

## **Appendix 5 Flow Calculation of Sewer Network**



Hydraulic Calculation for Design of Sewers

( A r e a I )

P

1

: 0.00028453 m<sup>3</sup>/s-ha

No. of Sewers	Downstream Sewers No.	Drainage Area		Length		Concentrated Time	Run-off Storm			Wastewater Flow		Other W.W		Grand Total	Designing of Sewers						Remarks				
		Area	Total	Area	Total		Run-off Coeff.	Area	Total	Rainfall	Pop. Density	Population	Design Flow		Sewer	Total	Design Flow	Diameter	Slope	Velocity		Flow	Elevation	Invert Level	Earth Cover
		ha	ha	m	m	min	m <sup>3</sup> /sec-ha	ha	ha	m <sup>3</sup> /sec	Person	m <sup>3</sup> /sec	m <sup>3</sup> /sec	m <sup>3</sup> /sec	m <sup>3</sup> /sec	mm	%	m/sec	m <sup>3</sup> /sec	M	M	M	m		
1	3	1368	1368	520	520					0.0039				0.0039	150	2.840	1.318	0.0233	8200	80331	150	8200	80331	150	
2		0.37	0.37	60	60					0.0001				0.0001	150	1.660	1.110	0.0186	8200	80331	150	8200	81327	150	
3	9	186	1591	200	720					0.0045				0.0045	150	3.050	1.805	0.0266	8200	80331	150	8200	80331	150	
4	6	319	319	155	155					0.0009				0.0009	150	5.000	0.609	0.0108	8200	80331	150	8200	80331	150	
5		332	332	110	110					0.0005				0.0005	150	5.000	0.509	0.0108	8200	80331	150	8200	79781	205	
5	8	0.00	651	50	205					0.0019				0.0019	150	4.640	1.316	0.0321	7900	79556	228	8200	79551	228	
7		0.80	0.80	130	130					0.0002				0.0002	150	7.600	0.751	0.0133	8000	76829	150	8000	76829	150	
8		0.95	827	188	393					0.0024				0.0024	150	1.640	1.104	0.0135	7900	77328	150	7900	77328	150	
9	11	0.35	2453	67	787					0.0070				0.0070	150	4.470	1.922	0.0322	7280	71231	150	7280	71231	150	
10		159	159	205	205					0.0005				0.0005	150	2.480	1.357	0.0240	7800	76828	150	7800	76828	150	
11	13	192	2804	210	987					0.0080				0.0080	200	3.330	1.919	0.0603	7280	71179	150	7280	71179	150	

# Hydraulic Calculation for Design of Sewers ( A r e a I )

: 0.00028453 m<sup>3</sup>/s-ha

No. of Sewers	Downstream Sewers No.		Drainage Area				Length		Concentrated Time		Run-off Storm				Wastewater Flow				Designing of Sewers						Remarks				
	ha	Total	ha	Total	m	Total	min	m <sup>3</sup> /sec-ha	Rainfall	Run-off Coeff.	Arranged Area		Rainfall	Pop. Density	Population		Design Flow	Sewer	Total	Grand Total	Diameter	Slope	Velocity	Flow		Elevation	Invert Level	Earth Cover	
											ha	ha			Person	Sewer													Total
12	0.85	0.85	90	90	90	90									0.0002	0.0002	0.0002	0.0002	0.0002	0.0002	150	6.770	2.242	0.0396	7.190	70.227	150		
13	2.00	3.039	205	1202											0.0088						200	2.490	1.647	0.0518	6.580	64.075	150		
16	2.19	2.19	308	308											0.0006						150	2.620	1.355	0.0247	8.100	79.326	150		
17	0.50	0.50	65	65											0.0001						150	5.000	0.609	0.0108	7.290	71.231	150		
18	0.54	3.23	85	393											0.0009						150	2.260	1.556	0.0275	7.290	70.907	150		
14	1.41	1.41	173	173											0.0004						150	3.860	0.799	0.0141	7.590	74.228	150		
15	1.94	2.45	165	338											0.0007						150	2.780	1.437	0.0254	7.440	72.727	150		
19	0.54	6.02	80	473											0.0017						150	5.620	2.043	0.0361	6.980	68.131	150		
20	0.73	0.73	70	70											0.0002						150	2.140	1.261	0.0223	6.580	65.130	150		
21	0.47	7.22	105	578											0.0021						150	4.380	1.804	0.0319	6.530	63.631	150		
22	0.96	3.907	120	1322											0.0111						200	2.500	1.651	0.0519	6.070	58.975	150		

# Hydraulic Calculation for Design of Sewers

A r e a i )

P

3

: 0.00028459 m<sup>3</sup>/s·ha

No. of Sewers	Downstream Sewers No.	Drainage Area		Length		Concentrated time	Run-off Storm			Wastewater Flow			Designing of Sewers						Remarks						
		Area	Total	Length	Total		Rainfall	Run-off Coeff	Area	Total	Rainfall	Pop. Density	Population Sewer	Design Flow	Sewer Flow	Total	Grand Total	Diameter		Slope	Velocity	Flow	Elevation	Invert Level	Earth Cover
		ha	ha	m	m	m <sup>3</sup> /sec·ha		ha	ha	m <sup>3</sup> /sec	Person/ha	m <sup>3</sup> /sec	m <sup>3</sup> /sec	m <sup>3</sup> /sec	m <sup>3</sup> /sec	mm	%	m/sec	m/sec	m <sup>3</sup> /sec	M	M	m		
33	35	801	801	220	220					0.0023					0.0023	150	2.810	1.310	0.0231	7.580	7.4227	1.50			
																					7.080	6.9131	1.50		
34		0.99	0.99	105	105					0.0003		0.0042	0.0042	0.0045	150	4.850	1.388	0.0335	7.580	7.4227	1.50				
																					7.080	6.9131	1.50		
35	36	2.74	1.174	300	570					0.0033		0.0042	0.0042	0.0075	200	1.830	1.412	0.0444	7.080	6.9075	1.50				
																					6.530	6.3575	1.50		
23		0.50	0.50	45	45					0.0001					150	2.220	1.284	0.0227	8.200	8.1330	1.50				
																					8.200	8.0331	1.50		
24	26	0.61	1.11	130	175					0.0003					150	2.070	1.240	0.0219	8.200	8.0328	1.50				
																					7.930	7.7531	1.50		
25		0.77	0.77	105	105					0.0002					150	2.570	1.362	0.0244	8.200	8.0330	1.50				
																					7.930	7.7531	1.50		
26	29	0.42	2.30	70	245					0.0007					150	3.420	1.594	0.0282	7.930	7.7528	1.50				
																					7.690	7.5231	1.50		
27		1.00	1.00	50	50					0.0003					150	4.000	1.724	0.0305	8.200	8.0331	1.50				
																					8.000	7.8331	1.50		
28		1.05	2.05	175	225					0.0006					150	1.770	1.147	0.0203	8.000	7.8330	1.50				
																					7.690	7.5231	1.50		
29	32	0.42	4.77	85	330					0.0014					150	4.730	1.868	0.0330	7.930	7.5228	1.50				
																					7.230	7.1231	1.50		

# Hydraulic Calculation for Design of Sewers

P 4

: : 0.0028453 m<sup>3</sup>/s-ba

No. of Sewers	Downstream Sewers No.	Drainage Area		Length		Concentrated Time	Run-off Storm			Wastewater Flow			Designing of Sewers					Remarks				
		Area	Total	Length	Total		Rainfall	Run-off Coeff.	Arranged Area	Rainfall	Pop. Density	Population Sewer	Design Flow	Other W. #	Grand Total	Diameter	Slope		Velocity	Flow	Elevation	Invert Level
		ha	ha	m	m	min	m <sup>3</sup> /sec-ha	ha	ha	Person/ha	m <sup>3</sup> /sec	m <sup>3</sup> /sec	m <sup>3</sup> /sec	mm	%	m/sec	m <sup>3</sup> /sec	M	M	M	m	
30		130	130	50	50						00005		00005	150	8200	00436	00436	8090	78331	78331	150	
31		335	335	215	265						00010		00010	150	1390	00180	00180	7590	74228	74228	150	
32		117	929	175	505						00026		00026	150	4340	00317	00317	7290	71229	71229	150	
36	38	030	2133	100	520						00061		00042	200	400	00207	00207	5530	63575	63175	150	
37		076	076	88	88						00002		00002	150	3400	00281	00281	6830	66627	66627	150	
38	40	010	2219	70	690						00068		00042	200	1640	00420	00420	6530	63175	62075	150	
39		096	096	140	140						00003		00003	150	3210	00273	00273	6830	65629	62131	150	
40	42	019	2334	75	765						00056		00042	200	2800	00549	00549	6320	62075	59975	150	
41		111	111	145	145						00003		00003	150	2430	00240	00240	6530	63530	60031	150	
42		051	2486	163	928						00071		00042	200	2450	00513	00513	6170	59975	55975	150	
43	45	054	6467	110	1432						00184		00042	300	1450	00184	00184	5770	55689	54289	150	
44		2334	2334	130	130						00058		00058	150	3530	00286	00286	6070	59027	54431	150	

# Hydraulic Calculation for Design of Sewers ( A R e a I )

P 5

: 0.00028453 m<sup>3</sup>/s-ha

No. of Sewers	Downstream Sewers No.	Drainage Area		Length		Concentrated Time	Run-off Storm			Wastewater Flow			Other W.W		Grand Total	Designing of Sewers						Remarks
		Area	Total	Length	Total		Run-off Coeff.	Area	Arranged Area	Rainfall	Pop. Density	Pop. Sewer	Design Flow	Total		Total	Diameter	Slope	Velocity	Flow	Elevation	
		ha	ha	m	m	min.	ha	ha	m <sup>3</sup> /sec	Person	m <sup>3</sup> /sec	m <sup>3</sup> /sec	m <sup>3</sup> /sec	mm	%	m/sec	m <sup>3</sup> /sec	M	M	m		
45		030	8851	1000	2432						00252	00042	00294	300	4300	0855	00612	3940	36969	210		
46	125	030	8851	250	2662						00252	00083	00377	375	10000	1537	03753	3940	36937	210		
47		156	136	118	118						00004		00004	150	930	0831	00147	8530	85130	150		
48	50	034	190	57	175						00005		00005	150	1030	0883	00156	8570	84029	150		
49		233	233	50	50						00007		00007	150	1200	0944	00167	8570	84021	150		
50	58	109	532	270	445						00015		00015	150	2850	1455	00257	8510	83430	150		
51	53	048	048	100	100						00001		00001	150	2750	1440	00254	8660	84925	150		
52		029	029	75	75						00001		00001	150	2530	1371	00242	8570	84030	150		
53	55	026	193	58	188						00003		00003	150	2240	1290	00228	8380	82330	150		
54		055	055	115	115						00002		00002	150	2260	1296	00229	8510	83400	150		
55	57	018	176	56	214						00005		00005	150	3370	1628	00286	8250	80830	150		

# Hydraulic Calculation for Design of Sewers ( A r e a I )

P. 6

: 0.00028453 m<sup>3</sup>/s-ha

No. of Sewers	Downstream Sewers No.	Drainage Area		Length		Concentrated Time	Run-off Storm			Wastewater Flow			Designing of Sewers						Remarks					
		Area	Total	ha	m		Length	Total	ha	ha	Pop. Density	Population Sewer	Design Flow	Sewer Flow	Total	Grand Total	Diameter	Slope		Velocity	Flow	Elevation	Invert Level	Earth Cover
		ha	ha	ha	m	m	min	Rainfall	Run-off Coeff.	Area	Total	Person	m <sup>3</sup> /sec	m <sup>3</sup> /sec	m <sup>3</sup> /sec	mm	%	m/sec	m <sup>3</sup> /sec	M	M	M	m	
56		0.54	0.54	115	115								0.0002		0.0002	150	1.810	0.986	0.0174	80.50	80.339	80.339	150	
57		0.30	2.60	98	307								0.0007		0.0007	150	3.830	1.573	0.0218	80.50	78.229	78.229	150	
58	60	0.15	8.07	40	485								0.0023		0.0023	150	1.250	0.964	0.0170	76.90	75.731	75.731	150	
59		0.38	0.38	80	80								0.0001		0.0001	150	4.630	1.828	0.0323	80.50	78.331	78.331	150	
60	69	0.16	8.61	55	540								0.0024		0.0024	150	1.810	1.159	0.0205	76.90	75.226	75.226	150	
61		0.95	0.95	95	95								0.0003		0.0003	150	5.00	0.609	0.0108	86.60	85.131	85.131	150	
62	64	0.22	1.17	57	152								0.0003		0.0003	150	2.150	1.254	0.0223	86.60	84.555	84.555	178	
63		0.46	0.46	88	88								0.0001		0.0001	150	5.00	0.609	0.0108	86.60	83.431	83.431	150	
64	66	0.15	1.78	56	207								0.0005		0.0005	150	4.830	1.894	0.0335	86.60	82.987	82.987	194	
65		0.41	0.41	93	93								0.0001		0.0001	150	5.00	0.609	0.0108	86.60	80.331	80.331	150	
66	68	0.14	2.33	56	263								0.0007		0.0007	150	3.450	1.601	0.0283	86.60	77.931	77.931	150	



# Hydraulic Calculation for Design of Sewers

3.00028453 m<sup>3</sup>/s-ha

No. of Sewers	Downstream Sewers No.	Drainage Area		Length		Concentrated Time	Run-off Storm			Wastewater Flow			Other W.H		Grand Total		Designing of Sewers						Remarks	
		Area	Total	Length	Total		Run-off Coeff.	Area	Total	Pop. Density	Pop. Sewer	Design Flow	Total	Sewer	Total	Design Flow	Diameter	Slope	Velocity	Flow	Elevation	Invert Level		Earth Cover
		ha	ha	m	m	min	m <sup>2</sup> /sec-ha	ha	ha	Person/ha	m <sup>3</sup> /sec	m <sup>3</sup> /sec	m <sup>3</sup> /sec	%	m/sec	mm	%	m/sec	m <sup>3</sup> /sec	M	M	M	m	
67		0.44	0.44	97	97						0.0001					0.0001	150	0.20	0.827	0.0146	8050	78327	150	
68		0.51	328	110	378						0.0009					0.0009	150	3.80	1.580	0.0279	7950	74231	150	
69	71	0.14	1203	58	538						0.0034					0.0034	150	5.30	0.903	0.0108	7590	74231	150	
70		0.81	0.81	155	155						0.0002					0.0002	150	1.670	1.114	0.0197	7850	76327	150	
71	84	0.31	1815	95	538						0.0037					0.0037	150	5.00	0.609	0.0108	7590	73467	226	
72		0.90	0.90	60	60						0.0000					0.0000	150	5.00	0.609	0.0108	8650	84831	150	
73		0.37	0.37	50	110						0.0001					0.0001	150	3.80	1.633	0.0289	8650	84826	181	
74	76	0.46	0.83	115	225						0.0002					0.0002	150	3.040	1.503	0.0266	8450	82320	150	
75		0.54	0.54	80	80						0.0002					0.0002	150	1.250	0.964	0.0170	8200	80831	150	
76	78	0.09	1.46	35	260						0.0004					0.0004	150	1.420	1.027	0.0181	8100	79328	150	
77		0.69	0.69	103	103						0.0002					0.0002	150	5.00	0.609	0.0108	8050	78306	203	

# Hydraulic Calculation for Design of Sewers

A r e a I )

P

8

: 0.00028453 m<sup>3</sup>/s · ha

No. of Sewers	Downstream Sewers No.	Drainage Area				Length			Concentrated Time		Run-off Storm			Wastewater Flow			Other W. F.		Designing of Sewers						Remarks			
		Area		Total		Length		Total		Rainfall	Run-off Coeff	Arranged Area		Rainfall	Pop. Density	Sewer	Design Flow	Sewer	Total	Grand Total	Diameter	Slope	Velocity	Flow		Elevation	Invert Level	Earth Cover
		ha	ha	ha	ha	ha	ha	ha	ha			ha	ha															
78	80	0.14	2.29	0.88	149	149	56	316							0.0007	0.0021	0.0028	150	2630	0.998	0.0247	80.50	7830.4	803	7830.4	803		
79																												
80	88	0.21	3.38	0.92	149	381	65	381							0.0010	0.0021	0.0031	150	2240	0.998	0.0228	78.50	7678.7	154	7678.7	154		
81																												
82																												
83																												
84	86	0.26	2.350	0.42	80	451	200	290							0.0008	0.0021	0.0040	150	620	0.979	0.0120	77.00	7473.1	210	7473.1	210		
85																												
86																												
87	95	0.95	24.22	0.13	85	1173	50	1173							0.0002	0.0021	0.0067	200	5900	0.934	0.0320	63.50	6177.5	150	6177.5	150		
88	90	3.95	3.95	1.35	135	135																						

# Hydraulic Calculation for Design of Sewers

A T E A I )

P. 9

: 0.00028453 m<sup>3</sup>/s · ha

No. of Sewers	Downstream Sewers No.	Drainage Area		Length		Concentrated Time	Run-off Storm			Wastewater Flow		Other W.W		Grand Total	Designing of Sewers						Remarks		
		Area	Total	Length	Total		Rainfall	Run-off Coeff	Arranged Area	Rainfall	Pop. Density	Population	Design Flow		Sewer	Total	Diameter	Slope	Velocity	Flow		Elevation	Invert
		ha	ha	m	m	min	m <sup>3</sup> /sec · ha	ha	ha	m/sec	Person/ha	Person	m <sup>3</sup> /sec	m <sup>3</sup> /sec	mm	%	m/sec	m <sup>3</sup> /sec	M	M	M	m	
89		0.68	0.68	80	80									0.0002	150	5.00	0.609	0.0108	6.790	6.5331	6.4931	1.50	
90	92	0.86	5.50	120	255									0.0016	150	3.240	1.551	0.0274	6.890	6.4931	6.1031	1.50	
91		2.91	2.91	210	210									0.0008	150	1.850	1.203	0.0213	6.690	6.5130	6.1931	1.50	
92	94	0.09	6.50	35	290									0.0024	150	5.710	2.059	0.0364	6.270	6.1029	5.9031	1.50	
93		0.14	0.14	45	45									0.0001	150	6.60	0.700	0.0124	6.190	5.9328	5.9031	1.50	
94		0.13	8.77	40	330									0.0025	150	4.240	1.775	0.0314	6.070	5.9027	5.7331	1.50	
95		0.62	3.651	70	1453									0.0096	200	4.860	2.302	0.0723	5.990	5.7275	5.3875	1.50	
96	98	0.68	34.29	100	1553									0.0093	200	4.600	2.239	0.0703	5.560	5.3875	4.9275	1.50	
97		0.79	0.79	100	100									0.0002	150	5.00	0.609	0.0108	5.020	4.8331	4.8031	1.50	
98	100	0.20	35.28	40	1593									0.0100	200	5.50	0.774	0.0243	5.190	4.7381	4.7161	1.50	
99		0.31	0.31	40	40									0.0001	150	5.00	0.609	0.0108	5.160	4.9931	4.9731	1.70	
100		0.26	3.685	100	1.693									0.0102	200	2.570	1.674	0.0526	5.160	4.7761	4.5275	1.50	

# Hydraulic Calculation for Design of Sewers

( A r e a I )

P 10

: 0.00028453 m<sup>3</sup>/s · ha

No. of Sewers	Downstream Sewers No.		Drainage Area		Length		Concentrated Time		Run-off Storm			Wastewater Flow			Other W.W		Grand Total		Designing of Sewers						Remarks
	Area	Total	Length	Total	Rainfall	Run-off Coeff.	Area	Total	Arranged Area	Rainfall	Pop. Density	Population		Design Flow	Sewer	Total	Design Flow	Diameter	Slope	Velocity	Flow	Elevation	Invert Level	Earth Cover	
												ha	ha												
101	0.32	0.617	75	1768										0.0103		0.021	0.0124	200	1.930	1.204	0.0378	4.900	4.931	1.50	
102	0.24	0.24	60	60									0.0001			0.0001	150	5.000	1.927	0.0341	4.900	4.931	1.50		
103	0.17	0.638	65	1833									0.0104		0.021	0.0125	200	1.830	1.357	0.0426	4.600	4.675	1.50		
104	0.45	0.45	110	110									0.0001			0.0001	150	1.000	0.362	0.0152	4.600	4.631	1.50		
105	0.24	0.727	95	1928									0.0106		0.021	0.0127	200	2.000	1.476	0.0464	4.300	4.375	1.50		
115	0.90	0.90	125	125									0.0003			0.0003	150	3.900	0.771	0.0136	5.560	5.591	1.50		
116	0.37	1.27	90	215									0.0004			0.0004	150	4.400	1.816	0.0321	5.460	5.491	1.50		
117	0.28	0.28	50	50									0.0001			0.0001	150	2.000	1.219	0.0215	5.160	5.191	1.50		
118	1.00	2.55	160	375									0.0007			0.0007	150	2.050	1.237	0.0219	5.060	5.091	1.50		
106	0.99	0.99	115	115									0.0003			0.0003	150	5.000	0.609	0.0108	6.100	6.131	1.50		

# Hydraulic Calculation for Design of Sewers

: 0.00026453 m<sup>3</sup>/s · ha

No. of Sewers	Domstream Sewers No.	Drainage Area		Length		Concentrated Time	Run-off Storm			Wastewater Flow		Other W. W.		Designing of Sewers						Remarks																								
		Area	Total	Length	Total		Rainfall	Run-off Coeff.	Arranged Area		Rainfall	Pop. Density	Population Sewer	Design Flow	Sewer Flow	Total	Grand Total	Design Flow	Diameter		Slope	Velocity	Flow	Elevation	Invert Level	Earth Cover																		
									ha	ha																	ha	ha	ha	Person	m <sup>2</sup> /sec	m <sup>2</sup> /sec	m <sup>2</sup> /sec	m <sup>2</sup> /sec	mm	%	m/sec	m <sup>3</sup> /sec	M	M	m			
107		0.38	0.38	105	105							0.0003			0.0003		0.0003	150	2.950	1.480	0.0252	6.330	62.130	150	5.9031	150																		
108	112	0.64	2.51	125	240							0.0007			0.0007		0.0007	150	5.140	1.954	0.0345	6.070	58.757	177	5.2331	150																		
109		0.38	0.38	70	70							0.0003			0.0003		0.0003	150	5.00	0.609	0.0108	5.310	51.431	150	5.1081	303																		
110	112	0.00	0.38	50	120							0.0003			0.0003		0.0003	150	5.00	0.609	0.0108	5.330	51.081	305	5.0831	300																		
111		0.38	0.38	115	115							0.0003			0.0003		0.0003	150	5.20	0.621	0.0110	5.460	52.330	150	5.2331	150																		
112	114	0.17	4.44	45	285							0.0013			0.0013		0.0013	150	2.030	12.19	0.0215	5.400	50.831	300	4.8831	150																		
113		0.30	0.30	40	40							0.0001			0.0001		0.0001	150	5.00	0.609	0.0108	5.130	49.931	150	5.160	49.781	170																	
114		0.35	5.10	85	370							0.0015			0.0015		0.0015	150	4.820	1.862	0.0334	5.130	49.781	170	4.730	45.631	150																	
119	121	0.00	7.65	15	350							0.0022			0.0022		0.0022	150	5.00	0.609	0.0108	4.730	45.631	150	4.730	45.555	158																	
120		0.46	0.46	60	60							0.0001			0.0001		0.0001	150	3.990	1.721	0.0304	4.970	48.025	151	4.730	45.631	150																	
121	124	0.35	8.46	100	490							0.0024			0.0024		0.0024	150	4.220	1.770	0.0313	4.730	45.555	158	4.730	41.831	150																	

# Hydraulic Calculation for Design of Sewers ( A r e a I )

: C. 00028453 m<sup>3</sup>/s.ha

No. of Sewers	Downstream Sewers No.	Drainage Area		Length		Concentrated Time	Run-off Storm			Wastewater Flow			Other W. W.		Grand Total	Designing of Sewers						Remarks	
		Area	Total	Area	Total		Run-off Coeff.	Arranged Area	Rainfall	Pop. Density	Population	Design Flow	Sewer	Total		Diameter	Slope	Velocity	Flow	Elevation	Invert Level		Earth Cover
		ha	ha	ha	ha	min	ha	m <sup>3</sup> /sec-ha	ha	ha	Person/ha	Person	m <sup>3</sup> /sec	m <sup>3</sup> /sec	m <sup>3</sup> /sec	mm	%	m/sec	m <sup>3</sup> /sec	M	M	M	
122		033	033			90							00001		00001	150	2990	1.490	00263	4970	48026	131	
						90														4700	45331	150	
123		032	065			190							00002		00002	150	4000	1.724	00305	4700	45331	150	
						190														4300	41331	150	
124		001	4639			2093							00132		00132	250	4050	2.441	01198	4300	41224	150	
						2093														3630	34524	150	
125	MH 101	000	13500			2832							00884		00884	450	170	0.739	01176	3830	34304	150	
						2832														3630	34049	176	

: 0.00037450 m<sup>3</sup>/s.ha

## Hydraulic Calculation for Design of Sewers

No. of Sewers	Downstream Sewers No.	Drainage Area		Length			Concentrated Time		Run-off Storm			Mastewater Flow				Designing of Sewers			Remarks						
		Area	ha	Area	ha	Total	ha	min	Total	ha	ha	ha	Population	Design Flow	Other W.W	Grand Total	Diameter	Slope		Velocity	Flow	Elevation	Invert Level	Earth Cover	
		ha	ha	ha	ha	ha	ha	m <sup>3</sup> /sec.ha	ha	ha	ha	Person	Person	m <sup>3</sup> /sec	m <sup>3</sup> /sec	m <sup>3</sup> /sec	mm	%	m/sec	m <sup>3</sup> /sec	M	M	m	m	
1-1		0.88	0.93										0.0003	0.0003		0.0003	150	4.490	1.826	0.022	3.390	4.225	1.51		
1-2	MH 108	0.00	0.93										0.0003	0.0003		0.0003	150	5.730	2.063	0.065	3.350	3.883	1.50		

# Hydraulic Calculation for Design of Sewers

: 0.00037450 m<sup>3</sup>/s-ha

No. of Sewers	Downstream Sewers No.	Drainage Area		Length		Concentrated Time	Run-off Storm			Wastewater Flow		Other W. W.		Grand Total	Designing of Sewers									
		Area	Total	Length	Total		Run-off	Coef.	Area	Total	Rainfall	Pop. Density	Person		Design Flow	Sewer	Total	Design Flow	Diameter	Slope	Velocity	Flow	Elevation	Invert Level
		ha	ha	m	m	min	m <sup>2</sup> /sec-ha	ha	ha	m <sup>3</sup> /sec	Person/ha	Person	m <sup>3</sup> /sec	m <sup>3</sup> /sec	m <sup>3</sup> /sec	mm	%	m/sec	m <sup>3</sup> /sec	M	M	M	m	
2	4	240	240	60	60					0.0009			0.0009		0.0009	150	5.00	0.609	0.0108	6070	58031	150		
3		534	534	20	20					0.0020			0.0020		0.0020	150	5.00	0.609	0.0108	6070	58031	150		
4	6	286	1060	185	245					0.0040			0.0040		0.0040	150	5.00	0.609	0.0108	6070	58131	180		
5		052	052	50	50					0.0002			0.0002		0.0002	150	5.00	0.609	0.0108	5990	58231	150		
6		086	1208	90	395					0.0045			0.0045		0.0045	150	1.970	1.210	0.0214	5990	58208	192		
7	9	023	1231	48	383					0.0046			0.0046		0.0046	150	3.120	1.522	0.0265	5770	58029	150		
8		026	026	60	60					0.0001			0.0001		0.0001	150	8.30	0.735	0.0139	5870	58029	150		
9	11	023	1280	50	453					0.0048			0.0048		0.0048	150	27.90	1.440	0.0254	5830	58131	150		
10		027	027	60	60					0.0001			0.0001		0.0001	150	1.930	0.894	0.0176	5860	58929	150		
11	13	018	1325	45	478					0.0050			0.0050		0.0050	150	2.110	1.520	0.0269	5830	58130	150		
12		022	022	60	60					0.0001			0.0001		0.0001	150	1.990	1.216	0.0215	5860	58925	150		



# Hydraulic Calculation for Design of Sewers

A r e a 2

P 15

: 0.00037450 m<sup>3</sup>/s·ha

No. of Sewers	Downstream Sewers No.	Drainage Area				Run-off Storm				Wastewater Flow				Other W.W.				Designing of Sewers						Remarks															
		Area		Length		Concentrated Time	Rainfall	Run-off Coeff.	Arranged Area		Rainfall	Pop. Density	Population		Design Flow	Sewer	Total	Grand Total Flow	Diameter	Slope	Velocity	Flow	Elevation		Invert Level	Earth Cover													
		ha	ha	m	m				ha	ha			Person	Person													m <sup>2</sup> /sec	m <sup>2</sup> /sec	m <sup>2</sup> /sec	m <sup>2</sup> /sec	mm	%	m/sec	m <sup>3</sup> /sec	M	M	m		
13	15	0.16	1.663	42	520							0.0051						0.0051	150	30.90	1.515	0.0268	5340	51729	150														
14		0.25	0.25	60	60							0.0001						0.0001	150	10.00	0.362	0.0152	5270	51031	150														
15	17	0.25	1.413	60	580							0.0053						0.0053	150	21.60	1.267	0.0224	5210	50427	150														
16		0.23	0.23	60	60							0.0001						0.0001	150	5.00	0.609	0.0108	5080	49241	150														
17	23	0.10	1.446	43	633							0.0054						0.0054	150	28.50	1.376	0.0243	5080	49327	170														
18		3.80	3.80	290	290							0.0013						0.0013	150	18.50	1.078	0.0190	5510	54430	150														
19	22	0.42	4.02	75	365							0.0015						0.0015	150	17.80	1.134	0.0200	5160	49930	150														
20	22	1.94	1.94	276	276							0.0007						0.0007	150	24.20	1.341	0.0237	5700	55327	150														
21		0.30	0.30	45	45							0.0001						0.0001	150	5.00	0.609	0.0108	4940	47731	150														
22		0.13	6.39	50	415							0.0024						0.0024	150	5.00	0.609	0.0108	5030	47506	263														
22	25	0.59	2.144	148	771							0.0080						0.0080	200	1.900	1.190	0.0374	4950	47206	207														

# Hydraulic Calculation for Design of Sewers (A F c a )

0.00037450 m<sup>3</sup>/s·ha

No. of Sewers	Downstream Sewers No.	Drainage Area		Length		Concentrated Time	Run-off Storm				Wastewater Flow			Other W.W		Grand Total Design Flow	Designing of Sewers																			
		Area	ha	Length	m		Total	ha	ha	ha	ha	ha	ha	Pop. Density	Pe/hal		Person	Design Flow	m <sup>3</sup> /sec	Total	m <sup>3</sup> /sec	Design Flow	m <sup>3</sup> /sec	Slope	%	Velocity	m <sup>3</sup> /sec	Flow	Elevation	M	Invert Level	M	Earth Cover	m	Remarks	
24			106		73	73													00004		00004					00004		00004		20.50	1.234	0.0218	4850	46829	150	
25	40-1	048	2238	72	843														00056		00056					00056		00056		43.10	2.167	0.0681	4700	45275	150	
30		059		150	150														00002		00002					00002		00002		30.80	1.508	0.0266	5770	55028	150	
31		053		170	320														00005		00005					00005		00005		27.00	1.416	0.0250	5310	51428	150	
32	33	028	130	76	396														00006		00006					00006		00006		50.00	0.609	0.0108	4850	46831	150	
26		042	042	60	60														00002		00002					00002		00002		5.00	0.609	0.0108	5770	56031	150	
27		046	088	143	203														00003		00003					00003		00003		19.50	1.203	0.0213	5770	55731	180	
28	33	056	144	140	343														00005		00005					00005		00005		39.90	1.721	0.0304	5460	52926	151	
29		022	022	60	60														00001		00001					00001		00001		8.00	0.785	0.0139	4850	47829	150	
33	35	014	330	45	441														00012		00012					00012		00012		19.20	1.168	0.0205	4500	46450	238	
34		024	024	60	60														00001		00001					00001		00001		21.60	1.267	0.0224	4880	46927	150	

# Hydraulic Calculation for Design of Sewers

( A r c a 2 )

:0.00037450 m<sup>3</sup>/s-ha

No. of Sewers	Downstream Sewers No.	Drainage Area		Length		Concentrated Time	Run-off Storm			Wastewater Flow			Other R.#	Grand Total	Designing of Sewers						Remarks		
		Area	Total	Length	Total		Rainfall	Run-off Coeff.	Arranged Area	Rainfall	Pop. Density	Population			Design Flow	Sewer	Total	Diameter	Slope	Velocity		Flow	Elevation
		ha	ha	m	m	min	m <sup>2</sup> /sec-ha	ha	ha	Pe/ha	Person	m <sup>3</sup> /sec	m <sup>3</sup> /sec	m <sup>3</sup> /sec	mm	%	m/sec	m <sup>3</sup> /sec	M	M	M	m	
35	37	028	332	53	494							0.0014		0.0014	150	3.200	1.542	0.0272	4.730	4.5327	1.50		
36		025	025	60	60							0.0001		0.0001	150	3.650	1.649	0.0291	4.780	4.5127	1.50		
37	40-1	021	428	50	544							0.0016		0.0016	150	3.690	1.587	0.0280	4.560	4.3926	1.51		
38		089	089	116	116							0.0003		0.0003	150	3.650	1.715	0.0303	4.850	4.6328	1.50		
39		035	104	85	201							0.0004		0.0004	150	5.00	0.609	0.0108	4.390	4.2931	1.50		
40-1		139	2959	105	948							0.0111		0.0111	200	2.450	1.587	0.0514	4.390	4.1758	1.92		
40-2	44	000	2959	45	993							0.0111		0.0111	200	3.650	0.618	0.0194	4.090	3.9175	1.50		
41	43-1	095	095	137	137							0.0004		0.0004	150	3.650	1.577	0.0279	4.700	4.5331	1.50		
42		098	098	80	80							0.0004		0.0004	150	3.870	1.179	0.0208	4.830	4.6329	1.50		
43-1		221	414	170	307							0.0016		0.0016	150	3.530	1.631	0.0288	4.700	4.5326	1.51		
43-2		000	414	65	372							0.0016		0.0016	150	5.00	0.609	0.0106	4.690	3.9231	1.50		
44	MH 113	052	3435	100	1098							0.0129		0.0129	200	5.630	2.521	0.0792	4.090	3.8357	1.82		

# Hydraulic Calculation for Design of Sewers ( A r c a 2 )

P 18

0.00037450 m<sup>3</sup>/s-ha

No. of Sewers	Downstream Sewers No.	Drainage Area			Run-off Storm				Wastewater Flow			Other W. W.		Grand Total Design Flow	Designing of Sewers						Remarks			
		Area	Total	Length	Total	Concentrated Time	Rainfall	Run-off Coeff.	Arranged Area	Rainfall	Pop. Density	Person	Design Flow		Sewer	Total	Diameter	Slope	Velocity	Flow		Elevation	Invert Level	Earth Cover
		ha	ha	m	min	m <sup>3</sup> /sec-ha	ha	ha	ha	m <sup>3</sup> /sec	Per/ha	Person	m <sup>3</sup> /sec	m <sup>3</sup> /sec	m <sup>3</sup> /sec	mm	%	m/sec	m <sup>3</sup> /sec	M	M	M	m	
45		192	192	80	80								0.0007		0.0007	150	1.870	1.179	0.0208	4850	48229	150		
46		309	501	148	228								0.0019		0.0019	150	4.120	1.749	0.0309	4700	48330	150		
47	MH 115	402	903	200	428								0.0034		0.0034	150	3.170	1.934	0.0271	4690	38231	150		

## Hydraulic Calculation for Design of Sewers ( A r e a 2 )

: 0.00037450 m<sup>3</sup>/s-ha

No. of Sewers	Downstream Sewers No.	Drainage Area			Length			Concentrated Pipe			Run-off Storm					Wastewater Flow				Other W.W		Designing of Sewers													
		Area	Total	ha	Area	Total	m	Length	Total	m	Area	Total	ha	Run-off Coeff.	Rainfall	Area	Total	ha	Pop. Density	Person	Design Flow	Sewer	Total	Grand Total	Diameter	Slope	Velocity	Flow	Elevation	Invert Level	Earth Cover	Remarks			
		ha	ha	ha	ha	ha	m	m	min	m <sup>3</sup> /sec-ha	m <sup>3</sup> /sec	m <sup>3</sup> /sec-ha	ha	ha	ha	ha	ha	ha	ha	ha	Person	m <sup>3</sup> /sec	m <sup>3</sup> /sec	m <sup>3</sup> /sec	m	mm	%	m/sec	m <sup>3</sup> /sec	M	M	M			
48		163	163			150	150														0.0006			0.0006		150	3060	1508	0.0266	4240	40731	150			
49	MH 123	1359	1522			460	460														0.0057			0.0057		150	850	0.795	3730	35131	150				

# Hydraulic Calculation for Design of Sewers

A r e a 2

P 20

0.00037450 m<sup>3</sup>/s-ha

No. of Sewers	Downstream Sewers No.	Drainage Area			Length			Concentrated Time		Run-off Storm				Wastewater Flow				Other W. W				Designing of Sewers						Remarks												
		Area	Total	ha	Length	Total	m	m	min	Run-off Coeff.	Rainfall	Area	Total	ha	Rainfall	Pop. Density	Person	Design Flow	Sewer	Total	Grand Total	Design Flow	Diameter	Slope	Velocity	Flow	Elevation		Invert Level	Earth Cover										
																															Area	Total	ha	ha	ha	ha	m <sup>3</sup> /sec-ha	m <sup>3</sup> /sec	m <sup>3</sup> /sec	m <sup>3</sup> /sec
50	54	157	157	157	180	180	180										0.0006			0.0006	150	25.50	1.376	0.0243	5.160	4.9829	150													
51		240	240	240	88	88											0.0009			0.0009	150	17.00	1.124	0.0199	4.850	4.6829	150													
52	54	040	280	280	76	164											0.0010			0.0010	150	5.00	0.809	0.0108	4.700	4.5331	150													
53		107	107	107	110	110											0.0004			0.0004	150	5.00	0.809	0.0108	4.600	4.4331	150													
54	56	289	813	813	210	390											0.0030			0.0030	150	28.80	1.463	0.0258	4.700	4.5331	150													
55		175	175	175	128	128											0.0007			0.0007	150	23.40	1.318	0.0233	4.340	4.1729	150													
56	58	187	1155	1155	138	528											0.0043			0.0043	150	22.40	1.290	0.0228	3.640	3.4729	150													
57		219	219	219	190	190											0.0008			0.0008	150	7.80	0.761	0.0135	3.780	3.6129	150													
58	MH 133	075	1449	1449	100	628											0.0054			0.0054	150	32.90	1.563	0.0276	3.630	3.4631	150													

# Hydraulic Calculation for Design of Sewers

Area 2

P 21

3.00037450 m<sup>3</sup>/s-ha

No. of Sewers	Downstream Sewers No.	Drainage Area		Length		Concentrated Time		Run-off Storm			Wastewater Flow			Other W.W		Designing of Sewers						Remarks				
		Area	Total	Length	Total	min	Total	Run-off Coeff.	Area	Arranged Area	Rainfall	Pop Density	Population Sewer	Total	Design Flow	Sewer	Total	Grand Total Flow	Diameter	Slope	Velocity		Flow	Elevation	Invert Level	Earth Cover
59		131	131	110	110	110							0.0005				0.0005	0.0005	150	5.00	0.609	0.0108	5150	49331	150	
60		131	262	155	265								0.0010				0.0010	0.0010	150	16.00	1.104	0.0195	5150	49331	150	
61	65	140	402	135	400								0.0015				0.0015	0.0015	150	5.00	0.609	0.0108	4850	46231	150	
62	64	110	110	113	113								0.0004				0.0004	0.0004	150	39.80	1.719	0.0304	5610	54430	150	
63		058	058	80	80								0.0003				0.0003	0.0003	150	30.00	1.493	0.0264	5400	52331	150	
64		016	194	40	153								0.0007				0.0007	0.0007	150	37.50	1.669	0.0235	5180	49931	150	
65	67	294	890	250	650								0.0033				0.0033	0.0033	150	27.60	1.432	0.0253	5010	48431	150	
66		373	373	318	318								0.0014				0.0014	0.0014	150	41.10	1.747	0.0309	5400	51993	150	
67	70	150	1423	137	787								0.0053				0.0053	0.0053	150	22.00	1.278	0.0226	4090	38249	150	
68		490	490	350	350								0.0018				0.0018	0.0018	150	33.10	1.568	0.0277	5400	52331	150	
69		306	796	270	620								0.0030				0.0030	0.0030	150	20.30	1.228	0.0217	4240	40727	150	
70	MH 136	099	2318	100	887								0.0087				0.0087	0.0087	200	4.220	21.45	0.0574	3590	35175	150	

# Hydraulic Calculation for Design of Sewers

P 22

: 0.00037450 m<sup>3</sup>/s-ha

No. of Sewers	Downstream Sewers No.	Drainage Area		Length		Concentrated Time		Run-off Storm			Wastewater Flow		Other W.W.		Grand Total		Designing of Sewers						Remarks						
		Area	ha	Area	ha	mit	sec	Rainfall	Run-off	Area	ha	Area	ha	Pop. Density	Population	Design Flow	m <sup>3</sup> /sec	Sewer	m <sup>3</sup> /sec	Total	m <sup>3</sup> /sec	Design Flow		Diameter	Slope	Velocity	Flow	Elevation	Invert Level
71		1059	1059	360	360										0.0040						0.0040	150	3.210	15.44	0.0273	4700	4531	150	
72		000	1059	10	390										0.0040						0.0040	150	5.00	0.609	0.168	3480	3313	150	
73		126	126	100	100										0.0005						0.0005	150	4.600	19.48	0.0327	4090	3923	150	
74		264	390	60	180										0.0015						0.0015	150	2.500	13.63	0.0241	3580	3413	150	
75	MH 138	096	1555	270	660										0.0058						0.0058	150	8.40	0.790	0.140	3480	3301	150	



## **Appendix 6 Design Calculation of Kauma Treatment Plant**



Appendix 6 Design Calculation of Kauma Sewage Treatment Plant

1) Design Criteria

① Basic Items

Name : Kauma STP  
 Location : Area 44, Lilongwe City  
 Land Area : Approximately 40 ha  
 Elevation : Present Elevation 1,007~1,030m  
 Land Use : Crop Field, Uncultivated Land  
 Type of Sewer : Separate Type  
 Treatment Method : Sewage Treatment Stabilization Pond Method  
 (Anaerobic Pond ⇔ Facultative Pond ⇔ Maturation Pond)  
 Sludge Treatment Pond Drainage ⇔ Sun Drying ⇔ Dumping  
 Effluent Point : Effluent Point Lilongwe River  
 and Water Level : Recorded Highest Flood Water Level Unknown  
 Design Highest Water Level HHWL+1,007m  
 Target Year : 2000 (This Project), 2005 (Future Plan)  
 Lowest Monthly  
 Average Temperature : 15.2°C (July)

② Design Sewage Volume

	Future Plan(m <sup>3</sup> /day)	This Project(m <sup>3</sup> /day)
Dayly Maxmum Q <sub>1</sub>	15,600	6,100
Hourly Maxmum Q <sub>2</sub>	35,880	14,030

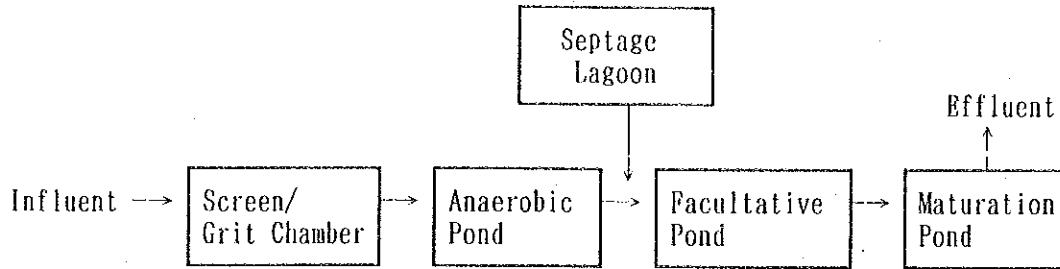
Design Population : Future Plan about 67,000 Persons  
 This Project about 22,000 Persons

③ Design Water Quality

	Influent	Effluent
BOD <sub>5</sub> (mg/l)	300	20
SS (mg/l)	350	30
Coliform (MPN/100ml)	2.0 × 10 <sup>7</sup>	1.0 × 10 <sup>3</sup>

\*Effluent water quality is target water quality

## 2) Flow Sheet



### • Volume of Sludge

Water content ratio of sedimented sludge is assumed at 97 %, and that of dumping sludge is 80 % after sun drying.

$$\text{Volumetric reducing ratio} = \frac{100-97}{100-80} = 0.15$$

## 3) Design Calculation

Items		Future Plan	This Project
<b>1. Grit Chamber</b>			
Type		Pallarel Flow Type	ditto
Hourly Maxmum Sewage Volume	Q <sub>2</sub>	35,880 m <sup>3</sup> /d = 0.415 m <sup>3</sup> /s	14,030 m <sup>3</sup> /d = 0.162 m <sup>3</sup> /s
Area Load	L <sub>g</sub>	1,800 m <sup>3</sup> /m <sup>2</sup> /d	ditto
Required Surface Area	A <sub>g</sub>	35,880 × 1/1,800 = 19.93 m <sup>2</sup>	14,030 × 1/1,800 = 7.79 m <sup>2</sup>
Average Velocity	v	0.3 m/sec	ditto
Effective Depth	h	About 0.6 m	ditto
Required Width	w	0.415 × 1/0.3 × 1/0.6 = 2.31 m → 2.40 m	0.162 × 1/0.3 × 1/0.6 = 0.90 m
Required Length	L	19.93 × 1/2.40 = 8.30 m	7.79 × 1/1.2 = 6.49 m
Dimension		1.2m W × 8.5m L × 2 Units	1.2m W × 8.5m L × 2 Units (1 unit stand-by)
Sedimentation Volume		0.01 m <sup>3</sup> for every 1,000 m <sup>3</sup> of sewage 15,600 × $\frac{0.01}{1,000}$ = 0.156 m <sup>3</sup> /d	ditto 6,100 × $\frac{0.01}{1,000}$ = 0.061 m <sup>3</sup> /d

(Check)			
Area Load	Lg	$35,880 \times 1 / (1.2 \times 8.5 \times 2)$ $= 1,759 \text{ m}^3 / \text{m}^2 / \text{d} \dots \text{OK}$	$14,030 \times 1 / (1.2 \times 8.5 \times 1)$ $= 1,375 \text{ m}^3 / \text{m}^2 / \text{d} \dots \text{OK}$
Average Velocity	v	$0.415 \times 1 / (0.6 \times 1.2 \times 2)$ $= 0.29 \text{ m/s} \dots \dots \text{OK}$	$0.162 \times 1 / (0.6 \times 1.2)$ $= 0.23 \text{ m/s} \dots \dots \text{OK}$
<b>2. Measuring Device</b>			
Type		Parshall Flume	ditto
Capacity		Hourly Max $35,880 = 1,495 \text{ m}^3/\text{h}$ $\rightarrow 1,800 \text{ m}^3/\text{h}$	Hourly Max $14,030 = 585 \text{ m}^3/\text{hr}$ $\rightarrow 1,800 \text{ m}^3/\text{hr}$
No. of unit		1 Unit	1 Unit
<b>3. Anaerobic Pond</b>			
Type		Embanked Rectangular Pond	ditto
BOD Load	Li	$15,600 \times 300 \times 10^{-3}$ $= 4,680 \text{ kgBOD}_5/\text{d}$	$6,100 \times 300 \times 10^{-3}$ $= 1,830 \text{ kgBOD}_5/\text{d}$
BOD Volumetric Load	v	Temperature $T_a = 15.2^\circ\text{C}$ $v = 160 \text{ gBOD}_5 / \text{m}^3 \cdot \text{d}$	ditto
Required Capacity	$V_1$	$4,680 \times 10^3 \times 1/160$ $= 29,250 \text{ m}^3$	$1,830 \times 10^3 \times 1/160$ $= 11,440 \text{ m}^3$
Sedimented Sludge Volume	$V_2$	Dumped once in every 5 year, $67,000 \text{ person} \times 0.04 \text{ m}^3/\text{person}/$ $\text{year} \times 5 \text{ years} = 13,400 \text{ m}^3$	ditto $22,000 \text{ person} \times 0.04 \text{ m}^3/\text{person}/$ $\text{year} \times 5 \text{ years} = 4,400 \text{ m}^3$
Dried Volume (Sun drying) 97w% → 80w%		$\frac{100-97}{100-80} \times 13,400 = 2,010 \text{ m}^3/$ 5 years	$\frac{100-97}{100-80} \times 4,400 = 660$ $\text{m}^3/5 \text{ years}$
Required Capacity	V	$V_1 + V_2$ $= 42,650 \text{ m}^3$	$V_1 + V_2$ $= 15,840 \text{ m}^3$
Depth	Da	4.0m	4.0m
Required Surface Area	Aa	$42,650 \times 1/4$ $= 10,663 \text{ m}^2$	$15,840 \times 1/4$ $= 3,960 \text{ m}^2$
Dimension		W (54 ~ 30)m × L (62 ~ 38)m × D 4.0m × 5 Units ( ) means length at surface /bottom.	W (54 ~ 30)m × L (62 ~ 38)m × D 4.0m × 3 Units (1 unit for stand-by)
(Check)			
Effective Capacity	$V_1$	$4.0 \times 1/3 \times (54 \times 62 + 30 \times 38$ $+ \sqrt{(54 \times 62 \cdot 30 \times 38)}) \times 5$ $- 13,400 = 29,545 \text{ m}^3$	$4.0 \times 1/3 \times (54 \times 62 + 30 \times 38$ $+ \sqrt{(54 \times 62 \cdot 30 \times 38)}) \times 2$ $- 4,400 = 12,778 \text{ m}^3$

BOD Volumetric Load	v	$4,680 \div 29,545 \times 10^3$ =158g-BOD <sup>5</sup> /m <sup>3</sup> /d.....OK	$1,830 \div 12,778 \times 10^3$ =143g-BOD <sup>5</sup> /m <sup>3</sup> /d.....OK
Effective Retention Days	Ra	$29,545 \div 15,600$ =1.9d .....OK	$12,778 \div 6,100$ =2.1d .....OK
Effluent BOD <sub>5</sub> Concentration	Le	$300 \times (1-0.5)$ =150 mg/ℓ	$300 \times (1-0.5)$ =150 mg/ℓ
<b>4. Septage Lagoon</b>			
Type		Embarked Rectangular Pond	ditto
Service Population		About 120,000 persons	About 119,000 persons ×60%(Collection Ratio) =71,400 ps
Sludge Volume		0.001 m <sup>3</sup> /person/day	ditto
BOD Concentration	L	5,000 mg/ℓ	ditto
Design Sludge Volume	S	$120,000 \times 0.001$ =120 m <sup>3</sup> /day	$71,400 \times 0.001$ =71m <sup>3</sup> /day
Required Capacity	Vs	①Retention days (>20days) $120 \times 20$ =2,400 m <sup>3</sup>  ②Volumetric Load (200g BOD/m <sup>3</sup> /day) $120 \times 5,000 \div 200$ =3,000 m <sup>3</sup> ①<②..... 3,000m <sup>3</sup>	① $71 \times 20$ =1,420 m <sup>3</sup>  ② $71 \times 5,000 \div 200$ =1,775 m <sup>3</sup> ①<②..... 1,775m <sup>3</sup>
Operation		Alternative Operation	ditto
Dimension/Capacity		W(28.2~10.2)m×L(42.2~24.2)m×D 3.0m×3 Units (1 unit shall always be left non-operational) $3.0 \times 1/3 \times (28.2 \times 42.2 + 10.2 \times 24.2 + \sqrt{(28.2 \times 42.2 \times 10.2 \times 24.2)}) \times 2 = 3,958\text{m}^3..OK$	W(28.2~10.2)m×L(42.2 ~24.2)m×D 3.0m×2 Units (1 unit shall always be left non-operational) $3.0 \times 1/3 \times (28.2 \times 42.2 + 10.2 \times 24.2 + \sqrt{(28.2 \times 42.2 \times 10.2 \times 24.2)}) = 1,979\text{m}^3..OK$
Effluent BOD Load		$120 \times 5,000 \times (1-0.5) \times 10^{-3}$ =300 kg-BOD <sub>5</sub> /d	$71 \times 5,000 \times (1-0.5) \times 10^{-3}$ =178 kg-BOD <sub>5</sub> /d
Sludge Dumping (Once per month)		Design SS volume SS 50 g/person/day×120,000p =6.0 ton/day Sedimentation rate 50 % 6.0 ton/day × 50 % =3.0 ton/day Water content of dry sludge 80%	Design SS volume SS 50 g/person/day×71,400 =3.6 ton/day Sedimentation rate 50 % 3.6 ton/day × 50 % =1.8 ton/day ditto

5. Facultative Pond

		$3.0 \text{ ton/day} \times \frac{100}{100-80}$ $= 15.0 \text{ ton/day}$ $= 15.0 \text{ m}^3/\text{day}$ $= 450 \text{ m}^3/\text{month}$	$1.8 \text{ ton/day} \times \frac{100}{100-80}$ $= 9 \text{ ton/day}$ $= 9 \text{ m}^3/\text{day}$ $= 270 \text{ m}^3/\text{month}$
Type		Embanked Rectangular Pond	ditto
Influent BOD Load		$15,600 \times 150 \times 10^{-3} + 300$ $= 2,640 \text{ kg-BOD}_5/\text{d}$	$6,100 \times 150 \times 10^{-3} + 178$ $= 1,093 \text{ kg-BOD}_5/\text{d}$
BOD Area Load		$s = 60.3 \times 1.0993^{T^a} \times 1/\alpha$ <p>Ta: Lowest Monthly Average Temperature (15.2°C)</p> <p><math>\alpha</math>: Safety Rate (Among 8 units 2 units are to be drained, 8/6 = 1.33)</p> $= 60.3 \times 1.0993^{15.2} \times 1/1.33$ $= 192 \text{ kg-BOD}_5/\text{ha/d}$	$s = 60.3 \times 1.0993^{T^a} \times 1/\alpha$ <p>ditto</p> <p><math>\alpha</math>: Safety Rate (Among 4 units 1 unit is to be drained, 4/3 = 1.33)</p> $= 60.3 \times 1.0993^{15.2} \times 1/1.33$ $= 192 \text{ kg-BOD}_5/\text{ha/d}$
Required Surface Area		$2,640 \times 1/192$ $= 13.8 \text{ ha}$	$1,093 \times 1/192$ $= 5.7 \text{ ha}$
Depth	Df	1.5m	1.5m
Dimension		W (105~96)m $\times$ L (144~135)m $\times$ D 1.5m $\times$ 4 Units $+ 2.1 \text{ ha} \times$ D 1.5m $\times$ 4 Units	W (105~96)m $\times$ L (144~135)m $\times$ D 1.5m $\times$ 4 Units
(Check) Capacity	V1	$1.5 \times 1/3 \times (105 \times 144 + 96 \times 135 + \sqrt{(105 \times 144 \times 96 \times 135)}) \times 4 + 21,000 \times 1.5 \times 4$ $= 210,157 \text{ m}^3$	$1.5 \times 1/3 \times (105 \times 144 + 96 \times 135 + \sqrt{(105 \times 144 \times 96 \times 135)}) \times 4 = 84,157 \text{ m}^3$
Surface Area		$210,157 \div 1.5 = 14.01 \text{ ha}$ $> 13.8 \text{ ha}$	$84,157 \div 1.5 = 5.61 \text{ ha}$ $\approx 5.7 \text{ ha OK}$

Retention Days	Rf	$210,964 \div 15,600$ =13.5d	$84,157 \div 6,100$ =13.8d
Area Load against Influent BOD <sub>5</sub> Load	s	Surface Area $1.05 \times 1.44 \times 4 + 2.1 \times 4$ =14.45 ha $2,640 \times 1/14.45$ =183 kg-BOD <sub>5</sub> /ha/d..... OK	$1.05 \times 1.44 \times 4$ = 6.05 ha $1,093 \times 1/6.05$ =181 kg-BOD <sub>5</sub> /ha/d.... OK
Volume of Sludge	V <sub>2</sub>	Dumped once in every 5 years $67,000 \text{人} \times 0.03 \text{m}^3/\text{person}/\text{year}$ $\times 5 \text{ years}$ =10,050 m <sup>3</sup>	$22,000 \text{人} \times 0.03 \text{m}^3/\text{person}/\text{year}$ $\times 5 \text{ years} = 3,300 \text{m}^3$
Dumping Sludge Volume		$10,050 \times 0.15 = 1,508 \text{m}^3/5\text{years}$	$3,300 \times 0.15 = 495 \text{m}^3/5\text{years}$
Depth of Sedimented Sludge	Ds	$10,050 \times 1/140,100$ =0.072 m	$3,300 \times 1/56,100$ =0.059 m
<b>6. Maturation Pond</b>			
Type		Embanked Rectangular Pond	ditto
Retention Days	Rm	3 days/Pond	3 days/Pond
No. of Units	N	3 units/1 series $\times$ 4 series =12 units	3 units/1 series $\times$ 2 series = 6 units
Check of Coliform Bacteria Number contained in effluent	NR	$NR = \frac{No}{(K Ra + 1)(K' Rf + 1)(K' Rm + 1)^n}$ <p>where: No: Coliform Bacteria Number contained in Influent (<math>2.0 \times 10^7/100 \text{ ml}</math>) k': Death Constant k' at 15.2 = <math>2.0 \times 1.07^{(15.2 - 20)}</math> n = 3 (number of units)</p>	



		NR	NR
		$= \frac{2.0 \times 10^7}{(1.446 \times 1.9 + 1) \times (1.446 \times 13.5 + 1) \times (1.446 \times 3 + 1)^3}$ $\approx 1 \times 10^3 \dots \text{OK}$	$= \frac{2.0 \times 10^7}{(1.446 \times 2.1 + 1) \times (1.446 \times 13.8 + 1) \times (1.446 \times 3 + 1)^3}$ $\approx 1 \times 10^3 \dots \text{OK}$
Influent volume per series	q	$6,100 \times 1/2 = 3,050 \text{ m}^3/\text{day}$ $9,500 \times 1/2 = 4,750 \text{ m}^3/\text{day}$	$6,100 \times 1/2 = 3,050 \text{ m}^3/\text{day}$
Required Capacity per unit	v	$3,050 \times 3 = 9,150 \text{ m}^3/\text{unit}$ $4,750 \times 3 = 14,250 \text{ m}^3/\text{unit}$	$3,050 \times 3 = 9,150 \text{ m}^3/\text{unit}$
Depth	Dm	1.5m	1.5m
Dimension		W (72~63)m × L (95~86)m × D 1.5m × 3 units/1 series × 2 series + 14,250 m <sup>3</sup> × 3 units/1 series × 2 series	W (72~63)m × L (95~86)m × D 1.5m × 3 units/1 series × 2 series
(Check) Capacity		$1.5 \times 1/3 \times (72 \times 95 + 63 \times 86 + \sqrt{(72 \times 95 \cdot 63 \times 86)}) \times 6 + 14,250 \times 6$ = 140,540 m <sup>3</sup>	$1.5 \times 1/3 \times (72 \times 95 + 63 \times 86 + \sqrt{(72 \times 95 \cdot 63 \times 86)}) \times 6$ = 55,040 m <sup>3</sup>
Retention Time		$140,540 \div 15,600 = 9.01$ $9.01 \div 3 = 3.0 \geq 3.0$	$55,040 \div 6,100 = 9.02$ $9.02 \div 3 = 3.0 \geq 3.0$

