

Unit : m<sup>3</sup>/s-d

Year	1986											
Date	JAN.	FEB.	MAR.	APR.	MAY	JUN.	JUL.	AUG.	SEP.	OCT.	NOV.	DEC.
1	5.01	3.97	2.21	2.42	3.97	11.11	11.91	16.00	37.00	23.20	26.20	9.92
2	4.73	3.97	2.42	2.21	3.97	10.32	11.91	13.70	32.50	25.00	25.00	9.53
3	4.73	3.97	2.42	2.62	4.73	9.92	12.31	13.70	29.20	20.80	24.40	9.53
4	4.73	3.97	2.42	2.04	4.45	8.53	12.31	13.70	31.00	19.00	23.00	9.53
5	4.73	3.97	2.42	2.04	5.28	9.53	11.91	14.90	34.70	19.00	22.00	9.13
6	4.73	3.97	2.42	1.87	5.01	9.13	11.91	20.20	37.00	18.40	21.40	9.13
7	4.73	3.72	2.42	1.87	6.15	9.53	11.91	16.00	44.50	17.20	20.80	9.13
8	4.45	3.72	2.42	1.87	6.15	9.13	11.91	16.60	45.20	16.60	19.60	9.13
9	4.45	3.72	2.42	2.04	6.46	8.73	11.91	22.00	41.50	16.00	19.00	8.78
10	4.21	3.43	2.21	3.24	7.09	8.44	11.91	26.20	41.50	14.90	18.41	8.14
11	4.21	3.03	2.62	2.83	7.40	8.78	11.91	34.00	38.50	14.90	17.82	8.09
12	4.21	2.83	2.42	2.21	7.75	10.72	11.11	37.80	37.80	14.90	18.41	8.09
13	4.21	2.83	2.21	2.42	8.44	13.69	9.13	38.50	45.20	14.90	17.32	8.09
14	4.21	2.83	2.21	2.62	8.78	12.70	8.78	54.50	40.00	20.80	17.23	7.40
15	4.21	2.83	2.21	2.83	8.78	10.72	11.51	128.00	35.50	18.40	17.32	7.40
16	3.97	2.83	2.04	2.83	9.92	11.51	13.10	58.30	28.60	17.20	16.05	7.40
17	3.97	2.83	2.04	3.03	11.11	11.91	15.45	43.00	28.60	16.00	14.37	7.40
18	3.97	2.83	2.62	3.48	10.32	11.91	16.54	39.30	28.00	17.20	14.28	6.78
19	3.97	2.83	2.62	3.97	16.05	11.91	19.60	37.00	28.20	17.80	14.28	6.46
20	3.97	2.83	2.83	4.21	19.00	12.70	19.60	34.00	25.60	14.90	13.59	6.15
21	3.97	2.83	3.72	5.01	13.10	13.69	12.31	30.40	22.00	14.30	13.10	5.84
22	3.97	2.83	3.03	5.56	11.91	11.11	11.91	32.50	22.00	54.50	12.70	5.56
23	3.97	2.83	2.62	5.28	11.91	10.32	12.31	31.80	22.00	26.80	13.40	5.23
24	3.97	2.83	2.62	5.01	13.69	12.31	13.59	29.20	21.40	26.80	11.91	5.01
25	3.97	2.83	3.03	5.46	13.10	12.70	13.59	34.00	22.60	26.80	11.51	5.91
26	3.97	2.83	3.03	6.46	12.31	13.40	14.23	32.50	23.20	26.80	10.32	5.56
27	3.97	2.62	2.62	5.84	11.91	13.63	14.28	29.20	22.00	26.80	9.92	5.84
28	3.97	2.42	2.62	5.01	9.92	14.23	17.28	37.80	21.40	27.40	9.92	5.84
29	3.97		2.42	4.45	10.72	14.23	16.64	35.50	22.00	29.20	9.92	5.84
30	3.97		2.42	3.72	11.11	12.70	16.05	45.20	23.20	28.60	9.92	5.84
31	3.97		2.42		11.11		16.64	38.50		26.80		5.56
TOTAL	131.07	88.93	78.15	104.45	291.60	339.24	415.46	1054.00	931.90	671.90	493.10	227.24
AVE.	4.23	3.18	2.52	3.48	9.41	11.31	13.40	34.00	31.06	21.67	16.44	7.33
MAX.	5.01	3.97	3.72	6.46	19.00	14.23	19.60	128.00	45.20	54.50	26.20	9.92
MIN.	3.97	2.42	2.04	1.87	3.97	8.44	8.78	13.70	21.40	14.30	9.92	5.01
DAYS	31	28	31	30	31	30	31	31	30	31	30	31
ANNUAL TOTAL			4827.0									
ANNUAL AVERAGE			13.22									
ANNUAL MAX.			128.00									
ANNUAL MIN.			1.87									
ANNUAL DAYS			365									

Unit : m<sup>3</sup>/s-d

YEAR	1989											
Date	JAN.	FEB.	MAR.	APR.	MAY	JUN.	JUL.	AUG.	SEP.	OCT.	NOV.	DEC.
1					4.44	26.44		22.42	24.68	24.10		
2					4.44	26.44	13.96	20.26	50.00	22.97		
3	4.44	3.46	2.40	2.59	6.43	20.26		31.40	39.58	21.87	10.63	6.75
4					4.44	17.71		51.00	35.38	30.12		
5					19.74	20.26		68.00	43.00	31.40		
6					9.12	20.26		51.00	100.00			
7					5.83	17.71	14.86	32.05	88.50			
8	3.94	2.80	2.02	2.02	13.96	17.71		31.40	75.04	19.74	10.24	6.43
9					6.43	15.78		31.40	38.16			
10					11.42	68.50		28.87	74.08			
11					29.49	25.84		62.95	38.16	18.21		
12					6.13	25.26	13.09	75.04	34.04	17.71		
13	4.44	2.80	2.02	2.02	22.97	22.42		51.00	38.16	46.30	9.86	5.54
14					13.96	21.33		43.00	50.00	24.10		
15					7.07	18.71		28.87	31.40	19.74		
16								34.04				
17								32.05				
18	3.94	2.59	2.20	3.23	9.12	15.32	13.52	60.30	33.37	17.71	8.41	5.26
19								65.65				
20								31.04				
21							28.87	43.00				
22	3.70	2.40	2.20	3.94	8.41	16.73	13.09	28.87	28.87	13.09	7.39	4.98
23							185.00	35.38				
24							67.00	28.87				
25							46.00	28.87				
26					7.39	20.79	37.00					
27					5.83	14.86	35.00	24.68	26.44	11.02	7.39	4.71
28	3.46	2.40	1.84	4.98	123.00	18.71	28.87					
29					20.26	18.71	28.87					
30					18.21	13.96	31.40					
31	3.46	2.40	1.84	4.98	22.97	13.96	28.87	32.71	25.26	10.24	7.07	4.44

TOTAL	27.38	18.85	14.52	23.76	381.06	497.67	585.40	1074.12	874.12	328.32	60.99	38.11
AVE.	3.91	2.69	2.07	3.39	16.57	21.64	39.03	39.78	46.01	21.89	8.71	5.44
MAX.	4.44	3.46	2.40	4.98	123.00	68.50	185.00	75.04	100.00	46.30	10.63	6.75
MIN.	3.46	2.40	1.84	2.02	4.44	13.96	13.09	20.26	24.68	10.24	7.07	4.44
DAYS	31	28	31	30	31	30	31	31	30	31	30	31

ANNUAL TOTAL	3924.30
ANNUAL AVERAGE	17.59
ANNUAL MAX.	185.00
ANNUAL MIN.	1.84
ANNUAL DAYS	365

Unit : m<sup>3</sup>/s-d

YEAR	1990											
Date	JAN.	FEB.	MAR.	APR.	MAY	JUN.	JUL.	AUG.	SEP.	OCT.	NOV.	DEC.
1	4.19	2.59	1.51	3.46		3.01	5.54	20.26	17.71	28.87	36.00	13.96
2												
3										80.00		
4			1.84	1.68		5.83		17.22		43.00		
5						3.23						
6												
7												
8									22.42			
9	3.70		4.71					17.71		34.71	22.97	
10			3.46					13.52				
11		1.68	3.23				6.13	11.02				13.09
12			1.84									
13						5.83	8.76			29.49		
14	3.46						9.12					
15							13.09			34.71	15.78	
16				3.70			15.32					
17						8.76	42.00			29.49		
18			1.51				15.32					
19						15.32						
20									43.00		13.52	
21						8.76				36.00		
22	2.80	1.51				5.83		43.00				
23						9.12		20.26				
24						12.24						
25						7.39			29.49			
26			3.23			5.83		15.32				
27				3.23								
28							17.71	22.97				
29												
30								17.71			13.96	
31												

TOTAL	14.15	5.78	21.33	12.07	0.00	91.15	132.99	198.99	112.62	316.27	102.23	27.05
AVE.	3.54	1.93	2.67	3.02		7.60	14.78	19.90	28.16	39.53	20.45	13.53
MAX.	4.19	2.59	4.71	3.70		15.32	42.00	43.00	43.00	80.00	36.00	13.96
MIN.	2.80	1.51	1.51	1.68		3.01	5.54	11.02	17.71	28.87	13.52	13.09
DAYS	31	28	31	30	31	30	31	31	30	31	30	31

ANNUAL TOTAL	1034.63
ANNUAL AVERAGE	14.10
ANNUAL MAX.	80.00
ANNUAL MIN.	1.51
ANNUAL DAYS	365

**A3.5.2 Rainfall Record for TANK MODEL Analysis**

**Daily Rainfall Record at Nonghin Station (1980-1990)**

Unit : mm

1980	1	2	3	4	5	6	7	8	9	10	11	12
1	0.0	0.0	1.0	0.0	0.0	58.0	0.0	29.4	2.1	2.0	0.0	0.0
2	0.0	0.0	0.0	8.6	0.0	3.1	4.8	0.0	10.7	0.0	4.0	0.0
3	0.0	0.0	0.0	0.0	8.8	2.0	16.0	0.0	2.7	0.0	6.1	0.0
4	0.0	0.0	0.0	25.0	12.2	0.0	14.9	0.0	57.3	0.0	0.0	0.0
5	0.0	0.0	0.0	7.2	6.3	0.0	12.4	0.0	11.9	1.1	0.0	2.2
6	0.0	6.0	0.0	4.2	18.0	0.0	2.8	12.9	3.3	13.1	0.0	0.0
7	0.0	0.0	0.0	11.0	1.8	29.0	14.8	0.0	1.3	0.9	0.0	0.0
8	0.0	0.0	0.0	6.8	0.0	34.5	0.1	0.0	0.0	4.1	0.0	0.0
9	0.0	0.0	0.0	0.0	0.0	1.8	0.0	0.0	0.0	13.2	0.0	0.0
10	0.0	0.0	0.0	0.0	37.8	27.6	0.4	7.2	0.0	0.0	0.0	0.0
11	0.0	0.0	0.0	0.0	0.0	15.4	0.2	7.0	15.1	26.4	0.0	0.0
12	0.0	0.0	0.0	7.2	0.0	0.6	0.3	10.0	0.0	0.0	0.0	0.0
13	0.0	0.0	22.0	0.0	46.0	1.0	0.1	10.6	10.4	0.2	0.0	0.0
14	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	2.7	0.0	0.0
15	0.0	0.0	0.0	0.0	18.2	47.8	0.0	21.2	20.2	3.9	0.0	0.0
16	0.0	0.0	0.0	48.6	134.8	1.4	10.1	26.1	42.0	4.0	4.0	0.0
17	0.0	0.0	0.0	0.0	59.0	16.2	0.0	0.0	5.4	2.3	9.0	0.0
18	0.0	0.0	4.0	0.0	0.0	7.0	10.4	0.0	35.0	2.1	0.6	0.0
19	0.0	0.0	0.0	0.0	82.4	6.5	10.4	13.4	6.0	9.2	0.0	0.0
20	0.0	0.0	31.6	0.0	0.0	6.8	13.4	0.0	2.4	0.0	0.0	0.0
21	0.0	0.8	5.0	0.0	0.0	0.2	67.8	24.2	16.4	0.0	0.0	0.0
22	0.4	27.2	8.6	0.0	0.0	39.2	57.6	15.4	41.0	0.0	0.0	0.0
23	0.0	0.0	34.2	0.0	0.0	4.0	49.9	12.4	5.1	8.2	0.0	0.0
24	0.0	0.0	0.0	0.0	0.0	16.4	1.5	3.2	0.0	41.4	0.0	0.0
25	0.0	0.0	0.0	0.0	0.0	3.0	0.2	95.6	0.0	0.0	0.0	0.0
26	0.2	0.0	0.0	36.0	0.0	73.4	4.4	3.9	0.0	1.2	0.0	0.0
27	0.0	0.0	0.0	0.0	0.0	11.3	10.8	10.1	20.0	22.6	0.0	0.0
28	0.0	0.0	0.0	0.0	31.4	24.8	0.1	11.8	4.0	5.1	0.0	0.0
29	0.0	0.0	0.0	0.0	4.0	10.8	18.0	36.0	30.8	9.9	0.0	0.0
30	0.0	0.0	0.0	0.0	0.0	0.1	1.1	14.5	27.3	4.9	0.0	0.0
31	0.0	0.0	0.0	0.0	0.0	0.0	0.0	8.0	0.0	0.0	0.0	0.0
TOTAL	0.6	34.0	113.6	148.2	582.1	450.7	322.5	372.9	370.4	178.5	23.7	2.2
AVERAGE	0.0	1.2	3.7	4.9	16.2	15.0	10.4	12.0	12.3	5.8	0.8	0.1
MAXIMUM	0.4	27.2	34.2	48.6	134.8	73.4	67.8	95.6	57.3	41.4	9.0	2.2
MINIMUM	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

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 ANNUAL  
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 TOTAL AVERAGE MAXIMUM MINIMUM  
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 2519.4 6.9 134.8 0.0  
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NONWINDY PRECIPITATION

1981

DAY	1	2	3	4	5	6	7	8	9	10	11	12
1	0.0	0.0	0.0	0.0	0.0	4.8	40.2	49.6	52.2	61.2	15.6	0.0
2	0.0	0.0	16.1	15.6	0.0	40.9	4.9	5.7	0.0	4.2	0.0	0.0
3	0.0	0.0	3.2	51.3	34.2	1.2	20.5	46.4	0.0	15.5	0.0	0.0
4	0.0	0.0	0.0	29.2	2.1	1.3	83.1	18.6	0.0	9.7	17.0	0.0
5	0.0	0.0	12.9	6.0	6.0	12.5	19.7	28.7	0.0	16.8	0.0	0.0
6	0.0	0.0	0.0	25.0	23.0	21.4	5.2	25.9	0.0	0.5	1.0	0.0
7	0.0	0.0	0.0	0.0	32.2	33.6	0.4	81.2	0.0	0.0	0.0	0.0
8	0.0	0.0	0.0	0.0	0.0	33.4	10.8	92.4	35.1	51.2	0.0	0.0
9	0.0	0.0	36.0	0.0	4.8	20.7	44.5	33.9	3.0	0.6	0.0	0.0
10	0.0	0.0	13.6	0.0	0.0	65.3	0.2	2.5	29.0	5.8	0.0	0.7
11	0.0	0.0	11.8	2.6	6.4	40.0	5.2	1.8	14.5	0.0	0.0	0.0
12	0.0	0.0	23.5	4.2	0.0	40.3	15.9	13.2	0.0	0.0	0.0	0.0
13	0.0	0.0	47.4	6.3	19.4	73.5	24.8	34.4	0.0	6.2	3.0	0.0
14	0.0	0.0	1.5	16.4	16.4	36.7	28.3	12.7	0.0	10.5	0.0	0.0
15	0.0	0.0	0.0	0.0	33.6	30.2	0.4	3.9	0.0	15.8	1.8	0.0
16	0.0	0.0	0.0	0.0	0.0	20.2	1.2	34.5	0.0	34.1	2.0	0.0
17	0.0	0.0	0.0	0.0	0.0	27.9	2.3	25.4	0.0	0.0	0.0	0.0
18	0.0	0.0	0.0	0.0	34.4	31.5	6.3	40.9	18.1	0.0	0.0	0.0
19	0.0	0.0	0.0	30.5	60.2	14.9	12.0	33.6	0.0	6.0	0.0	0.0
20	0.0	5.2	0.0	18.7	10.2	25.9	56.9	3.5	10.6	0.0	0.0	0.0
21	0.0	0.0	0.0	0.0	34.4	60.1	15.5	0.0	0.0	1.6	0.0	0.0
22	0.0	0.0	0.0	0.0	57.5	30.7	11.9	2.6	0.0	0.0	0.0	0.0
23	0.0	0.0	0.0	0.0	0.0	6.2	9.6	0.9	12.7	0.0	8.8	0.0
24	0.0	4.4	0.0	13.0	0.0	0.0	10.1	0.0	0.0	0.0	0.0	0.0
25	0.0	6.0	4.0	15.9	15.5	0.0	0.0	25.1	0.0	0.0	0.0	0.0
26	0.0	85.2	3.7	0.0	16.5	0.0	2.0	12.9	0.0	0.0	0.0	0.0
27	0.0	46.6	0.4	0.0	0.0	11.8	13.7	0.0	14.6	0.0	0.0	0.0
28	5.8	22.0	0.0	14.6	0.0	6.4	1.3	12.8	0.0	0.0	0.0	0.0
29	2.3	0.0	0.0	30.9	0.5	0.5	3.7	10.3	3.5	0.0	0.0	0.0
30	0.0	0.0	0.0	21.9	0.0	0.0	0.6	13.5	5.2	3.5	0.0	0.0
31	0.0	0.0	4.2	0.0	0.0	0.0	0.5	5.7	0.0	0.0	0.0	0.0
TOTAL	8.1	169.4	178.3	285.7	407.3	61.9	463.7	705.6	193.3	243.2	49.8	0.7
AVERAGE	0.3	6.1	5.8	9.5	13.1	23.1	15.0	22.8	6.4	7.8	1.7	0.0
MAXIMUM	5.8	85.2	47.4	51.3	60.2	73.5	83.1	92.4	35.1	61.2	17.0	0.7
MINIMUM	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

ANNUAL  
 TOTAL AVERAGE MAXIMUM MINIMUM  
 3397.0 9.5 92.4 0.0

1982

NONSHINE PRECIPITATION

#DAY*	1	2	3	4	5	6	7	8	9	10	11	12
1	0.0	0.0	16.1	4.6	0.0	0.0	4.8	0.0	71.0	9.8	0.0	43.2
2	0.0	0.0	0.0	0.0	0.0	0.0	1.7	0.0	43.2	5.4	0.0	0.0
3	0.0	0.0	6.8	7.9	46.1	4.7	7.3	12.8	6.6	2.6	0.0	0.0
4	0.0	0.0	0.0	16.2	0.0	0.0	110.0	4.6	11.4	0.0	0.0	0.0
5	0.0	0.0	17.2	2.5	0.0	17.4	30.5	30.2	21.9	0.0	0.0	0.0
6	0.0	0.0	0.0	0.0	0.0	0.0	7.6	5.0	108.7	0.0	11.2	0.0
7	0.0	0.0	26.1	0.0	0.0	5.9	0.0	17.8	8.2	0.0	40.0	0.0
8	0.0	0.0	7.5	18.9	0.0	3.3	20.8	16.8	17.8	0.0	21.5	0.0
9	0.0	0.0	30.2	0.0	0.0	17.5	1.5	14.4	2.5	0.0	0.0	0.0
10	0.0	0.0	5.4	0.0	0.0	25.8	0.0	53.2	26.2	10.2	18.5	0.0
11	0.0	0.0	0.0	15.8	0.0	3.0	4.4	12.5	2.2	2.9	0.0	0.0
12	0.0	0.0	0.0	38.2	1.2	2.6	0.0	36.8	19.2	5.0	8.4	0.0
13	0.0	0.0	0.0	25.0	0.0	8.7	3.3	1.0	0.0	0.0	33.2	0.0
14	0.0	0.0	0.0	7.8	0.0	3.3	76.2	2.3	0.0	0.0	5.8	0.0
15	0.0	0.0	0.0	0.0	0.0	2.4	3.2	8.1	0.0	0.0	48.8	0.0
16	0.0	0.0	0.0	0.0	15.3	3.4	50.4	15.1	0.0	0.0	22.5	0.0
17	0.0	0.0	0.0	0.0	8.5	3.2	4.5	15.0	18.4	0.0	14.4	0.0
18	0.0	0.0	0.0	54.2	3.7	3.0	37.9	15.7	0.0	0.0	0.0	0.0
19	0.0	0.0	0.0	9.3	4.6	21.2	3.2	22.8	0.0	13.1	10.4	0.0
20	0.0	0.0	0.0	0.4	3.8	0.4	15.3	60.4	0.0	7.2	0.0	0.0
21	0.0	1.7	0.0	0.4	63.5	0.0	17.6	17.9	12.6	0.0	13.1	0.0
22	0.0	2.1	0.0	0.0	0.0	6.8	0.0	9.3	2.4	0.0	0.0	0.0
23	0.0	30.1	0.0	5.1	0.0	6.2	2.2	10.5	11.4	0.0	0.0	0.0
24	0.0	5.9	0.0	10.6	0.0	20.8	39.5	4.2	9.2	10.2	18.3	0.0
25	0.0	0.0	0.0	0.0	8.9	33.9	16.2	3.7	1.3	2.1	20.6	0.0
26	0.0	0.0	20.4	0.0	0.0	60.5	87.2	0.0	2.9	6.2	0.0	0.0
27	0.0	2.7	5.5	1.2	0.0	29.5	36.5	3.7	0.0	5.1	15.8	0.0
28	0.0	0.0	0.0	0.0	49.8	48.8	14.1	13.7	54.2	0.0	0.0	0.0
29	0.0	0.0	0.0	0.0	0.0	15.5	3.1	26.4	5.7	0.0	19.8	0.0
30	0.0	0.0	5.2	0.0	0.0	9.4	16.3	6.4	23.2	0.0	0.0	0.0
31	0.0	0.0	4.0	0.0	0.0	7.6	7.6	2.4	2.4	0.0	0.0	0.0
TOTAL	0.0	42.5	144.4	218.1	205.4	377.2	622.9	442.6	480.2	93.2	322.3	43.2
AVERAGE	0.0	1.5	4.7	7.3	6.6	12.6	20.1	14.3	16.0	3.0	10.7	1.4
MAXIMUM	0.0	30.1	30.2	54.2	63.5	68.8	110.0	60.4	108.7	13.1	48.8	43.2
MINIMUM	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

\*\*\*\*\* ANNUAL \*\*\*\*\*  
 \*\*\*\*\* TOTAL AVERAGE MAXIMUM MINIMUM \*\*\*\*\*  
 \*\*\*\*\* 2992.0 8.2 110.0 0.0 \*\*\*\*\*

1983

NONSHINE PRECIPITATION

DAY*	1	2	3	4	5	6	7	8	9	10	11	12
1	0.0	0.0	0.0	0.0	2.8	0.0	7.4	19.0	29.1	31.1	2.3	0.0
2	9.3	0.0	0.0	23.4	4.2	0.0	0.0	65.0	0.0	29.1	0.0	0.0
3	0.0	0.0	0.0	0.0	105.8	0.0	0.0	0.0	11.8	26.1	0.0	0.0
4	0.0	0.0	0.0	0.0	1.0	0.0	0.0	21.6	0.0	10.3	9.1	0.0
5	0.0	0.0	0.0	0.0	56.7	0.0	6.2	0.0	0.0	4.1	1.8	0.0
6	0.0	0.0	0.0	0.0	3.8	17.8	6.4	0.0	72.6	6.4	0.0	0.0
7	0.0	0.0	0.0	0.0	66.5	23.2	12.4	0.0	12.2	12.0	0.0	0.0
8	0.0	0.0	0.0	0.0	3.0	51.8	3.4	17.8	0.0	18.0	0.0	0.0
9	0.0	0.0	0.0	0.0	16.8	0.0	0.0	0.0	0.0	10.6	0.0	0.0
10	0.0	0.0	38.2	0.0	95.5	0.0	0.0	0.0	0.0	0.0	0.0	5.3
11	0.0	0.0	0.0	0.0	0.0	0.0	0.0	14.2	0.0	4.5	0.0	1.2
12	0.0	0.0	0.0	0.0	0.0	49.8	7.8	0.0	0.0	1.6	10.3	0.3
13	0.0	0.0	0.0	0.0	0.0	79.7	0.0	5.2	0.0	56.0	0.0	0.0
14	0.0	0.0	0.0	0.0	0.0	18.8	0.0	55.2	0.0	0.0	0.0	0.0
15	0.0	0.0	0.0	6.0	1.2	0.0	0.0	59.6	0.0	0.0	0.0	0.0
16	0.0	0.0	0.0	10.4	50.1	2.1	19.8	0.0	0.0	0.0	0.0	0.0
17	0.0	0.0	0.0	7.0	1.8	0.0	27.3	17.0	3.0	0.0	0.0	0.0
18	0.0	0.0	12.4	0.0	0.0	2.2	42.3	4.5	5.8	13.4	0.0	0.0
19	0.0	0.0	0.0	0.0	0.0	0.0	0.0	4.2	6.0	0.0	0.0	0.0
20	0.0	0.0	0.0	0.0	0.0	0.0	37.8	1.3	23.6	30.2	0.0	0.0
21	0.0	0.0	0.0	12.6	0.0	0.0	27.2	8.4	0.0	0.0	0.0	0.0
22	0.0	0.0	0.0	16.4	0.0	0.0	8.9	5.2	2.4	0.0	0.0	0.0
23	0.0	0.0	0.0	0.0	0.0	14.8	1.1	64.2	1.1	0.0	2.4	0.0
24	0.0	0.0	0.0	0.0	0.0	17.2	1.4	18.6	33.8	0.0	0.0	1.5
25	0.0	0.0	0.0	66.8	0.0	178.0	2.5	15.2	44.8	18.3	0.0	0.0
26	0.0	0.0	0.0	55.1	0.0	116.3	5.1	7.3	14.6	14.8	0.0	0.0
27	0.0	0.0	0.0	0.0	45.0	14.2	2.2	3.1	43.2	0.0	0.0	0.0
28	0.0	0.0	0.0	0.0	5.1	0.0	0.0	0.0	50.7	8.5	0.0	0.0
29	0.0	0.0	0.0	9.1	9.7	0.0	0.0	0.0	39.5	0.0	0.0	0.0
30	0.0	0.0	0.0	35.2	11.2	0.0	0.0	22.2	26.4	0.0	0.0	0.0
31	0.0	0.0	19.3	0.0	3.4	0.0	2.3	4.8	0.0	9.8	0.0	0.0
TOTAL	9.3	0.0	69.9	242.0	480.6	585.9	219.5	442.5	420.6	309.8	34.0	8.3
AVERAGE	0.3	0.0	2.3	8.1	15.5	19.5	7.1	14.3	14.0	10.0	1.1	0.3
MAXIMUM	9.3	0.0	38.2	66.8	105.8	178.0	42.3	65.0	72.6	56.0	10.3	5.3
MINIMUM	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

ANNUAL  
 TOTAL AVERAGE MAXIMUM MINIMUM  
 2822.4 7.7 178.0 0.0

1984

NONHGHINE PRECIPITATION

DAY*	1	2	3	4	5	6	7	8	9	10	11	12
1	0.0	0.0	0.0	10.5	1.9	6.9	3.4	66.6	99.5	6.3	0.0	0.0
2	0.0	0.0	5.2	8.2	3.2	0.0	0.0	75.8	64.5	9.0	2.1	0.0
3	0.0	0.0	0.0	3.4	29.1	25.0	10.3	15.5	3.3	2.4	15.3	0.0
4	0.0	0.0	0.0	0.0	0.0	6.3	5.9	7.0	13.5	12.5	0.0	0.0
5	0.0	2.3	0.0	0.0	0.0	0.0	0.0	13.5	0.0	45.9	0.0	0.0
6	0.0	0.0	0.0	27.5	0.0	14.6	2.3	5.2	82.5	26.5	0.0	0.0
7	0.0	0.0	10.9	35.2	0.0	7.1	2.4	4.3	2.2	41.2	3.4	0.0
8	0.0	0.0	23.8	17.3	0.0	9.5	22.0	14.0	1.9	9.9	54.3	0.0
9	0.0	0.0	2.2	17.3	0.0	6.0	2.0	9.1	27.3	5.0	3.8	0.0
10	0.0	0.0	0.0	12.8	24.3	4.5	0.0	26.1	16.9	29.8	13.5	0.0
11	0.0	12.3	0.0	0.0	24.6	30.0	3.0	31.2	68.4	5.2	0.0	0.0
12	0.0	0.0	0.0	0.0	0.0	23.9	23.0	27.3	16.1	0.2	33.1	0.0
13	0.0	0.0	0.0	0.0	0.0	0.0	0.0	9.0	41.3	62.8	44.2	0.0
14	0.0	26.9	0.0	0.0	0.0	4.8	0.0	83.9	15.8	8.9	35.1	0.0
15	0.0	10.1	111.0	0.0	0.0	3.2	0.0	50.9	7.0	22.4	0.0	0.0
16	0.0	2.3	22.1	0.0	0.0	10.4	0.0	62.3	4.8	11.1	0.0	0.0
17	0.0	0.0	2.8	13.2	0.0	6.2	25.6	105.3	4.0	6.1	0.0	0.0
18	0.0	0.0	0.0	0.0	0.0	15.4	22.4	72.2	1.0	3.2	0.0	0.0
19	0.0	0.0	0.0	0.0	0.0	25.6	0.0	0.0	24.3	0.0	0.0	0.0
20	0.0	2.6	2.0	17.9	0.0	14.3	0.0	8.7	3.2	2.3	0.0	0.0
21	0.0	31.8	3.4	0.0	5.6	28.0	0.0	32.9	9.9	0.0	0.0	0.0
22	0.0	0.0	1.9	0.0	0.0	52.3	0.0	1.0	3.2	0.0	0.0	0.0
23	9.2	0.0	0.0	0.0	33.9	13.5	6.0	0.0	8.6	0.0	0.0	0.0
24	0.0	0.0	0.0	38.3	12.2	7.4	0.0	4.9	15.8	0.0	0.0	0.0
25	0.0	0.0	3.4	0.0	4.3	53.9	0.0	0.0	0.3	0.0	0.0	0.0
26	0.0	0.0	0.0	0.0	12.1	0.0	0.0	69.3	0.0	0.0	0.0	0.0
27	0.0	0.0	0.0	0.0	11.8	30.8	5.0	51.8	0.0	0.0	0.0	0.0
28	3.2	0.0	0.0	0.0	5.0	25.3	14.2	5.0	4.3	0.0	0.0	0.0
29	0.0	0.0	6.2	30.3	0.0	38.8	13.2	28.6	10.0	4.2	0.0	0.0
30	0.0	0.0	0.0	10.5	0.0	5.4	81.0	59.9	6.1	42.9	0.0	0.0
31	0.0	0.0	0.0	0.0	0.0	50.2	0.0	30.7	0.0	0.0	0.0	0.0
TOTAL	12.4	88.3	194.9	282.4	168.0	469.1	293.9	926.0	557.7	357.8	264.4	0.0
AVERAGE	0.4	3.0	6.3	8.1	5.4	15.6	9.5	29.9	18.6	11.5	6.6	0.0
MAXIMUM	9.2	31.8	111.0	38.3	33.9	53.9	81.0	105.3	99.5	62.8	54.3	0.0
MINIMUM	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

ANNUAL  
 TOTAL AVERAGE MAXIMUM MINIMUM  
 3515.1 9.6 111.0 0.0



1965

NONSHINE PRECIPITATION

*DAY*	1	2	3	4	5	6	7	8	9	10	11	12
1	0.0	0.0	0.0	0.0	0.0	10.8	7.9	2.2	10.7	0.4	0.0	0.0
2	0.0	0.0	22.9	0.0	0.0	0.0	7.1	7.8	52.3	10.0	0.0	0.0
3	0.0	0.0	12.6	15.9	0.0	0.0	26.6	11.4	0.0	1.0	0.0	0.0
4	0.0	0.0	7.3	16.9	0.0	1.1	11.7	47.6	4.2	0.0	0.0	0.0
5	0.0	0.0	3.2	134.0	5.1	0.3	12.6	2.6	39.3	0.0	0.2	0.0
6	0.0	0.0	0.0	4.1	10.8	2.2	5.2	1.4	30.0	0.0	0.0	22.3
7	0.0	0.0	0.0	18.2	0.0	0.0	56.6	17.6	3.1	0.0	15.4	0.0
8	0.0	1.5	0.0	3.9	0.0	2.7	83.0	72.1	0.0	10.3	1.5	1.2
9	0.0	0.0	0.0	10.9	37.3	0.0	24.8	2.3	38.3	51.7	10.4	0.0
10	0.0	0.0	0.0	16.1	15.6	0.0	71.0	15.2	25.4	0.9	0.0	0.0
11	0.0	16.3	0.0	0.2	2.0	6.1	1.6	71.8	36.2	0.0	0.0	0.0
12	0.0	0.0	0.0	0.0	37.2	5.8	6.6	19.8	2.4	0.5	2.6	0.0
13	0.0	25.5	0.0	0.0	0.0	61.1	7.8	46.9	21.0	3.0	25.2	0.0
14	0.0	20.9	0.0	0.0	11.1	37.6	0.0	38.0	2.3	0.4	26.6	0.0
15	0.0	0.0	0.0	13.2	0.7	28.6	0.0	3.4	4.2	2.3	0.0	0.0
16	0.0	0.0	0.0	42.0	15.9	18.1	67.6	15.0	8.9	43.2	0.0	0.0
17	0.0	0.0	0.0	5.1	27.7	113.0	3.7	4.2	14.2	8.4	0.0	0.0
18	0.0	0.0	0.0	9.5	0.0	48.7	19.6	1.7	11.0	16.5	0.0	0.0
19	0.0	0.0	0.0	35.2	21.0	109.1	6.4	11.3	0.5	0.0	0.0	0.0
20	0.0	1.2	0.0	21.2	2.0	20.1	0.0	21.2	9.5	0.0	1.1	0.0
21	0.0	62.0	0.0	9.0	21.7	31.4	10.8	0.6	2.4	2.5	0.0	0.0
22	0.0	22.0	0.0	12.0	3.1	18.8	11.5	39.4	0.0	0.8	0.0	0.0
23	0.0	41.5	3.2	6.1	6.0	7.3	23.5	15.1	0.0	0.0	0.0	0.0
24	0.0	10.9	0.0	40.0	0.2	9.3	0.0	12.5	0.0	2.5	0.0	0.0
25	3.1	0.0	0.0	70.1	11.7	3.8	0.0	21.1	0.4	2.8	0.0	0.0
26	4.7	0.0	0.0	12.1	6.9	5.3	4.6	9.2	0.0	0.0	0.2	0.0
27	12.0	21.7	0.0	3.8	0.0	27.6	3.6	28.7	0.0	3.5	1.2	0.0
28	10.0	6.3	0.0	38.2	0.0	8.1	17.2	58.6	0.0	20.4	29.8	0.0
29	0.0	0.0	15.3	5.7	0.0	21.2	7.2	3.6	0.0	0.0	0.0	0.0
30	0.0	0.0	4.9	0.0	2.9	4.6	0.0	3.9	0.0	0.0	0.0	0.0
31	0.0	0.0	0.0	0.0	0.1	0.0	13.4	1.6	0.0	0.0	0.0	0.0
TOTAL	29.8	229.6	67.4	583.7	239.0	602.7	511.6	607.8	315.3	131.1	114.2	23.5
AVERAGE	1.0	8.2	2.2	18.1	7.7	20.1	16.5	19.6	10.5	5.8	3.8	0.8
MAXIMUM	12.0	62.0	22.9	134.0	37.3	113.0	83.0	72.1	52.3	51.7	29.8	22.3
MINIMUM	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

\*\*\*\*\* ANNUAL \*\*\*\*\*  
 \*\*\*\*\* TOTAL AVERAGE MAXIMUM MINIMUM \*\*\*\*\*  
 \*\*\*\*\* 3465.7 9.5 134.0 0.0 \*\*\*\*\*

1986

NONSHINE PRECIPITATION

*DAY*	1	2	3	4	5	6	7	8	9	10	11	12
1	0.0	0.0	0.0	30.2	29.0	0.0	1.2	0.0	1.0	26.3	0.0	0.2
2	0.0	0.0	0.0	0.0	3.9	0.0	0.0	0.0	13.4	6.3	0.0	7.9
3	0.0	0.0	0.0	0.0	9.6	0.0	16.9	9.4	12.8	0.0	0.0	0.0
4	0.0	0.0	0.0	0.0	2.7	0.5	41.1	53.3	0.8	0.0	0.0	0.0
5	0.0	0.0	0.0	0.0	41.5	2.0	3.1	47.2	23.1	0.0	0.0	13.2
6	0.0	0.0	0.0	0.0	30.4	9.0	7.4	31.2	83.9	0.0	0.0	0.0
7	0.0	0.0	0.0	0.0	0.5	5.2	0.0	23.3	4.3	0.0	0.0	0.0
8	0.0	0.0	0.0	0.0	3.9	0.0	17.8	15.6	17.6	0.0	0.0	1.0
9	0.0	0.0	0.0	0.0	41.1	0.0	2.5	73.4	16.6	3.2	0.0	0.0
10	0.0	0.0	0.0	57.9	17.8	7.2	5.2	21.0	20.4	0.0	0.0	0.0
11	0.0	0.0	0.0	0.0	28.6	10.6	13.8	7.2	19.3	0.0	0.7	0.0
12	0.0	0.0	0.0	15.1	0.8	16.8	63.4	26.5	11.7	0.8	11.4	0.0
13	0.0	0.0	0.0	11.4	0.5	16.1	3.6	67.3	30.8	0.0	2.7	0.0
14	0.0	0.0	0.0	3.6	0.3	21.5	10.0	56.4	0.0	6.5	9.2	0.0
15	0.0	0.0	0.0	0.9	0.0	32.4	31.0	25.2	0.0	21.3	0.0	0.0
16	0.0	0.0	0.0	1.5	8.2	0.8	1.4	41.8	0.0	1.2	0.0	0.0
17	0.0	0.0	0.0	0.0	18.6	0.5	13.4	0.0	0.0	0.0	0.0	0.0
18	0.0	0.0	0.0	0.0	29.6	0.0	4.6	3.5	0.0	10.8	0.0	0.0
19	0.0	0.0	20.5	2.5	33.9	0.8	117.6	2.0	3.6	0.5	0.0	0.0
20	0.0	16.8	0.0	0.0	30.8	1.2	75.3	6.9	0.0	0.0	0.0	0.0
21	0.0	0.0	0.0	16.2	3.9	0.0	5.3	4.2	0.0	0.0	0.0	0.0
22	0.0	0.0	10.2	25.5	10.4	5.3	15.5	4.4	0.0	75.0	0.0	0.0
23	0.0	0.0	13.2	6.2	5.9	70.6	0.0	3.7	0.0	28.7	0.0	0.0
24	0.0	0.0	0.0	0.0	51.8	25.7	0.0	36.8	0.0	0.0	0.0	0.0
25	0.0	0.0	1.1	79.5	17.6	5.6	3.0	5.5	16.2	0.0	0.0	0.0
26	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
27	0.0	0.0	0.0	7.5	4.8	9.9	3.7	21.3	52.2	4.1	0.0	0.0
28	0.0	0.0	0.0	0.0	35.8	8.3	4.8	26.5	52.2	22.4	0.0	0.0
29	0.0	0.0	0.0	16.1	0.0	2.3	4.8	110.1	5.1	5.0	0.0	0.0
30	0.0	0.0	0.0	0.0	6.3	0.0	2.3	1.6	23.2	6.2	0.0	0.0
31	0.0	0.0	0.0	0.0	0.0	0.0	0.0	2.8	8.8	3.4	0.0	0.0
TOTAL	0.0	16.8	130.2	274.1	469.0	252.3	471.3	731.9	417.0	221.7	24.0	22.3
AVERAGE	0.0	0.6	4.2	9.1	15.1	8.4	15.2	23.6	13.9	7.2	0.8	0.7
MAXIMUM	0.0	16.8	85.2	79.5	51.8	70.6	117.6	110.1	83.9	75.0	11.4	13.2
MINIMUM	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

\*\*\*\*\* ANNUAL \*\*\*\*\*  
 \*\*\*\*\* TOTAL AVERAGE MAXIMUM MINIMUM \*\*\*\*\*  
 \*\*\*\*\* 3030.6 2.3 117.6 0.0 \*\*\*\*\*

NONSHINE PRECIPITATION

1987

DAY*	1	2	3	4	5	6	7	8	9	10	11	12
1	0.0	0.0	0.0	0.0	1.6	0.0	23.2	0.0	1.7	0.0	0.4	0.0
2	0.0	0.0	19.8	6.8	13.9	2.4	26.3	0.0	6.4	6.2	0.0	0.0
3	0.0	0.0	0.0	0.4	14.2	20.5	83.3	6.8	20.8	44.2	1.4	0.0
4	0.0	0.0	0.0	0.0	1.4	4.2	21.3	26.3	2.7	16.0	0.0	0.0
5	0.0	0.0	21.6	0.0	1.8	3.5	24.8	3.0	4.5	16.0	0.0	0.0
6	0.0	0.0	0.0	0.0	0.0	0.0	19.6	0.0	22.1	0.0	0.0	0.0
7	0.0	0.0	0.0	0.0	0.0	6.5	3.0	16.6	17.4	1.4	0.0	0.0
8	0.0	0.0	0.0	5.2	0.0	10.1	12.1	12.1	1.1	6.8	0.0	0.0
9	0.0	0.0	0.0	16.0	0.0	32.6	5.3	29.3	9.3	8.8	0.4	0.0
10	0.0	0.0	0.0	13.2	0.0	10.8	6.6	4.0	7.6	17.8	0.0	0.0
11	0.0	0.0	17.8	0.0	11.8	0.0	1.6	0.0	27.8	11.4	2.4	0.0
12	0.0	0.0	0.8	0.0	25.5	4.2	26.6	11.7	0.6	15.6	14.7	0.0
13	0.0	0.0	0.0	0.0	1.2	25.9	17.5	0.0	0.0	24.5	0.0	0.0
14	0.0	0.0	0.0	42.8	0.0	2.0	132.6	0.0	0.0	0.0	0.0	0.0
15	0.0	0.0	0.0	30.0	0.0	0.0	29.4	0.4	0.0	0.0	0.0	0.0
16	0.0	0.0	0.0	0.0	0.0	7.8	4.6	67.9	0.0	1.5	0.0	0.0
17	0.0	0.0	0.0	13.8	0.0	3.4	3.5	22.2	0.0	22.2	0.0	0.0
18	0.0	0.0	2.6	0.0	30.6	39.0	5.7	38.8	0.0	0.0	0.8	0.0
19	0.0	0.0	0.0	15.0	0.0	14.7	11.7	3.2	0.0	0.0	24.3	0.0
20	0.0	0.0	0.0	0.0	0.0	12.8	3.8	105.1	0.0	5.5	0.3	0.0
21	0.0	0.0	0.0	0.0	0.0	32.9	1.1	123.8	27.5	0.0	0.9	0.0
22	0.0	0.0	0.0	0.0	36.0	0.0	3.1	42.1	30.7	0.0	0.0	0.0
23	0.0	0.0	0.0	0.2	0.4	0.0	3.2	18.4	40.9	0.0	0.0	0.0
24	0.0	0.0	0.0	0.0	0.0	0.6	0.0	8.2	20.4	2.2	4.8	0.0
25	0.0	1.0	5.5	40.4	0.0	0.0	0.0	0.0	2.5	0.0	3.2	0.0
26	0.0	4.8	12.4	13.2	0.0	0.0	0.0	0.0	1.3	0.0	0.0	0.0
27	0.0	0.0	15.6	5.0	0.0	0.0	0.0	8.5	20.7	0.0	3.4	0.0
28	0.0	0.0	0.0	0.0	1.2	1.3	11.7	19.3	0.0	21.5	0.0	0.0
29	0.0	0.0	0.0	0.0	0.0	3.2	10.7	22.8	12.8	2.5	0.0	0.0
30	0.0	0.0	0.0	0.0	0.0	0.0	5.6	0.0	0.0	16.7	0.0	0.0
31	0.0	3.5	3.5	0.0	0.0	21.3	0.0	0.0	23.8	0.0	0.0	0.0
TOTAL	0.0	5.8	99.4	200.0	139.6	228.3	517.2	591.0	276.9	241.6	57.0	0.0
AVERAGE	0.0	0.2	3.2	6.7	4.5	7.6	16.7	19.1	9.2	8.4	1.9	0.0
MAXIMUM	0.0	4.8	21.6	42.8	36.0	39.0	132.6	123.8	40.9	44.2	24.3	0.0
MINIMUM	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

\*\*\*\*\* ANNUAL \*\*\*\*\*  
 \*\*\*\*\* TOTAL AVERAGE MAXIMUM MINIMUM \*\*\*\*\*  
 \*\*\*\*\* 2377.0 6.5 132.6 \*\*\*\*\*  
 \*\*\*\*\* 0.0 \*\*\*\*\*

1988

NONSHINE PRECIPITATION

#DAY#	1	2	3	4	5	6	7	8	9	10	11	12
1	0.0	0.0	2.4	0.0	0.0	26.9	8.2	50.9	0.0	0.0	0.0	0.0
2	0.0	0.0	5.6	12.2	0.0	30.3	14.6	46.8	0.0	0.0	0.0	0.0
3	0.0	0.0	3.8	0.0	0.0	33.8	0.0	9.2	0.0	3.5	0.0	0.0
4	0.0	0.0	0.0	0.0	0.0	2.2	23.6	0.0	0.0	35.8	0.0	9.7
5	0.0	0.0	0.0	0.0	0.0	35.0	3.7	4.0	2.2	0.0	7.5	5.4
6	0.0	0.0	0.0	0.0	0.0	7.4	3.0	18.2	0.0	0.0	0.0	0.0
7	0.0	0.0	31.1	0.0	0.0	1.3	0.0	0.0	17.8	45.5	0.0	0.0
8	1.4	2.7	0.0	0.0	0.0	2.2	2.2	6.0	13.2	2.4	0.0	0.0
9	0.0	6.7	0.0	0.0	1.2	0.0	0.0	16.4	33.4	20.6	0.0	0.0
10	0.0	1.2	0.0	0.0	0.0	0.0	4.5	24.0	17.0	5.7	0.0	0.0
11	0.0	0.0	0.0	6.1	0.0	8.0	15.0	0.0	0.0	15.7	0.0	0.0
12	0.0	0.0	32.1	0.0	0.0	8.8	2.6	27.4	3.8	14.7	0.0	0.0
13	0.0	0.0	0.0	5.9	97.8	35.8	0.0	75.4	1.6	16.1	0.0	0.0
14	0.0	0.0	0.0	16.3	29.2	4.7	21.6	4.2	0.0	10.1	0.0	0.0
15	0.0	0.2	20.5	75.4	48.6	7.5	13.7	6.3	0.0	25.2	0.0	0.0
16	0.0	0.0	11.1	0.0	6.4	21.6	11.5	3.8	0.0	16.2	0.0	0.0
17	0.0	0.0	0.0	0.0	9.7	10.5	23.5	11.3	13.7	20.4	0.0	0.0
18	0.0	0.0	0.0	0.0	32.6	1.7	0.0	0.0	9.2	11.5	0.0	0.0
19	0.0	0.0	31.0	14.4	6.4	7.1	5.7	1.8	10.1	5.3	0.0	0.0
20	0.0	0.0	0.0	0.0	0.0	5.6	48.2	3.1	0.0	10.4	0.0	0.0
21	0.0	0.0	0.0	0.4	0.0	1.4	9.7	0.0	0.0	0.0	0.0	0.0
22	0.0	0.0	0.0	0.0	0.0	7.8	2.3	0.0	0.0	0.0	0.0	0.0
23	0.0	0.0	0.0	13.0	32.2	1.2	3.8	30.8	5.7	0.0	0.0	0.0
24	0.0	8.5	0.0	37.8	0.0	1.8	0.0	19.0	20.4	10.8	0.0	0.0
25	0.0	24.5	0.0	6.0	0.0	0.6	0.0	9.1	5.3	0.0	0.0	0.0
26	0.0	0.0	37.8	0.0	0.0	0.0	11.0	0.0	0.0	35.8	0.0	0.0
27	0.0	0.0	0.0	0.0	2.2	0.0	22.4	22.8	10.3	0.0	0.0	0.0
28	0.0	0.0	0.0	1.4	10.2	2.4	5.2	0.0	15.4	0.0	0.0	0.0
29	0.0	18.2	0.0	57.4	4.0	16.2	17.5	1.2	0.0	0.0	0.0	0.0
30	0.0	0.0	0.0	20.4	8.6	0.0	9.9	1.7	0.0	0.0	0.0	0.0
31	0.0	0.0	0.0	0.0	84.0	0.0	25.8	0.0	0.0	0.0	0.0	0.0
TOTAL	1.4	62.0	112.2	329.9	373.1	271.6	328.7	396.3	179.1	305.7	18.3	15.1
AVERAGE	0.0	2.1	3.6	11.0	12.0	9.1	10.6	12.8	6.0	9.9	0.6	0.5
MAXIMUM	1.4	24.5	37.8	75.4	97.8	35.8	62.2	75.4	33.4	45.5	10.8	9.7
MINIMUM	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

ANNUAL  
 TOTAL AVERAGE MAXIMUM MINIMUM  
 233.4 6.5 97.8 0.0

1989

NONSHINE PRECIPITATION

DAY*	1	2	3	4	5	6	7	8	9	10	11	12
1	0.0	0.0	0.0	0.0	17.0	39.4	0.0	2.6	59.4	0.0	0.0	0.0
2	0.0	0.0	0.0	0.0	0.0	11.0	0.0	2.8	5.8	0.0	0.0	0.0
3	0.0	0.0	1.5	0.0	4.3	0.0	21.2	5.5	0.4	12.6	12.0	0.0
4	0.0	0.0	1.6	0.0	2.3	0.0	0.0	45.6	5.8	2.1	0.0	0.0
5	0.0	0.0	0.0	0.0	0.0	0.0	28.8	50.2	7.4	0.0	25.4	0.0
6	0.0	0.0	0.0	0.0	5.6	0.0	2.2	7.0	63.1	0.0	0.0	0.0
7	0.0	0.0	0.0	7.2	3.4	0.0	0.6	4.6	2.3	0.0	0.0	0.0
8	0.0	0.0	0.0	15.8	0.0	14.0	2.4	2.4	3.0	0.0	0.0	0.0
9	0.0	0.0	0.0	1.6	22.0	2.0	10.4	16.8	24.7	0.0	0.0	0.0
10	0.0	0.0	0.0	1.8	2.5	75.2	0.0	12.9	29.0	0.8	0.0	0.0
11	0.0	0.0	0.0	0.0	0.0	22.4	22.6	31.8	1.2	0.0	0.0	0.0
12	0.0	0.0	0.0	0.0	0.0	10.0	42.6	103.6	0.0	25.1	0.0	0.0
13	0.0	0.0	0.0	21.8	0.0	0.0	15.1	10.6	4.2	3.3	0.0	0.0
14	0.0	0.0	0.0	48.6	11.0	0.0	7.0	0.0	5.8	0.0	0.0	0.0
15	0.0	0.0	0.0	0.0	3.0	0.0	6.6	0.0	4.0	0.0	0.0	0.0
16	0.0	0.0	0.0	8.8	0.0	0.0	0.0	0.0	2.2	0.0	0.0	0.0
17	0.0	0.0	27.3	0.0	2.0	1.0	0.0	0.0	9.0	0.0	0.0	0.0
18	6.9	0.0	6.2	0.0	4.8	1.2	6.0	6.4	4.2	27.1	0.0	0.0
19	0.0	0.0	0.0	0.0	3.0	0.0	21.4	19.6	10.6	2.2	0.0	0.0
20	0.0	0.0	0.0	68.2	0.0	0.0	21.0	16.2	18.0	0.0	0.0	0.0
21	0.0	0.0	1.4	0.0	0.0	1.0	6.3	2.6	5.4	0.0	0.0	0.2
22	0.0	0.0	10.1	0.0	0.0	0.0	8.1	3.8	0.0	0.0	0.0	0.0
23	0.0	0.0	0.0	4.1	4.2	1.2	153.4	5.2	0.8	0.0	0.0	0.0
24	0.0	0.0	0.0	1.8	21.6	0.0	15.2	8.2	1.4	0.0	0.0	0.0
25	0.0	0.0	6.7	0.0	74.0	7.4	3.9	6.6	8.7	14.5	0.0	0.0
26	0.0	0.0	39.5	23.5	22.0	4.9	8.3	11.2	18.8	1.0	0.0	0.0
27	0.0	0.0	7.4	0.0	17.4	4.9	5.7	7.0	0.0	12.2	27.8	0.0
28	0.0	0.0	0.0	58.4	0.0	2.8	2.4	24.6	0.0	0.0	1.2	0.0
29	0.0	0.0	9.3	20.5	3.3	0.0	10.4	16.7	0.0	0.0	0.0	0.0
30	0.0	0.0	3.4	0.0	6.8	0.0	19.3	0.0	28.8	0.0	0.0	0.0
31	0.0	0.0	22.3	0.0	23.8	0.0	17.8	5.7	0.0	0.0	0.0	0.0
TOTAL	6.9	0.0	136.7	282.1	254.0	198.4	458.7	480.3	323.6	100.9	66.4	0.2
AVERAGE	0.2	0.0	4.4	9.4	8.2	6.6	14.8	13.9	10.8	3.3	2.2	0.0
MAXIMUM	6.9	0.0	39.5	68.2	74.0	75.2	153.4	103.6	65.1	27.1	27.8	0.2
MINIMUM	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

ANNUAL  
 TOTAL AVERAGE MAXIMUM MINIMUM  
 258.2 6.2 153.4 0.0

1990

NONSHINE PRECIPITATION

#DAY#	1	2	3	4	5	6	7	8	9	10	11	12
1	0.0	0.0	7.2	0.0	0.0	1.8	7.1	16.8	0.0	1.8	0.0	0.0
2	0.0	0.0	65.8	0.0	22.4	2.6	21.7	0.0	12.8	1.3	0.5	0.0
3	0.0	0.0	1.7	0.0	21.6	4.5	4.2	16.4	1.2	48.7	0.0	0.0
4	0.0	0.0	0.0	0.0	2.9	0.0	0.0	32.2	41.2	15.3	0.0	0.0
5	0.0	0.0	0.0	0.0	11.5	0.0	0.0	19.0	29.6	10.6	0.0	0.0
6	0.0	0.0	0.0	0.0	5.2	0.0	0.0	7.6	0.0	29.8	0.0	0.0
7	0.0	0.0	1.0	0.0	50.4	0.0	0.0	1.3	31.8	0.0	0.0	0.0
8	0.0	0.0	1.1	0.0	0.0	0.0	21.0	2.7	16.9	10.8	37.8	0.0
9	0.0	0.0	9.5	0.0	0.0	0.0	1.0	0.0	42.5	0.0	2.0	0.0
10	2.6	6.0	10.5	0.0	0.0	38.2	6.5	0.0	23.6	0.0	1.5	0.0
11	0.0	0.0	18.5	0.0	0.0	3.1	0.0	0.0	2.0	0.0	2.1	0.0
12	0.0	0.0	1.0	2.4	0.0	7.2	2.8	2.2	6.7	0.0	0.0	0.0
13	0.0	0.0	0.0	11.8	40.2	3.8	12.8	7.0	1.0	0.0	0.0	0.0
14	0.0	26.8	0.0	27.8	0.0	36.8	0.0	0.0	0.0	30.7	0.0	0.0
15	0.0	0.0	1.8	0.0	0.0	9.8	0.9	0.0	1.2	28.4	0.0	0.0
16	0.0	0.0	0.0	0.0	0.0	21.8	0.0	3.5	0.6	1.0	0.0	0.0
17	0.0	0.0	0.0	0.0	14.2	18.5	0.0	0.0	0.6	0.0	0.0	0.0
18	0.0	0.0	3.2	0.0	9.3	12.3	0.0	0.6	8.9	4.4	0.0	0.0
19	0.0	0.4	0.0	1.6	3.6	3.0	32.8	9.3	53.3	39.8	0.0	0.0
20	0.0	0.0	0.0	0.0	0.0	20.6	0.0	36.7	25.9	0.3	0.4	0.0
21	0.0	0.0	0.0	12.6	4.7	15.6	5.1	8.0	5.0	4.2	0.0	0.0
22	0.0	0.0	0.8	0.0	4.1	8.0	35.9	2.8	11.6	0.0	0.0	0.0
23	0.0	0.0	0.9	0.0	21.8	21.3	5.8	0.0	31.2	1.0	0.0	0.0
24	0.0	0.0	0.0	0.8	27.6	35.1	1.0	1.7	5.8	3.8	0.0	0.0
25	0.0	0.0	1.8	4.2	17.6	20.1	2.2	59.9	1.2	0.0	0.0	0.0
26	0.0	0.0	12.8	32.8	13.2	24.7	6.8	25.7	3.4	0.0	0.0	0.0
27	0.0	0.0	58.1	0.0	0.0	8.5	0.0	5.4	6.0	0.0	0.0	0.0
28	0.0	0.0	108.5	0.0	0.0	18.6	6.8	7.4	3.8	0.0	0.0	0.0
29	0.0	0.0	40.2	0.0	5.8	1.0	16.5	104.1	39.8	0.0	0.0	0.0
30	0.0	0.0	2.5	0.0	2.3	2.2	9.4	44.8	0.0	0.0	0.0	0.0
31	0.0	0.0	0.0	0.0	3.3	5.6	5.6	11.9	0.0	5.6	0.0	0.0
TOTAL	2.6	27.2	324.9	94.0	281.7	339.2	295.9	425.7	377.1	267.3	44.3	0.0
AVERAGE	0.1	1.0	10.5	3.1	9.1	11.5	6.6	13.7	12.6	8.6	1.5	0.0
MAXIMUM	2.6	26.8	108.5	32.8	50.4	38.2	35.9	104.1	53.3	48.7	37.8	0.0
MINIMUM	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

\*\*\*\*\* ANNUAL \*\*\*\*\*  
 \*\*\*\*\* TOTAL AVERAGE MAXIMUM MINIMUM \*\*\*\*\*  
 \*\*\*\*\* 2389.9 6.5 108.5 0.0 \*\*\*\*\*

A3.5.3 Results of Discharge Estimation

Estimated Daily Discharge at Xe Katam Intake Site (1980-1990)

		1980											
		CALCULATED											
#DAY*	1	2	3	4	5	6	7	8	9	10	11	12	
1	3.30	1.52	0.88	0.79	0.74	14.14	13.72	16.22	17.41	18.10	9.85	4.17	
2	3.22	1.47	0.88	0.78	0.74	11.71	13.23	14.35	17.36	16.26	9.50	4.14	
3	3.14	1.42	0.87	0.78	0.73	10.89	14.11	13.21	16.16	15.33	9.27	4.10	
4	3.07	1.36	0.87	0.78	0.73	9.45	14.70	12.64	22.82	14.67	8.94	4.07	
5	3.01	1.31	0.87	0.78	0.73	8.96	14.78	13.07	21.45	14.03	8.61	4.04	
6	2.95	1.26	0.86	0.78	0.73	8.48	13.51	12.15	19.42	14.06	8.29	4.01	
7	2.89	1.21	0.86	0.77	0.73	10.16	14.24	11.43	17.71	13.31	7.98	3.97	
8	2.83	1.16	0.85	0.77	0.73	13.17	12.70	10.94	16.48	12.88	7.69	3.94	
9	2.77	1.11	0.85	0.77	0.73	10.96	12.05	10.46	15.76	13.29	7.40	3.91	
10	2.71	1.06	0.85	0.77	1.88	13.04	11.49	10.13	15.05	12.37	7.13	3.87	
11	2.65	1.01	0.84	0.77	0.73	12.95	10.94	9.93	15.33	14.68	6.87	3.84	
12	2.59	0.96	0.84	0.77	4.32	10.94	10.42	10.32	14.27	12.97	6.62	3.78	
13	2.54	0.92	0.84	0.76	8.19	9.98	9.93	10.72	14.36	12.15	6.38	3.73	
14	2.48	0.92	0.83	0.76	4.97	9.48	9.46	9.70	13.51	11.74	6.15	3.68	
15	2.43	0.92	0.83	0.76	5.43	14.18	9.02	11.60	14.93	11.37	5.93	3.62	
16	2.37	0.92	0.83	1.66	61.11	11.90	8.76	13.82	19.28	11.03	5.72	3.57	
17	2.31	0.92	0.82	0.75	44.84	12.40	8.40	11.90	17.46	10.66	5.57	3.52	
18	2.26	0.92	0.82	0.75	15.16	11.53	8.40	10.58	20.40	10.28	5.42	3.46	
19	2.21	0.92	0.82	0.75	40.36	10.85	8.89	11.48	18.63	10.12	5.26	3.41	
20	2.15	0.92	0.81	0.75	17.63	10.39	9.67	10.35	16.96	9.80	5.10	3.36	
21	2.09	0.91	0.81	0.75	14.68	9.75	21.30	12.69	17.70	9.45	4.94	3.31	
22	2.04	0.91	0.81	0.75	13.07	13.58	36.69	13.27	21.65	9.10	4.78	3.25	
23	1.99	0.91	0.81	0.75	12.35	11.93	37.79	13.57	19.65	8.89	4.63	3.20	
24	1.93	0.90	0.81	0.75	11.65	12.82	19.78	12.22	17.58	13.45	4.47	3.15	
25	1.88	0.90	0.81	0.75	10.97	11.35	17.30	42.26	16.17	11.38	4.35	3.10	
26	1.83	0.90	0.81	0.74	10.32	24.76	16.13	19.48	15.51	10.16	4.32	3.05	
27	1.78	0.89	0.80	0.74	9.71	17.83	16.14	18.35	16.67	12.27	4.29	3.00	
28	1.73	0.89	0.80	0.74	10.93	18.58	14.64	17.85	15.67	11.41	4.26	2.95	
29	1.67	0.89	0.80	0.74	9.57	17.36	15.90	20.94	18.64	11.50	4.23	2.90	
30	1.62	0.79	0.79	0.74	8.96	15.09	14.44	20.32	20.34	10.89	4.20	2.85	
31	1.57	0.79	0.79	0.74	8.47	13.63	19.09	13.63	19.09	10.24	4.20	2.80	
TOTAL	74.01	30.31	25.76	23.70	331.89	378.64	452.16	443.82	524.33	377.84	188.15	109.75	
AVERAGE	2.39	1.05	0.83	0.79	10.71	12.62	14.59	14.32	17.48	12.19	6.27	3.54	
MAXIMUM	3.30	1.52	0.88	1.66	61.11	24.76	37.79	42.26	22.82	18.10	9.85	4.17	
MINIMUM	1.57	0.89	0.79	0.74	0.73	8.48	8.40	9.70	13.51	8.89	4.20	2.80	
***** ANNUAL *****													
TOTAL AVERAGE MAXIMUM MINIMUM													
***** 2960.36 8.09 61.11 0.73 *****													

1981

XE KATAM CALCULATED

*DAY*	1	2	3	4	5	6	7	8	9	10	11	12
1	2.75	1.29	2.71	0.87	3.62	10.32	23.84	22.10	25.83	20.07	9.73	5.22
2	2.70	1.25	2.63	0.87	3.12	14.21	22.04	20.32	23.36	17.72	9.33	5.18
3	2.64	1.20	2.46	4.02	6.26	12.14	22.90	24.78	21.54	17.80	9.07	5.14
4	2.59	1.16	2.24	5.33	4.68	10.71	48.25	34.61	20.61	17.16	9.62	5.09
5	2.55	1.11	2.02	3.46	4.18	11.20	29.43	28.33	19.76	17.75	8.98	5.05
6	2.50	1.07	1.83	4.97	6.23	12.77	26.68	28.50	18.95	15.96	8.72	5.01
7	2.45	1.02	1.61	3.00	9.09	15.65	24.10	60.56	18.19	14.93	8.47	4.96
8	2.40	0.98	1.39	2.85	17.84	23.62	82.42	82.42	21.06	20.85	8.22	4.92
9	2.35	0.93	1.38	2.66	17.89	27.86	51.21	51.21	19.63	18.11	7.97	4.86
10	2.30	0.89	1.43	2.46	5.56	34.69	24.81	33.74	21.87	17.10	7.73	4.80
11	2.25	0.84	1.41	2.26	5.37	31.38	23.31	30.87	21.67	15.83	7.49	4.74
12	2.20	0.83	2.38	2.07	5.10	31.62	23.64	30.29	19.57	15.08	7.27	4.67
13	2.15	0.83	6.83	1.91	6.00	55.41	25.03	32.70	18.13	14.66	7.05	4.61
14	2.11	0.83	3.93	1.77	6.98	39.22	27.92	31.39	17.47	14.73	6.85	4.55
15	2.06	0.83	3.00	1.61	10.17	30.64	24.84	29.19	16.80	15.72	6.66	4.49
16	2.01	0.83	2.79	1.46	7.98	29.27	22.75	31.76	16.15	18.97	6.47	4.42
17	1.96	0.82	2.56	1.31	6.65	29.71	21.30	32.32	15.83	16.69	6.31	4.36
18	1.92	0.82	2.32	1.15	10.23	30.64	20.67	34.93	16.12	15.09	6.14	4.30
19	1.87	0.82	2.07	1.79	19.90	29.12	20.85	35.91	15.00	14.73	5.96	4.24
20	1.82	0.82	1.84	2.84	14.42	29.61	27.13	33.01	15.23	14.19	5.80	4.18
21	1.78	0.82	1.63	1.79	16.69	42.40	26.03	30.05	14.41	13.69	5.65	4.12
22	1.73	0.81	1.42	1.70	30.41	34.57	24.88	28.17	13.91	13.19	5.60	4.06
23	1.68	0.81	1.22	1.58	17.32	30.89	23.79	26.57	14.11	12.71	5.56	4.00
24	1.64	0.81	1.03	1.45	14.86	27.77	23.06	25.43	13.40	12.25	5.52	3.95
25	1.59	0.80	0.85	1.87	15.25	25.40	21.08	26.84	12.95	11.81	5.48	3.89
26	1.55	0.80	0.85	1.59	15.67	24.14	19.95	26.50	12.51	11.40	5.43	3.83
27	1.50	0.79	0.85	1.50	13.69	23.54	20.66	24.37	12.88	11.00	5.39	3.77
28	1.46	0.79	0.86	1.52	12.76	22.56	19.17	24.49	13.85	10.61	5.35	3.71
29	1.42	0.86	0.86	4.36	12.11	21.40	18.40	24.11	13.00	10.25	5.31	3.66
30	1.38	0.87	0.87	5.55	11.49	20.37	17.68	24.17	12.83	9.90	5.26	3.60
31	1.34	0.87	0.87	10.89	10.89	16.93	16.93	23.05	12.51	9.59	5.26	3.54
TOTAL	62.85	40.34	60.16	71.57	319.61	767.08	742.60	992.69	515.92	459.14	208.39	136.92
AVERAGE	2.02	1.44	1.94	2.39	10.31	25.57	23.95	32.02	17.20	14.81	6.95	4.42
MAXIMUM	2.75	6.79	6.83	5.55	30.41	55.41	48.25	82.42	25.83	20.65	9.73	5.22
MINIMUM	1.34	0.80	0.85	0.87	3.12	10.32	16.93	20.32	12.51	9.59	5.26	3.54

\*\*\*\*\*  
 | ANNUAL  
 |\*\*\*\*\*  
TOTAL	AVERAGE	MAXIMUM	MINIMUM
\*\*\*\*\*	\*\*\*\*\*	\*\*\*\*\*	\*\*\*\*\*
4377.07	11.99	82.42	0.80
\*\*\*\*\*	\*\*\*\*\*	\*\*\*\*\*	\*\*\*\*\*



1982

XE KATAM CALCULATED

*DAY*	1	2	3	4	5	6	7	8	9	10	11	12
1	3.48	1.84	0.96	0.89	0.86	2.77	14.52	21.54	31.34	19.65	7.56	16.28
2	3.43	1.79	0.96	0.89	0.86	2.59	12.98	20.28	36.34	18.41	7.32	13.84
3	3.37	1.74	0.95	0.88	0.85	2.41	12.62	20.27	26.25	17.11	7.09	12.26
4	3.32	1.69	0.95	0.88	0.85	2.25	11.07	19.28	25.06	16.23	6.87	11.79
5	3.26	1.63	0.95	0.88	0.85	2.36	30.22	22.00	25.69	15.58	6.66	11.33
6	3.20	1.58	0.95	0.88	0.85	2.23	21.42	20.42	70.57	14.96	6.51	10.88
7	3.15	1.53	0.94	0.88	0.85	2.13	18.70	21.05	31.77	14.36	10.66	10.44
8	3.09	1.48	0.94	0.87	0.85	2.02	19.65	21.33	30.77	13.79	11.62	10.03
9	3.04	1.43	1.08	0.87	0.86	2.87	17.66	21.21	26.00	13.25	9.54	9.83
10	2.98	1.38	0.94	0.87	0.86	5.27	16.04	26.53	29.28	12.88	10.72	9.24
11	2.93	1.33	0.94	0.87	0.86	3.97	15.40	24.88	26.84	12.49	9.08	8.83
12	2.87	1.28	0.94	2.50	0.86	3.32	14.66	27.23	27.37	12.16	9.11	8.53
13	2.82	1.23	0.94	2.92	0.86	3.38	13.98	24.11	25.00	11.73	12.58	8.20
14	2.77	1.18	0.94	1.37	0.86	3.30	26.22	22.09	23.34	11.31	11.39	7.89
15	2.71	1.13	0.93	0.87	0.86	3.13	19.65	22.38	22.33	10.91	16.62	7.59
16	2.66	1.08	0.93	0.87	0.86	2.97	25.17	21.74	21.33	10.52	16.97	7.30
17	2.60	1.03	0.93	0.87	0.86	2.82	21.36	21.91	21.82	10.15	16.35	7.02
18	2.55	0.99	0.93	4.70	0.86	2.67	24.14	22.09	20.18	9.80	14.07	6.76
19	2.50	0.97	0.92	3.46	0.85	4.12	21.47	23.18	19.36	9.65	13.93	6.51
20	2.45	0.97	0.92	2.09	0.85	3.09	21.31	34.34	18.53	9.58	12.68	6.27
21	2.40	0.97	0.92	1.96	0.85	2.95	21.52	27.59	18.38	9.31	13.03	6.03
22	2.34	0.97	0.91	1.81	0.84	2.89	19.24	25.74	17.50	9.03	12.16	5.82
23	2.29	0.97	0.91	1.64	0.84	2.88	17.84	24.65	17.73	8.96	11.69	5.60
24	2.24	0.97	0.91	1.49	0.84	4.60	21.91	22.97	17.62	9.36	12.39	5.40
25	2.19	0.97	0.90	1.38	0.84	8.01	21.62	21.61	16.43	8.91	13.84	5.20
26	2.14	0.96	0.90	1.25	0.83	19.82	51.48	20.28	15.92	8.88	12.07	5.01
27	2.09	0.96	0.90	1.11	0.83	14.74	38.37	19.49	15.31	8.80	12.99	4.83
28	2.04	0.96	0.90	0.97	0.83	39.12	27.83	18.76	21.23	8.55	11.64	4.75
29	1.99	0.96	0.90	0.86	0.83	17.98	25.09	21.87	19.39	8.30	13.08	4.72
30	1.94	0.96	0.89	0.86	0.83	16.12	24.97	20.61	20.61	8.05	11.57	4.68
31	1.89	0.96	0.89	0.86	0.83	23.64	19.12	19.12	15.31	7.80	6.51	4.65
TOTAL	82.73	35.01	28.87	42.64	57.13	188.68	711.75	700.55	741.29	360.47	341.79	247.36
AVERAGE	2.67	1.25	0.93	1.42	1.84	6.29	22.96	22.60	24.71	11.63	11.39	7.98
MAXIMUM	3.48	1.84	1.08	4.70	6.49	39.12	51.48	34.34	70.57	19.65	16.97	16.28
MINIMUM	1.89	0.96	0.89	0.86	0.85	2.02	12.62	19.12	15.31	7.80	6.51	4.65

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 ANNUAL  
 \*\*\*\*\*  
 TOTAL | AVERAGE | MAXIMUM | MINIMUM |  
 3538.27 | 9.69 | 70.57 | 0.85 |  
 \*\*\*\*\*

XE KATAM CALCULATED

DAY*	1	2	3	4	5	6	7	8	9	10	11	12
1	4.62	3.04	1.50	0.94	3.00	7.62	18.96	11.83	19.13	27.18	12.70	5.09
2	4.58	2.98	1.45	0.94	2.83	7.24	18.06	20.85	17.01	27.82	12.23	5.05
3	4.54	2.92	1.40	0.94	2.75	6.86	17.17	17.56	17.12	28.08	11.77	5.02
4	4.51	2.87	1.35	0.94	9.69	6.50	16.32	18.19	15.54	26.24	11.46	4.98
5	4.47	2.81	1.30	0.94	14.80	6.15	15.52	15.82	14.85	24.12	11.08	4.94
6	4.44	2.75	1.25	0.93	11.45	6.03	14.90	14.16	25.05	22.87	10.69	4.90
7	4.40	2.70	1.20	0.93	22.41	7.99	15.03	13.40	21.71	22.55	10.31	4.86
8	4.37	2.64	1.15	0.93	13.62	13.51	14.11	14.39	19.08	23.23	9.94	4.83
9	4.33	2.58	1.11	0.92	13.30	10.56	13.51	12.99	17.24	22.57	9.58	4.79
10	4.30	2.53	1.06	0.92	43.05	8.62	12.89	12.49	16.19	20.55	9.24	4.75
11	4.25	2.47	1.05	0.92	17.91	7.96	12.29	12.96	15.51	19.67	8.91	4.71
12	4.19	2.42	1.02	0.92	14.90	13.22	11.83	11.99	14.84	18.89	8.67	4.67
13	4.13	2.36	0.99	0.91	13.26	39.06	11.31	11.67	14.20	24.93	8.41	4.63
14	4.07	2.31	0.98	0.91	12.55	18.86	10.81	18.19	13.60	21.92	8.14	4.59
15	4.01	2.25	0.98	0.91	11.86	15.62	10.33	34.35	13.02	19.60	7.88	4.55
16	3.95	2.20	0.98	0.90	16.05	13.72	11.18	19.35	12.47	18.55	7.63	4.49
17	3.89	2.14	0.95	0.90	13.53	12.46	13.69	19.38	11.96	17.76	7.38	4.43
18	3.83	2.08	0.98	0.90	11.84	11.87	11.70	17.78	11.57	18.44	7.14	4.37
19	3.77	2.03	0.98	0.90	11.22	11.28	14.94	16.61	11.28	16.95	6.91	4.31
20	3.72	1.98	0.98	0.89	10.62	10.72	18.36	15.33	13.36	19.75	6.68	4.25
21	3.66	1.92	0.98	0.89	10.03	10.18	19.54	15.30	11.82	17.67	6.47	4.20
22	3.60	1.87	0.98	0.89	9.48	9.66	18.07	14.75	11.23	16.38	6.27	4.14
23	3.54	1.82	0.97	0.89	8.94	9.37	16.05	22.50	10.87	15.73	6.11	4.08
24	3.48	1.76	0.97	0.89	8.43	10.44	14.66	21.93	10.19	15.08	5.95	4.02
25	3.43	1.71	0.97	0.89	7.95	95.85	13.98	21.28	18.63	15.75	5.79	3.96
26	3.37	1.65	0.96	0.88	7.50	101.65	13.53	19.86	17.88	16.36	5.62	3.91
27	3.32	1.60	0.96	0.88	10.73	36.69	13.03	18.29	21.55	14.74	5.46	3.85
28	3.26	1.55	0.96	0.88	9.22	24.42	12.46	16.72	34.07	14.70	5.30	3.80
29	3.21	1.50	0.95	0.88	8.87	21.59	11.91	16.05	32.63	13.93	5.17	3.74
30	3.15	1.45	0.95	0.88	8.86	19.68	11.38	17.51	26.25	13.39	5.13	3.69
31	3.09	1.40	0.95	0.88	8.00	10.88	10.88	16.51	13.09	13.09	5.09	3.63
TOTAL	121.48	63.94	33.29	48.55	395.65	573.38	444.40	529.99	513.85	608.79	244.02	137.23
AVERAGE	3.92	2.28	1.07	0.92	12.76	19.18	14.34	17.10	17.13	19.64	8.13	4.43
MAXIMUM	4.62	3.04	1.50	0.94	43.05	101.65	19.54	34.35	34.07	28.08	12.70	5.09
MINIMUM	3.09	1.55	0.95	0.89	2.83	6.03	10.33	11.67	10.87	13.09	5.13	3.63

\*\*\*\*\* ANNUAL \*\*\*\*\*  
 \*\*\*\*\* TOTAL AVERAGE MAXIMUM MINIMUM \*\*\*\*\*  
 \*\*\*\*\* 3716.57 10.18 101.65 0.89 \*\*\*\*\*

XE KATAM CALCULATED

DAY*	1	2	3	4	5	6	7	8	9	10	11	12
1	3.57	1.98	0.99	0.96	1.62	2.65	17.07	48.69	82.48	20.58	15.07	10.05
2	3.52	1.94	0.99	0.96	1.54	2.52	15.30	61.10	74.57	20.35	14.60	9.71
3	3.46	1.89	0.99	0.96	3.79	4.05	15.38	26.31	39.31	19.28	15.05	9.39
4	3.41	1.84	0.99	0.96	2.35	3.57	14.73	23.83	37.49	19.59	14.10	9.08
5	3.35	1.79	0.99	0.96	2.14	3.06	13.75	23.07	34.24	24.47	13.62	8.78
6	3.30	1.74	0.99	0.96	2.02	3.69	13.15	21.40	55.75	25.39	13.16	8.49
7	3.24	1.69	0.99	3.04	1.89	3.55	12.56	20.03	37.83	28.27	12.70	8.21
8	3.19	1.64	0.99	2.71	1.76	3.84	14.04	20.30	34.75	26.23	18.40	7.95
9	3.13	1.59	0.98	2.99	1.62	3.63	12.75	19.73	35.92	24.23	16.30	7.70
10	3.08	1.55	0.98	2.75	2.34	3.60	11.87	15.87	35.21	26.26	16.25	7.45
11	3.03	1.50	0.98	1.92	4.52	6.56	11.40	23.69	51.26	24.34	14.43	7.22
12	2.97	1.46	0.98	1.80	2.61	8.23	13.19	24.74	38.78	22.27	17.72	7.00
13	2.92	1.42	0.98	1.65	2.58	6.29	11.64	23.10	41.83	29.40	21.71	6.78
14	2.87	1.38	0.97	1.49	2.44	5.66	10.92	50.04	38.80	27.17	23.57	6.57
15	2.81	1.36	13.57	1.32	2.30	5.33	10.41	48.76	36.22	27.58	20.32	6.37
16	2.76	1.33	7.42	1.16	2.15	5.54	9.93	57.35	33.96	26.38	18.10	6.31
17	2.71	1.31	4.22	1.01	2.01	5.35	11.85	90.79	32.05	24.84	16.91	6.26
18	2.66	1.27	3.09	0.97	1.86	6.50	13.48	60.74	30.12	23.27	16.27	6.21
19	2.60	1.23	2.88	0.98	1.73	11.65	37.27	31.55	21.93	15.84	15.64	6.16
20	2.55	1.19	2.54	0.98	1.59	9.01	10.43	34.83	29.55	21.11	15.05	6.11
21	2.50	1.15	2.39	0.99	1.46	11.20	10.02	36.37	28.87	20.26	14.48	6.06
22	2.45	1.14	2.16	0.99	1.37	17.30	9.57	33.01	27.27	19.44	13.93	6.01
23	2.40	1.12	1.93	1.00	3.68	14.94	9.25	30.34	26.68	18.67	13.41	5.96
24	2.36	1.09	1.71	2.66	3.78	13.41	8.85	28.88	27.08	17.93	12.92	5.91
25	2.31	1.05	1.48	1.17	2.90	18.92	8.46	27.32	25.07	17.23	12.45	5.86
26	2.27	1.02	1.28	1.12	3.43	15.64	8.08	34.93	23.87	16.56	12.00	5.81
27	2.22	0.99	1.08	1.04	3.85	17.75	7.78	31.78	22.86	15.93	11.57	5.76
28	2.17	0.99	0.86	1.03	3.29	18.64	8.72	29.52	21.98	15.33	11.16	5.71
29	2.13	0.99	0.96	2.22	3.14	21.34	9.46	31.08	21.58	14.77	10.78	5.64
30	2.08	0.96	0.96	2.11	2.98	18.87	30.63	42.83	21.26	18.82	10.41	5.57
31	2.03	0.96	0.96	2.82	2.82	34.74	34.74	36.82	21.26	16.56	10.41	5.49
TOTAL	86.05	40.64	63.48	44.86	77.66	269.43	401.06	1120.27	1078.19	674.44	452.08	215.58
AVERAGE	2.79	1.40	2.05	1.50	2.51	8.98	12.94	36.14	35.94	21.76	15.07	6.95
MAXIMUM	3.57	1.98	13.57	3.04	4.52	21.34	34.74	90.79	82.48	29.40	23.57	10.05
MINIMUM	2.03	0.99	0.96	0.96	1.37	2.52	7.78	19.73	21.26	14.77	10.41	5.49

\*\*\*\*\* ANNUAL \*\*\*\*\*  
 \*\*\*\*\* TOTAL AVERAGE MAXIMUM MINIMUM \*\*\*\*\*  
 \*\*\*\*\* 4523.74 \*\*\*\*\* 12.36 \*\*\*\*\* 90.79 \*\*\*\*\* 0.96 \*\*\*\*\*

1985

XE KATAM CALCULATED

*DAY*	1	2	3	4	5	6	7	8	9	10	11	12
1	5.42	3.40	3.21	1.88	12.22	7.99	20.86	17.61	24.68	14.39	9.48	6.34
2	5.35	3.34	3.24	1.85	11.62	7.67	20.11	17.43	30.14	14.01	9.19	6.17
3	5.27	3.28	3.23	1.81	11.04	7.35	22.18	17.71	26.89	13.55	8.91	6.00
4	5.20	3.22	3.26	1.79	10.48	7.04	21.57	22.89	25.16	13.10	8.64	5.82
5	5.13	3.16	3.15	1.74	9.95	6.74	21.26	20.50	28.58	12.66	8.38	5.65
6	5.06	3.10	3.01	1.76	9.42	6.45	19.98	18.69	29.93	12.24	8.14	5.95
7	4.99	3.04	2.86	1.77	9.18	6.19	26.16	19.62	27.27	11.83	8.08	5.67
8	4.91	2.98	2.70	1.72	8.75	5.93	60.07	35.90	24.94	11.56	7.90	5.55
9	4.84	2.92	2.53	1.67	8.32	5.70	34.29	23.87	28.49	17.39	7.92	5.42
10	4.77	2.86	2.38	1.64	7.91	5.47	58.61	23.48	29.27	15.01	7.74	5.29
11	4.71	2.80	2.37	1.60	7.50	5.24	50.80	42.69	31.42	13.31	7.56	5.26
12	4.64	2.76	2.36	1.58	7.11	5.11	28.58	29.03	28.41	12.67	7.36	5.22
13	4.56	2.71	2.35	1.54	6.73	4.98	27.13	37.82	28.87	12.31	7.02	5.19
14	4.50	2.68	2.34	1.51	6.36	4.85	24.90	37.70	26.59	11.92	6.80	5.15
15	4.43	2.67	2.34	1.48	6.00	4.72	23.51	29.54	25.15	11.54	6.60	5.11
16	4.36	2.64	2.33	1.45	5.65	4.60	31.03	28.85	24.63	15.88	6.42	5.07
17	4.30	2.60	2.31	1.42	5.31	4.48	27.94	26.84	24.86	14.92	6.22	5.04
18	4.23	2.56	2.30	1.39	5.00	4.36	27.99	24.97	24.49	15.48	6.04	5.00
19	4.16	2.51	2.29	1.36	4.70	4.24	26.21	24.81	22.68	13.68	5.84	4.96
20	4.09	2.46	2.28	1.33	4.42	4.12	23.99	25.93	22.50	12.85	5.64	4.93
21	4.03	2.43	2.26	1.30	4.15	4.00	23.75	23.80	21.25	12.47	5.44	4.89
22	3.96	2.41	2.25	1.27	3.88	3.88	23.54	27.57	20.46	12.06	5.24	4.85
23	3.90	2.38	2.20	1.24	3.62	3.76	26.33	26.93	19.65	11.67	5.04	4.81
24	3.83	2.35	2.16	1.21	3.37	3.64	22.66	26.15	18.86	11.28	4.84	4.78
25	3.77	2.32	2.12	1.18	3.12	3.52	21.02	26.78	18.12	10.92	4.64	4.74
26	3.71	2.29	2.08	1.15	2.87	3.40	20.29	25.59	17.42	10.59	4.44	4.69
27	3.65	2.26	2.04	1.12	2.62	3.28	19.56	27.43	16.75	10.26	4.24	4.63
28	3.60	2.23	2.00	1.09	2.37	3.16	20.41	38.82	16.11	11.49	4.04	4.57
29	3.56	2.20	1.96	1.06	2.12	3.04	19.75	29.13	15.51	10.35	3.84	4.51
30	3.51	2.17	1.94	1.03	1.87	2.92	18.30	26.91	14.94	10.06	3.64	4.45
31	3.45	2.14	1.91	1.00	1.62	2.80	18.81	24.96	14.33	9.77	3.44	4.40
TOTAL	135.89	92.72	75.86	308.15	338.03	610.40	810.22	829.95	714.02	391.22	238.42	160.11
AVERAGE	4.38	3.31	2.45	10.27	10.90	20.35	26.14	26.77	23.80	12.62	7.95	5.16
MAXIMUM	5.42	6.92	3.23	47.40	14.07	82.83	60.07	42.69	31.42	17.39	11.30	6.34
MINIMUM	3.45	2.46	1.91	1.79	8.17	5.11	18.30	17.43	14.94	9.77	6.22	4.40

\*\*\*\*\* ANNUAL \*\*\*\*\*  
 \*\*\*\*\* TOTAL AVERAGE MAXIMUM MINIMUM \*\*\*\*\*  
 \*\*\*\*\* 4704.99 12.89 82.83 1.79 \*\*\*\*\*

XE KATAM CALCULATED

*DAY*	1	2	3	4	5	6	7	8	9	10	11	12
1	4.34	2.64	1.26	1.18	4.36	11.18	9.76	14.56	24.80	24.31	13.15	5.66
2	4.28	2.59	1.25	1.18	3.27	10.61	9.31	13.90	24.97	22.62	12.67	5.62
3	4.22	2.54	1.25	1.18	3.30	10.07	9.83	13.46	24.86	20.54	12.21	5.58
4	4.17	2.49	1.25	1.18	3.12	9.55	14.40	19.63	23.01	19.40	11.76	5.53
5	4.11	2.43	1.25	1.18	7.11	9.06	12.56	26.72	23.63	18.61	11.34	5.49
6	4.05	2.38	1.25	1.18	9.11	8.69	11.95	24.22	49.17	17.86	10.93	5.45
7	3.99	2.33	1.25	1.18	6.65	8.39	10.53	24.14	29.34	17.13	10.55	5.41
8	3.94	2.28	1.25	1.18	5.50	8.00	11.96	23.26	28.83	16.44	10.18	5.37
9	3.88	2.22	1.25	1.18	9.92	7.62	10.84	45.87	28.38	15.79	9.82	5.33
10	3.82	2.17	1.25	1.18	10.03	7.29	10.46	28.40	28.39	15.18	9.49	5.28
11	3.77	2.12	1.24	1.18	11.82	7.25	11.31	25.96	28.61	14.60	9.17	5.24
12	3.71	2.07	1.24	1.18	9.45	8.49	19.48	26.99	27.59	14.05	8.91	5.20
13	3.66	2.01	1.23	1.18	7.95	9.32	16.19	45.54	29.48	13.52	8.66	5.15
14	3.60	1.96	1.23	1.18	7.55	10.76	15.42	52.52	26.60	13.03	8.56	5.12
15	3.54	1.91	1.23	1.18	7.15	13.43	17.89	34.52	24.47	14.33	8.34	5.07
16	3.49	1.86	1.22	1.18	6.80	11.15	15.69	40.80	23.17	12.97	8.11	5.01
17	3.44	1.81	1.22	1.18	7.76	9.58	15.84	31.37	22.17	12.54	7.88	4.94
18	3.38	1.76	1.22	1.18	10.43	9.03	14.76	28.99	21.22	12.47	7.65	4.88
19	3.33	1.71	1.21	1.17	13.08	8.62	59.85	26.96	20.34	12.03	7.42	4.82
20	3.27	1.65	1.21	1.17	14.80	8.22	65.20	25.99	19.49	11.64	7.20	4.76
21	3.22	1.62	1.20	1.17	12.55	7.83	26.45	24.72	18.66	11.25	6.99	4.70
22	3.17	1.58	1.20	1.17	11.98	7.46	25.40	23.67	17.91	20.18	6.78	4.64
23	3.11	1.54	1.20	1.17	11.01	15.73	22.61	22.76	17.17	20.59	6.59	4.58
24	3.06	1.49	1.19	1.17	16.72	16.21	20.57	26.19	16.48	17.54	6.39	4.52
25	3.01	1.44	1.19	1.17	18.25	14.09	19.46	24.43	16.61	15.52	6.21	4.46
26	2.96	1.40	1.19	1.17	14.35	13.35	18.70	25.38	22.57	14.64	6.03	4.40
27	2.90	1.35	1.19	1.17	17.09	12.67	18.00	26.71	33.33	16.52	5.86	4.34
28	2.85	1.30	1.19	1.17	14.50	11.38	17.35	79.58	23.61	15.46	5.78	4.28
29	2.80	1.25	1.19	1.17	13.56	10.74	16.85	31.68	24.28	14.88	5.74	4.22
30	2.75	1.20	1.19	1.17	12.33	10.24	15.98	28.72	22.89	14.14	5.70	4.17
31	2.70	1.15	1.19	1.17	11.77	10.24	15.35	26.72	22.89	13.65	5.70	4.10
TOTAL	108.52	54.65	42.44	57.68	311.27	306.01	579.83	908.36	763.25	493.43	256.07	153.33
AVERAGE	3.50	1.95	1.37	1.92	10.04	10.20	18.70	29.30	24.78	15.92	8.54	4.95
MAXIMUM	4.34	2.64	5.55	8.21	17.09	16.21	65.20	73.98	49.17	24.31	13.15	5.66
MINIMUM	2.70	1.30	1.19	1.17	3.12	7.85	9.31	13.46	16.48	11.25	5.70	4.10

\*\*\*\*\* ANNUAL \*\*\*\*\*  
 \*\*\*\*\* TOTAL AVERAGE MAXIMUM MINIMUM \*\*\*\*\*  
 \*\*\*\*\* 4014.64 11.00 73.58 1.17 \*\*\*\*\*

1987

XE KATAM CALCULATED

DAY*	1	2	3	4	5	6	7	8	9	10	11	12
1	4.05	2.37	1.26	1.16	1.12	1.10	4.55	11.31	20.26	14.18	11.01	4.59
2	3.99	2.32	1.26	1.16	1.12	1.10	7.04	10.54	19.44	13.73	10.21	4.56
3	3.93	2.26	1.26	1.16	1.12	1.10	32.26	10.25	20.63	13.45	9.85	4.53
4	3.88	2.21	1.25	1.15	1.11	1.10	14.95	12.80	19.16	18.28	9.50	4.50
5	3.82	2.16	1.25	1.15	1.11	1.10	15.42	11.53	18.27	18.30	9.15	4.47
6	3.76	2.11	1.25	1.15	1.11	1.10	15.35	10.34	19.95	16.18	8.82	4.44
7	3.71	2.05	1.25	1.14	1.11	1.10	13.22	11.56	20.47	14.86	8.50	4.41
8	3.65	2.00	1.24	1.14	1.11	1.10	12.79	11.89	18.57	14.57	8.19	4.38
9	3.59	1.95	1.24	1.14	1.11	2.72	11.85	14.56	18.31	14.56	7.89	4.34
10	3.53	1.90	1.24	1.14	1.11	2.14	11.34	13.08	17.81	15.75	7.61	4.31
11	3.48	1.85	1.23	1.13	1.11	1.09	10.51	11.53	20.19	15.72	7.34	4.28
12	3.42	1.80	1.23	1.13	1.11	1.09	12.87	12.03	18.14	16.28	7.30	4.25
13	3.37	1.75	1.23	1.13	1.11	3.03	13.53	10.87	16.56	17.96	7.10	4.22
14	3.31	1.70	1.22	1.12	1.11	1.80	72.19	10.43	15.90	15.84	6.89	4.19
15	3.26	1.64	1.22	1.12	1.11	1.63	35.97	9.96	15.21	14.44	6.67	4.15
16	3.21	1.59	1.22	1.12	1.11	1.63	21.58	17.86	14.55	13.93	6.46	4.12
17	3.15	1.54	1.21	1.12	1.11	1.56	19.34	18.01	13.92	15.49	6.25	4.08
18	3.09	1.49	1.21	1.12	1.83	5.52	18.06	20.72	13.32	13.88	6.05	4.03
19	3.04	1.44	1.20	1.12	1.11	5.64	17.92	18.03	12.75	13.25	7.08	3.97
20	2.99	1.39	1.20	1.12	1.11	5.65	16.65	60.48	12.21	12.87	6.25	3.92
21	2.94	1.34	1.20	1.12	1.11	8.60	15.53	99.09	14.06	12.37	6.10	3.87
22	2.88	1.29	1.19	1.12	2.84	6.36	14.86	58.10	16.77	11.88	5.92	3.82
23	2.83	1.26	1.19	1.12	1.10	5.06	14.21	31.72	20.29	11.42	5.75	3.76
24	2.78	1.26	1.19	1.12	1.10	4.80	13.50	29.16	20.22	10.97	5.57	3.71
25	2.73	1.26	1.18	1.88	1.10	4.54	12.83	26.23	17.92	10.56	5.42	3.66
26	2.68	1.26	1.18	1.43	1.10	4.28	12.19	24.04	16.19	10.16	5.28	3.61
27	2.62	1.26	1.18	1.12	1.10	4.03	11.58	23.46	17.63	9.77	5.13	3.55
28	2.57	1.26	1.17	1.12	1.10	3.79	11.23	24.44	15.80	10.78	4.99	3.50
29	2.52	1.26	1.17	1.12	1.10	3.56	11.58	25.46	16.23	9.84	4.85	3.45
30	2.47	1.26	1.17	1.12	1.10	3.36	11.12	23.00	14.75	10.87	4.71	3.40
31	2.42	1.26	1.17	1.12	1.10	3.16	12.94	21.15	12.76	12.76	4.57	3.35
TOTAL	99.67	47.71	37.66	36.76	37.40	90.68	518.96	593.83	515.48	429.90	211.84	125.42
AVERAGE	3.22	1.70	1.21	1.23	1.21	3.02	16.74	22.38	17.18	13.87	7.06	4.05
MAXIMUM	4.05	2.37	1.26	2.64	2.84	8.60	72.19	99.09	20.63	18.45	11.01	4.59
MINIMUM	2.42	1.26	1.17	1.12	1.10	1.09	4.55	9.96	12.21	9.77	4.71	3.35

\*\*\*\*\*  
 ANNUAL  
 \*\*\*\*\*  
 TOTAL AVERAGE MAXIMUM MINIMUM  
 \*\*\*\*\*  
 2845.11 7.79 99.09 1.09  
 \*\*\*\*\*

XE KATAM CALCULATED

1986

DAY*	1	2	3	4	5	6	7	8	9	10	11	12
1	3.30	1.81	1.12	1.03	5.29	16.91	7.35	20.28	12.79	8.62	10.25	4.16
2	3.25	1.77	1.11	1.03	4.33	17.92	8.38	30.98	12.24	8.49	9.83	4.13
3	3.20	1.72	1.11	1.03	4.07	19.45	7.39	20.81	11.72	8.17	9.44	4.10
4	3.15	1.67	1.11	1.03	3.82	16.45	9.63	18.18	11.23	11.69	9.06	4.07
5	3.10	1.62	1.10	1.02	3.58	19.01	8.68	15.88	10.75	9.89	8.70	4.04
6	3.05	1.57	1.10	1.02	3.34	17.18	7.95	17.97	10.32	8.89	8.39	4.01
7	3.00	1.53	1.10	1.02	3.11	15.09	7.61	16.12	11.01	14.11	8.08	3.98
8	2.95	1.48	1.10	1.02	2.89	13.76	7.32	15.57	11.67	12.20	7.78	3.95
9	2.90	1.44	1.09	1.02	2.68	13.07	7.02	16.54	13.00	13.49	7.48	3.92
10	2.85	1.40	1.09	1.02	2.48	12.41	6.77	18.27	15.26	12.45	7.21	3.89
11	2.80	1.36	1.09	1.01	2.29	11.85	7.56	16.21	13.24	13.17	6.94	3.86
12	2.75	1.31	1.08	1.20	2.11	11.48	6.88	18.53	12.37	13.60	6.68	3.82
13	2.71	1.27	1.08	1.01	20.22	15.09	6.67	39.67	11.64	14.18	6.43	3.77
14	2.65	1.23	1.07	1.01	12.47	13.50	8.41	22.63	11.21	13.85	6.20	3.72
15	2.61	1.18	1.07	8.37	20.93	12.79	9.07	20.85	10.76	15.76	5.97	3.67
16	2.56	1.14	1.07	4.67	12.75	14.24	9.33	19.27	10.33	15.99	5.76	3.61
17	2.51	1.14	1.07	2.53	11.50	13.79	11.27	19.14	10.49	16.85	5.55	3.56
18	2.46	1.14	1.06	2.39	13.92	12.26	9.54	17.41	10.73	16.34	5.35	3.51
19	2.42	1.14	1.06	2.30	12.18	11.88	9.14	16.62	11.03	15.19	5.16	3.46
20	2.37	1.14	1.06	2.16	10.14	11.37	16.75	15.98	10.06	15.09	4.98	3.42
21	2.32	1.13	1.06	2.00	9.57	10.91	15.18	15.26	9.71	13.60	4.81	3.37
22	2.27	1.13	1.06	1.83	10.39	14.04	14.04	14.60	9.35	13.08	4.64	3.32
23	2.23	1.13	1.05	1.65	11.25	9.89	12.78	17.25	9.09	12.53	4.48	3.27
24	2.18	1.13	1.05	4.62	9.30	9.43	11.38	18.02	10.66	12.27	4.36	3.22
25	2.13	1.13	1.05	3.30	8.69	8.98	10.91	17.27	10.11	11.80	4.33	3.17
26	2.09	1.13	1.05	2.52	8.24	8.56	13.02	15.50	9.26	15.29	4.30	3.12
27	2.04	1.12	1.04	2.36	7.81	8.15	13.06	17.37	9.61	13.30	4.27	3.07
28	1.99	1.12	1.04	2.19	7.51	7.76	12.17	15.54	10.77	12.06	4.25	3.02
29	1.95	1.12	1.04	7.47	7.20	8.03	13.28	14.48	9.50	11.60	4.22	2.97
30	1.90	1.12	1.04	7.67	7.02	7.46	13.07	13.96	9.15	11.13	4.19	2.93
31	1.86	1.12	1.04	24.73	24.73	15.17	13.36	13.36	15.17	10.68	2.88	2.88
TOTAL	79.55	38.20	33.26	72.50	264.49	379.06	314.78	570.52	331.07	395.56	189.09	110.99
AVERAGE	2.57	1.32	1.07	2.42	8.53	12.64	10.15	18.40	11.04	12.76	6.30	3.58
MAXIMUM	3.30	1.81	1.12	8.37	24.73	19.45	16.75	39.67	15.26	16.85	10.25	4.16
MINIMUM	1.86	1.12	1.04	1.01	2.11	7.46	6.67	13.36	9.09	8.17	4.19	2.88

\*\*\*\*\* ANNUAL \*\*\*\*\*  
 \*\*\*\*\* TOTAL \*\*\*\*\*  
 \*\*\*\*\* AVERAGE \*\*\*\*\*  
 \*\*\*\*\* MAXIMUM \*\*\*\*\*  
 \*\*\*\*\* MINIMUM \*\*\*\*\*  
 \*\*\*\*\* 2779.07 \*\*\*\*\*  
 \*\*\*\*\* 7.59 \*\*\*\*\*  
 \*\*\*\*\* 39.67 \*\*\*\*\*  
 \*\*\*\*\* 1.01 \*\*\*\*\*

1989

XE KATAM CALCULATED

*DAY*	1	2	3	4	5	6	7	8	9	10	11	12
1	2.83	1.43	1.01	0.91	4.78	11.86	3.99	15.51	22.15	12.55	6.06	3.79
2	2.73	1.39	1.00	0.91	3.85	10.83	3.79	14.36	19.91	11.69	5.85	3.77
3	2.73	1.35	1.00	0.91	3.43	8.74	4.61	13.85	17.70	12.12	5.78	3.74
4	2.69	1.30	1.00	0.90	3.21	7.79	3.95	18.97	16.86	11.35	5.63	3.71
5	2.64	1.26	0.99	0.90	2.99	7.38	6.75	26.46	16.42	10.95	7.52	3.68
6	2.59	1.21	0.99	0.90	2.77	6.98	5.51	20.69	23.80	10.53	6.14	3.66
7	2.54	1.17	0.99	0.90	2.57	6.60	4.79	18.79	20.85	10.12	5.97	3.63
8	2.50	1.12	0.98	0.89	2.38	6.41	4.65	17.15	18.97	9.73	5.79	3.60
9	2.45	1.08	0.98	0.89	2.82	6.13	4.78	17.93	20.65	9.35	5.61	3.57
10	2.40	1.06	0.97	0.89	2.44	16.63	4.56	17.92	22.49	9.00	5.43	3.54
11	2.36	1.06	0.97	0.89	2.31	14.73	6.40	20.53	20.05	8.66	5.25	3.51
12	2.31	1.06	0.97	0.89	2.16	13.23	11.06	63.05	18.16	10.22	5.07	3.46
13	2.28	1.06	0.96	0.88	2.00	10.95	10.83	26.73	17.30	9.31	4.90	3.41
14	2.22	1.06	0.96	3.41	1.85	9.64	9.78	23.38	16.79	8.74	4.74	3.36
15	2.17	1.06	0.96	0.88	1.74	9.15	9.11	21.03	16.15	8.44	4.57	3.31
16	2.12	1.05	0.95	0.88	1.63	8.67	7.79	19.33	15.56	8.15	4.42	3.27
17	2.08	1.05	0.95	0.88	1.52	8.21	7.47	18.46	15.33	7.86	4.27	3.22
18	2.03	1.05	0.95	0.88	1.40	7.78	7.26	17.74	14.79	7.77	4.15	3.17
19	1.99	1.05	0.94	0.88	1.31	7.36	9.05	18.94	15.12	8.66	4.12	3.12
20	1.95	1.04	0.94	5.22	1.21	6.97	10.60	19.44	16.33	8.12	4.09	3.07
21	1.91	1.04	0.94	2.22	1.11	6.59	9.79	17.88	15.44	7.85	4.06	3.03
22	1.87	1.04	0.93	1.42	1.01	6.24	9.54	16.87	14.10	7.58	4.04	2.98
23	1.82	1.03	0.93	1.32	0.90	5.90	79.40	16.24	13.60	7.32	4.01	2.93
24	1.78	1.03	0.93	1.20	1.61	5.59	27.40	16.09	13.07	7.06	3.98	2.88
25	1.74	1.02	0.92	1.07	1.37	5.29	19.05	15.67	12.76	7.14	3.95	2.84
26	1.70	1.02	0.92	1.09	1.04	5.03	17.76	15.92	14.35	6.92	3.92	2.79
27	1.65	1.02	0.92	1.04	9.85	4.84	16.53	15.46	12.88	7.32	4.69	2.75
28	1.61	1.01	0.92	6.59	7.43	4.62	15.17	17.53	12.25	6.89	3.87	2.70
29	1.56	1.01	0.92	6.61	6.27	4.41	15.23	17.94	11.77	6.69	3.85	2.65
30	1.52	1.01	0.91	4.05	6.06	4.20	16.45	15.98	14.25	6.47	3.82	2.60
31	1.48	1.01	0.91	8.09	5.09	17.12	17.12	15.34	15.34	6.26	3.82	2.56
TOTAL	66.28	31.12	29.61	51.30	114.64	238.76	380.17	611.18	499.85	272.82	145.35	100.30
AVERAGE	2.14	1.11	0.96	1.71	3.70	7.96	12.26	19.72	16.66	8.80	4.85	3.24
MAXIMUM	2.83	1.43	1.01	0.91	4.78	11.86	3.99	15.51	22.15	12.55	6.06	3.79
MINIMUM	1.48	1.01	0.91	0.88	1.01	4.20	3.79	13.85	11.77	6.26	3.82	2.56

\*\*\*\*\*  
 ANNUAL  
 \*\*\*\*\*  
 TOTAL | AVERAGE | MAXIMUM | MINIMUM  
 | 2541.38 | 6.96 | 79.40 | 0.88  
 \*\*\*\*\*



1990

XE KATAM CALCULATED

#DAY*	1	2	3	4	5	6	7	8	9	10	11	12
1	2.51	1.16	0.88	4.57	0.90	3.60	10.92	10.04	19.66	16.04	9.78	4.59
2	2.47	1.11	2.62	4.24	0.90	3.41	12.81	8.66	19.38	15.15	9.41	4.36
3	2.42	1.07	0.88	3.90	1.67	3.22	11.75	9.97	17.57	20.43	9.05	4.33
4	2.38	1.03	0.88	3.56	0.91	3.06	10.57	13.15	23.77	19.93	8.70	4.29
5	2.33	0.98	0.88	3.24	0.91	2.89	10.08	13.75	23.26	19.04	8.37	4.26
6	2.28	0.94	0.87	2.92	0.91	2.72	9.58	12.78	20.36	21.14	8.06	4.23
7	2.24	0.93	0.87	2.62	5.67	2.55	9.11	11.13	18.50	23.04	7.75	4.19
8	2.19	0.93	0.87	2.34	3.19	2.39	10.25	10.32	19.27	21.65	10.58	4.16
9	2.15	0.93	0.87	2.07	2.12	2.23	9.08	9.89	23.33	19.25	9.02	4.13
10	2.10	0.93	0.87	1.82	2.01	5.13	8.98	9.44	23.75	17.51	8.30	4.09
11	2.06	0.93	0.87	1.57	1.88	3.91	8.51	9.01	21.19	16.79	8.03	4.05
12	2.02	0.93	0.87	1.35	1.74	3.68	8.18	8.59	20.03	16.04	7.76	4.02
13	1.97	0.93	0.87	1.14	4.61	3.39	8.61	8.35	18.36	15.32	7.49	3.96
14	1.93	0.93	0.86	1.38	2.74	7.29	7.91	8.01	17.35	17.58	7.22	3.90
15	1.89	0.93	0.86	1.20	2.46	6.72	7.59	7.66	16.57	19.35	6.96	3.85
16	1.84	0.93	0.86	1.10	2.32	8.14	7.24	7.35	15.80	17.18	6.71	3.79
17	1.80	0.92	0.86	0.97	2.34	8.88	6.90	7.05	15.07	15.49	6.47	3.74
18	1.76	0.92	0.85	0.86	2.43	8.70	6.58	6.74	14.57	14.97	6.24	3.68
19	1.71	0.92	0.85	0.86	2.37	7.40	9.45	6.65	20.71	19.15	6.02	3.63
20	1.67	0.91	0.85	0.87	2.25	8.98	7.89	10.82	21.55	16.85	5.81	3.57
21	1.62	0.91	0.84	0.87	2.11	9.49	7.53	10.09	19.46	15.77	5.60	3.51
22	1.58	0.91	0.84	0.88	1.99	8.89	11.58	8.98	18.97	14.72	5.41	3.46
23	1.54	0.90	0.84	0.88	3.32	10.38	10.44	8.00	21.38	14.13	5.22	3.41
24	1.50	0.90	0.84	0.88	5.83	13.47	9.08	7.75	19.67	13.60	5.04	3.35
25	1.45	0.89	0.83	0.89	6.43	13.81	8.34	14.86	17.85	13.04	4.86	3.30
26	1.41	0.89	0.83	1.43	6.44	14.89	8.33	15.82	16.79	12.50	4.70	3.25
27	1.37	0.89	1.12	0.89	4.77	13.60	7.94	13.99	16.29	11.99	4.54	3.19
28	1.33	0.88	29.11	0.89	4.38	14.19	7.78	13.12	15.61	11.50	4.48	3.14
29	1.29	1.09	14.09	0.90	4.23	12.26	9.08	51.22	19.87	11.03	4.45	3.08
30	1.24	7.37	0.90	0.90	4.01	11.10	9.18	39.87	17.53	10.58	4.42	3.03
31	1.20	4.87	0.86	0.86	3.80	8.76	8.76	22.59	14.57	10.16	4.42	2.98
TOTAL	57.25	26.43	80.67	51.99	91.64	220.42	280.03	395.65	571.47	500.92	206.65	116.32
AVERAGE	1.85	0.94	2.60	1.73	2.96	7.35	9.03	12.76	19.05	16.16	6.88	3.75
MAXIMUM	2.51	1.16	29.11	4.57	6.44	14.89	12.81	51.22	23.75	23.04	10.58	4.29
MINIMUM	1.20	0.88	0.83	0.86	0.90	2.23	6.58	6.65	14.57	10.16	4.42	2.98

\*\*\*\*\*  
 ANNUAL  
 \*\*\*\*\*  
 TOTAL | AVERAGE | MAXIMUM | MINIMUM  
 \*\*\*\*\*  
 2599.24 | 7.12 | 51.22 | 0.83  
 \*\*\*\*\*

Some observations in chpt 3 Meteorology and Hydrology  
of the interim report July '91

- ① Precipitation estimation areal rainfall from point measurement and rainfall variation with altitude  $h$ .
- ② Temperature: in addition, Temperature at ATTAPU compare with 2 periods 1900-1900 and 1970.  
Potential Evapotranspiration (Penman) in 1970, ATT. at Pakse 1983, monthly mean temp. were completed in table 3.3/2. Exceptional temp. were observed during 82/83 ENSO event.
- ③ Temperature Regimes: AT PAKSE: (T<sub>h</sub>) and (T<sub>T</sub>) at Paksong
- ④ Evaporation, Evapotranspiration Potential PET at Pakse 1560 mm/year and at Paksong 1192 mm/year
- ⑤ 'PET' at Xiengkhouang Plain of Jarres computed with 4 methods (to compare). Pick reaching are not <sup>use</sup>
- ⑥ Assessment of Length of the main wet Period at NIKHOM 34 and AT PAKSONG TOWN. (relation between P and PET)
- ⑦ AT PAKSE in the plain The LWP is shorter than in the Plateau and the availability of moisture is also smaller.
- ⑧ Summarizing table of the surface wind in 1986 at NIKHOM 34 and at PAKSONG (more clear for utilization for general purpose)
- ⑨ Surface wind rose, 1986 in Dec, the NE prevailing wind direction and mean velocity of about 7.0 m/s produced late rainfall for the dry season.  
9a and 9b wind rose for the period of 1983-87 AT PAKSONG and 1961-1980 AT PAKSE respectively.
- ⑩ Remoff calculation from Water Balance 1986 data at NK34
- ⑪ calculations of peak Flood Discharges at Project sites (for information only)

Vientiane 22 July 91  
D. Sivilly

## ① Precipitations

Some observations on the estimation of the areal rainfall from point's rainfall.

In general, the average areal rainfall inputs to the area (catchment) are normally estimated from observation made of a number of gauging stations over the basin by using one or several methods of computation. For example, for flood forecast purpose, the standard error in areal average rainfall is given as  $e = C_v / \sqrt{n}$ , where  $C_v = \left[ \frac{\int_A (x-p)^2 dA}{A P^2} \right]^{1/2}$

and  $x$  = the point rainfall for a given duration,  $P$  = the areal rainfall for the same duration,  $p$  = the average areal rainfall of that duration, and  $A$  = the area with  $n$  gauging stations distributed at random (see Relative errors of rain gauge networks after Ishizaki, 1979) where the minimum of number of stations  $n = 10 - 100$ . Another logical approach to design of forecast rain gauge network after Sugawara (1981) by using a statistical evaluation of spatial and temporal distribution of sampled showers in temperate and tropical climates, ex: in temperate zones five rain gauges are sufficient (Japan) to provide a representative sample of the rainfall for forecast purposes, but in tropical zones this number is 15 (3 times more) being independent of the size of the basin.

In case of the Bolaven Plateau especially in the project area only one or two stations (not exactly located in the catchment area).

Although a trend of decrease in rainfall amount is seen from the west to the east (Paksong km 42 to Nakhom 34), uncertainties still exist in the areas surrounding the peaks of 1500m and 1700m altitude, located in the ridge of the limit of the catchment where the exposure to the South west air flow will receive more rainfall than that observed in 1200m ---

Solution: From Fogel - Duckstein model: (Small mountainous watershed data based on 1 single rain gauge) mainly on the variation of rainfall with altitude  $h$  by using regression equation of daily rainfall with altitude.

The study by Fogel and Duckstein of thunderstorms in the Tucson area (ARIZONA) shows rainfall amounts at storm centers to be distributed as Type I extremal (Gumbel) and thunderstorm rainfall given there is precipitation somewhere on the watershed from a thunderstorm, to be distributed exponentially. Example in the Santa Catalina Mountains adjacent to Tucson, gives the following regression equation of daily rainfall with altitude  $h$  (feet)

$$R(h) = 0.065675 + 0.00001791(-h - 2500)$$

Assuming that the difference in rainfall with altitude is due to a difference frequency of storms and assuming unit likelihood of storm at 2500 feet (762m), the relative likelihood of a storm at altitude  $h$  is

$$L(h) = [1 + 0.0002727(h - 2500)]$$

This equation is obtained by dividing  $R(h)$  above by 0.065675; the relative likelihood of a storm at 3500 ft is 1.27. This means a storm at 3500 feet (1067m) is 1.27 times as likely as a storm at 2500 feet. The different height is 307 m.

In the Bolaven Plateau with a different of 307 m (5000 feet - 3936 feet = 1064 feet or 324 m) is it possible or why not to consider the relative likelihood of storm of 1.27 at 1500 m altitude

Example: MAXI DAILY RAINFALL 29/AUGUST '90 from Typhoon BECKY

At Paksong 42 = 307.5 mm

Pak Song Town = 202.4 mm

~~Phong Hin~~ = 104.1 mm  $\rightarrow$  at 1500 m = 133 mm at least

Nikhom 34 = 75.1 mm  $\rightarrow$  at 1700 m  $\approx$  150 mm

① Temperature at ATTARU STATION ( $14^{\circ}48'N/106^{\circ}50'E$ ,  
EL: 106m)

AVERAGE FROM 1900-1910

	Jan	Feb	Mar	Apr	May	Jun	Jly	Aug	Sept	Oct	Nov	Dec	ANN
mean t <sup>o</sup> c	24.6	25.6	28.6	29.5	28.3	27.2	26.8	26.9	26.7	27.1	25.5	24.8	26.8
mean Maxi	32.2	33.2	35.9	35.8	33.5	31.2	30.5	30.7	30.6	31.9	31.2	30.8	
mean Mini	16.9	17.9	21.4	23.1	23.5	23.2	22.8	23.0	22.7	21.9	19.8	18.3	
<u>Absolute</u>													
Axi	39.6	38.8	41.3	40.4	41.2	35.9	34.8	34.8	36.7	37.8	35.0	38.7	
Mini	9.8	10.5	14.5	18.5	20.2	14.5	20.0	19.8	20.0	18.2	13.7	10.5	

1990

mean t <sup>o</sup> c	26.2	27.7	29.4	30.6	28.8	27.5	27.5	27.4	28.0	27.4	25.9	24.5	27.6
mean Maxi	34.1	34.9	36.3	36.0	34.2	31.0	31.6	31.5	32.4	32.6	31.7	32.2	
mean Mini	18.3	20.5	22.5	24.4	24.3	24.0	23.4	23.3	23.6	22.3	20.1	16.8	
<u>Absolute</u>													
MAXi	36.5	37.0	38.0	40.0	39.0	33.7	33.5	35.0	34.8	35.8	34.7	34.8	
Mini	14.8	15.5	20.5	22.0	22.1	22.8	21.0	21.2	21.5	19.6	16.6	14.1	

Note: Due to the global climatic change (environment chan  
all kind of air temperature recorded in 1990<sup>3</sup>)  
was higher than that observed from 1900-1910.

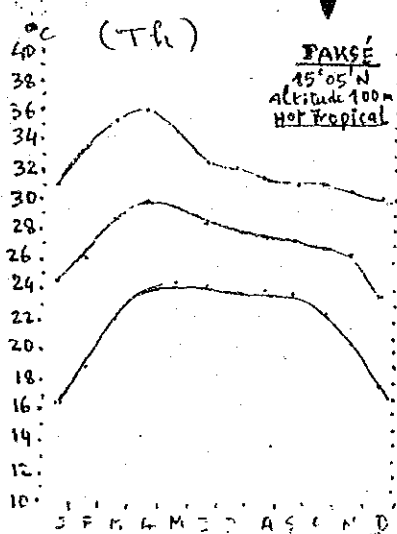
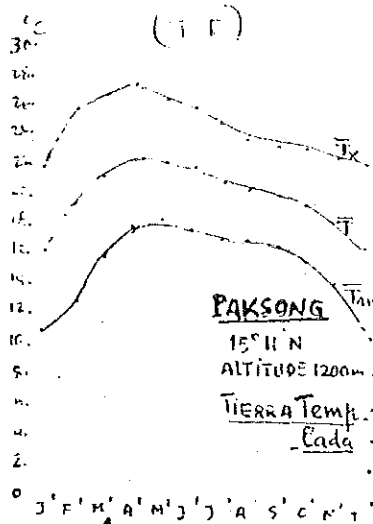
② Potential Evapotranspiration (PET from Penman's)  
1990. monthly mean and annual total

	Jan	Feb	Mar	Apr	May	Jun	Jly	Aug	Sept	Oct	Nov	Dec	ANNUAL
PET (mm)	117.8	137.4	145.0	155.7	140.7	123.0	131.4	130.8	124.2	123.6	105.0	106.6	1541.2
Piche													890.00

### ③ TEMPERATURE REGIMES (FAO 1986)

#### The Tropical Temperature regime.

The mean minimum temperature of the coldest month varies between 13°C and 18°C, while the mean minimum t° of the warmest month is higher than 20°C



#### The Tierra Templada regime

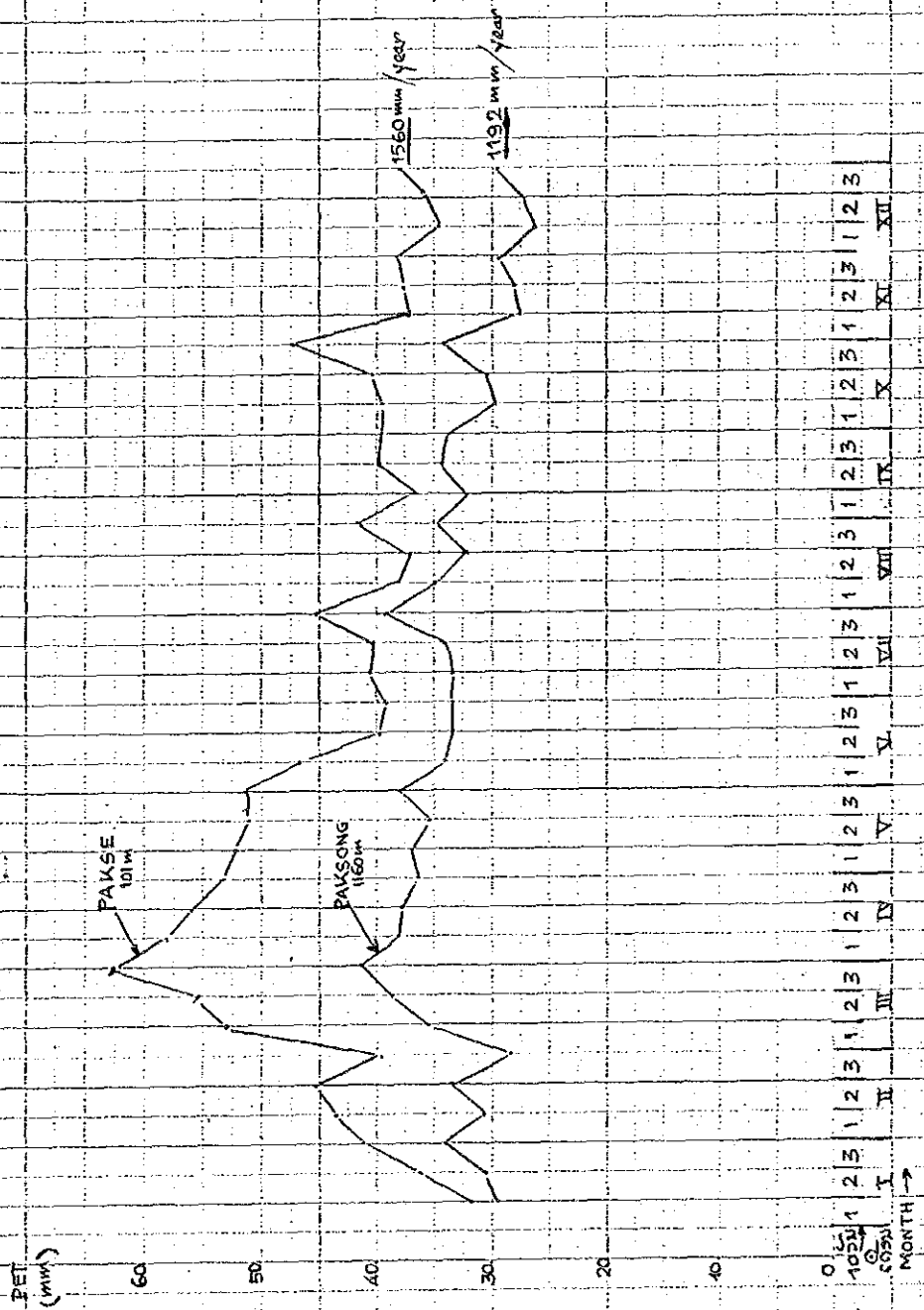
The mean minimum temperature of all months is less than 20°C, while no frost risk occurs. Exceptionally once in several years...  
(in the Bolaven Plateau)  
-2°C Jan 1983

#### \* The Tierra Fria regime

has in general mean minimum t° of the coldest months 8-13°C a certain frost risk is present. Summers are relatively hot but the mean maxi t° of the warmest month remains below 33°C  
(in the North LAOS with Alt > 1500m)

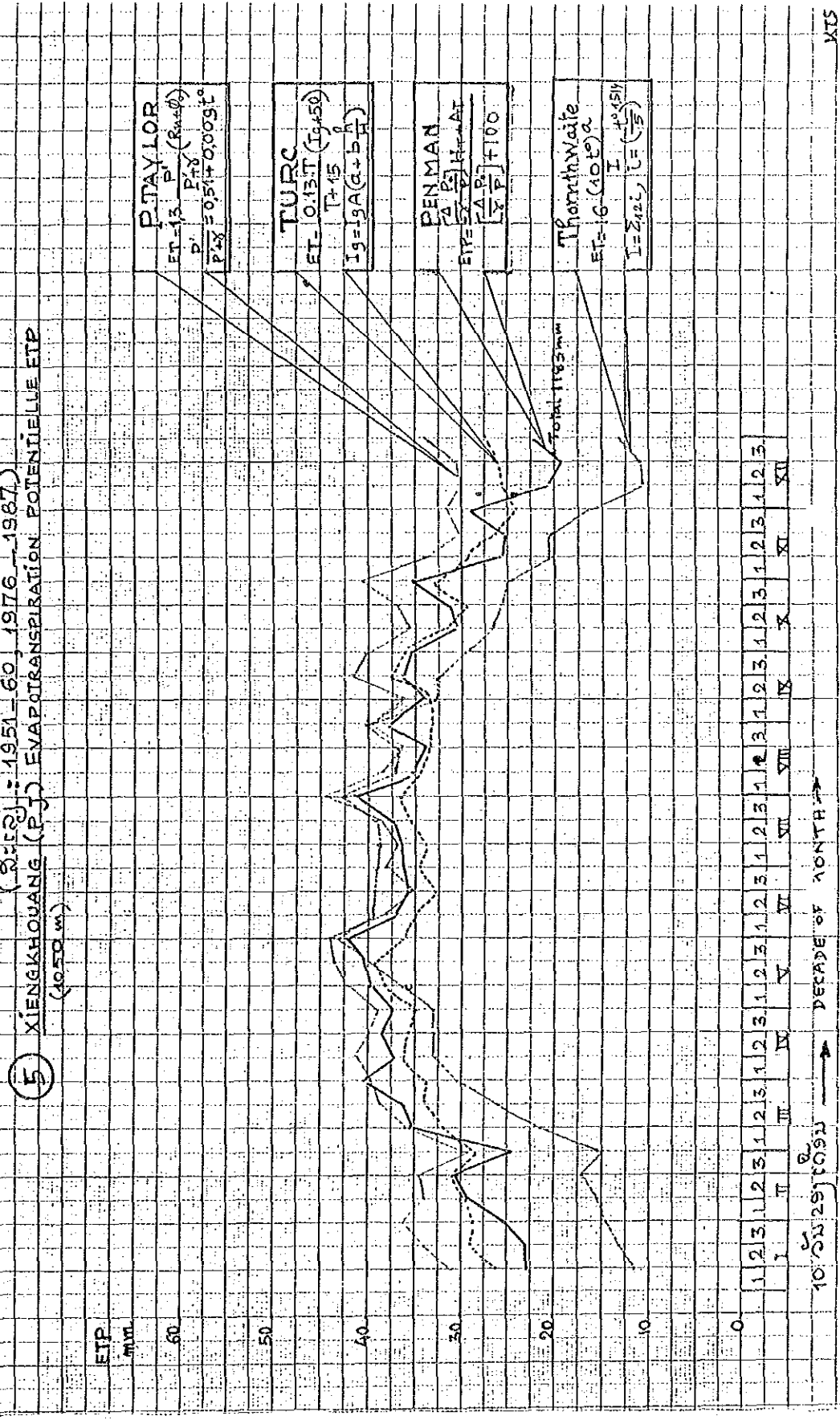
④ EVAPOTRANSPIRATION POTENTIELLE (PENMAN)

TOTAL DECADAIRES  
 PAKSE 1961-1986  
 PAKSONG 1983-1990

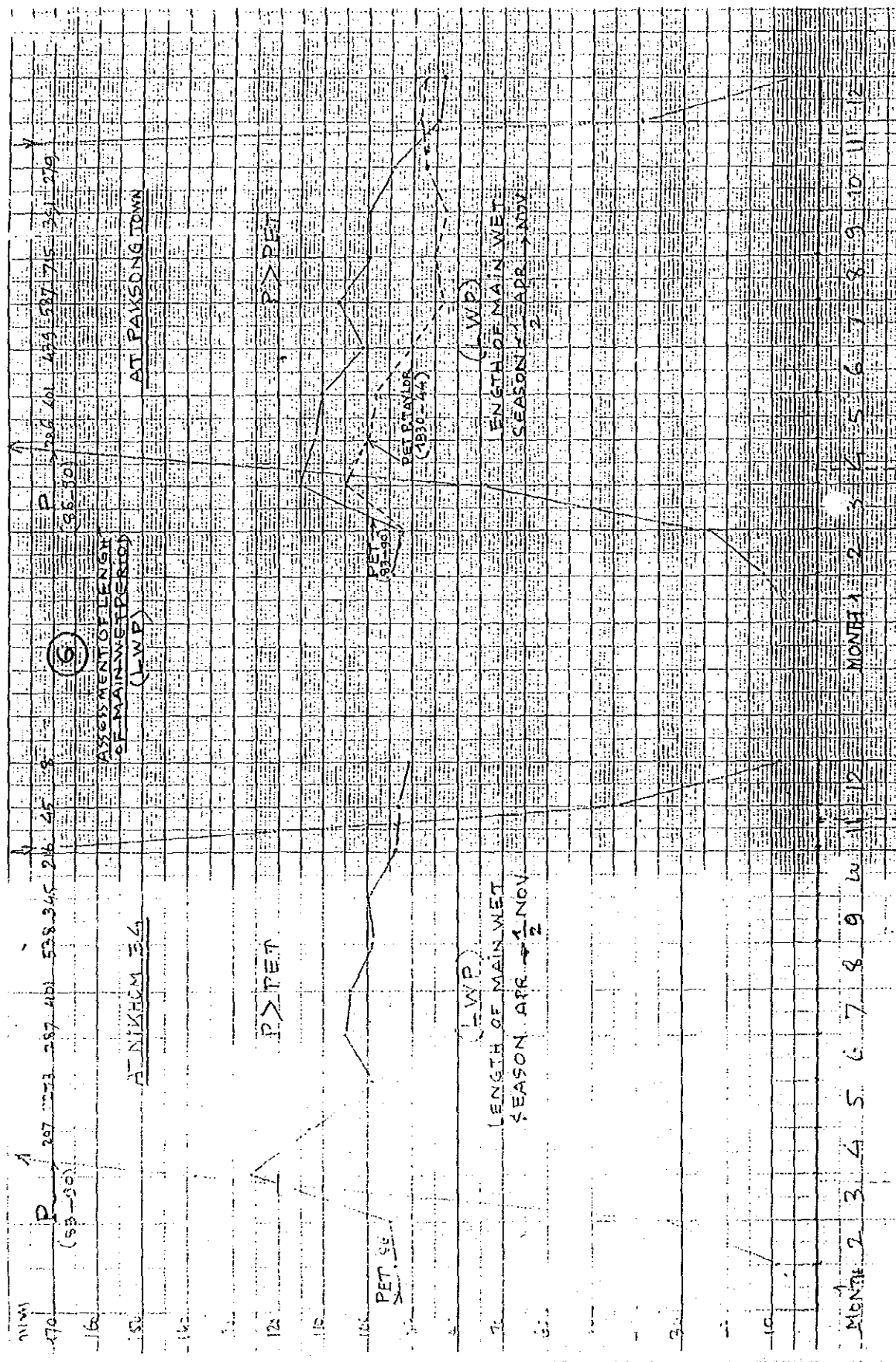


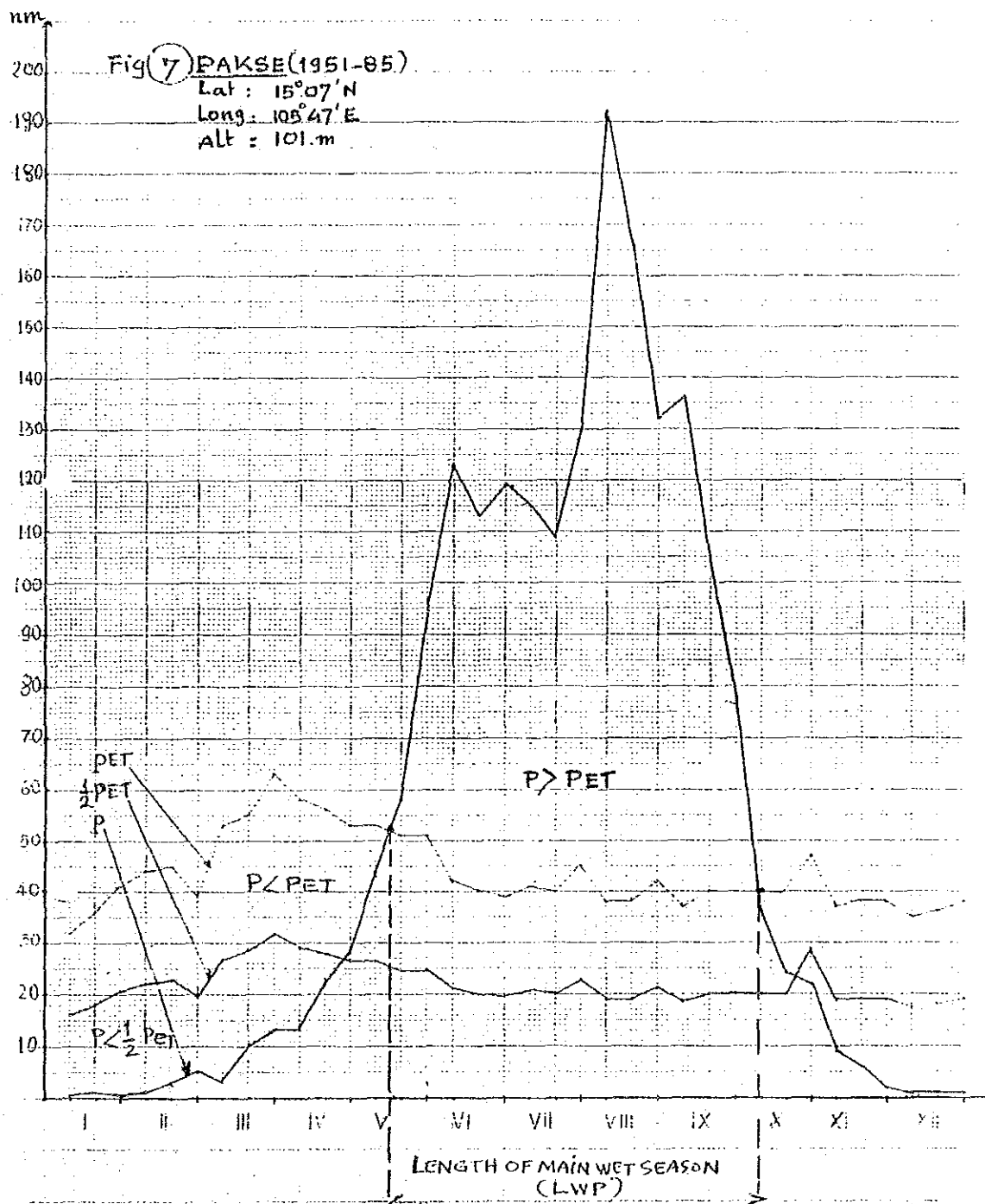
KT5  
90

ရန်ကင်း ဘုံစာတမ်း : စာမေးပွဲအတွက် အသုံးပြုရန် (၁၀၀၀)









⑧ 1986 Surface wind at NIKHOM 34 (Bolaven Plateau)

	calm	N		NE		E		SE		S		SW		W		NW	
	%	%	W	%	W	%	W	%	W	%	W	%	W	%	W	%	W
JAN	7.36	17	3.4	2.2	3.6	13	3.6	8	2.8	7	2.3	7	3.5	6	1.5	20	1.8
Feb	7.86	12	2.4	7	2.9	13	2.4	9	2.1	16	2.4	27	2.2	13	1.7	10	1.6
Mar	16.13	11	2.9	5	3.6	7	2.4	12	1.9	24	2.1	21	1.9	11	1.3	14	0.9
Apr	14.0	9	2.7	11	1.4	7	2.2	8	1.7	17	2.6	10	1.3	11	1.6	29	1.7
May	9.03	0	0.0	-	-	-	-	4	1.8	14	5.0	40	4.8	26	4.3	15	2.4
Jun	12.66	3	1.5	-	-	1	2.0	1	4.0	3	3.7	36	4.3	42	4.2	8	4.2
Jul	9.0	6	1.4	-	-	3	2.0	1	4.0	6	4.5	22	4.0	53	5.4	8	4.9
Aug	13.97	8	3.4	-	-	2	1.7	1	2.0	3	3.2	17	4.5	40	4.9	31	4.4
Sept	11.33	6	2.4	8	4.2	10	4.2	15	3.5	6	2.6	10	5.5	16	4.1	30	5.0
Oct	16.13	15	6.0	15	5.3	19	5.4	5	3.3	12	4.8	10	5.2	5	5.0	25	5.1
Nov	15.33	22	5.8	35	5.9	16	6.4	6	5.2	1	1.0	-	-	4	3.0	18	5.4
Dec	7.09	14	5.8	31	6.9	15	7.4	12	6.0	3	4.8	5	5.3	-	-	18	6.4

1986 AT PAKSONG KM 42

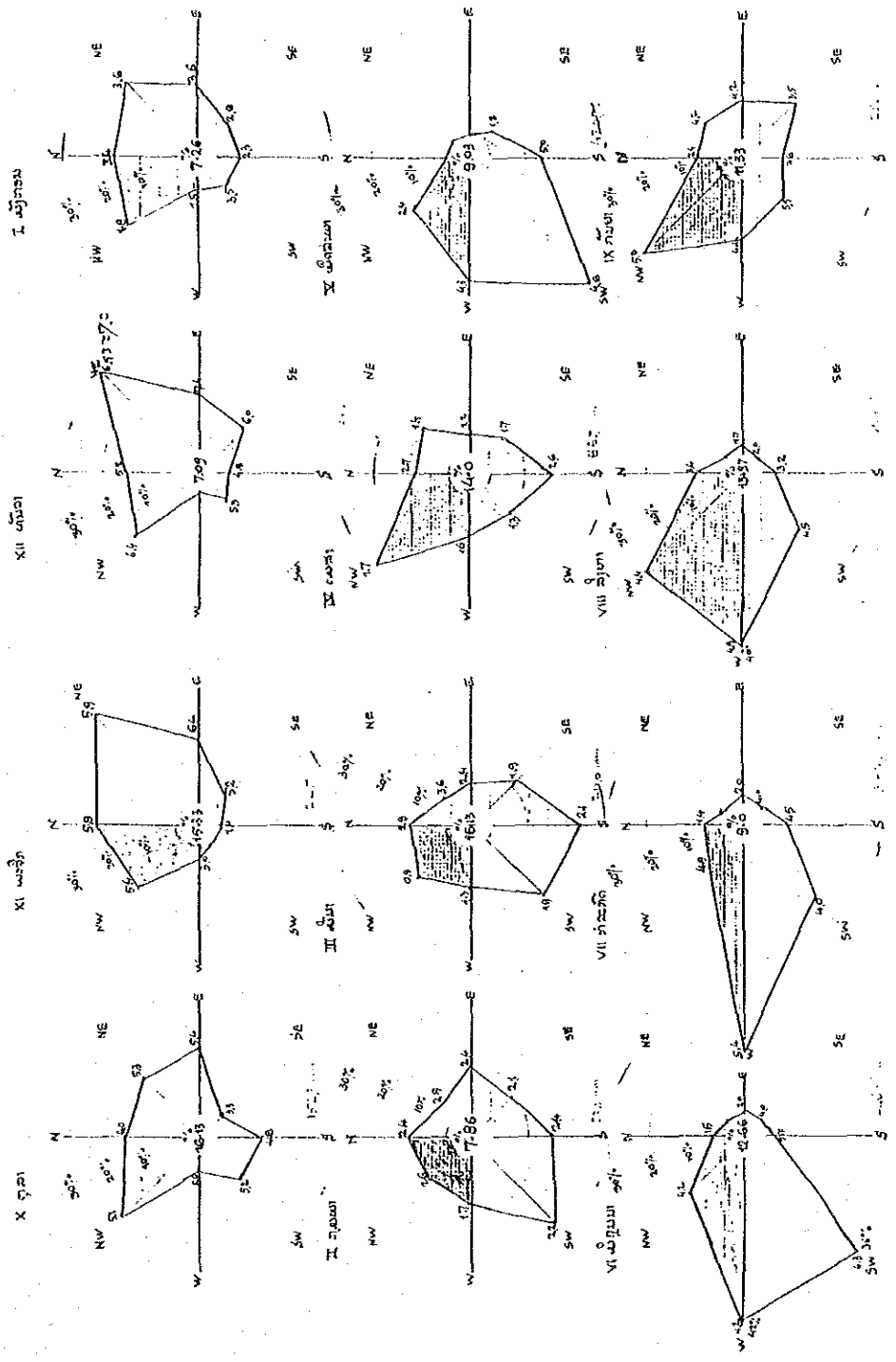
	calm	N		NE		E		SE		S		SW		W		NW	
	%	%	W	%	W	%	W	%	W	%	W	%	W	%	W	%	W
Jan	6.45	20.7	1.7	8.9	4.1	26.9	4.4	21.3	2.3	5.5	2.1	14.4	8.2	9.6	1.9	6.2	2.2
Feb	1.43	2.9	4.0	7.3	2.7	10.1	2.3	18.1	1.2	18.1	2.4	33.3	1.9	3.6	1.0	5.8	2.1
Mar	8.39	0.7	2.0	4.9	2.7	13.9	3.3	12.5	1.6	35.6	2.0	18.9	3.1	10.4	1.7	-	-
Apr	3.33	1.4	1.0	2.0	1.3	4.8	1.7	29.6	1.3	26.2	1.4	22.7	2.0	8.9	1.5	4.1	2.2
May	6.90	0.7	1.0	-	-	3.4	1.4	11.0	2.4	39.3	2.4	31.0	2.2	11.7	2.6	2.7	1.0
Jun	9.34	-	-	-	-	-	-	1.4	1.5	41.9	1.8	50.0	1.7	6.0	1.9	-	-
Jul	20.00	0.7	2.4	-	-	-	-	4.8	1.2	16.1	1.6	65.3	1.9	10.4	1.8	2.4	2.3
Aug	23.87	3.3	1.3	0.7	1.0	-	-	1.7	1.0	11.0	1.2	59.3	1.7	22.0	1.7	1.7	1.0
Sept	12.67	6.1	3.4	3.0	1.5	7.6	1.3	21.3	1.7	19.8	2.1	25.9	1.5	9.9	1.3	6.1	1.5
Oct	7.10	5.5	5.3	7.6	2.7	22.9	2.5	7.6	1.3	15.9	1.8	31.2	1.6	6.2	2.8	2.0	1.0
Nov	2.67	4.8	2.4	13.0	3.1	36.3	2.4	14.3	1.2	6.1	1.0	17.1	1.4	4.1	1.3	4.1	1.2
Dec	17.42	1.5	1.0	7.8	4.6	26.5	4.2	25.7	2.1	5.4	2.0	22.6	2.0	7.8	1.8	2.3	1.3

% = percentage of wind direction (frequency)  
W = wind velocity in m/s (monthly mean value of each direction)

9 పంపిణీ ప్లాన్ 86  
(VENT AUSOL)

పల్లెటూరు గ్రామం పట్టణం నుండి 1150 m దూరంలో  
N/K-34  
FERME D'ETAT  
P.L. DEBOLAVEN

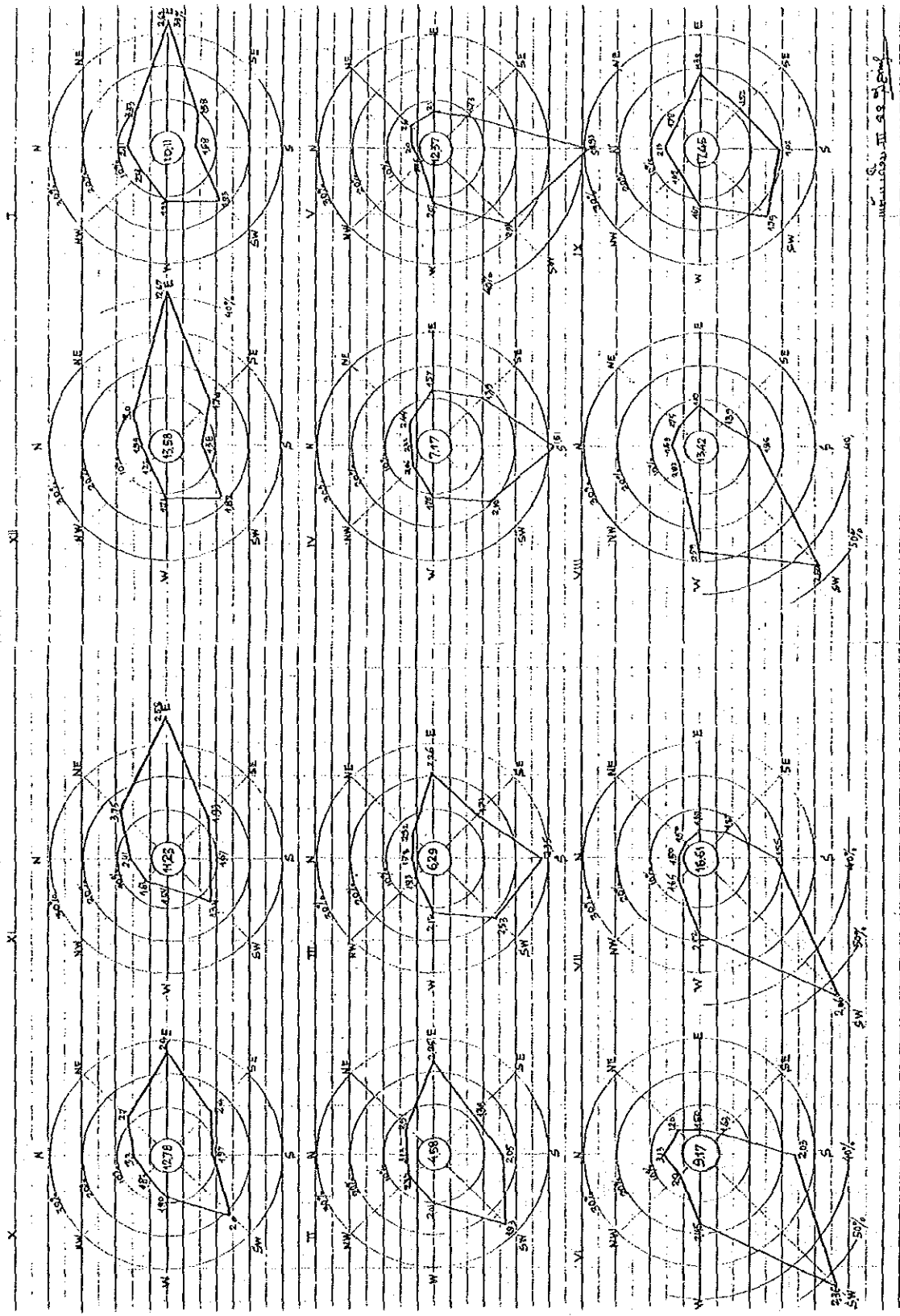
10% వరకు = 10% వరకు  
పంపిణీ ప్లాన్ 86



1160 JLNK - ಮೈಯ 1955-57

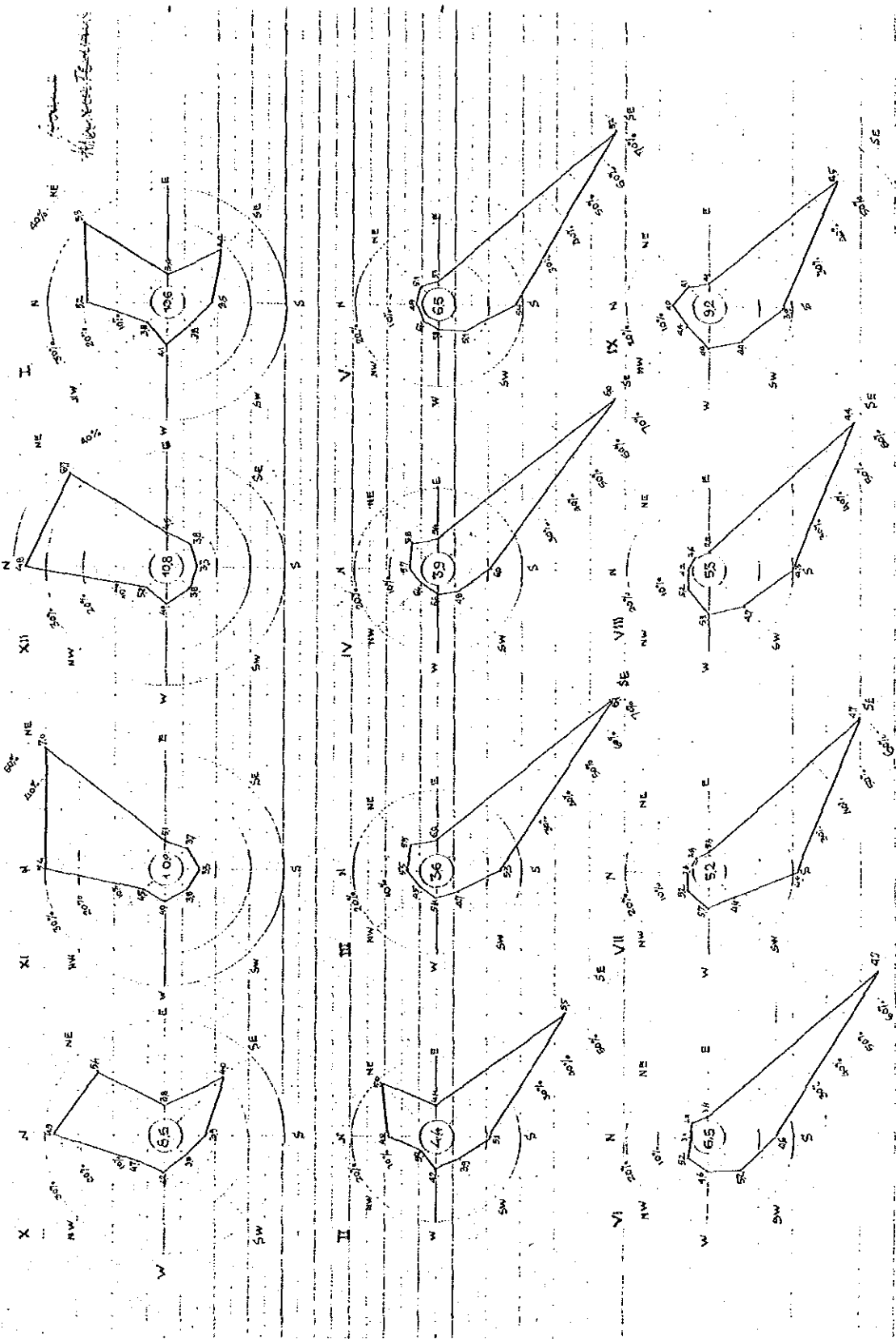
PARSONS VENT. AU SOL 1983-87

92



9 b) **ಪಟ್ಟಿ:** ಕೃಷ್ಣಾಭಯ ನದಿ ಜೊತೆ, 8 ದಿಕ್ಕಿನಲ್ಲಿ ವಾಹನಗಳ ಸಂಖ್ಯೆ (1961-1980)

**ಟಿಪ್ಪಣಿ:** SURFACE WIND ROSE, 8 DIRECTIONS,  $V_m$  m/s (20-YEAR AVERAGE)



⑩ Rin-off calculation from Water balance  
 1986 data at NIKHOM 34. (unit = mm),  $Q = m^3/s$

Parameter	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sept	Oct	Nov	Dec	ANN.
P mm	0	33	24	67	431	241	444	650	243	280	23	31	2467
PET (Penman)	94	95	126	113	99	105	104	99	100	94	93	91	1213
P - PET	-94	-62	-102	-46	+332	+136	+340	+551	+143	+186	-70	-60	
Reserve	0	0	0	0	100	100	100	100	100	100	0	0	
Deficiency	-94	-62	-102	-46									
Surplus					+232	+136	+340	+551	+143	+186			
Run-off					116	184	238	445	347	164.5	93	46.5	1634.5
Q at A784 (Table 3.9)	9.62	5.76	3.20	4.19	24.13	25.94	47.00	73.15	63.54	41.59	22.94	13.24	28.03
Mo. $l/A/km^2$	12.27	7.35	4.08	5.34	30.78	33.09	59.75	93.30	81.04	53.04	29.26	16.87	
Q calculated					33.95	55.00	69.67	130.4	105.48	3.28	12.13	13.75	
at %					65	65	65	65	70	70			
Q at %					22.06	35.75	45.3	84.8	73.5	43.5	28.12	13.75	

\* The basic principle.

To assess the loss of water from the soil, Thornthwaite takes the difference between potential Evapotranspiration PET and actual evapotranspiration when there is sufficient water available for vegetation (that is the case of the Bolaven plateau from May ---)

Assuming that the reserve  $RS = 100$  mm. If the precipitation  $P$  is in excess of the needs of PET or water need, the actual evapotranspiration is equal to the PET and any excess water remains in Reserve up to 100 mm, the remainder (water surplus) going to underground run-off, half of it during the month and the other half during subsequent months.

In the example of computation we assume that the surface run-off is approximately 65% of the total run-off from May - August and more higher from September - October for 1986 year. That is an assumption and result, can be compared with the other methods ---

1

(11) calculations of peak Flood Discharges at Projectsites

Two methods are used

- (1) - the SCS method
- (2) - the D. Sakolovsky method

(1) At XEKATAM Project Damsite (SCS method)

Catchment Area:  $290 \text{ km}^2$

H, the difference elevation between the farthest point and Damsite, being  $1700 \text{ m} - 450 \text{ m} = 1250 \text{ m}$

Length of the river approx  $45 \text{ km}$

$$L = 45000 \text{ m} \times 3.28 = 147600 \text{ feet}$$

$$S = 0.02777 \text{ (1:3600)}$$

For computation of maximum peak flow from a daily rainfall of  $200 \text{ mm}$  reasonably accepted as  $T_r = 10$  year period, and giving intensity in 4 hours  $i = 50 \text{ mm/h}$ , uniformly over the catchment (hourly rainfall at Paksong town for  $T_r = 10$  y is about  $65 \text{ mm/h}$  computed from the period 1966-1970).

Estimation of direct run off or  $P_e$  on the basis of

$$P_e = \frac{(P - 0.2S)^2}{P + 0.8S}$$

and the curve number

$$CN = 60$$

The time of concentration  $T_c$  is estimate from the Kirpich's formula or from the kinematic wave and SCS average velocity formula. for Kirpich's formula:  $T_c = 0.0078 L^{0.77} S^{-0.385}$

$$T_c = 0.0078 \times 9554.2477 \times 3.973534 = 296.1202 \text{ minutes} \approx (5 \text{ h})$$

consider 1h unit hydrograph,

$$t_r = 1 \text{ h}; t_p \approx 0.6 T_c = 0.6 \times 5 = (3 \text{ h})$$

$$T_p = \frac{t_r}{2} + t_p = 0.5 + 3 = 3.5 \text{ h}$$

$$\text{and } t_b = 2.67 \times 3.5 \approx 9.4 \text{ h}$$

for  $A = 290 \text{ km}^2$

$$q_p = \frac{2.08 \cdot A}{T_p} = \frac{2.08 \cdot 290}{3.5} \approx 172 \text{ m}^3/\text{s} \cdot \text{cm}$$

This means that each  $1 \text{ cm}$  of excess rainfall,  $P_e$  (direct runoff) will produce  $172 \text{ m}^3/\text{s}$  from this catchment.



The peak discharge  $Q_p$ , from each hr,  $P_e$  increment is

$$Q_{p,t} = q_p \times P_{e,t}$$

For  $CN = 60$

$P = 200 \text{ mm/day}$  and  $50 \text{ mm/h}$  in 4 hours.

time h	cumul. Rain $P_{e, \text{mm}}$	cumulative Run-off $P_e$	$P_e$ (i.e) cm	Peak disch $Q_p \text{ m}^3/\text{s}$
0	0	0	0	0
1	50	3	0.3	58.6
2	100	20	1.7	292.4
3	150	50	3	516
4	200	86	3.6	619.2

## 2/ XENAMNOY at BAN LAISASIN.

$$CA = 537 \text{ km}^2 \quad \text{Altitude} = 720 \text{ m}$$

$$H = 1000 - 720 = 280 \text{ m}$$

$$L = 40 \text{ km} \quad (40000 \times 3.28 = 131200 \text{ feet})$$

$$S = 0.007$$

$$T_c = 0.0078 \cdot 131200^{0.77} \cdot 0.007^{-0.385} = 0.0078 \cdot 8725.8763 \cdot 6.755 = 459.7577 \text{ minutes} = 7.66 \text{ h}$$

$$\text{for } t_r = 1 \text{ h}, \quad t_p = 0.6 T_c = 0.6 \times 7.66 = 4.596 \approx 4.6 \text{ h}$$

$$T_p = \frac{t_r}{2} + t_p = 0.5 + 4.6 = 5.1 \text{ h}, \quad t_b = 2.67 \cdot 5.1 = 13.6 \text{ h}$$

$$A = 537 \text{ km}^2$$

$$q_p = \frac{2.08 \cdot 537}{5.1} = 219 \text{ m}^3/\text{s} \cdot \text{cm}$$

for the same  $CN = 60$

$P_e$  (i.e)

$Q_p$

0

0

0.3

$$0.3 \times 219 = 65.7 \text{ m}^3/\text{s}$$

1.7

$$1.7 \times 219 = 372.3 \text{ m}^3/\text{s}$$

3

$$3 \times 219 = 657.0 \text{ m}^3/\text{s}$$

3.6

$$3.6 \times 219 = 788.4 \text{ m}^3/\text{s}$$

Vnam, 22/7/91

Khamthong S

(2) calculation of  $Q_{max}$  from Sakolovsky method.

Ex: A small tributary of Sedone near the toe of the Bolaven plateau with a mean height  $H = 6.25$  m, the catchment area is  $F = 203$  km<sup>2</sup> no lake or pond in the basin.

The calculation of a peak flood discharge  $Q_{max}(1\%)$  for this example is:

$$q_{200} = 5.4 \text{ m}^3/\text{s}/\text{km}^2; \quad m = 0.63 \text{ and } \lambda = 1, \quad P = 1\%$$

$$Q_{max, 1\%} = 5.4 \left( \frac{200}{203} \right)^{0.63} \times 1 \times 203 = 1085 \text{ m}^3/\text{s}$$

The formula is:

$$Q_p = q_{200} \left( \frac{200}{F} \right)^m \cdot \lambda \cdot \delta_1 \cdot F$$

where

$F$  = the catchment area in sq. km

$m = 0.63$  for mountainous regions

$$q_{200} = 5.4 \text{ m}^3/\text{s}/\text{km}^2$$

$\lambda$ : coefficient depend upon the size of river basin

$$\delta = \frac{HT}{(60 \cdot T)^{0.33}} \quad \text{or } \delta = A + B \log N \quad \text{for precipitation mete}$$

$T = 100$  year period ( $P = 1\%$ ),  $A$  and  $B \rightarrow$  figures

For the same area with  $F = 51$  km<sup>2</sup> (upper Sedone)

with  $L = 22.5$  km, mean height of the basin = 4.10 m

$$Q_{max, 1\%} = \frac{0.28 \cdot HT \cdot \delta \cdot F}{t_{\pi}} \cdot f$$

$$\text{where } t_{\pi} = \frac{L}{3.6 V_{\pi}} = \frac{22.5}{3.6 \times 2.5} = 2.5 \text{ h}$$

$$V_{\pi} = 0.7 \times 3.57 = 2.50 \text{ m/s}, \quad V_{max} = 3.57 \text{ m/s} \quad (\text{seems to be higher})$$

$$T = t_{\pi} \cdot \mu = 2.5 \cdot 0.78 = (2 \text{ h}) \quad \mu = 0.78 \text{ (coefficient of reduction)}$$

$$HT = \delta (T \cdot 60)^{0.33} = 24.8 (2.0 \cdot 60)^{0.33} = 120 \text{ mm (precipitation)}$$

$$\lambda = \frac{Q_{p, max}}{Q_p} \quad (\text{table})$$

$$\lambda = 0.35 \text{ for } 81 < HT < 150 \text{ mm and } F \text{ between } 10 \sim 100 \text{ km}^2$$

$f = 0.923$  coefficient of the hydrograph shape.

$$\text{Hence } Q_{max, 1\%} = \frac{0.28 \cdot 120 \cdot 0.35 \cdot 51 \text{ km}^2 \cdot 0.923}{2.5} = 221 \text{ m}^3/\text{s}$$

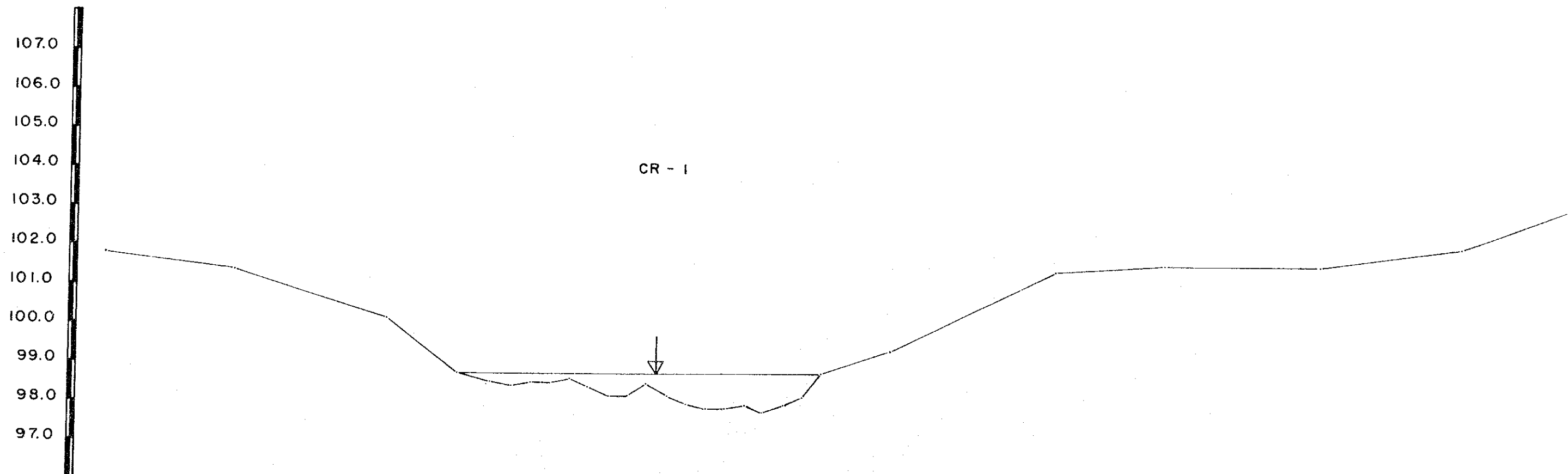
(all of these computations are only for information)



**CROSS SECTION OF XE KATAM RIVER  
AT BAN NONGHIN (1/3)**

(10 m upstream from staff gauge)

SCALE 1:  $\frac{100}{200}$

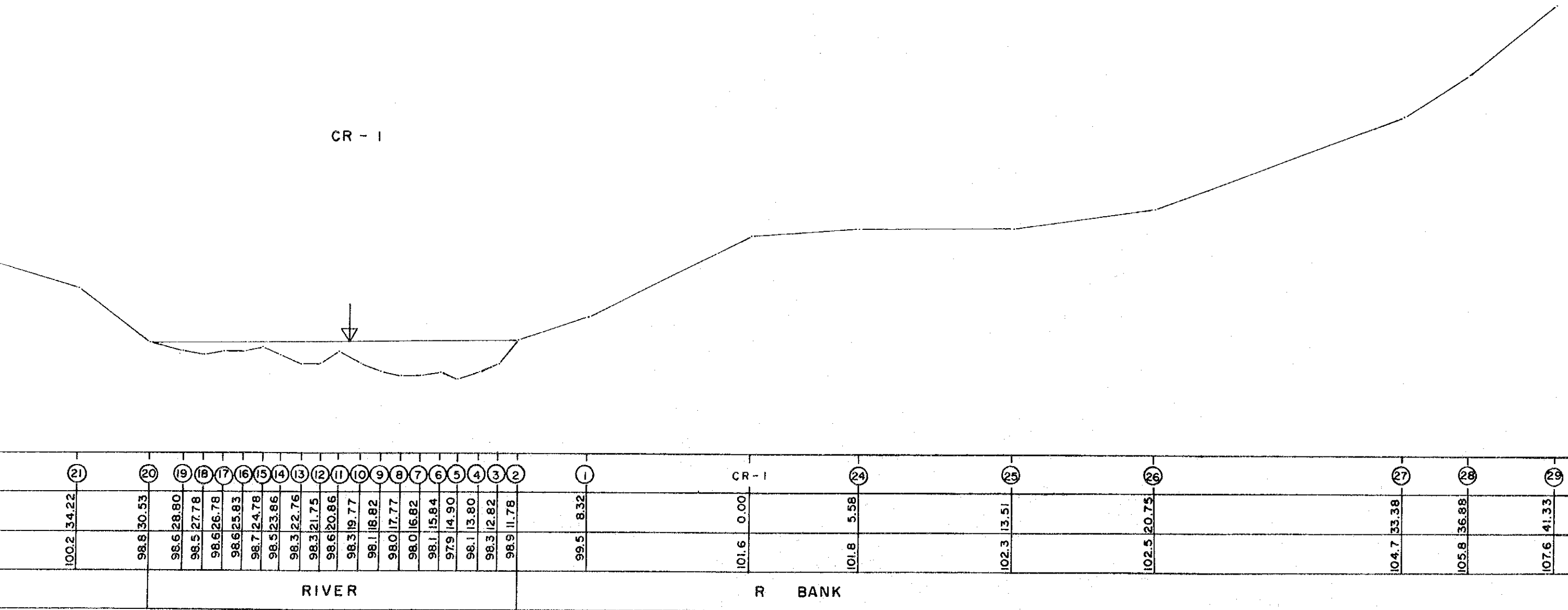


POINT	23	22	21	20	19	18	17	16	15	14	13	12	11	10	9	8	7	6	5	4	3	2	1	CR-1	24	25	26
DISTANCE	49.35	41.86	34.22	30.53	28.80	27.78	26.78	25.83	24.78	23.86	22.76	21.75	20.86	19.77	18.82	17.77	16.82	15.84	14.90	13.80	12.82	11.78	8.32	0.00	5.58	13.51	20.75
ELEVATION	101.8	101.4	100.2	98.8	98.6	98.5	98.6	98.6	98.7	98.5	98.3	98.3	98.6	98.3	98.1	98.0	98.0	98.1	97.9	98.1	98.3	98.9	99.5	101.6	101.8	102.3	102.5
REMARK	L BANK				RIVER																		R BANK				

**CROSS SECTION OF XE KATAM RIVER  
AT BAN NONGHIN (1/3)**

(10 m upstream from staff gauge)

SCALE 1:  $\frac{100}{200}$

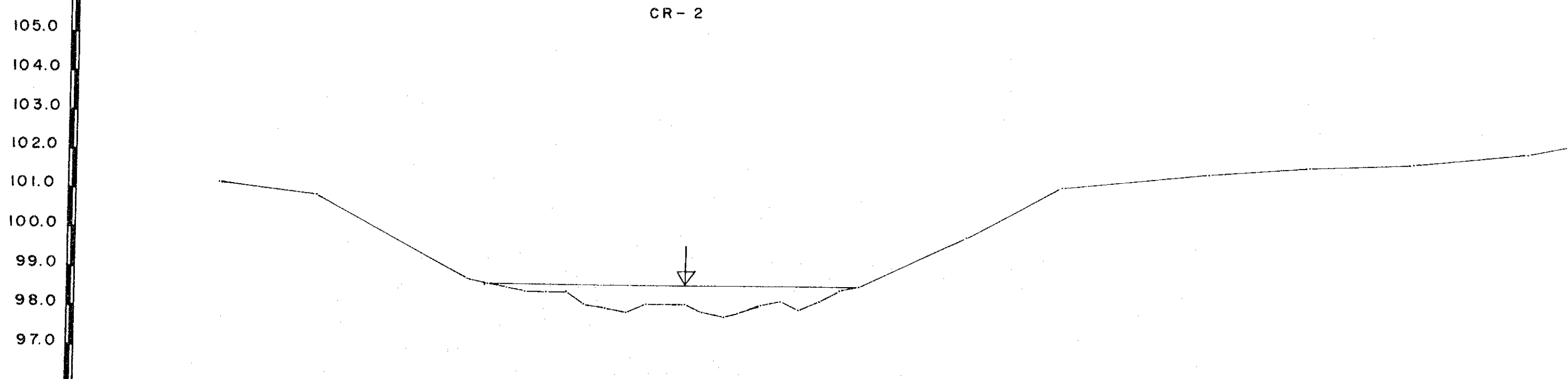
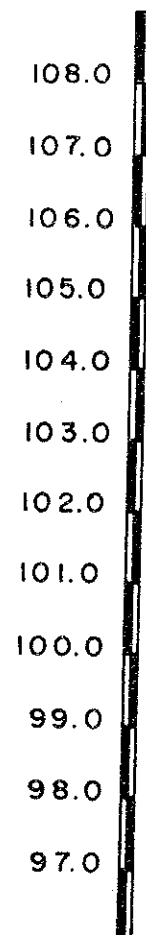


by HEC



**CROSS SECTION OF XE KATAM RIVER  
AT BAN NONGHIN (2/3)**

(at staff gauge)



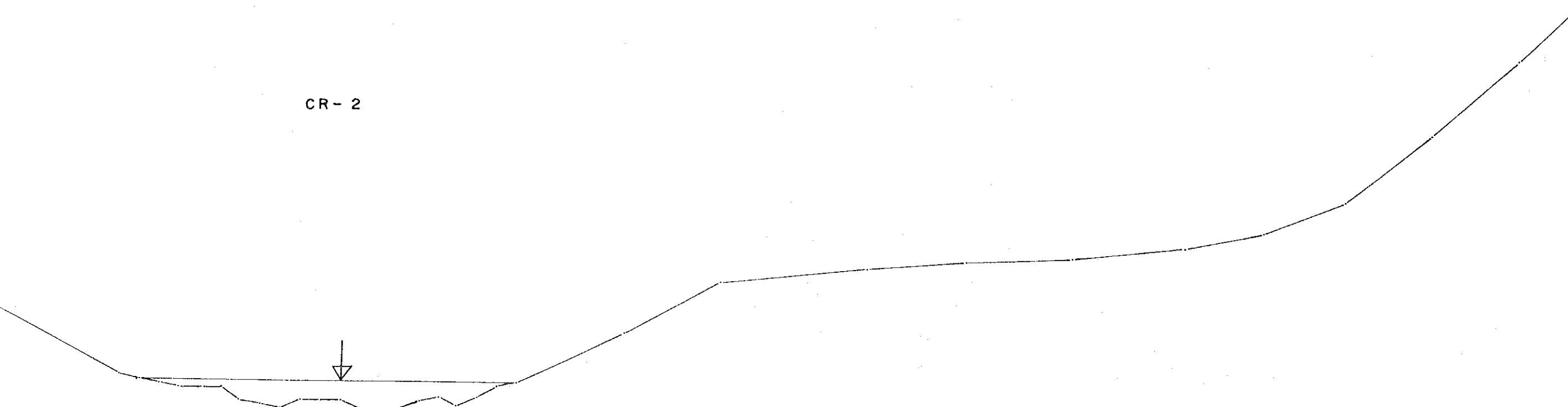
POINT	33	32	31	30	29	28	27	26	25	24	23	22	21	20	19	18	17	16	15	14	13	12	11	10	CR - 2	1	2	3	4
DISTANCE	42.65	37.67	29.97	29.00	28.03	27.08	26.00	25.05	24.06	23.05	21.99	21.00	20.06	19.00	18.12	17.03	16.32	15.09	14.13	13.15	12.16	11.21	10.22	4.80	0.00	7.43	12.36	17.68	23.44
ELEVATION	101.2	100.9	98.8	98.7	98.6	98.5	98.5	98.5	98.2	98.1	98.0	98.2	98.2	98.2	98.0	97.9	98.0	98.2	98.3	98.1	98.3	98.6	98.7	100.0	101.3	101.7	101.9	102.0	102.3
REMARK	L BANK		RIVER																				R BANK						

CROSS SECTION OF XE KATAM RIVER

AT BAN NONGHIN (2/3)

(at staff gauge)

CR - 2



31	30	29	28	27	26	25	24	23	22	21	20	19	18	17	16	15	14	13	12	11	10	CR - 2	1	2	3	4	5	6	7	8	9
98.8	98.7	98.6	98.5	98.5	98.5	98.2	98.1	98.0	98.2	98.2	98.0	97.9	98.0	98.2	98.3	98.1	98.3	98.6	98.7	100.0	101.3	101.7	101.9	102.0	102.3	102.7	103.5	105.2	107.1	108.5	
29.97	29.00	28.03	27.08	26.00	25.05	24.06	23.05	21.99	21.00	20.06	19.00	18.12	17.03	16.32	15.09	14.13	13.15	12.16	11.21	10.22	4.80	0.00	7.43	12.36	17.68	23.44	27.33	31.45	35.60	39.93	42.78
RIVER											R BANK																				

by HEC





### CROSS SECTION OF XE KATAM RIVER

AT BAN NONGHIN (3/3)

(10 m downstream from staff gauge)

109.0  
108.0  
107.0  
106.0  
105.0  
104.0  
103.0  
102.0  
101.0  
100.0  
99.0  
98.0

CR - 3

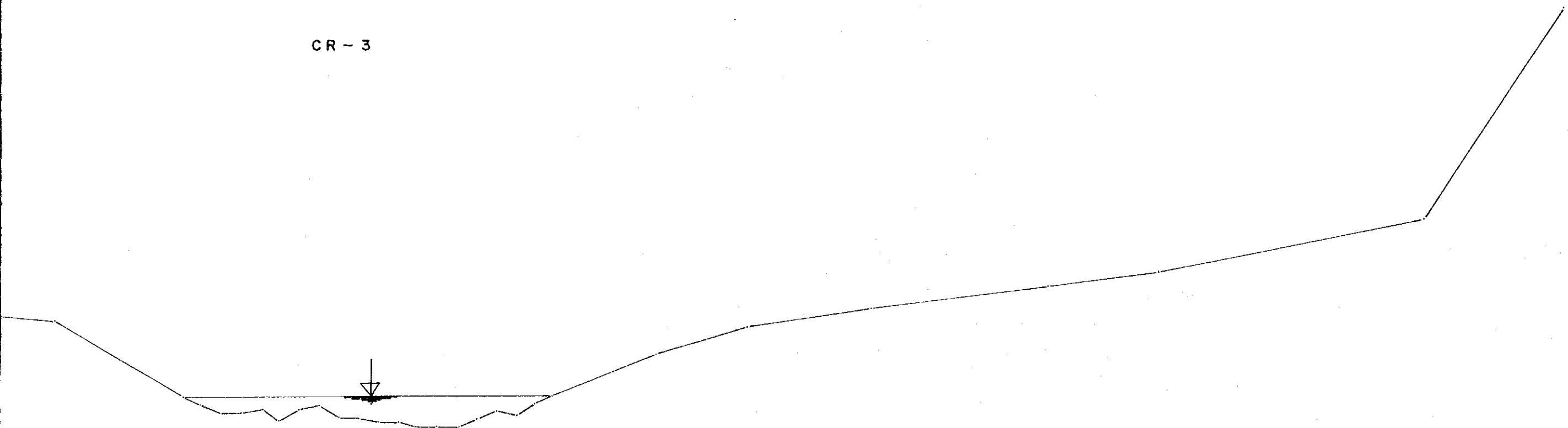
POINT		23	22	21	20	19	18	17	16	15	14	13	12	11	10	9	8	7	6	5	4	3	2	1	CR-3	25	26	27
DISTANCE		42.22	34.75	28.51	27.56	26.59	25.53	24.44	23.62	22.63	21.64	20.57	19.55	18.60	17.62	16.67	15.62	14.45	13.57	12.65	11.63	10.68	9.77	4.28	0.00	6.06	15.03	20.64
ELEVATION		100.9	100.6	98.7	98.5	98.3	98.3	98.4	98.1	98.4	98.5	98.2	98.2	98.1	98.1	98.0	98.0	98.2	98.4	98.3	98.3	98.6	98.8	99.9	100.6	101.1	101.7	102.1
REMARK		L BANK			RIVER																		R BANK					

CROSS SECTION OF XE KATAM RIVER

AT BAN NONGHIN (3/3)

(10 m downstream from staff gauge)

CR - 3



22	21	20	19	18	17	16	15	14	13	12	11	10	9	8	7	6	5	4	3	2	1	CR - 3	25	26	27	28	29	
100.6	98.7	98.5	98.3	98.3	98.4	98.1	98.4	98.5	98.2	98.1	98.1	98.0	98.0	98.0	98.2	98.4	98.3	98.6	98.8		99.9	100.6	101.1	101.7	102.1	103.5	108.9	
34.75	28.51	27.56	26.59	25.53	24.44	23.62	22.63	21.64	20.57	19.55	18.60	17.62	16.67	15.62	14.45	13.57	12.65	11.63	10.68	9.77		4.28	0.00	6.06	15.03	20.64	33.00	41.01
	RIVER																					R BANK						

by HEC



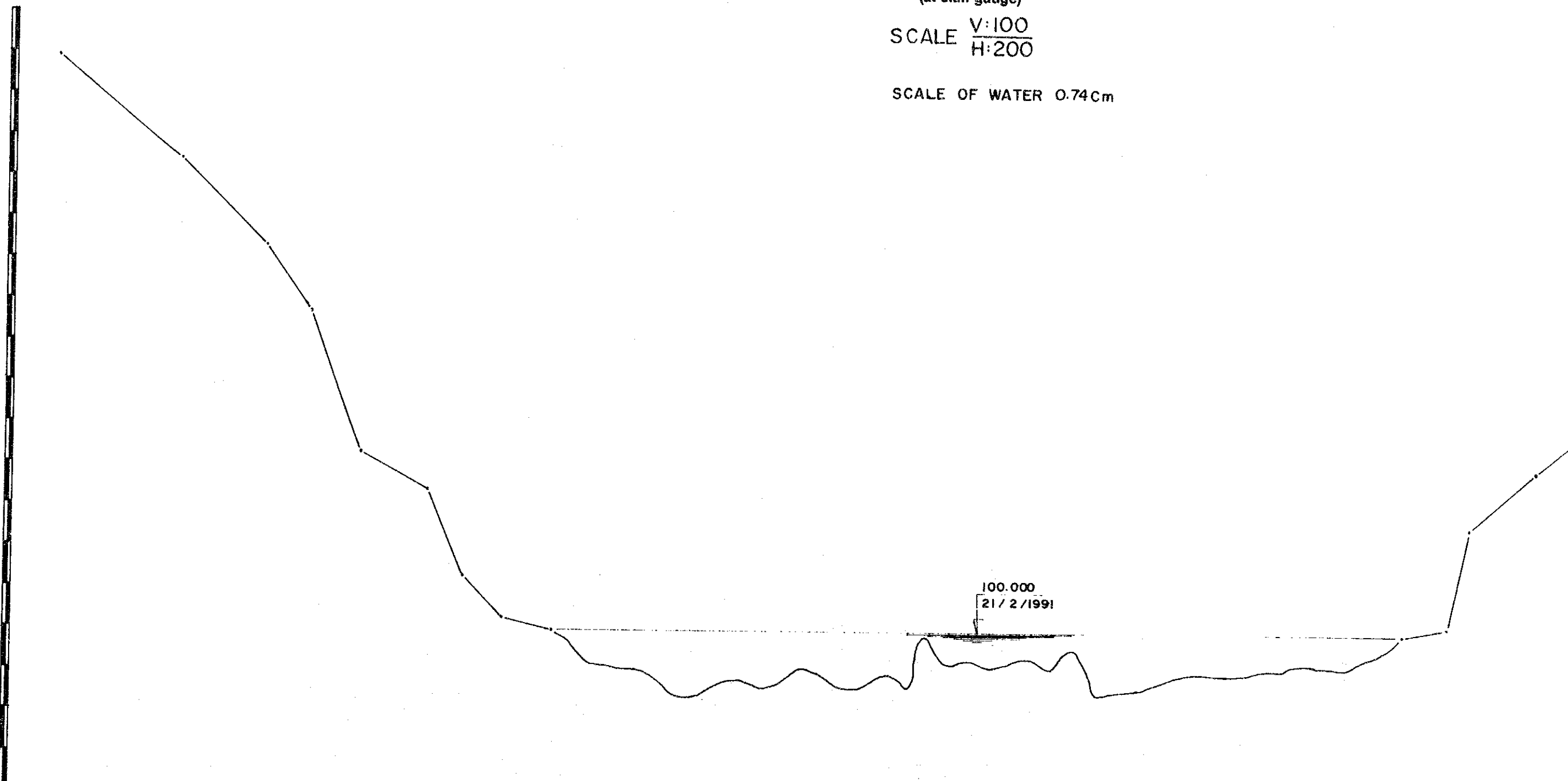
**CROSS SECTION OF XE NAMNOY RIVER  
AT BAN LATSASIN (2/2)**

(at staff gauge)

SCALE  $\frac{V:100}{H:200}$

SCALE OF WATER 0.74cm

115  
114  
113  
112  
111  
110  
109  
108  
107  
106  
105  
104  
103  
102  
101  
100  
99  
98  
97

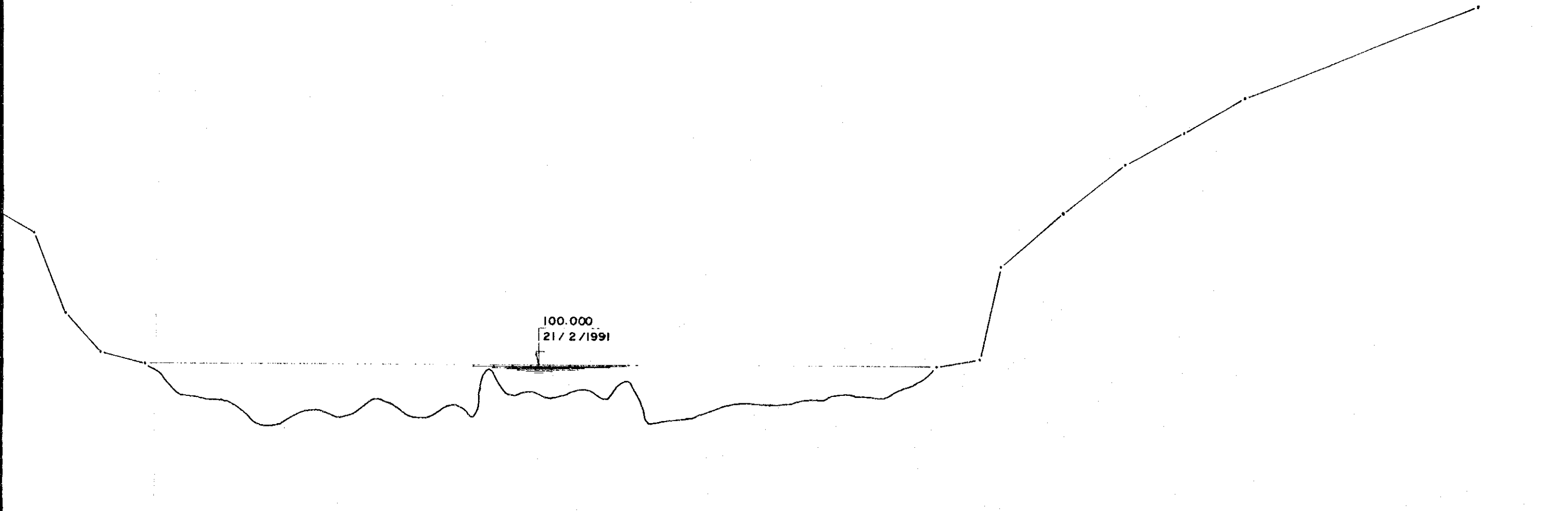


POINT	9	8	7	6	5	4	3	2	14	13	12	11	10	9	8	7	6	5	4	3	2	1	1'	2'	3'	4'																															
DISTANCE		6.00	4.15	2.5	2.25	3.10	1.70	1.90	2.35																																																
ELEVATION	103.968	111.418	109.302	107.731	104.384	103.408	101.395	100.374	100.00	99.275	99.195	99.050	98.810	98.495	98.460	98.760	98.840	98.620	98.860	99.190	98.935	98.610	98.720	98.920	98.670	99.920	99.250	99.380	99.190	99.200	99.350	99.130	99.620	98.570	98.660	98.670	98.840	98.930	99.040	99.050	99.085	99.115	99.140	99.200	99.260	99.200	99.340	99.450	99.520	100.00	2.10	100.253	1.10	102.625	3.35	104.094	3.33
REMARK	SOIL LEFT								RIVER (ROCK)																																																

**CROSS SECTION OF XE NAMNOY RIVER  
AT BAN LATSASIN (2/2)**

(at staff gauge)  
SCALE  $\frac{V:100}{H:200}$

SCALE OF WATER 0.74cm



4	3	2	1	41	40	39	38	37	36	35	34	33	32	31	30	29	28	27	26	25	24	23	22	21	20	19	18	17	16	15	14	13	12	11	10	9	8	7	6	5	4	3	2	1	1'	2'	3'	4'	5'	6'	7'	8'
103.408	101.396	100.374	100.00	99.275	99.195	99.050	98.810	98.495	98.460	98.760	98.840	98.620	98.860	99.190	98.935	98.610	98.720	98.920	98.670	99.200	99.250	99.380	99.190	99.200	99.350	99.130	99.620	98.570	98.660	98.670	98.840	98.930	99.040	99.050	99.085	99.115	99.140	99.200	99.260	99.200	99.340	99.450	99.520	100.00	100.210	100.253	102.625	104.094	105.324	106.106	107.038	109.598
RIVER (ROCK)																							RIGHT SOIL																													

by HEC



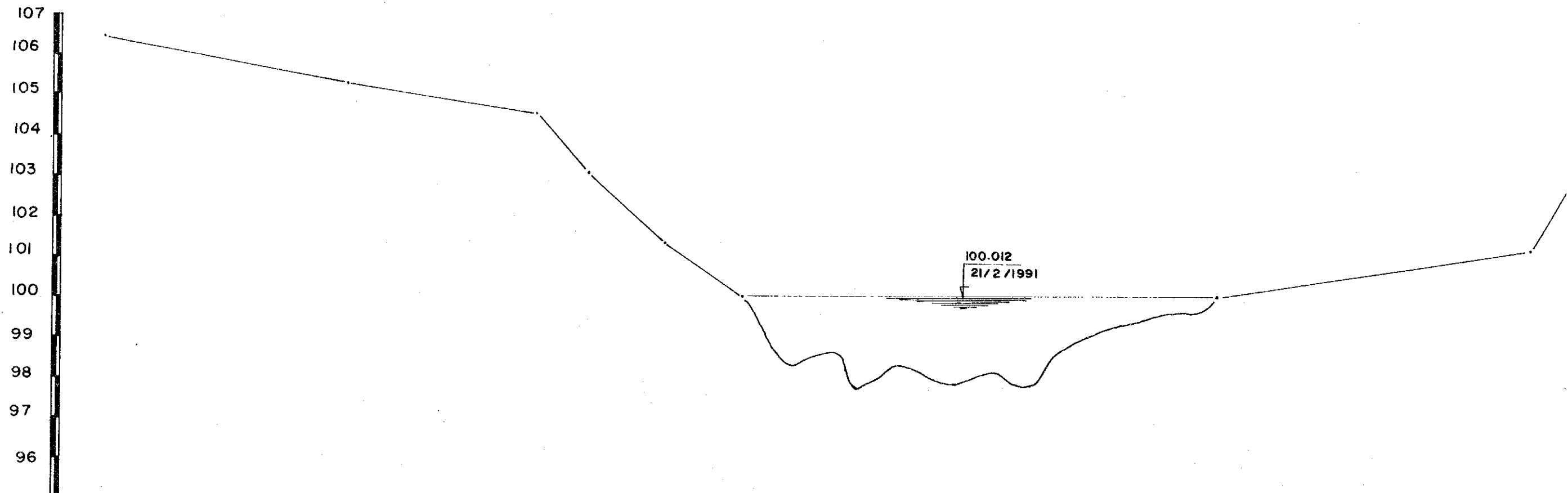
# CROSS SECTION OF XE NAMNOY RIVER

## AT BAN LATSASIN (1/2)

(15 m upstream from staff gauge)

SCALE  $\frac{V: 100}{H: 200}$

SCALE OF WATER 0.74 Cm



POINT	6	5	4	3	2	1	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	1	2
DISTANCE		12.6	8.9	2.5	4.4	3.76	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	2.4
ELEVATION	106.457	106.284	104.562	103.054	101.395	100.012	99.767	98.762	98.287	98.512	98.637	97.642	97.997	98.332	98.152	97.827	97.717	98.012	98.172	97.772	97.732	98.682	98.892	99.072	99.342	99.352	99.562	99.402	100.012	101.266	101.395
RE MARK	SOIL LEFT			ROCK			RIVER (ROCK)																RIGHT ROCK								

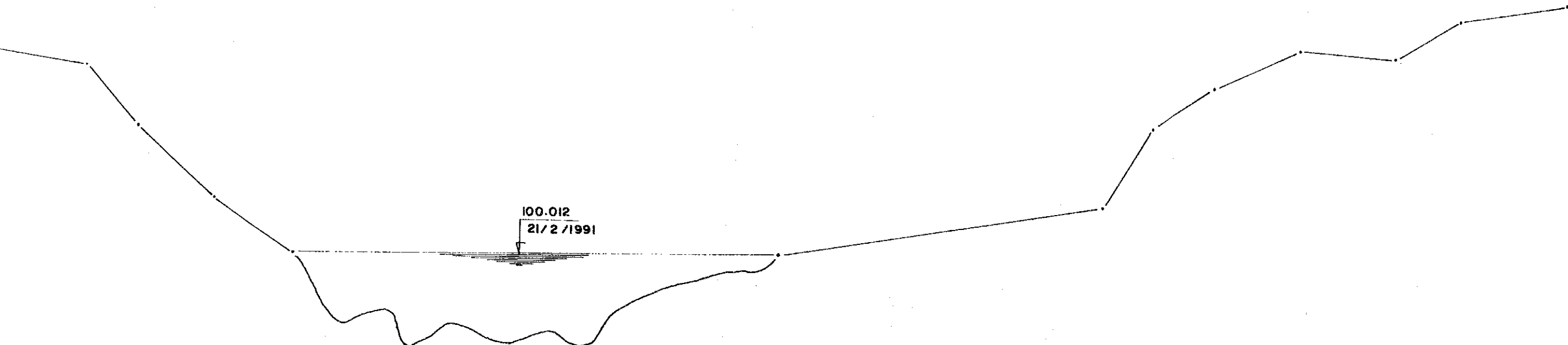


**CROSS SECTION OF XE NAMNOY RIVER  
AT BAN LATSASIN (1/2)**

(15 m upstream from staff gauge)

SCALE  $\frac{V: 100}{H: 200}$

SCALE OF WATER 0.74 Cm



4	3	2	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	2'	3'	4'	5'	6'	7'	8'								
104.562	103.054	101.395	100.012	99.767	98.762	98.287	98.512	98.637	97.642	97.997	98.332	98.152	97.827	97.717	98.012	98.172	97.772	97.732	98.682	98.892	98.072	99.342	99.352	99.562	99.402	100.012	15.3	101.266	2.4	103.194	3.00	104.175	4.4	105.084	4.75	104.864	4.05	105.890	5.15	106.240
ROCK			RIVER (ROCK)																			RIGHT ROCK		SOIL																

by HEC

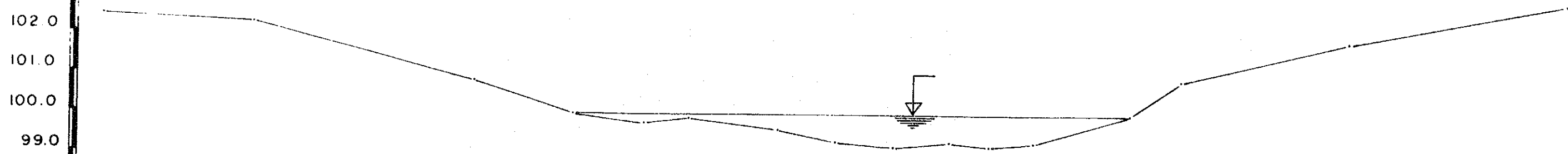
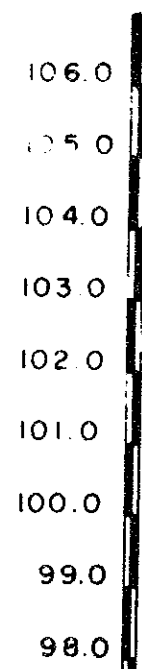


# CROSS SECTION OF HOUAY MAKCHAN RIVER

AT NIKHON34 (1/3)

(10 m upstream from staff gauge at bridge)

SCALE 1  $\frac{100}{100}$



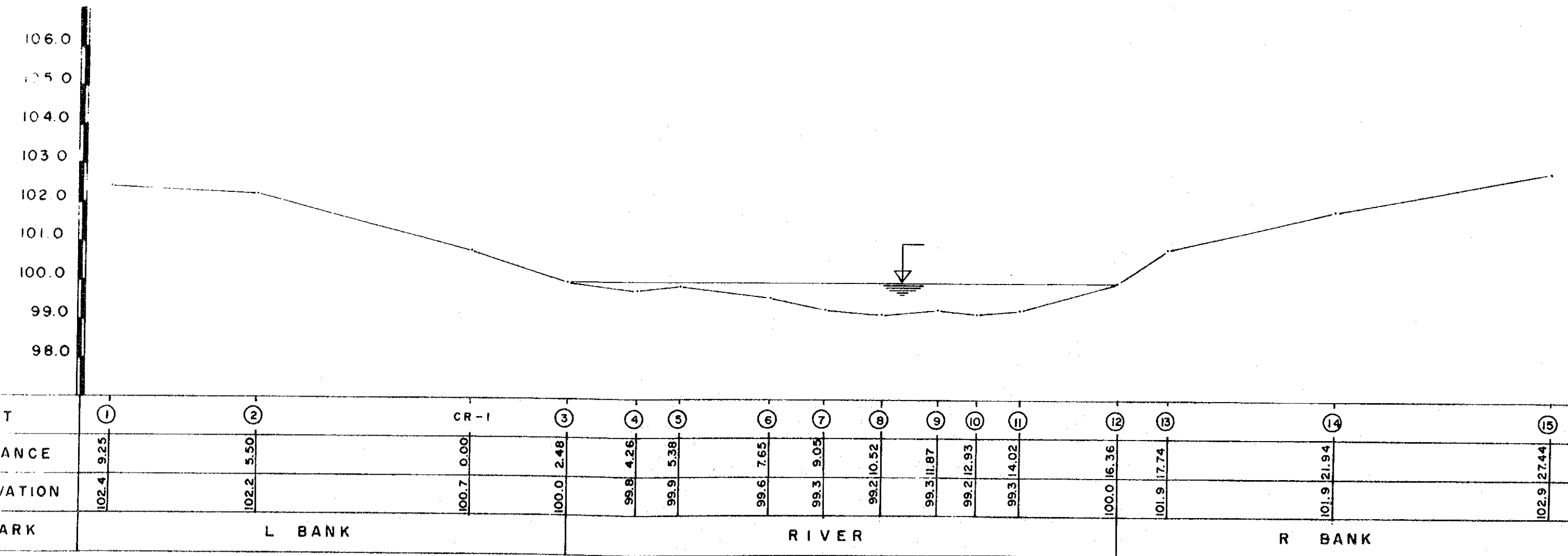
POINT	①	②	CR-1	③	④	⑤	⑥	⑦	⑧	⑨	⑩	⑪	⑫	⑬	⑭	⑮
DISTANCE	9.25	5.50	0.00	2.48	4.26	5.38	7.65	9.05	10.52	11.87	12.93	14.02	16.36	17.74	21.94	27.44
ELEVATION	102.4	102.2	100.7	100.0	99.8	99.9	99.6	99.3	99.2	99.3	99.2	99.3	100.0	101.9	101.9	102.9
REMARK	L BANK			RIVER									R BANK			

# CROSS SECTION OF HOUAY MAKCHAN RIVER

## AT NIKHON34 (1/3)

(10 m upstream from staff gauge at bridge)

SCALE 1  $\frac{100}{100}$



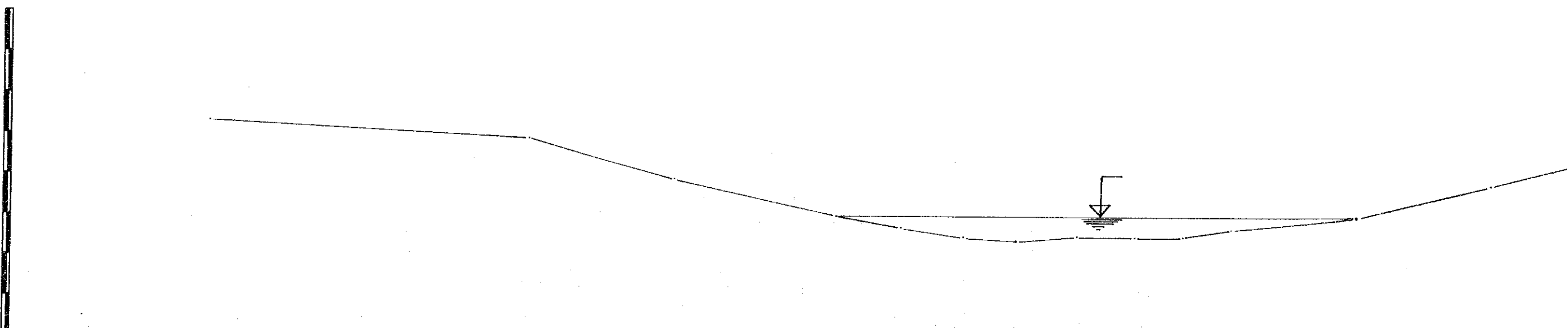
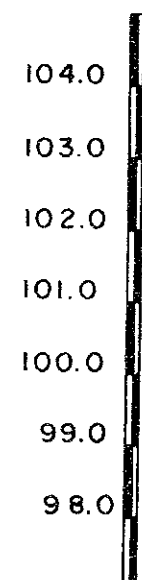
by HEC



# CROSS SECTION OF HOUAY MAKCHAN RIVER

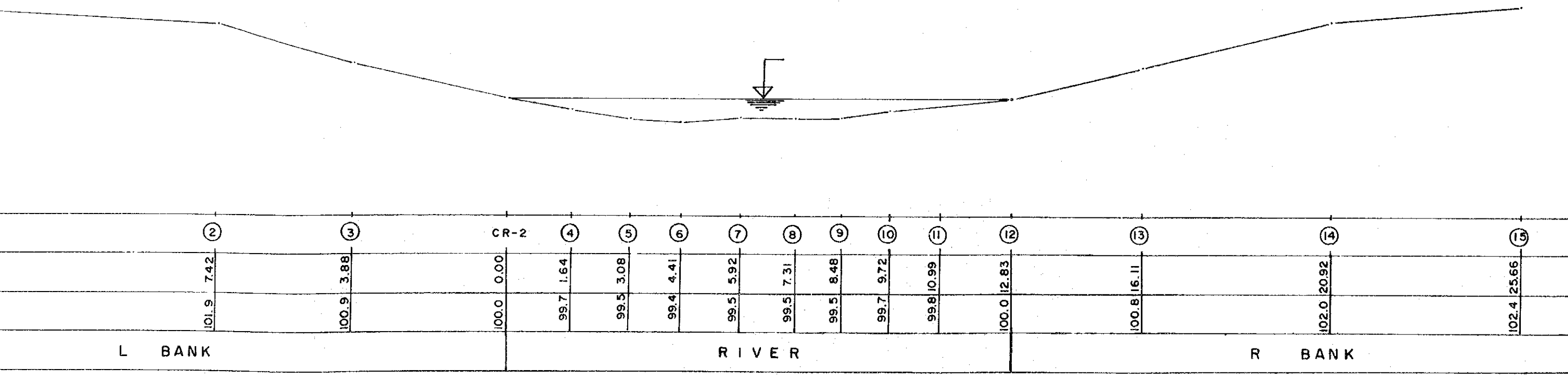
AT NIKHON34 (2/3)

(at staff gauge)



POINT	①	②	③	CR-2	④	⑤	⑥	⑦	⑧	⑨	⑩	⑪	⑫	⑬
DISTANCE														
ELEVATION	102.3	101.9	100.9	100.0	99.7	99.5	99.4	99.5	99.5	99.5	99.7	99.8	100.0	100.8
REMARK	L BANK				RIVER									

**CROSS SECTION OF HOUAY MAKCHAN RIVER  
AT NIKHON34 (2/3)  
(at staff gauge)**



by HEC

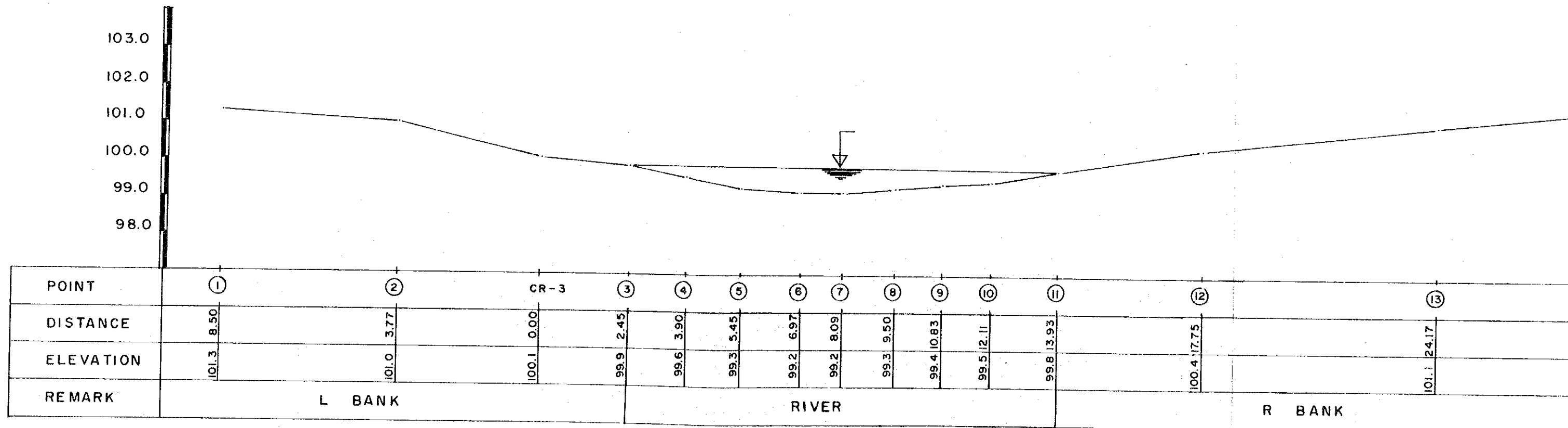




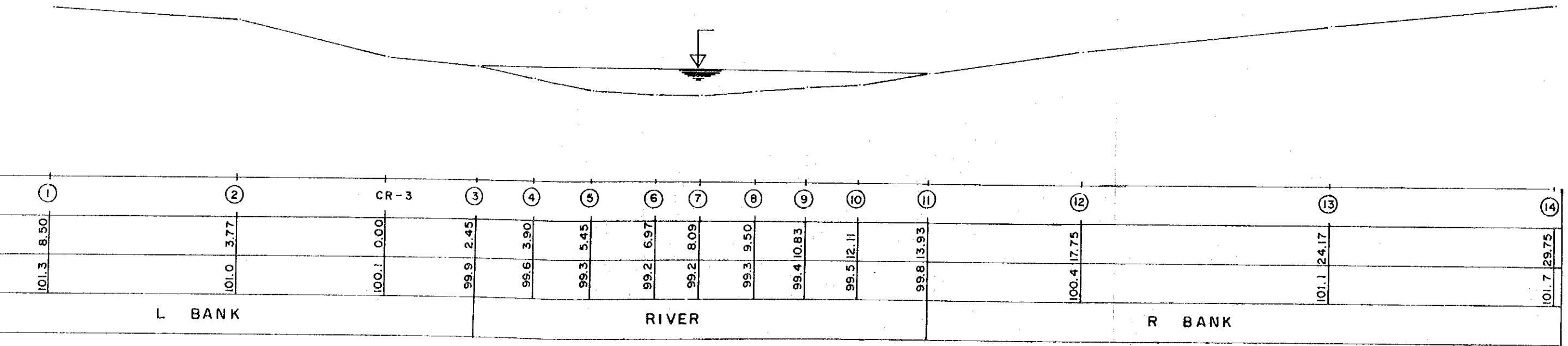
# CROSS SECTION OF HOUAY MAKCHAN RIVER

AT NIKHON34 (3/3)

(10 m downstream from staff gauge)



**CROSS SECTION OF HOUAY MAKCHAN RIVER  
AT NIKHON34 (3/3)  
(10 m downstream from staff gauge)**



by HEC



**APPENDIX-4 Regulating Pond Capacity**



APPENDIX-4

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A4.1 Necessity of Pond . . . . .	AP-4-1
A4.2 Calculation Condition . . . . .	AP-4-1
A4.3 Results of Analysis . . . . .	AP-4-2
A4.4 Regulating Pond Operation . . . . .	AP-4-2



## Appendix 4

### A4.1 Necessity of Pond

Xe-Katam Small-scale Hydroelectric Power Station is a run-of-river, therefore basically generating power is limited up to current stream energy at anytime.

On the other hand, power demand fluctuates from bottom load to peak load during day. After commissioning, according to demand growth in future, this power station will spill water in midnight as bottom load and will not generate enough power to demand at peak load.

Provided that, spill energy can be used for peak load portion, stream energy will be used in so high effectivity. As following daily load curve, this power station can serve stable power to Se Kong and Attapeu up to stream capability.

For this solution, required capacity has been calculated and examined. Calculation conditions are shown in A4.2.

### A4.2 Calculation Condition

Calculation condition are follows.

Inflow : 1.10 m<sup>3</sup>/sec equivalent value to 95% firm capacity  
Spill Energy : Adjustment to zero  
Effective Head : Constant  
Efficiency : Depend on load  
Received Power : Required power for adjusting spill energy to zero  
Study Team : From 1995 to the year, that expected bottom load at midnight will exceed 1,400 kW (1.1 m<sup>3</sup>/sec equivalent power) (1995 ~ 2006)

Results of these calculation are shown in Table A-4-1.



#### **A4.3 Results of Analysis**

From the results of this analysis, the maximum required pond capacity is estimated as 11,100 m<sup>3</sup> in 2000. However, after 2001 this value will be reduced, and daily load curve is calculated by estimated value. Therefore, adequate capacity of this pond is recommended as 10,000 m<sup>3</sup>.

#### **A4.4 Regulating Pond Operation**

Since above analysis was carried out based on 95% firm capacity, it appeared that this pond is not effective after 2001. However, at that time it will be still effective, if the critical inflow set larger value than 1.1 m<sup>3</sup>/sec, such as 1.5 m<sup>3</sup>/sec.

















ation of Regulating Pond Capacity (3)

1328	1328	1328	1382	1520	1651	1809	2212	2658	2605	2605	2658	1664	1961	2605	2658	2802	2418	1768	1713	1606	1489	1382	1328	46479	
2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	
				500	500	700	1000	1000	1000	1000	1000	1000	900	1000	1000	1000	1000	400	300	200	45			13545	
1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10 (Average)
1.06	1.06	1.06	1.11	0.82	0.92	0.89	0.97	1.33	1.29	1.29	1.33	0.53	0.85	1.29	1.33	1.44	1.14	1.10	1.13	1.13	1.16	1.11	1.06	1.10 (Average)	
0.04	0.04	0.04	-0.01	0.28	0.18	0.21	0.13	-0.23	-0.19	-0.19	-0.23	0.57	0.25	-0.19	-0.23	-0.34	-0.04	0.00	-0.03	-0.03	-0.06	-0.01	0.04	0.00 (Average)	
0.25	0.38	0.51	0.48	1.49	2.13	2.88	3.34	2.52	1.85	1.19	0.36	2.40	3.30	2.63	1.81	0.57	0.44	0.45	0.34	0.24	0.03	0.00	0.13	0.00	
							3.34															0.00	Start		

.34

1421	1421	1421	1478	1626	1767	1936	2367	2844	2787	2787	2844	1780	2098	2787	2844	2999	2588	1892	1833	1719	1593	1478	1421	49732	
2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	
200	200	200	500	1000	1000	1000	1000	1000	1000	1000	1000	1000	1000	1000	1000	1000	1000	600	400	400	150	90	50	16790	
1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10 (Average)
0.98	0.98	0.98	0.79	0.50	0.62	0.75	1.10	1.48	1.43	1.43	1.48	0.63	0.88	1.43	1.48	1.60	1.27	1.04	1.15	1.06	1.16	1.11	1.10	1.10 (Average)	
0.12	0.12	0.12	0.31	0.60	0.48	0.35	0.00	-0.38	-0.33	-0.33	-0.38	0.47	0.22	-0.33	-0.38	-0.50	-0.17	0.06	-0.05	0.04	-0.06	-0.01	0.00	0.00 (Average)	
0.44	0.87	1.31	2.44	4.59	6.33	7.59	7.60	6.25	5.05	3.86	2.51	4.21	5.00	3.81	2.45	0.66	0.04	0.27	0.09	0.25	0.04	0.00	0.00	0.00	
							7.60															0.00	Start		

.60



**APPENDIX-5 Preliminary Design**



## APPENDIX 5 Preliminary Design

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### A5.1 Stability Calculation of Intake Dam

The stability calculation of the dam was conducted for the following two cases concerning its overflowing section.

Water Level	Case	Earthquake
1. At Design Flood	Normal Condition	---
2. At Normal Full Water Level Under Earthquake		K = 0.02 to downstream direction

#### (a) Stability Calculation

The dam stability calculation are performed concerning turn-over, slide and bearing force.

##### i) Stability Study Concerning Turn-Over

$$X_0 = \Sigma M / \Sigma V$$

$$e = X_0 - (B/2 - B_1) \leq e_a = B/6$$

- Where:  $X_0$  ; position at which composite force is applied  
(distance from dam axis) (m)  
 $e$  ; distance of eccentricity (m)  
 $B$  ; width of foundation  
 $B_1$  ; distance from dam axis to upstream end of  
foundation (m)  
 $e_a$  ; Middle Third (m)

##### ii) Stability Calculation Concerning Sliding

The stability condition against sliding is that the shearing friction safety factor is 4 or more.

$$N = (\tau_a B + f \cdot \Sigma V) / H \geq 4$$

- Where:  $N$  ; safety factor against sliding  
 $\tau_a$  ; shearing strength between concrete and foundation  
rock ( $t/m^2$ )  
 $f$  ; friction coefficient between concrete and  
foundation rock  
 $H$  ; total horizontal force acting on the dam (t)

iii) Stability Study Concerning Bearing Force

The stability condition concerning bearing force is studied by examining the allowable bearing force of dam and foundation rock.

$$P_1 = \Sigma V/B(1 + 6 \cdot e/B) \leq \sigma_a$$

$$P_2 = \Sigma V/B(1 + 6 \cdot e/B) \leq \sigma_a$$

Where:  $P_{12}$ : reaction force of foundation rock against vertical force (t/m<sup>2</sup>)

$\sigma_a$  : allowable bearing force of foundation rock (t/m<sup>2</sup>)



(b) Design Condition

i) Dam Dimensions

Dam Crest Elevation EL 468.000 (m) Dam Height 6.200 (m)  
Dam Foundation Elevation EL 462.000 (m) Crest Width b = 2.000 (m)  
Slope of Upstream Side 1:0.00  
Slope of Downstream side 1:0.80

ii) Regulating Pondage Dimensions

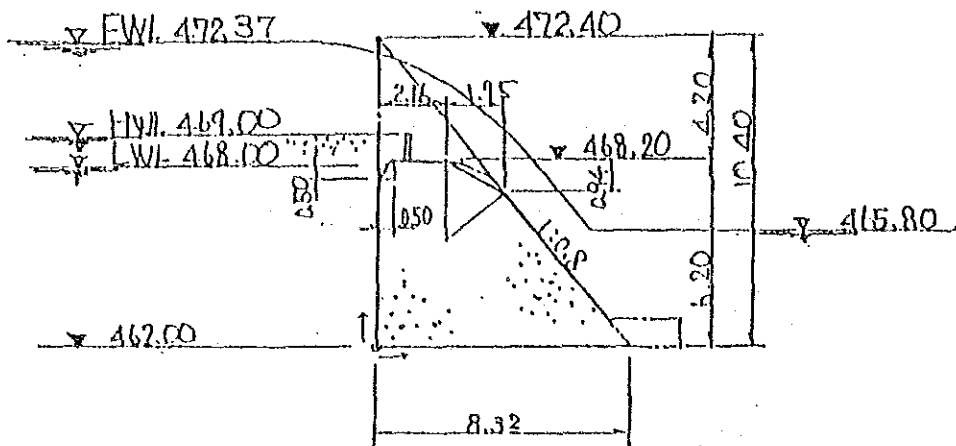
Design Flood Water Level WL. 472,370 (m)  
Normal Full water Level WL. 469,000 (m)  
Downstream Water Level (with flood design water level) WL  
465.800 (m)  
Downstream Water Level (with normal full water level) WL  
462.000 (m)  
Sediment Level EL 469.00 (m) Wave Height hW = 0.000 (m)  
With no drain hole

iii) Unit Weight

Concrete  $\gamma_c = 2.30 \text{ (t/m}^3\text{)}$   
Sediments  $\gamma_d = 1.20 \text{ (t/m}^3\text{)}$

iv) Coefficients

Sediment Pressure Coefficient  $C_e = 0.50$   
Friction Coefficient between  
Concrete and Foundation Rock  $f = 0.70$   
Shearing Unit Stress of Foundation Rock  $T_o = 50.0 \text{ (t/m}^2\text{)}$   
Allowable Bearing Capacity of Foundation Rock  $R_a = 100.0 \text{ (t/m}^2\text{)}$   
Design Seismic Intensity  $K_h = 0.02$



(c) Calculation

i) At Design Flood (under Normal Conditions)

[1] EL 462.000 (m)

Item	V (t)	H (m)	X (m)	Y (m)	M (m)
Dead Load	81.684	0.000	3.104	0.000	253.533
Water Pressure	12.000	45.136	3.000	2.660	156.061
Sediment Pressure	0.000	14.700	0.000	2.333	34.300
Upstream Surface Water Weight	0.000	0.000	0.000	0.000	0.000
Upstream Surface Sediment Weight	0.000	0.000	0.000	0.000	0.000
Downstream Surface Water Weight	5.776	0.000	7.307	0.000	42.203
Downstream Surface Sediment Weight	0.000	-7.220	0.000	1.267	-9.145
Uplift Force	-40.740	0.000	3.849	0.000	-156.827
Total	58.720	52.616			320.126

[2] EL 463.000 (m)

Item	V (t)	H (m)	X (m)	Y (m)	M (m)
Dead Load	63.468	0.000	2.857	0.000	181.336
Water Pressure	12.000	32.256	3.000	2.268	115.949
Sediment Pressure	0.000	10.800	0.000	2.000	21.600
Upstream Surface Water Weight	0.000	0.000	0.000	0.000	0.000
Upstream Surface Sediment Weight	0.000	0.000	0.000	0.000	0.000
Downstream Surface Water Weight	3.136	0.000	6.773	0.000	21.241
Downstream Surface Sediment Weight	0.000	-3.920	0.000	0.933	-3.659
Uplift Force	-29.303	0.000	3.407	0.000	-99.843
Total	49.301	42.136			236.625

[3] EL 464.000 (m)

Item	V (t)	H (m)	X (m)	Y (m)	M (m)
Dead Load	47.092	0.000	2.611	0.000	122.977
Water Pressure	12.000	26.376	3.000	1.866	85.216
Sediment Pressure	0.000	7.500	0.000	1.677	12.500
Upstream Surface Water Weight	0.000	0.000	0.000	0.000	0.000
Upstream Surface Sediment Weight	0.000	0.000	0.000	0.000	0.000
Downstream Surface Water Weight	01.296	0.000	6.240	0.000	8.087
Downstream Surface Sediment Weight	0.000	-1.620	0.000	0.600	-0.972
Uplift Force	-19.466	0.000	2.936	0.000	-57.150
Total	40.923	32.256			170.657

EL (m)	V (t)	H (t)	X (m)	Y (m)	M (t·m)	N	B (m)	P1 (t/m <sup>2</sup> )	P2 (t/m <sup>2</sup> )
462.00	58.720	52.161	320.126	5.452	1.292	8.688	8.320	13.632	0.483
463.00	49.301	42.136	236.526	4.800	1.040	9.742	7.520	11.994	1.118
464.00	40.923	32.256	170.657	4.170	0.810	11.305	6.720	10.495	1.684

ii) At Normal Full Water Level (under Earthquake)

[1] EL 462.000 (m)

Item	V (t)	H (m)	X (m)	Y (m)	M (m)
Dead Load	81.684	1.634	3.104	2.598	257.777
Water Pressure	0.000	24.180	0.000	2.279	55.097
Hydraulic Pressure	0.000	0.550	0.000	2.651	1.457
Sediment Pressure	0.000	14.700	0.000	2.333	34.300
Upstream Surface      Water Weight	0.000	0.000	0.000	0.000	0.000
Upstream Surface      Sediment Weight	0.000	0.000	0.000	0.000	0.000
Uplift Force	-9.707	0.000	2.773	0.000	-26.920
Total	71.978	41.063			321.711

[2] EL 463.000 (m)

Item	V (t)	H (m)	X (m)	Y (m)	M (m)
Dead Load	63.468	1.269	2.857	2.202	181.336
Water Pressure	0.000	17.680	0.000	1.937	34.251
Hydraulic Pressure	0.000	0.432	0.000	2.240	0.967
Sediment Pressure	0.000	10.800	0.000	2.000	21.600
Upstream Surface      Water Weight	0.000	0.000	0.000	0.000	0.000
Upstream Surface      Sediment Weight	0.000	0.000	0.000	0.000	0.000
Uplift Force	-7.520	0.000	2.507	0.000	-18.850
Total	55.948	30.181			222.100

[3] EL 464.000 (m)

Item	V (t)	H (m)	X (m)	Y (m)	M (m)
Dead Load	47.092	0.942	2.611	1.798	124.670
Water Pressure	0.000	12.180	0.000	1.593	19.404
Hydraulic Pressure	0.000	0.323	0.000	1.828	0.590
Sediment Pressure	0.000	7.500	0.000	1.667	12.500
Upstream Surface Water Weight	0.000	0.000	0.000	0.000	0.000
Upstream Surface Sediment Weight	1.296	0.000	0.000	0.000	0.000
Uplift Force	-5.600	0.000	2.240	0.000	-12.544
Total	41.492	20.945			144.620

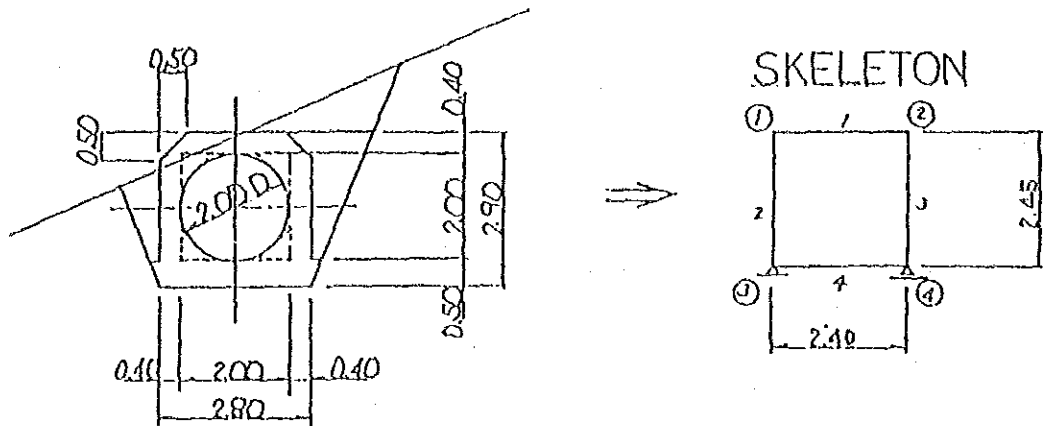
EL (m)	V (t)	H (t)	X (m)	Y (m)	M (t-m)	N	B (m)	P1 (t/m <sup>2</sup> )	P2 (t/m <sup>2</sup> )
462.00	71.978	41.063	321.711	4.470	0.310	11.358	8.320	10.583	6.720
463.00	55.948	30.181	222.100	3.970	0.210	13.756	7.520	8.685	6.195
464.00	41.492	20.945	144.620	3.485	0.125	17.429	6.720	6.866	5.483

#### A5-2 Stress Analysis of Culvert

The stress is analyzed concerning the external water pressure in river flood and internal water pressure of the channel.

##### (1) Analysis Concerning External Water Pressure At River Flood

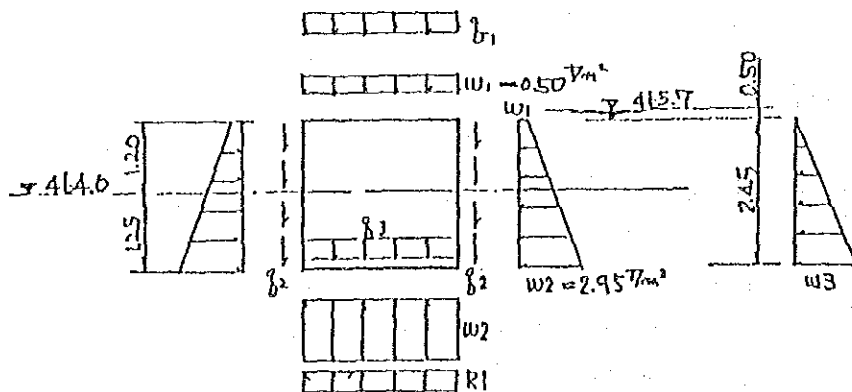
- (a) The stress is calculated by assuming the cross section of the channel as a rahmen structure having the skeleton given in the figures below.



(b) Load

The river water level of 465.7 m at the most upstream section of the culvert, which is reached at the time of 1/200 flood with 840 m<sup>3</sup>/s is assumed to be the external water pressure.

External water pressure:  $W_1 = 0.50 \text{ t/m}^2$ ,  $W_2 = 2.95 \text{ t/m}^2$



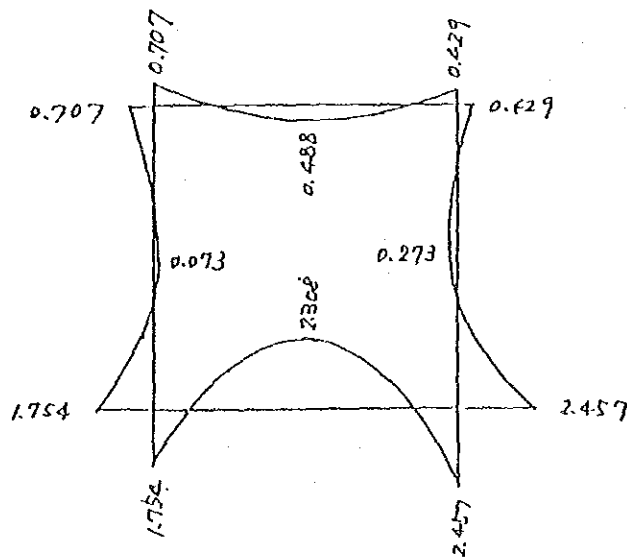
Dead-weight:  $q_1 = q_2 = 0.40 \text{ m} \times 2.4 \text{ t/m}^3 = 0.96 \text{ t/m}^2$   
 $q_3 = 0.50 \text{ m} \times 2.4 \text{ t/m}^3 = 1.20 \text{ t/m}^2$

Reaction force:  $R_1 = 0.96 \text{ t/m}^2 \times 2.45 \text{ m} \times 2/2.4 \text{ m} + 0.96 \text{ t/m}^2$   
 $+ 1.20 \text{ t/m}^2 = 4.12 \text{ t/m}^2$

Earth pressure:  $W_3 = 0.5 \times 0.8 \text{ t/m}^3 \times 2.45 \text{ m} = 0.98 \text{ t/m}^2$   
 (Static earth pressure)

(c) Calculation Result

MOMENT DIAGRAM



**Stress Calculation for Rectangular Cross Section**

Cross Section No.		1. (Inside)	2. (Outside)
M	[t·m]	2.31	2.46
N	[t]	0.00	0.00
S	[t]	0.00	7.64
b	[cm]	100.0	100.0
h	[cm]	50.0	50.0
d	[cm]	43.0	43.0
d'	[cm]	7.0	7.0
As	[cm <sup>2</sup> ]	11.500 D19@250	15.500 D22@250
As'	[cm <sup>2</sup> ]	15.500	11.500
n=Es/Ec		15.00	15.00 Ec=220,000 kg/cm <sup>2</sup>
P=As/(b x d)	[%]	0.267	0.360
u=d-h/2	[cm]	18.000	18.000
f=M/N+u	[cm]	18.000	18.000
f/d		0.419	0.419
d' / d		0.163	0.163
As' / As		1.348	0.742
M' =M + N x u	[t·m]	2.308	2.457
X	[cm]	9.996	11.458
C		8.278	7.437
S		27.331	20.472
Z		1.096	1.107
$\sigma_c$	[kg/cm <sup>2</sup> ]	10.3	9.9
$\sigma_s$	[kg/cm <sup>2</sup> ]	511.7	408.1
$\sigma'_s$	[kg/cm <sup>2</sup> ]	46.5	57.7
$\tau$	[kg/cm <sup>2</sup> ]	0.00	1.97
$\tau_m$	[kg/cm <sup>2</sup> ]	0.00	1.78
$\sigma_{ca}$	[kg/cm <sup>2</sup> ]	70.0	70.0
$\sigma_{sa}$	[kg/cm <sup>2</sup> ]	1,800.0	1,800.0
$\tau_a$	[kg/cm <sup>2</sup> ]	4.00	4.00

Note: The negative sign of  $\sigma_s$  signifies compression. The negative sign of  $\sigma'_s$  signifies tension.  $\tau_m$  signifies the average unit shearing stress.



(2) Analysis for Internal Pressure

(2)-1 Check of Stress in Lining Concrete of Culvert

- \* The calculation is performed by assuming the culvert to be a cylindrical culvert having internal diameter of 2 m and concrete thickness of 0.4 m.
- \* The circumferential stress  $\sigma$  of a cylindrical culvert having internal diameter of  $\gamma$  and concrete thickness of  $\delta$  is expressed by the following equation.

$$\sigma = \frac{P}{\rho^2 - 1} \left( 1 + \sigma^2 \frac{\gamma_i^2}{\gamma^2} \right) \text{ (kg/cm}^2\text{)}$$

Here, the internal water pressure is:

$$470 - 461.5 + 1.63 = 10.13 \text{ m/m}^2 = 1.013 \text{ kg/cm}^2$$

(static pressure)(water hammer)

Letting  $\gamma$  be the distance from the center of the cylinder to the cross section to be analyzed (m), and

$$\rho = \frac{\gamma_i + \delta}{\gamma_i} = \frac{1 + 0.40}{1} = 1.40$$

The stress  $\sigma_1$  on the internal surface of concrete is, as  $\gamma = \gamma_i$ ,

$$\sigma_1 = \frac{P}{\rho^2 - 1} (1 + \rho^2) = P \cdot \frac{1.40^2 + 1}{1.40^2 - 1} = 3.08 \text{ kg/cm}^2$$

The stress  $\sigma_2$  on the outside surface of concrete is, as  $\gamma = \gamma_i + \delta$ ,

$$\sigma_2 = \frac{P}{\rho^2 - 1} (1 + 1) = 1.0136 \times \frac{2}{1.40^2 - 1} = 2.11 \text{ kg/cm}^2$$

The allowable tensile stress can be regarded as  $\sigma_{st} = 10 \text{ kg/cm}^2$  when concrete rich in cement is placed at the site and sufficiently compacted.

Therefore, it is safe with  $\sigma_{1,2} < 10 \text{ kg/cm}^2$

Reinforcing steel bars of 19 mm diameter will be placed circumferentially in double layer on the inside and outside surface with 25 cm intervals, to improve strength and prevent crack.

The effect of reinforcement per 1 cm is:

$$\lambda = \frac{\delta}{\delta + 8a_{st}} = \frac{40}{40 + 8 \times 11.46} = 0.303$$

$$\lambda \sigma_1 = 0.303 \times 3.08 = 0.933 \text{ kg/cm}^2$$

$$< \delta_{ct} 10 \text{ kg/cm}^2$$

$$\lambda \sigma_2 = 0.303 \times 2.11 = 0.639 \text{ kg/cm}^2$$

(2)-2 Check of Stress Based on Assumption that All Water Pressure is Borne by Steel Reinforcing Bars

The steel reinforcing bars are regarded as thin cylinders to calculate the stress of steel bars,  $\sigma_t$  ( $\text{kg/cm}^2$ ).

$$\sigma_t = \frac{P \cdot D}{2 \cdot t}$$

P: internal water pressure, 1.013 kg

D: inside diameter to steel bars, 214 cm

t: thickness of thin plate equivalent to bar cross section (cm)

Assuming the steel bar of 19@250,  $\frac{2.865 \text{ cm}^2 \times 4}{100 \text{ cm}} = 0.1146 \text{ cm}^2/\text{cm}$

Therefore,  $\sigma_t = \frac{1.013 \times 214}{2 \times 0.1146} = 946 \text{ kg/cm}^2 < \sigma_{ta} = 1,800 \text{ kg/cm}^2$

### A5.3 Stress Analysis of Internal Water Pressure of Headrace Tunnel

The analysis of the stress caused by the tunnel internal water pressure has been performed by Ott-Frey-Bear's equation.

The water pressure at the most downstream part of the lined section of the tunnel, where the internal pressure (static pressure plus water hammer pressure) becomes maximum, of  $P = 2.439 \text{ kg/cm}^2$  was used to calculate the tensile stress of the reinforcing steel bars. In the analysis, the following figure has been utilized.

When the steel reinforcing bars, D19, are installed with interval of 30 cm, the amount of steel bars in the circumferential direction;

$$F_e = 0.0955 \text{ cm}^2/\text{cm}$$

$$(F_e/D) \times 10^3 = (0.0955/200) \times 10^3 = 0.4775$$

where  $D =$  internal diameter of 200 cm.

\* Assuming the modulus of elasticity of the surrounding rocks,  $E_r$ , to be  $10,000 \text{ kg/cm}^2$ ,

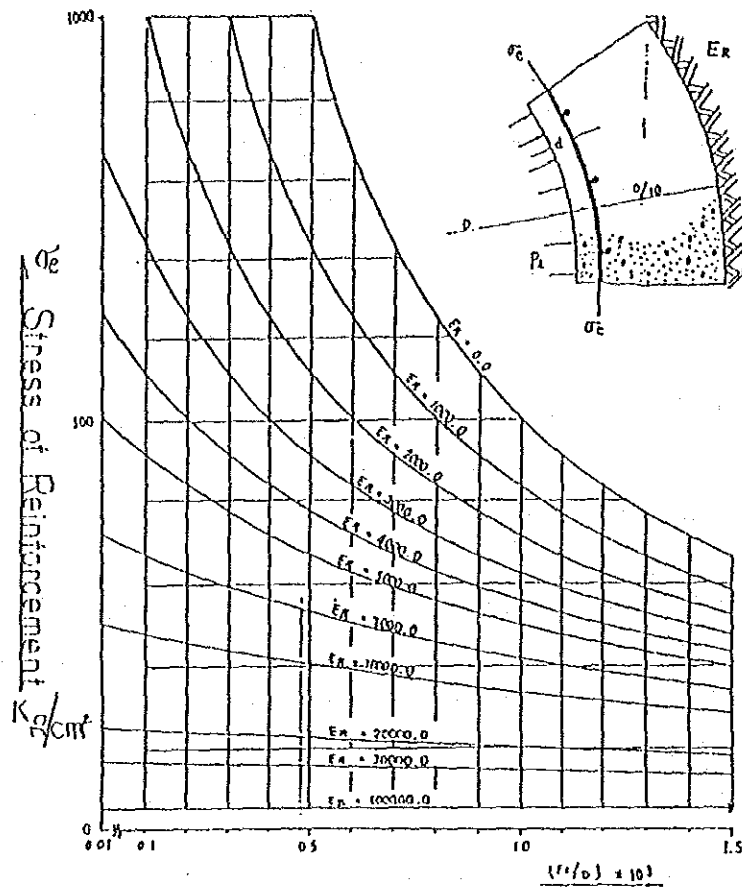
The unit stress of the steel bar,  $\sigma_e = 200 \text{ kg/cm}^2$

Therefore, the unit stress of the real steel bars;

$$\sigma_{e1} = 200 \times 2.439 = 488 \text{ kg/cm}^2 < \sigma_{ta} = 1,800 \text{ kg/cm}^2$$

\*  $E_r = 30,000 \text{ kg/cm}^2 \rightarrow \sigma_e = 80 \text{ kg/cm}^2$

$$\therefore \sigma_{e1} = 80 \times 2.439 = 195 \text{ kg/cm}^2 < \sigma_{ta} = 1,800 \text{ kg/cm}^2$$



Water Pressure:  $P_i = 1 \text{Kg/cm}^2$   
 $F_e$ : Area of Reinforcement ( $\text{cm}^2/\text{cm}$ )  
 $D$ : Tunnel Dia. ( $\text{cm}$ )

#### A5.4 Bearing Capacity of Penstock Foundation

According to the penetration test of (III-2), the N-value down the depth of 2.3 m is from 5 to 18, and its average value is 11.

In the construction work, the talus pile will be excavated and removed for the depth below this range, the penstock is installed on a continuous concrete foundation, and soil will be filled back on the penstock for a depth of more than 1 m. Therefore, the foundation will be installed at depth which is more

than 2 m below the ground surface. The ultimate bearing capacity of continuous footing,  $q_d$ , is given by the following equation according to Terzaki and Peck.

$$q_d = \alpha \cdot \beta \cdot \gamma \cdot N_r + D_t \cdot \gamma \cdot N_q$$

$\alpha$  = coefficient determined by the geometry of the loading surface of the foundation; 0.5 for continuous foundation

$\beta$  = width of footing; 1.5 m

$\gamma$  = unit weight of soil; 1.8 t/m<sup>3</sup>

$D_t$  = setting depth of footing; 2.5 m

For the value of  $N = 11$ ,  $N_r = 7$ ,  $N_q = 10$  (with internal friction angle of 30°) is obtained from the Related Graph (Omission).

From these values, we obtain the ultimate bearing capacity of approximately 55 t/m<sup>2</sup>. Taking a safety factor of 3, the allowable bearing capacity is 18 t/m<sup>2</sup>. In this case, the load on the foundation is no more than 5 t/m<sup>2</sup>, and sufficient safety is assured.

#### A5.5 Calculation of FRP Pipe Strength

##### (a) Load

- i) Maximum design head = High water level 469.0 m + Flood water level 1.0 m - Minimum elevation of penstock 305.2 m = 164.8 m  
Maximum water hammer head = (469.0 + 1.0 - water turbine nozzle center elevation 306.7 m) x 0.1 = 16.3 m  
Maximum design head = 181.1 m

- ii) Earth cover: First Stage: 1.0 m, Latter Stage: 1.2 m or more  
(refer to \*1)

- iii) Bulldozer load: 3 ton class, 1 bulldozer

- iv) Weight of unit volume of earth: 0.0018 kg/cm<sup>4</sup>

- v) Passive earth pressure resistance coefficient: 14 kg/cm<sup>2</sup>

\*1. (Reference)

Earth Cover on the Buried Section

It is possible that the ground water level rises to the ground surface due to very heavy rain. The pipe must be buried in such a depth that the pipe does not float up when the ground water rises. The minimum earth cover, H (m), with which the FRP pipe does not float up, is calculated by the following equation.

$$H \geq \frac{\pi D_c}{4} \cdot \frac{S - \left\{ 1 - \left( \frac{D}{D_c} \right)^2 \right\} \gamma_p}{w}$$

S: safety factor of 1.2

$D_c$ : pipe outside diameter; 0.936 and 1.144 m

D: pipe inside diameter; 0.9 m and 1.1 m

$\gamma_p$ : weight per unit volume of pipe material; 1.8 t/m<sup>3</sup>

W: weight per unit volume of filled back earth which is saturated; 1.8 t/m<sup>3</sup>

D = 0.9 m ----> H = 0.98 m ==> 1.0 m

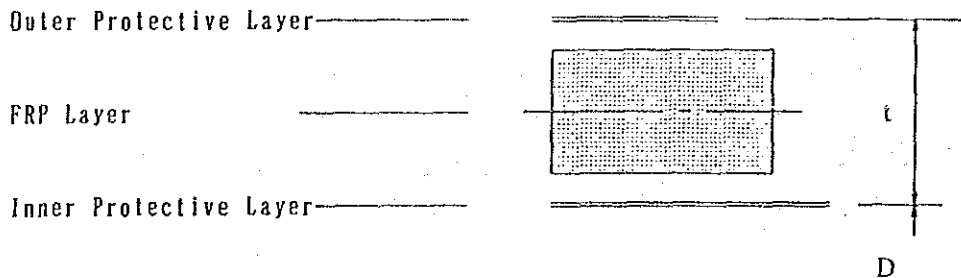
D = 1.1 m ----> H = 1.19 m ==> 1.2 m

(The required depth of earth cover.)

(b) Strength Calculation

i) Pipe Structure

The structure and dimensions of FRP pipe are given below.



	D	t
	mm	
FRP Pipe	900	18
	1100	22

The pipe thickness is the value set forth in JIS A 5350.  
No marginal wall thickness is provided.

ii) Strength and Cross Section Characteristics of Pipe

ii-1 Pipe Strength

ii-1-1 Pipe Strength

The strength of the FRP layer is illustrated below.

Classification	Symbol	FRP Pipe	
		Circumferential Direction	Axial Direction
Modulus of Elasticity	Ej	181300	90510
Tensile Strength	$\sigma_t$	3780	1641
Compression Strength	$\sigma_c$	3333	1389
Shearing Strength	$\tau$	550	

ii-1-2 Allowable Stress of Pipe

The allowable stress of the FRP layer is the fracture strength of FRP divided by the safety factor of 6.

The allowable stress values of the FRP layer is given below.