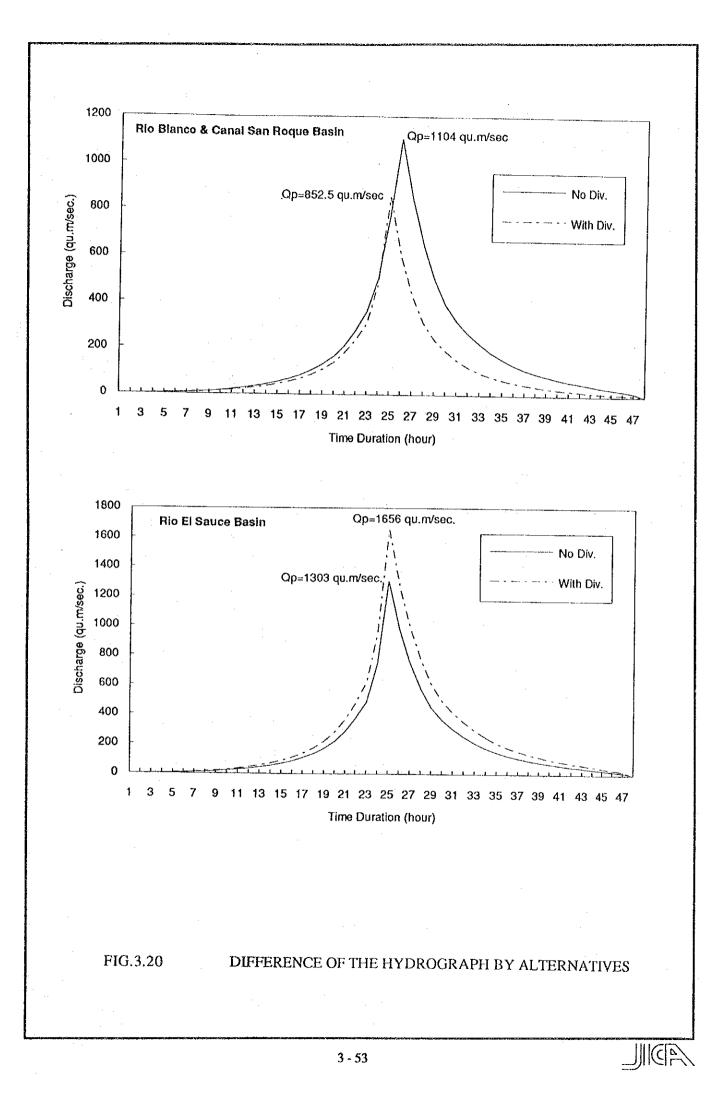
# RAINFALL PATTERNS FOR RUN-OFF SIMULATION

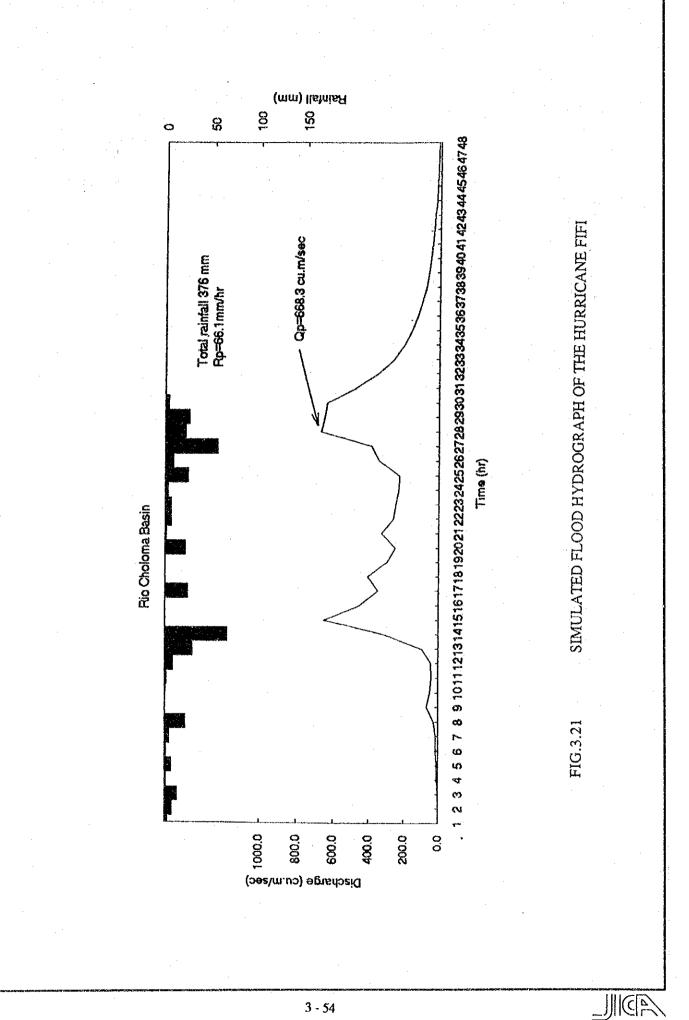


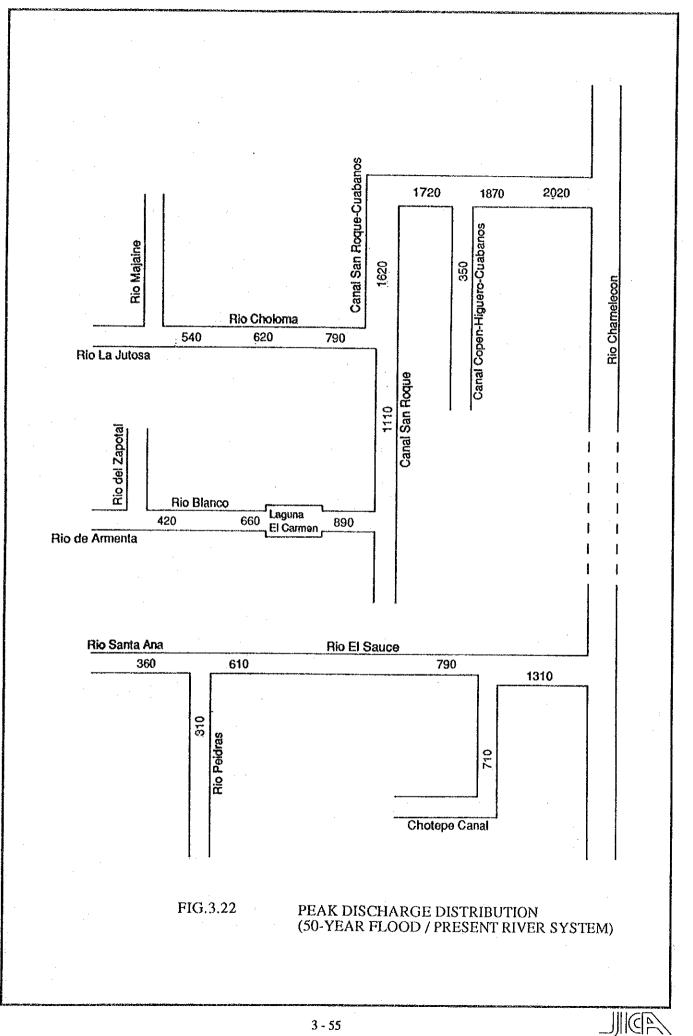


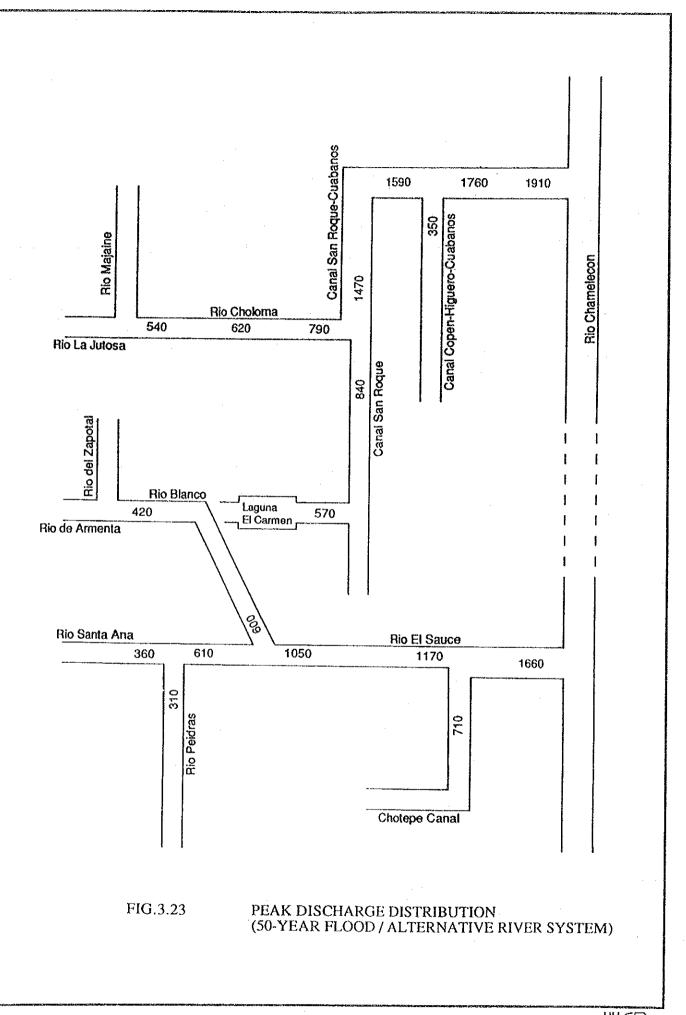




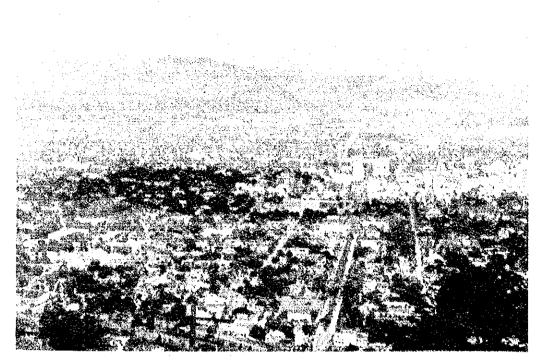








ADIL



SAN PEDRO SULA (1993)



BANANA PLANTATION ALONG THE RIO CHAMELECON (1993)

CHAPTER 4 SOCIO - ECONOMIC STUDY

## CHAPTER 4 SOCIO-ECONOMIC STUDY

### 4.1 National Socio-Economic Background

## 4.1.1 Gross Domestic Product(GDP) and Gross National Product(GNP)

GDP and GNP of Honduras showed the average annual growth rate of about 19 % at current prices (real rate of 3 %) during the period 1987-1991, and amounted to Lps. 16,406 million and Lps. 15,274 million respectively at the 1991 market prices (*Table* 4.1). Agricultural sector, which is the major economic activity of the country, accounted for more than 20 % of the total GDP during the said period.

The per capita GNP at current price amounted to Lps. 3,107 in 1991 on the growth at the average annual rate of 14.8 % during the same period, but the real growth was only an average rate of 0.2 % per annum. This real rate indicates that the average living standard of Honduran people was little improvement during the period 1987-1991.

# 4.1.2 External Trade

In 1991, Honduran exports were about US\$ 780 million, being composed of almost agricultural products such as bananas, coffee and sugar. On the other hand, the imports reached about US\$ 880 million which consisted of commodities required for the daily life and the social and economic activities of Honduran people.

During the period 1987-1991, the imports exceeded the exports every year, and such a trade deficit was trending toward increase in proportion to decrease in the exports. Actually, the deficit in 1991 amounted to about US\$ 100 million.

#### 4.1.3 Balance of International Payments

In the international payments of Honduras, the current account indicated a deficit every year for the period 1989-1991, due to the deficits of trade balance and service account. Its deficit was US\$ 194.3 million in 1989, US\$ 112.4 million in 1990 and US\$ 219.8 million in 1991 *Table* 4.2.

Such a deficit of current account was compensated by the capital account which included an external loan, with the aim of maintaining the balance of international payments. As a result, the external loan of Honduras was being accumulated year by year.

## 4.1.4 Government Finance

Revenue (or expenditure) of the Central Government increased at the average rate of

17.9 % per annum during the period 1987-1991, and amounted to Lps. 4,643.7 million in 1991. In the same year, more than 80 % of the Government revenue were occupied by tax revenue and internal and external debts. Among the revenue, it is noticeable that the external debt was being considerably increased in recent years. During the said period, a new external debt exceeded amortization of the debt every year, for example, in 1991 the difference between both amounts was Lps. 507.8 million.

On the other hand, the expenditure of the Central Government in 1991 was mainly composed of current expenditure (58 %), investment (17 %) and amortization of public debt (22 %). Details are given in *Table* 4.3.

# 4.1.5 National Integral Development Strategy

According to the National Integral Development Strategy of Honduras for the period 1990-1994 which was published by the SECPLAN in 1990, the real economic growth is projected at a rate of 3 or 4 % for this period, expecting increase in exports and private investment. During the same period, it is expected that the financial requirements of the government will amount to US\$ 2,489 million, which include grants for the balance of payments, operations to pay-off arrears and repayments.

# 4.2 Socio-Economic Condition in the Study Area

### 4.2.1 Location and Administration

The Study Area is located in the northern part of Honduras, and extends from  $15^{0}24'$  to  $15^{0}48'$  north latitude and from  $87^{0}46'$  to  $88^{0}07'$  west longitude. It has an area of 717 km².

The Study Area is included in the Department of Cortes and spreads over four Municipalities; San Pedro Sula, Choloma, La Lima and Puerto Cortes. Each Municipality is divided into cities (municipios), towns (pueblos) and villages (aldeas) in accordance with a population-scale. San Pedro Sula, the second largest city of Honduras, forms the central area of administration, industry and commerce in the northern part of Honduras.

# 4.2.2 Population and Housing

### 1) Population and Housing Censuses

Since 1961, the Republic of Honduras has conducted three population and housing censuses in 1961, 1974 and 1988. The Honduran population amounted to 4,443,721

in 1988, on the increase from 2,656,948 in 1974 and 1,884,765 in 1961. The average annual growth rate was 2.68 % for the period 1961-1974 and 3.74 % for the period 1974-1988. The average population density in the country as a whole was 40 persons/km² in 1988 (*Table 4.4*).

The total population of four Municipalities, San Pedro Sula, Choloma, La Lima and Puerto Cortes, which include the Study Area, reached 500,886 in 1988 on the increase rapidly from 281,247 in 1974 and 137,988 in 1961. The average annual growth rate showed 5.63 % for the period 1961-1974 and 4.21 % for the period 1974-1988. The population density came to 268 persons/km² in 1988 (*Table* 4.5).

The population censuses indicate that the Study Area and its surrounding areas were trending toward urbanization rapidly since 1961. In the said four Municipalities, urban population accounted for 77 % of the total population in 1988, on the increase from 66 % in 1974 and 58 % in 1961. Especially in the Municipality of San Pedro Sula, the urban population reached 88 % in 1988 (*Table* 4.6).

According to the National Housing Census in 1988, the said four Municipalities had 106,302 houses in total at the average density of 57 houses/km², composed of 77 houses/km² in San Pedro Sula, 29 houses/km² in Choloma, 85 houses/km² in La Lima and 35 houses/km² in Puerto Cortes (*Table 4.7*).

Further, the 1988 Census indicates that the average family size per house was 4.71 persons/house in the whole area of the four Municipalities, 4.70 persons/house in San Pedro Sula, 5.06 persons/house in Choloma, 4.64 persons/house in La Lima and 4.48 persons/house in Puerto Cortes.

### 2) Estimates of Population and Number of Buildings in the Study Area

The population and number of buildings in the Municipality shown in *Table 4.7* are the whole figures in each Municipality, not figures in the Study Area where is composed of a part of each municipal territory. Therefore, with the object of getting a basic data required for analyzing the potential flood damage, the JICA Study Team surveyed the 1992 population together with number of buildings in the Study Area.

The 1992 population in the Study Area is approximately estimated at 520,000, composed of 384,000 in the San Pedro Sula area, 101,000 in the Choloma area, 20,000 in the La Lima area and 15,000 in the Puerto Cortes area.

Out of the total population in the Study Area in 1992, the urban population is estimated at 467,000 accounting for 90 % of the whole, and the remaining populations of 48,000