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JAPAN INTERNATIONAL COOPERATION AGENCY(JICA)

DIRECTORATE GENERAL OF WATER MINISTRY OF PUBLIC WORKS
THE REPUBLIC OF CHILE

# THE STUDY

ON

# THE DEVELOPMENT OF WATER RESOURCES IN

# NORTHERN CHILE

SUPPORTING REPORT D: WATER SUPPLY DEVELOPMENT



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#### SUPPORTING REPORT - D MUNICIPAL WATER SUPPLY DEVELOPMENT

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# D-I ARICA CITY

#### SUPPORTING REPORT D.

#### MUNICIPAL WATER SUPPLY DEVELOPMENT

#### D-I. ARICA CITY

#### Chapter I. EXISTING WATER SUPPLY SYSTEM

#### 1.1 Water Supply System

#### 1.1.1 Outline of the System

The existing public water supply system for Arica City, operated and maintained by a semi-governmental organization, ESSAT (Tarapaca Sanitary Services Enterprise), is outlined below:

The water source is groundwater, submerged along the San Jose River, which flows east to west towards the Pacific Ocean, penetrating the city area. The groundwater is extracted from deep wells, located in the Azapa Valley along the river and in the city area, by submersible pumps installed in each well.

The water extracted from the wells is first transmitted through an intake pipeline to a collection tank located at approximately the center of the well-field. At the collection tank, the water is sterilized by using hypo-chlorite, which is either injected to the inlet pipeline or to the outlet pipeline of the tank. The chlorinated water is then delivered to the distribution tanks located on hilly areas in the east of the city. From the distribution tanks, water flows by gravity through the distribution networks to the consumers in the city.

#### 1.1.2 Area and Population Served

The existing water supply system covers about 1,680 ha, which is divided into 4 service sectors of the urbanized area of the city, serving almost the entire population of the city. The total population was 169,200 in 1992, as per the preliminary census results.

The number of service connections registered was 39,040 as of Feb. 1994. The number of connections added during a period of one year from Feb. 1993 to Feb. 1994 was 1,210.

A sketch of the existing water supply system including the areas of service sectors, is shown in Figs. D-I,1.0, 1.1 and 1.2.

#### 1.1.3 Water Production and Consumption

The rate of water production, consumption by category and losses in 1992 are summarized as follows:

	1992 Quantit (x1,000 m <sup>3</sup> )		
Production	16,940.7	$=(46,410 \text{ m}^3/\text{day} = 537 \text{ l/sec})$	
Consumption	10,635.2	100	1 1 7
- Residential	8,170.8	76.8	Are in
- Commercial	1,087.3	10.2	
- Industrial	919.3	8.6	100
- Other	457.8	4.3	
Losses	6,305.5	(= 37.2% of production)	

Note:

The water losses indicated above consist of physical leakage and commercial losses, including the unbilled water consumption occurring in the residential category as a result of meters in poor operating conditions and illegal connections.

The above production was obtained from 28 deep wells (16 in the Azapa Valley and 12 in the city area), with an authorized capacity of 503 l/sec. During the year 1993, additional deep wells, producing 227 l/sec, were developed and commissioned by ESSAT as an Emergency Project.

Thus, the per capita water production and consumption on capacity basis in 1992 and 1994 are estimated as follows:

	*	Year	1992	1994
(A)	Proc	fuction capacity of deep wells	503 l/sec = 43,500 m <sup>3</sup> day	730  l/sec = 63,070 m <sup>3</sup> /day
(B)	Pop	ulation served	169,212 (Census)	175,100 (linear growth rate of 1982-1992)
(C)	Per	capita water use available		
	(1)	Production basis	257 l/day	360 l/day
	(2)	Total consumption basis	161 l/day	226 l/day
	(3)	Residential consumption basis	124 l/day	174 l/day
N	Vote:	(1)=A/B		
		$(2)=(1)\times62.8\%$		

(3)=(2)x76.8%

#### 1.1.4 Present Problems

Technical problems in the present water supply for Arica are summarized below:

#### 1) Supply Capacity

Due to the shortage of water, water supply in Arica City was limited to 10.5-15 hours per day in all the service sectors until the year 1992. Additional deep wells were developed in 1993, both in the upper Azapa Valley and in the city, in order to supplement the supply capacity. Due to above supplement, the water restriction has been remarkably decreased. However, even now (early 1994) water restrictions are imposed in two service sectors (II & III) and the supply is limited to 14-15 hours per day.

#### Water Source 2)

The present water source is solely concentrated in the San Jose River Basin (the Azapa Valley and in the city) from which the water is mainly extracted for agricultural purposes by the commercial sector and for municipal water supply by ESSAT. Thus, the river has been already developed beyond its' full capacity. It has been verified that further development of water source could not be possible.

The future water demand will inevitably increase according to the growth of the population and the development of the city. In order to meet the future demand, new water sources need to be developed in places other than the Azapa Valley.

#### 3) Water Quality

The groundwater extracted from the existing deep wells located along the San Jose River contains considerably high concentration of TDS which causes salty taste. This occurs particularly at deep wells located in the city area, being near to the sea. This might be due to the excessive extraction of groundwater. Such an excessive intake of the groundwater from the San Jose River Basin will undoubtedly deteriorate the water quality in future.

Therefore, from the points of view of quality and quantity, new water sources other than the Azapa Valley need to be developed without any further delay.

#### 4) Water Loss

The water loss, ie. the unpaid water in the system, is comparatively higher which was found to be 37.2% of the total production in 1992.

The loss consists of (i) physical loss (leakage from pipelines, distribution networks in particular) and (ii) commercial loss (unbilled water consumption occurring mainly in the residential areas as a result of meters in poor operating conditions and illegal connections).

An unaccounted amount of water loss take place between the service meters and the consumers due to domestic leakage and wastage.

The above losses are apparently recognized as the physical waste of the water resource and a financial loss to ESSAT.

A well-maintained public water supply system would be able to keep the water loss in a range of 20% of the production.

There is no specific methodology to reduce the water loss. The problem of the loss could be solved only by the steady and continuous effort by the maintenance body, ESSAT.

In order to minimize the water loss through the system, the following tasks are to be considered in general and performed.

- Immediate repair work for visual leakage on main pipelines.
- Replacement and repair of old/deteriorated pipes.
- Detection and repair of invisible leakage on distribution networks, particularly on service connections.

- Technical check on existing service water meters in doubt, and repair or replacement of deteriorated meters.
- Detection of illegal connections.
- Reduction of high pressure in the pipelines to a reasonable lower pressure.
- Public education program in order to increase the awareness of water saving among the public.
- Encourage the consumers to use water saving devices, plumbing fixtures, etc.

#### 1.2 Water Supply Facilities - Arica

#### 1.2.1 Deep Wells and Intake Pumps

There were 45 deep wells for the public water supply system for Arica, as of February 1994. Their locations are distributed (i) in the Azapa Valley and (ii) in the city area. The details of wells are; depth approximately 50-110 m, casing pipe diameters 250-340 mm, and yields approximately 15-25 l/sec (18 l/sec on average). All wells are equipped with submersible pumps, capacity of which is 20-35 l/sec in majority, 50-130 m in total head. The electric power for all the pumps is supplied by a public electric enterprise (EMELARI). The groundwater extracted from the deep wells is raised by the submersible pumps to a collection tank through intake pipelines.

All the existing deep wells are listed in Table D-I, 1.1 and the details of the submersible pumps are given in Table D-I, 1.2.

A typical well structure is sketched in Fig. D-I,1.13 and a typical layout of a pump house is shown in Fig. D-I,1.14.

#### 1.2.2 Collection Tanks

The groundwater extracted from the wells first flows into a collection tank. At the site of the collection tank, the water is chlorinated by adding hypo-chlorite. There are three (3) collection tank stations: (i) Pago de Gomez Station and (ii) Azapa Station, both located in the Azapa Valley; and (iii) Estadio Station in the city area. The tanks which were made of steel or reinforced concrete, are located slightly below the ground level.

From the tanks, the potable water is transmitted to the distribution tanks, through the transmission pipelines. The collection tanks located at higher places in the Azapa Valley supply water by gravity to the Chuno distribution tank, the Saucache distribution tank and the Pampa Nueva distribution tank. In the case of the Estadio station, being situated at a lower altitude within the city area, water is lifted by pumps at the station to the La Cruz distribution tank.

All the collection tanks are listed in Table D-I,1.3 and layout of the three tank stations is shown in Figs. D-I, 1.3, 1.4, 1.5 and 1.12.

#### 1.2.3 Distribution Tanks

There are six (6) distribution tanks in Arica City, as of February 1994. All the tanks are made of steel or reinforced concrete and are constructed slightly below the ground level, on the eastern side or southern side hills of the city.

#### They are:

- (i) Chuno Tank (V=5,000 m<sup>3</sup>, HWL=+84.5 m; to Service Sector II)
- (ii) Saucache Tank (V=2,500 m<sup>3</sup>, HWL=+104.6 m; to Sector III)
- (iii Pampa Nueva Tank (V=1,000 m<sup>3</sup>, HWL=+129.1 m; to higher land of Sector III)
- (iv) La Cruz Tank (V=2,500x2=5,000 m<sup>3</sup>, HWL=+81.5 m; to Sector I)
- (v) Rosado Tank (V=800 m<sup>3</sup>, HWL=+109.5 m; to Sector IV)
- (vi) La Lisera Tank (V=200 m<sup>3</sup>, HWL=+30 m; to south-coastal area of Sector I)

The Pago de Gomez and Azapa Collection Tanks supply water by gravity to the Chuno, Saucache and Pampa Nueva Tanks. To the La Cruz Tank, water is pumped from the Estadio Station. Some of the water in the La Cruz Tank is transferred to the Rosado Tank by booster pumps installed at the site of the La Cruz Tank. The La Cruz Tank also supplies water to the La Lisera tank. The water flows by gravity up to the foot of the La Lisera hill and then is lifted up to the tank using the booster pumps installed at the foot of the hill.

The potable water stored in the distribution tanks is supplied, by gravity through distribution networks to service sectors in Arica City.

The distribution tanks are listed in Table D-I,1.4 and layout of the tank station is shown in Figs. D-I, 1.6 to 1.11; and structural drawings of typical distribution tanks in Figs. D-1, 1.15 and 1.16.

#### 1.2.4 Transmission Pipeline

The groundwater extracted from the wells in the Azapa Valley is transmitted to distribution tanks located in the city area by gravity transmission pipelines.

The water from the well fields in Cabuza and San Miguel is transmitted to Pago De Gomez via an AC pipeline, 400 mm in dia. at the maximum. Wells located between San Miguel and Pago de Gomez, are also connected to this transmission line. There are collection tanks at Pago de Gomez, which collect the water within the Pago De Gomez well field (4)

wells); and at Azapa collecting the water in the Azapa well field (9 wells). Between Pago de Gomez and Azapa, there are two parallel transmission pipelines, each approx. 3.2 km long and 450 mm diameter. One transmission line carries water to the Chuno distribution tank and the other to the Saucache distribution tank.

Details of the transmission pipelines are shown in Table D-I, 1.5.

#### 1.2.5 Transmission/Booster Pumps

#### Transmission Pumps

The Estadio collection tank is situated at a low altitude place. Therefore, water in the tank is transmitted to the La Cruz distribution tank (HWL=+81.5 m) by the transmission pumps which are installed in the Estadio Station.

Specification of the transmission pumps is given in Table D-I, 1.6.

#### **Booster Pumps**

In addition to the above, there are three small booster pump stations located in some fringe areas of the city, in order to boost water pressure for the supply to high altitude areas or remote areas. They are at places of (i) La Cruz tank (for Rosado tank) (ii) Rosado tank (for higher altitude hinterlands) and (iii) at the foot of the La Lisera hill (for La Lisera tank).

Specifications of the booster pumps are given in Table D-I, 1.7.

#### 1.3 Organization and Operation

(Note): The following information and data cover both Arica and Iquique Cities at an aggregated level.

#### 1.3.1 Organization

Water supply management is being carried out by Empresa de Servicios Sanitarios de Tarapaca (Tarapaca Sanitary Services Enterprise, ESSAT). It is a stock company established on April 9, 1990 and replaced the Servicio Nacional de Obras Sanitarias, SENDOS Region I (National Service for Sanitary Works, SENDOS Region I).

ESSAT has only two shareholders, Corporacion de Fomento para la Produccion (Production Promotion Corporation, CORFO) and the Public Treasury. The former owns 99% of the shares and the latter, 1%.

The objective of ESSAT is to produce and distribute drinking water, recollect, treat and dispose sewage, and carry out other services related to such activities in the way and manner established in the Decree-Laws No. 382 and No. 70, both issued in 1988 by the Ministry of Public Works.

ESSAT serves the Tarapaca Region (Region I) that includes Arica, Iquique, Pozo Almonte, Pica, Matilla, Huayca, La Tirana, La Huara, and Pisagua.

ESSAT is managed by a Board of Directors composed by seven members who are appointed for a two-year period. The directors are designated by the Ordinary Meeting of Shareholders.

In turn, the Board of Directors elects its own President and Vice-President and designates the General Manager.

There are the following Managerial Departments:

- General Management
- Engineering
- Administration and Finance
- Planning

Arica and Iquique have Provincial Branches and their managers also belong to the top management of ESSAT.

Besides the above mentioned Managerial Departments, ESSAT has the following Counseling Units.

- Legal
- Internal Control
- Rural Drinking Water
- Public Relations

The Engineering, Administration and Finance, and Planning Departments, and the Iquique and Arica Provincial Branches all depend on the General Management Department.

The Engineering Management Department carries out studies concerning drinking water and sewerage, and set the technical norms for operation of the firm; it also coordinates and controls the operation and maintenance programs, water quality, and sewage collection.

The Administration and Finance Management Department is in charge of administration of human, financial and information resources related to administrative and commercial

aspects of the firm. This department sets norms and policies seeking to maximize revenues, minimize costs and optimize utilization of human and material resources, and required services for operation of the firm. It also sets the norms regulating the relationship on commercial matters between the firm and its clients.

The Arica and Iquique Provincial Branches are responsible for the operation and maintenance of the installations, works and equipment of the drinking water and sewerage systems. They are also in charge of the invoicing and collection of tariff for water and sewerage services.

Personnel is made up of 289 people at the end of 1993 and is classified as shown below:

Top management	10
Technical staff	53
Other staff	226
Total	289

Source: ESSAT 1993 Annual Memory

Second-level chiefs, professionals and qualified personnel are considered as technical staff. Administrative staff, drivers, watchmen, etc. are within the "Other Staff" category.

Remuneration are decided based on collective agreements reached between the management and workers. These agreements contemplate the semi-annual readjustment of remuneration up to a 100% of the variation of the consumer's price index.

#### 1.3.2 Operation and Maintenance

#### 1) Budget and Revenues

The budget for the year 1993 was 17% higher than that for 1992. See below for details.

Budget for the Period 1990-1993 (Unit: Million Pesos)

1993	%	1992	%	1991	%	1990	%
839	25	733	26	571	24	364	19
303	9	225	35	167	20	139	7
167	- 5	176	6	125	5	46	2
1,579	48	1,395	50	1,328	55	1,258	66
52	2	42	1	35	2	22	2
356	11	249	9	171	7	84	4
3,296	100	2,820	100	2,397	100	1,913	100
	839 303 167 1,579 52	839 25 303 9 167 5 1,579 48 52 2 356 11	839 25 733 303 9 225 167 5 176 1,579 48 1,395 52 2 42 356 11 249	839     25     733     26       303     9     225     35       167     5     176     6       1,579     48     1,395     50       52     2     42     1       356     11     249     9	839     25     733     26     571       303     9     225     35     167       167     5     176     6     125       1,579     48     1,395     50     1,328       52     2     42     1     35       356     11     249     9     171	839     25     733     26     571     24       303     9     225     35     167     20       167     5     176     6     125     5       1,579     48     1,395     50     1,328     55       52     2     42     1     35     2       356     11     249     9     171     7	839     25     733     26     571     24     364       303     9     225     35     167     20     139       167     5     176     6     125     5     46       1,579     48     1,395     50     1,328     55     1,258       52     2     42     1     35     2     22       356     11     249     9     171     7     84

Source: ESSAT 1993, 1992 Annual Memories

Total revenue collected during 1993 was \$7,627 million. Breakdown of the revenue is shown below:

Revenues for the 1990-1993 Period (Unit: Million Pesos)

Item	1993	%	1992	%	1991 %	1990 %
Water sale	5,349	70	4,244	79	3,372 83	2,237 98
Investment profit	126	- 2	47	1	17 -	17 1
Transfers	1,913	25	1,077	20	680 17	
Other revenues	240	3	2		1, -	18 1
Total	7,627	100	5,370	100	4,070 100	2,272 100

Source: ESSAT 1993, 1992 Annual Memories

Invoicing is done on a monthly basis for all clients and during 1993, 97% of total invoiced tariff was actually collected.

Tariff Collection Efficiency (Unit: Million Pesos)

Year	Invoiced \$	Collected \$	Collection efficiency %
1990	2,371	2,237	94
1991	3,549	3,372	95
1992	4,430	4,244	96
1993	5,529	5,349	97

Source: ESSAT 1993, 1992 Annual Memories

#### 2) Production and Invoicing

Water production and invoicing for the period 1990-1993 are shown below:

Production and Invoicing for Period 1990-1993 (Unit: Million Pesos)

Y	<b>Теаг</b>	Production	Invoiced	- %
	990	35,891	21,327	59
	991	34,787	22,031	63
· .	992	35,692	22,494	63
	993	38,189	21,466	56

Source: ESSAT 1993 Annual Memories

#### 3) Tariff Structure

Tariffs are set by the Government through the Ministry of Economy, Foment and Reconstruction. The tariffs are denominated "self-financing tariffs" because they are set aiming to recover the costs of the water supply companies; they are based on an economic efficiency point of view and taken into consideration the long-term development and growth costs of the companies. The government sets the "self-financing tariffs" or "target tariffs" which must be reached within a period of 5 years; it means that ESSAT has 5 years to reach those tariffs. Tariffs are adjusted based on two concepts:

- Through adjustment of the tariffs themselves: They correspond to an annual percentage increase determined by the Government in order to reach the "self-financing tariffs" within the established period of 5 years (1990 to 1994).
- Through readjustment of the price indexes considered in the formulas applied for setting up the tariffs: Re-adjustments will be applied each time any of the indexes shown below raises above 3%.

#### The indexes are:

PIC : Price Index of Cement

PII : Price index of Iron

PIR : Price Index of Remunerations

PIIE : Price Index of Industrial Electricity

PIPD: Price Index of Petrol. Diesel

WPI: Wholesale Price Index

WPIIP: Wholesale Price Index of Imported Products

Tariffs applied at present by ESSAT correspond to 75% of the "self-financing tariffs" set up by the government.

The present "self-financing tariffs" were established through Legislative Decree No. 376 issued on November 15, 1990, by the Ministry of Economy, Foment and Reconstruction.

The tariff structure contemplates imposing following charges to the clients:

- Fixed Charges for Water and Sewerage: These charges are independent from water and sewerage services consumption.
- Fixed "Client Charges": These charges aim to cover the expenses incurred by ESSAT when attending the clients like water meter reading, inspections, etc.

- Variable Charges: These charges depend on the water consumption measured in cubic meters.
- Over-consumption Variable Charges: They are applied during the summer season, denominated "peak season", from December to March, to the consumption over the average consumption obtained during the "off-peak season" from April to December. These charges apply to consumption over 30 m<sup>3</sup>.

The tariffs applied at present by ESSAT for Arica and Iquique are as follows:

Water and Sewerage Tariffs (February 1994)

# (a) Group No.1: Arica-Pica-Matilla-La Huayca

Monthly Fixed Charge

Service Diameter (mm)	Water Tariff \$	Sewerage \$	Client \$
13 Rebated	231	104	206
15 Normal	334	151	297
19 Normal	667	302	297
25 Normal	1,335	605	297
32 Normal	2,002	907	297
38 Normal	3,003	1,361	297
50 Normal	5,005	2,268	297
75 Normal	11,678	5,293	297
100 Normal	20,019	9,074	297
125 Normal	30,028	13,611	297
150 Normal	45,042	20,416	297
200 Normal	80,075	36,295	297

#### Monthly Variable Charge

	\$ Normal	\$ Rebated
- Water consumption	140.02	108.68
- Water consumption during peak season	140.02	108.68
- Water over consumption	363,74	363.74
- Sewerage without treatment	43.45	25.01
- Sewerage with treatment	51.68	29.75

(b) Group No.2: Iquique-Pozo Almonte-La Tirana-Huara-Pisagua

Monthly Fixed Charge

Service Diameter (mm)	Water Tariff \$	Sewerage \$	Client \$		
15 Rebated	209	93	194		
15 Normal	337	149	298		
19 Normal	675	298	298 298		
25 Normal	1,350	597			
32 Normal	2,025	895	298		
38 Normal	3,037	1,343	298		
50 Normal	5,062	2,239	298		
75 Normal	11,812	5,223	298		
100 Normal	20,249	8,954	298		
125 Normal	30,373	13,432	298		
150 Normal	45,560	20,147	298		
200 Normal	80,995	35,817	298		

#### Monthly Variable Charge

	\$ Normal	\$ Rebated
- Water consumption	233.44	130.90
- Water consumption during peak season	230.82	130.20
- Water over consumption	564.20	564.20
- Sewerage without treatment	41.90	19.65

#### 4) Clients and Coverage

By the end of 1993, ESSAT was supplying water services to 77,264 clients and sewerage services to 73,845 clients. It means that almost 98% of the population of the region in 1993 received drinking water services while 96% was covered by sewerage services. See below for details.

Clients and Water Service Coverage

	19	93	19	92	19	91	
Service	Clients (number)	Coverage (%)	Clients (number)	Coverage (%)	Clients (number)	Coverage (%)	
Arica	38,821	99	37,423	99	34,770		
Iquique	35,126	98	33,332	98	30,175	98	
P. Almonte	1,061	95	920	95	786	94	
Pica	967	100	943	100	865	100	
Matilla	270	100	261	100	248	100	
La Tirana	1,042	85	1,000	85	928	78	
La Huayca	132	100	122	100	110	100	
Huara	172	100	159	100	148	100	
Pisagua	73	100	57	100	53	100	
Total	77,664	98	74,217	98	68,083 98		

Source: ESSAT 1993 Annual Memories

Clients and Sewerage Service Coverage

	19	93	19	92	1991			
Service	Clients (number)	Coverage (%)	Clients (number)	Coverage (%)	Clients (number)	Coverage (%)		
Arica	38,238	98	36,924	98	34,222	97		
Iquique	34,502	96	32,815	96	28,961	94		
P. Almonte	709	95	587	61	484	58		
Pica	396	41	388	41	244	28		
Total	73,845	96	70,714	96	63,911	95		

Source: ESSAT 1993 Annual Memories

Clients can be classified as follows:

Classification by Type of Client

Year			1993			1992			
Type of Client	Water	%	Sewerage	%	Water	%	Sewerage	%	
Residential	71,462	92	68,516	93	68,123		65,426	93	
Commercial	3,262	. 4	2,725	4	3,442	5	2,879	4	
Industrial	2,098	3	1,916	2	1,849	2	1,730	2	
Other	842	1	688	- 1	803	1	676	1	
Total	77,664	100	73,845	100	0 74,217		70,714	100	

Source: ESSAT 1993, 1992 Annual Memories

In order to offer better services to its clients, ESSAT has 24 offices where they can make payments and 5 centers for general consultation.

#### 5) Main Installations for Water and Sewerage Services

Main installations of ESSAT correspond to the infrastructure for production and distribution of drinking water and collection system and final disposal of sewage.

#### Main Installations of ESSAT

Water intake	Arica: 29 deep wells					
Water intake	Iquique: 26 deep wells					
Water transmission pipeline	Arica: 19,175 m					
Water transmission pipeline	Iquique: 177,348 m					
Water pumping station in Arica:	4 with a capacity of 277 1/s					
Water pumping station in Iquique:	5 with a capacity of 706 1/s					
Water reservoirs in Arica:	8 with a storing capacity of 14,500 m <sup>3</sup>					
Water reservoirs in Iquique:	38 with a storing capacity of 78,700m <sup>3</sup>					
Water distribution network in Arica:	372,637 m					
Water distribution networks in Iquique:	246,942 m					
Sewerage collection networks in Arica:	350,629 m					
Sewerage collection networks in Iquique:	227,678 m					
Sewerage pumping stations in Arica:	2 with capacity of 600 1/s					
Sewerage pumping stations in Iquique:	3 with capacity of 1,300 l/s					

Source: ESSAT 1993 Annual Memories

Table D-I, 1.1 List of Deep Wells - Arica/ESSAT (No. 1)

		_			_									5 5 5 5			4.15		
Remarks									Rented				Rented	Rented	Stopped				
Depth of	Pump	Installed	(m)	72		99				99	72	99				8/	8	ቖ	
Water Level	Drawdown		(m)	17.56		23.23			2	32.31	18.5	12.89				28.07	24.7	19.8	
Dynamic	Water	Levei	(m)	50.25		49.93		40	94	64.3	49.5	43.15	55.25			65.5	57.75	54.05	
Static	Water	Level	(m)	32.7		26.7		I/S	S	31.93	£	30.26			22.94	39.43	33.05	34.25	
Actual	Yield		(1 / sec)	18.5		22.7		88	83	12.9	6.9	9	12.9	8.5	12	10	8.5	12.5	
Total	Screen	Length	(m)	# 16	- 23	# 15	* 35			# 15 * 30	# 15 * 10	* 15 • 30				505.	\$	88	
Well	Depth		(E)	8		8		20	84	100	8	100				8	8	86	
Casing	Diameter		(mm)	302		305		254	2 <u>5</u> 2	254-305	254-305	305	- 10 m			350-400	320	350-400	
Year of	Constuction			1993		1992		1947	1951	1992	1992	1975/84	1893	1993	1983	1987	1987	1987	
Altitude	above sea	levei	(+ m)	248		249		248	248	149.8	150.85	153.05				101	108	111	
Location		-		San Miguel Plant		San Miguel Plant		San Miguel Plant	San Miguel Plant	Pago de Gomez Plant	Pago de Gomez Plant	Pago de Gomez Plant	4.5km Azapa Road	4.5km Azapa Road	4.5km Azapa Road	Azapa Plant	Azapa Plant	Azapa Plant	
Well No.				1471	Sn. Miguel	1472	Sn. Miguel	92 Agricultores	93 Agricultores	1113 (No. 1)	1114 (No. 2)	1142 (No. 3)	Centella	Ordonez	Femandez	47	48	184	

\* slotted pipe

Table D-1, 1.1 List of Deep Wells - Arica/ESSAT (No. 2)

ļ	Location	Altitude	Year of	Casing	Well	Total	Actual	Static	Dynamic	Water Level	Depth of	Remarks
		above sea	Constuction	Diameter	Dept	Screen	Yield	Water	Water	Drawdown	Pump	
		jevej				Length		Level	Level		Installed	
		α		(mm)	(m)	(m)	(I / Sec)	(m)	(m)	(m)	(w)	-
Azapa Plant		102	1987	350/400	86	* 41	13.5	33.56	57.71	24.15	<b>&amp;</b>	
Outside near		106.5	1962	254/340	8		1.8 1.8	35.3	63.6	28.3	72	
Azapa Plant	ĭ											
Azapa Plant	ŧ	i/S		350/400	8	* 46	11.3	36.77	57.03	20.26	99	
Azapa Plant	ant	110.5	1987	254/400	8	• 40	11.6	36.21	57.8	21.59	2/	
	City	88	1989	355	<b>98</b> .	÷ 36	14.92	33.03	52.2	19.17	72	
	City	ន	1962	355	96.15		21.6	40.1	60.45	20.35	99	
	City C	27	1989	325	85.15	* 40	17.5	28.1	65.1	37	09	
	City	88	1963	350	8	*65,12	23.8	31.75	44.9	13.15	99	
	Ç.	46	1965	340	56		18.8	34.41	50.81	16.4	78	
	City	16.5	1963	300	001		14.3	24.25	43.5	19.25	8	
	City	3	1967	004	001		21.8	25.88	50.12	24.24	99	
	Çî Cîş	68	1965	320	100		30.3	24.33	54.7	26.08	78	
	Cit <sub>2</sub>	I/S	1989	350	8	40.69	15	36.4	47.75	11.35	89	
١												

Table D-I, 1.1 List of Deep Wells - Arica/ESSAT (No. 3)

			٦		T	1			İ	,			Γ		<u> </u>			·
Remarks					Rented		Remad	Rented		Rented	Rented	Rented	Rented	Rented				
Depth of	Pump	Installed	(£)															
Water Level	Drawdown		(E)													8.36		
Dynamic	Water	Level	(E)		38.35		37.96	29.12		22.8	44.17		49.25	57.2		49.7	52.6	62.5
Static	Water	Levei	(സ)	<b>4</b> 1		:						1. 1. 2.		: :		41.34		
Actual	Yield		(1 / sec)	60	23.8		47	4		ଷ	6.1	24	22.2	14.2	<b>9</b> 2	12	မ	7
Total	Screen	Length	(m)					98		-		-			æ		8	115
Well	Depth	·	(m)	08				8							97		88	115
Casing	Diameter		(mm)	320			<del> </del>									8		
Year of	Constuction			1987	1993		1993	1983		1983	1983	1993	1983	1983	1991	1891	1983	1993
Altitude	above sea	level	(+ m)	l/S														
Location				ÁĮ.O	Cabuza Setor	-	Cabuza Setor 2	Km 15	Azapa Road									
Well No.				Rodoviario	108		115	Lido Carbone		Las Riveras	Ordonez 2	Ortuno	Devoto 2	Devoto 1	Tambo Quemado	Nueva Esperanza	J. Aracena	Pon

Table D-1, 1.1 List of Deep Wells - Arica/ESSAT (No. 4)

_	_		_							 	 	_	 
Remarks						Rented	Rented						
Depth of	Pump	Installed	<b>(E)</b>	:	:	,							
Water Level	Drawdown		(m)		- 2. 1								
Dynamic	Water	Level	(m)	75.85	70.3	52.25	63.75						
Static	Water	Level	(m)		: :								
Activa	Yield		(l / sec)	15	24	10.1	7.6						:
Total	Screen	Length	(m)	30	<b>\$</b>			36					
Weil	Depth		E	109	143			120					
Casino			(mim)						-				
Year of	Constuction				1983	1993	1993	1983					
Attitude	above sea	level	Ę.										
Location													
A Albert				O' Higgins	Angelmo	Sobraya 1	Sobraya 2	Las Torres					

Table D-I, 1.2 List of Imake Pump Arica/ESSAT (No. 1)

Technical Specification of Pump Name of Manufacturer Remarks	Discharge Total Head Ele. Power and Model Number	(m) (x(m))	**************************************		07 92 (				***************************************		70 40 Rented	22 Rented		40 Rented		52 18			8	}	70. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1.	60 03
Technical	Diameter Discr	(mm) (l/sec)	17.8		8	-		2			23					83			8		8	*
Year of	instal-	lation	1983		1933						1983	1993	•."	1983	1992	1992		1987	5003	<u> </u>	<u>8</u>	1002
Type of	Pump		Submersible	motor pump	•				Submersible	motor pump	*	•		•	* *							
Flow direction			Sn. Miguel-Pago de	Gomez Conveyance	Sn. Miguel-Pago de	Gomez Conveyance	Sn. Miguel-Pago de	Gomez Conveyance	Sn. Miguel-Pago de	Gomez Conveyance	Pago de Gomez Tank	Pago de Gomez Tank		Pago de Gomez Tank	Pago de Gomez Tank	Pago de Gomez-	Azapa Conveyance	Pago de Gomez-	Azana Tank	VI TO A TOP TOP TO A	Azapa Tank	Azona Tank
Well No.			1471	Sn. Miguel	1472	Sn. Miguel	92 Agricultores		93 Agricultores		1113 (No. 1)	1114 (No. 2)		1142 (No. 3)	Centella	Ordonez 1		Femendez	47		84	187

Table D-I, 1.2 List of Intake Pump Arica/ESSAT (No. 2)

			[.							٢
Well No.	PIOW GIRBCTION	5 ed.	rear or	De;	technical specification of Pump	Cation of P		Name of Manufacturer	Kemarks	
		Pump	Instal-	Diameter	Discharge	Total Head	Ele. Power	Total Head Ele. Power and Model Number		-
			ation	(mm)	(1/800)	Ξ	(KWh)			-
434	Azapa Tank		1988		8	8	೫			
491	Azapa Tank	Submersible	1993		8	22	22			
		motor pump	:							
491 A	Azapa Tank	*	1992		\$	02	37			
492	Azapa Tank		0661		52	88	æ			T
568 Los Pinos	Estadio Station		1983		82	70	81			T
569 Pilon 18			1990		88	89	8			T
650 Saucache			1990		æ	8	45.5			
714 R, Estadio		* .	0861		હ	8	8			T
715 Copaja			1993		z	8	e e			
San Jose	•									I
Liga Empleados		Submersible motor pump	1993		ĸ	8	e R			
4897 Tucapel	*		1992		જ	8	45.5			
Planta Estadio			1993		22	20	22			

Table D-I, 1.2 List of Intake Pump Arica/ESSAT (No. 3)

14/oil his	Characters	Times of	Vans of	1001	Technical Coorification of Piemo	G to cotton	, man	Name of Manufacturer	Į.		Remarks	
	CORORION MOL	5 R.	5	B	שמה השנו		1	None of the second				
		Pump	Instal-	Diameter	Discharge	Total Head	Ele. Power	Discharge Total Head Ele. Power and Model Number				
			iation	(mm)	(J/Sec)	(m)	(KWh)					
Rodoviario	La Red										•	
8	Pago de Gomez				ନ	2	୫ .		٠			
Cabuza 1												
115	Pago de Gomez	Submersible			₹	8					Rented	
Cabuza 2		motor pump								: :		
Lido Carbone		,			9	09	37				Rented	
Las Riveras	•	¥			24	35	13				Rented	
Ordonez 2		•							. 42 . 4		Remised	
									1			
Ortuno	**************************************	*	= · · · · · · · · · · · · · · · · · · ·								Remised	
Devoto 2		•			8	86	24				Rented	
Devoto 1		*			520	105	æ				Rented	
	Saucache Tank	•			ĸ	116	23					
Quemado					: :		:					
Nueva			. :		8	92	37					
ESperanza												
J. Aracene	Chuno Tank				<b>6.</b> 5	52	路					
									·			
<b>B</b> Ø7		•		\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	6.5	38	<b>&amp;</b>					

Rented Name of Manufacturer Technical Specification of Pump Name or manner Diameter Discharge Total Head Ele. Power and Model Number (MWh) Table D-I, 1.2 List of Intake Pump Arica/ESSAT (No. 4) R ន 8 ∞ £ 5 8 6 70 X Z 8 8 Year of 1993 Instal-Submersible motor pump Type of Pump Flow direction Estadio Station La Red La Red Well No. Sobraya 2 Sobraya 1 Angelmo

Table D-I, 1.3 List of Collection Tanks - Arica/ESSAT

of Tanks Construction Meterial Level Depth  (+ m)  (+ m)  (+ m)  (+ m)  (A m.)   Name of Tank	Location	Direction of Flow	Ground	Number	Year of	Construction	Water	Dimension & Water	Capacity	Total	
Sea level   Sea				Attitude	of Tanks	Construction	Material	Level	Depth	3 2 2 4	Capacity
Sea level   Sea		·		apove							: .
se Gornez         Azapa         From deep wells in city area         (+ m)         (m3)           Selection deep wells in city area         Azapa         From deep wells in city area         + 150         Concrete         + 154.63         Diem. 17.03m         m3           Chuno Tank, Saucache         + 150         Chuno Tank, Saucache         + 150         + 150.23         1,000           Azapa         Same as the above         + 100         1         Reinforced         HWL=         Diam. 11.98m         5.7m           Valley         Azapa         Same as the above         + 100         Concrete         LWL=         H         + 150.23         5.7m           Valley         From deep wells         + 100         Concrete         HWL=         Diam. 11.98m         5.00           City area         From deep wells         + 433.1         1         1984         Rein forced         H= 2.5m         100           Conzerte         From deep wells         + 433.1         Concrete         H= 2.56m         100				sea level							
Azapa   From deep wells in   150   1   Reintoced   HWL=   Diem 17.03m   m3			-	t E				(E +		(£m3)	(£m)
Valley         Azapa Valley to Tank, Saucache         +150         Concrete         +154.63         1,000           Chuno Tank, Saucache         Chuno Tank, Saucache         +150         +150.23         +150         +150.23         +150           Azapa         Same as the above         +100         1         Reinforced         HHL=         557m         557m           Valley         +100         1         Reinforced         HHL=         560         560           Concrete         LWL=         H         4.25m         500         60 <th>Pago de Gomez</th> <th>· · ·</th> <th>From deep wells in</th> <th></th> <th>_</th> <th></th> <th></th> <th>HW.L.</th> <th>Diam. 17.03m</th> <th>т3</th> <th>m3</th>	Pago de Gomez	· · ·	From deep wells in		_			HW.L.	Diam. 17.03m	т3	m3
Chuno Tank, Saucache		Valley	Azapa Valley to	+ 150				+ 154.63		98,	00.1
Tank and Pampa			Chuno Tank, Saucache					LWL=		1:	
Nueva Tank			Tank and Pampa					_+			
Azapa         Same as the above         1         Reinforced         HWL=         Diam. 11.98m         500           Valley         + 100         Concrete         LWL=         BL=         H         4.25m           0         City area         From deep wells         1         1984         Rein forced         L= 11.10m         100           0         Cruz Tank         A+43.1         Concrete         W= 4.35m         H= 2.65m			Nueva Tank		:			BL.=	<b>T</b>		
Azapa         Same as the above         1         Reinforced         HWL=         Diam. 11.98m         500           Valley         + 100         Concrete         LWL=         LWL=         H         4.25m           City area         From deep wells         1         1984         Rein forced         L= 11.10m         100           Cruz Tank         Cruz Tank         H= 2.66m         H= 2.66m								+ 150.23	5.7m		
Azapa         Same as the above         1         Reinforced         HWL=         Diam. 11.98m         500           Valley         +100         Concrete         LWL=         H         500           City area         From deep wells         +106.66         4.25m         +100.66           City area         From deep wells         1         1984         Rein forced         L= 11.10m         100           Cruz Tank         Cruz Tank         W= 4.35m         W= 4.35m         H= 2.65m											:
Azapa         Same as the above         1         Reinforced         HWL=         Diam. 11.98m         500           Valley         LWL=         LWL=         LWL=         BL=         H           City area         From deep wells         1         1984         Rein forced         L= 11.10m           in city area to La         +43.1         Concrete         W= 4.35m         100           Cruz Tank         H= 2.66m         H= 2.66m	····							· .			-
Valley         +100         Concrete         LWL=         500           City area         From deep wells         1 1984         Rein forced         L= 11.10m         100           Cruz Tank         +43.1         Concrete         W= 4.35m         100	Azapa	Azape	Same as the above		-		Ī	HWL=	Diam. 11.98m	- 1	
City area From deep wells 1 1984 Rein forced L= 11.10m 100 100 In city area to La +43.1 Concrete W= 4.35m H= 2.65m		Valley		4 18			Concrete			88	200
City area       From deep wells       1       1984       Rein forced       4.25m         City area       From deep wells       1       1984       Rein forced       L= 11.10m         In city area to La       +43.1       Concrete       W= 4.35m         Cruz Tank       W= 2.55m						:		LWL=			
City area       From deep wells       1 1984       Rein forced       4.25m         City area       From deep wells       1 1984       Rein forced       L= 11.10m         In city area to La       +43.1       Concrete       W= 4.35m         Cruz Tank       W= 2.65m										:	
City area From deep wells 1 1984 Rein forced L= 11.10m in city area to La +43.1 Concrete W= 4.35m W= 2.65m								BL.	I		
City area From deep wells 1 1984 Rein forced L= 11.10m in city area to La +43.1 Concrete W= 4.35m Cruz Tank H= 2.65m								+ 106.66	4.25m		
City area From deep wells 1 1984 Rein forced L= 11.10m in city area to La +43.1 Concrete W= 4.35m W= 2.65m											
+43.1 Concrete W= 4.35m H= 2.65m	Estadio	City area	From deep wells		_	Г	Rein forced		L= 11.10m		
			in city area to La	+43.1		<del>- E</del> .	Concrete			8	
H= 2.66m			Cruz Tank						W= 4.35m		
H= 2.66m											-
									H= 2.65m		
						: .					
		-				٠.					

Table D-I, 1.4 List of Distribution Tanks - Arica/ESSAT

Location		Flow Direction	Ground	Number	Year of	Construction	Water	Dimension	Capacity	Total	Remarks
		Akii	Altitude	of Tanks	Construction	Material	Level	& Water Depth		Capacity	a a
эроле	apor	ág	٩						-	:	
sea level	sealev	sea lev	क								<i>2</i> .
(+ w)	<b>(+w</b>	(+ m	_				(+ m)	(m)	(m3)	(m3)	
Eastern hill From Pago de Gomez + 81		+81		1	1985	Reinforced	HWL=84.50	D: 32.57	2,000	2,000	
of the city and Azapa to	and Azapa to					Concrete	LWL=+				
service Sector II	service Sector II						BL=79.00	H: 6.4		:	÷
Southwester From Pago de Gomez + 99.1		+ 99	-	Į.		Reinforced	HWL:104.6	D: 22.03	2,500	2,500	
hill and Azapa to Sector	and Azapa to Sector					Concrete	LWL:				
=							BL:99.10	H: 6.4			
			-								
Vicinity of From Pago de Gomez + 124.5		+ 124.	5	1	1994	Reinforced	HWL=+129.1 D:	:a	1,000	1,000	Commissioned
Saucache and Azapa to highland	and Azapa to highland					Concrete	BL=+124.5				in Feb. 1994
Tank of Sector III	of Sector III			<del>, ·     ,</del>				Ï			
Southern From Estadio to + 76		+ 76		2		Steel	HWL:81.5	D: 23.3	2,500	5,000	
hill Sector I	Sector				•		LWL:		X X		
			,				BL:76.00	H: 6.0			٠.
South of La From La Cruz to + 105		+ 105		-		Steel	HWL:109.50	D: 15.1	800	800	
Cruz Tank Sector IV	Sector IV				¥.		LWL:				
							BL:105.00	H: 4.9			
South hill From La Cruz to + 2.55		105	$\top$	-		Reinforced	HWL	D: 7.85	200	200	
Jo costs   stateon		}	_			Concrete			) }	<u> </u>	
TIEST COSTS TO COSTS OF THE STATE OF THE STA	ייטשים מומק מון					e de co	LW.L BL:25	H: 4, 15			
	:										
			1					Total Capacity	acity	14,500	

Table D-I, 1.5 List of Transmission Pipelines - Arica/ESSAT (No. 1)

						,																			
Remarks											Valve Normal Position:	Ciose			Pago de Gomez - Azapa	Plant section		Azapa Plant - Callejon	Motel Section			Callejon Motel - Diego	Portales Section	Parallel pipes	
Other Fittings				10 Air-release valve	2 Drain					D AU-remette valve	1 Valve in front of	Pago de Gomez		4 Air-refease valve	1 Fire hydrant	2 valves	1 Flow meter out of order	1 Air-reiease valve	out of order			2 Valves for connection	pipes		
Pipe Material				Asbest of Cement							Asbest of Cement				Asbest of Cement				Asbest of Cement	Ż		Asbest of Cement		Asbest of Cement	
Distance			€	7,258							6,025				3,214	•			2,350		:	630		00	
Diameter			(mm)	300						•	\$			-	450				38		:	952	·	320	
Year of	Construction			1983																					
Design Flow			(l/sec)	00Z	•				8	788				226				226				226			
Gravity	Flow or	Pumped		Gravity		:		:	,	Gravity				Gravity				Gravity				Gravity			
Site Name of	Pipeline			From	Cabuza	2	Sen Miguel			From	San Miguel +246.60	To +154.23	Pago de Gomez	From	Pago de Gomez	2	Chuno Tank					l			

Table D-I, 1.5 List of Transmission Pipelines - Arica/ESSAT (No. 2)

Site Name of	Gravity	Design Flow	Year of	Diameter	Distance	Pipe Material	Other Fittings	Remarks
Pipeline	Flow or		Construction					
	Pumped		•					
		(1/sec)		(mm)	(m)			
From	Gravity	226		400	1,310	PVC		Rotonda Diego Portales-
Pago de Gomez	•		•	400	260	Steel		Estanque Chuno Section
To				\$	574	Asbest Cement		
Chuno Tank								
From						-	•	
Pago de Gomez	Gravity	526			90.6		-	Total
To Chino Tank			:					
From								+150.43 +103.08
Pago de Gomez	Gravity	178.5		9	3,207	Asbest Cement	3 Air-release valve	Pago de Gomez - Azapa
To						-		Plant section
Saucache Tank								
				250	2,002	Asbest Cement		Azapa Plant - Callejon
	•	178.5						Motel Section
				S2 220	2,002	Asbest Cement		Parallel pipes
		178.5		320	1,050	Asbest Cement	2 Scour valves	Callejon Motel - Saucache
	-		15 11 111					Tank Section
From		178.5			8,261			Total
Pago de Gomez								
To .		•						
Saucache Tank								

Table D-I, 1.5 List of Transmission Pipelines - Arica/ESSAT (No. 3)

					ALT/O				IZ							
Remarks		•		Connection with Page de	Gomez - Chuno Tank Pipeline	in Diego Portales Sector			From Azapa Plant - La Cruz	Tank, Pipeline in Diego	Portales Sector					
<del> </del>				8	ලි	<u>,Ę</u>			F	<u>r</u>	ď	-	-	:	<u>.</u>	+
Other Fittings				3 Air-refease valve	1 Scour valve					1 Drain						
	:			3 Air-r	1 Sœu			1.		: .						
Pipe Material					Steel					PVC (?)						
90							-		-			_			-	
Distar			<b>E</b>												• .	
Diameter Distance			(mm)		450		:.			8						
Year of	Construction				-											
Design Flow			()es/J)		980											
Gravity	Fiow or	Pumped	-													
Site Name of	Pipeline			From	Azapa Plant	70	La CIUZ Latin		From	Azapa Plant	To Chuno Tank					

Table D-1, 1.6 List of Transmission Pumps - Arica/ESSAT

Name of	Pump	Type of	Year of	Tech	nnical Specif	Technical Specification of Pump	dwi	Name of Manufacturer	Remarks	
Pumping	o N	Pump	Installation	Diameter	Discharge	Diameter Discharge Total Head		and Model Number		
Station				(mm)	(1 / sec)	(m)	(KWh)			
Estadio	No. 1	No. 1 Double Suction	1984		303	6.02	110	110 Pump: KSB / RETA 200-400	Equipments in Parallel	
(From city		Volute Pump						Motor: IP-54 315M	one Stand-by	
wells to	No. 2	No. 2 Double Suction	1984		303	50.3	110	Pump: KSB / RETA 200-400		
La Cruz Tanks)		Volute Pump						Motor: IP-54 315M		
	No. 3	No. 3 Double Suction	1984		303	50.3	110	Pump: KSB/RETA 200-400		
		Volute Pump	:					Motor: IP-54 315M	:	

Table D-I, 1.7 List of Booster Pumps - Arica/ESSAT

Name of	Pump	Type of	Year of	Ter	Technical Specification of Pump	cation of Pu	du	Name of Manufacturer	Remarks
Pumping	Š.	Pump	Installation	Diameter	Discharge	Total Head		and Model Number	
Station				(mm)	(1 / sec)	(E)	(KWh)		
Rosado Booster	No. 1	Single Suction	1984		08	83	51	Pump: ROTOS POMP NE6X18-M	The Station has 3
Pumps		Volute Pump		<del></del>		-		Motor: ERCOLLI MARTELLI	Hydropak Tanks
(To make sure	No. 2	Single Suction	1984		09	83	15	Pump: ROTOS POMPE NE6X16-M with 200 I capacity each	with 200 I capacity each
minimas		Volute Pump		<del>i</del>	<del></del>			Motor: ERCOLLI MARTELLI	
Pressures in	No. 3	Single Suction	1984		8	83	15	Pump: ROTOS POMPE NE6X16-M	Equipment in parallel
the Sector)	<del></del>	Volute Pump						Motor: ERCOLLI MARTELLI	
÷	No. 4	Single Suction	1984		98	52	15	Pump: ROTOS POMPE NE6X16-M	
		Volute Pump					1	Motor: ERCOLLI MARTELLI	
La Lisera	₹ 0.1	Submersible			12			Pump : PLEUGER	
(From Distribution									
system to	No. 2								Out of Service
La Lisera Tank)									
Cerro La Cruz	No. 1	Single Suction	1984		100-150-200	63-58-45	37	Pump: Rotos Pompe NEBX25M	Equipments in perallel
(From La Cruz		Volute Pump	(Remodeled)			: :		Motor : ERCOLL! MARTELL!	one Stand-by
Tank to Rosado	No. 2	Single Suction	1984		100-150-200	63-58-45	37	Pump: ROTOS POMP NEBX25M	
Tank		Volute Pump (Remodeled)	(Remodeled)					Motor: ERCOLLI MARTELLI	

Table D-I, 1.8 Hydraulic Calculations for Existing Transmission Pipelines, Arica

Point to Point	Dia. (mm)	Distance (m)	Material	С	Flow (l/sec)	I	H=IL	V (m/sec)
(+??)	800			166	400	0.000=		
Cabuza - San Miguel?? ()	300 Sondaje	7,258 Riego??	ACP	130	200	0.0235	170.36	2.83
(+246.6m)	:			•				
San Miguel - Pago de Gomez	400 (+154.2	6,025 (m)	ACP	130	392	0.0201	120.98	3.11
(+246.6m)-(+154	.2m) = 9	2.4m						:
	400	6,025	ACP	130	350	0.0163	98.10	
	400	6,025	ACP	130	335	0.0150	90.46	> OI
	400	6,025	ACP	130	320	0.0138	83.11	· · · · · · · · · · · · · · · · · · ·
(+150.4m) Pago de Gomez -Azapa Plant (+1	450 02.9m)	3,214	ACP	130	226	0.00409	13.14	1.42
(+102.9m) Azapa Plant - Callejon Motel	350 (+65.3m)	2,350	ACP	130	226	0.0139	32.64	2.35
(+65.3m)								
Callejon Motel	250	430	ACP	130	66	0.00733	3.15	1.34
	350	430	ACP	130	160	0.00733	3.15	1.66
- Diego Portales	399	430	ACP	130	226	0.00733	3.15	
	(+36.1111	<i>.</i>						· · · · · · · · · · · · · · · · · · ·
(+58.1m) Diego Portales	400	1,310	PVC	130	226	0.00725	9.50	1.79
Diego I ortales	400	760	Steel	90	226	0.0143	10.88	1.79
· · · · · · · · · · · · · · · · · · ·	400	574	ACP	130	226	0.00725	4.16	1.79
		2,644					24.54	
- Chuno Tank (+	85.1m)	· · ·		·		-		•
Total Head Loss (Pago de Gomez			the state of the s		73.47m 65.3m			
(+150.4m) Pago de Gomez - Azapa Plant (+1	400 103.1m)	3,207	ACP	130	178.5	0.00469	15.02	
(+103.1m)								
Azapa Plant	250	2,002	ACP	130		0.0128	25.63	1.82
_	250	2,002	ACP	130		0.0128	25.63	1.82
- Callejon Motel	325	2,002		130	178.5	0.0128	25.63	
Callejon Motel - Saucache Tank	350 (+105.51	1,050 m)	ACP	130	178.5	0.00898	9.43	1.86
Total Head Loss {Pago de Gomez					50.08m			<del></del>

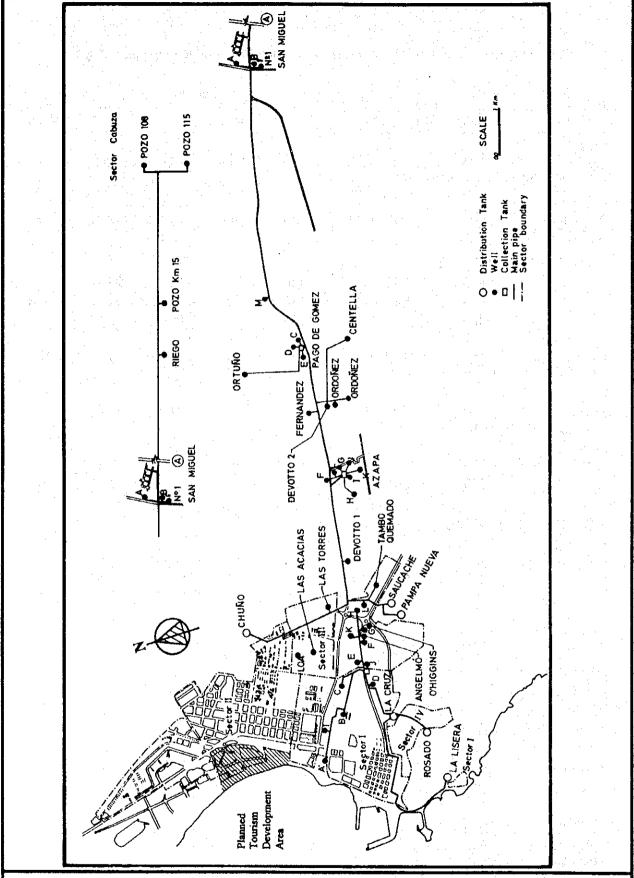


Fig. D-I, 1.0 General Plan of Existing Water Supply System - Arica 
< Planta General del Sistema de Abastecimiento de Agua Existente - Arica > 
THE STUDY ON THE DEVELOPMENT OF WATER RESOURCES IN NORTHERN CHILE 

JICA

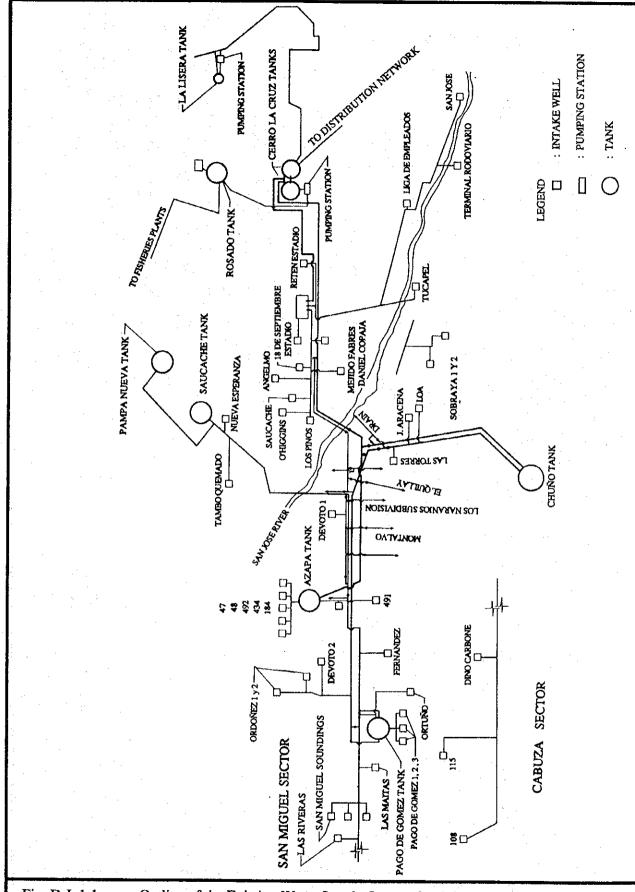
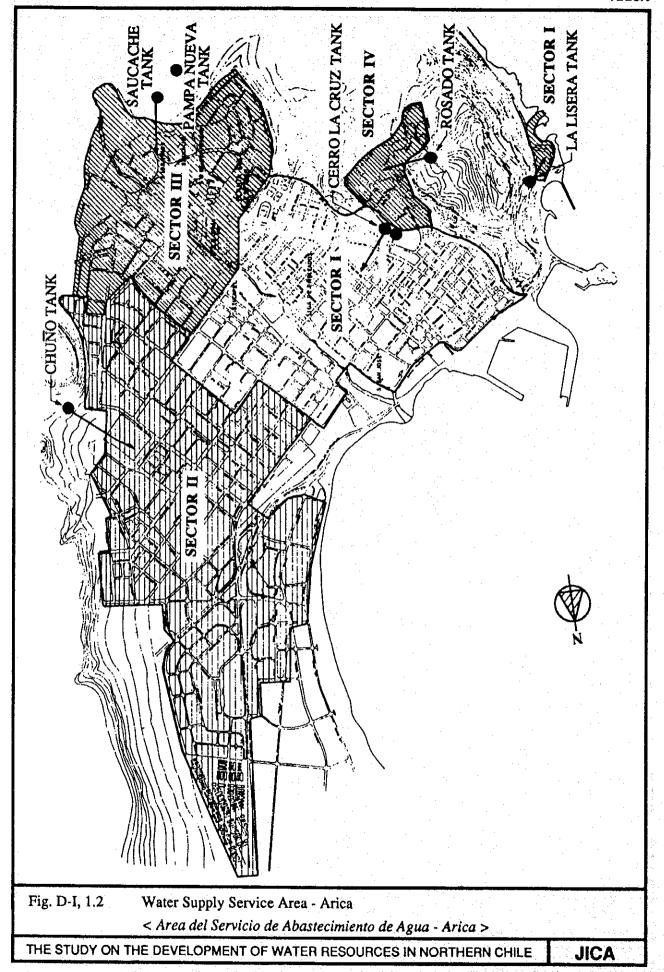


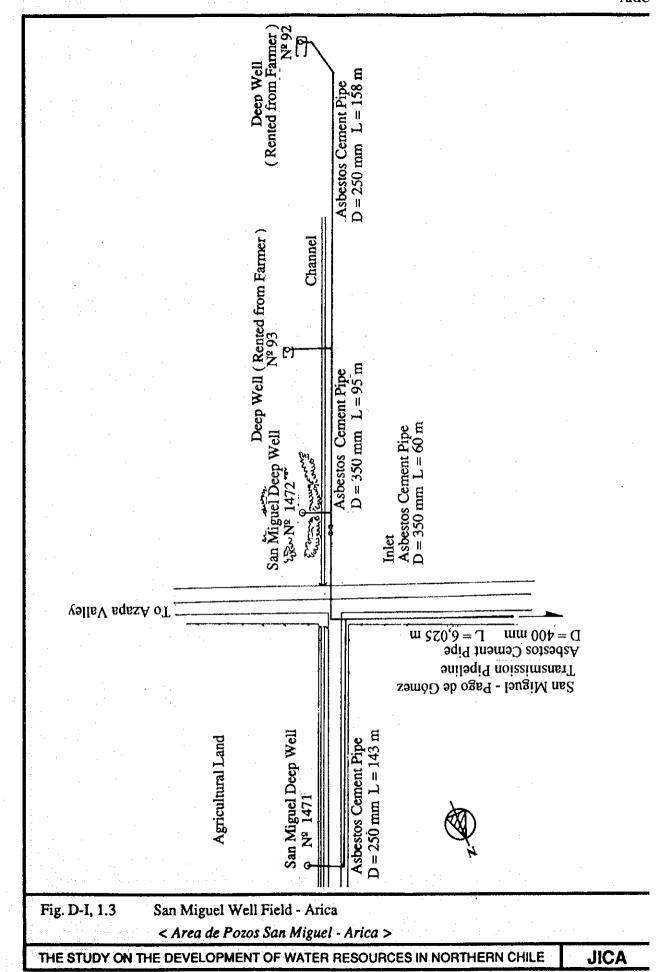
Fig. D-I, 1.1 Outline of the Existing Water Supply System for Arica < Bosquejo del Sistema Existente del Abastecimiento de Agua para Arica >

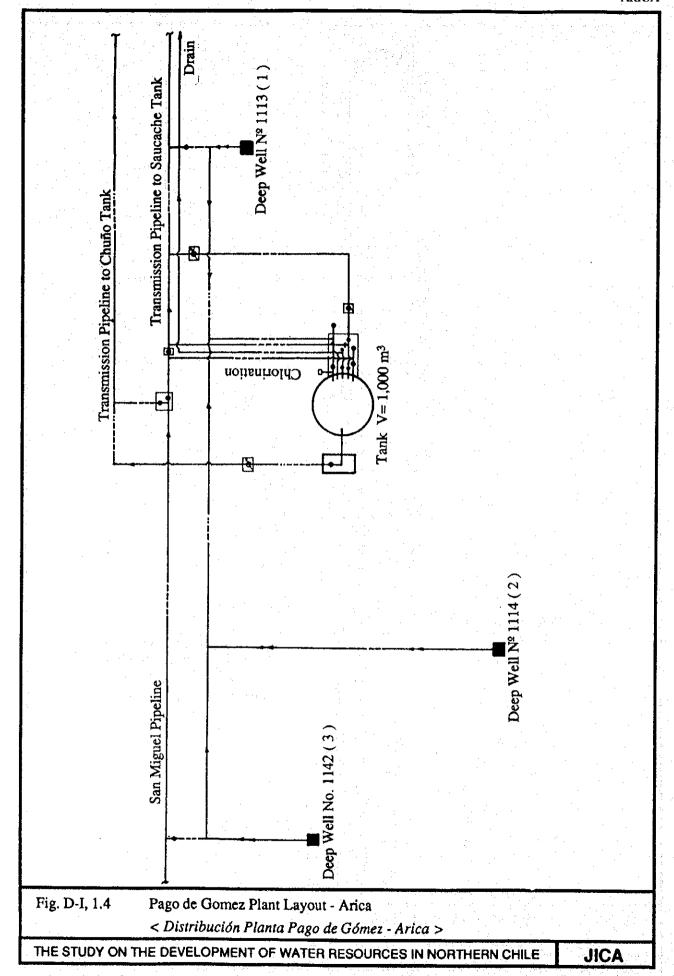
THE STUDY ON THE DEVELOPMENT OF WATER RESOURCES IN NORTHERN CHILE

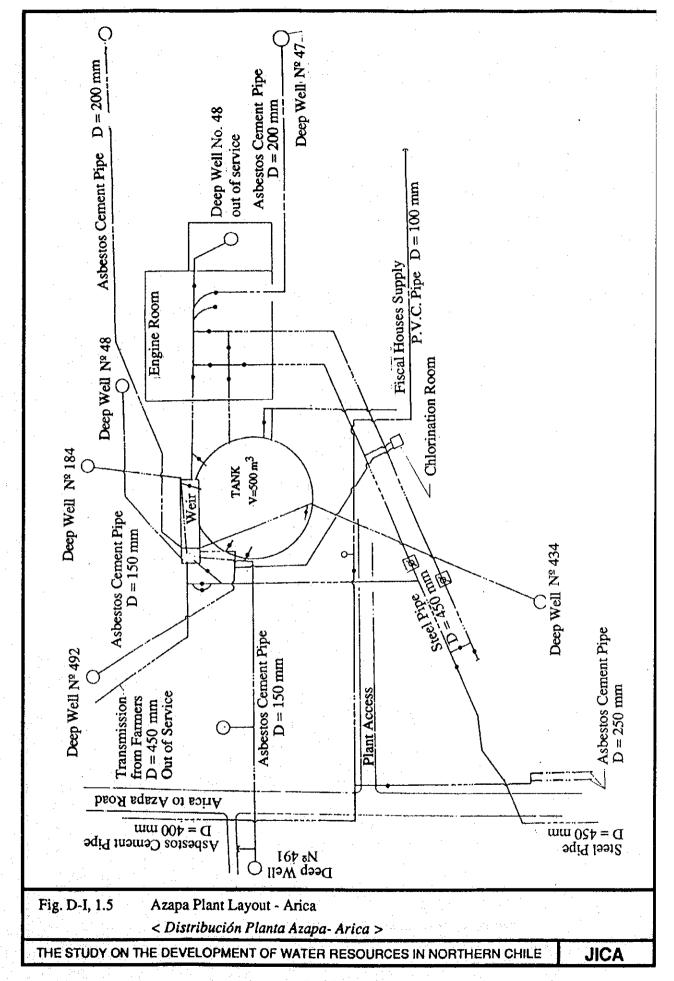
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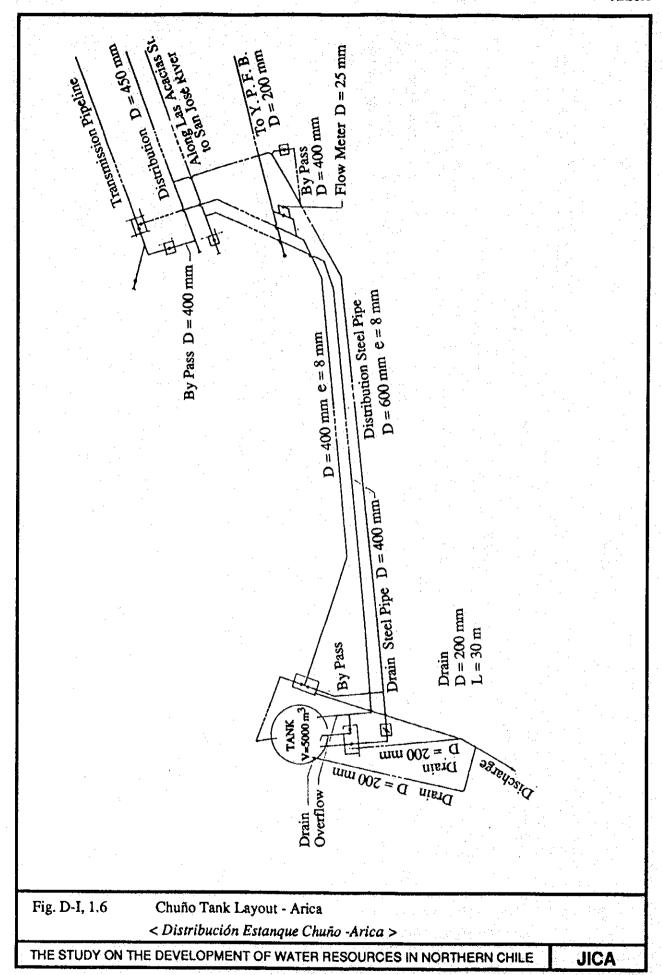


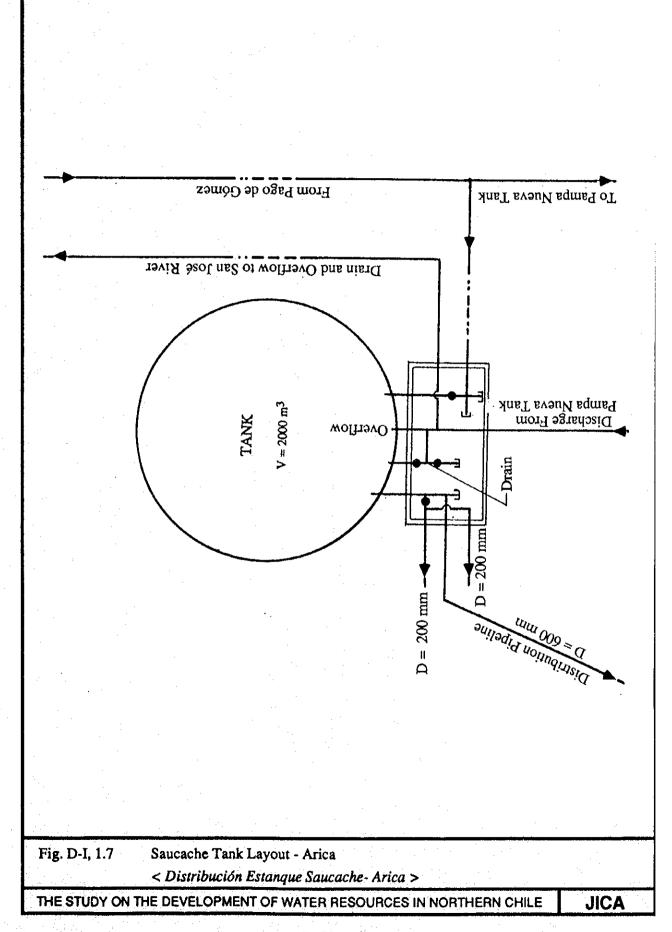
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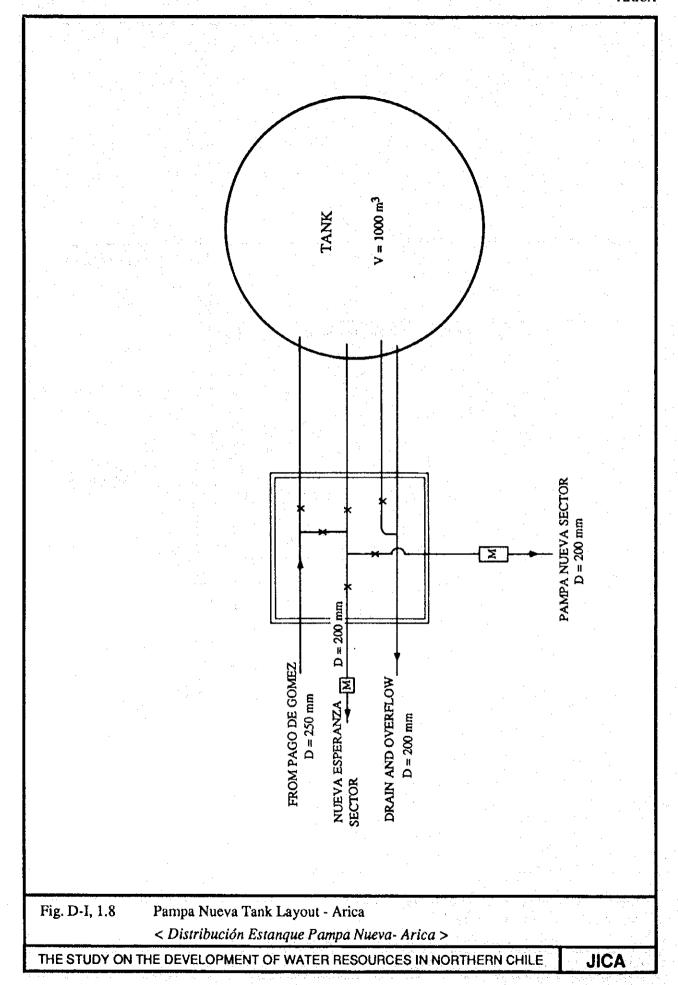




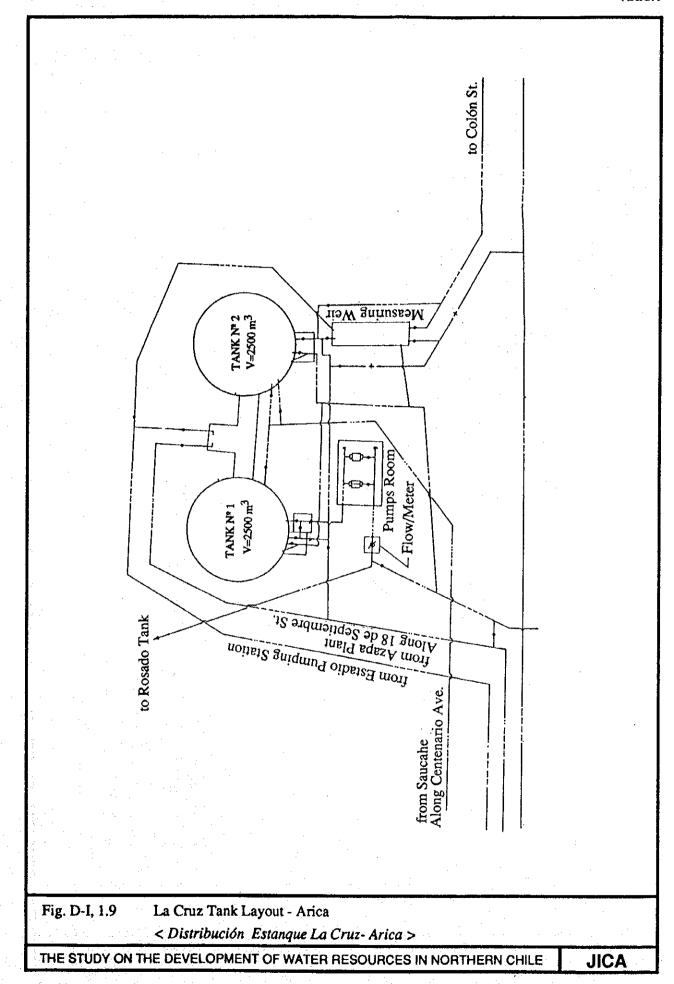


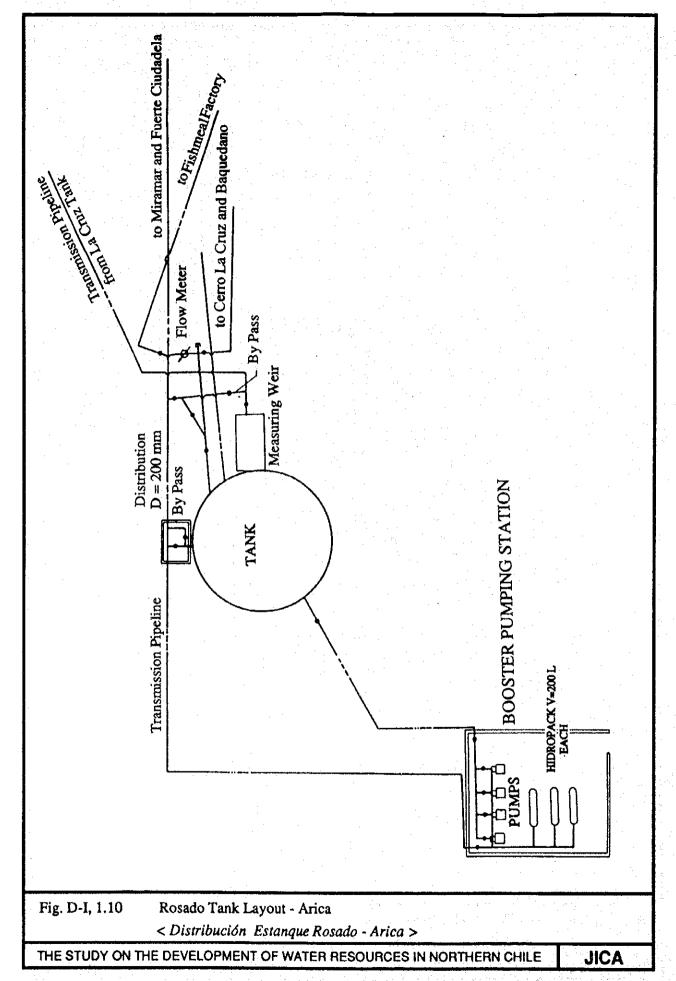


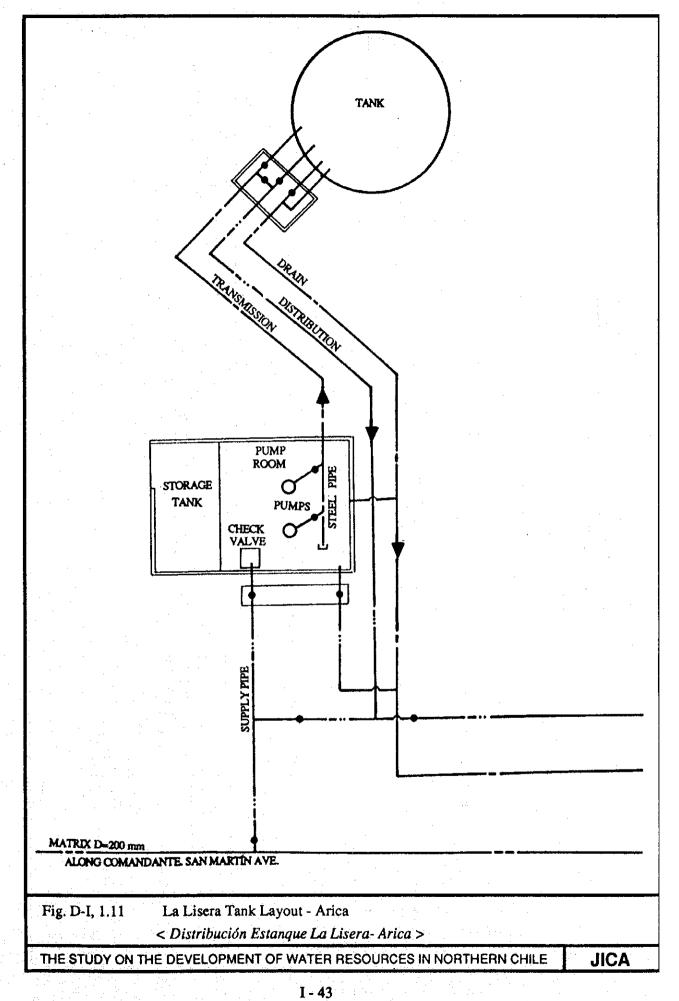


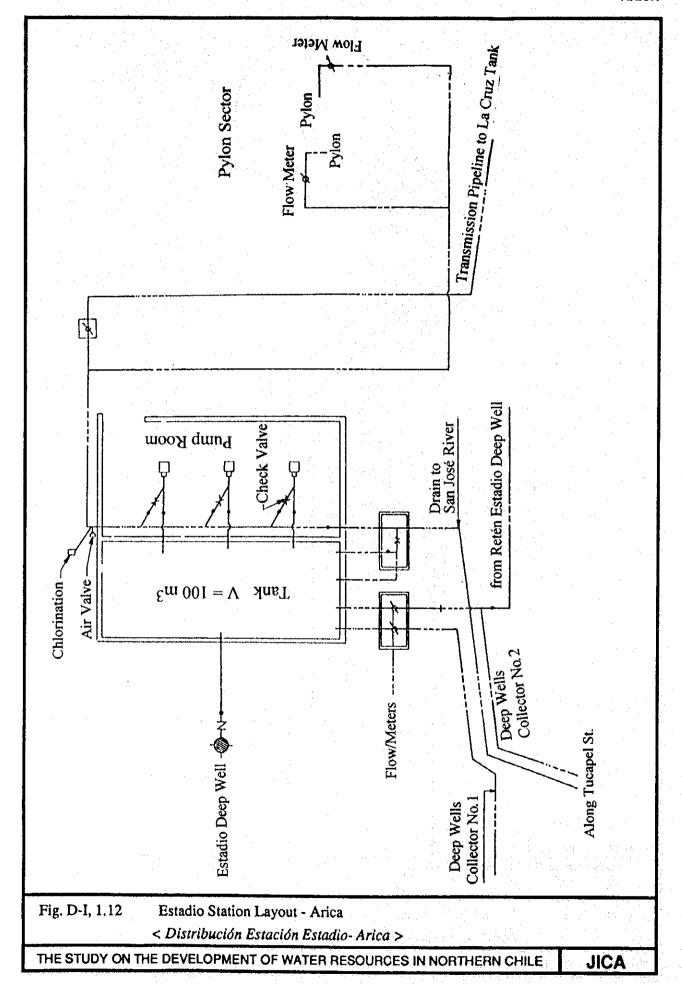


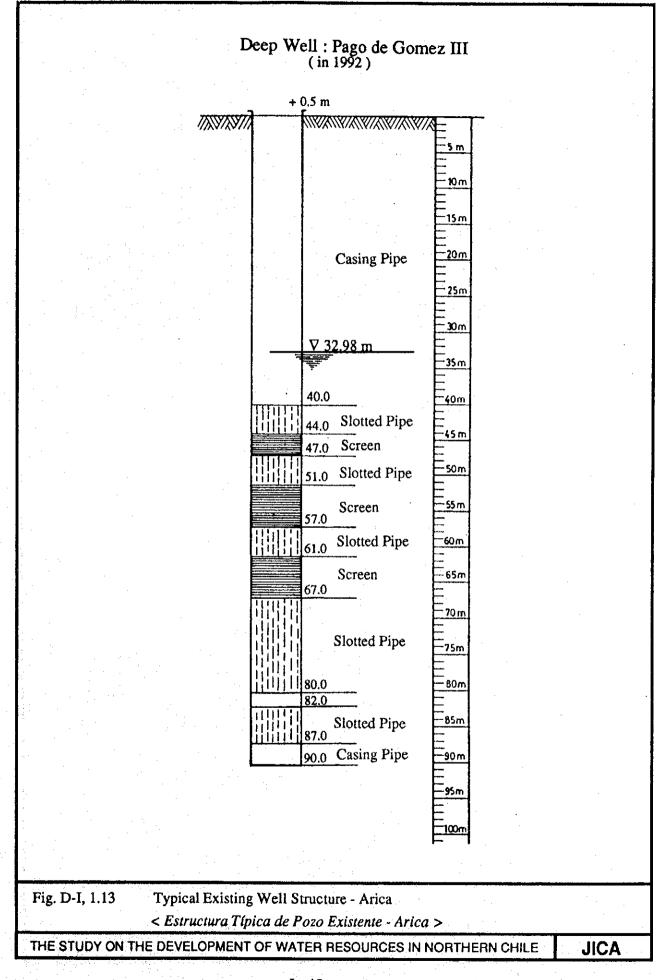
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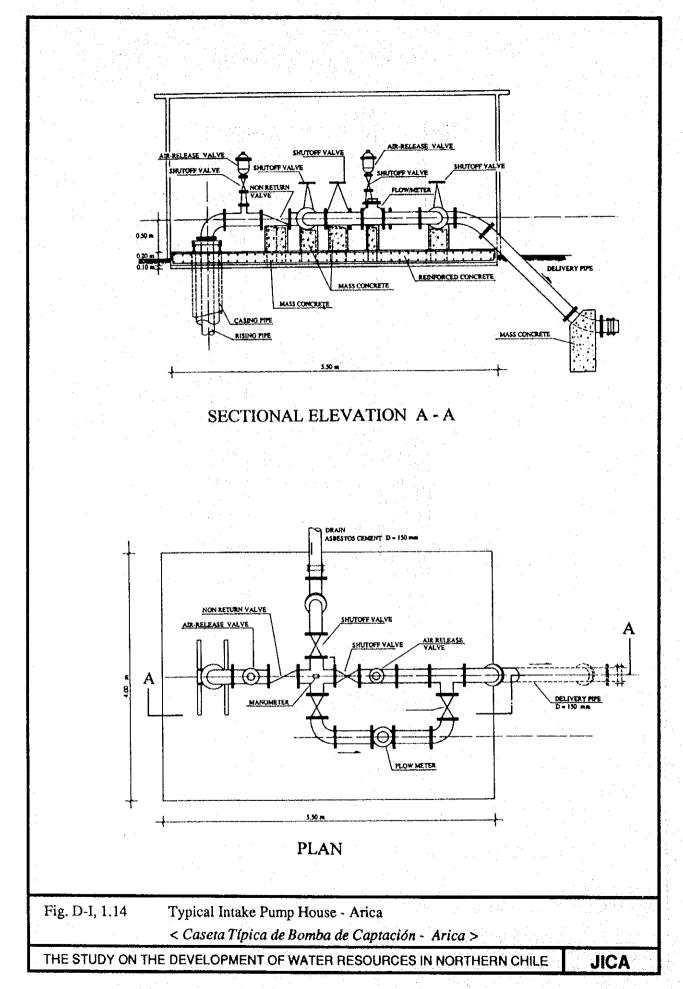


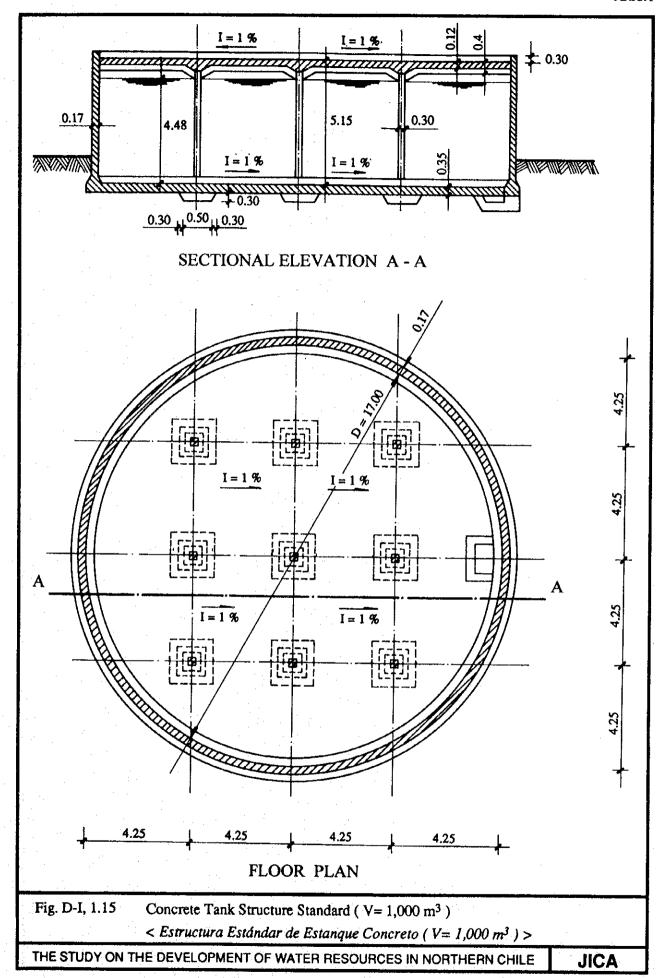


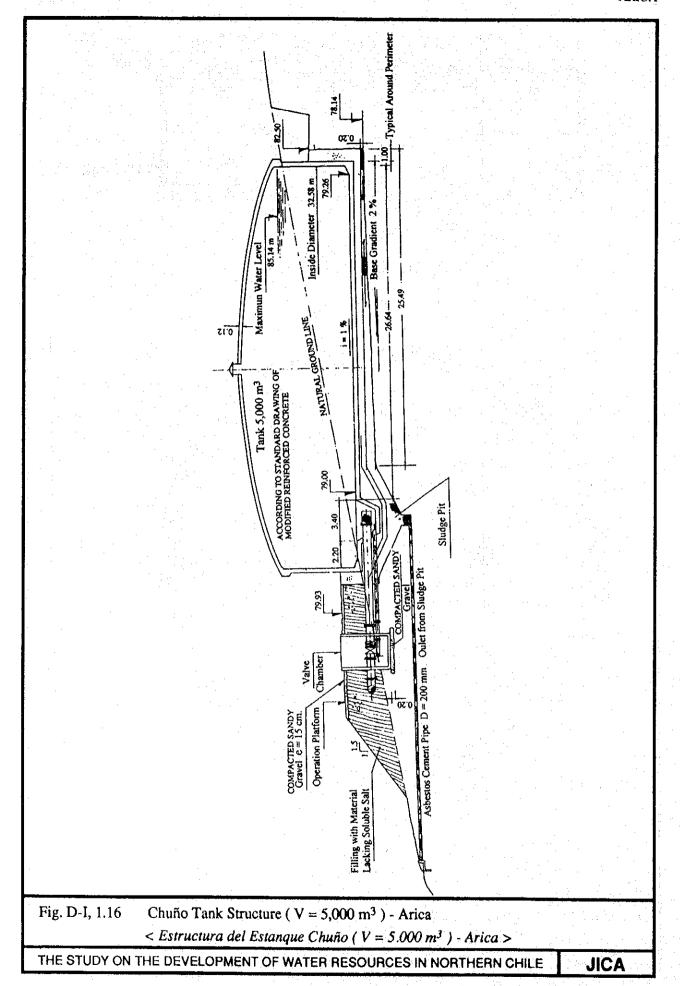












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- <2: "Analisis Programa de Desarrollo de Empresa de Servicios Sanitarios de Tarapaca S.A": Prefactibilidad. Book II: Definicion Proyecto Inversion Ciudad de Arica. By Bustamante y Schudeck, Ingenieros Consultores Ltda., March 1992.</p>

## Chapter II. SUPPLY AND DEMAND BALANCE

#### 2.1 Future Water Demand

According to the forecast of future population and the projection of future water demand, the following figures of water production demand are estimated for the Arica City. (See the Supporting Report C "Water Use" for detail.)

- For the year 2005, a production capacity of 840 l/sec on the daily average basis (= 1,092 l/sec on the daily maximum)

and,

- For the year 2015, a production capacity of 1,091 l/sec on the daily average basis (= 1,419 l/sec on the daily maximum).

(Note): Daily Maximum Demand = 130% of Daily Average Demand

# 2.2 Supply Capacity and the Balance

The authorized capacity was 503 l/sec in the year 1992, hence the deficit, the capacity to be added to the existing capacity will be as follows:

- For the year 2005:

The deficit = 1,092 - 503 = 589 l/sec (Daily maximum),

and,

- For the year 2015:

The deficit = 1,419 - 503 = 916 l/sec (Daily maximum)

(Refer to Table D-I, 2.1 and Fig. D-I, 2.1.)

Table D-I, 2.1 Water Demand Projection for Arica

(Unit: 1/sec)

						Cinc. Dace
	Year	1995	2000	2005	2010	2015
: * V	Target			Short- Term		Long- Term
(A)	Daily Average Production Demand	779	797	840	959	1,091
(B)	Daily Maximum Production Demand	1,013	1,036	1,092	1,247	1,419
(C)	Existing Production Capacity as of 1992	503	503	503	503	503
(D)	Deficit (Capacity to be Added to 1992 Capacity)	510	533	589	744	916

(Note): (A) From the report of "Water Use" (Supporting Report "C" / JICA)

- (B) Criteria: (Daily Average): (Daily Maximum) = 1.00: 1.30 (B) = (A) x 130%
- (C) Existing capacity: 503 l/s
  Additional capacity by Emergency Project is considered as a temporary one.
- (D) = (B)-(C)

# Water Demand Projection for Arica

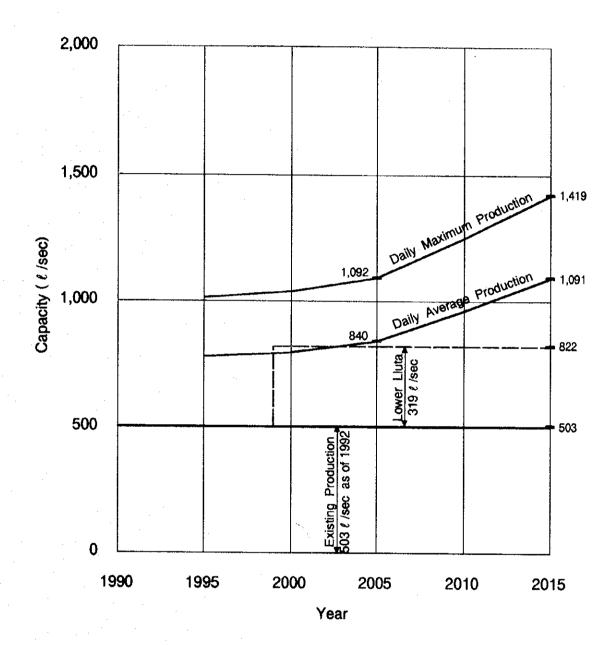
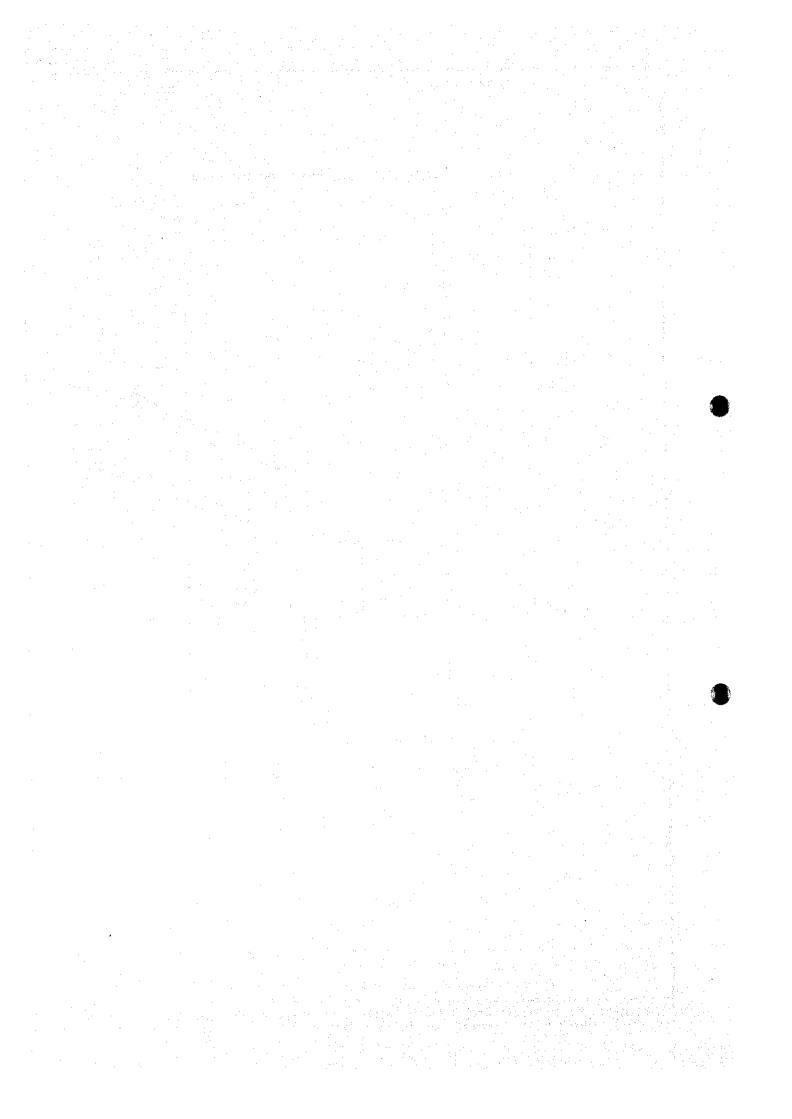


Fig. D-I, 2.1 Water Demand Projection for Arica < Proyeccio'n de Demanda de Agua para Arica >

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### Chapter III. DEVELOPMENT PLAN

### 3.1 Water Source and Development Capacity

The short-term goal is to develop the water resources to meet the water demand until the year 2005. The water production requirement in the year 2005 is estimated to be 1,092 l/sec on the daily maximum basis (840 l/sec on the daily average basis), and the long-term target is to produce water at a rate of 1,419 l/sec on the daily maximum basis (1,091 l/sec on the daily average basis) by the year 2015. The required additional production capacity in future is shown in Table D-I, 3.1.

According to the field investigations, the number of potential water sources in the surrounding area are very limited. The groundwater submerged in the downstream areas of the Lluta river (Lower Lluta Valley) has been identified as a potential water source. However, this source is not sufficient to meet even the short-term (Year 2005) water intake requirement. Although the water intake capacity is large enough to meet the year 2005 water demand, the rate of water production is reduced due to dissolved salts present in the water. The proposed treatment process produces wastewater quantity equivalent to 25% of the intake capacity. Therefore, the supply is adequate to meet the water intake requirement until the year 2003. Due to the scarcity of water, it is decided to develop this water source to meet the immediate water demand of the Arica city. However, as the demand increases with time, more field investigations shall be carried out, in order to find suitable sources to meet the future demand.

The field investigations revealed that the maximum water extraction capacity from the above source is 552.5 l/sec on the daily maximum basis, or 425 l/sec on the daily average basis. It is proposed to construct 26 deep wells (including 3 wells on standby) with a depth of 150 m or 120 m, at 500 m apart, along the Highway No.11, to extract groundwater. The first well (No.1) is located approx. 3,600 m away from the intersection of Highways No.5 (Pan-American Highway) and No.11. The wells are equipped with submersible pumps with an average discharge capacity of 25 l/sec and the pump head of 54-88 m. The Fig. D-I, 3.1 shows the location of the proposed well field. The typical deep well structure is shown in Fig. D-I, 3.4.

### 3.2 Transmission Facilities

#### 3.2.1 Pipelines

The wells are located on a gradually sloping terrain, along the road, and the elevation difference between the furthest well (No.26: GL=+347 m) and the Lluta Treatment Plant

(GL+131 m) is not so extremely high, 216 m. Therefore, asbestos cement pipes (ACP) can be used for the raw water transmission pipeline, if break-pressure tanks are provided. The water flow in the transmission pipelines is by gravity. The diameters vary from 150 to 500 mm.

The hydraulic calculations for the transmission pipeline are given in the Table D-I, 3.2 and Fig. D-I,3.2. The longitudinal profile of the transmission pipeline is shown in Fig. D-I, 3.3.

The specification for the submersible pumps are given in Table D-I, 3.3.

The pipes will be laid in trenches with a 1.2 m distance from the ground level to the top of the pipe surface. The pipes will rest on a 200 mm thick sand bed and will be covered with sand 300 mm above top of the pipe. Fig. D-I, 3.5 show cross-sections of typical pipe trenches. The pipe bridge of the transmission pipeline is shown in Fig. D-I, 3.8.

#### 3.2.2 Break-Pressure Tanks

There are four (4) break-pressure tanks located along the pipeline, between the wells, at strategically important locations. The tanks are located at chainages L=0 m (near the Well No.26), L= 3,000 m (near the well No.20), L= 5,500 m (near the Well No.15) and at L= 8,500 m (near the Well No.9). The tanks are made of concrete and are designed to have 10 minutes' detention time. The tanks are designed to serve as water collection tanks as well as break pressure tanks. The specifications and standard design of the tanks are shown in Fig. 3.6 and Fig. 3.7 respectively.

# 3.3 Water Quality and Treatment

The Lower Lluta groundwater contains substantially high concentrations of TDS (total dissolved solids). The level of Boron (B) is also high, although the Chilean Drinking Water Standards NCH 409 does not specify the allowable limits of Boron (B) in drinking water.

As the raw water contains high concentrations of TDS, a desalination plant is required to be constructed to reduce the TDS contents to an acceptable level. The desalination process shall also be capable of removing about 75-80% of Boron present in water. Location of the new treatment plant is recommended to be at Well No.1 site, along the Highway No. 11, at the ground elevation of +131 m above MSL (See, Fig. D-I, 3.9).

Following desalination techniques are considered for treating the raw water:

- Reverse Osmosis (RO)

or,

# - Electrodialysis Reversal (EDR)

The final selection of the desalination technique, will depend on the cost for installation and operation and maintenance, of the treatment plant. (See Appendix-2)

However, there is no experience of the treatment of such brackish water with a high content of Boron as the groundwater of Lower Lluta Valley in actual potable water supply. Therefore, the groundwater of Lower Lluta Valley was experimentally treated in this Study. The experiment was conducted by using a small-scale RO treatment with a cooperation of ESSAT in Arica during August to September 1994. The results are summarized below.

· · · · · · · · · · · · · · · · · · ·	TDS	Na	SO4	Cl	Fe	В
Raw Water (mg/l)	3,438	565	1,018	929	0.12	22
Treated Water (mg/l)	43	8.7	2.5	14	0.02	4.5
Treatment Efficiency (%)	99	98	99	98	82	79
Permissible Limit (mg/l)	1,000	200	250	250	0.3	5.0

In the above test, the recovery efficiency was 76.2% under the pump pressure of 20 kg/cm<sup>2</sup>.

As evident from the above table, RO method is effective for the treatment of the groundwater of Lower Lluta Valley. Boron is considered the most critical water quality element for the treatment. Therefore, RO method is adopted in this Study.

For details of the experiment, see Appendix 5.

The calculation of the equipment capacity of the treatment plant is shown in Table D-I, 3.4. The flow diagram and layout of the proposed treatment plant are shown in Fig. D-I, 3.10 to Fig. D-I, 3.13.

A high quantity of wastewater, about 25% of the intake capacity (552.5 l/sec x  $25\% = 11,900 \text{ m}^3/\text{day}$ ), will be produced during the desalination process.

It is proposed to discharge wastewater directly into the sea, through an exclusive drain pipeline by gravity, from the Lluta Treatment Plant.