	···· - ('A X'X'''		·			,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,		.,		OHILO	. що,	
Year Date	1986 JAN.	FEB	. MAR.	APR.	МАУ	JUN.	JUL.	AUG.	SEP.	OCT.	NOV.	DEC.
Date		3. 97	2. 21	2. 42	3.97		11. 91		37.00			9.92
2		3. 97	2. 42	2. 21	3.97	10.32		13.70	32. 50		25.00	9.53
3		3. 97	2. 42	2, 62	4.73				29. 20		24.40	9.53
4		3. 97		2.04	4. 45			13.70	31.00	19.00		9.53
5		3. 97		2.04	5. 28	9.53	11. 91	14.90			22.00	9.13
ð	4. 13	3. 51	4. 46	2.04	0. 20	3, 00	11. 01	14.00	04.10	10.00		3.10
6	4.73	3. 97	2. 42	1.87	5.01		11.91	20.20	37.00	18, 40		
7	4.73	3.72	2. 42	1.87	8.15		11.91	16.00	44.50		20.80	9.13
8	1.45	3.72		1.87	6.15	9.13		16.60	45, 20	16.60		9.13
9	4.45	3.72				8.73		22.00			19.00	8.78
10	4.21	3, 43	2. 21	3.24	7.09	8.44	11.91	26.20	41.50	14.90	18.41	8.14
11	4. 21	3. 03	2. 62	2.83	7 40	8.78	11, 91	34 00	38.50	14.90	17.82	8.09
12		2. 83				10.72	11.11	37.80		14. 90		8.09
13		2. 83		2.42		13.69	9. 13	38.50	45, 20	14.90	17.32	8.09
14		2. 83		2.62				54.50		20.80		7.40
15		2. 83		2.83	8.78		11.51				17.32	7.40
10	4.61	2.00	2. 41	A. 00	0. 10	10, 12	11. 01	160,00	33, 33	10. 10	41.00	11.20
16	3.97	2.83	2.04	2, 83	9,92	11.51	13.10	58, 30	28, 60	17. 20	16.05	7.40
17		2.83	2.04	3.03	11.11	11.91		43.00		16.00	14.37	
18		2.83		3.48	10.32	11.91	16.54			17, 20		6.78
19		2. 83		3.97	16.05			37.00	28, 20	17.80		6.46
20		2. 83		4. 21					25.60		13.59	
								•				•
21	3.97	2.83	3.72	5.01	13.10	13.69	12.31		22.00		13.10	5.84
22	3.97	2.83	3.03	5.56	11.91	11.11	11.91	32.50	22.00	54, 50	12.70	5.56
23.	3.97	2.83	2.62	5. 28	11.91	10.32	12.31	31.80	22.00	26.80	13.40	5.23
24	3.97	2.83	2.62	5.01	13.69	12.31	13.59	29.20		26.80		5.01
25	3.97	2.83	3.03	5.46	13.10	12,70	13.59	34.00	22.60	26.80	11.51	5.91
26	3.97	2. 83	3.03	6.46	10 21	13.40	11 22	32, 50	23. 20	26.80	10.32	5. 56
20 27		2. 62		5.84	11.91		14. 28				9.92	5.84
28		2. 42		5.01	9.92		17. 28		21.40	27.40		5.84
2 9 2 9		6.46	2. 42	4.45					22.00			
30			2. 42			12.70			23. 20			5.84
30	3.31		C. 40	0.12	*1.11	12.10	10.00	40.00	40. 20	20,00		0.01
31	3.97		2. 42		11, 11	•	16.64	38.50	· ·	26.80		5, 56
TOTAL	131.07	88. 93	78. 15	104.45	291.60	339.24	415.46	1054.00	931. 90	671.90	493.10	227.24
AVE.	4, 23	3. 18	2.52	3.48	9.41	11.31	13.40	34.00	31.06	21.67	16.44	7.33
MAX.		3. 97		6.46	19.00	14.23	19.60	128.00	45. 20	54.50	26. 20	9.92
MIN.	3, 97	2.42	2.04	1.87	3.97	8.44	8.78	13.70	21.40	14, 30	9, 92.	5.01
DAYS	31	28			31	30	. 31	31	30	31	30	31
ANNIIAI	TOTAL		4827.0									: ;
	AVERAGE		13. 22						. *			11/11/11
ANNUAL			128.00			-						
ANNUAL			1. 87							•		
	DAYS		365									
11111101111	. 7.11.7.											

YEAR Date	1989 JAN.	FEB.	MAR.	APR.	MAY	JUN.	JUL.	AUG.	SEP.	OCT.	NOV.	DEC.
: 1					4.44	26. 44		22. 42	24.68	24. 10		
2					4.44	26.44	13.96	20. 26	50.00	22.97		
3	4,44	3.46	2.40	2.59	6.43	20. 26		31.40	39.58	21.87	10.63	6.7
4	2, 21		0. 10	3, 5,	4.44	17. 71		51.00	35. 38	30.12	15.00	
5					19.74			68.00	43.00	31.40		
6					9. 12	20. 26		51.00	100.00			
7					5.83	17.71	14.86	32.05	88.50			
8	3.94	2.80	2.02	2.02	13.96	17.71	14.00	31.40		19.74	10.24	6.4
9	0. 34	2.00	<i>L</i> . V <i>L</i>	6. V <i>t</i>	6.43	15. 78		31. 40	38. 16	19.13	10, 44	V. 4
10					11.42	68.50		28. 87	74.08			
11					06 00	25. 84		70 c2	90 16	10 91		
11					29.49			62.95	38.16	18.21		
12				0.00	6.13	25. 26	13, 09		34.04	17.71	0.00	
13	4.44	2.80	2.02	2.02	22.97	22. 42		51.00	38.16	46.30	9.86	5. 5
14					13.96	21. 33		43.00	50.00	24.10		
15					7.07	18. 71		28. 87	31.40	19.74		
. 16								34.04				
17.								32.05				
18	3.94	2.59	2. 20	3.23	9.12	15. 32	13.52	60.30	33.37	17.71	8.41	5. 2
19	0.04	5,00	2.50	0.50		20, 20	10.00	65.65		1,,,,	••••	•. 5
20								31.04		•	•	÷
21				٠			28. 87	43.00				
22	3.70	2.40	2. 20	3.94	8.41	16 73	13.09	28.87	28.87	13.09	7.39	4. 9
23	9. 10	4.40	2. 20	. 0.04	0.41	10.10	185.00	35, 38	40.01	10.00	1.00	4. 0
24							67.00	28.87				
25							46.00	28. 87				
26					7.39	20.79	37.00					
27					5.83	14.86	35.00	24.68	25.44	11.02	7.39	4.7
28	3.46	2.40	1.84	4.98	123.00	18.71	28.87					
29					20.26	18.71	28.87					
30					18, 21	13.96	31.40					
31	3.46	2.40	1.84	4.98	22, 97	13, 96	28. 87	32. 71	25. 26	10. 24	7.07	4.4
LATO	97 90	10 00	14 50	20 70	201 00	107 67	505 40	107/ 19	074 10	298 22	60.99	38. 1
OTAL	27.38	18.85						1074.12		21.89		
VE.	3.91	2.69	2.07	3.39		21.64	39.03	39. 78	46,01		8.71	5.4
IAX.	4.44	3.46	2.40		123.00		185.00		100.00	46.30	10.63	6.7
IIN.	3.46	2.40	1.84	2.02	4.44	13. 96	13.09		24.68	10.24	7.07	4. 4
AYS	31	28	31	30	31	30	31	31	30	. 31	30	3
MMITAT	ምር ምል ፤		2004 20								<u></u>	
	TOTAL	n.	3924.30									
	AVERAG	B .	17.59	-								
INNUAL			185.00									
NNUAL	MIN. Days		1.84 365									

Unit: m^3/s-d

YEAR Date	1990 JAN.	FEB.	MAR.	APR.	MAY	JUN.	JUL.	AUG.	SEP.	OCT.	NOV.	DEC.
1 2	4. 19	2. 59	1.51	3.46		3. 01	5. 54	20. 26	17.71		36.00	13.96
2 3 4 5			1.84	1.68		5. 83 3. 23		17. 22		80.00 43.00	;	
6 7							٠.					
8 9 10	3. 70		4.71 3.46					17. 71 13. 52	22. 42	34.71	22. 97	
11 12		1.68	3. 23 1. 84		·		6. 13	11.02	·			13.09
13 14	3. 46	e.				5. 83	8.76 9.12	•	•	29.49		
15						•	13.09	•		34.71	15. 78	
16 17 18			1. 51	3.70		8. 76 15. 32	15. 32 42. 00 15. 32			29. 49	÷	
19 20						10. 52			43.00		13.52	
21 22 23 24	2.80	1.51			·	8. 76 5. 83 9. 12 12. 24		43.00 20.26		36.00		":
25						7. 39			29.49			
26 27			3. 23	3. 23		5. 83		15. 32				
28 29				0.20			17. 71	22. 97				
30								17. 71			13.96	
31												
OTAL	14. 15	5. 78	21. 33	12.07	0.00		132. 99			316.27		27.05
VE. IAX.	3.54 4.19	1.93 2.59	2.67 4.71	3.02 3.70	*	7.60 15.32	14. 78 42. 00	19.90 43.00	28. 16 43. 00	39. 53 80. 00	20.45 36.00	13.53 13.96
IN. Ays	2.80 31	1.51 28	1.51	1.68 30	. 31	3. 01 30	5. 54 31	11.02 31	17. 71 30	28.87 31	13.52 30	13.09 31
											· · · · · ·	
	TOTAL		1034.63									
NNUAL	AVERAGE MAX.		14.10 80.00									
NNUAL	MIN.		1.51							·	e e saifs La	
NNUAL	DAYS		. 365									

A3.5.2 Rainfall Record for TANK MODEL Analysis

Daily Rainfall Record at Nonghin Staton (1980-1990)

2	H 4	## ## ## ## ##	n	11		\$\$ \$\$ \$\$ \$\$ \$\$ \$\$ \$\$ \$\$ \$\$		\\ 	14 14 14 16 16 18	## ## ## ## ## ## ## ##	28 28 28 39 39 31 31 31 31 31 31 31 31 31 31 31 31 31	6 6 6 11 11 11 11 11	11 11 11 11 11 11 11 11 11 11 11 11 11	# # # # # # # # # # # # # # # # # # #
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0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0			_	·	0.0	6.8	0.0	34.5	0.1	0.0	0.0	4-1	0.0	0
0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	o		_	ċ	0.0	0.0	0.0	1.8	0.0	0.0	1.0.0	13.2	- 0.0	0.0
0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	01			ċ	0.0	0.0	37.8	27.6 1	0.4	7.2 !	0.0	0.0	0.0	0.0
0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	11				0	0.0	0.0	15.4	0.5	7.0	15.1	26.4	• • •	0
0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	12	_	1.0.0	•	7.2	0.0	48.2.1	0.6	0.3	10.01	0.0	.10.0	0.0	0.0
0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	13		0.0		22.0 (0.0	46.0	7.0	4:0	10.6	10.4	2.0	0.0	0.0
0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	4 11		0.0	•	0 0	0 0	0 0	0.0	 o o	0 6	0 6	N 10	0.0	0 0
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0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	9 #		 C	•	0.0	48.6	134.8	1.4	10.1	26.1	42.0	4.0	4.0	0.0
0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	17		- 0.0		0.0	0.0	1 0.65	16.2	- 0.0	0.0	. 5.4	2.3	9.0	0.0
0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	13		- 0.0		1.0.4	0.0	0.0	7.0 +	10.4 1	- 0.0	35.0	2.1	9.0	0.0
0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	6				0	0	82.4	 	4.01	13.4	0.9	2.6	0.0	0.0
0.0 0.0 0.0 0.0 15.4 15.4 16.4 0.0 0.0 0.0 0.0 10.0 12.4 12.4 15.4 14.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	20		 0	0	31.6	0	0.0	 	13.4	 0	4	•	•••	0
0.0 0.0 0.0 0.0 0.0 0.0 1.5.4 41.4 0.0 0	21			٥	5.0.2	0.0	0.0	6.0	67.8	24.2	16.4	0.0	0.0	0.0
0.0 0.0	22		0.4 1	27.2	8.6	0.0	0.0	39.2	57.6 1	15.4	41.0 1	0.0	0.0	0.0
0.0 0.0	23		- 0.0	0.0	34.2	0.0	0.0	.0.4	1 6.67	12.4	υ 1	8.2	0.0	0.0
0.0 0.0 0.0 1.2 0.0 1.2 0.0 1.2 0.0 1.2 0.0	54		- 0.0	0.0	0.0	0.0	0.0	18.4	 ທຸ	e S	0.0	41.4	0.0	0.0
0.0 0.0 0.0 11.3 10.1 20.0 22.6 0.0 0.0 0.0 0.0 0.0 11.2 0.0	23			•		0	0.0	 M	0.0	. 9.56	•	 0	 0	0.0
0.0 0.0 0.0 31.4 32.4 0.1 11.3 4.0 5.1 0.0 0	26				0.0	36.0	0.0	73.4	4.4	3.9	0.0	4.6	0.0	0.0
0.0 0.0	27		- 0.0		0.0	0.0	0.0	11.3	10.8	10.1	20.0	22.6 1	- 0.0	0.0
0.0 0.0	23		0 :		1 0.0	0.0	32.4	24.8		11.8	0.4		0	0.0
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1.2 3.4 1.48.2 502.1 4.50.5 3.72.9 3.70.9	9 F		 - 0		000		000	;	10	n 0	· -	 - c		
13.6 148.2 148.2 150.0 12.9 178.5				•	2	-	;		-	,		?		
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		# # # # #	# . 4 · 0	27.	- 24.20 - 24.20	48.6	+ m = 0 = 0 = 0 = 0 = 0 = 0 = 0 = 0 = 0 =	73.4	67.8.1	95.4	# H H H H H H H H H H H H H H H H H H H	47.4	日子は万田は非非な日	2.2
		# # # # + —	+ H B B B O T O	# # # O	# # # # # # # # # # # # # # # # # # #	**************************************	4 H D O O O	+ # O * O # B B B B	4 4 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	1 0 0 0	N + 11 11 11 11 11 11 11 11 11 11 11 11 1	H + II I	# + # # # O * O . O	19 O · O · D · D · D · D · D · D · D · D ·
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						* -	新州村村村村村村村	(关切处外分类的分类分类	***	****	· · · · · · · · · · · · · · · · · · ·	华州州州州州州州	计计划分析设计计划	***
の						*	# 张芳 茶 转 赞 处 茶 关 书]	计分类分类分类表	计技术分类并补充数计块	7. 接触 等 被 被 被 被 被 被 被	*****	特殊被害的复数形式	计分数法规划计算技术	*****
"转锋场外有效应该有效的现在分词 计分类系统 医多种性细胞 医多种性细胞 医多种性细胞 医多种性 医多种性 医多种性 医多种性 医多种性 医多种性 医多种性 医多种性						_	ь Б	 	¥ ≥	A G E	х	Ξ 2 Σ	Η 2 Η Σ	E D E
7.7 - 1.86 - 1.77 - 1.78 - 1.7			:				长花科技学教物用价格	***	*****	***	*	****	*******	***

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- ~ -	- 0.0	0.0	0.0	- 0.0	32.2	33.6	1 4.0	81.2	0.0	- 0.0	- 0.0	0.0
- m	- 0.0	- 0.0	~ 0.0	0.0	0.0	33.4	10.8	92.4	35.1	51.2	- 0.0	0.0
. 6	0.0	0.0	36.0 1	0.0	4.8	20.7	1 5.77	33.9	0.8	0.6	- 0.0	0.0
1 10	- 0.0	- 0.0	13.6	0.0	0.0	65.3	0.2	2.5	29.0 1	N.8 .	0.0	0.7
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}	?	- ~ }		?))	4	•			-		
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13	0.0	-0.0	0	11.8	40.2	д. В.	12.8	7.0	10.1	-0.0	0	0.0
14	- 0:0	26.8	0.0	27.8 1	-0.0	36.8	~ 0.0	0.0	0.0	30.7	0.0	0.0
12	0.0	0.0		0.0	-0.0	- 8.6	6.0	0.0	1.2	26.4	0.0	0.0
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16	0.0	- 0.0	0.0	- 0.0	0.0	21.8	0.0	3.5	. 9.0	- 0	- 0.0	- 0 0
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- 13	- 0.0	0.4	- 0:0	1.6	₩ 9.0	9.0	32.8	9.3	53.4	39.8	0.0	- 0.0
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54	0.0	- 0.0	0.0	ດ ຄ.	27.6	35.1	- 0.1	1.7	5.8	 8	0.0	 0 0
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TOTAL TOTAL	2.5 L	27.2	324.9 (94.0	281.7	339.2	205.9 (425.7 1	377.1	267.3	44.3	0.0
一日山山城城市 田田村村 十月日日日	H 11 + 11 11 11 11 11 11 11 11 11 11 11 1	11 11	***************************************	11 本村田田田田田田田田田田田田田田田田田田田田田田田田田田田田田田田田田田田	11年11日日日日日日	11年11日日日日日日日日日日日日日日日日日日日日日日日日日日日日日日日日日	11+11111111111111	8 + # 11 65 82 14 14 14 14 14 14 14 14 14 14 14 14 14	***************************************	***********	************	********
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1 .	2.6 1	29	108.5	32.8	50.4 1	38.2	35.9	104.1	53.3	48.7	37.8	0.0
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A3.5.3 Results of Discherge Estimation

Estimated Daily Discharge at Xe Katam Intake Site (1980-1990)

* PA * III *					## ## ### ############################							
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·	cc	H .	88.0	0.79	0.74 1	14.14	13.72	16.22	17.41	18.10	58.6	6.17
u m	47. E	. 4	0.87	0 00	- * K	17.11	15.25	14.00	17.56		76.0	41.4
4	3.07	M	0.87	0.78	0.73	9.45	14.70	12.64	22.82	14.67	94-	4.07
	3.01	w	0.87	0.78	0.73	B.96	14.78	12.07	21.45	14.03	8.61	4.04
-0	÷ - 56.62	1.26	1 98.0	0.78	0.73	a7-a			1 67 61	14.04		- 10.4
, ~	2.89	i N	2 98.0	7.70	0.73	10.10	14:24	11.43	17.71	12.37	7.98	3.97
. 00	2.83		0.85	0.77	0.73	13.17	12.70	10.94	16.43	12.88	7.69	3.94
o.	1 2.77	1.11	0.85	0.77	0.73 (10.96 1	12.05	10.40	15.76	13.29	7.40	3.91
	2.71	1.06	0.85	0.77	1.88	13.04	11,49	10.13	15.05	12.37	7.13 [3.87
	2.65	1.01	. 46.0	0.77	0.73	12.95	10.94			14.68	6.87	3.84
, 01		0.96	- 48.0	0.77	4 32	10.94	10.42	10,32	14.27	12.97	6.62	3.78
	2.54	0.92	0.84	0.76	8.19	9.98	M6.6	10.72	14.36	12.15	6.36	3.73
4,	2.48	0.92	0.83	0.76	4.97	9.48	9.46	9.70	13,51	11.74	6.15	3.68
1,5	2.43	0.92	0.83 1	0.76	5.43	14.18	9.02	11.60	14.93	11.37	5.93	3.62
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ού 1 +1	2.26	- 26.0	- 28.0	27.0	15.16	11.53	04.0	10.58	20.40	10.28	5.45	19.5
0.	2.21	0.92	0.82	0.75	40.36	10.85	8.89	11.48	18.63	10.12	5.26	3.41
20	2.15	0.92	0.81	0.75	17.63	10.39	9.67	10.35	16.96	9.80	5.10 1	3,36
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54		1 06.0	0.81	0.75	11.65	12.82	19.78	12.22	17.58	13.45	1 74.4	3.15
52	1.88	1 06.0	0.81	0.75	10.97	11.38	17.30	42.26	16.17	11.38	4.35 1	3.10 !
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1 60 100 100 100 100 100 100 100 100 100 1	500	0.84	0.80	0.74	10.93	18.58	14.64	17.85	15.67	11.41	4.26	2.95
5	1.67	0.89	0.80	0.74	9.57	17.36	15.90	20.94	18.64	11.50	4.23	2.90
9	-	••	0.79	0.74	8.96	15.09	14.44	20.32	20.34	10.89	4.20	2.85
₩ ∓4	1.57	- -	0.79		8.47		13.63	19.09	- -	10.24		2.80
FOTAL	+======+ 74.01	11 143	25.76 1	23.70	331,89	378.64	452,16	449.00.	524.33	377.84	+======+==+==+==+==+===+====+=========	109.75
FRAGE	+=====================================	# ÷		+ 24.0 0.79	**************************************	12.62	14.59	14.32	17.48 I	12.19	++++++++++++++++++++++++++++++++++++++	3.54 L
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2 -	2.70 1	1.25	2.63	0.87	3.12	14.21	22.04 1	20.32	23.36	17.72 1	9.33	5.18
 ₽0	2.64	1.20	2.46	4.02	6.26	12,14	22.90	24.78	21.54	17.80 1	9.07	5.14
- *	2.59	1.16	2.26	5.33	4.68	10.71 1	48.25	34.61	20.61	17.16	9.62	5.09
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 o t	00.0	/0-1	90.1		57.0	12.77	26.65	05.82	18.95	9.00	8.72	10.0
	2.45	20.1	101	00.8	60.6	15.65	24.10	99.09	18.19	14.93	8.47	4.96
 	2.40	96.0	1.39	2.85	6.87	17.84	23.62	82.42	21.06	20.65	8.22	4.92
 	2.35	0.93	1.38	2.66	90.9	17.89	27.86	51.21	19.43	18:11	7.97	4.86
- or -	2.30	68.0	1.43	2.46	5.56	34.69	24.81	33.74 1	21.87	17.10	7.73	4.80
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6	1.87	0.82	2.07	1.79	19.90	29.12	20.85	35.91	18.00	14.73	26.5	4.24
00	64	0.82	7.84	8	14.42	29.41	27.13	0 44	70.74	0. 7.	C C	α. 7
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21	1.78	0.82	1.63.1	1.79	16.69	42.40	26.03	30.05	14.41	13.69	5.65	4.12
755	1.73	0.81	1.42	1.70	30.41	34.57	24.88	28.17	13.91	13.19	5.60	4.05
. 23	1.68	0.81	1.22	1.58	17.32	30.89	23.79	26.57	14.11	12.71	5.56	4.00
24	1.64 1	0.81 1	1.03	1.45	14.86	27.77	23.06	25.43	13.40	12.25	5.52	3.95
1 25 1	1.59 1	0.80	0.85	1.87	15.25	25.40	21.08	26.84	12,95	11.81	5.48	3.89
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1 26 1	1.55	4.78	0.85	1.59	15.67	24.14	19.95	26.50	12.51	11.40	5.43	3.83
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28	1.46 1		0.86	1.52	12.76	22.56	19.17	24.49	13.85	10.61	5.35	3.71
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30	1.38	-	0.87	5.55	11.49	20.37	17.68	24.17	12.63	- 06.6	5.26	3.60
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	3.09	1.48	0.34	0.87	- 68.0	2.02	19.65	21,33	30.77	13.79	11.62	10.03
	3.04	1.43	1.08	0.87	0.86	2.87	17.66	21.21	28.00	13.25	9.54	9.63
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٠. ص	2.55	1 66.0	0.93	4.70	0.86	2.67	24.14	22.09	20.18	9.80	14.07	6.74
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AVERAGE 1	2.67)) }} **	0.93 I	1.42	1.84	0.29 ~ C.29 ~ ~	22.96	22.50	24.71	11.63.1	11.39	7.98
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- WUM	1.89	6.0	0.89	0.86	0.85	2.02	12.62	19.12	15.31	7.80 1	6.51	4.65
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- 5	1.58	2.98	1.45	0.94	2.83	7.24	18.06	20.85	17.01	27.82	12.23	5.05
- n	4.54	2.92	1.40	1 76.0	29.75	6.86	17.17	17.56 1	17.12	28.08	11.77	5.05
- -	4.51	2.87	 M	76.0	69.6	6.50	16.32	18.19	15.54 (26.24	11.46	4.98
 	4.47	2.81	- OR - E	0.94	14.80	6.15	15.52	15.82	14.85	24.12	11,08	4.94
	7 77	, , , , , , , , , , , , , , , , , , ,	20	, FO	1 17 11	·	00.4	4.45		20.87	7 07 01	4.40
- - -	4.4	2.70	1.20	86.0	22.41	7.99	15.03	13.40	21,71	22.65	10.31	4.86
-	4.37	2.64	1,15	5.93	13.62	13.51	14.11	14.39	19.08	23.23	76.6	4.83
6	4.33	2,58	1.11	1 26.0	13.30	10.56	13.51	12.99	17.24	22.57	9.58	4.79
1 07	4.30	2.53	1.06	0.92	43.05	8.62	12.89	12.49	16:19 1	20.55	9.24	4.75
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1	4.25	2.47	1.05	0.92	17.91 1	7.96	12.29	12.96	15.51	19.67	8,91	4.71
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74	1.07	2.32	0.98	0.91	12.55	18.86	10.81	18.19	13.60	21.92	8,14 1	4.59
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16	3,95	2.30	0.98	06.0	16.05	13.72	11.18	19,35	12.47	18,55	7.63	67.7
177	9.89	2.14	0.98	0,90	13.53	12.46	13.69	19.38	11.96	17.76	7,38	4.43
18	3.83	2.08	0.98	~ 06.0	11.84	11.87	17.70	17.78	11.57	18.44	7,14	4.37
1 19	3.77	2.03	₹ 86.0	0.90	11.22	11.28	14.94	16,61	11.28	16.95	6,91	4.31
- 50	3.72	1.98	0.98	68.0	10.62	10.72	18.36	15.33	13.36	19.75	6.48	4.25
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- 51	3.66 ~		0.98	0.89	10.03	10.18	19.54	15.30	11.82	17.67	6.47	4.20
22	3.60	1.87	0.98	0.89	9.48	99.6	18.07	14.75	11.23	16.38	6.27	4.14
73	M.554	1.82	0.97	0.89	8.94	9.37	16.05	22.50	10.87	15.73	6.11	4.08
1 24 1	3.48	1.76	0.97	0.89	8.43	10.64	14.66	21.93	14.19	15.08	. 90.N	4.02
1 25 1	3.43	1.71	0.97	4.24	7.95	95.85	13.98	21.28	18.63	15.75	5,79	3.96
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9 2 2	5.57	1.65	0.96	62.23	7.50	101.65	13,53	19.89	17.88	16.36	2,62	3.91
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- 2	2.97	1.46	96.0	1.80	2.81	8,23	13,19	24.74	38.78	22.27	17.72	7.00
13	2.92	1.42	1 86.0	1.65	2.58	5.29	11.64 1	23.10	41.83	29.40	21.71	6.78
14	2.87	1.88	0.97	1.49	2.44	5.66	10.92	50,04	38.80	27.17 1	23.57	6.57
5	2.81	1.36	13.57	1.32	2.30	5.33	10.41	48.76 1	36.22	27.58 1	20.32	6.37
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~	2.76	1.33	7.42	1.16	2.15	5.54	9.93	57.35	33.96	26.38	18:10	6.31
17	2.71	1,31	- 52	1.01	2.01	5,35	11.85 (1 62.06	32.05	24.84	16.91	6.26
18	2.66 1	1.27	3.09	1 76.0	1.86	6.50	13.48	80.74 1	30.12	23.27	16.27	6.21
19	2.60	1.23	2.88	0.98	1.73	8.79	11.65	37.27	31.55 (21.93	15.64	6-16
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27	2.50 1	1.15 1	2.39	66.0	1.46	11.20	10.02	36.37	28.87	20.26	14.48	90.9
	2.45		2.16	66.0	1.37	17.30	9.57	33.01	27.27	19.44	18.93	10.9
23	2.40	-1	1.93 1	7 00.1	3.68	14.94	9.25	30.34	26.68	18.67	13.41	5.96
54	2.36	1.09	1.71	2.66	3.78	13,41	8.85	28.88 1	27.08 1	17.93	12.92	5.91
 St.	2.31 1	•	1.48	1.17	2.90	18.92	8.46	27.32	25.07	17.23	12.45	5.86
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58	2.27	1.02	1.28	1.12	3.43	15.64	80.8	34.93	23.87	16.55	12.00	5.81
27	2.22	0.99	1.08	1.04	- 88. 8.	17.75	7.78	31.78	22.86	15.93	11.57	5.76
 83 83	2.17	66.0	0.96	7.03	3.29	18.64	8.72	29.52	21.98	15.33	11.16	5.71
- 0.0	2,13	66.0	0.96	2.22	3.14	21.34	9.46	31.08	21.58	14.77	10.78	5.64
30	2.08		96.0	2.11	2.98	18.87	30.63	42.83	21.26	18.82	10.41	5.57
33	2.03	-	1 96.0	-	63 63	_	34.74	36.82	~	16.56	-	64.6
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•	5.06	- of t	3.01	9.76	9.62	6.45	19.98	18.69	29.93	12.24	8.14	- 56°S
- 2	1 66.4	3.04	2.86	9.17	9.18	6.19	26.16	19.62	27.27	11.83	3.08	5.67
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E .	4.56	2.71	15	2.49	11.73	12.24	27.13	37.82	28.87	12.31	20.6	5.19
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91	4.36	2.64	20	8.57	11.20	15.33	31.03	28.85	24.63	15.88	8.62	5.07
17	1 05.4	2,60	2.31	6.92	13.58	64.68	27.94	26.84	24.86	14.92	95.38	5.04 1
ω : τι :	4.23	2.56	2.30	6.51	11.54	45.82	27.99	24.97	24.49	15.48	9.14	n.00.
- 16	4.16	2.51	2.29	9.82	13.02	82.83	26.21	24.81	22.68	13.68	7.90	4.96
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27	4.03	4.73	2.26	9.34	13.14	29.95	23.75	23.80	21.25	12.47	7.44	4.89
22	3.96	4.11	2.25	9.11	11.78 :	28.69	23.54	27.57	20.46	12.06	7.22	
23	3.90	6.92	2.20	8.19	11.21	26.33	54.96	26.93	19.65	11.67	7.00 1	4.81
1 24 1	м В М	5,15	2.16	12.27	10.52	24.92	22.66	26.15	18.86	11.28	6.80	4.78 1
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. 26	3.71	3.57	2.08	16.56	10.36	21.81	20.29	25.59 1	17.42	10.59 8	6.41	4.69
1 27.	3.65	٠	2.04	14.15	9.80	23,85	19.54	27.43 1	16.75	10.26	6.22	4.63 1
1 28 1	3.60		2.00	17.32	9.37	22.54	20.41	38.82	16.11	11.49	7.86	4.57
- 52	3.5è	-	1.96	15.24	8,95	23.36	19.75	29.13	15.51	10,35	6.64	4.51
30 -	3.51	-	1.94	13.05	8.55	21.61	18.30	26.91	14.94	10.06	6.50	4.45
- th	3.45	-	1.91		8.17		18.81	24.96 1		9.77		4.40
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2.4.6 1.0.8 0.98 2.48 6.13 4.78 17.92 20.65 9.35 5.61 2.40 1.00 0.97 0.09 2.44 1.653 4.76 17.92 20.65 9.35 2.62 2.31 1.06 0.97 0.99 2.31 1.66 0.05 1.66 0.97 0.99 2.31 1.67 2.67 1.06 0.97 0.99 2.31 1.67 2.62 1.76 1.66 1.67 2.64 9.10 2.64 9.29 1.60 9.20 1.60 9.20 1.60 9.20 1.60 9.20 1.60 9.20 1.60 9.20 1.60 9.20 1.60 9.20 1.60 9.20 1.60 9.20 1.60 9.20 1.60 9.20 1.60 9.20 1.60 9.20 1.60 9.20 1.60 9.20 1.60 9.20 1.60 9.20 1.60 9.20 1.60 1.60 1.60 1.60 1.60 <t< td=""><td> co</td><td>2.50 1</td><td>1.12</td><td>6.0</td><td>_ •</td><td>0.89</td><td>2.38</td><td>6.41</td><td>4.65</td><td>17.15</td><td>18.97</td><td>9.73</td><td>5.79</td><td>3.60</td></t<>	 co	2.50 1	1.12	6.0	_ •	0.89	2.38	6.41	4.65	17.15	18.97	9.73	5.79	3.60
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1.56 0.92 6.61 6.27 4.41 15.23 17.94 11.77 6.69 3.85 1.42 1.52	1 28 1	1.61	1.01	6 0		6.59	7.43	4.62	15.17	17.53	12.25	6.89	3.87	2.70
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66.28 31.12 29.61 51.30 114.64 238.76 380.17 611.18 499.85 272.82 145.35 10 2.14 1.11 0.94 1.71 3.70 7.96 12.26 19.72 16.66 8.80 4.85 1.15 1.	- + the second of the second o	- + :: :: :: :: :: :: ::	gi Fi	11 11 11 11 11 11 11	- # - # - #	- + 8 8 8 8 8	- + U	- 4 0 0 0 0	- 	- + HU 17 17 18 18 19 19 19 19 19 19 19 19 19 19 19 19 19		- + IIII	· + 11	# # # # # # # # # # # # # # # # # # #
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		2-14	! ! ? !	6.0		1.71	3,70	7.96	12.26	19.72	16.66	08.80	4.85	3.24
	-	2.83	; !	1.0	 !	6.61	13.79 f	16.63	79.40	63.05 1	23.80	12.55	7.52	8.79
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	u II	11 11 11 11	И	111111111111111111111111111111111111111	11 + 11 H	11年11日11日11日11日11日11日1日1日1日1日1日1日1日1日1日	141111111111111111111111111111111111111	- 新国日世出出自由十二	***********	**************	***********	+ 4 11 11 11 11 11 11 11 11 11 11 11 11 1	1) 中性自从日本籍自日日	1,62004480

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A3.6 Comment from Meteorology and Hydorology Depertment, MAF of Lao on Hydrological Analyses in the Study

	Some observations in the 3 Motorology and Hydrology. of the interior Report July 91
	of the interior Enjoy of
(1)	Precipitation estimation areal rainfall from point measurement
<u> </u>	Temperature: in addition, Temperature at ATTAPU compare
	with 2 periods 1700-1900 and 1990.
	Potential Evapotrauspination (Penman) in 1990 ATT.
	at lake 1983 monthly moun temp were completed in table 3.3 /2
	Exoptionnal temp were observed during 82/83
	ENSO event.
	Temperature regimes: Ob- AT Pokse (Th) and (TT) at Pakong
4)	Evaporation, Evapotranspiration Potential PET at Pakse
	1560mm/year and at Paksong 1192 mm/year
(5)	'PET' at Xiengkhoung Plan of James computed with 4
	methods (15 compare). Pick reading are not
. 6	methods (Lo Compare). Pich reading are not use. Assessment of Length of the main wet Period at NikHon 34
	and AT PAICONG TOWN, (rolation between Pand PET)
7	AT PAKSE in the plain The LIVP is shorter than in the Platam
	and the availability of moisture is also smaller.
8	Summarizing table of the Eurface wind in 1986 int
	NikHom 34 and at PAKSong (more clear for
	utilizating for general purpose)
<u> </u>	Surface wind rose. 1986 in Dec the NE prevailling wind
- · · · · · · · · · · · · · · · · · · ·	direction and mean valocity of about 7 a m/s produced late
	rainfall for the dry ceasion -
ga_	and 96 wind rose for the period of 1983-87 AT PAKSong
<u> </u>	and 1961-1980 ATPAKSe respectively.
(0)	Remoss calculation from Water Balance 1986 data at NK34
(1)	calculations of peak Flood Discharges at Project sites
	(for information only)
	Vientian 22 July 91
No.	D. Com

Precipitations Some Observations on the estimation of the areal rainfall from roints rainfall. Ingeneral, the awarage areal rainfall injuts to the area (cake ment) are normally estimated from observation made of number of gauging stations over the basin by using one or sever al methods of computation from example for floor forecast purpose, the Standard error in areal average rainfall is given as e=Cv/VT, where Cv=[(x-p)2dA/Ap]= and x= the point rainfall for a given duration, P- the areal rainfall for the same duration, p - the average areal rainfall of that duration, and A = the area with or gauging stations distributed at random (see Relative errors of raing auge net works after Ishizaki 1979) who the minimum of sumber of Stations no 10 100 Another Cogical appro ach to design of forecast rainfauge network after Sugarana (1981) by using a statistical evaluation of spatial and temporal distribution of sampled showers in temperaternal tropical climates : ex: in temperate zones fine saing ough are sufficient (Japan) to provide a sepresenta two sample of the rainfall for forecast purposes, but in tropical zonces this number is 15 (3 times ande being independent of the size of the leaving In case of the Bolaven Plateau specially in the project area only one or two stations (not exactly located in the Catchment area Although a trend of decrease in rainfall amount is seen from the west to the east (Paksong km 42 to Nikhom 34), incentainties still wist in the areas suprounding the Peaks of 1500m and 1700m altitude, located in the ridge of the limit of the catchment where the exposure to the South west air flow will receive more rainfall than Hot observed 1200 m

Golution: From Fogel Duckstein model: (Small mountain
nous watershed data leased on 15 ingle raingange
mainly on the variation of rainfall with allitude he by using
regression equation of daily rainfall with alletide
The study by Fogel and Duckstein of thunderstorms
in the Tueson area (ARIZONA) shows rainfall amounts
at storm conters to be distributed as Type I extremal (Gumbo)
and thunders torm rainfall given there is precipitation some
where on the watershed from a thunderstarm, to be distributed
exponentially. Example in the Santa catalina Mountains
adjacent to Tucson gives the following regression equation
of daily rainfall with allitude h (feet)
R(h)=0.065675+0.00001791(-h-2500)
Assuming that the difference in rainfall with allitude
is due to a difference frequency of storms and rosuming
unit likelihood of storm at 2500 feet (760 m) the
relative likelihood of a Storm at altitude h is
L(h) = [1+0.0002727 (h-2500)]
This equation is obtained by dividing RCh) above by
0.065675; the relative likelihood of a storm at 3500 ft
is 1.27. This means a starm at 3500 feet (1067.11) is
1.27 times as likely as a storm at 2500 feet. The diffe
rent height h 307 m.
In the Bolaven Plateau with a different of 30 ym
(5000 feet - 3936 feet = 1064 feet on 324 my 15 it /www.hr
or why not to consider the relative likelihood of storm,
of 1.27 at 1500 mallitude
example, MAXIDAILY Rainfall 29/August 90 from Typ-
how Becky
At Paksoly 42 = 307.5 mm
Pak Souf Town - 20241
Mikhorn 34 = 7-514 at 1700m = 150 mm at least
Mikhon 34 = 7-514 at 1700m = 150mm

Temperature at ATTAPU STATION (14°48 N/106°50'E,
EL:106m)
AVERAGE FROM 1900_1910
Jan Feb Mar Apr May Jun Jly Acio Sold Oct Nov Dec ANN
mean te 246 256 286 295 283 272 268 269 267 271 255 248 268
mean Maxi 327 332 359 358 335 312 305 307 306 31.9 31.2 308
mean Mini 169 179 214 23.1 235 232 228 230 227 219 19.8 18.3
Absolute
Axi 39.6 38.8 41.3 40.4 412.35.9 34.8 34.8 367.37.8 350 38.7
Mini 98 105 145 185 202 445 200 198 200 182 137 105
1990
mean t°c 262 27.7 294 306 288 275 27.5 27.4 280 274 25.9 245 27.6
mean May: 34.1 349 363 360 342 310 316 315 324 326 317 322
mean Mini 18.3 205 225 244 243 240 234 233 236 223 201 168
MAXI 365 370 380 400 390 337 335 350 348 358 347 348
1ini 14.8 155 205 220 221 228. 210 212 215 19.6 16.6 14.1
Note: Due to the global climatic change (environment chan all Kind of air temperature precorded in 1990 31) was higher than that observed from 1900-17103.
all Kind of air temperature precorded in 1990
was higher than that observed from 1900-1710),
(2) Potential Evapotranspiration (PET from lenmans)
1990 monthly mean and annual total
Tay Fel han Abo May Ter TR. Art Sout not Now ANALINA
Jan Feb Har Apr May Jun Jly AUG Sept Oct Nov Dec ANNUAL PET (mm) 117.8 1374 1450 1557 140.7 123.0 131.4 130.8 124.2 123.6 1050 106.6 1541.2
Piche 890,mm

TEMPERATURE REGIMES (FAO 1986)

The Tropical Temperature regime.

The mean minimum lemperature of the coldest month varies between 13°c and 18°c, while the mean minimum to of the warment month is higher than 20°c

(Th)

Hot Propical

40:

38. 36:

34.

32. 30.

28.

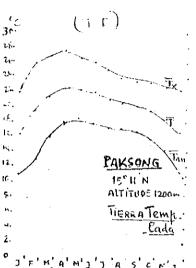
26 24.

22. 20.

18 16 .

14.

12.



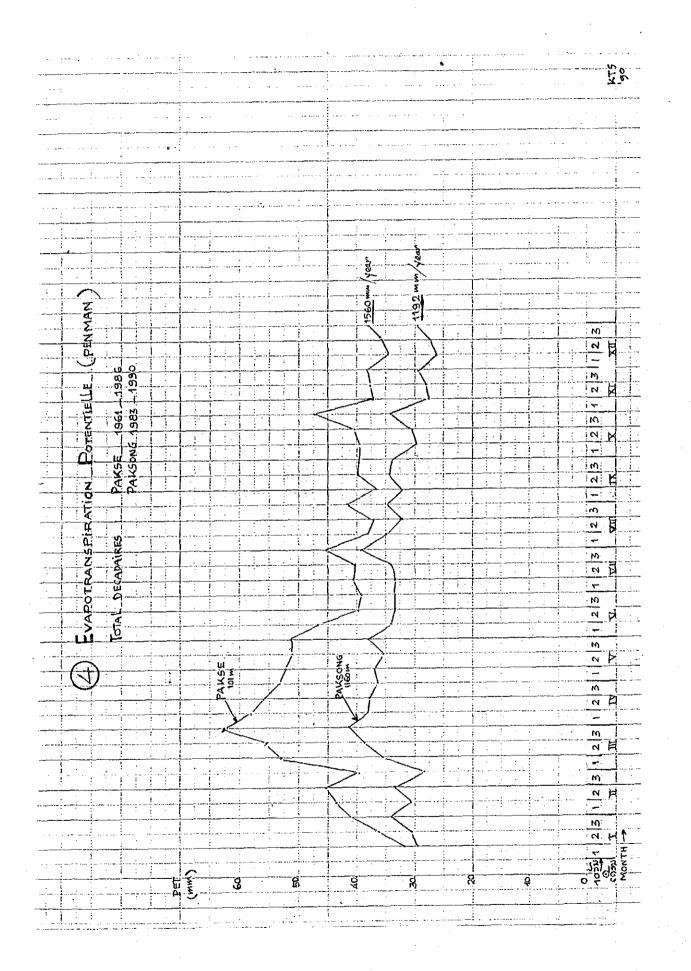
The Tierra Templada regime

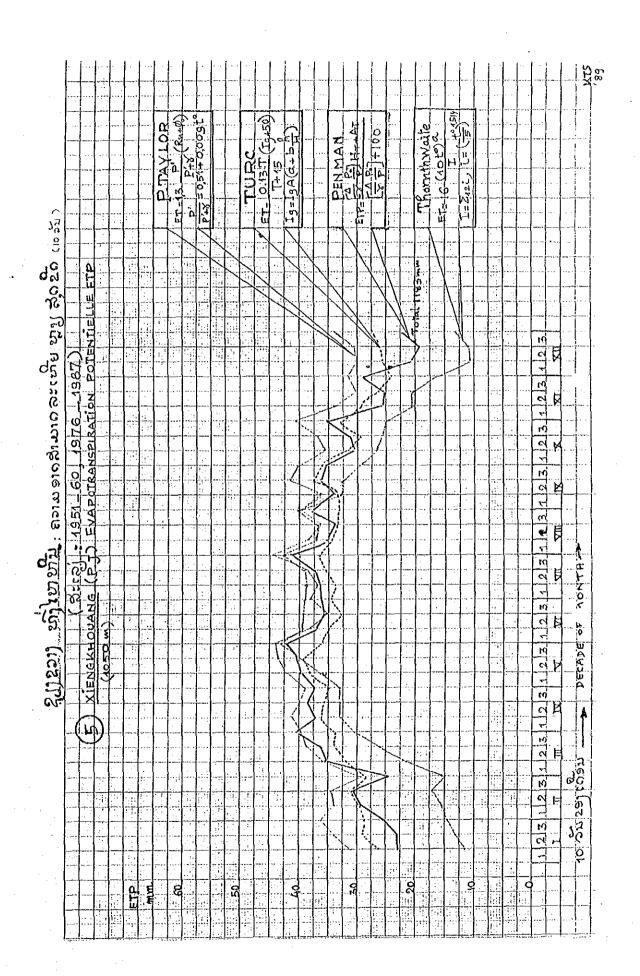
The mean minimum temperature of all months is less than 20°C. while no frost risk occurs. Except tionnaly once in several years. (in the Bolaven Plateau)

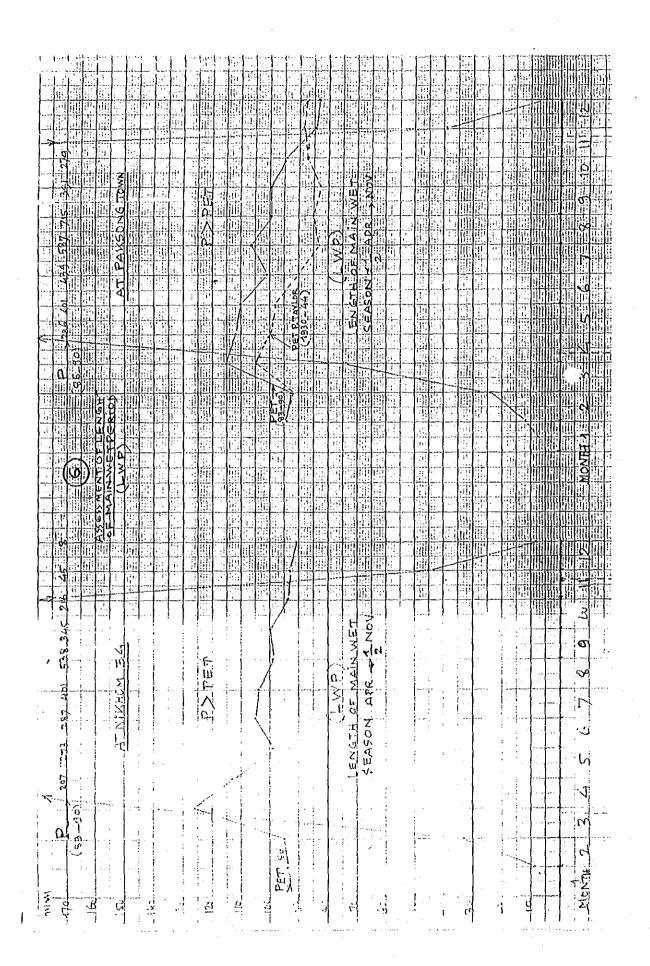
* The Tierra Fria Aegime

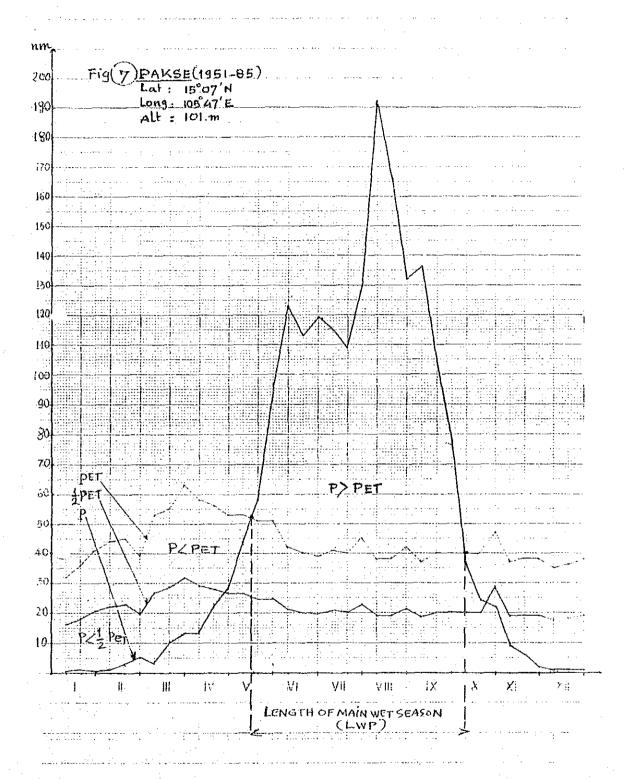
has ingeneral mean minimum to of the coldent months 8213°C a certain frost risk is present. Summero are relatively hot but the mean maxi to of the warment month remains below 33°5C

(in the worth LAOS with All > 1,500 m)







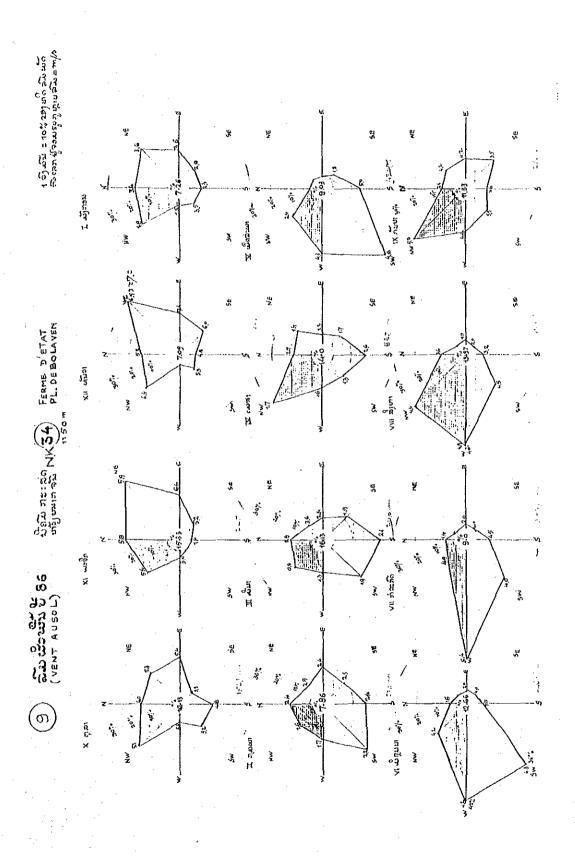


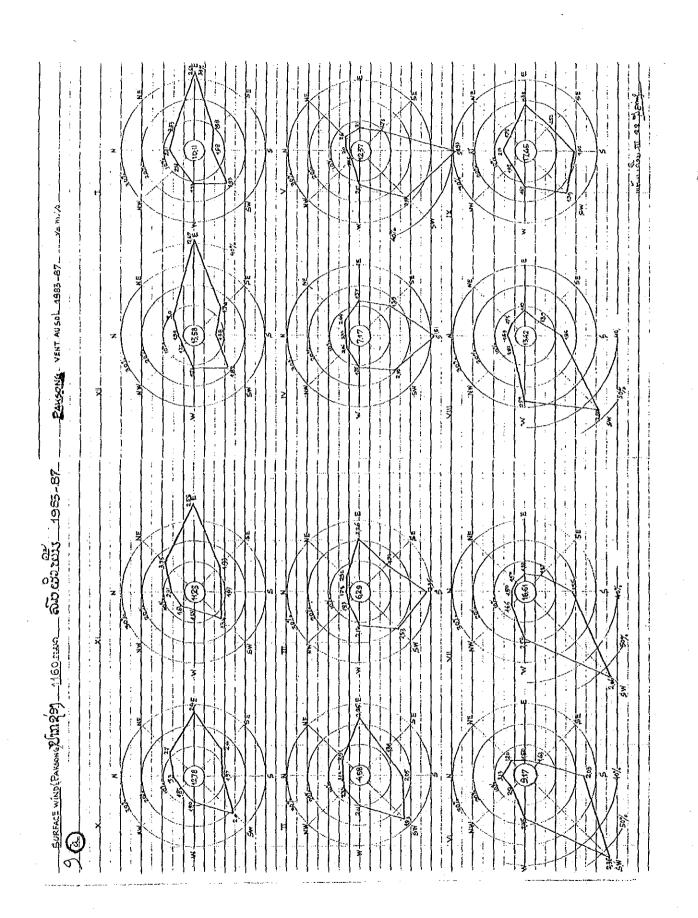
(8) 1986 Surface Wind at NikHoM34 (Bolaven Plateau)

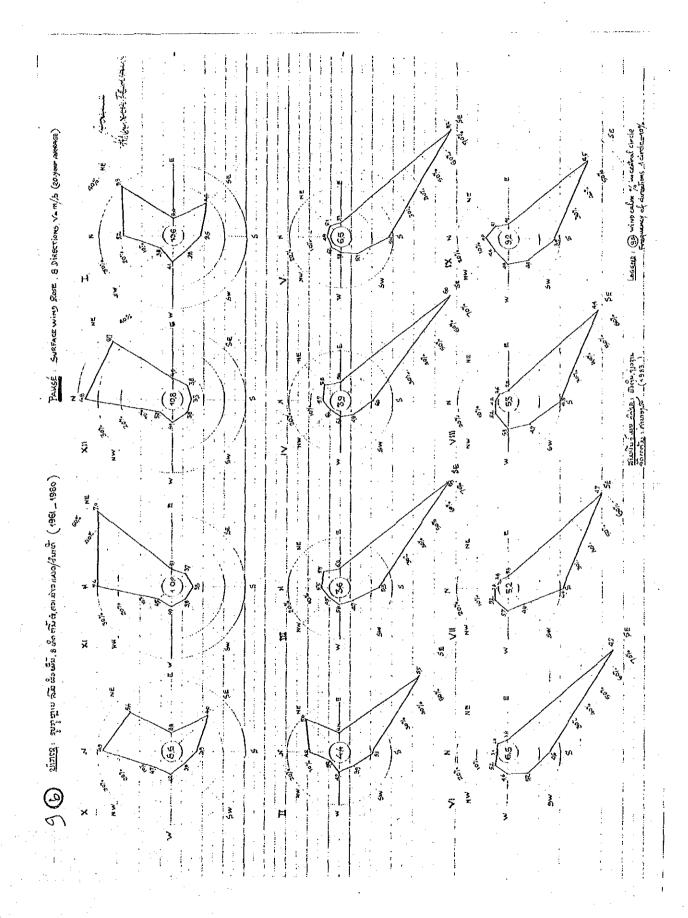
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								11 1.6		
May	9.03	6 00			4 18	14 50	40 4.8	26 43	15 24	
Jun	12,66	3 15		1 20	1 40	3 37	36 43	42 42	8 42	
	9.0	6 14		3 20	1 40	6_45	22 40	53. <u>5.4</u>	8 49	
	13.97	8_34	· · · · · · · · · · · · · · · · · · ·	2 17	1 20	3 3.2	17.45	40.49	31 44	
Sept	11.33	6 24	8 42	10 42	15.35	6 26	10 5,5	16 41	30 5.0	
Det	16.13	15 60	15 53	19.54	5 33	12 4.8	19 52	5 50	25.51	
Nov	1533	22 5.8	35 5.9	16 64	6 52	1 1.0		4_30	18 54	-
De	e 7.09	14 58	31 69	15 7.4	12 60	3 4.8	5.53		18 64	
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Falu	1.43	290	40	7.25 27	10:15 23	18,12	12	1812	24	33.3 3	19	363	10	5.80	2,1	
Har	839	0.7	20	490.27	1399 33	1259	16	3567	20	1838	31	1049	17			
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thy	6.90	0.7	10		3,45 14	1104	24	3931	24	3104	22	41.73	26	276	10	
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Nov	2,617	480	24	1301 311	<u>3630 24</u>	1439	1.2	6.16	1.0	17.13	14	411	1.3	49	1,2	
Dec	17.42	156	၂့ပ	782 46	26.56 h	75.78	2)	547	20	726 G	2υ	782	1.8	234	13	

o/o = percentage of wind direction (frequency)
wind velocity in m/s (monthly mean value of each direction)







(19) <u>Lun-off calculation from Water balance</u> 1986 data at Nikhom 34. (unit=mm), Q= m³/s

	1			^	L	·								
Paramelon	Jon	Fob.	Mur	-Apv	May	Jun	Jl	Aug	Sent	oct,	No	De	ANN.	-
Pmm	0_	_33	24	67_	431	241	444	650	243	280	. 23.	31	2467.	ļ
PET	94	95	126	_113_	99	105	104	99	100	194	93	91	1213	
(Penman) P-PET	-94	_62	-102	<u>-46</u>	+332	<u>+136</u>	+340	+551	+143	¥186	-70	60	} L_>======	
Raserve	_0_	_0_	0	<u>.ç</u> _	100	100	100	100	100	100	O	, o_	! !	·
Deficiency	-94	62	-102	-46						ļ	-70	6 a		
Surplus					+ 232	+136	+340	+551	+143	+186			· !	1_
Run off.				<u> </u>	116	184	238	4455	347.	164.5	93	46.5	16345	<u>, </u>
		<u>-</u>				·			! 	 			! 	····
Qare A784	9,62	5.76	3,20	4.19	24.13	2594	47.00	.73.0	6354	4159	22.94	1324	28.03	:
(Table 3.9)		: 												<u>:</u>
Mo. 4/ A/V	122)	735	4.08	534	30,78	33.09	5975	9330	_8104	53.04	29.26	16.89	<u>.</u>	. •
Q calculate	ļ	- -			3395	55.00	69.67	130.4	105	48.3	28/13	1371		~ .
at % -		· ·			65_	65	65	65	70	7c	=			
Qat %	<u></u>			<u> </u>	2206	35.45	453	848	73.51	43.5	28/2	1371		.i
	<u> </u>	1						,					<u>.</u>	. 1
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methods -- --

in an assumption and Result, can be compared with the other

(11) calculations of peak Flood Discharges at Projectsites
Two methods are used
(i) the SCS method
0
(2) _ the D. Sakolos Ky method: (I) At YEKATAM Project Damsile (Ses method) Catchment Area: 290 km²
H, the difference elevation between the fartest point and
Dansite, leing, 1700m 450m= 1250m
Longth of the river approx 45 km
L = 45000 mx 3,28 = 147600 feet
\$=0.02777 (11000)
For computation of maximum peak flow from a daily
rainfall of 200 mm reasonnably accepted as Tr= 10 year period.
and giving intensity in 4 hours is 50 mm/h, uniformly
over the catchment (hourly rainfall at Paksong Town for
Tr=10.4 is about 65 mm/h. compated from the period 1966_1970)
Estimation of direct rum off on Pe on the basis of Pe - CP-0.25)2 and the curve number:
Pe - (P-0.25)2
and the curve number:
CN = 60.
The time of concentration To is extrinate from the kingwich formula
for Kinpich's formula: To = 0.0078 L 0.77 5-0.385
for Kirpich's formula: Te = 0.0078 L 5
Tc = 0.0078,9554,2477,3973534 = 296 1202 minutes (5#)
Consider 1h unit hydrograph,
tr=1h; tp=0.67c=0.6.5=(3H)
$T_{p} = \frac{T_{p}}{2} + t_{p} = 0.5 + 3 = 3.5 \text{ f}$
and $t_{b} = 2.67 \times 3.5 = 9.4 \text{ h}$
for A= 290 Km2
$\frac{q_{p} = \frac{2.08.A}{Tp} = \frac{2.08,290}{3.5} = 172 \text{ m}^{3}/\text{s. cm}$
This means that each four of excess nampall, le (and most)
will produce 172 m3/s from this catchment.

The peak discharge Qp, from each hir, Re, increment

Qp,	t =	-9p	r. Pe,	Ł
-----	-----	-----	--------	---

 المنط بخوستين بداد والوال المالي	For Cl	V=60			
			d 50mm/h	in 4 liveurs.	-
 time_	cumul. Ram.	cumulation	Pa(U	Poakdisch]
 -h	1	Remoss Pe.	Ciy ,	Qpm/p	
 0	0		<u> </u>	O	
 <u> </u>	<u> 50</u>	3	0.3	57.G	
 2	100	20	1.7	2924	
 3	150	50	3	516	
 4	200	86	3.6	619.2	
· · · · · ·					

2/ XENAMNDY at BAN LATSASIN.

CA = 537 Km^2 Alliliade = 720 m

H = 1000 = 720 = 280 m

L = 40 Km (40000 = 3,28 = 131200 feet = 0.007TC= 0.0078.131200 = 0.007 = 0.0078 = 8725.8763 = 6.755

= 457.7579 minutes = 7.66 h

for tr=1h, tp=0.6 Tc = 6.6 × 7.66 = 4.576 = 4.6 h

Tp = $= \frac{t}{2}$ + tp=0.5 + 4.6 = 5.1 h, 1b= 2.67,51=13.6 h

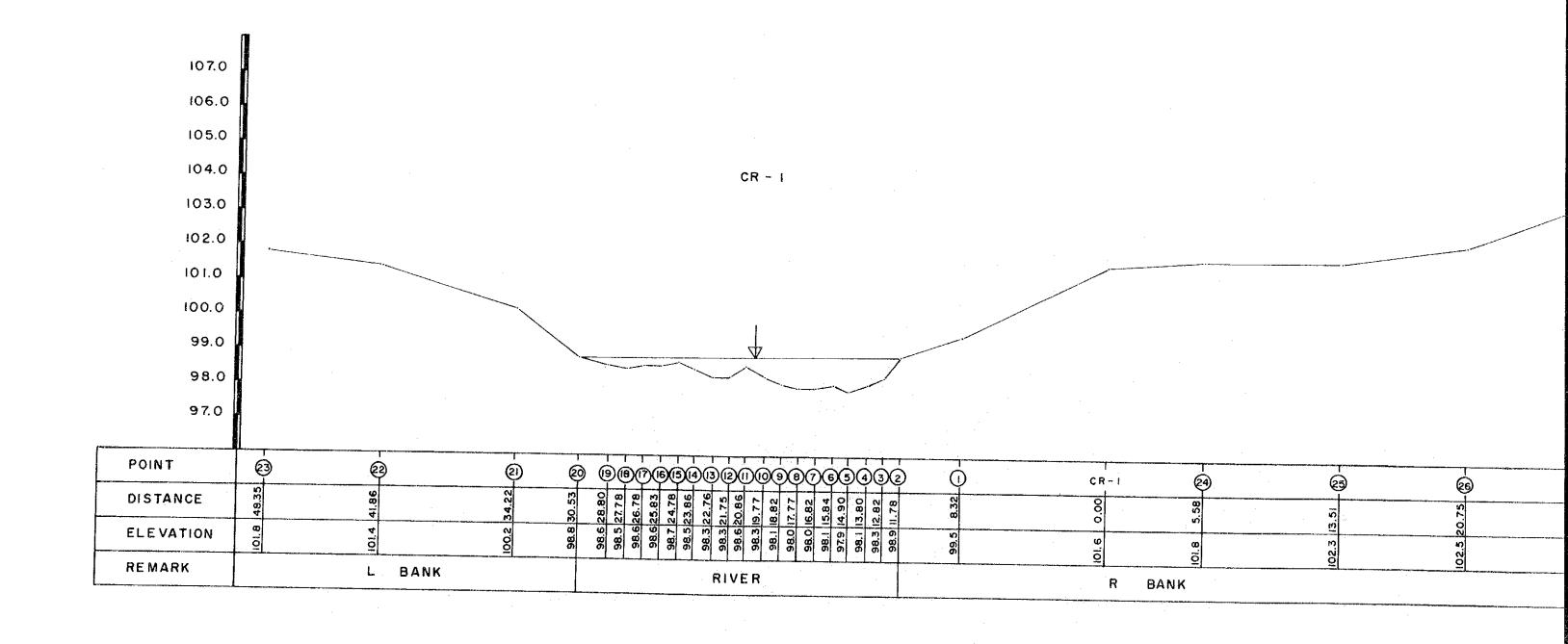
A = 53.7 Km^2 $q_p = \frac{2.08.532}{5.4} = 2.19 m²/s. cm$ for the same CN = 60

Pect)
$$QP$$
0
0
0
0
03
03,219 = 65.7 m3/s
1.7 1.7 ×219 = 372.3 m3/s
3 ×219 = 6,570 m
3.6 ×219 = 788,4 m

· · · · · · · · · · · · · · · · · · ·
Vtian, 22/7/91
Chamthong S
(2) calculation of Qmax from Sakolosky method. Gyo: A Smell tributary of Scolone mean the toe of the Bolaven
6x : A Small tributary of Scalone mean the toe of the Bolaven
plateau with a mean height It = 625m, the catchment area
so F= 203 Um2 no lake of pond in the basin
the calculation of a peak flood discharge Qmax(14,) for
This promple is:
$9200 = 5,4 \frac{m^{3}}{s} / \frac{km^{2}}{m} = 0,63 \text{ and } \lambda = 1, P = 1\%$ $9200 = 5,4 \frac{m^{3}}{s} / \frac{km^{2}}{m} = 0,63 \text{ and } \lambda = 1, P = 1\%$ $9200 = 5,4 \frac{m^{3}}{s} / \frac{km^{2}}{m} = 0,63 \text{ and } \lambda = 1, P = 1\%$ $9200 = 5,4 \frac{m^{3}}{s} / \frac{km^{2}}{m} = 0,63 \text{ and } \lambda = 1, P = 1\%$ $9200 = 5,4 \frac{m^{3}}{s} / \frac{km^{2}}{m^{3}} = 0,63 \text{ and } \lambda = 1, P = 1\%$ $9200 = 5,4 \frac{m^{3}}{s} / \frac{km^{2}}{m} = 0,63 \text{ and } \lambda = 1, P = 1\%$ $9200 = 5,4 \frac{m^{3}}{s} / \frac{km^{2}}{m} = 0,63 \text{ and } \lambda = 1, P = 1\%$ $9200 = 5,4 \frac{m^{3}}{s} / \frac{km^{2}}{m} = 0,63 \text{ and } \lambda = 1, P = 1\%$ $9200 = 5,4 \frac{m^{3}}{s} / \frac{km^{2}}{s} = 5.4 \frac{(200)^{6.63}}{(203)^{6.63}} + 1,203 = 1085 \frac{m^{3}}{s} / \frac{km^{3}}{s} = 1.$ $9200 = 5,4 \frac{m^{3}}{s} / \frac{km^{3}}{s} = 1.$ $9200 = 5,4 \frac{m^{3}}{s} / \frac{km^{3}}{s} = 1.$
Qmay 19 = 5.4 (200)0.63 1, 203 = 1085 m3/s
The formuly is:
$Q_{p} = g_{2ro} \left(\frac{2\sigma_0}{F} \right)^m \lambda \cdot S_{1x} F$
where
to the eatchment area in \$9.600
M: 0.63 for mountainous ragions
9200=54 m3/s/km2 -1
2. coefficient depend upon the size of river basin
S=HT or S=A+Blog N for free: hilation into (60.7)0.33 The 100 year, harvord (P=1%), AanB-, Rights
(60.1) The 100 year, hard (P=1%) AanB-, ligny
For the Same area With F=51km² (upper Sections)
with L= 22,5 km, mean Height of the basin = 400m
amax 1% = 0.28. Hr cd. F
where to = = 22.5 - 2,5 h. 3.6×250
$V_{17} = 0.7 \times 3.57 = 250 \text{ m/s}, V_{max} = 3.57 \text{ m/s} (Seems to be higher)$ $T = t_{17} \cdot \mu = 2.5 \cdot 0.78 = (2H) \cdot \mu = 0.78 (coefficient of radication)$ $H_{7} = S(T.60)^{0.33} = 24.8 (2.0 \times 60)^{0.33} = 120 \text{ mm} (Avocapilahi)$ $A = Q_{40} \text{ max} (Table)$
$T = \Pi - M = 25.078 + (2\%) M = 078 (coefficient of reduction)$
HT = 0 (1.60) - 24,8 (2.0 × 60) - 120 mm (hocipilaly)
L= 035 Pfor 81 (HT 650 mm and F between 10 100 km²
d= 035 for 87 (HT 6150 mm that I begin 10 100 km
f = 0923 cofficient of the hydrograph shape.
Hence Qmax 1/s = 0.28 * 120 * 035 * 51 km² 0923 - 221 m/
(all of these computations are only for information)

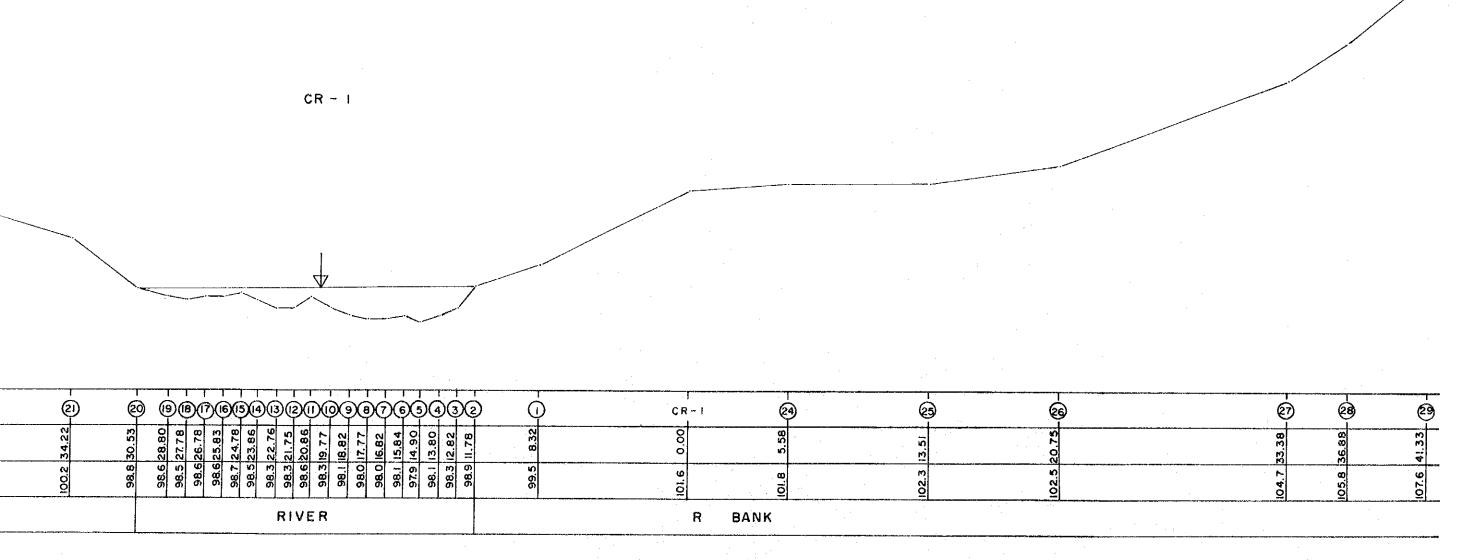
CROSS SECTION OF XE KATAM RIVER AT BAN NONGHIN (1/3)

(10 m upstream from staff gauge)



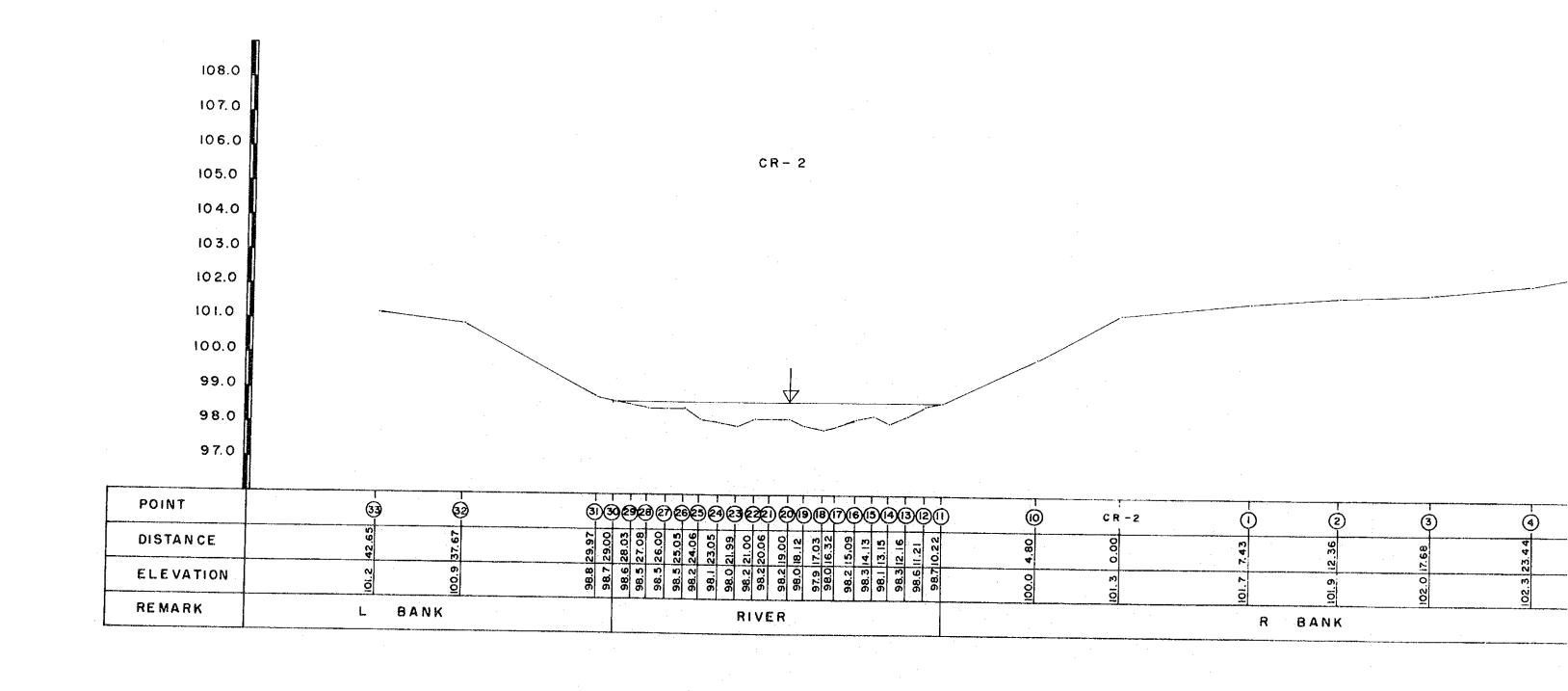
CROSS SECTION OF XE KATAM RIVER AT BAN NONGHIN (1/3)

(10 m upstream from staff gauge)



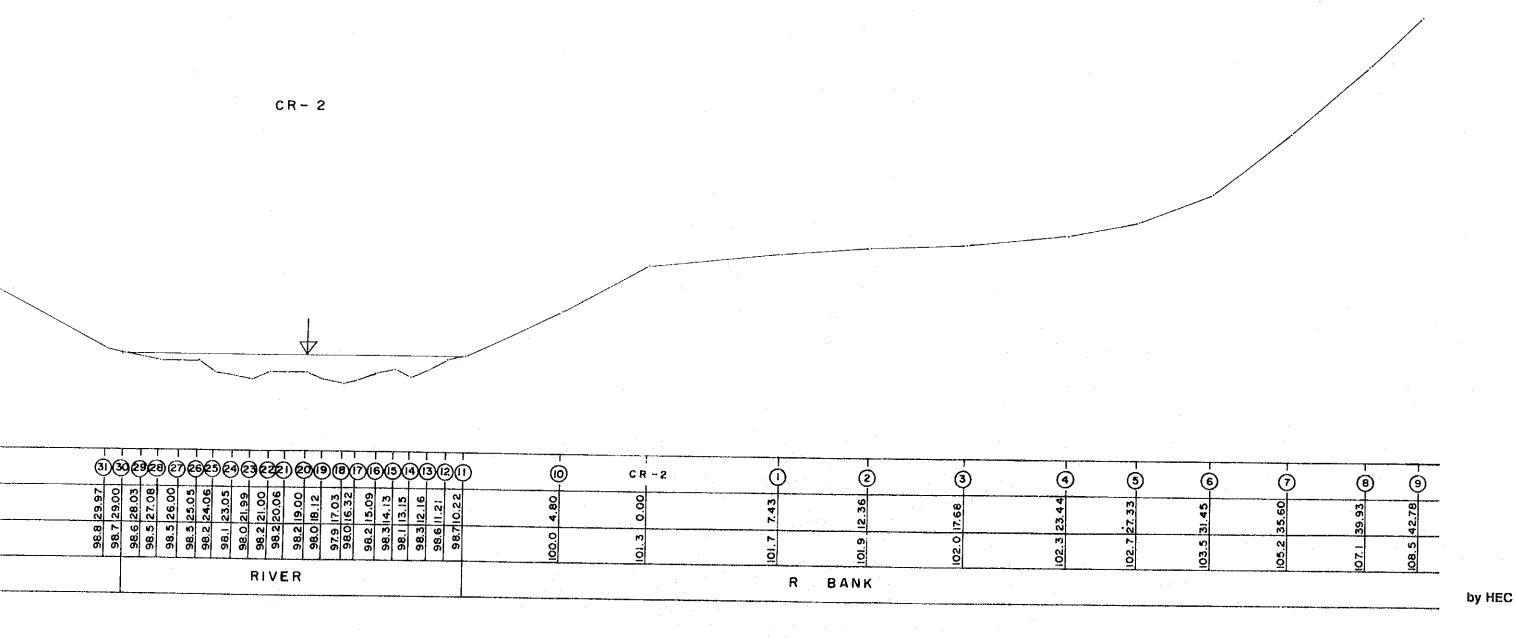
CROSS SECTION OF XE KATAM RIVER AT BAN NONGHIN (2/3)

(at staff gauge)



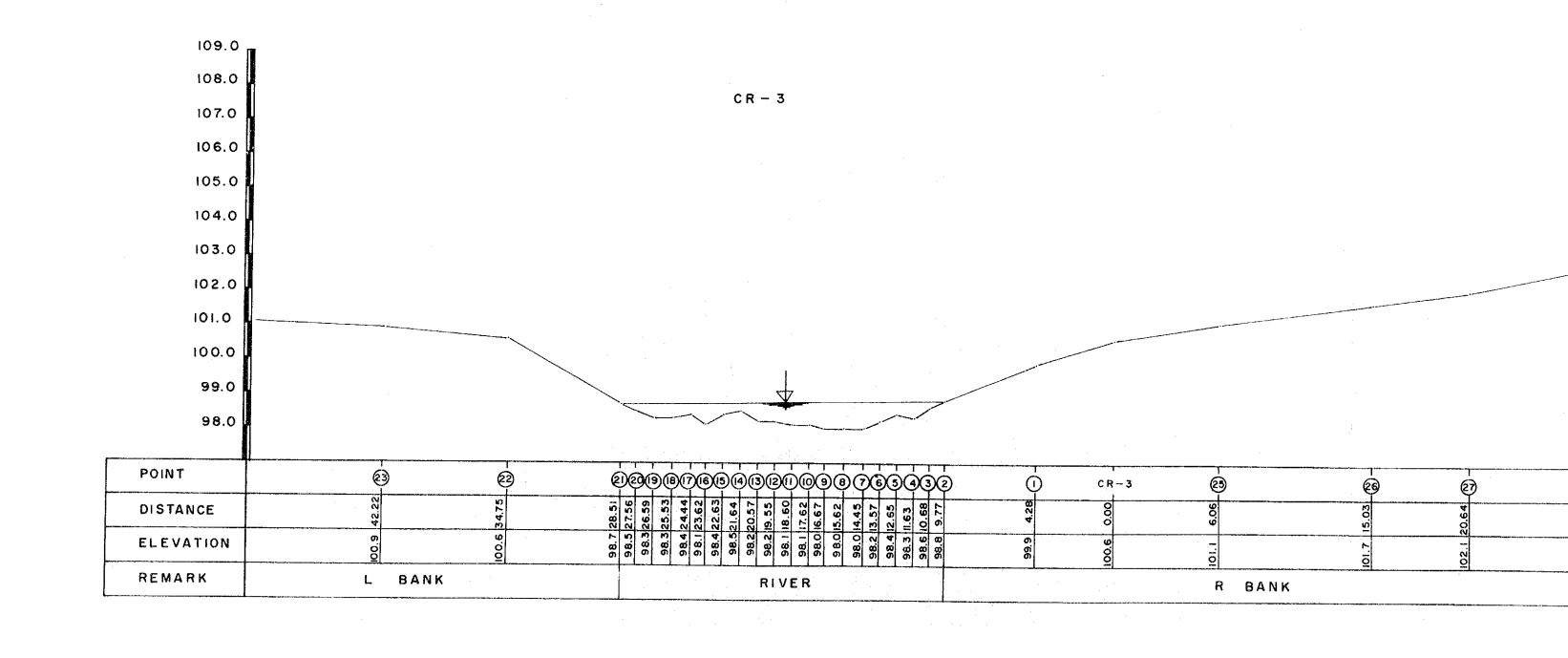
CROSS SECTION OF XE KATAM RIVER AT BAN NONGHIN (2/3)

(at staff gauge)



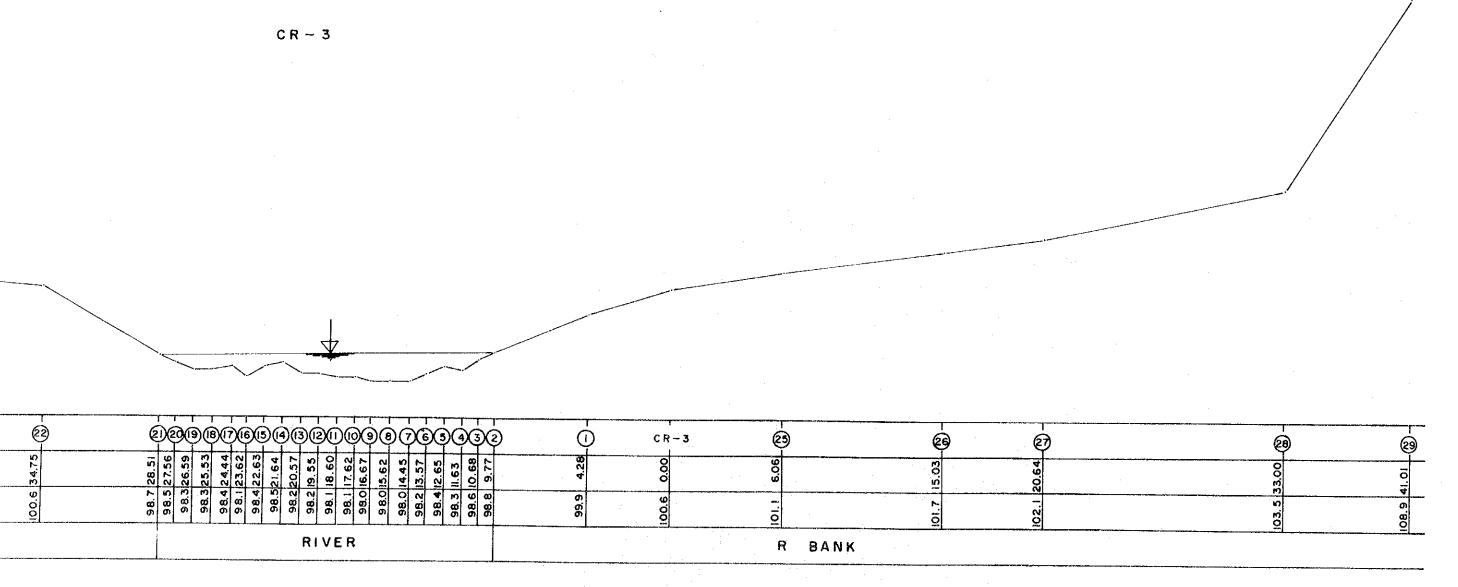
CROSS SECTION OF XE KATAM RIVER AT BAN NONGHIN (3/3)

(10 m downstream from staff gauge)

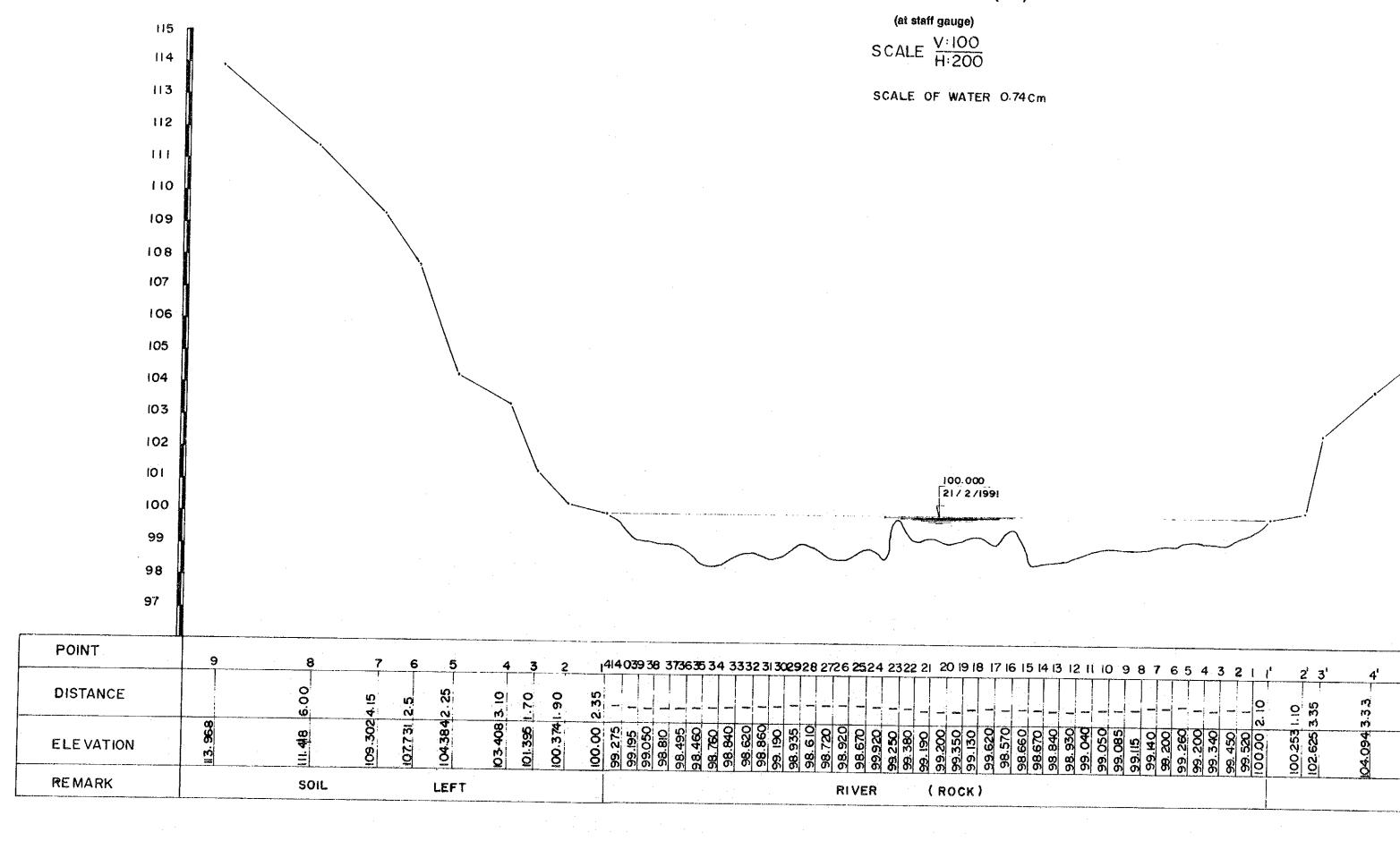


CROSS SECTION OF XE KATAM RIVER AT BAN NONGHIN (3/3)

(10 m downstream from staff gauge)



CROSS SECTION OF XE NAMNOY RIVER AT BAN LATSASIN (2/2)

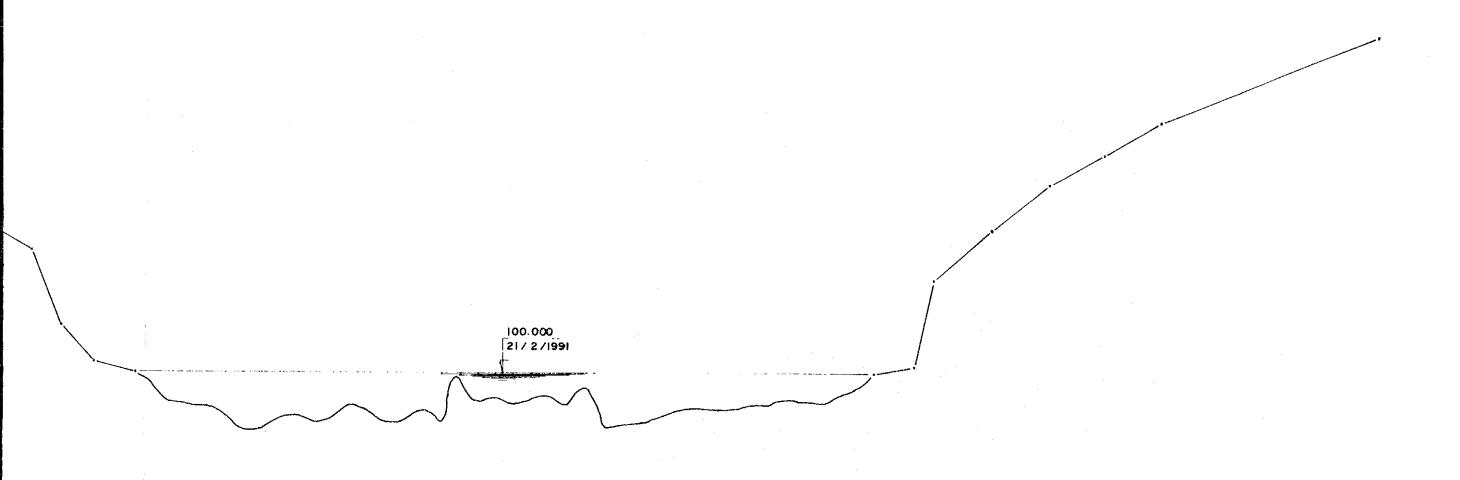


CROSS SECTION OF XE NAMNOY RIVER AT BAN LATSASIN (2/2)

(at staff gauge)

SCALE V: 100 H: 200

SCALE OF WATER 0.74Cm



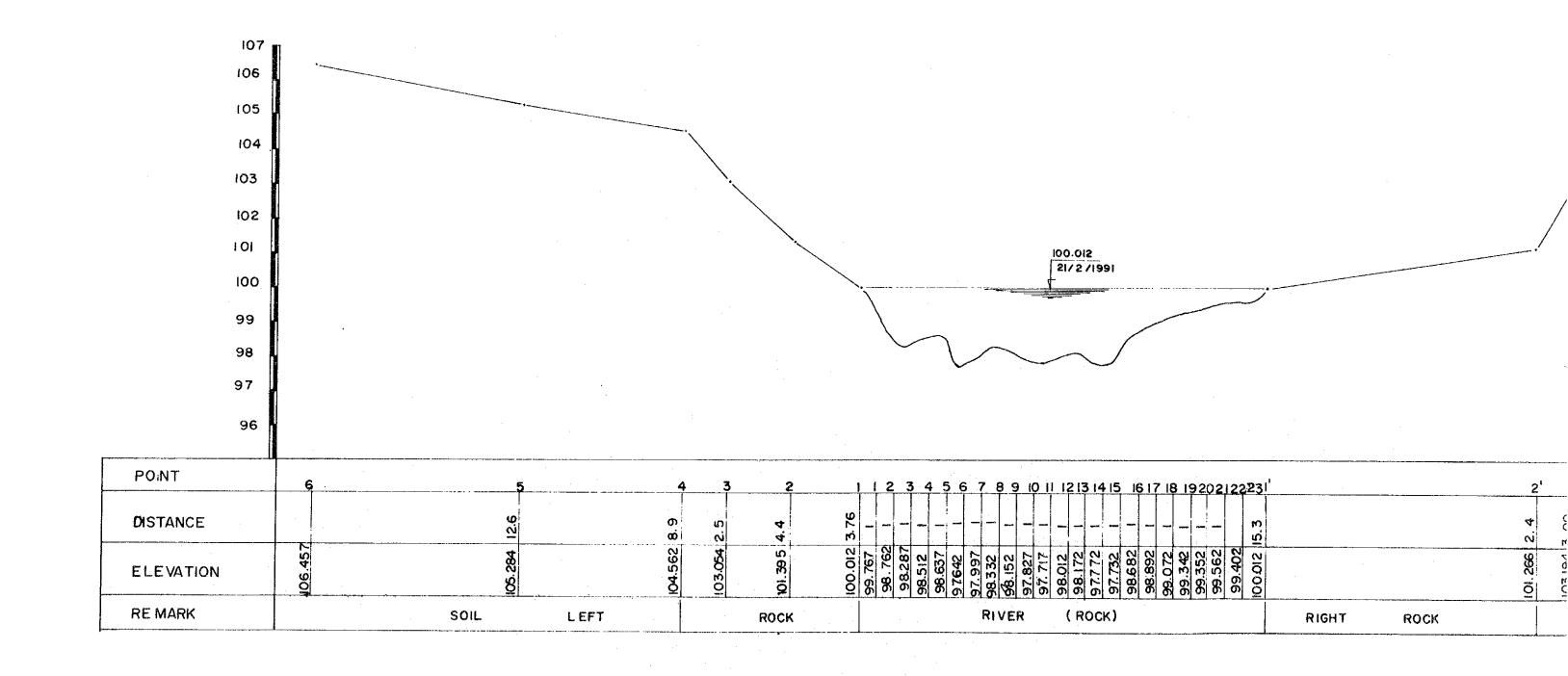
4	3	2_	i41403938 37363534 33323130292	28 2726 2524 2322	2 21 20 19 18 17 16 15 14 13	12 11 10 9 8 7 6 5 4 3 2 1 1	2 3 4	5' 6'	71	ه ^ا
3.10	02.	06.1	88				6.35 5.35 5.33 5.33	3.13	0 0 0	
103.408		100.374	99.275 99.195 98.495 98.495 98.840 98.840 98.860 98.860 98.860	98.610 98.720 98.920 98.920 99.920 99.250	99.190 99.350 99.130 99.130 99.570 98.670 98.670	99.200 99.200 99.200 99.200 99.340	100.253 102.625 04.094	06.106	107.038	09.598
				RIVER	(ROCK)			RIGHT	SOIL	

CROSS SECTION OF XE NAMNOY RIVER AT BAN LATSASIN (1/2)

(15 m upstream from staff gauge)

S CALE V: 100 H: 200

SCALE OF WATER 0.74 Cm

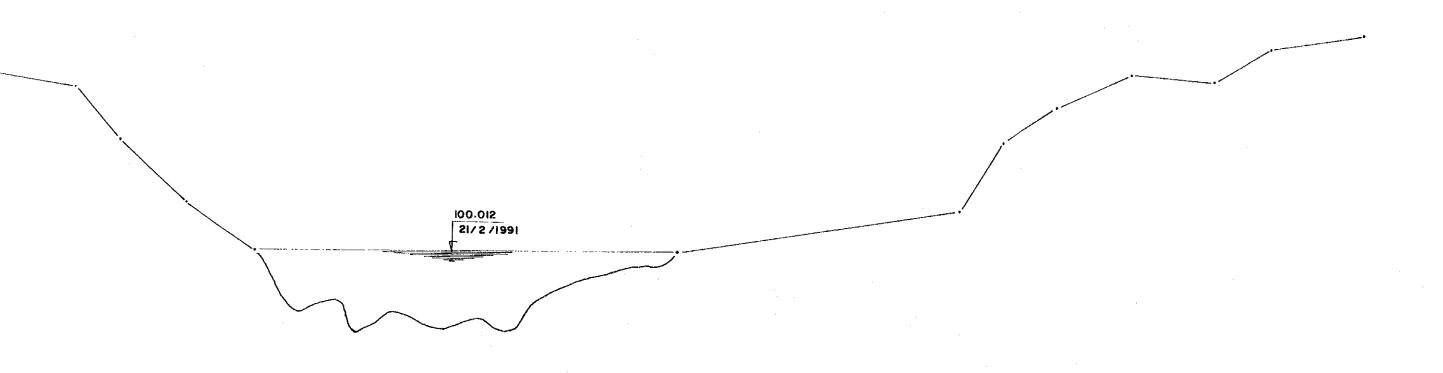


CROSS SECTION OF XE NAMNOY RIVER AT BAN LATSASIN (1/2)

(15 m upstream from staff gauge)

S CALE V: 100 H: 200

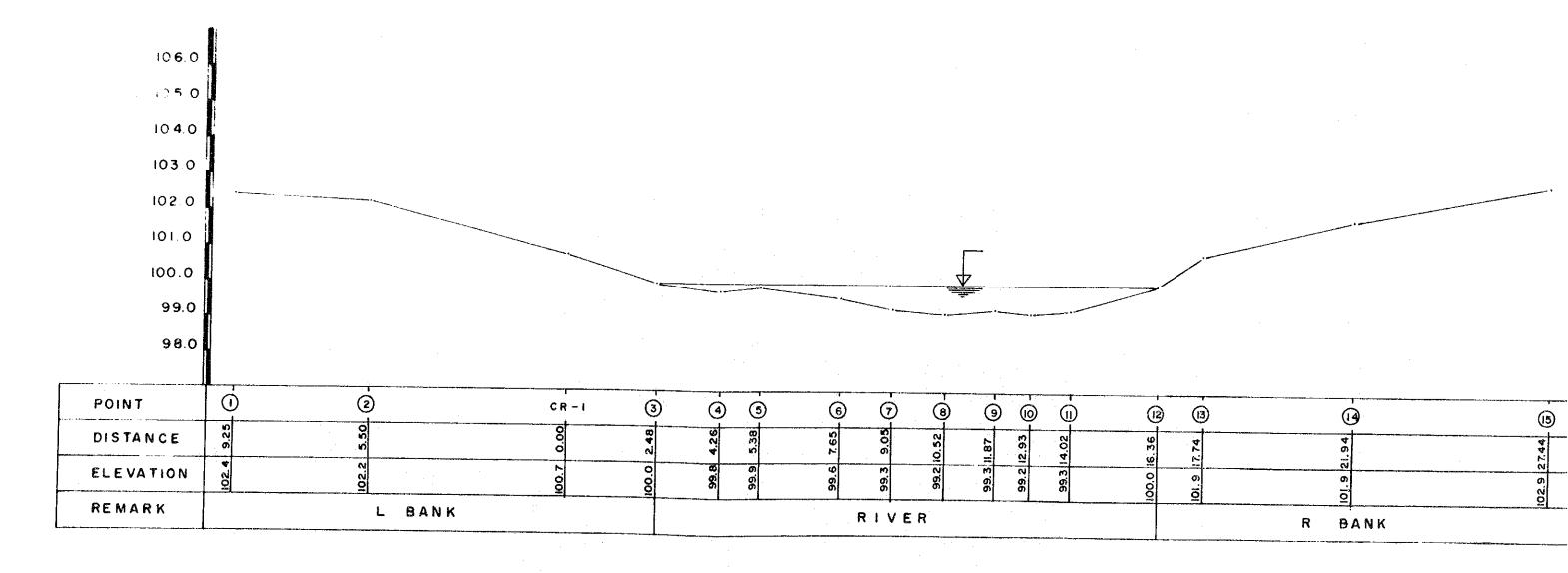
SCALE OF WATER 0.74 Cm



4	3 2	1 1 2 3 4 5 6 7 8 9 10 1	1 12131415 16	17 18 19202122 2 31	· · · · · · · · · · · · · · · · · · ·		2!	3 ¹	4'	5'	6'	7'	8'	
თ დ	2. 4. 2. 4.	92.6		E			2.4	0	4	π	4.05	<u>.</u>		
104.562	103.054 101.395	99.767 98.762 98.762 98.512 98.537 97.997 97.997 98.332 98.332	98.012 98.172 97.772 97.732	98.892 99.342 99.352 99.562 99.402			101.266	103-194	<u>5</u>	05.084	04.864	05.890	06.240	· · · · · · · · · · · · · · · · · · ·
	ROCK	RIVER	(ROCK)		RIGHT	ROCK					SOIL		·	

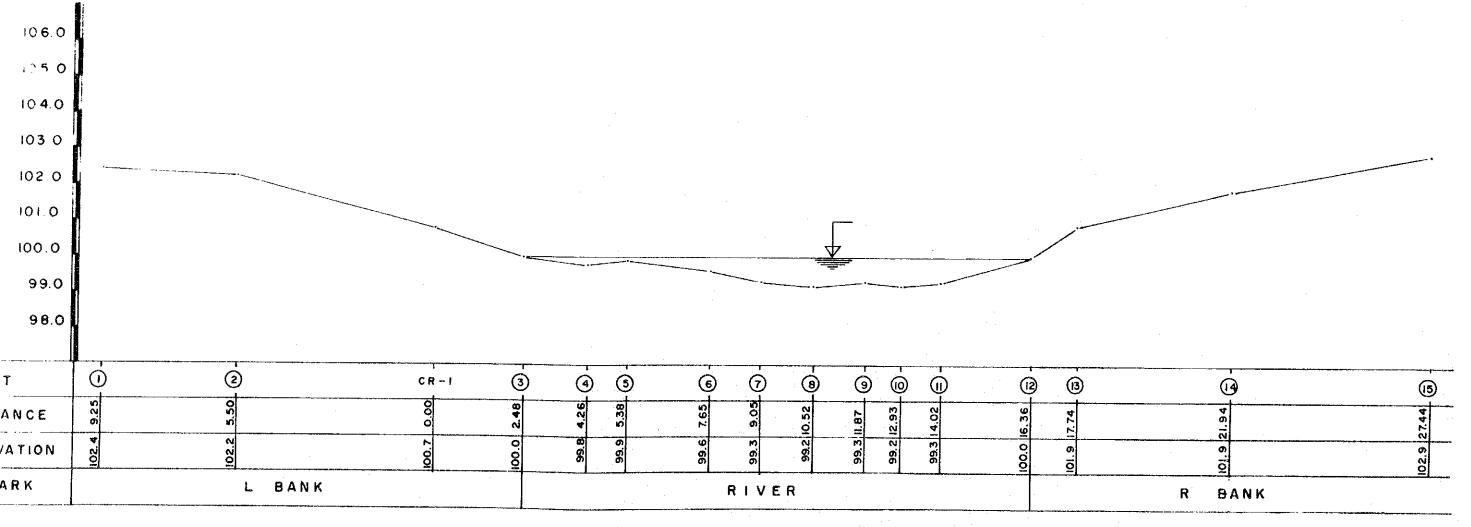
CROSS SECTION OF HOUAY MAKCHAN RIVER AT NIKHON34 (1/3)

(10 m upstream from staff gauge at bridge)



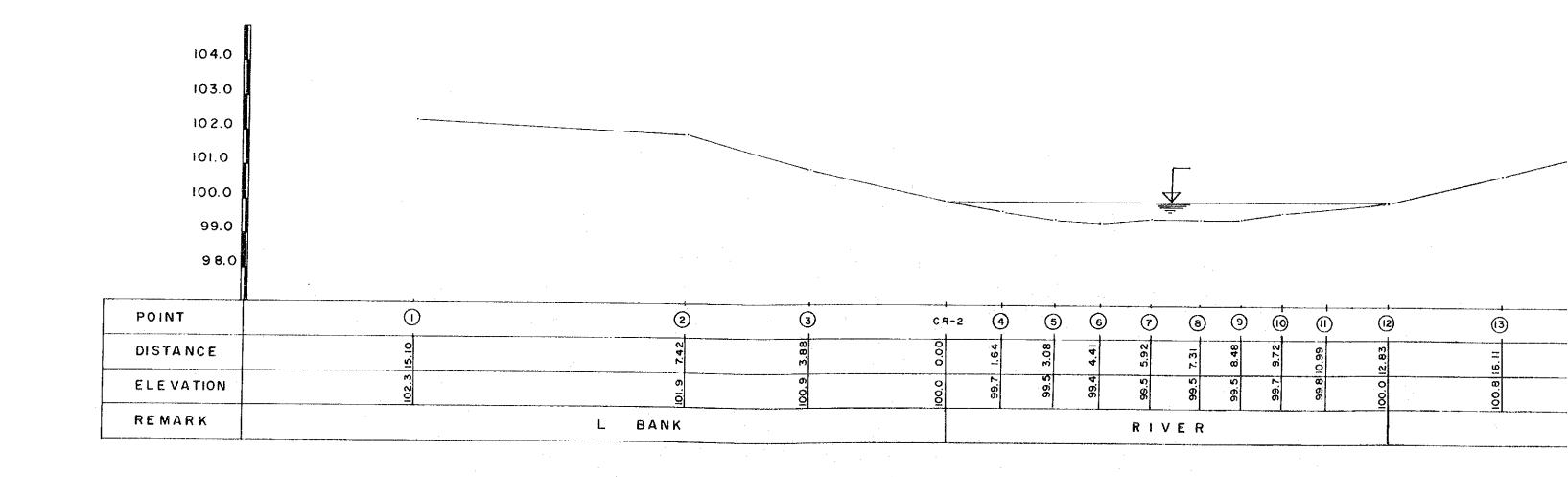
CROSS SECTION OF HOUAY MAKCHAN RIVER AT NIKHON34 (1/3)

(10 m upstream from staff gauge at bridge)



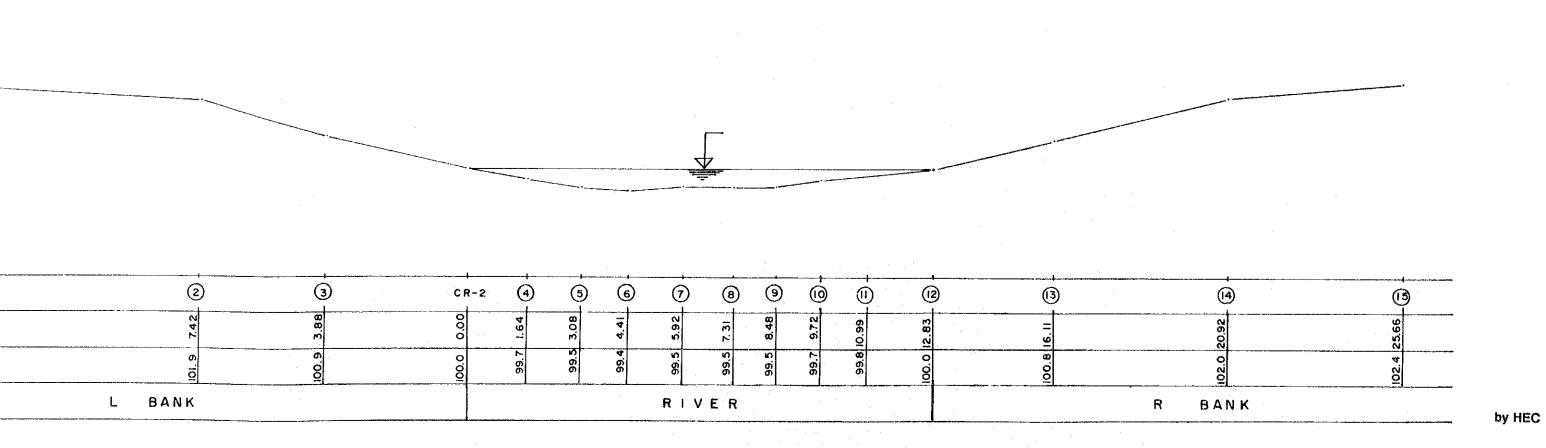
CROSS SECTION OF HOUAY MAKCHAN RIVER AT NIKHON34 (2/3)

(at staff gauge)



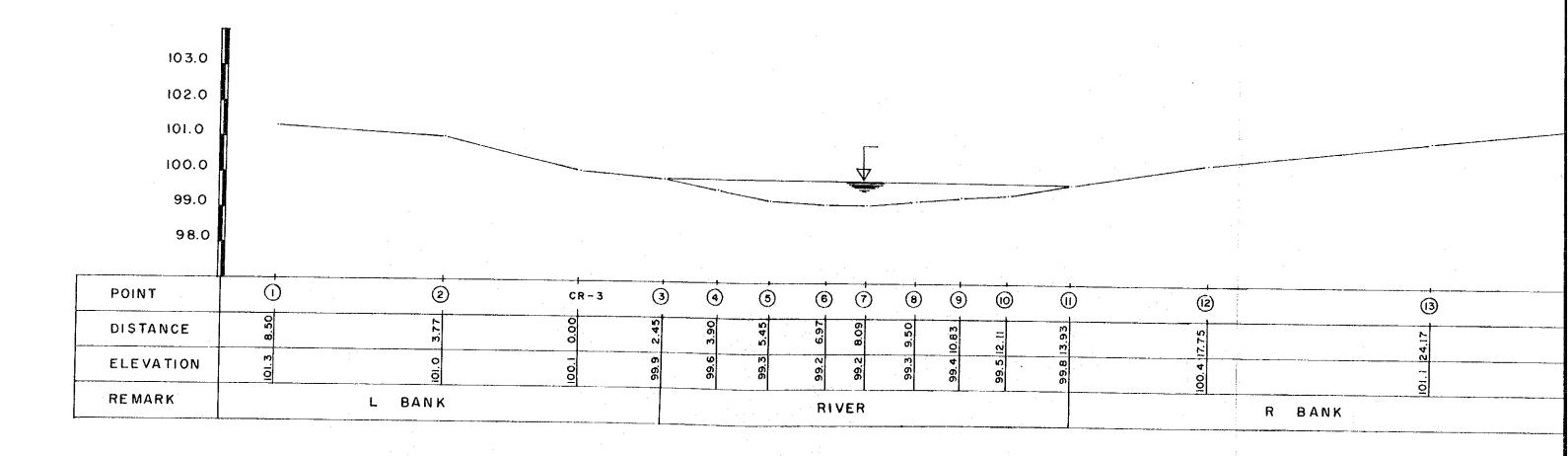
CROSS SECTION OF HOUAY MAKCHAN RIVER AT NIKHON34 (2/3)

(at staff gauge)



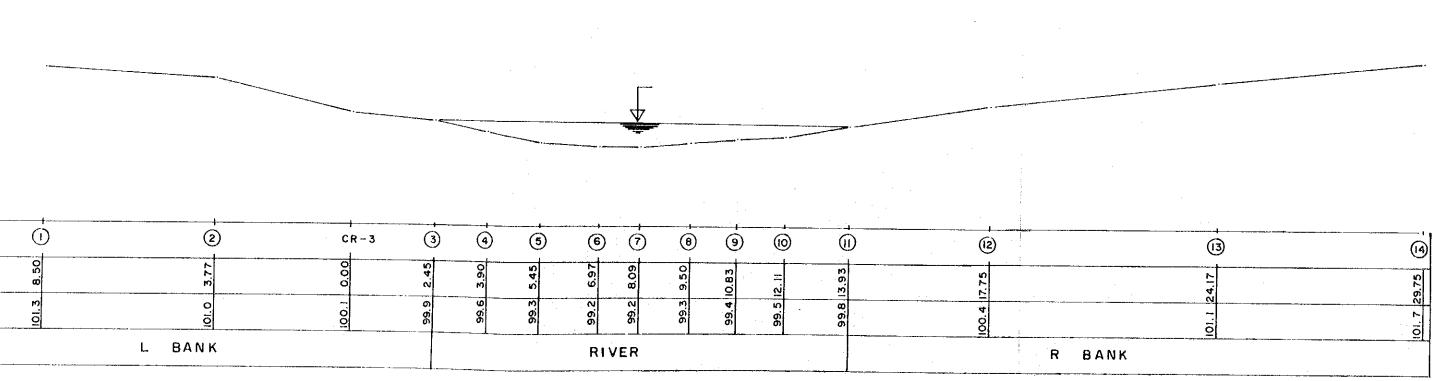
CROSS SECTION OF HOUAY MAKCHAN RIVER AT NIKHON34 (3/3)

(10 m downstream from staff gauge)



CROSS SECTION OF HOUAY MAKCHAN RIVER AT NIKHON34 (3/3)

(10 m downstream from staff gauge)



APPENDIX-4 Regulating Pond Capacity

APPENDIX-4

Contents

		Page
A4.1	Necessity of Pond	. AP-4-1
A4.2	Calculation Condition	. AP-4-1
A4.3	Results of Analysis	. AP-4-2
A4.4	Regulating Pond Operation	. AP-4-2

Appendix 4

A4.1 Necessity of Pond

Xe-Katam Small-scale Hydroelectric Power Station is a run-of-river, therefore basically generating power is limited up to current stream energy at anytime.

On the other hand, power demand fluctuates from bottom load to peak load during day. After commissioning, according to demand growth in future, this power station will spill water in midnight as bottom load and will not generate enough power to demand at peak load.

Provided that, spill energy can be used for peak load portion, stream energy will be used in so high effectivity. As following daily load curve, this power station can serve stable power to Se Kong and Attapeu up to stream capability.

For this solution, required capacity has been calculated and examined. Calculation conditions are shown in A4.2.

A4.2 Calculation Condition

Calculation condition are follows.

Inflow : 1.10 m³/sec equivalent value to 95% firm capacity

Spill Energy : Adjustment to zero

Effective Head: Constant

Efficiency: Depend on load

Received Power: Required power for adjusting spill energy to zero

Study Team : From 1995 to the year, that expected bottom load at

midnight will exceed 1,400 kW (1.1 m³/sec equivalent power)

 $(1995 \sim 2006)$

Results of these calculation are shown in Table A-4-1.

A4.3 Results of Analysis

From the results of this analysis, the maximum required pond capacity is estimated as $11,100 \text{ m}^3$ in 2000. However, after 2001 this value will be reduced, and daily load curve is calculated by estimated value. Therefore, adequate capacity of this pond is recommended as $10,000 \text{ m}^3$.

A4.4 Regulating Pond Operation

Since above analysis was carried out based on 95% firm capacity, it appeared that this pond is not effective after 2001. However, at that time it will be still effective, if the critical inflow set larger value than 1.1 $\rm m^3/sec$, such as 1.5 $\rm m^3/sec$.

Table A-4-1 Calculation of Regulating Pond Capacity (1)

Inflow=1.10m³/S

Hours		1.	2	3	4	5	6	5 7	' 8	9	10) 13	12	1 2	1.4	1.5					
Power Demand as of 1995 (KW)		645	645	645	671	738	802	879	1074	1201						15	16	17	18	3 19	
Number of Operation Units Purchase (KWh)		1	1	1	1	1	1	1	2						952 1	1265 2	1291 2	1361 2			1
Inflow (Minimum Inflow) (m^3/S)		1.10		~	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1 10	1 10	_		. 4	2	1.	
Surplus or Shortage (m^3/S)		0.52 0.58	0.58	0.58	0.56	$0.59 \\ 0.51$	$0.64 \\ 0.46$	0 40	0.86	1.04	1.01	1.01	1.04	0.65	0.76	1.01	1.04	1.10 1.09	$1.10 \\ 0.94$	• •	
Accumulation (10 ³ *m ³) Sub Total (10 ³ *m ³)		12.85	14.95	17.05	19.07	20.90	22.55	23.97	0.24 24.83	0.06 25.06	0.09 25.37	0.09 25.68	0.06 25.91	0.45 27.54	0.34	0.09	0.06	0.01	0.16	0.41	Ö.
Required Reservoir Volume(10^3*m^3)	0.00														20.70	25.07	49.30	29.33		1.48 Start	3.
Power Demand as of 1996 (KWh)		722	722	722	752	827	898	984	1203	1116	1 / 1 17	1 4 1 5		_ 1.							
Number of Operation Units Purchase (KWh)		1	1	1	1	1	1	1	2	1446 2	1417 2	1417 2	1446 2	905 1	1067 2	1417 2	1446 2	1524	1315 2		9
Inflow (Minimum Inflow) (m^3/S)		1.10	1.10	1.10	1.10	1.10	1.10		1.10	1.10	1.10	1.10	1.10	1.10	1 10	_		2,	2	1	
Surplus or Shortage (m^3/S)		0.58 0.52	0.52	0.52	$0.60 \\ 0.50$	0.44	0.72 0.38	0 21	0.97	1.16	1.14	1.14	1.16	0.72	$\frac{1.10}{0.86}$	1.14	$1.10 \\ 1.16$	$\substack{1.10\\1.22}$	$\frac{1.10}{1.05}$	$\frac{1.10}{0.77}$	
Accumulation (10 ³ *m ³) Sub Total (10 ³ *m ³)		11.24	13.11	14.99	16.78	18.36	19.73	20.85	21.34	21.13	-0.04 21.00	-0.04 20.87	-0.06 20.66	0.38	0.24	-0.04	-0.06	-0.12	0.05	0.33	0.
Required Reservoir Volume(10^3*m^3)	0.78								21.34				20.66		22.89	22.70	44.00	22.11 22.11	0.16 Start	1.35	2.
Power Demand as of 1997 (KWh)		773	773	773	804	884	961	1053	1288	1547	1516	3516	1 ~ 4 ~								
Number of Operation Units Purchase (KWh)		1	1	1	1	1	1	2	2	2	1516 2	1516 2	1547 2	968 1	1141 2	1516 2	1547	1631 2	1407	1029	9
Inflow (Minimum Inflow) (m ³ /S)		1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1 10	_	- .		2	2	2	
Surplus or Shortage (m^3/S)		0.62 0.48	$0.62 \\ 0.48$	$0.62 \\ 0.48$	$0.64 \\ 0.46$	U 30	0.77 0.33	$0.84 \\ 0.26$	1.03	1.24	1.21	1.21	1.24	$\frac{1.10}{0.78}$	$\frac{1.10}{0.92}$	$\frac{1.10}{1.21}$	$\frac{1.10}{1.24}$	$\frac{1.10}{1.31}$	1.10	$\frac{1.10}{0.83}$	1.
Accumulation (10 ³ *m ³) Sub Total (10 ³ *m ³)		9.91	11.64	13.37	15.01	16.42	17.61	18.53	18.77	-0.14 18.27	-0.11 17.85	-0.11 17.44	-0.14 16.94	0.32	0.18	-0.11	-0.14	-0.21 17.11	-0.03	0.27	0.
Required Reservoir Volume(10°3*m°3)	1.83								18.77				16.94	10.11	18.77	10,30	17.03	17.11	17.01 17.01	0.99 Start	2.4
Power Demand as of 1998 (KWh)		827	827	827	860	946	1028	1127	1378	1655	1620										
Number of Operation Units Purchase (KWh)		1	1	1	1	1	2	2	2	1655 2	1622 2	1622 2	1655 2	1036 2	1221 2	1622 2	1655	1745 2	1506		100
Inflow (Minimum Inflow) (m ³ /S)		1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1 10	_		_	Z		2	2	
Surplus or Shortage (m^3/S)		$0.66 \\ 0.44$	$0.66 \\ 0.44$	0.66	0.41	0.34	0.83	በ ጎበ	1.10	1.33	1.30	1.30	$\frac{1.10}{1.33}$	1.10 0.83	$\frac{1.10}{0.98}$	$\frac{1.10}{1.30}$	1.10 1.33	$\frac{1.10}{1.40}$	$\frac{1.10}{1.21}$	0.0	1.1
Accumulation (10 ³ *m ³)		8.65	10.22	11.80	13.28	14.51 1	0.27 5.50	0.20 16.20	0.00 - 16.19 1	-0.23 - 15.37 :	-0.20 14.66	-0.20	-0.23	0.27	0.12	-0.20	-0.23	-0.30 -	-0.11	0.22	0.7
Sub Total (10^3*m^3) Required Reservoir Volume(10^3*m^3)	3.06	•							16.19			±0.5±	13.12	14.09	14.52	13.81	12.99	11.92 [11.54 11.54	$0.78 \\ 0.78$	1.6
Power Demand as of 1999 (KWh)		885	885	885	921	1013	1100	1205	1 4 7 4	1 = = = =						•					
Number of Operation Units Purchase (KWh)		1	1	1	1	2	2	2	14/4 2	2	1736 2	1736 2	1771 2	1109 2	1307 2	1736		1867		1178	114
Inflow (Minimum Inflow) (m ³ /S)		1.10	1.10	1.10	1.10	1.10	1.10	1.10	1 10	1 10	1 10	1 10			_	4,	2	2	2	2	
Discharge (m^3/S) Surplus or Shortage (m^3/S)		0.71	0.71 0.39	0.71	0.74									0.89	$\frac{1.10}{1.05}$	1.10 1.39	1.10	$1.10 \\ 1.49$	1.10	1.10	1.1
Accumulation (10 ³ *m ³)		~ . ~ .	0.00	V + J J	V.JU	U - Z Y	11. 2.2	11 PK -	-0.08 - 3.44 1	11 ")")	A 2A		-0.32	0.21	0.05 -	-0.29 -	0.32	-0.39 -	-0.19		0.90.1
Sub Total (10^3*m^3) Required Reservoir Volume(10^3*m^3)	4.67						1	3.74			ا لايده د. د	.V.4I	9.06 9.06	y.82]	0.01 .0.01	8.97	7.82		5.71 5.71 S	0.56	1.2
,																			○• , 1	JULEL	

culation of Regulating Pond Capacity (1) $Inflow=1.10m^3/S$ 2 3 1 9 10 11 12 13 14 15 16 17 18 1.9 20 21 22 23 24 Total 738 645 645 671 802 879 1074 1291 1265 1265 1291 808 952 1265 1291 780 723 671 22573 1361 1174 859 832 645 $1.10 \quad 1.10$ 1.10 1.101.10 1.10 1.10 1.10 1.10 1.10 1.10 1.10 (Average) 0.59 0.64 0.70 0.86 1.04 1.01 1.01 1.04 0.65 0.76 1.01 1.04 1.09 0.94 0.69 0.67 0.62 0.52 0.52 0.52 0.54 0.75 (Average) 0.58 0.58 0.58 0.56 0.51 0.46 0.40 0.24 0.06 0.09 0.09 0.06 0.45 0.34 0.09 0.06 0.01 0.16 0.41 0.43 0.48 0.35 (Average) 0.52 12.85 14.95 17.05 19.07 20.90 22.55 23.97 24.83 25.06 25.37 25.68 25.91 27.54 28.76 29.07 29.30 29.33 29.90 1.48 3.05 4.76 6.63 8.65 10.75 29.90 29.90 Start 827 984 1203 1446 1417 1417 1446 905 1067 1417 1446 1524 1315 25281 722 722 752 898 962 932 874 810 752 722 1.10 (Average) 1.10 1.10 1.10 1.10 1.10 1.10 1.10 1.10 $0.58 \ \ 0.58 \ \ 0.58 \ \ 0.60 \ \ 0.66 \ \ 0.72 \ \ 0.79 \ \ 0.97 \ \ 1.16 \ \ 1.14 \ \ 1.14 \ \ 1.16 \ \ 0.72 \ \ 0.86 \ \ 1.14 \ \ 1.16$ 1.22 1.05 0.77 0.75 0.70 0.65 0.60 0.84 (Average) $0.52 \ 0.52 \ 0.52 \ 0.50 \ 0.44 \ 0.38 \ 0.31 \ 0.13 \ -0.06 \ -0.04 \ -0.04 \ -0.06 \ 0.38 \ 0.24 \ -0.04 \ -0.06 \ -0.12 \ 0.05 \ 0.33 \ 0.35 \ 0.40$ 0.45 0.50 0.52 0.26 (Average) 11.24 13.11 14.99 16.78 18.36 19.73 20.85 21.34 21.13 21.00 20.87 20.66 22.01 22.89 22.76 22.55 22.11 0.16 1.35 2.63 4.07 5.69 7.4922.11 21.34 20.66 22.89 22.11 Start 968 1141 1516 1547 773 773 804 884 961 1053 1288 1547 1516 1516 1547 1631 1407 1029 997 935 866 804 27051 1 2 2 2 2 2 1 2 1 1.10 (Average) $1.10 \quad 1.10 \quad$ 1.10 1.10 1.10 1.10 1.10 0.90 (Average) $0.62 \quad 0.62 \quad 0.62 \quad 0.64 \quad 0.71 \quad 0.77 \quad 0.84 \quad 1.03 \quad 1.24 \quad 1.21 \quad 1.21 \quad 1.24 \quad 0.78 \quad 0.92 \quad 1.21 \quad 1.24 \quad 1.31 \quad 1.13 \quad 0.83 \quad 0.80 \quad 0.75 \quad 0.94 \quad$ 0.69 0.64 $0.48 \quad 0.48 \quad 0.48 \quad 0.46 \quad 0.39 \quad 0.33 \quad 0.26 \quad 0.07 \quad -0.14 \quad -0.11 \quad -0.11 \quad -0.14 \quad 0.32 \quad 0.18 \quad -0.11 \quad -0.14 \quad -0.21 \quad -0.03 \quad 0.27 \quad 0.30 \quad 0.35 \quad 0.48 \quad$ 0.20 (Average) 0.41 0.46 0.48 9.91 11.64 13.37 15.01 16.42 17.61 18.53 18.77 18.27 17.85 17.44 16.94 18.11 18.77 18.36 17.85 17.11 17.01 0.99 2.08 3.34 4.80 6.44 8.17 18.77 16.94 18.77 946 1028 1127 1378 1655 1622 1622 1655 1036 1221 1622 1655 1745 1506 1101 1067 1000 927 827 28945 827 827 827 860 860 2 O 1.10 (Average) 1.10 1.10 1.10 $0.66 \ 0.66 \ 0.66 \ 0.69 \ 0.76 \ 0.83 \ 0.90 \ 1.10 \ 1.33 \ 1.30 \ 1.33 \ 0.83 \ 0.98 \ 1.30 \ 1.33 \ 1.40 \ 1.21 \ 0.88 \ 0.86 \ 0.80 \ 0.74 \ 0.69$ 0.66 0.97 (Average) $0.44 \quad 0.44 \quad 0.44 \quad 0.41 \quad 0.34 \quad 0.27 \quad 0.20 \quad 0.00 \quad -0.23 \quad -0.20 \quad -0.23 \quad 0.27 \quad 0.12 \quad -0.20 \quad -0.23 \quad -0.30 \quad -0.11 \quad 0.22 \quad 0.24 \quad 0.30 \quad 0.36 \quad 0.41 \quad 0.44 \quad$ 0.13 (Average) 8.65 10.22 11.80 13.28 14.51 15.50 16.20 16.19 15.37 14.66 13.94 13.12 14.09 14.52 13.81 12.99 11.92 11.54 0.78 1.66 2.73 4.02 5.50 16.19 13.12 11.54 0.78 14.52 921 1013 1100 1205 1474 1771 1736 1736 1771 1109 1307 1736 1771 1867 1611 1178 1142 1070 992 885 30971 885 885 885 1.10 (Average) $1.10 \quad 1.10 \quad 1.10$ 1.10 1.10 1.10 1.10 1.10 1.10 1.10 $0.71 \quad 0.71 \quad 0.71 \quad 0.74 \quad 0.81 \quad 0.88 \quad 0.97 \quad 1.18 \quad 1.42 \quad 1.39 \quad 1.39 \quad 1.42 \quad 0.89 \quad 1.05 \quad 1.39 \quad 1.42$ 0.71 1.49 1.29 0.94 0.92 0.86 0.79 0.74 1.03 (Average) $0.39 \quad 0.39 \quad 0.39 \quad 0.36 \quad 0.29 \quad 0.22 \quad 0.13 \quad -0.08 \quad -0.32 \quad -0.29 \quad -0.29 \quad -0.32 \quad 0.21 \quad 0.05 \quad -0.29 \quad -0.32 \quad -0.39 \quad -0.19 \quad 0.16 \quad 0.18 \quad 0.24 \quad 0.31 \quad 0.36 \quad 0.39 \quad -0.39 \quad -0.$ 0.07 (Average) 7.32 8.73 10.13 11.44 12.48 13.26 13.74 13.44 12.30 11.25 10.21 9.06 6.40 5.71 0.56 5.71

9.82 10.01 8.97 7.82

10.01

9.06

13.74

1.22 2.09

5.71 Start

3.19

4.50

Table A-4-1 Calculation of Regulating Pond Capacity (2)

Nu	ower Demand as of 2000 umber of Operation Unit urchase	(KWh) ts (KWh)		947 1	947	947 1	985 1	1083 2	1177 2	1290 2	1577 2	1895 2	1857 2	1857 2	1895 2	1186 2	1398 2	1857 2	1895 2	1998 2	1724 2	1260 2	1221
In Di Su	nflow (Minimum Inflow) ischarge urplus or Shortage	(m^3/S) (m^3/S) (m^3/S)		1.10 0.76 0.34	0.76 0.34	0.34	0.79	0.87 0.23	1.10 0.94 0.16		1.10 1.26 -0.16	1.10 1.52 -0.42	1.10 1.49 -0.39	1.49	40 1.10 1.49 -0.39	1.10 0.95 0.15	1.10 1.12 -0.02	40 1.10 1.45 -0.35	50 1.10 1.48	50 1.10 1.56	1.10 1.38	1.10 1.01	1.10 0.98
	ccumulation Sub Total equired Reservoir Volum	(10^3*m^3) (10^3*m^3) ne(10^3*m^3)	11.10	5.89	7.12	8.35	9.47	10.30			10.51	9.01				5.37	5.29	4.02	2.66	-0.46 1.01	0.00	0.09 0.32 Start	0.12 0.75
Nu Pu	ower Demand as of 2001 umber of Operation Unit urchase	(KWh) s (KWh)		1013 2	1013 2	1013 2	1054 2	1159 2	1260 2	1380 2	1688 2	2028	1987	1987	2028	1269 2	1496 2	1987	2028	2138	18 4 5 2	1349 2	1307 2
Di Su	aflow (Minimum Inflow) scharge arplus or Shortage	(m ³ /S) (m ³ /S) (m ³ /S)	·	1.10 0.81 0.29	1.10 0.81 0.29	1.10 0.81 0.29	1.10 0.85 0.25	1.10 0.93 0.17	1.10 1.01 0.09	1.10 1.11 -0.01	150 1.10 1.23	300 1.10 1.38 -0.28	300 1.10 1.35 -0.25	300 1.10 1.35 -0.25	300 1.10 1.38 -0.28	1.10	1.10	300 1.10 1.35	300 1.10 1.38	300 1.10 1.47	273 1.10 1.26	1.10	1.10 1.05
	cumulation Sub Total equired Reservoir Volume	(10^3*m^3) (10^3*m^3) ne(10^3*m^3)	8.25	4.34	5.37	6.40	7.32	7.93	8.25 8.25	8.23	7.75	6.73	5.83	4.92	3.90	0.08 4.20	-0.10 3.84	-0.25 2.94	-0.28 1.91	-0.37 0.58	-0.16 0.00 0.00	0.02 0.07 Start	0.05 0.25
Nui	wer Demand as of 2002 mber of Operation Units rchase	(KWh) s (KWh)		1084	1084 2	1084 2	1128 2	1240	1348	1477	1806	2170	2126 2	2126 2	2170 2	1358 2	1601	2126 2	2170 2	2288 2	1974 2	1443 2	1398 2
In: Dis	flow (Minimum Inflow) scharge rplus or Shortage	(m ³ /S) (m ³ /S) (m ³ /S)		1.10 0.87 0.23	1.10 0.87 0.23	1.10 0.87 0.23	1.10	200 1.10 0.83	200 1.10 0.92	200 1.10 1.02	500 1.10 1.05	500 1.10 1.34	500 1.10 1.30	500 1.10 1.30	500 1.10 1.34	1.10 1.09	300 1.10 1.04	400 1.10 1.38	400 1.10 1.42	500 1.10 1.43	250 1.10 1.38	56 1.10	1.10
Acc	cumulation Sub Total quired Reservoir Volume	(10^3*m^3) (10^3*m^3)		2.99			0.20 5.35	0.27 6.30	0.18 6.94	0.08 7.22	0.05 7.41 7.41	-0.24 6.55	-0.20 5.82	-0.20 5.09	-0.24 4.24	0.01 4.28	0.06 4.48	-0.28 3.47	-0.32 2.32			~0.01 ~ 0.08	-0.02 0.00 0.00
Pow Num	wer Demand as of 2003 mber of Operation Units	(KWh)		1160 2	1160 2	1160 2	1207 2	1327 2	1442 2	1580 2	1932 2	2322	2275 2	2275	2322	1453 2	1 7 13	2275	2322	2448	2112	1544	1496
Inf Dis	rchase flow (Minimum Inflow) scharge	(KWh) (m^3/S) (m^3/S)						180 1.10 0.92	200 1.10 1.00	300 1.10 1.03	700 1.10 0.99	700 1.10 1.30	700 1.10 1.26	700 1.10 1.26	700 1.10 1.30	200 1.10 1.01	300 1.10 1.13	700 1.10 1.26	700 1.10 1.30	700 1.10	650 1.10	2 220 1.10	12. 1.10
Acc		(m^3/S) (10^3*m^3) (10^3*m^3)							0.10 4.62	0.07		-0.20 4.57			-0.20 2.68					1.40 -0.30 - 0.53		0.04 -	1.19 -0.09 0.09
Pow Num	ver Demand as of 2004 wher of Operation Units	(KWh)	•	1241	1241	1241	1291 2	1420 2	15 4 3 2	1691	2068			2434				2434	2484			1652	1601
Inf Dis	chase low (Minimum Inflow) charge	(KWh) (m^3/S) (m^3/S)		1.10		1.10	1.10	1.10	1.10	1.10	1000 1.10	1.10	1000 1.10 1.15	1000	1000 1.10	500 1.10	1.10		1000 1.10			2 400 1.10	2 155 1.10
Acci	Sub Total	(m ³ /S) (10 ³ *m ³) (10 ³ *m ³)	•	0.10	0.10	$0.10 \\ 1.74$	0.06 -	-0.04 - 1.83	-0.14 -	-0.21	0.24 -	-0.09 -	-0.05 - 0.94	1.15 -0.05 0.77		0.25	0.03 -	0.05 -	1.19 -0.09 0.97	-0.20	0.01	0.10 -	1.16 -0.06 0.42
Requ	uired Reservoir Volume	(10^3*m^3)	1.97																				, , , , , , , , , , , , , , , , , , ,

ulation of Regulating Pond Capacity (2)

	947 1	9 4 7	947 1	985 1	1083 2	1177 2	1290 2	1577 2	1895 2	1857 2	1857 2	1895	1186 2	1398 2	1857	1895	1998	1724 2	1260 2	1221 2	1145 2	1061 2	985 1	947 1	33139	
1.10	1.10 0.76 0.34 5.89	1.10 0.76 0.34 7.12	1.10 0.76 0.34 8.35	1.10 0.79 0.31 9.47	0.87 0.23	0.16	1.03 0.07	-0.16	1.10 1.52 -0.42 9.01	-0.39			1.10 0.95 0.15 5.37	1.10 1.12 -0.02 5.29	40 1.10 1.45 -0.35 4.02	50 1.10 1.48 -0.38 2.66	1.10 1.56 -0.46 1.01		1.10 1.01 0.09 0.32 Start	1.10 0.98 0.12 0.75	1.10 0.92 0.18 1.41	1.10 0.85 0.25 2.30	1.10 0.79 0.31 3.42	1.10 0.76 0.34 4.65	1.10	(Average) (Average) (Average)
	1013 2	1013 2	1013 2	1054 2	1159 2	1260 2	1380 2	1688 2 150	2028 2 300	1987 2 300	1987 2 300	2028 2 300	1269 2	1 4 96 2	1987 2 300	2028 2 300	2138 2 300	1845 2 273	1349 2	1307 2	1225 2	1136 2	1054 2	1013 2	35458	
8.25	1.10 0.81 0.29 4.34	1.10 0.81 0.29 5.37	1.10 0.81 0.29 6.40	1.10 0.85 0.25 7.32	1.10 0.93 0.17 7.93	1.10 1.01 0.09 8.25 8.25		1.10 1.23 -0.13	1.10	1.10 1.35 -0.25	1.10 1.35 -0.25	1.10 1.38		1.10 1.20 -0.10 3.84	1.10 1.35	1.10 1.38 -0.28	1.10 1.47 -0.37 0.58	1.10 1.26 -0.16 0.00	1.10 1.08 0.02 0.07 Start	1.10 1.05 0.05 0.25	1.10 0.98 0.12 0.68	$0.91 \\ 0.19$	1.10 0.85 0.25 2.27	1.10 0.81 0.29 3.30	1.10	(Average) (Average) (Average)
	1084 2	1084 2	1084 2	1128 2	1240 2	13 4 8 2	1477 2	1806 2	2170 2	2126 2	2126 2	2170 2	1358 2	1601 2	2126 2	2170 2	2288 2	1974 2	1443 2	1398	1311	1215 2	1128 2	1084 2	37940	
	1.10 0.87 0.23 2.99	0.87	1.10 0.87 0.23 4.64	1.10 0.90 0.20 5.35	200 1.10 0.83 0.27 6.30	200 1.10 0.92 0.18 6.94	200 1.10 1.02 0.08 7.22	500 1.10 1.05 0.05 7.41 7.41	-0.24			500 1.10 1.34 -0.24 4.24	1.10 1.09 0.01 4.28	300 1.10 1.04 0.06 4.48	400 1.10 1.38 -0.28 3.47		500 1.10 1.43 -0.33 1.13	-0.28			0.18	0.97 0.13	0.90 0.20	1.10 0.87 0.23 2.16	1.10	(Average) (Average) (Average)
7.41		1160	1160	1207	1327	1442	1580	1932	2322	2275	2275	2322	1453	1713	2275	2322	2448	2112	1544	1496	1403	1300	1207	1160	40596	
	0.93 0.17	0.93 0.17	1.10 0.93 0.17 3.12	1.10 0.97 0.13 3.60	180 1.10 0.92 0.18 4.24	200 1.10 1.00 0.10 4.62	300 1.10 1.03 0.07 4.88	700 1.10 0.99 0.11 5.28 5.28	-0.20		-0.16	700 1.10 1.30 -0.20 2.68	2 200 1.10 1.01 0.09 3.02	300 1.10 1.13 -0.03 2.91	-0.16	700 1.10 1.30 -0.20 1.61	700 1.10 1.40 -0.30 0.53		2 220 1.10 1.06 0.04 0.41	12 1.10 1.19 -0.09 0.09	1.12 -0.02	1.04 0.06 0.21	0.97 0.13	2 1.10 0.93 0.17 1.29	1.10	(Average) (Average) (Average)
5.29	12/11	12/1	1241	1201	1420	15/2	1601		2484	2424	2424	2494	1555	1022	2424	2404	2610	2260	1652	1601			1001	7041	10100	•
.97	1.10 1.00 0.10 0.99	1.10 1.00 0.10	1.10 1.00 0.10 1.74	1.10 1.04 0.06	2 1.10 1.14 -0.04	1.10 1.24 -0.14	50 1.10 1.31 -0.21	1000 1.10 0.86 0.24	2 1000 1.10 1.19 -0.09	2 1000 1.10 1.15 -0.05	2 1000 1.10 1.15 -0.05	2 1000 1.10 1.19 -0.09	2 500 1.10 0.85 0.25	500 1.10 1.07 0.03	2 1000 1.10 1.15 -0.05	2 1000 1.10 1.19 -0.09	1000 1.10 1.30 -0.20	900 1.10 1.09 0.01	400 1.10 1.00	155 1.10 1.16 -0.06		1.10 1.12	1.10 1.04 0.06 0.23	2 1.10 1.00 0.10	1.10	(Average) (Average) (Average)
,																										

Table A-4-1 Calculation of Regulating Pond Capacity (3)

Number of Operation Units Purchase (KWh) Inflow (Minimum Inflow) (m^3/S) Discharge (m^3/S) Surplus or Shortage (m^3/S) Accumulation (10^3*m^3) Sub Total (10^3*m^3) Required Reservoir Volume(10^3*m^3)	1.1 1.0 0.0 0.2	2 2 0 1.10 6 1.06 4 0.04	1328 2 1.10 1.06 0.04 0.51	1382 2 1.10 1.11 -0.01 0.48	1520 2 500 1.10 0.82 0.28 1.49	1651 2 500 1.10 0.92 0.18 2.13	1809 2 700 1.10 0.89 0.21 2.88	2212 2 1000 1.10 0.97 0.13 3.34 3.34	2658 2 1000 1.10 1.33 -0.23 2.52	-0.19	2605 2 1000 1.10 1.29 -0.19 1.19	2658 2 1000 1.10 1.33 -0.23 0.36	1664 2 1000 1.10 0.53 0.57 2.40	1961 2 900 1.10 0.85 0.25 3.30	2605 2 1000 1.10 1.29 -0.19 2.63	2658 2 1000 1.10 1.33 -0.23 1.81	2802 2 1000 1.10 1.44 -0.34 0.57	2418 2 1000 1.10 1.14 -0.04 0.44	1768 2 400 1.10 1.10 0.00 0.45	30 1.1 1.1 -0.0 0.3
Power Demand as of 2006 (KWh) Number of Operation Units Purchase (KWh) Inflow (Minimum Inflow) (m^3/S) Discharge (m^3/S) Surplus or Shortage (m^3/S) Accumulation (10^3*m^3) Sub Total (10^3*m^3) Required Reservoir Volume(10^3*m^3)	200 1.10 0.98 0.12 0.44	2 2 20 200 0 1.10 3 0.98 2 0.12	0.12	1478 2 500 1.10 0.79 0.31 2.44	1626 2 1000 1.10 0.50 0.60 4.59	1767 2 1000 1.10 0.62 0.48 6.33	1936 2 1000 1.10 0.75 0.35 7.59	2367 2 1000 1.10 1.10 0.00 7.60 7.60	2844 2 1000 1.10 1.48 -0.38 6.25	2787 2 1000 1.10 1.43 -0.33 5.05	2787 2 1000 1.10 1.43 -0.33 3.86	2844 2 1000 1.10 1.48 -0.38 2.51	1780 2 1000 1.10 0.63 0.47 4.21	2098 2 1000 1.10 0.88 0.22 5.00	2787 2 1000 1.10 1.43 -0.33 3.81	2844 2 1000 1.10 1.48 -0.38 2.45	2999 2 1000 1.10 1.60 -0.50 0.66	2588 2 1000 1.10 1.27 -0.17 0.04		183 40 1.1 1.1 -0.0 0.0

ation of Regulating Pond Capacity (3)

	1328	1328	1328	1382	1520	1651	1809	2212	2658	2605	2605	2658	1664	1961	2605	2658	2802	2418	1768	1713	1606	1489	1382	1328	46479	
	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2.	2	2		
					500	500	700	1000	1000	1000	1000	1000	1000	900	1000	1000	1000	1000	400	300	200	45			13545	
	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.10	(Average)
	1.06	1.06	1.06	1.11	0.82	0.92	0.89	0.97	1.33	1.29	1.29	1.33	0.53	0.85	1.29	1.33	1.44	1.14	1.10	1.13	1.13	1.16	1.11	1.06	1.10	(Average)
	0.04	0.04	0.04	-0.01	0.28	0.18	0.21	0.13	-0.23	-0.19	-0.19	-0.23	0.57	0.25	-0.19	-0.23	-0.34	-0.04	0.00	-0.03	-0.03	-0.06	-0.01	0.04	0.00	(Average)
	0.25	0.38	0.51	0.48	1.49	2.13	2.88	3.34	2.52	1.85	1.19	0.36	2.40	3.30	2.63	1.81	0.57	0.44	0.45	0.34	0.24	0.03	0.00	0.13	0.00	-
								3.34															0.00	Start		
4																										
	1421	1421	1421	1478	1626	1767	1936	2367	2844	2787	2787	2844	1780	2098	2787	2844	2999	2588	1892	1833	1719	1593	1478	1421	49732	
	1421 2	1421 2	1421 2	1478 2	1626 2	1767 2	1936 2	2367 2	2844 2	2787 2	2787 2	2844 2	1780 2	2098 2	2787 2	2844 2	2999 2	2588 2	1892 2	1833 2	1719 2	1593 2	1478 2	1421 2	49732	
	1421 2 200	1421 2 200	1421 2 200	1478 2 500	1626 2 1000	1767 2 1000	1936 2 1000	2367 2 1000	2844 2 1000	2787 2 1000	2787 2 1000	2844 2 1000	1780 2 1000	2098 2 1000	2787 2 1000	2844 2 1000	2999 2 1000	2588 2 1000	1892 2 600	1833 2 400	1719 2 400	1593 2 150	1478 2 90	1421 2 50	49732 16790	
	2	2	2	2	2	2	2	2	2	2 1000	2	2	2 1000	2	2	2 1000	2	2	2	2	2	2	2	2	16790	(Average)
	2 200 1.10	2 200	2 200 1.10	2 500	2 1000	2 1000	2 1000	2 1000 1.10	2 1000	2 1000	2 1000	2 1000 1.10	2 1000	2 1000	2 1000	2 1000	2 1000	2 1000	2 600	2 400	2 400	2 150 1.10	2 90	2 50 1.10	16790 1.10	(Average) (Average)
	2 200 1.10 0.98	2 200 1.10	2 200 1.10	2 500 1.10 0.79	2 1000 1.10	2 1000 1.10	2 1000 1.10	2 1000 1.10 1.10	2 1000 1.10 1.48	2 1000 1.10 1.43	2 1000 1.10 1.43	2 1000 1.10	2 1000 1.10 0.63	2 1000 1.10 0.88	2 1000 1.10 1.43	2 1000 1.10	2 1000 1.10 1.60	2 1000 1.10 1.27	2 600 1.10 1.04	400 1.10	2 400 1.10 1.06	2 150 1.10 1.16	2 90 1.10 1.11	2 50 1.10	16790 1.10 1.10	
	2 200 1.10 0.98 0.12	2 200 1.10 0.98	2 200 1.10 0.98	2 500 1.10 0.79	2 1000 1.10 0.50	2 1000 1.10 0.62	1000 1.10 0.75	2 1000 1.10 1.10 0.00	2 1000 1.10 1.48	1000 1.10 1.43 -0.33	2 1000 1.10 1.43	1000 1.10 1.48 -0.38	2 1000 1.10 0.63 0.47	2 1000 1.10 0.88 0.22	2 1000 1.10 1.43	2 1000 1.10 1.48 -0.38	2 1000 1.10 1.60	2 1000 1.10 1.27	2 600 1.10 1.04	2 400 1.10 1.15	2 400 1.10 1.06	2 150 1.10 1.16	2 90 1.10 1.11	2 50 1.10 1.10 0.00	16790 1.10 1.10	(Average)
	2 200 1.10 0.98 0.12	2 200 1.10 0.98 0.12	2 200 1.10 0.98 0.12	2 500 1.10 0.79 0.31	2 1000 1.10 0.50 0.60	2 1000 1.10 0.62 0.48	2 1000 1.10 0.75 0.35	2 1000 1.10 1.10 0.00	1000 1.10 1.48 -0.38	1000 1.10 1.43 -0.33	2 1000 1.10 1.43 -0.33	1000 1.10 1.48 -0.38	2 1000 1.10 0.63 0.47	2 1000 1.10 0.88 0.22	2 1000 1.10 1.43 -0.33	2 1000 1.10 1.48 -0.38	2 1000 1.10 1.60 -0.50	1000 1.10 1.27 -0.17	2 600 1.10 1.04 0.06	400 1.10 1.15 -0.05	2 400 1.10 1.06 0.04	2 150 1.10 1.16 -0.06	2 90 1.10 1.11 -0.01 0.00	2 50 1.10 1.10 0.00	16790 1.10 1.10 0.00	(Average)

			1
			•
(主)医疗 (1) (注) 医精神性炎 医二氏	APPENDIX-5 Prelimi	nary Daeian	
	WLLFIADIV-2 LIGHT	mary besign	
		en afrikasje kritigion i tropinski	
		医甲基氯化物 医电子性多种	4. 大學的 100 mm (100 mm)
그 강하다 옷 그냥 하는 얼룩하다.			
그 공항 지역한 경상을 반으면 원모들이			
그 말을 하셨다. 공급하셨다고 있다면 되고요.			
	医的医性衰竭 医电影电影 化二十二		
	· 회의학생학자는 시험 등으로 구인됩니	的复数美国 医海巴氏病 电电流	

APPENDIX 5 Preliminary Design

Contents

		<u>Page</u>
A5.1	Stability Calculation of Intake Dam	AP-5-1
A5.2	Stress Analysis of Culvert	AP-5-7
A5.3	Stress Analysis of Internal Water Pressure of Headrace Tunnel	AP-5-13
A5.4	Bearing Capacity of Penstock Foundation	AP-5-14
A5.5	Calculation of FRP Pipe Strength	AP-5-15
A5.6	Calculation of Combined Angle of Penstock	AP-5-22
A5.7	Calculation of Water Hammer	AP-5-24
A5.8	Calculation of Wall Thickness of Penstock Steel Pipe	AP-5-30
A5.9	Stability Calculation of Anchor Block	AP-5-35
A5.10	Records of FRP(M) Pipe Construction Works in Japan .	AP-5-46
A5.11	Comparison of Construction Costs of FRP Pipe and Steel Pipe	AP-5-48
A5.12	Calculation of Accommodation Capacity of Spoil Bank .	AP-5-49
A5.13	Calculation of Head Loss	AP-5-50
A5.14	Structures of T-Joint and Sleeve Joint	AP-5-56

A5.1 Stability Calculation of Intake Dam

The stability calculation of the dam was conducted for the following two cases concerning its overflowing section.

Water Level

Case

Earthquake

1. At Design Flood

Normal Condition

2. At Normal Full Water Level Under Earthquake K = 0.02 to down-

stream direction

(a) Stability Calculation

The dam stability calculation are performed concerning turn-over, slide and bearing force.

i) Stability Study Concerning Turn-Over

 $X_0 = \Sigma M/\Sigma V$

 $e = X_0 - (B/2 - B_1) \le e_a = B/6$

Where: X_o ; position at which composite force is applied

(distance from dam axis) (m)

e ; distance of eccentricity (m)

B ; width of foundation

 \mathtt{B}_1 ; distance from dam axis to upstream end of

foundation (m)

e, ; Middle Third (m)

ii) Stability Calculation Concerning Sliding

The stability condition against sliding is that the shearing friction safety factor is 4 or more.

 $N = (\tau_a B + f \cdot \Sigma V) / H \ge 4$

Where: N ; safety factor against sliding

 τ_a ; shearing strength between concrete and foundation

rock (t/m²)

f ; friction coefficient between concrete and

foundation rock

H ; total horizontal force acting on the dam (t)

iii) Stability Study Concerning Bearing Force

The stability condition concerning bearing force is studied by examining the allowable bearing force of dam and foundation rock.

 $P_1 = \Sigma V/B(1 + 6 \cdot e/B) \le \sigma_a$

 $P_2 = \Sigma V/B(1 + 6 \cdot e/B) \le \sigma_a$

Where: P_{12} : reaction force of foundation rock against vertical force (t/m^2)

 σ_a : allowable bearing force of foundation rock (t/m²)

(b) Design Condition

i) Dam Dimensions

Dam Crest Elevation EL 468.000 (m) Dam Height 6.200 (m) Dam Foundation Elevation EL 462.000 (m) Crest Width b = 2.000 (m) Slope of Upstream Side 1:0.00 Slope of Downstream side 1:0.80

ii) Regulating Pondage Dimensions

Design Flood Water Level WL. 472,370 (m)

Normal Full water Level WL. 469,000 (m)

Downstream Water Level (with flood design water level) WL

465.800 (m)

Downstream Water Level (with normal full water level) WL

462.000 (m)

Sediment Level EL 469.00 (m) Wave Height hW = 0.000 (m)

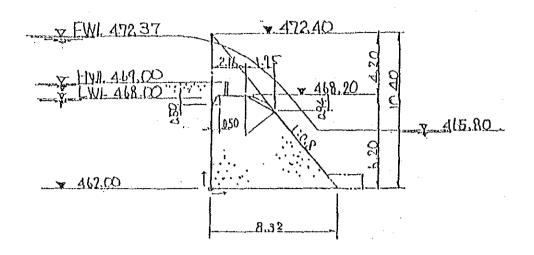
With no drain hole

iii) Unit Weight

Concrete $\gamma c = 2.30 \text{ (t/m}^3)$ Sediments $\gamma d = 1.20 \text{ (t/m}^3)$

iv) Coefficients

Sediment Pressure Coefficient Ce = 0.50 Friction Coefficient between f = 0.70 Concrete and Foundation Rock To = 50.0 (t/m^2) Allowable Bearing Capacity of Foundation Rock Ra = 100.0 (t/m^2) Design Seismic Intensity Kh = 0.02



(c) Calculation

i) At Design Flood (under Normal Conditions)

[1] EL 462.000 (m)

Iten	(t)	H (m)	X (m)	Y (m)	(m)	
Dead Load		81.684	0.000	3.104	0.000	253.533
Water Pressure		12.000	45.136	3.000	2.660	156.061
Sediment Pressure		0.000	14.700	0.000	2,333	34.300
Upstream Surface	Water Weight	0.000	0.000	0.000	0.000	0.000
Upstream Surface	Sediment Weight	0.000	0.000	0.000	0.000	0.000
Downstream Surface	Water Weight	5.776	0.000	7.307	0.000	42.203
Downstream Surface	Sediment Weight	0.000	-7.220	0.000	1.267	-9.145
Uplift Force		-40.740	0.000	3.849	0.000	-156.827
Tota	58.720	52.616			320.126	

[2] EL 463.000 (m)

Iter	V (t)	H (m)	X (m)	Y (m)	M (m)	
Dead Load		63.468	0.000	2.857	0.000	181.336
Water Pressure		12.000	32.256	3.000	2.268	115.949
Sediment Pressure		0.000	10.800	0.000	2.000	21.600
Upstream Surface	Water Weight	0.000	0.000	0.000	0,000	0.000
Upstream Surface	Sediment Weight	0.000	0.000	0.000	0.000	0.000
Downstream Surface	Water Weight	3.136	0.000	6.773	0.000	21.241
Downstream Surface	Sediment Weight	0,000	-3.920	0.000	0.933	-3.659
Uplift Force		-29.303	0.000	3.407	0.000	-99.843
Tota	1	49.301	42.136		for cut deal Dis. pag pet 45 HT proj	236.625

[3] EL 464.000 (m)

Item	(t)	(m)	X (m)	(m)	M (m)
Dead Load	47.092	0.000	2.611	0.000	122.977
Water Pressure	12.000	26.376	3.000	1.866	85.216
Sediment Pressure	0.000	7.500	0.000	1.677	12.500
Upstream Surface Water	Weight 0.000	0.000	0.000	0.000	0.000
Upstream Surface Sedime	ent Weight 0.000	0.000	0.000	0.000	0.000
Downstream Surface Water	Weight 01.296	0.000	6.240	0.000	8.087
Downstream Surface Sedimo	ent Weight 0.000	-1.620	0.000	0.600	-0.972
Uplift Force	-19.466	0.000	2.936	0.000	-57.150
Total	40.923	32.256			170.657

EL (m)	V (t)	H (t)	X (m)	(m)	M (t·m)	N	B (m)	P1 (t/m²)	P2 (t/m²)
462.00	58.720	52.161	320.126	5.452	1.292	8.688	8.320	13.632	0.483
463.00	49.301	42.136	236.526	4.800	1.040	9.742	7.520	11.994	1.118
464.00	40.923	32.256	170.657	4.170	0.810	11.305	6.720	10.495	1.684

ii) At Normal Full Water Level (under Earthquake)

[1] EL 462.000 (m)

Item	γ (t)	H (m)	(m)	Y (m)	(m)
Dead Load	81.684	1.634	3.104	2.598	257.777
Water Pressure	0.000	24.180	0.000	2.279	55.097
Hydraulic Pressure	0.000	0.550	0.000	2.651	1.457
Sediment Pressure	0.000	14.700	0.000	2.333	34.300
Upstream Surface Water Weight	0.000	0.000	0.000	0.000	0.000
Upstream Surface Sediment Weight	0.000	0.000	0.000	0.000	0.000
Uplift Force	-9.707	0.000	2.773	0.000	-26.920
Total	71.978	41.063			321.711

[2] EL 463.000 (m)

Item	γ (t)	Н (m)	X (m)	Y (m)	M (m)
Dead Load	63.468	1.269	2.857	2.202	181.336
Water Pressure	0.000	17.680	0.000	1.937	34.251
Hydraulic Pressure	0.000	0.432	0.000	2.240	0.967
Sediment Pressure	0.000	10.800	0.000	2.000	21.600
Upstream Surface Water Weight	0.000	0.000	0.000	0.000	0.000
Upstream Surface Sediment Weight	0.000	0.000	0.000	0.000	0.000
Uplift Force	-7.520	0.000	2.507	0.000	-18.850
Total	55.948	30.181			222.100

[3] EL 464.000 (m)

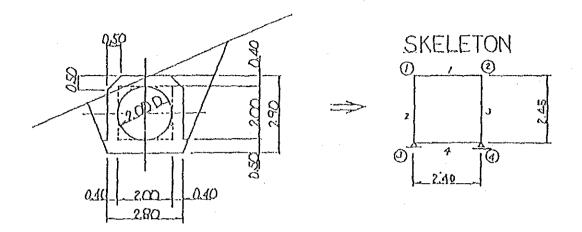
Iten	V (t)	H (m)	X (m)	Y (m)	M (m)	
Dead Load	305643465	47.092	0.942	2.611	1.798	124.670
Water Pressure		0.000	12.180	0.000	1.593	19.404
Hydraulic Pressure		0.000	0.323	0.000	1.828	0.590
Sediment Pressure		0.000	7.500	0.000	1.667	12.500
Upstream Surface	Water Weight	0.000	0.000	0.000	0.000	0.000
Upstream Surface	Sediment Weight	1.296	0.000	0.000	0.000	0.000
Uplift Force		-5.600	0.000	2.240	0.000	-12.544
Tota	41.492	20.945			144.620	

EL (m)	V (t)	H (t)	X (m)	Y (m)	M (t·m)	N	B (m)	P1 (t/m ²)	P2 (t/m²)
462.00	71.978	41.063	321.711	4.470	0.310	11.358	8.320	10.583	6.720
463.00	55.948	30.181	222.100	3.970	0.210	13.756	7.520	8.685	6.195
464.00	41.492	20.945	144.620	3.485	0.125	17.429	6.720	6.866	5.483

A5-2 Stress Analysis of Culvert

The stress is analyzed concerning the external water pressure in river flood and internal water pressure of the channel.

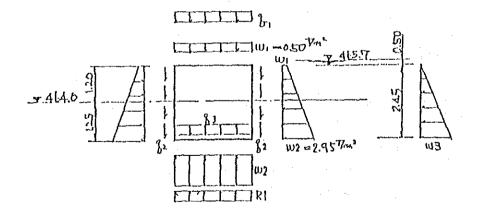
- (1) Analysis Concerning External Water Pressure At River Flood
 - (a) The stress is calculated by assuming the cross section of the channel as a rahmen structure having the skeleton given in the figures below.



(b) Load

The river water level of 465.7 m at the most upstream section of the culvert, which is reached at the time of 1/200 flood with 840 $\rm m^3/s$ is assumed to be the external water pressure.

External water pressure: $W_1 = 0.50 \text{ t/m}^2$, $W_2 = 2.95 \text{ t/m}^2$



Dead-weight:
$$q_1 = q_2 = 0.40 \text{ m} \times 2.4 \text{ t/m}^3 = 0.96 \text{ t/m}^2$$

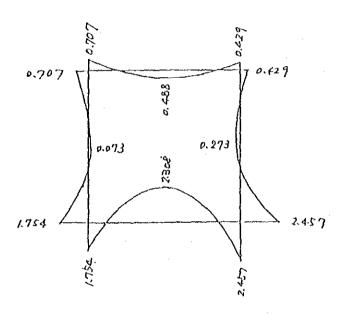
 $q_3 = 0.50 \text{ m} \times 2.4 \text{ t/m}^3 = 1.20 \text{ t/m}^2$

Reaction force: $R_1 = 0.96 \text{ t/m}^2 \times 2.45 \text{ m} \times 2/2.4 \text{ m} + 0.96 \text{ t/m}^2 + 1.20 \text{ t/m}^2 = 4.12 \text{ t/m}^2$

Earth pressure: $W_3 = 0.5 \times 0.8 \text{ t/m}^3 \times 2.45 \text{ m} = 0.98 \text{ t/m}^2$ (Static earth pressure)

(c) Calculation Result

MOMENT DIAGRAM



Stress Calculation for Rectangular Cross Section

Cross Secti	on No.	1. (Inside)	2. (Outside)
М	[t•m]	2.31	2.46
И	[t]	0.00	0.00
S	{t}	0.00	7.64
b	[cm]	100.0	100.0
h	[cm]	50.0	50.0
d	[cm]	43.0	43.0
d'	[cm]	7.0	7.0
As	$[cm^2]$	11.500 D19@250	15.500 D22@250
As'	$[cm^2]$	15.500	11.500
n=Es/Ec		15.00	15.00 Ec=220,000 kg/cm ²
P=As/(b x d)	[%]	0.267	0.360
u=d-h/2	[cm]	18.000	18.000
f=M/N+u	[cm]	18.000	18.000
f/d		0.419	0.419
a, / a	•	0.163	0.163
As' / As		1.348	0.742
M' =M + N × u	[t•m]	2.308	2.457
Х	[cm]	9.996	11.458
С		8.278	7.437
S		27.331	20.472
z		1.096	1.107
$\sigma_{ m c}$	[kg/cm ²]	10.3	9.9
σ_{s}	[kg/cm ²]	511.7	408.1
σ',	[kg/cm ²]	46.5	57.7
τ	[kg/cm ²]	0.00	1.97
$ au_m$	[kg/cm ²]	0.00	1.78
σ_{ea}	$[kg/cm^2]$	70.0	70.0
σ_{sa}	[kg/cm ²]	1,800.0	1,800.0
$ au_{\mathbf{a}}$	[kg/cm ²]	4.00	4.00

Note: The negative sign of σ_s signifies compression. The negative sign of σ'_s signifies tension. τ_m signifies the average unit shearing stress.

- (2) Analysis for Internal Pressure
- (2)-1 Check of Stress in Lining Concrete of Culvert
 - * The calculation is performed by assuming the culvert to be a cylindrical culvert having internal diameter of 2 m and concrete thickness of 0.4 m.
 - * The circumferential stress σ of a cylindrical culvert having internal diameter of γ and concrete thickness of δ is expressed by the following equation.

$$\sigma = \frac{P}{\rho^2 - 1} \left(1 + \sigma^2 \frac{{\gamma_i}^2}{{\gamma^2}} \right) (kg/cm^2)$$

Here, the internal water pressure is: $470 - 461.5 + 1.63 = 10.13 \text{ m/m}^2 = 1.013 \text{ kg/cm}^2$ (static pressure)(water hammer)

Letting γ be the distance from the center of the cylinder to the cross section to be analyzed (m), and

$$\rho = \frac{\gamma_i + \delta}{\gamma_i} = \frac{1 + 0.40}{1} = 1.40$$

The stress σ_1 on the internal surface of concrete is, as $\gamma = \gamma_1$,

$$\sigma_1 = \frac{p}{\rho^2 - 1} (1 + \rho^2) = P \cdot \frac{1.40^2 + 1}{1.40^2 - 1} = 3.08 \text{ kg/cm}^2$$

The stress δ_2 on the outside surface of concrete is, as $\gamma = \gamma_1 + \delta$,

$$\sigma_2 = \frac{P}{\rho^2 - 1} (1 + 1) = 1.0136 \times \frac{2}{1.40^2 - 1} = 2.11 \, kg/cm^2$$

The allowable tensile stress can be regarded as $\sigma_{\rm st}$ = 10 kg/cm² when concrete rich in cement is placed at the site and sufficiently compacted.

Therefore, it is safe with $\sigma_{1,2} < 10 \text{ kg/cm}^2$

Reinforcing steel bars of 19 mm diameter will be placed circumferentially in double layer on the inside and outside surface with 25 cm intervals, to improve strength and prevent crack.

The effect of reinforcement per 1 cm is:

$$\lambda = \frac{\delta}{\delta + 8a_{st}} = \frac{40}{40 + 8 \times 11.46} = 0.303$$

 $\lambda \sigma_1 = 0.303 \times 3.08 = 0.933 \text{ kg/cm}^2$

 $\langle \delta_{cr} 10 kg/cm^2$

 $\lambda \sigma_2 = 0.303 \times 2.11 = 0.639 \text{ kg/cm}^2$

(2)-2 Check of Stress Based on Assumption that All Water Pressure is Borne by Steel Reinforcing Bars

The steel reinforcing bars are regarded as thin cylinders to calculate the stress of steel bars, $\sigma_{\rm t}$ (kg/cm²).

$$\sigma_{\mathbf{t}} = \frac{P \cdot D}{2 \cdot t}$$

P: internal water pressure, 1.013 kg

D: inside diameter to steel bars, 214 cm

t: thickness of thin plate equivalent to bar cross section (cm)

Assuming the steel bar of 190250, $\frac{2.865 \text{ cm}^2 \times 4}{100 \text{ cm}} = 0.1146 \text{ cm}^2/\text{cm}$

Therefore, $\sigma_t = \frac{1.013 \times 214}{2 \times 0.1146} = 946 \ kg/cm^2 < \sigma_{ta} = 1,800 \ kg/cm^2$

A5.3 Stress Analysis of Internal Water Pressure of Headrace Tunnel

The analysis of the stress caused by the tunnel internal water pressure has been performed by Ott-Frey-Bear's equation.

The water pressure at the most downstream part of the lined section of the tunnel, where the internal pressure (static pressure plus water hammer pressure) becomes maximum, of $P = 2.439 \text{ kg/cm}^2$ was used to calculated the tensile stress of the reinforcing steel bars. In the analysis, the following Figure has been utilized.

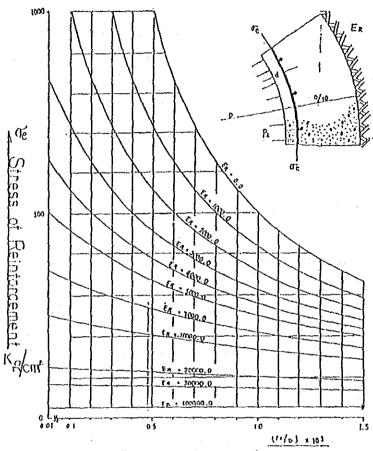
When the steel reinforcing bars, D19, are installed with interval of 30 cm, the amount of steel bars in the circumferential direction;

Fe = $0.0955 \text{ cm}^2/\text{cm}$ (Fe/D) x 10^3 = (0.0955/200) x 10^3 = 0.4775where D = internal diameter of 200 cm.

* Assuming the modulus of elasticity of the surrounding rocks, Er, to be 10,000 kg/cm²,

The unit stress of the steel bar, $\sigma e = 200 \text{ kg/cm}^2$ Therefore, the unit stress of the real steel bars; $\sigma_{el} = 200 \text{ x } 2.439 = 488 \text{ kg/cm}^2 < \sigma_{ta} = 1,800 \text{ kg/cm}^2$

Er = 30,000 kg/cm² --> σ_e = 80 kg/cm² σ_{e1} = 80 x 2.439 = 195 kg/cm² < σ_{ta} = 1,800 kg/cm²



Water Pressure: Pi = 1 kg/cm²

Fe: Area of Reinforcement (cm/cm)

D: Tunnel Dia. (om)

A5.4 Bearing Capacity of Penstock Foundation

According to the penetration test of (III-2), the N-value down the depth of 2.3 m is from 5 to 18, and its average value is 11.

In the construction work, the talus pile will be excavated and removed for the depth below this range, the penstock is installed on a continuous concrete foundation, and soil will be filled back on the penstock for a depth of more than 1 m. Therefore, the foundation will be installed at depth which is more

than 2 m below the ground surface. The ultimate bearing capacity of continuous footing, $q_{\rm d}$, is given by the following equation according to Terzaki and Peck.

$$q_d = \alpha \cdot \beta \cdot \gamma \cdot N_r + D_t \cdot \gamma \cdot N_q$$

- α = coefficient determined by the geometry of the loading surface of the foundation; 0.5 for continuous foundation
- β = width of footing; 1.5 m
- γ = unit weight of soil; 1.8 t/m³
- $D_r =$ setting depth of footing; 2.5 m

For the value of N = 11, $N_r = 7$, $N_q = 10$ (with internal friction angle of 30°) is obtained from the Related Graph (Omission).

From these values, we obtain the ultimate bearing capacity of approximately 55 t/m^2 . Taking a safety factor of 3, the allowable bearing capacity is 18 t/m^2 . In this case, the load on the foundation is no more than 5 t/m^2 , and sufficient safety is assured.

A5.5 Calculation of FRP Pipe Strength

- (a) Load
 - i) Maximum design head = High water level 469.0 m + Flood water level 1.0 m Minimum elevation of penstock 305.2 m = 164.8 m Maximum water hammer head = (469.0 + 1.0 water turbine nozzle center elevation 306.7 m) x 0.1 = 16.3 m Maximum design head = 181.1 m

 - iii) Bulldozer load: 3 ton class, 1 bulldozer
 - iv) Weight of unit volume of earth: 0.0018 kg/cm4
 - v) Passive earth pressure resistance coefficient: 14 kg/cm2

*1. (Reference)

Earth Cover on the Buried Section

It is possible that the ground water level rises to the ground surface due to very heavy rain. The pipe must be buried in such a depth that the pipe does not float up when the ground water rises. The minimum earth cover, H (m), with which the FRP pipe does not float up, is calculated by the following equation.

$$H \geq \frac{\pi D_c}{4} \cdot \frac{S - \left\{1 - \left(\frac{D}{D_c}\right)^2\right\} \gamma_p}{w}$$

S: safety factor of 1.2

 $D_{\rm c}$: pipe outside diameter; 0.936 and 1.144 m

D: pipe inside diameter; 0.9 m and 1.1 m

 γ_n : weight per unit volume of pipe material; 1.8 t/m³

W: weight per unit volume of filled back earth which is saturated; 1.8 t/m^3

D = 0.9 m ----> H = 0.98 m =>> 1.0 m

D = 1.1 m ----> H = 1.19 m ==> 1.2 m

(The required depth of earth cover.)

(b) Strength Calculation

i) Pipe Structure

The structure and dimensions of FRP pipe are given below.

Outer Protective Layer t

Inner Protective Layer D

	D	t		
	mm			
FRP Pipe	900	. 18		
į	1100	22		

The pipe thickness is the value set forth in JIS A 5350. No marginal wall thickness is provided.

ii) Strength and Cross Section Characteristics of Pipe

ii-1 Pipe Strength

ii-1-1 Pipe Strength

The strength of the FRP layer is illustrated below.

kg/cm²

	:	FRP Pipe			
Classification	Symbol	Circumferential Direction	Axial Direction		
Modulus of Elasticity	Ej	181300	90510		
Tensile Strength	σt	3780	1641		
Compression Strength	ac	3333	1389		
Shearing Strength	τ	550			

ii-1-2 Allowable Stress of Pipe

The allowable stress of the FRP layer is the fracture strength of FRP divided by the safety factor of 6.

The allowable stress values of the FRP layer is given below.