Geology

The three large rivers of the Ganges, Brahmapura and Meguna developed an extreme sediment accumulation which is called the Great Bengal Delta. The alluvial stratum consists of loose to medium silt and clay deposits ranging around 50 m deep.

Earthquake

Bangladesh is not an earthquake prone area. No disastrous earthquakes have been recorded except for very local earthquakes in 1885, 1897 and 1918.

(2) Socio-economic Conditions

Population

The land area, population and population density of the 15 districts covered by the project are presented in Table 3-3. The major indicators of socio-economic conditions (as of 1988) are as follows:

- Population growth rate: 2.03%

Literacy rate : 24.8%

- Urban population rate: 10.7%

- Life expectancy : 56.1 years

- School attendance rate

Primary school : 94.6%

Middle school : 27.6%

High school : 4.2%

- Population below poverty line

Urban : 10.5 mil. (44%)

Rural : 40.5 mil. (48%)

Table 3-3 LAND AREA, POPULATION AND POPULATION DENSITY IN THE PROJECT AREA

(As of 1991)

District	Area (sq.km)	Population (thousand)	Population Density (persons/sq.km)
Cox's Bazar	2,242	1,465	653
Feni	984	1,156	1,175
Chittagong	5,213	5,730	1,099
Habigonj	2,514	1,566	623
Moulvibazar	3,164	1,446	457
Chandpur	1,655	2,140	1,293
B. Baria	1,875	2,140	1,205
Comilla	3,069	4,254	1,386
Noakhali	3,053	2,346	768
Laxmipur	1,421	1,383	973
Narsingdi	1,116	1,708	1,530
Faridpur	2,082	1,552	745
Dhaka	1,468	6,146	4,187
Munshigonj	986	1,222	1,239
Gazipiur	1,820	1,703	935
Whole country (64 dist.)	148,393	109,877	740

Source: Statistic Pocket Book of Bangladesh 92

Industry

The project area's economy is largely dominated by agriculture. Major agricultural products are rice, jute, wheat and sugar cane. Jute is widely cultivated as the second crop of rice field. The jute industry earns 60 % of the total exports of the country. Cereal production has been increasing steadily but is still short of domestic consumption.

Heavy, medium and light industry are located around Chittagong which is the second largest city and the international sea port. Printing, paper mill, fertilizer, cement, steel and other production factories are located there.

(3) Roads conditions

Regarding Type-B feeder roads and rural roads, road length by surface condition and by district is presented in Table 3-4. The length of structures by condition is also presented in the table.

Only a few sections of Type-B feeder roads and rural roads have been paved as mentioned in the table. Unpaved roads in the project area become muddy in the rainy season since the earth here is clayey. It is hard to drive over rough and narrow road in more than half of the rural roads even in the dry season. Where numerous gaps cut the roads, travelers cross rivers by boat or other means which are inconvenient and unsafe.

Table 3-4 TYPE-B FEEDER ROAD AND RURAL ROADS LENGTH BY SURFACE CONDITION IN THE PROJECT AREA

									יינינים יינים ואינים	•						
	Dietrict	Total		Surface Type	e Type		Sr	rface	Surface Condition				Existing		Structure Condition	tion
			Paved (km)	88 884	Earthen Ro) (%)	(%) (mx)	8	Poor (km)	(%)	Good (km)	8	Poor (Km)	8	No. of Gap	Total Length of Gap
-	Cox's Bazar	29.7	5,0	17	24.7	83	0.0	0	29.7	100	34.9	1,7	4.64	55	72	108.3
'n	Feni	8.61	0.0	۵	16.8	100	0.0	0	16.8	ã	6.44	25	36.9	45	3	27.9
m	Chittagong	0.39	37.9	0.7	57.1	9	10.5	1.1	84.5	8	283.6	71	117.8	ઇ	25	204.2
7	Habigonj	258.1	29.5	11	228.9	68	20.3	60	237.8	26	980.0	89	451.2	32	112	1,390.2
ın	Moutvibazar	125.3	30.5	54	8.46	9/	3.5	ĸ	121.8	26	380.5	3	4.094	'n	82	655.7
ø	Chandpur	154.4	8.7	9	145.7	76	51.9	14	132.5	88	683.4	\$	388.8	36	58	349.6
^	B. Baria	124.6	0.0	co	124.6	100	11.3	6	113.3	6	596.7	88	81.8	12	22	164.2
ω	Comilla	274.9	8,3	n	210.6	1	3.9	-	271.0	8	1,060.4	69	0.084	m	120	7.489
٥	Noakhalí	5.79	8,1	ထ	7.08	26	25.1	92	72.4	7/2	198.4	53	174.5	27	17	42.8
9	Laxmipur	2.44	18,3	4	25.9	59	10.3	23	33.9	22	8.99	32	142.2	89	7	7.81
Ξ	Narsingdi	76.1	31.6	ĸ	64.5	29	26.5	28	9.69	2	167.8	29	83.8	K	20	182.9
72	Faridpur	259.3	24,5	ES.	174.8	29	84.5	33	174.8	29	1,858.4	82	414.3	18	33	462.0
ŭ	Dhaka	82.6	54.9	30	57.7	22	12.6	15	70.0	85	107.1	63	62.1	37	83	361.4
2	Munshigonj	9758	11.4	13	74.2	87	0.0	0	85.6	100	312.7	87	335.5	25	27	7.505
5	Gazipur	£.751	56.4	*	110.9	99	53.1	32	114.2	88	507.7	15	2.967	67	23	9.78!
_	Total	1,911.3	410.7	21	1,500.6	5/	283.5	15	1,627.8	85	7,783.3	29	3,775.0	33	243	5,342.0
							-	Rural R	Road (R1, R	2, R3)						
	1	Total		Surfac	Surface Type		3	Surface	Condition				Existing	1	Structure Condition	ition
	מומרו		Paved (Km)	Road,	Earthen R (km)	(S	(%) (wx)	%	(km)	(%)	Good (km)	8	Poor (km)	89	No. of Gap	Total Length of Gap
	. Cox's Bazar	1,142.9	46.3	7	1,096.6	96	215.2	δl	7.726	81	1,332.0	50	1,341.3	50	595	3,102.5
N	Feni	1,271.2	21.0	~	1,250.2	86	23.2	2	1,248.0	86	1,358.5	34	2,650.9	99	455	1,444.8
m	. Chittagong	4,427.1	232.3	5	4,194.8	95	94.2	2	4,332.9	86	7,989.6	75	7,011.6	58	1,384	4,662.3
4	Habigonj	1,215.7	19.0	2	1,196.7	98	37.0	23	1,178.7	26	1,191.4	54	9.666	97	212	5,074.9
5	Moulvibazar	851.0	0.0	0	851.0	100	70.7	2	830.6	86	1,317.4	20	1,319.8	- 20	255	2,182.4
9	Chandpur	2.780,1	5.5	-	1,079.2	66	78.1	7	1,306.6	93	1,575.1	25	1,762.3	53	319	2,496.1
۲.	B. Baria	9.0%	13.5	-	927.1	86	26.7	2	913.9	26	1,527.9	65	1,083.4	15	102	2,510.9
æ	. Comilia	7,536.3	58.1	-	4,478.2	8	7.8	0	4,527.9	100	6,373.9	52	5,210.8	67	788'2	12,279.3
٥	Noakhali	3,282.6	39.3	-	3,243.3	8	2 29	2	3,214.9	86	7 277 2	27	3, 194, 2	22	196	3,673.7
2	Laxmipur	1,471.0	26.2	2	1,444.8	86	73.1	5	1,397.9	95	1,727,1	97	2,005.5	24		1,691.6
11	Narsingdi	6, 280,1	20.4	2	1,065.5	98	100.3	6	985.6	91	1,233.7	75	1,692.4	28	:	1,142.6
32	Faridpur	6.418,1	56.9	1	1,788.0	8	20.2	1	1,794.7	8	1,723.0	26	1,357.4	477		4,584.7
M		1,502.6	52.7	7	1,449.9	96	80.8	ζ.	1,421.8	95	1,526.5	67	1,581.8	51		4,770.2
14.	Munshigonj	745.4	3.9		741.5	8	11.5	7	733.9	98	1,133.3	23	3,689.7	. 77	169	3,135.6
ĸ	. Gazipur	681.6	6-1-5	9	2.629	76	41.5	9	1.049	76	9.218	79	451.4	35		1,212.7
_	Total	26,053.5	607.0	~	25,446.5	86	898.3	M	25, 155, 2	97	30, 267, 4	7,6	35,352.1	7,	9.891	53.964.3

Source : Inventory of Road & Road Structure of FRB & Rural Roads, 1993, LGED

3.3 STUDY AND EXAMINATION OF THE REQUEST

3.3.1 Appropriateness and Necessity of the Project

(1) Investigation of request bridge sites

To examine the appropriateness and necessity of the project under Japan's Grant Aid, the Study Team investigated the 83 requested bridge sites to collect the necessary data.

Summary of site data of requested bridges are presented in Table 3-5.

The survey items of the investigation were as follows:

- Present condition of bridges (bridge type, length, condition)
- Socio-economic conditions in affected areas (population, landuse, industry)
- Approach road conditions (road class, width, surface type, surface condition)
- Traffic volume (ADT) by type
- Engineering data (topography, geology, river condition, construction condition)
- Recommendations for bridge planning (bridge length, height)

(2) Selection of high priority bridges

Based on the results of site investigation, the socio-economic effects, appropriateness and necessity of construction of each request bridge were evaluated, and high priority bridges were selected. The evaluation criteria are as follows:

Socio-economic effects

Evaluation items are:

- Transportation conditions to be improved
- Traffic volume
- Population and area to have benefit
- Contribution to socio-economic activities

Engineering necessity

Evaluation items are:

- Deterioration of bridge
- Inconvenience and obstacle for transportation
- Hazardousness for passengers
- Existence of detour

Appropriateness of project

Evaluation items are:

- Urgency and necessity of implementation
- Population and area to have benefit
- Improvement of transportation and living standard
- Availability of maintenance

The evaluation of each bridge project is as shown in Table 3-5. As a result, 79 bridges were evaluated as having high priority. The other 4 bridges were evaluated as having low priority and excluded from the project. The low priority bridges have less necessity of implementation since their present condition is reparable damage only.

Socio-economic effects, engineering necessity and appropriateness of the project as a whole are summarized as follows:

Socio-economic effects

Since the roads on which project bridges are located connect villages, farms and markets in agricultural areas, construction of bridges contributes to activate farming and production of marketable products. A population of 36,076,000 in 32,662 sq.km is

expected to have direct and indirect benefits by construction of the bridges.

Engineering necessity

The bridges for the project have an urgent necessity to be constructed to provide safe and reliable traffic facilities across rivers. The present conditions of the bridges are:

- Washed out or collapsed by flood, which cut roads.
- Dilapidated or seriously deteriorated, which are too weak to use.
- Not yet constructed, where travelers cross by boat.

Appropriateness

- Urgency and necessity of the project is very high.
- Implementation of the project benefits a great number of ordinary citizens.
- Providing safe and reliable transportation means promotes more employment opportunities and higher income for people in the project area.
- No difficulty in implementation of the project is foreseen.

Table 3-5 SITE DATA OF REQUESTED BRIDGES AND EVALUATION OF PRIORITY OF BRIDGES (1/4)

		Prese	nt Cond	ition o	f Bridge			Soci	o Econ	omic a	nd Appro	ach Roa	d Informa	ation	- : .					:	Engine	ering	Informati	on			Recon			tion o		
No	Bridge No.	Exist	Bridge	Bridge	Present	No. of Affect- ed	Land	Main	Road	Road	Road	Road Pre-	(1	Rainy	lume (AD Season)	r	Geo.	Topo	Dept	R h (m)	·	ydrogi h (m)	cal, Condi	tion River		Present Condi-	dat	tion	nomic t	00000		Remarks
		None	Туре	Length (m)	Condi- tion	Popula- tion (Thou.)		Prod- ucts	Class	Width	Surface Type	sent Condi- tion	Vehicle	Rick- shaw	Walking	Boat	Condi- tion	Condi- tion	LWL	T	LWL	HWL	Current Velocity (M/S)	Dike		tion of Access Road		Bridge Height (m)	Socio Ecc Effec	Urgency	Judgment	
1	01-01-01 COX'S BAZAR	Yes	Timber	52.0	Bad	30	Farm	Salt Shrimp	R1	3.0	Earthen	Bad	0	0	2,000	0	Flat	Clay	0.8	4.7	36.0	70.0		None	Fixed	Pass- able	70.0	7.0		}		• Fairway for small boats
2	01-02-01 COX'S BAZAR	None	-	-	-	30	Farm	Rice Veg.	FR8	7.0	Earthen	Fair	0	0	0	300	Flat	Sand	0.9	4.3	60.0	160.0		None	Fixed	Pass- able	120.0	5.0	Yes	Yes Ye	s Yes	
-	02-01-01 FENI	Yes		70.0	Col- lapsed	55	Farm	Rice	R1		Earthen	Fair	0	0	0	1,000	Flat	Clay	2.0	7.0	30.0	80.0	0.3	None	Fixed	Pass- able	70.0	8.0	Yes	Yes Ye	s Yes	• Irrigation canal
L	02-02-01 FENI	Yes	RC	26.7	Bad	45	Farm	Rice	R2		Earthen	Fair	0	100	2,000	0	Flat	Clay	1.0		9.0		0.3	Yes	Fixed	Pass- able	40.0	5.5	Yes	Yes Ye	s Yes	• Irrigation canal
_	02-02-02 FENI	Yes	RC	27.5		60	Farm	Rice	R2		Earthen	Fair	0	200	2,500	0	Flat	Silt	1.0		8.0		0.4	Yes	Fixed	Pass- able	40.0	6.0		Yes Ye	s Yes	
6	03-01-01 CHITTAGONG		Balley	27.5		50	Farm	Veg.	FR8		Earthen	Fair	50	290	3,000	0	Flat	Silt	8.0				0.6	None	Fixed	Pass- able	28.0	5.4		No Ye		No need reconstruction
7	03-01-02 CHITTAGONG		Balley	27.5	Damaged		Farm	Rice Veg.	FRB		Earthen	Fair	50	250	3,000		Flat	Silt	0.9			36.0		None	Fixed	Pass- able	36.0	7.5		No Ye	s No	Damage is reparable No need reconstruction
	03-02-01 CHITTAGONG	None	-	-	-	50	Farm	Rice Veg.	FRB		Earthen	Fair	0	0	0	600		Sand	2.0				1	None	Fixed	Pass- able	80.0	7.5		Yes Ye	_	
	03-03-01 CHITTAGONG	None			-	30	Farm	Rice Veg.	R1	3.0	HBB	Good	0	60	0	300	Moun- tains	Clay		5.8	8.0		1.0	None	Fixed	Pass- able	70.0	6.3		Yes Ye		
	03-04-01 CHITTAGONG	None	-	-	-	60	Farm	Rice Veg.	FRB		Earthen	fair	0	0	0	500	Flat	Clay		5.7		35.0	1.0	None	Fixed	Pass- able	35.0			Yes Ye	<u> </u>	
	03-05-01 CHITTAGONG	Yes	RCDG		Settle- ment	50	Farm	Rice	FRB		Earthen	Poor	35	125	2,000	0	Flat	Sand				22.0		None	Fixed	Pass- able	22.0			Yes Ye	<u> </u>	
	03-06-01 CHITTAGONG		Steel	85.0	Çol- lapsed	25	Farm	Salt Rice	FRB		Earthen	Poor	0	0	0	1,000		Clay	. :	13.1	37.0			None	Fixed	Pass- able	95.0	14.0		Yes Ye	<u> </u>	·
	04-01-01 HABIGONJ	None	-		-	15		Rice	FR8	3.0	Earthen	Bad	0	0	0	200	Flat	Clay		10.5		Flood	0.9	None	Fixed	Pass- able	60.0	12.0		Yes Ye	- ·	
<u> </u>	04-02-01 HABIGONJ	None	-	-	- -		Farm	Rice	FRB	· ··	Earthen	Fair	0	0	0		Flat					84.0	.	None		able	80.0			Yes Ye	-	
	04-03-01 HABIGONJ	None	-	-	7	20	Farm	Rice			Earthen		0	0	0		Flat					Flood		None	Fixed	Pass- able	30.0			Yes Ye		
	04-04-01 HABIGONJ	None	-			10	farm				Earthen	, N	0	0	0		Flat	Clay				48.0		None	Fixed	Pass- able	48.0			Yes Ye		
	04-04-02 HABIGONJ	None	- :	-	-	10	Farm				Earthen		0	0	0	200	Flat	Clay				Flood	1.0	None	Fixed	Pass- able	50.0			Yes Ye		
	05-01-01 MOULVIBAZAR	Yes	RCBX		Settle- ment	15		Veg.			Earthen		0	250	1,000	0	flat			2.5		17.0		None	Fixed	Pass- able	20.0 35.0			Yes Yes	_	
	05-01-02 MOULVIBAZAR	Yes	RCDG /		Settle- ment		Farm -	Veg.			Earthen			<u> </u>	1,000		Roll- ing	Clay				30.0		None	Fixed	Pass- able	25.0	·		Yes Ye		
	05-02-01 MOULVIBAZAR	Yes	Steel	21.0				Veg.			Earthen	Fair		350			Roll- ing	Clay		,		21.0		Yes	Fixed	Pass-				Yes Ye		
	05-02-02 MOULVIBAZAR	Yes	Steel	32.0	Col- lapsed	40	Farm	Rice Veg.	FRB	6.0	Earthen	Fair	0	0	1,000	0	Roll- ing	Silt	1.0	4.3	20.0	32.0		Yes	Fixed	Pass- able	35.0	0.5	168	res Ye	sres	

Table 3-5 SITE DATA OF REQUESTED BRIDGES AND EVALUATION OF PRIORITY OF BRIDGES (2/4)

		Prese	nt Condi	ition o	f Bridge			Soci	o Econo	omic a	nd Approa	ach Roa	d Inform	ation	: .			:		E	ngine	ering	Informati	on	···		Reco	nmen-	Selec Prior	tion o	of Hi	gh s
No.	Bridge No.	1	Ĭ .			No. of Affect- ed		Main		Road		Road Pre-	(Rainy	lume (AD) Season)		Geo.	Topo	Dept	Ri h (m)		h (m)	cal Condi	River		Present Condi-		tion	onomic		ateness	Remarks
		None	Туре	Length (m)	Condi- tion	Popula- tion (Thou.)	Use	Prod- ucts	Class	Width	Surface Type	sent Condi- tion	Vehicle	Rick- shaw	Walking	Boat	Condi- tion	Condi- tion	LWL	HWL	LWL	HWL	Current Velocity (M/S)	Dike Exis- tence				Bridge Keight (m)	Socio Econ Effect	Urgency	Appropriater	nagamen
22	05-03-01 MOULVIBAZAR	None	-	<u> </u>	-	10	Farm	Rice Veg.	FRB	6.0	Earthen	Fair	0	0	0	500	Roll- ing	Clay	1.3	11.0	13.0	55.0	1.0	Yes	Fixed	Pass- able	55.0	12.5	Yes	Yes		95
23	05-04-01 MOULVIBAZAR	Yes	RCDG	10.3	Poor	15	Farm	Rice	FRB	4.5	Bit Seal	Fair	0	120	1,000	0	Flat	Clay	1.0	5.3	8.0	13.0		Yes	Fixed	Pass- able	20.0	7.0	Yes	Yes	es Y	
24	06-01-01 Chandopur	Yes	Bamboo	25.0	-	20	Farm	Rice	FRB	5.6	Earthen	Fair	0	0	1,000	0	Flat	Clay	1.0	5.8	12.0	25.0	0.8	None	Fixed	Pass- able	25.0	7.0	Yes	Yes \	es Y	es
25	06-02-01 CHANDOPUR	Yes	Bamboo	17.0		7	Farm	Rice	R2	3.0	Earthen	Fair	0	0	1,000	0	Flat	Clay	0.0	4.5	0.0	15.0	0.1	None	Fixed	Pass- able	15.0	5.0	Yes	Yes	es Y	38
26	06-03-01 CHANDOPUR	Yes	Bamboo	40.0	-	10	Farm	Rice	R1	2.4	Earthen	Fair	0	0	1,000	0	Flat	Clay	2.0	6.0	20.0	40.0	0.5	None	Fixed	Pass- able	40.0	8.0	Yes	Yes	es Y	28
27	06-03-02 CHANDOPUR	Yes	Валюоо	25.0	Poor	5	Farm	Rice	R1	2.0	Earthen	Fair	0	225	1,000	0	Flat	Clay	1.9	2.5	18.0	25.0	0.1	None	Fixed	Pass- able	25.0	3.0	Yes	Yes	es Y	38
28	06-04-01 CHANDOPUR	Yes	Bamboo	28.0	-	4	Farm	Rice	FRB	2.4	Earthen	Fair	0	0	500	0	Flat	Clay	3.0	5.0	28.0	30.0	0.1	None	Fixed	Pass- able	30.0	7.0	Yes	Yes	res Y	28
29	06-04-02 CHANDOPUR	Yes	Bamboo	27.0	•	5	Farm	Rice	FRB	2.3	Earthen	Fair	0	0	500	0	Flat	Clay	3.0	4.5	24.0	27.0	0.0	None	Fixed	Pass- able	25.0		Yes			
	07-01-01 B. BARIA	None		-	-	25	Farm	Rice Jute	FR8	6.0	Earthen	Fair	0	0	0	200	Flat	Clay	1.0	4.5			0.5	None		Pass- able	20.0		,	Yes		
31	07-01-02 B. BARIA	None	-	•	-	30	Farm	Rice Jute	FRB	6.0	Earthen	Fair	0	0	0	200	Flat	Clay	1.5	6.5	10.0	90.0	0.5	None	Fixed	Pass- able	85.0			Yes	-	
32	07-02-01 B. BARIA	None	-	•	-	20	Farm	Rice Jute	FRB	6.0	Earthen	fair	0	0	0	200	Flat	Clay	0.5			30.0	0.5	None	Fixed	Pass- able	30.0			Yes	\perp	
33	07-02-02 B. BARIA	Yes	Bamboo		Bad	20	Farm	Rice Jute	FŖB		Earthen	Fair	0	0	500		Flat	Clay	0.3		* ;	13.0	0.5	None		Pass- able	15.0		Yes			
34	07-03-01 8. BARIA	None	-	-	•	30	Farm	Rice Jute	R2		Earthen	Fair	0	0	0	200		Clay	1.5			60.0	2.0	None		Pass- able	60.0		Yes			
	07-03-02 B. BARIA	Yes	REDG		Settle- ment	25	Farm	Rice			Earthen		0	10	8 0 0	0	Flat	Silt				30.0	:		Fixed	able	30.0		Yes		_	
	07-04-01 B. BARIA	Yes	Bamboo	28.5	-		* * * *	Rice			Earthen		0	0	200		Flat			4.5		27.0			Fixed	Pass- able	25.0		Yes		_	
	08-01-01 COMILLA		Balley		lapsed			Veg.	-		Earthen		0	500	2,000	: :	Roll- ing				<u> </u>	24.0	1 i i i i	Yes	Fixed	able	25.0 25.0		Yes Yes		_	
-	08-01-02 COMILLA	Yes	RC Arch	22.0	Bad		Farm		R2		Earthen		50	250	2,500		Roll- ing					18.0			Fixed	able	37.0		Yes			
	08-01-03 COMILLA	Yes	RCDG	43.3	Poor		Farm		R2		Earthen		25	425	2,000		Flat								Fixed	able	30.0		Yes			
ļ	08-02-01 COMILLA		Timber	28.0	Bad		Farm				Earthen		0	40	2,000	14.	Flat								Fixed Fixed	able	25.0		Yes		_	
	08-02-02 COMILLA	Yes	RCDG	17.0	Poor		Farm		R2		Earthen		0	250	500		Flat					17.0				able	30.0		Yes			
42	08-03-01 COMILLA	Yes	Bamboo	26.0	-	6	Farm	Rice	R1	2.4	Earthen	Fair	0	0	200	0	Flat	Clay	0.0	3.7	0.0	28.0	0.2	None	Fixed	Pass- able	30.0	3.0	ies	ies	es	25

Table 3-5 SITE DATA OF REQUESTED BRIDGES AND EVALUATION OF PRIORITY OF BRIDGES (3/4)

		Preser	nt Cond	dition o	f Bridge			Soci	o Econ	omic a	nd Approa	nch Roas	d Inform	ation					·	E	ngine	ering	Informati	on ·			Recon	ımen-	Select Priori			
No.	Bridge No.	Exist	Bridge	Bridge	Present	No. of Affect-	Land	Main .	Road	Road		Road			lume (AD Season)	Γ)	Geo.	Topo					cal Condi	r		Present	dat	tion	omic	8 8 9 0		Remarks
	/Location	None	Туре	Length	Condi- tion	Popula- tion (Thou.)	Use	Prod- ucts	Class	Width	Surface	Pre- sent Condi- tion	Vehicle	Rick-	Walking	Boat	Condi- tion	Condi-	Depth LWL	HWL	Widt LWL	h (m) HWL	Current Velocity (M/S)	River Dike Exis- tence		Condi- tion of Access Road	Bridge Length (m)		iocio Ecor Effect	Urgency	Judgment	
1 4	08-03-02 COMILLA	None	-	-	-	5	Farm	Rice	R1	2.4	Earthen	Fair	0	0	0	200	Flat	. Clay	1.0	4.3	5.0	25.0	0.5	None	Fixed	Pass- able	25.0		Yes	Yes Ye		
	08-04-01 COMILLA	None	-	-	-	8	Farm	Rice	FRB	1.5	Earthen	Bad	0	0	- 0	200	Flat	Clay	0.0	4.5	0.0	25.0	0.5	None	Fixed	Pass- able	25.0	6.5	Yes	Yes Ye	s Yes	
	08-04-02 COMILLA	None	-		•	7	Farm	Rice	FRB	1.5	Earthen	Bad	0	0	0	200	Flat	Clay	1.0	5.0	4.5	30.0	0.5	None	Fixed	Pass- able	30.0	6.5	Yes	Yes Ye	s Yes	
	08-04-03 COMILLA	None	-	-	-	25	Farm	Rice	FRB	6.0	Earthen	Fair	0	0	0	200	Flat	Clay	2.0	6.0	45.0	60.0	0.7	None	Fixed	Pess- able	65.0	8.0	Yes	Yes Ye	s Yes	:
	09-01-01 NOAKHALI	Yes	RC	45.2		90	Farm		R1		Earthen	Fair	0	200	500	. 0	flat	Clay	1.2	4.0	25.0	60.0	0.4	None	Fixed	Pass- able	50.0	5.3	Yes	Yes Ye	s Yes	
	09-01-02 Noakhali	Yes	Timber	<u> </u>		70	ļ	Rice	R1		Earthen	Fair	0	250	500		Flat	Clay	2.0		10.0		0.3		Fixed	Pass- able	15.0					• Irrigation canal
	09-02-01 NOAKHALI	Yes	RCDG	ļ: 	Damaged		dence	ļ	R2	3.4	НВВ	Fair	25	120	3,000	0	Flat	Clay			17.0		0.3	None	Fixed	Pass- able	20.0			Yes Ye		
	09-02-02 NOAKHALI	Yes	RCDG		Damaged		Farm		R2		Earthen	Fair	2	200	1,500	200	Flat	Clay			10.0		0.3	None	Fixed	Pass- able	15.0 40.0		Yes			
	09-03-01 NOAKHALI	None		-	-	40	Farm		R2		Earthen	Fair	0	0	1 (00	-	Flat	Clay			15.0		0.4		Fixed	Pass- able	18.0		Yes			• Existing bridge is
	09-04-01 NOAKHALI	Yes	RCDG	16.4	Fair	35	Farm	Rice	R2	3.5	HBB	Good	15	56	1,600	U	Flat	Clay	2.0	4,4	10.0	10.0	0.3	None	rixea	Pass- able	10.0	5.5	res	No Ye	S NO	fair No need reconstruction
	09-05-01 NOAKHALI	Yes	RC Arch	15.6	fair	50	Resi- dence	1 .	R1	3.0	Earthen	Fair	3	70	2,000	0	Flat	Clay	2.0	4.0	10.0	16.0	0.4	None	Fixed	Pass- able	15.0	4.5	Yes	No Ye	s No	Existing bridge is fair No need reconstruction
- 1	10-01-01 AXMIPUR	Yes	Bamboo	33.5	-	50	Farm	Rice	R2	3.0	Earthen	Fair	0	0	1,500	0	Flat	Clay	1.8	5.0	20.0	30.0	0.3	None	Fixed	Pass- able	34.0	6.0	Yes	Yes Ye	s Yes	• Irrigation canal
	10-02-01 .AXMIPUR	Yes	-	80.0	Col- lapsed	75	Farm	Rice	R1	5.0	Earthen	Fair	0	0	0	1,000	Flat	Clay	3.0	5.0	75.0	87.0	0.4	None	Fixed	Pass- able	75.0	7.7	Yes	Yes Ye	s Yes	
56	10-03-01 .AXMIPUR	Yes	-	-	-	45	Farm	Rice	R3	2.5	Earthen	Bad	0	0	5	400	Flat	Clay					0.3		Fixed	able	61.0	4.5	Yes	Yes Ye	s Yes	
57	11-01-01 WARSINGDI	Yes	Timber	73.0	Bad	100	Resi- dence		FRB	2.5	Earthen	Bad	0	100	10,000	0	Flat	Clay				73.0	0.0		Fixed	able	75.0					• Fairway for small boats
	11-02-01 PARSINGDI	None	- , :	-	-	5	Farm	Rice			Earthen		0	0	0		Flat	Clay			18.0		0.3		Fixed	Pass- able	25.0		Yes		_	
1	1-02-02 IARSINGDI	None	- 1.	-	-		Farm				Earthen		0	0	0		Flat	Clay	1.0			16.0	0.3		Fixed	able	16.0		Yes			
	1-02-03 IARSINGDI	None	- 1, 1,	-	-		farm				Earthen		0	0	0		Flat	Clay				15.0			Fixed Fixed	able	15.0 65.0	· .	Yes			
!	1-02-04 MARSINGDI	None		-	-		Farm						0	0			Flat	Clay				65.0	0.3			able	400.0					• Fairway for small
4	1-03-01 IARSINGDI	None				15	Farm		R1		Earthen		0	15	0	700	Flat Flat	Silt				150.0	0.3		Fixed	able Pass-	125.0			· .		boats • Fairway for small
	1-03-02 IARSINGDI	None	-	_	-	30	Farm	Rice	R1	5. 0	Earthen	Bad	0	40	0	700	TBIT	SILL	0.5	4.7	ט. כט	150.0	J.,	HORIC	1,7,03	able						boats

Table 3-5 SITE DATA OF REQUESTED BRIDGES AND EVALUATION OF PRIORITY OF BRIDGES (4/4)

		Preser	nt Cond	ition o	f Bridge			Soci	o Econ	omic a	nd Appro	ach Road	d Inform	ation							Engine	ering	Informati	on			Recon			ion of ty Brid		
No.	Bridge No. /Location	Exist	Bridge	Bridge	Present	No. of Affect- ed	Land	Main	Road	Road	Road	Road Pre-	(1	Rainy :	lume (AD Season)	: r	Geo.	Topo	Dept		T	lydrogi :h (m)	cal Condi	tion River		Present Condi-	dat	tion	onomic st	ateness	يا	Remarks
		None	Туре		Condi- tion	Popula- tion (Thou.)	Use	Prod- ucts	Class	Width	Surface Type	sent Condi- tion	Vehicle	Rick- shaw	Walking	Boat	Condi- tion	Condi-	LWL	T -			Current Velocity (M/S)	Dike	Align-	tion of	Bridge Length (m)		Socio Ec Effec	Urgency Appropriat	Judgmer	
	12-01-01 ARIDPUR	None	-	•	-	50	Farm	Jute	R1	4.0	Earthen	Bad	0	0	0	500	Flat	Silt	4.5	15.0	50.0	120.0	1.5	None	Fixed	Pass- able	120.0	16.0	Yes	Yes Ye	Yes	• Fairway for small boats
	12-01-02 ARIDPUR	Yes	Bamboo	70.0	Dad	70	Farm	Jute	R1	4.5	Earthen	Fair	0	0	3,000	0	Flat	Clay	1.0	6.0	15.0	75.0	1.0	None	Fixed	Pass- able	65.0	7.5	Yes	Yes Ye	Yes	
	2-02-01 ARIDPUR	Yes	Bamboo	60.0	Bad	70	Farm	Jute	R1	4.0	Bit. Seal	Good	0	0	2,000	0	Flet	Clay	1.0	5.5	10.0	65.0	1.0	None	Fixed	Pass- able	60.0	7.0	Yes	Yes Ye	Yes	
	3-01-01 HAKA	None				40	Farm	Rice	FR8	7.0	Earthen	Fair	0	0		1,000	Flat	Clay		14.0	ļ	142.0		None	Fixed	Pass- able	140.0	16.0		Yes Ye		
D	3-01-02 Haka	None	-	_	-	40		Rice	FRB	7.0	Earthen		. 0	0		1,000	Flat	Clay	1.5					None	1.	able	50.0			Yes Ye		·
D	3-01-03 HAKA	None	-	-	-	30		Rice	R2		Earthen	Fair	0	0	0	500	Flat	Clay	ļ	2.0		15.0		None		able	15.0		:. •	Yes Ye		·
D	3-02-01 HAKA	None	<u>-</u>	<u>-</u>	-	100		Rice	FRB	3.0	нвв	Good	0	0		2,000	1	Clay	ļ	10.7		123.0				able	125.0					• Fairway for vessels
D	3-02-02 HAKA	None	-	-	-	25		Rice	FRB		Earthen	Fair	0	0	0	500		Clay		4.5	Ŀ			None	Fixed	Pass- able	40.0		: .	Yes Ye		- Pošavav fan vennala
DI	3-03-01 HAKA	None		70.0	-	100		Rice	FRB		Earthen	Bad	0	0		2,000	Flat	Clay	<u> </u>	10.7		30.0		None	Fixed	Pass- able Pass-	30.0			Yes Yes		• Fairway for vessels
DI	3-03-02 HAKA		Bamboo	30.0	-	20		Rice	FRB		Earthen	Fair	0	150	1,000 5,000		Flat Flat	Clay	0.3	3.0			0.0	None None	Fixed	able	20.0	, .		Yes Yes	1	
MI	4-01-01 UNSHIGONJ		Timber	19.0	Bad	25		Jute	R2	3.7	HBB Earthen	Good	0		3,000		Flat	Clay	ļ	3.4		ļ.				able	20.0			Yes Yes		
MI	4-01-02 UNSHIGONJ		Timber	14.0	Bad	15 30		Jute	R2 FRB		Earthen	Fair Fair	0	105	5,000		Flat	Clay	2.3			ļ	:		Fixed	able	40.0			Yes Yes		· ·
Mt	4-02-01 UNSHIGONJ 4-02-02		Timber Timber	39.5	Damaged Bad			Jute Jute			Earthen		0	170	5,000		Flat	Clay			40.0			None	·	able	40.0			Yes Yes	-	
М	UNSKIGONJ 4-03-01	None	- Imper	39.5	Bau	60		Jute	R1		Earthen		0	0	0		Flat	Clay	<u></u>			117.0	de e			able Pass-	115.0					● Fairway for small
MR	UNSHIGONJ 4-04-01		Steel	39.5	Poor	15		Jute			Earthen		0	135	2,000		Flat	Clay				47.0			Fixed	able Pass-	47.0					• Fairway for small
MU	UNSHIGONJ 4-05-01		Timber	40.0	Poor		Resi-	*			Earthen		. 0		10,000		Flat	Clay		4.5		40.0			Fixed	able Pass-	40.0	8.0	Yes	Yes Yes	Yes	• Fairway for small
ML	UNSHIGONJ 5-01-01		Timber	53.5		70	dence Farm	-	R2		Earthen	1	0	0	1,000		Moun-				ļ	53.0			Fixed	able Pass-	65.0			Yes Yes	-	boats
G/	AZIPUR 5-02-01		Валюоо	110.0	lapsed	80		Rice			Earthen		0	0	500	200	tains	Clay				110.0		None	Fixed		100.0	8.0	Yes	Yes Ye	Yes	
G/	AZIPUR 5-03-01		Timber	59.5	Bad			Potato	R1		Earthen		0	0	1,000		Flat	Clay	1.5	5.0	20.0	60.0	0.1	None	Fixed		60.0	8.5	Yes	Yes Yes	Yes	
	AZIPUR				oted are			Wheat		:]	<u>L.</u>		<u> </u>					able		·.]	<u>l</u>		

<sup>Population in affected area and ADT are estimated based on hearing survey.
ADT of vehicle includes auto-rickshaw and rickshaw includes motor-cycle.</sup>

3.3.2 Technical Evaluation on the Project

The project aims to construct bridges along Type-B feeder roads and rural roads in 15 districts in eastern Bangladesh. The bridge type is portable steel bridge. The Government of Bangladesh is responsible for construction of substructures and approach roads. The superstructures will be procured under this project.

This project will provide portable steel bridge materials for those bridges that the Government of Bangladesh can construct without technical difficulty.

The 79 high priority bridges were evaluated on their foreseen difficulties in construction and bridges having no difficulty in construction were selected for the project.

The evaluation criteria are as follows:

- Bridges can be planned with standard type substructures and foundations.
- Bridges do not need any special machinery or technology in construction including temporary works and erection works.

The evaluation result of each bridge is presented in Table 3-6. To evaluate the bridges, the bridge structures and site topography developed on the bridge general views were employed.

As a result, 74 bridges were evaluated as having no difficulty in construction, and were selected for the project. The other 5 bridges were evaluated as being difficult to construct. The difficulties are as follows:

- The water level of river is deep, even in the dry season, so that construction of the foundation and substructure requires a special construction method.
- When flooded, the water level is very deep and current velocity is fast, so that special structure type piers are required.
- The size of the river is large (wider than 200 m), so that a long span bridge is required.

Table 3-6 SELECTION OF BRIDGES FOR THE PROJECT (1/7)

			Substructure			E	ngine	eering	Info	mation				
No.	Bridge No.	Layout of Bridge	A : Abutment	Geo.	Торо.		Riv	er Hy	drogi	cal Con	dition)	Evaluation for Selection	ment
	Location		· · · · · · · · · · · · · · · · · · ·		Condition	. Dept	th (m)	Wid	th (m)	Current Velocity	Dike	Align-	(on construction difficulty)	Judgment
			in; Height (m)	11011	don	LWL	HWL	LWL	HWL	(M/S)	Exis- tence	ment		
1	01-01-01 COX'S BAZAR	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	$\begin{vmatrix} A_1 & : H = 4.5 \\ P_1 \sim P_2 : H = 7.5 \\ A_2 & : H = 4.5 \end{vmatrix}$		Clay	0.8	4. 7	36. 0	70. 0		Nane	Fixed	 Piers in river can be constructed using cofferdams. Construction is not difficult. 	Yes
2	01-02-01 COX'S BAZAR	Al 25e Pl 25e P2 25e P3 25e P4 25e A2 \triangle $L = 125 \text{ m}, W = 85.8 \text{ t}$	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$		Sand	0. 9	4. 3	60. 0	160. 0		None	Fixed	• Construction is easy.	Yes
3	02-01-01 PENI	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	$\begin{array}{cccccccccccccccccccccccccccccccccccc$		Clay	2. 0	7. 0	30. 0	80. 0	0. 3	None	Fixed	 Pile bent type pier can be constructed in deep river. Construction is not difficult. 	Yes
	02-02-01 Feni	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	A_1 : H = 4.5 P_1 : H = 8.0 A_2 : H = 4.5	Flat	Clay	1. 0	5. 4	9. 0	31. 0	0. 3	Yes	Fixed	 Piers in river can be constructed using cofferdams. Construction is not difficult. 	Yes
	02-02-02 Feni	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	A_1 : H = 4, 5 P_1 : H = 8.0 A_2 : H = 4, 5	Flat	Silt	1. 0	5. 6	8. 0	35. 0	0. 4	Yes	Fixed	 Piers in river can be constructed using cofferdams. Construction is not difficult. 	Yes
	03-01-01 CHITTAGONG	L = 30 m, W = 20.6 t	$ \begin{array}{cccc} A_1 & : H = 4.5 \\ P_1 & : H = 7.5 \\ A_2 & : H = 4.5 \\ \end{array} $	Flat	Silt	0. 8	4. 8	14. 0	28. 0	0. 6	None	Fixed	 Damage is very slight. No need reconstruction. 	No
	03-01-02 CHITTAGONG	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	A_1 : H = 4.5 P_1 : H = 9.0 A_2 : H = 4.5	Flat	Silt	0. 9	6. 5	20. 0	36. 0		None	fixed	 Replacement of deck slab is needed. No need reconstruction. 	No
	03-02-01 Chittagong	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	A_1 : H = 4.5 $P_1 \sim P_2$: H = 10.0 A_2 : H = 4.5	Flat	Sand.	2, 0	7. 5	50. 0	70. 0		None	Fixed	 Piers in river can be constructed using cofferdams. Construction is not difficult. 	Yes
	03-03-01 CHITTAGONG	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$				0. 5	5. 8	8. 0	62. 0	1. 0	None	Fixed	• Construction is easy.	Yes
	03-04-01 CHITTAGONG	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	A_1 : H = 4.5 P_1 : H = 6.5 A_2 : H = 4.5	Flat	Clay	1. 9	5. 7	25. 0	35. 0	1. 0	None	Fixed	Pile bent type pier can be constructed in deep river. Construction is not difficult.	Yes
	03-05-01 HITTAGONG	$\begin{array}{cccc} & \begin{array}{ccccc} & A1 & 25n & A2 \\ & \Delta & \Delta & \Delta \end{array} $ $L = 25 \text{ m}, W = 17.2 \text{ t}$	A_1 : $H = 4.5$ A_2 : $H = 4.5$	Flat	Sand	2. 0	4. 0	16. 0	22. 0		None	Fixed	• Construction is very easy due to no pier.	Yes
	3-06-01 HITTAGONG	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	A_1 : $H = 4.5$ P_1 , P_3 : $H = 6.5$ P_2 : $H = 14.0$ A_2 : $H = 4.5$	Flat	Clay	4. 5	13. 1	37. 0	87. 0		None	Fixed	 Pile bent type pier can be constructed in deep river. Construction is not difficult. 	Yes

Table 3-6 SELECTION OF BRIDGES FOR THE PROJECT (2/7)

			Substructure			E	ngine	ering	lnfor	matio	n			
No.	Bridge No.	Layout of Bridge	A: Abutment	Geo.	Торо.	14 1	Rive	er Hy	drogic	cal Co	nditio	n	Evaluation for Selection	ment
	Location	Layout of bridge	P: Pier	Condi-	Condi-	Dept	h (m)	Wid	th (m)	Currer		Align	(on construction difficulty)	Judgment
			H: Height (m)	tion	tion	LWL	HWL.	LWL	HWL	Veloci (M/S)	Exis-			'
13	04-01-01	Δ1 20π P1 25π P2 20π Δ2 ΔΔ ΔΔ ΔΔ ΔΔ	A_1 : H = 5.5 $P_1 \sim P_2$: H = 11.0	Flat	Clav	3.0	10.5	25 0	F1.00D	0.9	None	Fixed	 Piers in river can be constructed using cofferdams. Construction is not difficult. 	¥.
-	HABIGONI	L = 65 m, W = 44.6 t	A_2 : H = 5, 5						1.000	0. 7	nva v	1120		Yes
14	04-02-01	11 25 m P1 25 m P2 25 m A2 \(\triangle \tria	A_1 : H = 4.5 $P_1 \sim P_2$: H = 8.0	Flat	Clav	1.0	6.5	10 0	84 0	1, 5	None	Fixed	Construction is easy.	V
_	HABIGONJ	$L = 75 \text{ m} \cdot W = 51.5 \text{ t}$	A_2 : H = 4.5				V. ,	10. 0	U1. 0	1, 7	None	FIXCU		Yes
15	04-03-01	A1 20m P1 20m A2 Δ ΔΔ Δ	A_1 : $H = 4.5$ P_1 : $H = 8.0$	Flat	Clav	n 8	5 5	20.0	\$1.00D	1.0	None	Fixed	 Piers in river can be constructed using cofferdams. Construction is not difficult. 	V
-	HABIGONI	L = 40 m, W = 27.4 t	A_2 : H = 4.5		,	v. 0		20, 9	1 200	1. 0		Tikey		Yes
16	04-04-01	$ \begin{array}{c ccccc} \lambda_1 & 25 \mathbf{w} & P_1 & 25 \mathbf{w} & \lambda_2 \\ \hline \Delta & & \triangle & & \Delta \end{array} $	A_1 : $H = 5.0$ P_1 : $H = 9.5$	Fiat	Clav	4.0	8 5	30 0	48 0	1. 0	None	Fixed	 Pile bent type pier can be constructed in deep river. Construction is not difficult. 	V
_	HABIGONG	L = 50 m, W = 34.3 t	$A_2 : H = 5.0$				0. 7	J	10.0	1. 0	Hone	I I ACU		Yes
17	04-04-02	$\begin{array}{c cccc} A1 & 25 & P1 & 25 & A2 \\ \hline \triangle & & \triangle & & \triangle \\ \end{array}$	A_1 : H = 5.0 P_1 : H = 9.5	Flat	Clav	2.5	7 7	35 0	F1.000	1, 0	None	Fixed	 Pile bent type pier can be constructed in deep river. Construction is not difficult. 	Yes
_	HABIGONG	L = 50 m, W = 34.3 t	$A_2 : H = 5.0$				"	J		1. 0		11100		168
18	05-01-01	$A1 \frac{20}{\Delta} \frac{A2}{\Delta}$	$A_1 : H = 4.5$	Flat	Clay	1 0	2.5	7 3	17 0		None	Fixed	Construction is very easy due to no pier.	Yes
<u></u>	MOULVIBAZAR	L = 20 m, W = 13.7 t	A_2 : $H = 4.5$,		u . ,		1			11200		168
19	05-01-02	Λ1 20m P1 20m A2 Δ ΔΔ ΖΔ	A_1 : H = 4.5 P_1 : H = 6.0	Roll-	Clav	กร	3 3	23 0	30 0	e!	None	Fixed	 Piers in river can be constructed using cofferdams. Construction is not difficult. 	Yes
ļ	MOULVIBAZAR	L = 40 m, W = 27.4 t	A_2 : $H = 4.5$			V. 7	3. 3	25. 0	501,0					169
20	05-02-01	$\frac{A_1}{\Delta} = \frac{25}{\Delta} = \frac{A_2}{\Delta}$	A_1 : $H = 4.5$	Roll-	Clav	1.0	5.2	15 0	21. 0		Yes	Fixed	Construction is very easy due to no pier.	V
	MOULVIBAZAR	L = 25 m, W = 17.2 t	A_2 : $H = 4.5$			1. 0	7 4	10.9	81, 0		100	11200		Yes
21	05-02-02	A1 20m P1 20m A2 Δ ΔΔ Δ	A_1 : H = 4.5 P_1 : H = 7.0	8011-	Silt	1.0	4 3	20 0	32.0		Yes	fixed	Piers in river can be constructed using cofferdams. Construction is not difficult.	Yes
	MOULVIBAZAR	L = 40 m, W = 27.4 t	$\begin{array}{ccc} A_2 & : H = 4, 5 \end{array}$		J	1. 0		20.0	72. 0			11200		168
22	05-03-01	A1 25 m P1 25 m A2 △	A_1 : H = 4, 5 P_1 : H = 13, 5	Roll-	Clav	1 2	11 0	12 n	55.0	1. 0	Yac	Fixed	 Piers in river can be constructed using cofferdams. Construction is not difficult. 	Yes
	MOULVIBAZAR	L = 50 m, W = 34, 3 t	$\mathbf{A_2} \qquad : \mathbf{H} = 4.5$		0,0,	1.)	11. 0	15. 0	77. 0	1. 0		11,00		162
23	05-04-01	A1 20 m A2	$A_1 : H = 4.5$	Flat	امدارا	1. 0	5. 3	χn	13 0		Yes	Fixed	Construction is very easy due to no pier.	Von
	MOULVIBAZAR	L = 20 m, W = 13.7 t	A_2 : $H = 4.5$		Viaj	1. U	ر ر	0. 0	17.0		1.03	FIACU		Yes
24	06-01-01	Λ1 25m A2 Δ Δ	$A_1 : H = 4, 5$	7121	Clay	1 0	5.0	12 0	25.0	ηΩ	None	fixed	Construction is very easy due to no pier.	Vac
	CHANDOPUR	L = 25 m, W = 17.2 t	$A_2 : H = 4.5$		0101	1. 0	<i>7</i> . 0	14. U	43,0	U. D	HULL	1.444		Yes

Table 3-6 SELECTION OF BRIDGES FOR THE PROJECT (3/7)

			Substructure			E	Engine	eering	j Info	mation)	· · · · · · · · · · · · · · · · · · ·		
.\0. Bi	ridge No.	Layout of Bridge	A: Abutment	Geo.	Торо,		Riv	er Hy	drogi	al Con	dition		Evaluation for Selection	Judgment
Lo	ocation		P: Pier	Condi-	Condi- tion	Dept	th (m)		th (m)	Current Velocity (M/S)	Dike Exis-	Align- ment	(on construction difficulty)	Judg
	6-02-01 Handopur	L = 15 m, W = 10.3 t	A_1 : H = 4.5 A_2 : H = 4.5	Flat	Clay	0. 0	4. 5	0. 0	15. 0	0. 1	None	fixed	Construction is very easy due to no pier.	Yes
	5-03-01 Tandopur	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	Flat	Clay	2. 0	6. 0	20. 0	40. 0	0. 5	None	Fixed	 Piers in river can be constructed using cofferdams. Construction is not difficult. 	Yes
	5-03-02 IANDOPUR	$\Delta = \frac{125 \cdot 12}{\Delta}$ $\Delta = 25 \cdot 17.2 \cdot t$	A_1 : H = 4.5 A_2 : H = 4.5	Flat	Clay	1. 9	2. 5	18. 0	25. 0	0. 1	None	Fixed	• Construction is very easy due to no pier.	Yes
	-04-01 IANDOPUR	L = 30 m, W = 20.6 t	A_1 : H = 4.5 P_1 : H = 7.0 A_2 : H = 4.5	Flat	Clay	3. 0	5. 0	28. 0	30. 0	0. 1	None	Fixed	 Pile bent type pier can be constructed in deep river. Construction is not difficult. 	Yes
29 06 CH	-04-02 IANDOPUR	L = 25 m, W = 17.2 t	A_1 : H = 4.5 A_2 : H = 4.5	Flat	Clay	3. 0	4. 5	24. 0	27. 0	0. 0	None	Fixed	Construction is very easy due to no pier.	Yes
30 07- B. E	-01-01 BARIA	$\begin{array}{c ccccc} A1 & 25 & A2 \\ \hline \Delta & \Delta & \Delta \\ \hline L = 25 & m, W = 17.2 & t \end{array}$	A_1 : $H = 4, 0$ A_2 : $H = 4, 0$	Flat	Clay	1. 0	4, 5	10. 0	22. 0	0. 5	None	Pixed	Construction is very easy due to no pier.	Yes
	-01-02 BARIA	L=100 m, W= 68.6 t	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	Flat	Clay	1. 5	6. 5	10. 0	90. 0	0. 5	None	Fixed	• Construction is easy.	Yes
32 07- B. 8	-02-01 BARIA	$\frac{\text{A1 15= P1 15= A2}}{\triangle}$ $L = 30 \text{ m}, W = 20.6 \text{ t}$	$\begin{array}{cccc} A_1 & : H = 4.5 \\ P_1 & : H = 7.0 \\ A_2 & : H = 4.5 \end{array}$	Flat	Clay	0. 5	3. 5	5. 0	30. 0	0. 5	None	Fixed	Piers in river can be constructed using cofferdams. Construction is not difficult.	Yes
33 07- B. B	-02-02 BARIA	L = 15 m, W = 10.3 t	A_1 : $H = 4.5$ A_2 : $H = 4.5$	Flat	Clay	0. 3	2. 2	4. 0	13. 0	0. 5	None	Fixed	Construction is very easy due to no pier.	Yes
34 07- B. B	-03-01 BARTA	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	Flat	Clay	1. 5	6, 5	25. 0	60. 0	2. 0	None	Fixed	 Piers in river can be constructed using cofferdams. Construction is not difficult. 	Yes
35 07- B, B	-03-02 Baria	$\frac{\text{Al } 15 \text{ s. Pl. } 15 \text{ s. A2}}{\Delta} \qquad \Delta$ $L = 30 \text{ m. W} = 20.6 \text{ t}$	$ \begin{array}{cccc} A_1 & : H = 4,5 \\ P_1 & : H = 7,5 \\ A_2 & : H = 4,5 \\ \end{array} $	Flat	Silt	3. 0	6. 5	18. 0	30. 0	1. 8	None	Fixed	 Pile bent type pier can be constructed in deep river. Construction is not difficult. 	Yes
36 07-1 B, B,	-04-01 DARIA	$L = 25 \text{ m} \cdot \text{W} = 17.2 \text{ t}$	A_1 : $H = 4.0$ A_2 : $H = 4.0$	Flat	Clay	0. 5	4. 5	8. 0	27. 0	0. 8	None	Fixed	• Construction is very easy due to no pier.	Yes

Table 3-6 SELECTION OF BRIDGES FOR THE PROJECT (4/7)

			Substructure			E	ngin	eering	Infor	mation		-		1
\\o.	Bridge No.	Layout of Bridge	A: Abutment	Geo.	Торо.		Riv	er Hy	drogic	cal Con	dition		Evaluation for Selection	nent
	Location	aayooo on anage	D ===	1	Condi-		T		T	Current Velocity	River Dike	Align-	(on construction difficulty)	Judgment
37	08-01-01	11 25 a A2	$A_1 : H = 4.5$			LWL	HWL	LWL	HWL	(M/S)	tence	4	Construction is very easy due to no pier.	
	COMILLA	L = 25 m, W = 17.2 t	A_2 : H = 4.5		Clay	1. 0	3. 5	10. 0	24. 0	0. 5	Yes	Fixed		Yes
38	08-01-02 COMILLA	L = 25 m, W = 17, 2 t	A_1 : H = 4, 5 A_2 : H = 4, 5	Roll	Clay	0. 5	2. 5	3. 0	18. 0	0. 2	Yes	Fixed	Construction is very easy due to no pier.	Yes
i [08-01-03 COMILLA	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	A_1 : H = 4.5 P_1 : H = 8.0 A_2 : H = 4.5	Flat	Clay	1. 0	3. 7	27. 0	43. 0	0. 3	Yes	Fixed	 Piers in river can be constructed using cofferdams. Construction is not difficult. 	Yes
10	08-02-01 COMILLA	$\frac{A1 15m P1 15m A2}{\Delta \Delta \Delta}$ $L = 30 m, W = 20.6 t$	A_1 : H = 4.5 P_1 : H = 8.0 A_2 : H = 4.5	Flat	Clay	1. 5.	5. 0	15. 0	28. 0	0. 0	None	Fixed	Piers in river can be constructed using cofferdams. Construction is not difficult.	Yes
	08-02-02 COMILLA	$\frac{A_1}{\Delta} = \frac{25s}{\Delta} = \frac{A_2}{\Delta}$ L = 25 m, W = 17, 2 t	A_1 : $H = 4.5$ A_2 : $H = 4.5$	Flat	Clay	0. 5	1. 7	8. 0	17. 0	0. 1	None	fixed	Construction is very easy due to no pier.	Yes
	08-03-01 COMILLA	$\Delta^{1} = \frac{15a}{\Delta} = \frac{P1}{\Delta} = \frac{15a}{\Delta}$ $L = 30 \text{ m}, W = 20.6 \text{ t}$	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	Flat	Clay	0. 0	3. 7	0. 0	28. 0	0. 2	None	Fixed	• Construction is easy because no water exists during dry season.	Yes
	08-03-02 COMILLA	L = 25 m, W = 17.2 t	A_1 : $H = 4.5$ A_2 : $H = 4.5$	Flat	Clay	1. 0	4. 3	5. 0	25. 0	0. 5	None	Fixed	Construction is very easy due to no pier.	Yes
	08-04-01 COMILLA	$ \begin{array}{c cccc} A1 & 25 & A2 \\ \Delta & \Delta \\ L = 25 & m, W = 17.2 & t \end{array} $	A_1 : $H = 4.5$ A_2 : $H = 4.5$	Flat	Clay	0. 0	4. 5	0. 0	25. 0	0. 5	None	Fixed	Construction is very easy due to no pier.	Yes
	08-04-02 COMILLA	L = 30 m, W = 20.6 t	A_1 : H = 4.5 P_1 : H = 7.5 A_2 : H = 4.5	Flat	Clay	1. 0	5. 0	4. 5	30. 0	0. 5	None	Fixed	 Piers in river can be constructed using cofferdams. Construction is not difficult. 	Yes
	08-04-03 COMILLA	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	A_1 : H = 4.5 $P_1 \sim P_2$: H = 7.0 A_2 : H = 4.5	Flat	Clay	2. 0	6. 0	45. 0	60. 0	0. 7	None	Fixed	• Construction is easy.	Yes
	19-01-01 10akhali	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	Flat	Clay	1. 2	4. 0	25. 0	60. 0	0. 4	None	Fixed	Piers in river can be constructed using cofferdams. Construction is not difficult.	Yes
	19-01-02 TOAKHALI	L = 15 m, W = 10, 3 t	A_1 : $H = 4.5$ A_2 : $H = 4.5$	Flat	Clay	2. 0	8. 0	10. 0	14. 0	0. 3	None	Fixed	• Construction is very easy due to no pier.	Yes

Table 3-6 SELECTION OF BRIDGES FOR THE PROJECT (5/7)

			Substructure			E	ngine	ering	Infor	mation) ·			
No.	Bridge No.	Layout of Bridge	A: Abutment	Geo.	Topo		Rive	er Hy	drogic	cal Cor	ndition)	Evaluation for Selection	ment
	Location			Condi	Condition	Dept LWL	h (m)	 	th (m)	Velocit	y Dike	Align- ment	(on construction difficulty)	Judgment
	09-02-01 Noakhali	L = 20 m, W = 13.7 t	A_1 : $H = 4.5$ A_2 : $H = 4.5$	Flat	Clay	1. 0	3. 8			(101/3)	Non e	Fixed	Construction is very easy due to no pier.	Yes
	09-02-02 Noakhali	L = 15 m, W = 10, 3 t	A_1 : $H = 4.5$ A_2 : $H = 4.5$	Flat	Clay	1. 0	4. 0	10. 0	16. 0	0.3	None	Fixed	Construction is very easy due to no pier.	Yes
	09-03-01 Noakhali	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	A_1 : H = 4, 5 P_1 : H = 8, 0 A_2 : H = 4, 5	Flat	Clay	1. 5	5. 0	15. 0	35, 0	0. 4	None	Fixed	 Piers in river can be constructed using cofferdams. Construction is not difficult. 	Yes
	09-04-01 IOAKHALI	L = 15 m, W = 10.3 t	A_1 : $H = 4.5$ A_2 : $H = 4.5$	Flat	Clay	2. 0	4. 4	10. 0	16. 0	0. 3	None	Fixed	 Condition of existing RCDG very good. No need reconstruction. 	No
	19-05-01 IOAKHALI	L = 15 m, W = 10, 3 t	A_1 : $H = 4.5$ A_2 : $H = 4.5$	Flat	Clay	2. 0	4. 0	10. 0	16. 0	0. 4	None	Fixed	 Condition of existing RC Arch bridge very good. No need reconstruction. 	No
	0-01-01 AXIPUR	L = 30 m, W = 20.6 t	A ₁ : H = 4.5 P ₁ : H = 8.5 A ₂ : H = 4.5	Plat	Clay	. 1. 8	5. 0	20. 0	30. 0	0. 3	None	Fixed	Piers in river can be constructed using cofferdams. Construction is not difficult.	Yes
	0-02-01 AXIPUR	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	A_1 : H = 4.5 $P_1 \sim P_2$: H = 7.0 A_2 : H = 4.5	Flat	Clay	3. 0	5. 0	75. 0	87. 0	0. 4	None	Fixed	 Pile bent type pier can be constructed in deep river. Construction is not difficult. 	Yes
	0-03-01 AXIPUR	L = 65 m, W = 44.6 t	A_1 : H = 4.5 $P_1 \sim P_2$: H = 7.0 A_2 : H = 4.5	Flat	Clay	0. 6	3. 0	30. 0	65. 0	0. 3	None	Fixed	Piers in river can be constructed using cofferdams. Construction is not difficult.	Yes
	1-01-01 Arsingdi	L = 75 m, W = 51, 5 t	A_1 : H = 4, 5 $P_1 \sim P_2$: H = 7, 0 A_2 : H = 4, 5	Flat	Clay	1. 0	5. 5	73. 0	73. 0	0. 0	None	Fixed	Piers in river can be constructed using cofferdams. Construction is not difficult.	Yes
ļ	1-02-01 Arsingdi	$ \begin{array}{c cccc} & \lambda 1 & 25s & \lambda 2 \\ & \Delta & \Delta & \Delta \end{array} $ $ L = 25 \text{ m}, W = 17.2 \text{ t} $	A_1 : $H = 4.5$ A_2 : $H = 4.5$	Flat	Clay	1. 0	3. 0	18. 0	18. 0	0. 3	None	Fixed	Construction is very easy due to no pier.	Yes
	-02-02 RSINGDI	L = 15 m, W = 10.3 t	A_1 : $H = 4.5$ A_2 : $H = 4.5$	Flat	Clay	1. 0	3. 0	16. 0	16. 0	0. 3	None	Fixed	Construction is very easy due to no pier.	Yes
	-02-03 RSINGD1	Δ Δ	A_1 : $H = 4.5$ A_2 : $H = 4.5$	Flat	Clay	1. 0	3. 0	15. 0	15. 0	0, 3	None	Pixed	• Construction is very easy due to no pier.	Yes

Table 3-6 SELECTION OF BRIDGES FOR THE PROJECT (6/7)

			Substructure			E	ngine	ering	Infor	mation	·			
No.	Bridge No.	Layout of Bridge		Geo.	lopo.	River Hydrogical Condition				al Con	·		Evaluation for Selection	udgment
	Location		P: Pier H: Height (m)]		Dept LWL	h (m)	Wid:	1	Current Velocity (M/S)	River Dike Exis- tence	Align- ment	(on construction difficulty)	
51	11-02-04 NARSINGDI	L = 65 m, W = 44.6 t	A_1 : H = 4.5 $P_1 \sim P_2$: H = 9.0 A_2 : H = 4.5	Flat	Clay	1. 5	3. 5	65. 0	65. 0	0. 3	None	Fixed	 Piers in river can be constructed using cofferdams. Construction is not difficult. 	Yes
62	11-03-01 NARSINGDI	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	A_1 : H = 5.0 P_1 , P_{15} : H = 6.0 $P_2 \sim P_{14}$: H = 11.0 A_2 : H = 6.0	Flat	Silt	1. 3	10. 0	40. 0	450. 0	0. 5	None	Fixed	 Fairway for small boats. Portable bridge (L max = 25m) is not applicable for such Long bridge (L = 400m). Long span bridge is needed. 	No
	11-03-02 NARSINGDI	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	A_1 : H = 4.5 $P_1 \sim P_4$: H = 7.5 A_2 : H = 4.5	Flat	Silt	0. 3	4.5	85. 0	150. 0	0. 3	None	fixed	Pile bent type pier can be constructed in deep river. Construction is not difficult.	Yes
	12-01-01 FARIDPIR	L = 125 m, W = 85, 8 t	A_1 : H = 4.5 P_1 . P_4 : H = 6.0 $P_2 \sim P_3$: H = 10.0 A_2 : H = 4.5	Flat	Silt	4. 5	15. 0	50. 0	120. 0	1. 5	None	Fixed	 Fairway for small boards, Construction is difficulty because tall piers are required and current velocity is fast. 	No
	12-01-02 FARIDPUR	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	A_1 : $H = 4.5$ $P_1 \sim P_2$: $H = 8.0$ A_2 : $H = 4.5$	flat	Clay	1. 0	6. 0	15. 0	75. 0	1. 0	None	fixed	Construction is easy.	Yes
	12-02-01 FARIDPUR	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	A_1 : H = 4.5 $P_1 \sim P_2$: H = 6.0 A_2 : H = 4.5	Flat	Clay	1. 0	5. 5	10. 0	65. 0	1. 0	None	Fixed	• Construction is easy.	Yes
	13-01-01 A	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	A_1 : H = 5.5 $P_1 \sim P_5$: H = 15.0 A_2 : H = 5.5	Flat	Clay	1. 5	14. 0	60. 0	142. 0	1. 0	None	Fixed	Construction is difficult because tall piers are required and current velocity is fast.	No
	13-01-02 DHAKA	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	A_1 : H = 4.5 P_1 : H = 6.5 A_2 : H = 4.5	Flat	Clay	1. 5	4. 6	35. 0	52, 0	2. 0	None	Fixed	This bridge shall not be constructed when 13-01-01 is not constructed because the bridge is adjacent to 13-01-01.	No
	13-01-03 DHAKA	L = 15 m, W = 10.3 t	A_1 : $H = 4.5$ A_2 : $H = 4.5$	Flat	Clay	0. 5	2. 0	12. 0	15. 0	0 . 0	None	Fixed	Construction is very easy due to no pier.	Yes
	13-02-01 DHAKA	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	A_1 : H = 4, 5 $P_1 \sim P_4$: H = 16, 0 A_2 : H = 4, 5	Flat	Clay	5. 5	10. 7	64. 0	123. 0	1. 2	None	Fixed	Fairway of vessels Construction is difficult because tall piers are required and current velocity is fast.	No
	13-02-02 DHAKA	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	A_1 : H = 4.5 P_1 : H = 7.0 A_2 : H = 4.5	Flat	Clay	0, 5	4. 5	15. 0	41. 0	0. 0	None	Pixed	Piers in river can be constructed using cofferdams. Construction is not difficult.	Yes

Table 3-6 SELECTION OF BRIDGES FOR THE PROJECT (7/7)

			Substructure			E	ngine	ering	Infor	mation				<u> </u>
No.	Bridge No.	Layout of Bridge	A : Abutment	Geo.	Торо.		Riv	er Hy	drogic	al Con	dition		Evaluation for Selection	nent
	Location				Condi- tion	Dept	h (m)	-	h (m)	Current Velocity (M/S)	Condition Trent Disco D	(on construction difficulty)	Judgment	
72	13-03-01 DHAKA	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	$\begin{array}{cccccccccccccccccccccccccccccccccccc$		Clay	5. 0	10. 7	45. 0	122. 0			Fixed	Piers in river can be constructed using cofferdams. Construction is not difficult.	Yes
73	13-03-02 DHAKA	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	$\begin{array}{cccc} A_1 & : H = 4.5 \\ P_1 & : H = 6.0 \\ A_2 & : H = 4.5 \end{array}$	Flat	Clay	0. 3	3. 0	15, 0	30. 0	0. 0	None	Fixed	Piers in river can be constructed using cofferdams. Construction is not difficult.	Yes
74	14-01-01 MUNSHIGONJ	L = 20 m, W = 13.7 t	A_1 : H = 4.5 A_2 : H = 4.5	Flat	Clay	1. 3	4. 2	18. 0	19. 0	0. 3	None	Fixed	Construction is very easy due to no pier.	Yes
75	14-01-02 Munshigonj	L = 20 m, W = 13.7 t	A_1 : $H = 4.5$ A_2 : $H = 4.5$	Flat	Clay	1. 3	3. 4	10. 0	14. 0	0. 0	None	Fixed	Construction is very easy due to no pier.	Yes
76	14-02-01 MUNSHIGONJ	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	Plat	Clay	2. 3	5. 6	40. 0	43. 0	0. 3	None	Fixed	 Pile bent type pier can be constructed in deep river. Construction is not difficult. 	Yes
17	14-02-02 Munshigonj	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	A_1 : $H = 4.5$ P_1 : $H = 5.0$ A_2 : $H = 4.5$	Flat	Clay	2. 3	5. 6	40. 0	43. 0	0. 3	None	Fixed	Pile bent type pier can be constructed in deep river. Construction is not difficult.	Yes
78	14-03-01 MUNSRIGONJ	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	Plat	Clay	3. 0	7. 2	65. 0	117. 0	1. 0	None	Fixed	 Pile bent type pier can be constructed in deep river. Construction is not difficult. 	Yes
79	14-04-01 MUNSHIGONJ	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	Flat	Clay	6. 5	8. 8	35. 0	47. 0	1. 0	None	Fixed	 Pile bent type pier can be constructed in deep river. Construction is not difficult. 	Yes
	14-05-01 Munshigonj	L = 40 m, W = 27.4 t	A_1 : H = 4.5 P_1 : H = 5.5 A_2 : H = 4.5	Flat	Clay	1. 5	4. 5	25. 0	40. 0	0. 3	None	Pixed	 Pile bent type pier can be constructed in deep river. Construction is not difficult. 	Yes
	15-01-01 GAZIPUR	, $\Delta 1 = 20\pi$ P1 25π P2 20π A2 $\Delta \Delta \Delta$	A_1 : H = 4.5 $P_1 \sim P_2$: H = 9.0 A_2 : H = 4.5	Flat	Clay	1. 5	8. 8	28. 0	53. 0	1. 2	None	Fixed	• Construction is easy.	Yes
	15-02-01 Gazipur	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	A_1 : H = 4.5 $P_1 \sim P_4$: H = 5.0 A_2 : H = 4.5	Plat	Clay	1. 5	6. 4	00. 0	10. 0	0. 5	None		 Piers in river can be constructed using cofferdams. Construction is not difficult. 	Yes
1	15-03-01 Gazipur	$\frac{A1}{\Delta} = \frac{20\pi}{2M} = \frac{P1}{25\pi} = \frac{P2}{20\pi} = \frac{A2}{\Delta}$ $L = 65 \text{ m}, W = 44.6 \text{ t}$	A_1 : H = 4.5 $P_1 \sim P_2$: H = 7.0 A_2 : H = 4.5	Flat	Clay	1. 5	5. 0	20. 0	60. 0	0. 1	None	Fixed	 Piers in river can be constructed using cofferdams. Construction is not difficult. 	Yes

Examination of the Request 3.3.3

Through discussions between involved officials of the Government of Bangladesh and the Study Team, the basic specifications of the portable steel bridge have been agreed as follows:

Basic specifications of portable steel bridge

Structure type

: Pony truss type

Bridge width

: Single lane carriageway

Design live load

: AASHTO HS-15

Bridge span length: 80 ft' (approx. 25 m)

Painting

: Galvanizing

Engineering discussions on the basic specifications are as follows:

(1) Structure type

The pony truss type bridge used for feeder and rural roads which rarely endure heavy traffic and are used for temporary detour bridges is common in Bangladesh. The features of this bridge type are as follows:

- The structure is simple, so no special technology or tools are required for construction.
- The bridge members are light, so cranes are not required in handling.
- Erection is easy and fast.
- The bridge can be used for long period as it is permanent bridge.

(2) Bridge width

The road design guidelines of LGED specify that the carriageway width for Type-B feeder roads and R1 rural roads be 12 ft' wide. The traffic volume and components along the project roads are being observed, and amounts which do not exceed the capacity of single lane bridges in general are foreseen.

(3) Design live load

The bridge design guidelines of LGED specify that the design live load for a 12 ft' wide feeder load bridge be HS-20 (which is equivalent to an 18 ton truck load). However, applying HS-20, which is used to design trunk highway bridges, seems excessive for the project bridges, taking the design concepts of low cost, safety and durability into consideration. Based on observation and forecast of traffic volume and the size of heavy vehicles along the project roads, it was decided to apply HS-15 (which is equivalent to a 14 ton truck load) to the project bridges.

(4) Bridge span length

The shortest span length is 15 m, which was decided as the shortest bridge length in planned bridges. The longest span length is 25 m, which was based on the following considerations:

- The longest span should not be longer than the truss members needed to change the size. To economize and ease fabrication and erection of the trusses, all trusses of all the bridges need to be composed of the same members.
- The longest span should not be so long as to require a special erection method.
- The longest span should not be so long that each truss member is too heavy to carry by hand.
- The span lengths should be long enough to lower the construction cost of entire bridges including superstructures and substructures.
- The span lengths should be long enough to not obstruct flood discharge except for special rivers.

(5) Painting

Generally, painting on steel needs repainting every 3 to 5 years. Steel galvanized generally does not need maintenance for more than 20 years. Galvanizing in members, for truss members despite being more costly in production, is proposed to eliminate this maintenance work.

CHAPTER 4

BASIC DESIGN

CHAPTER 4

BASIC DESIGN

4.1 DESIGN PRINCIPLES

For the basic design of the project, the following design principles were established:

(1) Principle conditions

- Detailed design of the superstructures will be made by the Japanese consultant. Detailed design and construction of the substructures and other necessary works will be the responsibility of the Government of Bangladesh.
- Basic specifications for the superstructures are specified as described in the Minutes of Discussions, which concerned officials of the Government of Bangladesh and the Study Team discussed and agreed. (The Minutes of Discussion are attached in Appendix-2.)
- The Government of Bangladesh will carry out detailed design of the substructures based on detailed topographic and geological surveys and hydrological analyses in the implementation stage. In the course of detailed design of the substructures, the bridge locations and dimensions of the substructures will be finally concluded.

(2) Design concept

The portable steel bridge should be designed so as to be:

- Low-cost, safe and reliable
- Easy to erect
- Able to be erected by hand

- Durable and requiring little maintenance
- The design criteria should be reflect the local conditions and traffic.
- The substructures and approach roads should be designed to be able to be constructed with local materials and technology.

(3) Consideration on a past similar project

In 1985, the Government of Bangladesh procured portable steel bridge materials necessary for the "Upazila Connection Road Project" under Japan's Grant Aid, which project is similar to this project. The basic design of the project should learn from a past similar project.

Regarding the former project, the following matters were noted for improvement in a report related to the former project and the Study Team's site investigation of the bridges constructed under the former project.

a) To examine the optimum span length for the portable steel bridge

In the former project, the span length of the portable steel bridge was 9 m to 48 m. Bridges longer than a 30 m span consisted of too many members and of too large members. The resulting bridges were not economical and were not easy to erect. Therefore, a range of optimum span lengths for the portable steel bridge should be examined in terms of cost, construction ease and safety against flooding.

b) To provide the necessary erection tools

In the former project, a shortage of erection tools resulted in erection of the portable steel bridges not being executed as well as planned in quality and speed. Therefore, the items and quantities of tools necessary for bridge erection should be examined based on the implementation schedule of this project.

c) To provide erection training

In the former project, a lack of technology of erection of portable steel bridges caused to be difficult in assembly and erection. Therefore, a technical manual and on-the-job training should be planned for this project.

(4) Coordination with LGED's implementation schedule

LGED plans to implement construction of the project bridges over a 2-year period, which is divided into 3 phases. The implementation schedule of this project should be based on LGED's schedule.

The list of bridges subject to each phase is listed in Table 4-1.

Table 4-1 LIST OF BRIDGES BY IMPLEMENTATION PHASE

Phase 1				Pha	se 2	Phase 3				
No.	Bridge No.	District	No.	Bridge No.	District	No.	Bridge No.	District		
1	01-01-01	COX'S BAZAR	1	02-01-01	FENI	1	03-05-01	CHITTAGONG		
2	01-02-01	COX'S BAZAR	2	03-03-01	CHITTAGONG	2	03-06-01	CHITTAGONG		
3	02-02-01	FENI	3	03-04-01	CHITTAGONG	3	04-04-01	HABIGONJ		
4	02-02-02	FENI	4	04-02-01	HABIGONJ	4	04-04-02	HABIGONJ		
5	03-02-01	CHITTAGONG	5	04-03-01	HABIGONJ	5	05-03-01	MOULVIBAZAR		
6	04-01-01	HABIGONJ	6	05-02-01	MOULVIBAZAR	6	05-04-01	MOULVIBAZAR		
7	05-01-01	MOULVIBAZAR	7	05-02-02	MOULVIBAZAR	7	06-04-01	CHANDPUR		
8	05-01-02	MOULVIBAZAR	8	06-01-01	CHANDPUR	8	06-04-02	CHANDPUR		
9	06-03-01	CHANDPUR	9	06-02-01	CHANDPUR	9	07-03-02	B. BARIA		
10	06-03-02	CHANDPUR	10	07-02-01	B. BARIA	10	07-04-01	B. BARIA		
11	07-01-01	B. BARIA	11	07-02-02	B. BARIA	11	08-02-01	COMILLA		
12	07-01-02	B. BARIA	12	07-03-01	B. BARTA	12	08-02-02	COMILLA		
13	08-03-01	COMILLA	13	08-01-01	COMILLA	13	09-01-01	NOAKHAL		
14	08-03-02	COMILLA	14	08-01-02	COMILLA	1.4	09-01-02	NOAKHAL		
15	08-04-01	COMILLA	15	08-01-03	COMILLA	15	10-03-01	LAXMIPUR		
16	08-04-02	COMILLA	16	09-03-01	NOAKHAL	16	11-03-02	NARSINGDI		
17	08-04-03	COMILLA	17	10-01-01	LAXMIPUR	17	13-01-03	DHAKA		
18	09-02-01	NOAKHALI	18	11-01-01	NARSINGDI	18	14-04-01	MUNSHIGONJ		
19	09-02-02	NOAKHALI	19	12-01-02	FARIDPUR	19	14-05-01	MUNSHIGONJ		
20	10-02-01	LAXMIPUR	20	13-02-02	DHAKA	20	15-01-01	GAZIPUR		
21	11-02-01	NARSINGDI	21	14-03-01	MUNSHIGONJ					
22	11-02-02	NARSINGDI	22	15-02-01	GAZIPUR					
23	11-02-03	NARSINGDI					,	* <u></u>		
23	11-02-03	NARSINGDI								
24	11-02-04	NARSINGDI	:			-				
25	12-02-01	FARIDPUR								
26	13-03-01	DHAKA								
27	13-03-02	DHAKA	-							
28	14-01-01	MUNSHIGONJ								
29	14-01-02	MUNSHIGONJ								
30	14-02-01	MUNSHIGONJ						<u> </u>		
31	14-02-02	MUNSHIGONJ								
32	15-03-01	GAZIPUR						<u> </u>		
Sı	ub-total: 3	32 Bridges	Ş۱	ub-total:	22 Bridges	S	ub-total:	20 Bridges		
			P	rotal: 74	Bridges	<u> </u>				

DESIGN CRITERIA 4.2

Based on the design principles in Section 4.1, the design criteria for designing the portable steel bridge were developed as follows:

(1) Basic specifications

The basic specifications of the portable steel bridge as agreed between Bangladesh officials involved and the Study Team are as follows:

Structure type

: Pony truss type

Design live load

: AASHTO HS-15

Bridge span length: 80 ft' (approx. 25 m)

Painting

: Galvanized

(2) Design standard specification

AASHTO (the American Association of State Highway and Transportation Officials) Standard Specification for Highway Bridges, 15th Edition, 1992 is applied to the project as commonly used for designing road bridges in Bangladesh. In addition, Japan Road Association's Highway Bridge Standard Specification is applied to provisions which AASHTO does not specify.

(3)Standard design

The following standard designs prepared by LGED are used in planning substructures and approach roads:

- Road Structure Manual, Part A Standard Design
- Road Structure Manual, Part B Guidelines and Design Criteria
- Standard Specifications and Schedule Rates

(4) Site conditions

Topographic surveys, geological surveys and hydrological analyses for bridge design were not done in the site investigation. In the Basic Design Study, the bridges will be planned based on following collected site condition data:

- Existing bridge length and river width measured by tape
- Water depth measured by tape
- Topographic sketch (plan and profile)
- Soil type observed at river bed
- Low water level (LWL) and High water level (HWL) surveyed by hearing
- Current velocity measured by ocular survey
- Traffic conditions of roads and rivers

(5) Design criteria governed by local conditions

a) Width of bridge

The width of bridge is 3.35 m (11 ft') which comes from the width of heavy vehicles (2.75 m) plus a side allowance (0.3 m) on both sides of the vehicle. The side allowance is minimized to 0.3 m since heavy vehicles will rarely pass over the project bridges.

b) Earthquake force

No provision for earthquake force is given for designing portable bridges. Earthquake coefficients specified in design guidelines for highway bridges and buildings are applied to the project. The following earthquake coefficients are given in the guidelines:

Northern Bangladesh : F = 0.08Central and eastern Bangladesh : F = 0.05Central and southern Bangladesh : F = 0.04 Since the project area is in central, eastern and southern Bangladesh, a earthquake coefficient of F = 0.05 is applied to the design of all the bridges of the project.

c) Wind force

Despite the prevalence of destructive cyclones, no provision for wind force is given in Bangladesh design guidelines. The wind force specified in AASHTO, equivalent to Japan's, is applied to the project. The wind force W is as follows:

W = 75 lb/sq.ft (for truss type structures)

d) Temperature range

Based on local climate data, a temperature range between +10°C and +50°C is applied for designing bearing shoes.

e) Freeboard

To prevent the bridges from being hit by drifting objects and obstructing flood discharge, 1.0 m high spaces between HWLs and the bottom of bridges are given in planning bridge heights. Where the rivers have boat traffic, the size of free-board is decided based on the height of the boats.

4.3 STRUCTURE OF BRIDGES

4.3.1 Superstructure (Portable Steel Bridge)

The structures of the portable steel bridges developed based on the design principals and the design criteria are as follows:

(1) Bridge member size

No member is designed toweigh more than 250 kg so as to be carried by hand.

(2) Strength of the materials

Common and inexpensive materials such as SS400 and SM490Y are designed to be used in major parts of the portable steel bridges.

(3) Tightening

High tension bolts (HTB) are designed to be used to connect truss members. Tightening with HTBs is commonly used for structural connection since it is reliable.

(4) Painting

The truss members are designed to be galvanized to be maintenance free, and the steel deck panels are designed to be painted because they can be disassembled for repainting.

(5) Deck slab

Steel panel type deck slabs are designed to lighten the bridge weight and to shorten the construction period.

(6) Pony truss

Pony truss structures were discussed by comparing several truss types as shown in Table 4-2. In the table, truss type Nos. 1 to 4 are ready-made trusses, while truss type Nos. 5 and 6 were newly proposed in this Study. Costs (which depend on steel weight), structural features and ease of construction were evaluated. As a result, Scheme No. 6 was selected for this project.

4.3.2 Substructures

In the Basic Design Study, standard types of substructures which are suitable for this project were developed for basic bridge planning. The standard substructures are proposed for detailed design of the substructures which will be conducted by LGED.

(1) Substructure types

The standard types of substructures given in LGED design guidelines are as follows:

Abutment: Inverted-T wall type abutment
Pier: Inverted-T column type pier

Where piers are located but the LWL is too deep to construct pier foundations, pile-bent type piers constructed with cast-in-place concrete piles are proposed.

The proposed standard abutments and piers are presented in Figures 4-1 to 4-3.

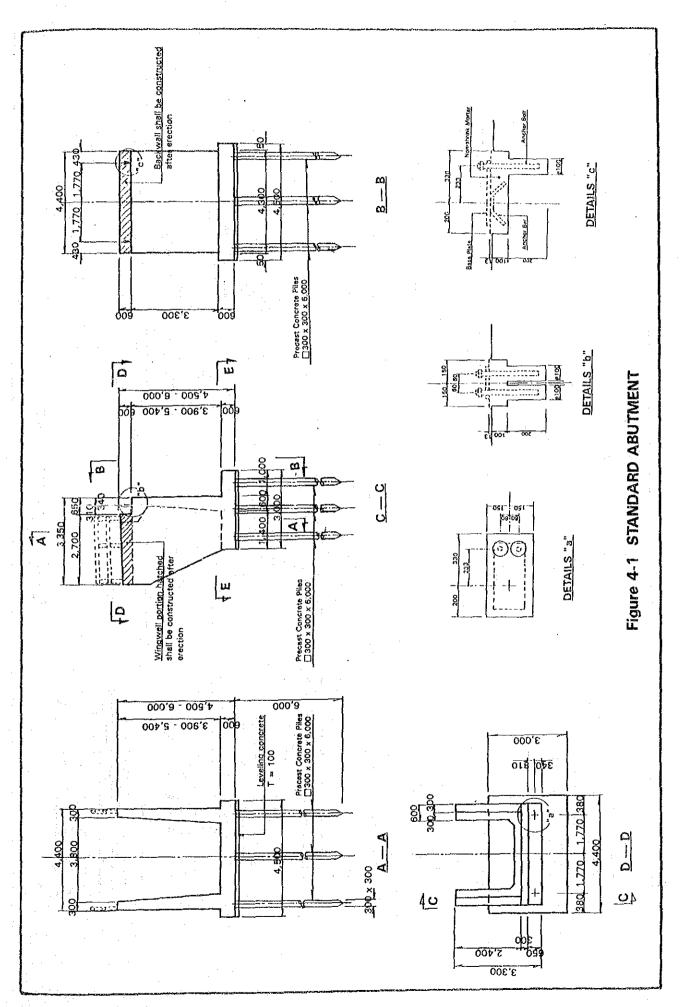
(2) Height of substructures

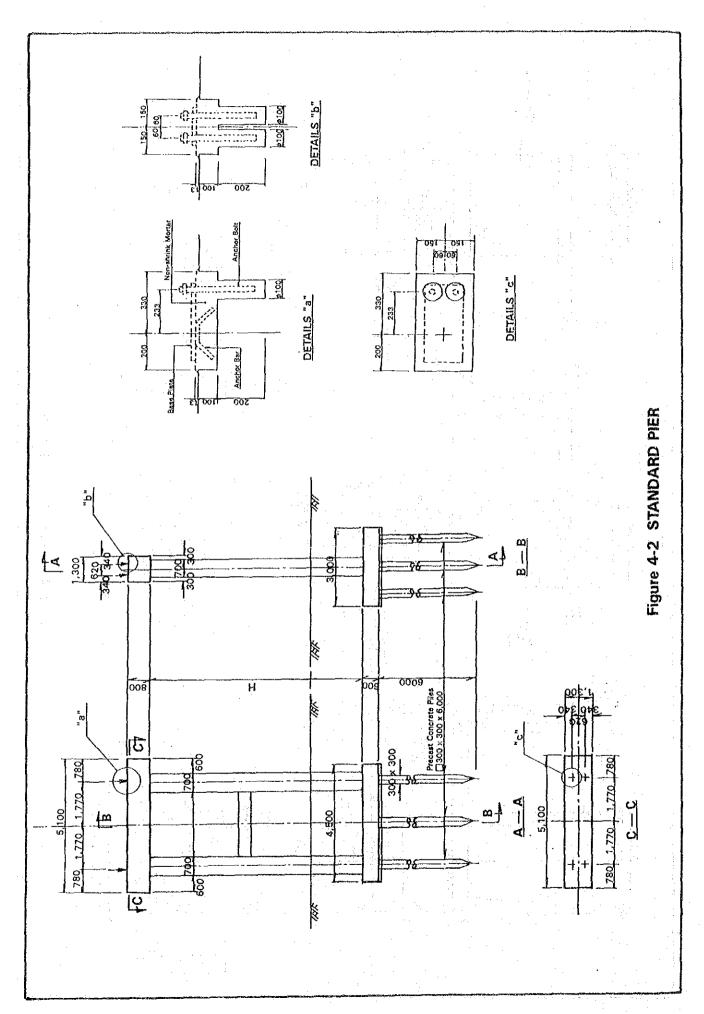
The height of the substructures "H" is given by the following formula:

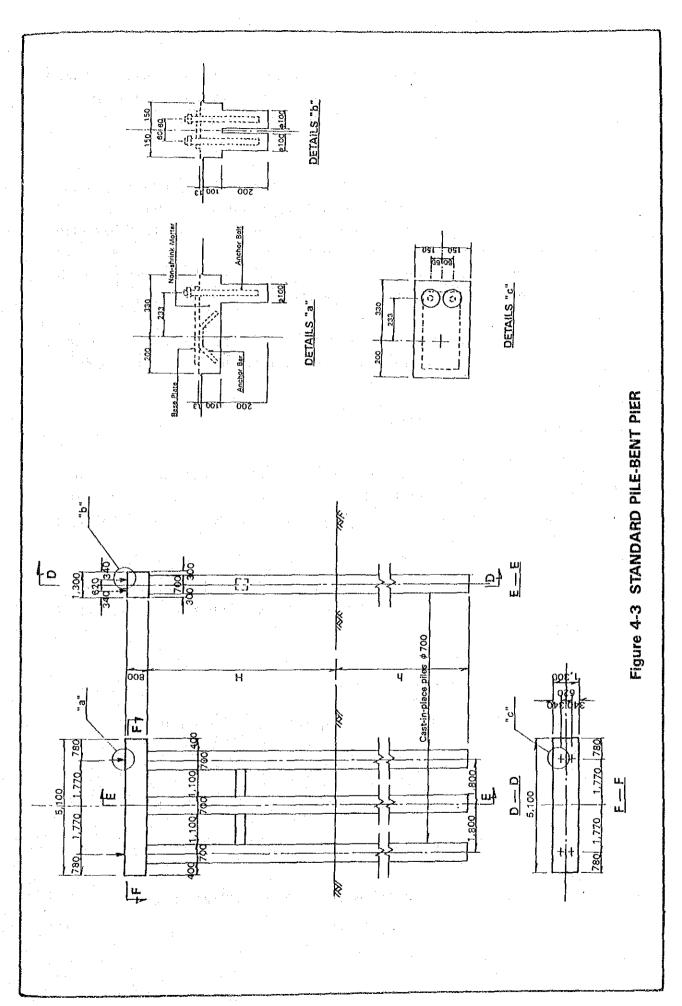
H = "Elevation of HWL" + "Freeboard" - "Elevation of footing"

Table 4-2 COMPARATIVE STUDY ON PONY TRUSS STRUCTURE

No.	Type of Truss	Section of Chord Member	Steel Deck Panel	Bridge Weight (t/m)	Max. Weight per Member (kg)	Structural Feature	Construction Easiness	Evaluation
1.	11 ×2272,7 = 25000	10 10 00 00 200	1825 1-6 1-75*75*9	Girder 0.423 Slab 0.291 Total 0.714	Siab 170	 Chord is welded H-beam. Gusset plates are welded. Flexural rigidity of slab is lower than other schemes. (I = 220 cm⁴) 	Members are light.Members are short.Erection is easy.	Deformation of the members may occur during transportation because gussets are incorporated into chord member.
2.	10 ×2500=25000	12 12 11 11 8	3350 500 81 D-100 1100 132	Girder 0.708 Slab 0.366 Total 1.074	Slab 160 Chord 170 Diagonal 65 Cross Beam 215	 All members are rolled steel. Bridge is comparatively heavy because stringer members are required due to slab structure. (I = 935 cm⁴) 	Members are light.Members are short.Erection is easy.	 This scheme is economically inferior to other schemes due to weight of bridge.
3.	13 ×1923 = 25000	10 10 10 10 10 10 10 10 10 10	1995 # t=45" "	Girder 0.564 Slab 0.420 Total 0.984	Slab 260 Chord 277	 All members are rolled steel. Bridge is composed of many members. Truss panel unit is heavy. (I = 7,500 cm⁴) 	 Slab unit is heavy. Erection is relatively difficult. 	 Erection is not easy as other schemes, since members are comparatively heavy and long.
4.	8 ×3125 = 25000	11 11 11 21 175	3043 t=45 H-150×75×5×7	Girder 0.499 Slab 0.472 Total 0.971	Oldb 207	 All members are rolled steel. Steel slab is heavy because panel is comparatively long. (I = 3,077 cm⁴) 	 Slab unit is little heavy. Erection is relatively difficult. 	 Erection is not easy as other schemes, since members are comparatively heavy and long.
5.	10×2500=25000	10 10 0	2443 XS-33 T=45 975 H-150 x75 x5 x7	Girder 0.348 Slab 0.199 Total 0.547	Chord 79 Diagonal 65	smaller than No.1	 Members are light. Some members are little long. Erection is easy. 	 Flexural rigidity of bridge and erection easiness of this scheme is inferior to Scheme No.6.
6.	10 ×2500 = 25000	10 10 0	2443 1-32 1-45 H-150×75×5×7	Girder 0.380 Slab 0.306 Total 0.686	Slab 187 Chord 79	 All members are rolled steel. Chord member is as small as No.5. Flexural rigidity of bridge is higher than No.5 because of warren truss type. (I = 2,700 cm⁴) 	Diagonal member is	 This plan is economical and very superior for erection. Modification of span length is very easy due to simple truss structure. Likewise, this plan is far superior to other plans.







The elevation of footings should be planned deep enough into the ground so that the footings will never be damaged by scouring.

(3) Piles

Pile foundations are required for most project substructures, since the project area is covered with clayey soil. For driving pre-cast concrete piles, drop-hammers are expected to be used because diesel hammer driving machines are difficult to procure. The size of the pre-cast concrete piles are $0.3 \text{ m} \times 0.3 \text{ m} \times 6.0 \text{ m}$ which decided based on the capacity of the driving hammers.

0.7 m diameter cast-in-place concrete piles constructed by the reverse circulation method are planned for the pile-bent type piers.

4.3.3 Approach Roads

Approach roads are planned to connect between the project bridges and existing roads. The typical cross section is proposed as shown in Figure 4-4 which is based on standard Type-B feeder roads in LGED design guidelines.

The vertical alignment of the approach roads should be planned to be smooth and to provide enough sight distance. The slope grade should not be greater than 6%.

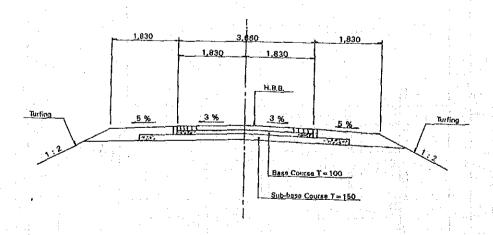


Figure 4-4 TYPICAL CROSS SECTION OF APPROACH ROADS

4.3.4 River Protection

River protections is planned where scouring at abutment foundations and approach embankments is foreseen.

The proposed structure of river protection is shown in Figure 4-5. The backfill of the river protection may be replaced by crushed bricks which are commonly available in Bangladesh.

The footings of the river protections should be embedded deep enough into the ground to be safe against future scouring.

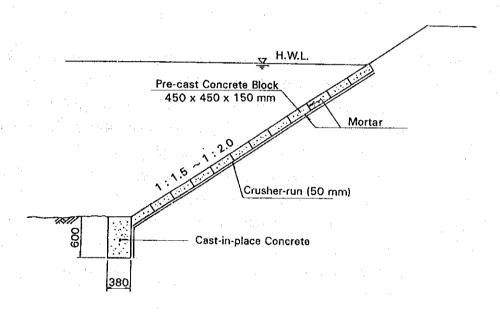


Figure 4-5 TYPICAL CROSS SECTION OF RIVER PROTECTION

4.4 BASIC DESIGN OF BRIDGES

4.4.1 Basic Bridge Planning

(1) Planning of bridge length

Almost all the rivers crossing the project bridge sites have neither river dikes nor plans for river dike construction. Imaginary river dikes were planned based on site investigation data to find the optimum location of abutments.

The bridge lengths were decided by the distances between abutments as shown in Figure 4-6.

To examine the locations of the imaginary river dikes, the following were taken into consideration:

- River widths between imaginary dikes are wide enough to discharge floods.
- Floods flow smoothly and do not endanger abutments and approach embankments by scouring.
- Countermeasures can be taken against changes in river alignment.
- Bridge lengths are not so long as to be unnecessary or uneconomical.

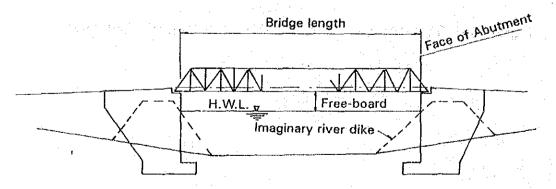


Figure 4-6 PLANNING OF BRIDGE LENGTH

(2) Planning of span length

The longest span is 25 m as decided in Section 4.2. The shortest span is 15 m which was decided as the shortest bridge length. The spans of the portable steel bridge were planned to be of 3 types, that is, 15 m, 20 m and 25 m. (Refer to Figure 4-7 Span Type of Portable Steel Bridge.)

To arrange the spans to compose the planned bridge lengths, the 25 m span was the maximum applied, because the longer the span the lower the cost for substructure construction and the less obstruction against flood discharge.

(3) Basic bridge planning

Based on the standard structures and the planning method developed herein, basic bridge planning of the project bridges was conducted and general views of the bridges were drawn.

The summary of basic bridge planning of the project bridges compiled by each phase is presented in Tables 4-3 to 4-5.

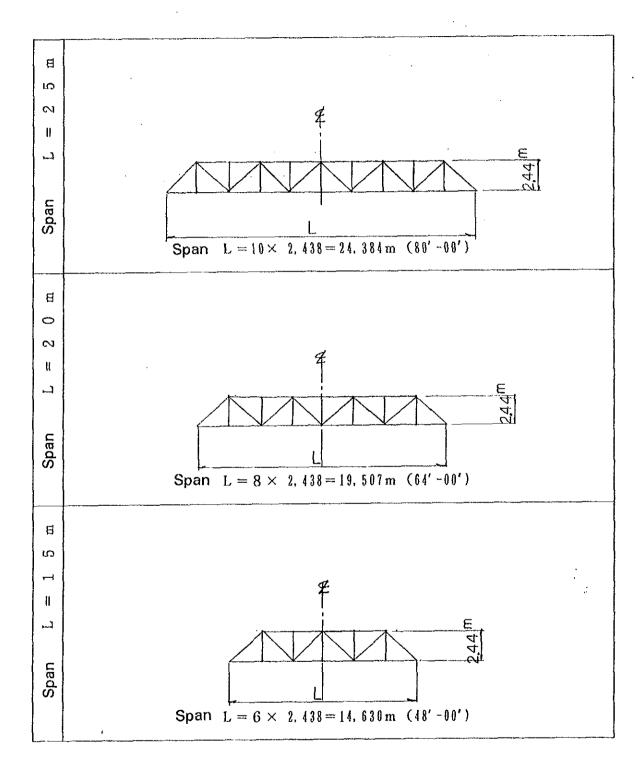


Figure 4-7 SPAN TYPE OF PORTABLE STEEL BRIDGE

Table 4-3 SUMMARY OF BASIC BRIDGE PLANNING (PHASE 1 BRIDGES) (1/2)

			Substructure	Piles	Approach	River
			· · · · · · · · · · · · · · · · · · ·	Pre-cast	Roads	Protection Up: Right
No	Bridge No.	Layout of Bridge	A : Abutment	(Length x Nos.)	Up: Right bank	bank Dn: Left
140.	Location		P: Pier	(Cast-in- place	Dn: Left bank (Length)	bank (Height x Length)
			H : Height (m)	(Length x Nos.))		
1	01-01-01	A1 25m P1 25m P2 25m A2	A_1 : $H = 4.5$ $P_1 \sim P_2$: $H = 7.5$	6. 0×36	10.0	4.5 × 10.0
	COX, 2 BYSYL	L = 75 m, W=50.8t	A ₂ : H = 4, 5		10.0	4.5 ×10.0
2	01-02-01	A1 25# P1 25# P2 25# P3 25# P4 25# A2	A_1 : $H = 4.5$ P_1 : $H = 5.0$	6, 0×54	15. 0	4.5 ×10.0
	COX'S BAZAR	L = 125 m, W = 84.6 t	$\begin{vmatrix} P_{2} \sim P_{4} & : H = 7.0 \\ A_{2} & : H = 4.5 \end{vmatrix}$		15. 0	4.5 ×10.0
-		A1 20m P1 20m A2	$A_1 : H = 4.5$	<u> </u>	10. 0	4, 5 × 10. 0
3	02-02-01	$ \begin{array}{cccc} & \Delta & \Delta & \Delta \\ & \Delta & \Delta & \Delta \\ & L = 40 \text{ m}, W = 27.1 \text{ t} \end{array} $	P ₁ : H= 8.0 A ₂ : H= 4.5	6. 0×27	10. 0	4.5 × 10.0
_	FENI				10. 0	4. 5 × 10. 0
4		A1 20 P1 20 A2	A_1 : $H = 4.5$ P_1 : $H = 8.0$	6. 0×27		
	FENI	L = 40 m, W=27.1 t	A ₂ : H = 4.5		10.0	4.5 ×10.0
5	03-02-01	A1 25 P1 25 P2 25 A2	$\begin{array}{c} A_1 & : H = 4.5 \\ P_1 \sim P_2 : H = 10.0 \end{array}$	6. 0×27	15. 0	4 5 ×10.0
	CHITTAGONG	L = 75 m, W=50.8 t	A_2 : $H = 4.5$		10.0	4.5 ×10.0
6	04-01-01	A1 20m P1 25m P2 20m A2	A_1 : H = 5.5 $P_1 \sim P_2$: H = 11.0	6. 0×50	15.0	4.0 ×10.0
	RABIGON)	L = 65 m, W=44.1t	$A_2 : H = 5.5$		15.0	4.0 ×10.0
7	05-01-01	A1 20m A2	$A_1 : H = 4.5$	6. 0×18	10.0	3.5 × 10.0
	MONTALBYSYK	L = 20 m. W=13.6t	A_2 : $H = 4.5$		10. 0	3.5 × 10.0
8.	05-01-02	A1 20 P1 20 A2	A_1 : $H = 4.5$ P_1 : $H = 6.0$	6. 0×27	10.0	3. 5 × 10. 0
	HOULVIBAZAR	L = 40 m, W = 27.1 t	$A_2 : H = 4.5$	0. 0. 2,	10. 0	3.5 ×10.0
9	06-03-01	A1 20m P1 20m A2	A ₁ : H = 4.5	. nv17	10.0	3. 5 × 10. 0
}	CHANDOPOR	L = 40 m, W=27.1 t	P_1 : H=10.0 A_2 : H= 4.5	6. 0×27	10.0	3, 5 × 10. 0
10	06-03-02	A1 25 ■ A2	$A_1 : H = 4.5$		10.0	3, 5 × 10. 0
	CHANCOPUR	L = 25 m, W = 16.9 t	A ₂ : H = 4.5	6.0×18	10.0	3. 5 × 10. 0
11	07-01-01	A1 25m A2	A ₁ : H = 4.0		10.0	3.0 ×10.0
	B. BARIA	L = 25 m, W = 16.9 t	$A_2 : H = 4.0$	6.0×18	10.0	3. 0 ×10. 0
12	07-01-02		$A_1 : H = 5.0$		10.0	4. 0 × 10. 0
,,,	er 23 (V.)	A1 25m P1 25m P2 25m P3 25m A2	P_1 , P_2 : $H = 6.0$ P_3 : $H = 10.0$	6. 0×45		
	B. BARIA	L=100 m, W=67.7 t	A_2 : $H = 5.0$		10.0	4. 0 ×10. 0
13	08-03-01	A1 15m P1 15m A2	A_1 : H = 4.5 P ₁ : H = 7.0	6. 0×27	10.0	3. 5 × 10. 0
	COMILLA	L = 30 m, W=20.5 t	$A_2 : H = 4.5$		10.0	3. 5 × 10. 0
14	08-03-02	A1 25 ■ A2	A ₁ : H= 4.5	£ 0×10	10.0	3.5 × 10.0
	CONFLEX	L = 25 m, W = 16.9 t	$A_2 : H = 4.5$	6. 0×18	10.0	3. 5 × 10. 0
15	08-04-01	A1 25m A2	A ₁ : H = 4.5		10. 0	3.5 × 10.0
	COMILLA	L = 25 m, W = 16.9 t	A ₂ : H = 4.5	6.0×18	10.0	3. 5 × 10. 0
16	08-04-02		A ₁ : H = 4.5	<u> </u>	10. 0	3. 5 × 10. 0
"		$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	P_1 : $H = 7.5$ A_2 : $H = 4.5$	6. 0×27	10.0	3. 5 × 10. 0
L	COMILLY	1 30 HL W - 20. 3 C	1 1 1 1 1	<u></u>	1	1,,,,,,,,,,

Table 4-3 SUMMARY OF BASIC BRIDGE PLANNING (PHASE 1 BRIDGES) (2/2)

_			Substructure	Piles	Approach Roads	River Protection
No.	Bridge No. / Location	Layout of Bridge	A : Abutment	Pre-cast (Length x Nos.) [Cast-in-	Up : Right bank On : Left bank	Up: Right bank Dn: Left bank
	2550000		H: Height (m)	place (Length x Nos.)]	(Length)	(Height x Length)
17	08-04-03	A1 20m P1 25m P2 20m A2	A_1 : $H = 4.5$ $P_1 \sim P_2$: $H = 7.0$	6. 0×36	10.0	3, 5 × 10, 0
	COMILLA	L = 65 m, W = 44.1 t	A ₂ : H = 4.5		10. 0	3.5 ×10.0
18	09-02-01	, A1 <u>20π</u> A2 Δ	$A_1 : H = 4.5$	6. 0×18	10.0	3.5 × 10.0
	NOAKBALI	L = 20 m, W=13.6 t	A_2 : $H = 4.5$		10. 0	3. 5 × 10. 0
19	09-02-02	A1 _ 15# _ A2	$A_1 : H = 4.5$	6. 0×18	10.0	3.5 ×10.0
	NOAKBALI	L = 15 m, W = 10.2 t	A_2 : H = 4.5		10.0	3.5 × 10.0
20	10-02-01	Λ1 25m P1 25m P2 25m A2 Δ ΔΔ ΔΔ ΔΔ Δ	A_1 : $H = 4.5$ $P_1 \sim P_2$: $H = 7.0$	6. 0×18	10.0	3.5 ×10.0
	LAXIPOR	L= 75 m, W=50.8t	A ₂ : H = 4.5	[22. 0×6]	10.0	3.5 ×10.0
21	11-02-01	A1 25m A2	$A_1 : H = 4.5$	6. 0×18	10.0	3.5 ×10.0
	HARSINGDI	L = 25 m \ W=16.9 t	A ₂ : H = 4.5		10. 0	3.5 ×10.0
22	11-02-02	A1 <u>15</u> ■ A2	A_1 : $H = 4.5$	6. 0×18	10.0	3.5 ×10.0
	NARSINGDI	L = 15 m, W=10.2 t	A ₂ : H= 4.5		10. 0	3.5 ×10.0
23	11-02-03	A1 <u>15m</u> A2	$A_1 : H = 4.5$	6. 0×18	10. 0	3.5 ×10.0
	NARSINGDI	L = 15 m, W = 10.2 t	A ₂ : H = 4.5	-	10.0	3. 5 ×10. 0
24	11-02-04	A1 20 P1 25 P2 20 A2	A_1 : H = 4.5 $P_1 \sim P_2$: H = 9.0	6. 0×36	15. 0	3.5 ×10.0
	NARSINGDI	$L = 65 \text{ m} \cdot \text{W} = 44.1 \text{ t}$	A_2 : $H = 4.5$		15. 0	3.5 ×10.0
25	12-02-01	A1 20m P1 25m P2 20m A2	A_1 : $H = 4.5$ $P_1 \sim P_2$: $H = 6.0$	6. 0×36	10.0	3.5 × 10.0
	FARIDPUR	L = 65 m, W=44.1 t	A_2 : $H = 4.5$		10.0	3. 5 ×10. 0
26	13-03-01	A1 20m P1 25m P2 25m P3 25m P4 20m A2	A_1 : H = 4.5 P_1 , P_4 : H = 7.0 $P_2 \sim P_3$: H = 10.5	6. 0×18	10.0	3.5 ×10.0
	DBAKA	L=115 m, W=?7, 9 t	A_2 : $H = 4.5$		10.0	3.5 ×10.0
27	13-03-02	A1 15m P1 15m A2	A_1 : $H = 4.5$ P_1 : $H = 6.0$	6. 0×27	10, 0	3.5 ×10.0
	DHYKY	L = 30 m, W=20.5 t	$\begin{array}{ccc} A_2 & : H = 4.5 \end{array}$		10.0	3.5 ×10.0
28	14-01-01	A1 20 ■ A2	$A_1 : H = 4.5$	6. 0×18	10.0	3. 5 ×10. 0
	NUNSBICONI	L = 20 m, W=13.6 t	A ₂ : H = 4.5		10.0	3.5 ×10.0
29	14-01-02	Λ1 <u>20m</u> Λ2	A ₁ : H= 4.5	6. 0×18	10.0	3.5 ×10.0
	MENSHIGONI	L = 20 m, W=13.6 t	A ₂ : H = 4,5		10.0	3.5 × 10.0
30	14-02-01	Λ1 20 m P1 20 m Λ2 Δ ΔΔ Δ	A_1 : $H = 4.5$ P_1 : $H = 8.0$	6. 0×18	10.0	3.5 ×10.0
	HUNSHIGONI	$L = 40 \text{ m} \cdot W = 27.1 \text{ t}$	A_2 : $H = 4.5$	[23. 0x 3]	10. 0	3.5 ×10.0
31	14-02-02	A1 20m P1 20m A2	A ₁ : H = 4.5 P ₁ : H = 5.0	6. 0×18	10, 0	3. 5 × 10. 0
	DONSHIGON)	L = 40 m, W = 27.1 t	P_1 : H = 5.0 A_2 : H = 4.5	[20, 0×3]	10.0	3.5 ×10.0
32	15-03-01	A1 20m P1 25m P2 20m A2	A_1 : $H = 4.5$ $P_1 \sim P_2$: $H = 7.0$	K U > 3K	10. 0	3.5 × 10.0
	GAZIPOR	L = 65 m, W = 44.1 t	$A_2 = H = 4.5$	6. 0×36	10.0	3.5 × 10.0

Table 4-4 SUMMARY OF BASIC BRIDGE PLANNING (PHASE 2 BRIDGES) (1/2)

[]			Substructure	Piles	Approach Roads	River Pratection
	Bridge No.	Layout of Bridge	A : Abutment	Pre-cast (Length x	Up : Right	Up : Right bank
No.	/ Location	Layout of Bhage	P: Pier	Nos.) (Cast-in-	bank Dn : Left bank	Dn : Left bank (Height x
			H: Height (m)	place (Length x Nos.))	(Length)	Length)
1	02-01-01	A1 25m P1 25m P2 25m A2	A_1 : H = 6.0 $P_1 \sim P_2$: H = 9.5	6.0×18	10. 0	5, 0 × 10.0
	CHITTAGONG	L = 75 m, W=50.8 t	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	6. 0×18 [25. 0×6]	10. 0	5. 0 ×10. 0:
2	03-03-01	A1 25m P1 25m P2 25m A2	A_1 : H = 4.5 P_1 : H = 6.5	6. 0×36	10. 0	4, 5 × 10.0
	CHITTAGONG	L = 75 m, W = 50.8 t	P_2 : H = 9.0 A_2 : H = 4.5		10. 0	4, 5 × 10. 0
3	03-04-01	A1 20m P1 20m A2	A, : H = 4.5	6. 0×18	15. 0	4.5 ×10.0
	FEHI	$\Delta = \Delta \Delta \Delta$ $L = 40 \text{ m}, W = 27.1 \text{ t}$	P_1 : H = 6.5 A_2 : H = 4.5	[25. 0×3]	15. 0	4.5 ×10.0
l a	04-02-01	A1 25m P1 25m P2 25m A2	A ₁ : H = 4.5	:	10.0	3.5 ×10.0
	HABIGONI	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	$ \begin{array}{ccc} P_1 \sim P_2 : H = 8.0 \\ A_2 : H = 4.5 \end{array} $	6, 0×36	10. 0	3.5 ×10.0
5	04-03-01	Λ1 20m P1 20m Λ2	A, : H = 4.5:	6.0 × 27	15. 0	3, 5 ×10, 0
	BABICONJ	L = 40 m, W = 27.1 t	$\begin{array}{ccc} P_1 & : H = 8.0 \\ A_2 & : H = 4.5 \end{array}$	6, 0×27	15. 0	3, 5 ×10, 0
6	05-02-01	λ1 25 m λ2	$A_1 : H = 4.5$	6.0×18	10.0	3,5 ×10.0
	MOULVIBAZAR	L = 25 m. W=16.9 t	A ₂ : H = 4.5		10.0	3, 5 × 10. 0
7	05-02-02	Λ1 <u>20m P1 20m</u> Λ2 Δ ΔΔ Δ	$A_1 : H = 4.5$ $P_1 : H = 7.0$	6. 0×27	10. 0	3.5 ×10.0
	MOULVIBAZAR	L = 40 m, W = 27.1 t	A ₂ : H = 4.5		10. 0	3.5 ×10.0
8	06-01-01	A1 25m A2	L = 40 m, W = 27.1 t A_2 : H = 4.5 A_1 : H = 4.5 A_2 A_3 : H = 4.5 A_4 A_4 : H = 4.5 A_4 A_5 A_5 A_5 A_6 A_7 A_8 A_8 A_8 A_9	10, 0	3.5 ×10.0	
	CHAHDOPUR	L = 25 m, W=16.9 t	A ₂ : H = 4.5		10.0	3.5 × 10.0
9	06-02-01	A1 <u>15■</u> A2	$A_1 : H = 4.5$	6. 0×18	10. 0	3.5 ×10.0
	CHANDOPUR	L = 15 m \ W=10.2 t	A_2 : H = 4.5		10.0	3. 5 × 10. 0
10	07-02-01	A1 15m P1 15m A2	A_1 : $H = 4.5$ P_1 : $H = 7.0$	6. 0×27	10.0	3. 5 × 10. 0
	B. BARIA	L = 30 m, W=20.5 t	A ₂ : H == 4.5		10. 0	3.5 × 10.0
11	07-02-02	A1 <u>∆15∎</u> ∆A2	A: : H= 4.5	6. 0×18	10.0	3.5 ×10.0
	B. BARIA	L = 15 m, W=10.2 t	A ₂ : H = 4.5		10. 0	3. 5 ×10. 0
12	97-93-01	A1 20 P1 25 P2 20 A2 △	$ \begin{array}{cccc} A_1 & : H = 4.5 \\ P_1 \sim P_2 & : H = 8.0 \end{array} $	6. 0×36	10.0	3. 0 ×10. 0
	B. BARIA	L = 65 m. W=44.1t	$A_2 : H = 4.0$		10.0	3. 0 ×10. 0
13	08-01-01	Λ1 25m Λ2	A ₁ : H = 4.5	6. 0×18	10.0	3.5 ×10.0
	CONITTY	L = 25 m, W=16.9 t	$A_2 : H = 4.5$		10.0	3.5 ×10.0
14	08-01-02	A1 25 ■ A2	$A_1 : H = 4.5$	6. 0×18	10.0	3. 5 × 10. 0
	COMILLA	L = 25 m, W=16.9 t	$A_2 : H = 4.5$		10.0	3.5 × 10.0
15	08-01-03	$\frac{\text{A1}}{\Delta} = \frac{20 \text{m}}{2 \text{M}} = \frac{\text{A2}}{\Delta}$	$\begin{array}{ccc} A_1 & : H = 4.5 \\ P_1 & : H = 8.0 \\ \end{array}$	6. 0×27	10.0	3. 5 × 10. 0
	CONIFFY	$L = 40 \text{ m} \cdot \text{W} = 27.1 \text{ t}$	$A_2 : H = 4.5$	<u> </u>	10.0	3.5 ×10.0
. }	09-03-01	A1 20m P1 20m A2	$\begin{array}{ccc} A_1 & : H = 4.5 \\ P_1 & : H = 8.0 \end{array}$	6. 0×27	15.0	3.5 × 10.0
_	ROAKHALI	L = 40 m, W = 27.1 t	$\Lambda_2 : H = 4.5$	<u>L</u>	15, 0	3.5 × 10.0

Table 4-4 SUMMARY OF BASIC BRIDGE PLANNING (PHASE 2 BRIDGES) (2/2)

			Substructure	Piles	Approach Roads	River Protection
No.	Bridge No. / Location	Layout of Bridge	A: Abutment P: Pier H: Height (m)	Pre-cast (Length x Nos.) (Cast-in- place (Length x Nos.))	Up: Right bank On: Left bank (Length)	Up: Right bank Dn: Left bank (Height x Length)
17	10-01-01 Laxipur	A1 $15m$ P1 $15m$ A2 $\Delta \Delta \Delta$	A ₁ : H = 4.5 P ₁ : H = 8.5 A ₂ : H = 4.5	6. 0×27	15. 0 15. 0	3. 5 × 10. 0 3. 5 × 10. 0
18	11-01-01 Narsingdi	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	A_1 : H = 4.5 $P_1 \sim P_2$: H = 7.0 A_2 : H = 4.5	6. 0×36	10. 0 10. 0	3. 5 ×10. 0 3. 5 ×10. 0
19	12-01-02 Faridpur	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	A_1 : H = 4.5 $P_1 \sim P_2$: H = 8.0 A_2 : H = 4.5	6. 0×36	10. 0 10. 0	3.5 ×10.0 3.5 ×10.0
20	13-02-02 Dhaka	$A1 20m P1 20m A2$ $\Delta \Delta \Delta \Delta$ $L = 40 m, W = 27.1 t$	A_1 : $H \approx 4.5$ P_1 : $H = 7.0$ A_2 : $H = 4.5$	6. 0×27	10. 0 10. 0	3.5 ×10.0
	14-03-01 NUNSBIGON)	A1 20m P1 25m P2 25m P3 25m P4 20m A2 L=115 m, W=77.9 t	A_1 : $H = 4.5$ P_1 , P_4 : $H = 7.5$ $P_2 \sim P_3$: $H = 11.5$ A_2 : $H = 4.5$	6. 0×18 [27. 0×6]	10. 0 10. 0	3. 5 ×10. 0
	15-02-01 GAZIPUR	A1 20 m P1 25 m P2 25 m P3 25 m P4 20 m A2 L=115 m, W=77.9 t	A_1 : $H = 4.5$ $P_1 \sim P_4$: $H = 5.0$ A_2 : $H = 4.5$	6. 0×54	10. 0 10. 0	3. 5 × 10. 0 3. 5 × 10. 0

Table 4-5 SUMMARY OF BASIC BRIDGE PLANNING (PHASE 3 BRIDGES) (1/2)

			C. b. aturatura	0.4	Approach	River
			Substructure	Piles Pre-cast	Roads	Protection
ļ.,	Bridge No.	Layout of Bridge	A : Abutment	(Length x Nos.)	Up : Right bank	Up : Right bank Dn : Left
No.	Location		우: Pier	(Cast-in- place	Dn : Left bank (Length)	bank (Height x
			H: Height (m)	(Length x Nos.)]	(cong.ii)	Length)
1	03-05-01	Λ1 25m Λ2	A_1 : H = 4.5	6. 0×18	10. 0	4.5 ×10.0
	CHITTACONG	L = 25 m, W = 16.9 t	A_2 : $H = 4.5$	V. V-10	10. 0	4. 5 × 10. 0
2	03-06-01	A1 25m P1 25m P2 25m P3 25m A2	A_1 : $H = 4.5$ P_1 , P_3 : $H = 6.5$	6.0×18	10.0	4.5 ×10.0
			P_2 : $H = 14.0$	[22. 0×6]	10.0	4.5 ×10.0
-	CHITTAGONG	L = 100 m, W = 67. 7 t	A ₂ : H = 4.5	[42. 0x 3]	10.0	
3	04-04-01	A1 25m P1 25m A2	A_1 : $H = .5, 0$ P_1 : $H = .9, 5$	6:0×18	10.0	4.0 ×10.0
	HABIGONG	L = 50 m, W = 33.8 t	$A_2 : H = 5.0$	[25, 0×3]	10.0	4 0 ×10 0
4	04-04-02	A1 25m P1 25m A2 Δ Δ Δ Δ	$ \begin{array}{ccc} A_1 & : H = 5.0 \\ P_1 & : H = 9.5 \end{array} $	6.0×18	15. 0	4. 0 ×10. 0
	HABIGONG	L = 50 m, W=33.8 t	A_2 : $H = 5.0$	(25. 0×3)	15.0	4.0 ×10.0
5	05-03-01	A1 25m P1 25m A2	A_1 : $H = 4.5$ P_1 : $H = 13.5$	6. 0×34	10.0	3.5 ×10.0
	MOULVIBAZAR	L = 50 m, W=33.8 t	A ₂ : H = 4.5	· .	10. 0	3. 5 ×10. 0
6	05-04-01	A1 20 m A2	$A_1 : H = 4.5$	6. 0×18	10.0	3.5 ×10.0
	MOULVIBAZAR	L = 20 m, W=13.6 t	A ₂ : II = 4.5		10.0	3.5 × 10.0
7	06-04-01	A1 15m P1 15m A2	A_1 : $H = 4.5$ P_1 : $H = 7.0$	6. 0×18	10.0	3.5 ×10.0
	CHANDOPUR	L = 30 m, W=20.5 t	A ₂ : H = 4.5	[22. 0×3]	10.0	3. 5 ×10. 0
8	06-04-02	A1 25m A2	$A_1 : H = 4.5$	6.0×18	10. 0	3.5 ×10.0
	CHANDOPUR	L = 25 m, W=16.9 t	$A_2 : H = 4.5$	0. 0 10	10. 0	3.5 ×10.0
9	07-03-02	A1 15m P1 15m A2	A ₁ : H = 4.5	6. 0×18	15. 0	3.5 ×10.0
	B. BARIA	$L = 30 \text{ m}_{\odot} \text{ W} = 20.5 \text{ t}$	P_1 : $H = 7.5$ A_2 : $H = 4.5$	[23. 0×2]	10. 0	3.5 ×10.0
10	07-04-01	A1 25m A2	A ₁ : H = 4.0		10.0	3. 0 ×10. 0
	8. 84814	L = 25 m, W = 16.9 t	$A_2 : H = 4.0$	6. 0×18	10. 0	3. 0 ×10. 0
11	08-02-01	A1 15m P1 15m A2	A ₁ : H = 4.5		10. 0	3. 5 × 10. 0
	CONTLLA	L = 30 m, W = 20.5 t	$P_1 : H = 8.0$ $A_2 : H = 4.5$	6. 0×27	10. 0	3.5 ×10.0
12.	08-02-02	A1 25 ≥ A2	$A_1 : H = 4.5$		10. 0	3.5 ×10.0
	COMILLA	L = 25 m, W = 16.9 t	A ₂ : H = 4.5	6. 0×18	10. 0	3 5 ×10 0
13	09-01-01		A ₁ : H = 5.0		10. 0	4. 0 ×10. 0
	NOARHALI	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	P_1 : H = 7.0 A_2 : H = 5.0	6. 0×27	10.0	4.0 ×10.0
14	09-01-02	A1 15m A2	A ₁ : H = 4.5		10. O	3. 5 × 10. 0
	NOARRALI	L = 15 m, W = 10.2 t	A ₂ : H = 4.5	6. 0×18	10.0	3.5 ×10.0
15	10-03-01		A ₁ : H = 4.5		10. 0	3.5 ×10.0
	LAXIPUR	A1 20m P1 25m P2 20m A2 \(\triangle \triangl	$P_1 \sim P_2 : H = 7.0$ $A_2 : H = 4.5$	6. 0×36	10.0	3.5 ×10.0
16				6. 0×18	15.0	3.5 ×10.0
10	11-03-02	A1 25 P1 25 P2 25 P3 25 P4 25 A2	$P_1 \sim P_4 : H = 7.5$		15. 0	3.5 × [0.0]
L,	HARSINGDI	L=125 m, W=84.6 t	A ₂ : H= 4.5	[23, 0.20]	13.0	カテヘ19.サ

Table 4-5 SUMMARY OF BASIC BRIDGE PLANNING (PHASE 3 BRIDGES) (2/2)

			Substructure	Piles	Approach Roads	River Protection
No.	Bridge No.	Layout of Bridge	A : Abutment	Pre-cast (Length x Nos.)	Up: Right bank Dn: Left	Up: Right bank Dn: Left
	Location		P: Pier H: Height (m)	[Cast-in- place (Length x Nos.)]	bank (Length)	bank (Height x Length)
17	13-01+03	A1 15m A2	$A_1 : H = 4.5$	6. 0×18	10. 0	3. 5 × 10. 0
	DRYKY	L == 15 m, W == 10.2 t	A_2 : $H = 4.5$	0. 0 × 10	10. 0	3.5 ×10.0
18	14-04-01	Λ1 25m P1 25m A2 Δ ΔΔ Δ	A_1 : H = 5.0 P_1 : H = 11.0	6. 0×18	10. 0	4. 0 × 10. 0
	MANSBIGONI	L = 50 m, W=33.8 t	$A_2 : H = 5.0$	[26. 0×3]	10.0	4. 0 ×10. 0
19	14-05-01	A1 20m P1 20m A2 Δ ΔΔ Δ	A_1 : $H = 4.5$ P_1 : $H = 5.5$	6. 0×18	10.0	3. 5 × 10. 0
	NGNSHIGON)	L = 40 m, W = 27.1 t	$A_2 : H = 4.5$	(21. 0×3)	10.0	3.5 × 10.0
20	15-01-01	A1 20m P1 25m P2 20m A2	A_1 : H = 4.5 $P_1 \sim P_2$: H = 9.0	6 0 2 3 6	10.0	3.5 ×10.0
	GAZIPUR	L = 65 m, W=44.1t	A_2 : $H = 4.5$	0 0 ~ 39	10.0	3.5 × 10.0

Basic Design of Superstructure (Portable Steel Bridge) 4.4.2

(1) Design criteria

The design criteria for designing the superstructure are as follows:

Design live load

: AASHTO HS-15

Bridge (carriageway) width: 11 ft' (3.35 m)

Superstructure type

: Pony warren truss

Span length

: 80 ft' (24.38 m), 64 ft' (19.507 m)

and 48 ft' (14.630 m)

Height of truss

: 8 ft' (2.44 m)

Tightening bolts

: HTB (F8T) M22

Deck slab

: Steel deck panel

Painting

: Galvanizing (truss members)

Painting (deck slabs)

Steel materials

: Rolled H-beam (SS400 and SM490Y)

plates and others (SS400)

(2) **Design Calculation**

A structural analysis of the superstructure (portable steel bridge) was conducted and the stability of the truss members was checked. The result of the design calculation of the 25 m span case is presented in Table 4-6. As shown in the Table, the stress of all the members of the truss under design loads is not greater than their allowable stress, and the bridge deflection by live load is not greater than the allowable deflection.

The truss members of the 25 m span bridge were designed to be used for 15 m and 20 m span bridges.

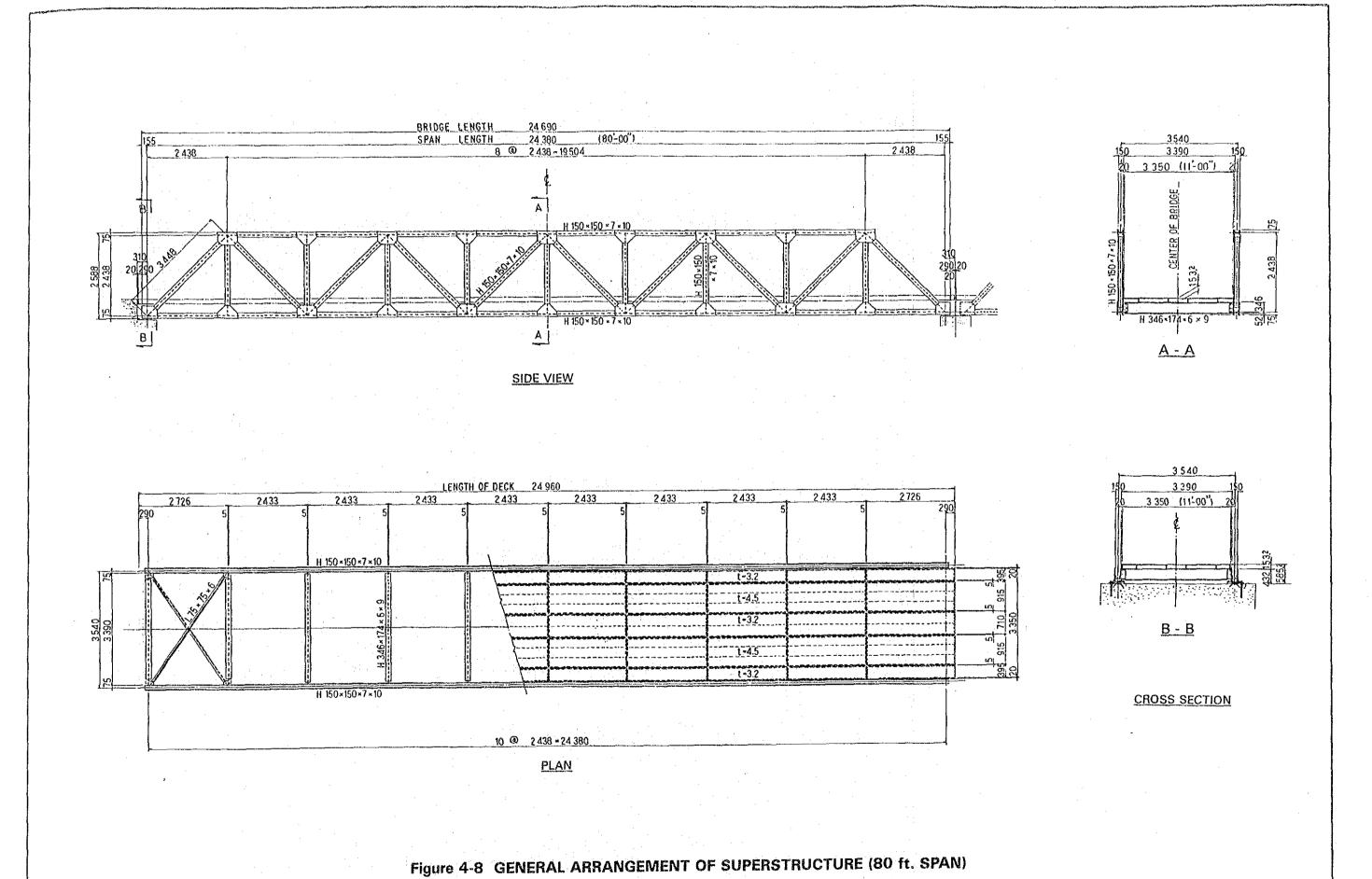
The deflection by dead loads is negligibly small (8 mm), so that no camber is required.

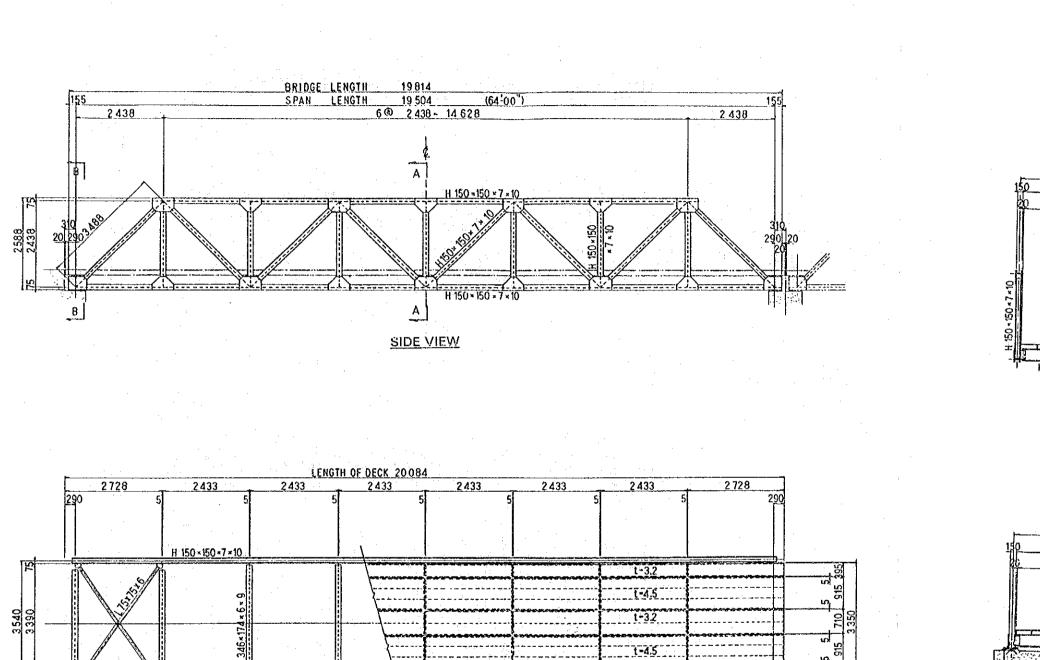
Table 4-6 RESULTS OF DESIGN CALCULATION OF SUPERSTRUCTURE

	Top Chord Bottom Chord Member		Vertical Member		
Shape	H-150x150x7/10	H-150x150x7/10	H-150x150x7/10	H-150x150x7/10	
Designation	SM490Y	SM490Y	SS400	SS400	
Radius of gyration of . area (cm)	3.75	3.75	3.75	3.75	
Sectional area (cm²)	40.14	40.14	40.14	40.14	
Axial force (ton)	-40,20	42.80	-18.10	7.40	
Axial stress (kg/cm²)	1,001	1,420	451	246	
Allowable axial stress (kg/cm²)	1,346	2,100	926	1,400	
Deflection	$ \delta_1 = 20 \text{m} $ $ \delta_d = 8 \text{m} $	n (= L / 1504) n			
Allowable deflection Allowable live load deflection $\delta a = 42 \text{mm}$ (= L /					

(3) Drawings of superstructure

Based on the design calculation, the drawings of the superstructure (portable steel bridge) were developed as shown in Figures 4-8 to 4-12.





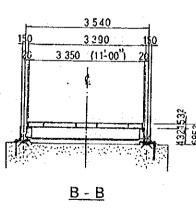
<u>PLAN</u>

80 2438 - 19504

H 150×150×7×10

Figure 4-9 GENERAL ARRANGEMENT OF SUPERSTRUCTURE (64 ft. SPAN)

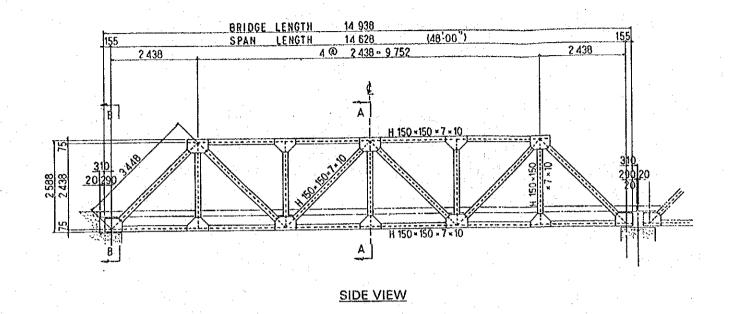
t -3.2

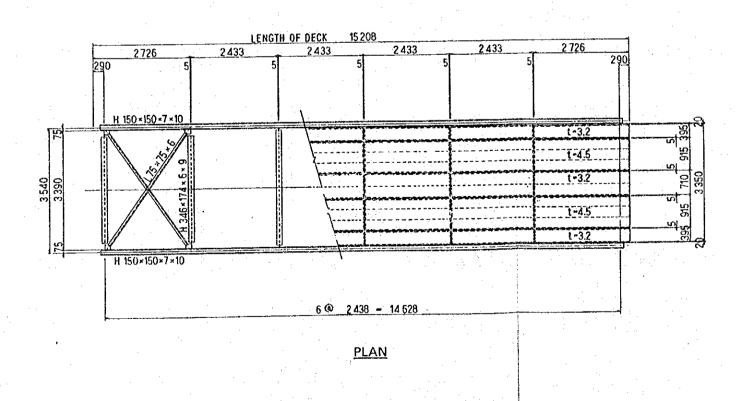


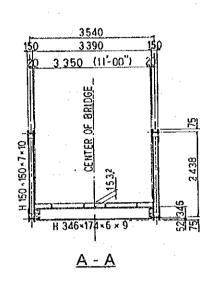
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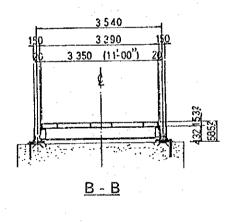
<u>A - A</u>

CROSS SECTION



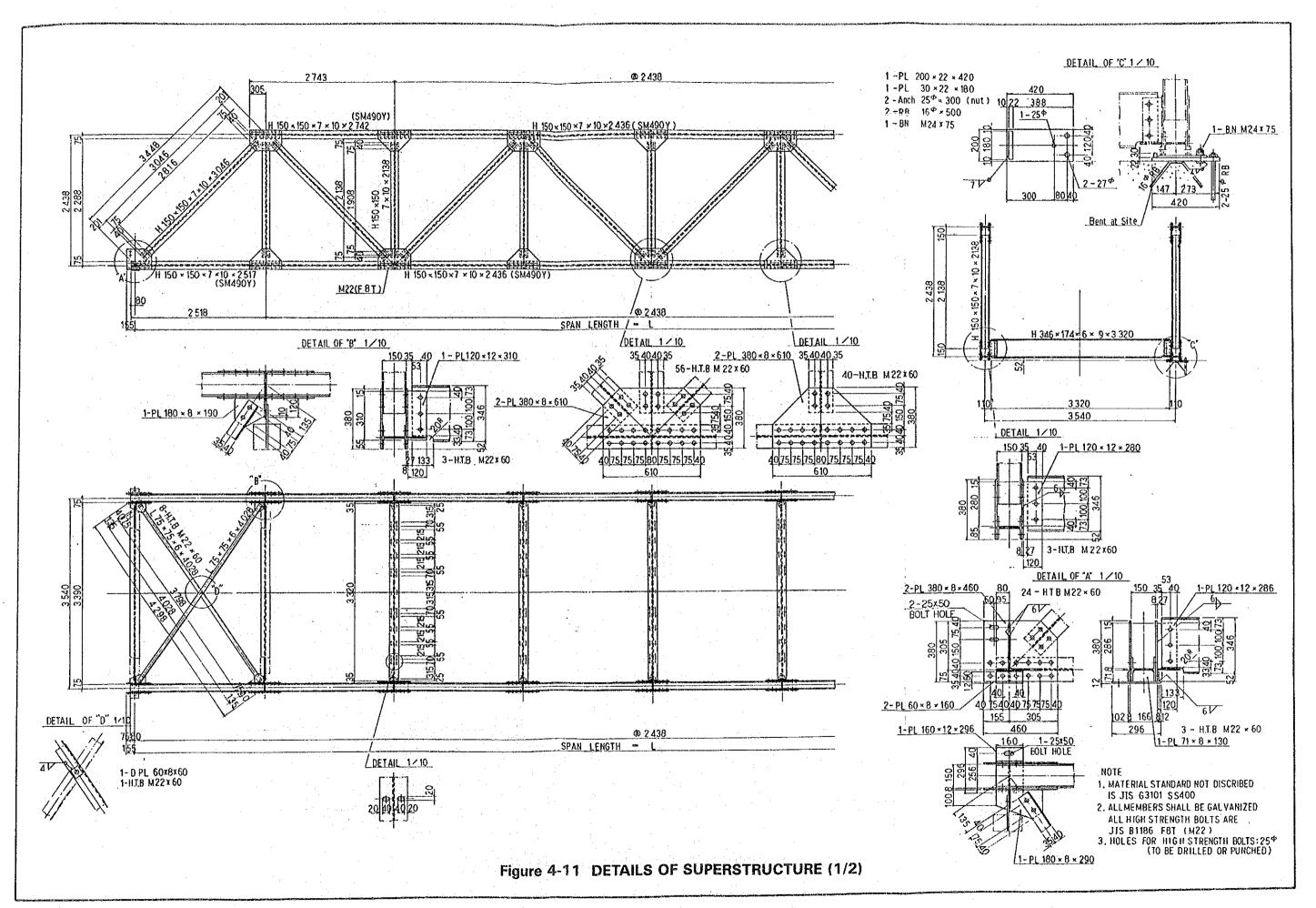


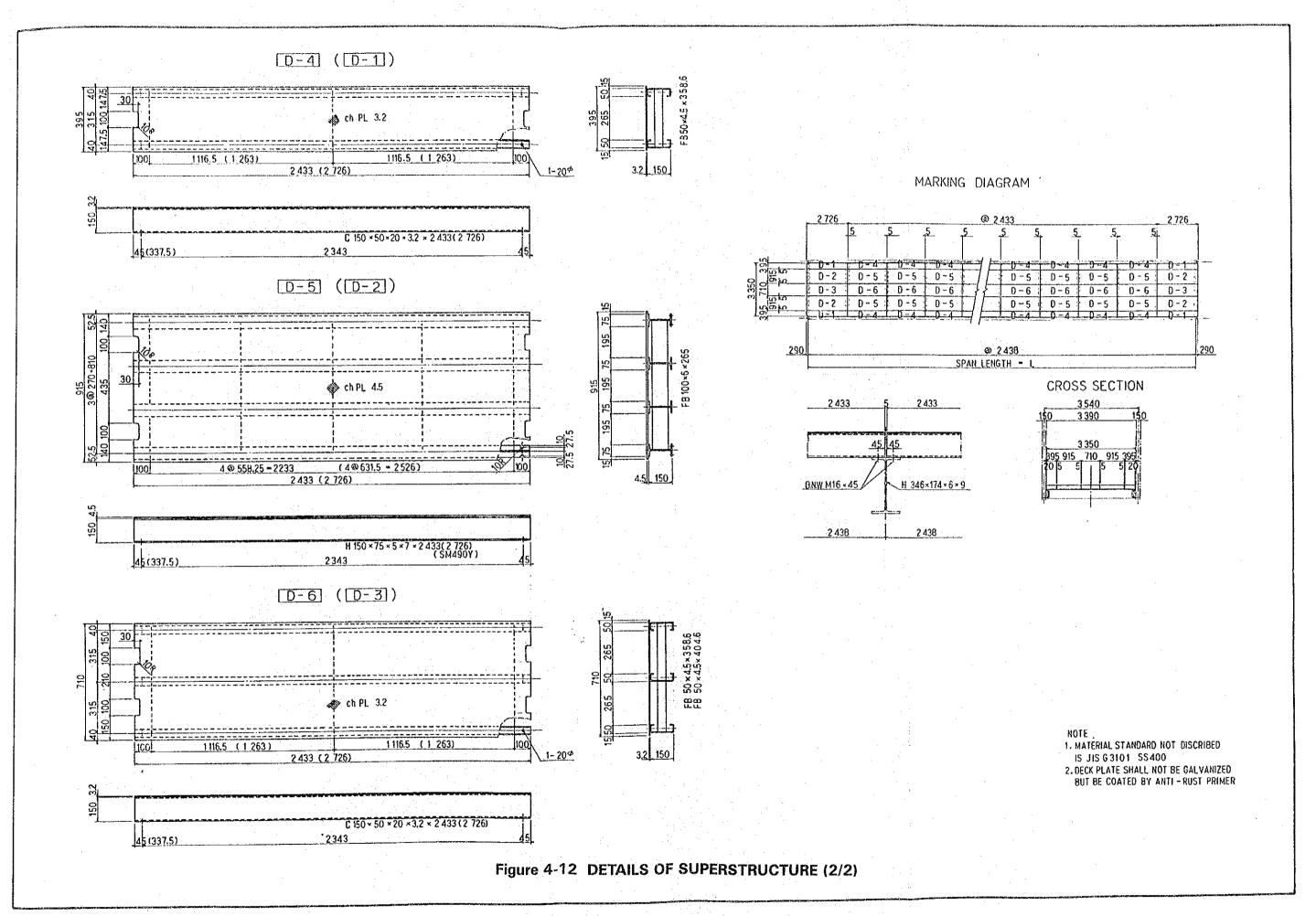




CROSS SECTION

Figure 4-10 GENERAL ARRANGEMENT OF SUPERSTRUCTURE (48 ft. SPAN)





4.4.3 Quantities of Bridge Construction Works

The quantities of bridge construction works for the project bridges are estimated based on the Summary of Basic Bridge Planning given in Tables 4-3 to 4-5. The summary of quantities of bridge construction works are presented in Table 4-7.

Table 4-7 SUMMARY OF QUANTITIES OF BRIDGE CONSTRUCTION WORKS

	Works		Unit	Phase 1	Phase 2	Phase 3	Total
Nı	umber of B	ridges	Bridge	32	22	20	74
:	No. of	15m Spans	Span	9 (135m)	6 (90m)	8 (120m)	23 (345m)
Gran and	No. of	20m Spans	Span	28 (560m)	20 (400m)	7 (140m)	55 (1100m)
Super- structures	No. of	25m Spans	Span	31 (775m)	24 (600m)	25 (625m)	80 (2000m)
	•	Potal	Span	68 (1470m)	50 (1090m)	40 (885m)	158 (3445m)
	Trans	sportation	Ton	997	997 739 600		
	1	Erection	Ton	997	739	600	2,335
		H= 4.5m	Unit	62	42	36	140
	Abutments	H= 5.0- 6.0m	Unit	2	2	4	8
		Total	Unit	64	44	40	148
		H= 5.0~ 7.5m	Unit	20	12	3	35
		H= 8.0- 9.5m	Unit	4	11	3	18
Sub-	Piers	H=10.0-13.5m	Unit	6	0	1	7
structures		Pile-bent Type	Unit	6	5	13	24
		Total	Unit	36	28	20	84
		Precast	Each	860	603	430	1,893
	Piles	Cast-in- Place	m	417	378	960	1,755
Ar	proach Roa	ads	m	675	480	425	1,580
Riv	rer Protect	ion	m²	2,350	1,600	1,470	5,420

4.4.4 Quantities of Superstructure (Portable Steel Bridge) Materials

The quantities of superstructure materials is estimated based on the drawings of the superstructure. The summary of the quantities of superstructure materials which will be procured under this project is presented in Table 4-8.

Other than these superstructure materials, tools for erection will be procured under this project. The details of the tools are described in Section 4.5.2.

Table 4-8 SUMMARY OF QUANTITY OF SUPERSTRUCTURE (PORTABLE STEEL BRIDGE) MATERIALS

(Unit:ton)

	Materia	ls					
Shape	Designation	Size	Phase 1	Phase 2	Phase 3	Total	Remarks
		150x150x7x10	169.972	120.952	98.358	382.282	Truss chord
	SM490Y	150x74x5x7	164.560	121.992	99.008	385.560	Deck plate
H-Beam		364x170x6x9	90.135	66.776	54.130	211.041	Cross beam
	SS400	150x150x7x10	182.774	135.584	110.174	428.532	Diagonal member
	Sub-1	total	600.441	445.304	361.670	428.532	
L-Beam	SS400	75x75x6	7.480	5.500	4.400	17.380	Cross beam
LIP-CH	SS40D	150x50x20x3.2	69.524	51.540	41.830	162.894	Deck plate
	204.00	100x6	21.164	15.696	12.744	49.608	Deck plate
Flat Bar	SS400	50x4.5	4.704	3.488	2.832	11.024	Deck plate
	Sub-total		25.872	19.184	15.576	60.632	
Check Plate	SS400	t = 4.5	99.152	73.504	59.656	232.312	Deck plate
	ss400	t = 3.2	59.256	43.928	35,652	138.836	Deck plate
	Sub-total		158.408	117.432	95.308	371.148	
		t = 22	4-420	3,250	2.600	10.270	Shoe
Plate	SS400	t = 12	5.639	4.162	3.405	13.206	Gusset
		t = 8	58.366	43/287	35,142	136.784	Gusset
	Sub-total .		68,425	50.688	41.147	160.260	
High	POT	M22 (kg)	64.048	47.478	38.469	149.995	For truss
Tension Bolt	F8T .	M22 (set)	121,766.000	90,265.000	73,115.000	285,146.000	
Bolt,	00/00	M16 (kg)	5.662	4.199	3.409	13.270	For deck plate
Nut, Washer	SS400	M16 (set)	28,941.000	21,460.000	17,424.000	67,825.000	
Ancher Bar	ss400	M24	0.884	0.650	0.520	2.054	Shoe anchor
Round Bar	SS400	ф16пт	0.408	0.300	0.240	0.948	Shoe anchor
Total weight of steel materials			996.592	733.894	599.829	2,335.315	
	Middle Coat		2.619	1.942	1.576	6.137	For field painting
Paint	Finish Coat	t	2.401	1.780	1.445	5.625	For field painting
:	Thinner		0.502	0.372	0.302	1.176	For field painting

4.5 Construction Plan

4.5.1 Bridge Materials Transportation Plan

The steel materials and the erection tools for the project (the items) which will be procured under this project will be delivered from Japan to Chittagong International Seaport. The items will be handed over to LGED after landing at Chittagong. Then the items will be transported to the storage yards of the involved LGED district offices for storage until erection.

4.5.2 Bridge Erection Plan

Erection of the portable steel bridges will be executed by local constructors under the responsibility of LGED. The erection method and tools necessary are planned as follows:

(1) Erection method

Comparative schemes of erection methods and evaluations are shown in Table 4-9. As a result, the launching on staging method is proposed. The features of this method are as follows:

- The bridge can be assembled on the approach road, so, assembly will be efficient and accurate.
- No special machines or skilled techniques are required, so, the erection cost is low.
- For erection of portable steel bridges, assembly on staging is the common method in Bangladesh. This method was introduced to improve the speed and accuracy of the assembly on staging method.

Conceptual figures of the launching on staging method are presented in Appendix 5.

Table 4-9 COMPARATIVE STUDY OF ERECTION METHODS

	Comparative Erection Schemes	Evaluation	Easi- ness	Cost	Speed	Conclu- sion
Assembly on staging		•impossible during flood season •difficult where river is deep •need small tools only •no need skill	В	A	c	В
Truck crane & bent		 impossible during flood season need access road for truck crane need staging for truck crane no need skill fast execution 	С	В	A	£
Cable suspension		∍no need large machine ∍need skill	С	A	В	C
Erection girder	Erection Girder	 erection girder is large no need skill fast, easy and accurate assembly 	A	С	A	8
Extension girder	Tis Beach Extension Girder	•extension girder is large •no need skill •fast, easy and accurate assembly	А	С	А	8
Assembly on staging	Tie Beam	• impossible in flood season • tools are small • no need skill • fast, easy and accurate assembly	8	А	Α	A
Not	te) A : good B : fair C : poor					

(2) Erection tools

The items and quantities of assembly and launching tools necessary with the launching on staging method were studied.

The proposed items and quantities per set of assembly tools and launching tools are presented in Tables 4-10 and 4-11, respectively.

According to the implementation schedule of the project proposed by LGED, the erection of the bridges in each phase is schedule for 6 months. Based on a study of the implementation schedule in each phase, the necessary quantities of tool sets are proposed as follows:

Proposed quantities of tool sets

Assembly tools: 12 sets

Launching tools (2 span continuous type): 6 sets Launching tools (3 span continuous type): 6 sets

Table 4-10 ASSEMBLY TOOL LIST

(Per set)

Tool	Designation	Quantity
[Survey Tools] Level Gauge Steel Measuring Tape	ST900 50m	1 pcs. 1 pcs.
[Election Tools] Torque Wrench Socket 60° Single Offset Wrench Sledge Hammer, Double Face Hand Hammer, Double Face Lever Block Bolt Clipper Wire Clip Crow Bar Crow Bar Crow Bar Election Bolt Drift Pin	7500QLE 36mm 60° x 22mm #8 (3.5kg) #3 (1.3kg) 1 ton KKW-2 10 φ L = 1.0m L - 1.5m M22 x 50 φ 24.5	4 pcs. 6 pcs. 10 pcs. 2 pcs. 10 pcs. 2 pcs. 1 pc. 20 pcs. 1 pc. 1 pc. 1 pc. 300 pcs. 150 pcs.
[Lifting Equipment] Three Pronged Lift Pulley Block Shackle Pipe Nylon Sling Portable Winch Steel Wire Rope Stay Wire Rope Base Beam	2 ton IS-Hooktype 5/8 60.5 x 7m 1.5ton x 3m NPW2000 9 \(\phi \) x 45m 9 \(\phi \) x 3m H1-150 x 1.5m	2 pcs. 4 pcs. 4 pcs. 6 pcs. 8 pcs. 2 units 2 rolls 2 pcs. 2 pcs.
[Scaffolding] Scaffolding Stage Plank Jack Base Ladder Brace	KA3055A HPS5183 KA752 KA3055S KA14	4 set 2 pcs. 1 pc. 8 pcs. 4 pcs.

Table 4-11 LAUNCHING TOOL LIST

(Per set)

		T	ype of Bridge	
Tool	Designation	One Span Type	Two Span Continuous Type	Three Span Continuous Type
[Erection Truss]	H150		1 set (0.53 ton)	2 sets (1.06 ton)
[Launching Rail]				
• Launching Rail	73.8kg/m	39 m	39m + 12m = 51m	39m + 12m x 2 = 63m
Base Plate	t = 25mm	0.50 ton	0.67 ton	0.85 ton
[Launching Equipment] Roller Screw Clamp Portable Winch Pulley Block Pulley Block Pulley Block Stay Wire Rope Steel Wire Rope Roller Staging Beam Filler Plate Filler Plate Filler Plate Winch Staging Beam	TIL-TANK25 T-10 NPW2000 3S-Hook 2S-Hook 1S-Hook 9\$\phi \times 2m 9\$\phi\$ H150 \times 4m 200\times 6\times 200 200\times 25\times 200 200\times 1.5m	4 pcs. 16 pcs. 2 units 2 pcs. 2 pcs. 6 pcs. 150m x 2 rolls 4 pcs. 16 pcs. 8 pcs. 8 pcs. 6 pcs.	24 pcs.	8 pcs. 32 pcs. 2 units 2 pcs. 2 pcs. 6 pcs. 200m x 2 rolls 12 pcs. 32 pcs. 24 pcs. 24 pcs. 6 pcs.
[Jack Up/Down Equipment]				
Mechanical JackMechanical JackSaddle	15t SLIDE 30t H150x0.5mR	4 pcs. 2 pcs. 32 pcs.	4 pcs. 2 pcs. 32 pcs.	4 pcs. 2 pcs. 32 pcs.

4.6 Project Implementation Plan

4,6.1 Basic Concept

The following are the basic concepts for implementing this project after the Exchange of Notes between Japan and Bangladesh:

- The Japanese Consultant (the Consultant) will implement the project under the Consultant Agreement between LGED and the Consultant.
- The Consultant will carry out detailed design, tendering works and supervision for procurement of portable steel bridge materials (the Materials) and erection tools (the Tools).
- The supervision works include training Bangladesh engineers and staff involved in erection of the bridges
- LGED will execute detailed design and construction of substructures, approach roads and other necessary works for the project, including erection of the project bridges.
- The tenderer for supplying the Materials will be qualified Japanese contractors.
- Supply of the Materials will be contracted by the successful Japanese contractor under the contract agreement between LGED and the contractor.

For the project, the undertakings of the two countries are as follows:

Scope of the project by the Government of Japan

The undertakings of the Government of Japan for the project are as follows:

- Materials of portable steel bridges
- Tools necessary for erection of the bridges
- Training for erection of the bridges

Japan's undertakings include delivery of the Materials and Tools from Japan to Chittagong International Seaport.

The Materials list of portable steel bridges is presented in Table 4-8 in Section 4.4.4 and the Tools list is presented in Tables 4-10 and 4-11 in Section 4.5.2.

Undertakings of the Government of Bangladesh

The undertakings of the Government of Bangladesh are as follows:

- Detailed design and construction of substructures, approach roads,
 river protection and other necessary works for the project.
- Customs clearance and inland transportation of the Materials and the Tools.
- Erection of the bridges.

The Government of Bangladesh is responsible for constructing the bridges within a 2-year period after the delivery of the Materials at Chittagong, and for reporting the progress of construction of the project bridges to JICA Bangladesh Office every one year until the completion, as well as to take the necessary measures stated in the Minutes of Discussions.

The quantities of the bridge construction works are presented in Table 4-7 in Section 4.4.3.

4.6.2 Implementation Supervisory Plan

(1) Detailed design

Related to procurement of the Materials and Tools, the following items will be prepared in detailed design by the Consultant:

- Detailed design report
- Drawings and specifications
- Procurement plan and cost estimation report
- Tender and contract documents
- Bridge erection training plan

(2) Tendering

Relevant to tendering for procurement of the Materials and Tools, the following services will be provided by the Consultant:

- Tender notice
- Tender pre-qualification
- Tendering
- Tender evaluation

(3) Supervision

The Consultant will execute the following services in supervision:

- Inspection of shop assembly of the portable steel bridges
- Inspection of delivery and handover of the Materials and Tools
- Training for bridge erection

4.6.3 Procurement Plan

Considering quality control and the time for delivery of the Materials and Tools for implementation of the project, the Materials and Tools will be procured in Japan.

4.6.4 Bridge Erection Training Plan

Training of Bangladesh engineers and staff involved in erection of the portable steel bridges is planned as follows:

Preparation of bridge erection manual

An erection manual for training in erection of the portable steel bridges will be prepared in the detailed design of the bridges. The manual will consist of explanations of erection planning and execution.

On-the-job training

Training teams will be sent from Japan to the project sites in Bangladesh to conduct on-the-job training in erection of the project bridges. Bangladesh engineers and staff involved in the erection will be trained directly by the bridge erection experts.

The training teams will be comprised of the following steel bridge erection members:

- Expert (1 person)
- Foreman (1 person)
- Skilled worker (2 persons)

On-the-job training is planned to be held at the following types of phase 1 bridges when erection is started:

- One span type (1 bridge)
- Two span type (1 bridge)
- Three span type (1 bridge)

4.6.5 Implementation Schedule

LGED plans implementation of the undertakings of the Government of Bangladesh over a 2-year period, which is divided into 3 phases.

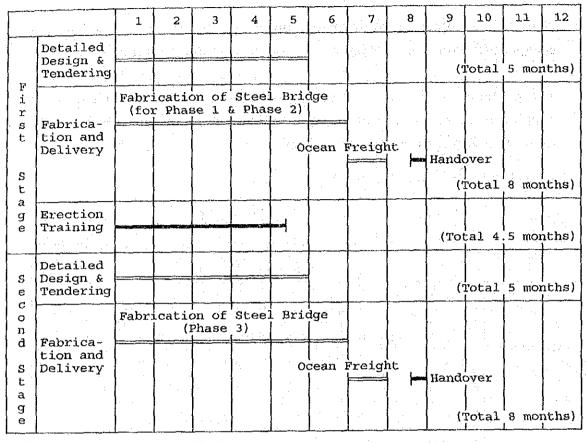
The portable steel bridge materials, erection tools and bridge erection training to be procured under Japan's Grant Aid is planned to be implemented in 2 stages as shown in Table 4-12.

Table 4-12 PROCUREMENT PLAN

Procurement Stage (Japan Side) Construction Phase (Bangladesh Side)			First Stage		Second Stage	Total	
		Pha	ase 1	Phase 2	Phase 3	IOLAI	
	For 15m spans	9	spans	6 spans	8 spans	23 spans	
Portable Steel Bridge	For 20m spans	28	spans	20 spans	7 spans	55 spans	
Materials	For 25m spans	31	spans	24 spans	25 spans	80 spans	
	Assembly tools	12	sets	0	. 0	12 sets	
Erection Tools	2 span continu- ous launching tools	6	sets	0	0	6 sets	
	3 span continu- ous launching tools	6	sets	0	Q	6 sets	
Erection T	raining	3 bi	ridges	0	0	3 bridges	

The implementation schedule of the project is shown in Figure 4-13.

Table 4-13 IMPLEMENTATION SCHEDULE



Note:

In Japan

· In

In Bangladesh

4.6.6 Construction Cost

The construction cost borne by the Government of Bangladesh is roughly estimated at 1,906.7 Lakh Taka, as shown in Table 4-14 and Appendix 7.

Table 4-14 COST BORNE BY THE GOVERNMENT OF BANGLADESH

(Take in Lakh)

Item	Phase 1	Phase 2	Phase 3	Total
Construction	765.1	558.0	498.5	1,821.6
Inland Transportation	36.2	26.7	20.5	83.4
Custom Clearance Fee	0.8	0.5	0.4	1.7
Total	802.1	585.2	519.4	1,906.7