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B. 1.1 Availability of Water Resources

1. Effective Rainfall

Effective rainfall is estimated at 0.75 of monthly rainfall as recommended in the M/P study.

 $\begin{array}{rll} \mathrm{Re}=0.75\ ^{*}\mathrm{R}\\ & & \mathrm{Where} & : & \mathrm{Re}=\mathrm{Effective\ rainfall} & (\mathrm{mm/month})\\ & & \mathrm{R} & = & \mathrm{Monthly\ rainfall} & (\mathrm{mm/month})\\ & & \mathrm{Re} < = & 200\ \mathrm{mm/month\ in\ paddy\ field}\\ & & \mathrm{Re} < = & 120\ \mathrm{mm/month\ in\ upland\ field} \end{array}$

Monthly effective rainfall during irrigation periods (Apr. ~ Sep.) was estimated in design year and normal year as follows:

(Unit:	mm)

Month Year	Apr.	May	Jun.	Jul.	Aug.	Sep.	Total
1967	36.5	9.5	36.3	5.7	16.0	75.7	179.7
1990	21.8	18.9	13.4	17.6	32.9	21.2	125.8

1990:Design year1967:Normal year

2. Surface Water

(1) River Runoff

Annual runoff of the Haraz river was estimated at around 1,086 MCM comprising the Irrigation Period (Apr. ~ Sep.) of 728 MCM and Non-Irrigation Periods (Oct. ~ Mar.) of 358 MCM.

(2) Probable Runoff in Haraz River

Probable annual runoff was calculated based on results of hydrological analysis for the Haraz river basin.

	. 2		<u>e Runon</u>				(Unit: MCM)
Station		Annum.		Runoff Discharg in Irrigation Per (Apr. ~ Aug.)				
:	1/2	1/5	1/10	1/2	1/5	1/10	Annual	Irri.Period
Polur	416	310	274	328	232	199	285	176
Direct River Basin	670	551	510	402	271	227	523	225
Karehsang	1,086	863	784	730	530	454	808	401

Probable Runoff in the Haraz River

The annual surface runoff at the design year in the Haraz river has been determined based on 227 MCM of the runoff in direct catchment area.

> Normal year 240 MCM + 402 MCM = 642 MCMDesign year 240 MCM + 225 MCM = 465 MCM

Note: 240 MCM is maximum guaranteed release water of Lar dam.

3. Groundwater

(1) Availability of Irrigation Water

Optimum utilization of groundwater for irrigation within the subjective project area was assessed as follows;

Wells :	137 MCM
Spring :	<u>8 MCM</u>
Total	145 MCM

(2) Irrigable Area

1) Water Requirement

The water requirement at the paddy field has been estimated under the consideration of effective rainfall in design year as follows.

Early Matured Variety	(EV)	$317.4\mathrm{mm}$
Middle Matured Variety	(HV)	341.8 🥢
Late Matured Variety	(LV)	281.5 🥖
Nursery		2.6 1/
Total		<u>943.3 mm</u>

Irrigation efficiency

•	Conveyance	efficiency (Ec)	= 0.89
---	------------	-----------------	--------

- Field canal efficiency (Eb) = 0.91
- Field application efficiency (Ea) = 0.87
- Overall efficiency (Eo)

Irrigation efficiency for water resources

•	Surface water	0.70	$= (0.89 \times 0.91 \times 0.87)$
•	Groundwater	0.79	$= (0.91 \times 0.87)$
•	Abbandan	0.79	$= (0.91 \times 0.87)$
٠	Return flow	0.79	$= (0.91 \times 0.87)$

Water requirement for water resources

•	Surface water	943.3/0.70	. ==	1,347.6 mm (1,347.6 m ³ /ha)
.•	Groundwater	943.3/0.79	=	1,194.1 mm (1,194.1 m ³ /ha)
٠	Abbandan	943.3/0.79	. ==	1,194.1 mm (1,194.1 m ³ /ha)
•	Return flow	943,3/0.79		1,194.1 mm (1,194.1 m ³ /ha)

= 0.70 (Note: Ecx Ebx Ea)

2) Groundwater Yield and Irrigable Area

Irrigable area by the groundwater at present are summarized as follows;

Water Resources	No. of Facilities	Tota	al Yield	Unit Yield	Irrigable area of Unit
Shallow-well Spring	5,837 48	135 8	MCM MCM	23,000 m³/unit 166,600/unit	23,000/11,941 = 2 ha 166,600/11,941 = 14 ha
5,837 w	vells $\times 2 = 1$	1,674	4 ha		
48 spri	$ngs \times 14 =$	672	2 ha		
Tota	1 1	2.34	6 ha		

The detail of irrigable area and kind of water resources (well and spring) are compiled in Table B.1.1-1.

4. Farm Pond (Abbandan)

(1) Present Situation of Farm Pond

There are 206 number of existing farm pond in the Project Area, and it's storage capacity was estimated about 36 MCM, furthermore, these ponds have a function to control excess water, return-flow of irrigation water and storing flood water appropriately.

Summary of Farm Ponds (Abbandan)

	e de la servició de l	and the second	
District	No. of Pond	Reservoir Area	Reservoir Capacity
		(ha)	$(10^3 m^3)$
H.W.D	3	28	364
H.E.D	25	449	6,687
A.W.D	81	1,106	10,200
A.E.D	92	1,703	16,683
Total	201	3,286	33,934
Urban Area	5	216	2,066
Ground total	206	3,502	36,000

The summary of the pond are shown at following table.

Note: One farm pond (AE 88) under the control of Ramsar Treaty includes the above table in A.E.D.

The list of the farm pond in the Project Area are prepared compiled in attached table B.1.1-2.

(2) Improvement Plan of the Farm Ponds

1) Objective for the Improvement

The existing farm ponds have problems of declining function due to heavy sedimentation. Therefore, it is required to increase the reservoir capacity with the digging of reservoir bed.

2) Estimated of excavation volume and incremental reservoir capacity.

The computation method of excavation volume at the reservoir bed are summarized as follows.

- One farm pond selected as a representative pond in each District

- Excavation volume of representative pond calculated with typical feature as shown in attached Figure B.1.1-1, and estimated excavation volume per (ha) of reservoir area

Excavation volume of the District calculated based on said excavation volume per (ha) and total surface area of the reservoir in each Districts.

The results of estimation on the excavation volume in each District are shown as follows.

Items District	Existing Storage Volume (10 ³ m ³)	Incremental Storage Volume (10 ³ m ³)	Total	Excavation Volume (10 ³ m ³)	Acreage (ha)
HWD	364	143	507	176	28
HED BUA Sub-Total	6,687 2,066 8,753	1,989 - 1,989	8,676 2,066 10,742	2,475	453
AWD	10,200	5,574	15,774	6,862	1,090
AED (AW 88) Sub-total	16,425 258 16,683	6,688 6,688	23,113 258 23,371	8,546	1,674
Total	36,000	14,394	50,394	18,059	3,245

Irrigable area; $50 \text{ MCM} / 11,941 \text{ m}^3/\text{ha} = 4,187 \text{ (ha)}$

5. Return Flow

(1) Mechanism of Return Flow

The main canals and secondary canals for the irrigation have been constructed with 10 km to 30 km long, generally. The total canal length is estimated at about 1,800 km in the Project Area, and the canal density is counted at around 22.0 m/ha.

On the other hands, these irrigation canals are functioned also as the drainage canals in rainy season (winter), and these canals are connected to abbandans is some cases at the down stream in order to keep and regulate return flow water. Typical reuse of the return flow in the Project Areas is as follows;

> - The terminal drainage canals of upper irrigation block is connected with the secondary or tertiary canals at downstream irrigation block

The excess water of the upper blocks flows into next lower blocks through above mentioned canals.

(2) Estimation of Volumes of Return Flow

The functions, or procedures of the return flow could be understood through the field investigation. However, it is required to have the systematic discharge measurement in the canals for the evaluation of return flow.

The measurement survey of return flow have been examined at three pilot project area, (Eslam Abad, Ejibar Kola and Suteh). However, the results of measurement could not achieved successfully due to shortage of irrigation water at each pilot project area.

Accordingly, the estimation of return flow has been carried out based on following assumptions:

- Operation allowance in canal system
 oa = 15%
- Application allowance at field level aa = 15%
- Return-flow (RF)

Return flow may appears at 50% (r) of (1) lateral percolation, (2) operation allowance and (3) application allowance.

 $r = (Overall efficiency)^2 = (0.70)^2 = 0.5$

- 1.0 mm of the percolation will flow out to irrigation canals, of which 50% will be return flow.
- 50% of the conveyance losses will flow out as return flow.
- 50% of the field application losses will flow out as return flow.

(3) Effective Return-flow

Since operation and application losses occur unsteadily in operation of irrigation system, it is rather difficult to reuse such losses without storage function as abbandans in the system.

On the other hand, lateral percolation is drained as steady outflow to drainage ditches. Therefore, return-flow of lateral percolation can be easily utilized without abbandans in downstream.

However, in case with abbandans, it is necessary to subtract evaporation and percolation losses from abbandans, because most of returnflow is reused through abbandans.

Based on above elements, effective return-flow rate has been computed as in Table B.1.1-3. According to the result of computation, effective returnflow rate are as follows;

Effective return-flow rate

with abbandans at present

Diversion Irrigation amount *	8%
with abbandans in future	
Diversion Irrigation amount *	12%
without abbandans in future	
Diversion Irrigation amount st	4%

6. Irrigation Area by Water Resources at Present Stage

(1) Water Requirement (Design Year)

The water requirement of paddy field has been estimated at present stage as follows.

Variety	Irrigation Requirement	Effective Rainfall	Field Requirement
Early Matured Medium Matured Late Matured Nursery Total	(mm) 872.8 980.7 1,185.1	(mm) - 90 - 90 - 104 284	(mm) $782.8 \times 0.44 = 344.4$ $890.7 \times 0.40 = 356.3$ $1,081.1 \times 0.16 = 173.0$ 12.1 885.8

On farm level: 885.8mm/0.87 = 1,018.2mm (10,182 m³/ha)Tertiary canal level: 885.8mm/0.79 = 1,121.3mm (11,213 m³/ha)Main and secondary canal level: 885.8mm/0.70 = 1,265.4mm (12,654 m³/ha)

(2) Water Requirement (Normal Year)

Variety	Irrigation Requirement	Effective Rainfall	Field Requirement
	(mm)	(mm)	(mm)
Early Matured	872.8	- 86,0	$786.8 \times 0.44 = 346.2$
Medium Matured	980.7	- 89.1	$891.6 \times 0.40 = 356.6$
Late Matured	1,185.1	-144.9	$1,040.2 \times 0.16 = 166.4$
Nursery			12.1
Total		320	881.3

On farm level: $881.3 \text{mm}/0.87 = 1,013.0 \text{mm} (10,130 \text{ m}^3/\text{ha})$ Tertiary canal level: $881.3 \text{mm}/0.79 = 1,115.6 \text{mm} (11,156 \text{ m}^3/\text{ha})$ Main and secondary canal level: $881.3 \text{mm}/0.70 = 1,259.0 \text{mm} (12,590 \text{ m}^3/\text{ha})$

TABLE B.1.1-1 LIST OF SHALLOW-WELL AND SPRING (1/3)

	Sub		Shallov	Shallow-Well		Shallow-Well		
District	District	Zone	No of Well	Irrigable Area	No of Spring	Irrigable Area	Total (Irrigable-A)	Remarks
HE	HE (I)	HE 1	75	150 ^(ha)	15	210 ^(ha)	360 ^(ha)	
		HE 2	12	24	5	70	94	
		HE 3	1	2	6	84	86	
	·	HE 4	_	-	-	-	-	<u></u>
		HE 5	-			-		
		HE 5A	-		2	28	28	
· · ·		HE 5B	2	4	~	-	4	
	Sub-Total		.90	180	28	392	572	
HE	HE (II)	KL 1	_ ·			-	-	KL1~KL6
:		KL 2	_		~			· · · · · · · · · · · · · · · · · · ·
	-	KL 3	21	42	-	-	42	
	· · ·	KL 4	19	38			38	······
		KL5	362	724	-	-	724	
		KL6	3	6			6	
		KL 6A	30	60	-	-	60	
		KL 6B	110	220	-	-	220	·····
	Sub-Total	· ·	545	1,090	-	-	1,090	
HE	HE (III)	KR 1	-		-	-	-	
		KR 2	-	-	-			
		KR 3	10	20	-	-	20	
		KR4	430	860		-	860	
		KR 5	27	.54	. 14 1 –	-	54	
	Sub-Total		467	934		-	934	
Total		· · · · · · · · · · · · · · · · · · ·	1,102	2,204	28	392	2,596	

TABLE B.1.1-1 LIST OF SHALLOW-WELL AND SPRING (2/3)

Sub			Shallow-Well		Shallo	w-Well		
District	District District		No of Well	Irrigable Area	No of Spring	Irrigable Area	Total (Irrigable-A)	Remarks
нw	: -	HW1	-	(ha)	-	_(ha)	_ (ha)	
	-	HW 2A	213	426	3	42	468	······································
	-	HW 2B	53	106	-		106	
	-	HW 3	62	124	7	98	222	
	~	HW4	6	12	1	14	26	· · · · ·
	-	HW 5	14	28	1	14	42	
	~	HW 6	10	20	3	42	62	
Total	÷	······································	358	716	15	210	926	
AW	AW(I)	AW 1	57	114		-	114 ^(ha)	
	"	AW 2	186	372	-	-	372	· · · · · · · · · · · · · · · · · · ·
	11	AW 3A	154	308		-	308	·····
·	11	AW 3B	98	196	-	-	196	
	"	AW 4	112	224		· -·	224	
	Sub-Total		607	1,214		-	1,214	· · · · · · · · · · · · · · · · · · ·
AW	AW (II)	AW 5	302	604	· _ ·		604	· · · ·
	"	AW 6	51	102	<u> </u>	-	102	
	11	AW 7	281	562	· · · ·	-	562	
	11	AW 8	2	4	5	70	74	
	11	AW 9	1	2	·	-	2	
	"	AW 9A	318	636	- ·	-	636	· · · · · · · · · · · · · · · · · · ·
	"	AW 9B	129	258	-	-	258	
	Sub-Total		1,084	2,168			2,238	
Total	AW-I + AW-II		1,691	3,382	5	70	3,452	

	0.1		Shallow-Well		Shallov	w-Well	m , 1	
District	Sub District	Zone	No of Well	Irrigable Area	No of Spring	Irrigable Area	Total (Irrigable-A)	Remarks
AE	AE (I)	AE 1	1	2	-	-	2	
		AE 2	15	30	-	-	30	
		AE 3A	6	12	-	-	12	<u> </u>
		AE 3Aa	84	168	-		168	:
		AE 3Ab	. –	-	-	-		
······································		AE 3B	8	16	-	-	16	
<u>.</u>		AE 3C	78	156	-	-	156	······
· ·	Sub-Total		192	384	-	-	384	••••••••••••••••••••••••••••••••••••••
AE	AE (II)	AE 4	3	6	-	-	6	· ·
		AE 4A	151	302	-	-	302	<u></u>
· · · · · · · · · · · · · · · · · · ·		AE 4B	165	330	-	-	330	
		AE 5	10	20	~ .		20	
		AE 6A	69	138		-	138	
<u></u>		AE 6B	46	92	-		92	
	Sub-Total	·	444	888	· –		888	
	AE (III)	AE 7	143	286	-	-	286	
		AE 8	186	372	-	-	372	
		AE 9	387	774	-	-		
		AE 10	534	1,068	-	-	1,068	
		AE 11	42	84	-	-	84	
		AE 11A	142	284	_	-	284	
		AE 11B	323	646	-	-	646	
	Sub-Total		1,757	3,514	-		3,514	
Total			2,393	4,786	-	-	4,786	Babol Urb Area
BUA			293	586	-	-	586	

TABLE B. 1.1-1 LIST OF SHALLOW-WELL AND SPRING (3/3)

48

672

12,346

11,674

5,837

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TABLE	8.1.1-2	LIST OF ABBANDAN (1/7)

	Sub-		Sub-		Abba	ndan		
District	Distrcit	Zone	Block	No. of Abbandan	A (ha)	MWD	V (10 ³ m ³)	Remarks
HW	HW		· · ·					
		HW 2B	5-2	HW 1	5	1.30	65	
		HW 3	12-1	/ 2	11	4	143	
		1	12-2	// 3	12	.4	156	
	· · ·	4				<u></u>		· · ·
	Total	1 1		-	28		364	
AW	AWI	AW1	5-1	AW 1	39	0.75	293	
······································		"	2-1	<i>*</i> 2	10	11	75	
		4	4-2	// 3	11	4	83	· · · · · · · · · · · · · · · · · · ·
		4	5-1	<i>"</i> 4	2	0.85	17	· · · · · · · · · · · · · · · · · · ·
		4	4-2	<i>*</i> 5	3	0.50	15	
		"	3-1	∞ 6	24	0.90	216	
			3-1	<i>*</i> 7	20		180	
		4	7-1	<i>*</i> 8	81	1.70	1,377	
		"	7-1	// 9	17	. 11	298	
		AW 2	3-1	/ 10	52	0.65	338	
		4	1-1	× 11	6	1.00	60	· · · ·
		11	1-1	/ 12	8	.11	80	1 1
	· · ·	4	3-3		106	4	1,060	
		AW 3A	2-5	<i>*</i> 14	20	0.75	150	
		"	2-4	/ 15	7	0.65	46	
		"	2-4	/ 16	6	11	39	
		"	3-1	/ / 17	18	0.70	126	
		4	5-1	/ 18	17	0.90	153	
		"	2-7	/ 19	31	1.00	310	
		AW 3B	3-1	# 20	6	1.25	75	
		4.	4-2	× 21	17	1.00	170	
		· 4	5-1	/ 22	7	"	70	
		AW 4	3-4	× 23	. 4	0.90	36	
		4	6-1	<i>*</i> 24	18	0.95	171	
		4	6-2	× 25	8	0.90	72	
		11	6-2	* 26	8	4	72	
		4	5-5	# 27	22	. 11	198	11 A
		"	5-4	/ 28	10	0.75	75	· .
		11	5-6	/ 29	17	0.90	153	
		"						
	Sub-t	11	· · .		595		6,008	

A; M.W.D;

; Reservoir Area (ha) ; Mean Water Depth (m)

V; Storage Capacity (10³m³)

	Sub-		Sub		Abba	ndan		
District	Distrcit	Zone	Block	No. of Abbandan	A (ha)	MWD	V (10 ³ m ³)	Remarks
AW	AWII	AW 5	3-1	AW 30	8	1.00	80	
		"	3-2	/ 31	13	1.00	130	
		11	11		3	"	30	
		11		// 33	15	"	150	
		"	5-4	// .34	23	1.15	265	
		11	5-3	∥ 35	12	0.65	78	
		4	5-3	<i>∥</i> 36	5	0.90	45	36-1
		. 4	5-4	/ 36	4	0.90	36	36-2
		"	5-4	/ 37	11	"	99	
· · · · ·		AW 6	5-2	<i>∥</i> 38	3	0.50	15	
		11	2-1	/ 39	8	"	40	
		AW 5	3-4	<i>*</i> 40	17	1.00	170	
		AW 7	11-1	<i>"</i> 41	8	0.90	72	[
		AW 6	3-1	: <i>*</i> 42	8	1.00	80	
· . ·		11	4-2	<i>»</i> 43	5	"	50	
		11	3-5	<i>»</i> 44	7	"	70	
		4	3-4	<i>*</i> 45	2	0.50	10	
		AW 7	4-2	× 46	3	0.90	27	
		11	11	<i>»</i> 47	10	"	90	
		11	4-4	× 48	8	4	72	:
		4	10-4	× 49	17	0.70	119	
		4	3-2		22	1.00	220	·
		11	5-1	× 51	· ⁻ 8	0.50	40	
· .		4	6-1	× 52	2	"	10	
		4	8-1	<i>∗</i> 53	23	0.75	173	
· · · · · · · · · · · · · · · · · · ·		4	7-1	<i>*</i> 54	14	0.70	98	
		"	7-1	× 55	11	11	77	
		"	8-1	<i>∞</i> 56	6	0.65	39	
		11	6-1	<i>∞</i> 57	11	0.90	. 99	
	i.	"	6-1	<i>≫</i> 58	10	0.70	70	<u> </u>
		AW 9A	6-2	<i>∞</i> 59	8	1.00	80	

TABLE B. 1.1-2 LIST OF ABBANDAN (2/7)

	TABLE B. 1.1-2	LIST OF ABBANDAN (3/7)	
ub-	Sub-	Abbandan	

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TABLE B. 1.1-2 LIST	OF ABBAN	DAN (3/7)
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D	Sub-		Sub-					
District	Distrcit	Zone	Block	No. of Abbandan	A (ha)	MWD	V (10 ³ m ³)	Remarks
AW	AW II	AW 9A	7-1	AW 60	10	0.75	75	
·····	 	4	8-1	<i>*</i> 61	8	1.25	100	
		"	8-1	∅ 62	20	0.95	190	
· ··· · · · · · · · · · · · · · · · ·		"	7-1	<i>∗</i> 63	6	0.75	45	
		11	7-3	<i>*</i> 64	17	11	128	
		"	7-1		14	11	30	
		· //	7-3		2	0.50	10	
		11	7-3	<i>∞</i> 67	15	0.75	113	
		11	7-4	<i>∞</i> 68	31	0.60	186	
		"	4-4	<i>∞</i> 69	4	0.75	30	
		4	7-5	<i>*</i> 70	12	11	90	
		41.5	2-1	/ 71	3	1.00	- 30	
		AW 9B	3-1		4	0.75	30	
		AW 9B	3-1	<i>*</i> 74	10	0.65	65	
	· · · ·	11	4-1	<i>*</i> 75	12	"	78	
		11	"	∅ 76	6	11	39	
		11	5-1	* 77	2	11	13	
		4	7-3		7	0.50	35	-
		AW 9A	9-1	♦ 79	18	0.90	162	
		AW 9	3-1	♦ 80	11	0.75	83	
		AW 9B	5-3	× 81	4	0.65	26	
	Sub-t				511		1,558	
			. 4 					
	Total				1,106		10,200	
								······

TABLE B.1.1-2 LIST OF ABBANDAN (4/7)

	Sub-		Sub-		Abba	ndan				
District	Distrcit	Zone	Block	No. of Abbandan	A (ha)	MWD	V (10 ³ m ³)	Remarks		
HE	HEI	HE 5B	6-5	HE 1	6	1.65	99			
	HEII	KL3	6-6	<i>#</i> 2	21	1.45	304			
		KL4	3-2	// 3	12	2.20	264			
		KL5	1-2	// 4	15	1.25	188			
		KL4	6-2	<i>v</i> 5	142	1.20	1,704			
		: <i>1</i> 1.	11	<i>*</i> 6	5	1.05	53			
	1	11.	"	/ 7	13	1.75	228			
· .		"	6-3	<i>*</i> 8 ·	14	1.45	203			
		KL 6B	1-2	<i>»</i> 9	6	1.95	117			
		KL 6A	2-2	× 1.0	15	1.45	217			
		11	2-1	<i>*</i> .11	5	1.45	72			
		11	."	/ 12	16	1.30	208			
,			1-3	/ 13	10	2.70	270			
		KL 5	4-1	* 14	12	0.90	108			
		2 11	5-3	<i>∥</i> 15	45	1.70	765			
<u>.</u>	·		3-1	// 16	4	1.70	68			
		"	3-2	/ 17	10	1.75	175			
			".	° / 18	4	1.70	68			
	1	KL5	5-1	× 20	4	2.75	110			
		"	4	<i>*</i> 21	22	2.85	627			
		KL 6B	2-2	/ 22	32	1.20	384			
		"	3-3	× 23	9	"	108			
		KR4	9-2	<i>*</i> 24	8	1.25	100			
· ·	1 .	"	9-2	/ 25	19	1.30	247	1		
· · · .	Total				449		6,687			
HE		BUA		26	12	1.10	132			
				27	17	1.10	187			
				28	15	0.80	40			
		·	1. ¹	29	177	0.95	1,682			
•	1			30	5	0.50	25			
	Total				216		2,066			

V; Storage Capacity (10³m³)

A; Reservoir Area (ha) A.D.; Mean Water Depth (m)

	Sub-		Sub-		Abba	ndan	· · · ·	
District	Distrcit	Zone	Block	No. of Abbandan	A (ha)	MWD	V (10 ³ m ³)	Remarks
AE	AEI	AE 3B	1-1	AE 1	6	0.75	45	
		AE 3Aa	2-8	* 2	24	0.90	216	
		11	2-8	// 3	12	4	108	
			2-9	* 4	9	4	81	
		AE 3B	3-1	15	14	11.	126	
	AE II	AE 4A	2-1	<i>*</i> 6	9	11	81	
		AE 4B	4-1	<i>»</i> 7	31	1.45	450	
		4	5-2	<i>*</i> 8	13	0.90	117	
		4	7-2	/ 9	9	11	114	
1		11	9-1		43	0.70	301	
	AE I	AE 3Ab	4-1	» 11	9	0.50	45	
			4-1	<i>∾</i> 12	64	1.10	704	
		11	4-2		33	0.75	248	
	AE II	AE 5	5-1	14	6	0.65	39	
			5-1	15	9	0.65	59	
		4	6-1	× 16	16	0.40	64	
		AE 4B	8-1	/ 18	3	0.50	15	
			7-1	/ 19	4	0.90	36	11 A. A.
		- 14	9-2		11	0.70	77	
		AE 5	5-1	AE 24	22	0.65	143	
		AE 5	5-1	× 26	8	0.65	52	
		AE 6A	4-1	<i>»</i> 27	22	<i>4</i>	143	· · · · · · · · · · · · · · · · · · ·
		AE 6B	4-3	// 28	15	0.75	113	-
		AE 6A	6-1	<i>*</i> 29	26	1.25	325	
		AE 6B	2-2	<i>∞</i> 30	6	0.65	39	
		11	3-3	<i>∞</i> 31	16	0.90	144	тар — на село С
		4	3-2	// 32	15	4	135	
·		4	4-1		3	0.50	15	
<u>i</u>		4	4-1	/ 34	4	"	20	· · ·
								Law =

TABLE B.1.1-2 LIST OF ABBANDAN (5/7)

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TABLE B.1.1-2 LIST OF ABBANDAN (6/7)

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· . :	Sub-		Sub-		Abba	ndan		
District	Distrcit	Zone	Block	No. of Abbandan	A (ha)	MWD	V (10 ³ m ³)	Remarks
AE	AE II	AE 6B	4-2	AE 35	8	0.50	40	******
		"	4-2	// 36	5	11	25	
·····	AEIII	AE 7	2-1	// 37	9	0.75	68	• • • • • • • • • • • • • • • • • • •
		11	3-1	<i>∥</i> 38	38	0.60	228	
	AE II	AE 6B	4-1	× 39	12	"	72	
	AEIII	AE 7	3-4	<i>»</i> 40	11	0.65	72	· · · ·
	AE II	AE 6A	6-3	<i>»</i> 41	14	1.15	161	
	AE III	AE 7	3-5	<i>*</i> 42	6	1.50	90	**************
		11	4-4	× 43	29	0.90	261	
			4-7	<i>»</i> 44	27	1.15	311	
		: #	. 4	<i>*</i> 45	11	0.90	99	<u></u>
		4	4-6		. 19	11	171	
		- 11	1-4	<i>*</i> 48	23	0.90	207	
•		AE 8	2-1	<i>*</i> 49	15	"	135	
		AE 7	1-6	<i>∲</i> 50	8	"	72	
		AE 8	3-1	<i>∞</i> 51	14	: 11 -	126	
· · · ·		4	3-2	/ 52	30	"	270	
		11	1-5	× 53	13	0.60	78	<u> </u>
		11	4	<i>*</i> 54	10	0.65	65	
		AE 9	4-4	× 55	11	0.75	83	
		4	2-1		4	0.90	36	
		11	2-1	<i>∞</i> 57	6	11	54	<u></u>
•		анан санан сан Санан санан сан	2-2	<i>∞</i> 58	5	11	45	
		11	2-4	× 59	14	0.75	105	·
		· // ·	"	<i>∗</i> 60	12	11	90	
		. 11	3-1	× 61	8	0.90	72	
		11	3-2	× 62	22	1.25	275	
		"	4	<i>~</i> 63	9	"	113	
		11	4-1	<i>*</i> 64	21	0.90	189	
		"	4-1	× 65	13	"	117	

TABLE	8.1.1-2	LIST O	F ABBANDAN	(7/7)

e de la	Sub-	Sub-	Sub-		Abbandan			
District	Distrcit	Zone	Block	No. of Abbandan	A (ha)	MWD	V (10 ⁵ m ³)	Remarks
AE	AE III	AE 9	4-3	AE 66	17	0.75	128	
		AE 10	1-1	× 67	16	1.25	200	
	1. A.		1-4	<i>∞</i> 68	44	"	550	
		11	2-5	<i>*</i> 70	24	1.25	300	
			"	/ 71	22	0.75	165	
		AE 11A	3-1	<i>*</i> 72	26	0.75	195	
		11	3-2	<i>*</i> 73	60	1,20	720	<u> </u>
		. 11	3-3	<i>»</i> 74	10	1.25	125	
		AE 10	2-3	<i>*</i> 75	54	0.75	405	
		14 - 14 - 14 - 14 - 14 - 14 - 14 - 14 -	2-4	* 76	23	11	173	
		4	2-3	» 77	5	0.55	28	
		4	2-4	<i>*</i> 78	23	0.85	196	
	· .	. 11	11	<i>»</i> 79	13	11	111	
		AE 11A	1-3	× 80	61	0.85	519	
		4.	2-3	<i>*</i> 81	11	0.75	83	
		AE 11B	2-3	<i>*</i> 82	9	4 s	68	<u> </u>
		AE 11A	2-3	/ 83	3	4	23	
- <u></u> ,		AE 11B	2-2	/ 84	68	. 11	510	
		11	6-1	× 85	30	1.60	480	<u> </u>
		11	6-4	<i>*</i> 86	90	4	1440	
		"	6-3	<i>*</i> 87	14	4	224	
· · · · · · · · · · · · · · · · · · ·		"	6-5	<i>*</i> 88	41	0.63	258	
		11	5-4	/ 89	42	1.60	672	
· · · · · · · · · · · · · · · · · · ·		"	5-5	<i>*</i> 90	15	1.13	170	
·····		"	1	× 91	3	0.60	18	
		· 4	4-3	<i>*</i> 92	56	1.30	728	
		"	4-4	<i>*</i> 93	34	11	442	
	<u> </u>				<u> </u>			
	Total				1,703		16,683	

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		· · ·		Future		
Description		Unit	Present	With Abbandans	W/O Abbandons	
rrigation Area	1A	ha	82,643	78,669	82,171	
anal Area	•••••					
Secondary Canals	AS	ha	2,001	2,001	2,001	
Tertiary to Irrigation Ditches	AT	ha	885	885	885	
Total bbandan Area	AC	ha	2,886	2,886	2,886	
otential Evapotranspiration	AB	ha	3,502	3,502	0	
ETo in June	ЕТо	mm/day	5.5	5.5	5.5	
rop Coefficient	••••••					
Free Water Surface	Kw		1.00	1.00	1.00	
Crop Coefficient	Ke		1.10	1.10	1.10	
ercolation of Canals and Abbandans Canal Bed and Abbandan Bottom	PC	mm/day	5.0	5.0	5.0	
Percolation at Paddy Fields						
Deep Percolation	PD	mm/day	2.0	2.0	2.0	
Lateral Percolation	PL	mm/day	0.0	1.0	1.0	
Total Percolation	P	_mm/day	2.0	3.0	3.0	
Vet Irrigation Requirement (ETo * Kc + P) * 1A * 10	Ĭm	au m/dar	6,652,762	7 110 545	E ADC 400	
(E10 * KC + P)* IA * 10 Deration Allowance in Canal Level	In Oa	cu.m/day %	0,002,762	7,119,545	7,436,476	
Application Allowance at Field Level	- 88	70 %	15	15	15	
.osses of Irrigation Water			•••••••••••••••••••••••••••••••••••••••			
Evaporation from Secondary Canals			••••••	•••••••	•••••••••••••••••	
ETo * Kw * AS * 10	Les	cu.m/day	110,055	110,055	110,055	
Evaporation from Tertiary & Below ETo * Kw * AS * 10	Let	cu.m/day	48,675	48,675	48,675	
Total of Evaporation Losses	Le	cu.m/day	158,730	158,730	158,730	
Percolation from Secondary Canals		····	••••••	•••••••••••••••••••••••••••••••••••••••		
PC * AS * 10	Lps	cu.m/day	100,050	100,050	110,050	
Percolation from Tertiary & Below PC + AS * 10	Lpt	cu.m/day	44,250	44,250	44,250	
Total of Operation Losses	Lp	cu.m/day	144,300	144,300	144,300	
Application Loss at Field			· · · · · · · · · · · · · · · · · · ·			
In + aa	La	cu.m/day	997,914	1,067,932	1,115,471	
Operation Loss in Secondary Canal		D	500 500	000 700		
(In + Le + Lp + La) * 0a * 0.5 Operation Loss in Tertiary & Below	Los	cu.m/day	596,528	636,788	664,123	
(In + Le + Lp + La)* oa * 0.5	Lot	cu.m/day	596,528	636,788	664,123	
Total of Operation losses	Lo	cu.m/day	1,193,056	1,273,576	1,328,246	
Total of Losses(Le + Lp + La + Lo)	Lt	cu.m/day	2,494,000	2,644,538	2,746,747	
Return-flow Rate	r		0.5	0.5	0.5	
Return-flow Return-flow by Lateral Percolation						
PL * r * 1A * 10	RFp	cu.m/day	0	393,345	410,855	
Return-flow by Operation Losses		-		-		
LO + r Potum flow by Application Loss	RFo	cu.m/day	596,528	636,788	664,123	
Return-flow by Application Loss La * r	RFa	cu.m/day	498,957	533,966	557,736	
Total of Return-flow	Rf	cu.m/day	1,095,485	1,564,099	1,632,714	
Effective Return-flow						
with Abbandans (RF - (ETo * Kw * PC) * AB * 10)		cu.m/day	727,775	1,196,389	410.055	
w/o Abbandans (RFp)	rr e	cu.m/day	• • • • • • • • • • • • • • • • • • • •		410,855	
Irrigation Efficiency w/o Return-flow						
Conveyance Efficiency						
$\ln/(\ln + \text{Les} + \text{Lps} + \text{Los})$	Ec		0.89	0.89	0.90	
Field Canal Efficiency						
$\ln/(\ln + \text{Let} + \text{Lpt} + \text{Lot})$	Eb		0.91	0.91	0.91	
Field Application Efficiency						
In/(In+La)	Ea		0.87	0.87	0.87	
Overall Irrigation Efficiency						
Ec * Eb * Ea	Eo		0.70	0.70	0.71	
Return-flow utilization rate				••••••••••••••••••	••••••	
RFe/(In+Lt)	RFr		0.08	0.12	0.04	
Increase Rate by Return-flow Utilization					0.01	
1+ RFr	lr		1.08	1,12	1.04	
Operation Efficiency with Return-flow						
Eo+ 1r	Oe		6 7.0	0.70	A.F.(
	Ve		0.76	0.79	0.74	

TABLE B. 1.1-3 **IRRIGATION EFFICIENCY AT PRESENT AND FUTURE**

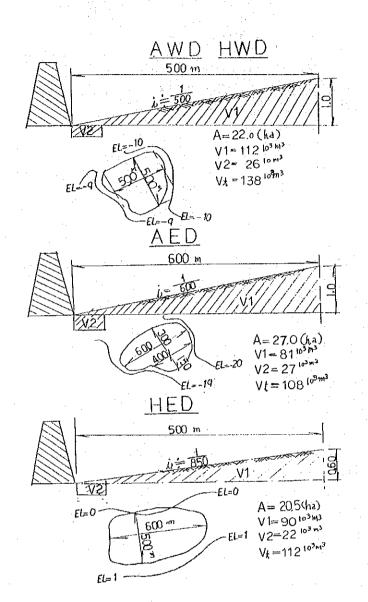


FIGURE B.1.1-1 TYPICAL FEATURE FOR IMPROVEMENT

B. 1.2 Preliminary Study of New Water Resources Development

1. Introduction

(1) Irrigation Area by Water Resources at Proposed Plan

The irrigable area depending on water resources has been examined in B. 1. 1 and summarized as below.

Groundwater;

Shallow-wells	5,387 places $ imes$	2 ha	==	11,674 ha
Springs	$48 \text{ places} \times$	14 ha	=	<u>672 ha</u>
Total	· · ·			12,346 ha

Abbandans;

Effective storage volume	50 MCM
Water requirement	11,941 m³/ha
Irrigation Area	50 MCM/11,941 = 4,187 ha

(2) Estimation of Return-flow and Its Irrigation Area

In case of full development with Mangol Dam, the return flow and its irrigable area have been estimated based on following procedure.

Required irrigable area by surface water

78,850 ha - 12,346 ha - 4,187 ha = 62,317 ha

Required irrigation water and its return flow

62,317 ha $\times 13,476$ m³/ha $\times 0.12 = 100,774$ 10^{3} m³

Irrigable area of said return flow

 $100,774 \, 10^3 \text{m}^3/11,941 \, \text{m}^3/\text{ha} = 8,439 \, \text{ha}$

Net irrigation area by surface water

78,850 - 12,346 - 4,187 - 8,439 = 53,878 ha

Accordingly, the balance of irrigation area based on water resources can be summarized as below.

Water Resources		Available Water	Irrigable Area
-	Surface water	726 MCM	53,878 ha
-	Ground water	143 MCM	12,346 ha
-	Small pond	50 MCM	4,187 ha
-	Reuse water	87 MCM	8,439 ha
	Total	1,006 MCM	78,850 ha

Note: Required surface water; 53,878 ha $\times 13,476$ m³/ha = 726 MCM Reuse water; 726 MCM $\times 0.12 = 87$ MCM

(3) Optimum Size of Regulating Reservoir

Maximum water release of 240 MCM from the Lar reservoir out of total surface water resources will be expected to use as irrigation water for the project. Therefore, the remaining 486 MCM will have to be developed with regulating reservoir in the residual area of Haraz river basin.

In order to estimate the optimum size of such regulating reservoir on the Haraz river, MOE proposed Mangol dam at most upstream of Karehsang gaging station.

According to the results of preliminary review on the geological conditions of the Mangol damsite nearby based on the geological map with scale of 1/250,000 which was provided by the Mazandaran Regional Water Board, geological features of original damsite are not suitable for dam construction due to predominant limestone extended around dam axis and reservoir area and some fault zone existed.

As an alternative plan proposed damsite at 15 km upstream of the original damsite is more suitable than original site. Reservoir storage capacity of the alternative site, however, is slightly smaller than that of original site and catchment area at alternative site will be decreased to 93% of Karehsang gauging station. (4,061 - 288 = 3,773 sq.km.)

General features of both damsites are as follows:

	Item	Original damsite	Alternative damsite
	chment area (km²) ment load (MCM)	4,061	3,773
2. Deul		180	170

The summary of geological conditions for both damsites will be described in the following section B. 1. 3 of this Appendix.

Therefore, this water balance study is made mainly determined optimum regulating reservoir size for two proposed damsites. (Refer to Figure B. 1. 2-1, Figure B.1. 2-3 and Figure B.1. 2-4)

2. Basic Input Data of the Simulation

Most of the basic data on irrigation water requirement was referred from B. 2 "the Irrigation" of this Appendix.

Summarized figures and factors to be used are described as follows.

(1) Irrigation Water Requirement

1) Crops and Varieties to be Irrigated

Crop to be irrigated is paddy rice only in summer season for the project and varieties of the rice proposed and cropping intensity are the followings.

Ir	ntensity (%)	
	25.0	
1.1	37.5	
	37.5	
	<u></u>	37.5

2) Crop Water Requirement

Crop water requirements for respective rice varieties can be estimated based on the cropping calendar, crop evapotranspiration, crop coefficient and percolation value etc.

Weighed irrigation water requirements for each ten days period is indicated in the Table B. 1. 2-1, and summarized figure is as follows.

Month Decade		Irrigation water	Irrigation Water Requirement (mm/10 days			
		Net I.W.R	Gross I.W.R. (Ei = 0.70)			
Apr. 3		47.3	67.6			
May	1	81.1	115.9			
	2	78.1	111.6			
· .	3	94.1	134.4			
Jun.	1	94.4	134.9			
2	91.9	131.3				
· .	3	92.2	131.7			
Jul.	1	90.2	128.9			
	2	89.7	128.1			
	3	95.9	137.0			
Aug.	1	69.3	99.0			
-	2	52.8	75.4			
	3	34.4	49.1			
Sep.	1	17.4	24.9			
-	2	5.3	7.6			
То	tal	1,034.1	1,477.4			

(2) Effective Rainfall

Effective areal rainfall of the Project Area during irrigation periods from 1951 to 1990 are tabulated in the Table B. 1. 2-2. The conversion method of effective rainfall from the monthly rainfall is described as follows.

- In case of monthly rainfall is less than 200 mm, $Re = Rm \times 0.75$
- In case of monthly rainfall is more than 200 mm, Re = $200 \times 0.75 = 150$ mm

(3) Evaluation on Runoff Discharge Records of Haraz River Basin

1) Available Data of Release Discharges at the Lar Dam

Water release records at the Lar damsite for irrigation are existed eight (8) years from 1984 (1363) to 1991 (1370) after completion of the Dam as tabulated in the Table B. 1. 2-3. According to the records on the water allocation agreement between Ministry of Energy and Irrigation Users in the Haraz river basin, the Ministry of Energy guarantees to release required irrigation water of 240 MCM as maximum volumes. The average release water to the downstream of Lar dam for the water balance simulation purpose is as followings.

					(Unit:cu.m/sec)
Month	Decade	Average	Maximum	Minimum	Applied
Apr.	3	13.27	22.02	1.38	19.91
May	1	16.76	35.94	1.96	25.14
	2	20.18	42.21	2.71	30.27
	. 3	24.34	29.89	18.52	36.51
Jun.	1	24.74	30.26	19.17	37.11
	2	21.03	31.49	12.89	31.55
	3	12.89	25.10	2.75	19.34
Jul.	1	14.40	31.09	0.74	21.60
	2	12.86	30.02	0.44	19.29
	3	10.27	27.92	0.68	15.41
Aug.	1	6.89	26,75	0.46	10.34
0-	2	2.67	12.27	0.46	4.01
	3	1.56	4.84	0.34	2.34
Sep.	1	1.53	4.85	0.38	2.30
~~~~	2	1.53	4.85	0.38	2.30

Applied release discharge assumed about 1.50 times of "Average value". Because, total release discharges during irrigation period are about 163.8 MCM, and the ratio between Guaranteed Volume by MOE and the average volume is about 1.5 times ( $240 \div 163.8 = 1.50$ ).

2) Discharge Records of Proposed Mangol Damsite Near the Karehsang

Available discharge data of the proposed Mangol damsite before operating Lar reservoir can be applied 28 years from 1956 to 1982 for water balance study of Mangol dam. The proposed Mangol damsite, however, has geological constraints. Therefore, careful studies on selection of the proposed damsite shall be made from the view points of geological conditions, stability of dam, hydrological availability and project economy.

Tentatively, the following two alternative damsites are considered for water balance simulation.

Original damsite (ODS)

Extremely upstream of Karehsang gauging station with catchment are of 4,061 sq.km.

Alternative damsite (ADS):

About 15 km upstream of the original damsite with catchment area of 3,773 sq.km. (93% of the original site)

### (4) Water Demands Other Than Irrigation Water

According to the Master Plan Report on the "Water Resources Planning of Haraz Plain and Mangol Dam" under the Water Development Plan of the Basin of Talar - Babol - Haraz Rivers, June 1990, water supply to the capital city of the Tehran and river maintenance flow in order to keep the environmental condition of the river are recommended the following values. These figures mentioned below shall be used for the water balance study unless otherwise any other suggestion from Irainan Government.

Month	Tehran Water Supply	(Unit : cu.m/sec Maintenance Flow
JAN.	0.00	0.30
FEB.	0.00	0.60
MAR.	0.00	0.60
APR.	6.85	0.30
MAY	6.85	0.30
JUN.	6.85	0.30
JUL.	9.34	0.30
AUG.	9,34	0.30
SEP.	9.34	0.30
OCT.	6.17	0.60
NOV.	6.17	0.60
DEC.	6.17	0.60

### 3. Water Balance Simulation

#### (1) Simulation Diagrams

Schematic diagrams of the Haraz river basin water balance (mainly surface water of Haraz river) can be illustrated in the Figure B.1.2-2.

#### (2) Reservoir Operation Rule of the Lar Dam

Operation of the Lar dam has not been functioned at full scale since the reservoir operation was commenced due to storage water leakage from the dam foundation and view points of keeping stability of the dam body. According to the operation records on the dam provided by MOE, maximum storage water level was recorded at about 2,507 m, which is equivalent to the gross storage volume of about 400 MCM including dead storage volume of 100 MCM. Therefore, maximum net storage volume of the dam for current water balance study shall be 300 MCM, and if there are water level exceeding 2,507 m (Effective reservoir volume 300 MCM) at Lar dam as the result of water balance, these water shall be spillaged to the downstream of Lar river, automatically.

(3) Case Study

The following two case study was conducted for two proposed damsites, such as "Case Study I" for original water allocation method for both Drinking (177 MCM) and Irrigation (248 MCM) and "Case Study II" for modified water allocation method.

	ODS	ADS
Case Study I	A = 53,878 (ha) For drinking 1771 For Irrigation 248	

Note :	ODS;	Original Dam site
	ADS;	Alternative Dam site

Case	Study	(II)
------	-------	------

ODS	ADS
A = 53,878 (ha)	A = 53,878 (ha)
For drinking	239 MCM (1.3)
For Irrigation	186 🧳 (0.75)

#### (4) Results of Water Balance Simulation

The results of water balance simulation for each cases can be summarized as follows.

### 1) Appropriate Mangol Dam Size

According to the results of water balance simulation combining the Lar and Mangol dams, required effective storage volume of Mangol dam for each cases is tabulated in the Table B. 1. 2-4. Appropriate reservoir size under once in ten years probable drought period are summarized as follows.

Case	Damsite	Live Storage	Total Loss	Dead Storage	Total Storage
· · · · · · · · · · · · · · · · · · ·	· ·	(MCM)	(5%) MCM	· · · · · · · · · · · · · · · · · · ·	(MCM)
I	ODS	280	14	180	474
**	ADS	305	15	170	490
II ·	ODS	323	16	180	519
	ADS	362	18	170	550

#### 2) Conclusion

Optimum sizing of the proposed Mangol dam is approximately 305 MCM live storage volume (Case Study I) at "Alternative Damsite" under the condition that allowable live storage capacity of existing Lar dam is upto 300 MCM with maximum water level of 2,507 m.

### TABLE B. 1.2 - 1 WEIGHTED AVERAGE IRRIGATION WATER REQUIREMENT

Month/I	Decade	Ta	rom	Kha	zar	Amo	1 - 3	
Crop Calen -Nursary -Land prej -End of irr	(20 days) paration	(CVR = (Apr. 01 - (Apr. 21 - (Apr. 01 -	May 31)	(CVR = (Apr. 11 - (May 01 - (Aug. 11 -	May 20) Jun. 10)	(Apr. 11 - (May 01 -	(CVR = 25.0%) (Apr. 11 - May 20) (May 01 - Jun. 10) (Sep. 01 - Sep. 15)	
W. Req'mer	nt	BWR	WWR	BWR	WWR	BWR	WWR	Total
(Month/I	Decade)	(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(mm)
April	1st		-	-			~	-
· · · ·	2nd	-	-	-	-	-	-	-
	3rd	60.2	22.6	-	-	98.9	24.7	47.3
a, t								
May	1st	64.9	24.3	64.9	24.3	129.8	32.5	81.1
	2nd	91,1	34.2	64.9	24.3	78.4	19.6	78.1
· · ·	3rd	97.1	36.4	96.3	36.1	86.2	21.6	91.1
June	lst	92.3	34.6	99.1	37.2	90.5	22.6	94.4
	2nd	93.3	35.0	91,4	34.3	90.5	22.6	91.1
	3rd	92.0	34.5	92.3	34.6	92.3	23.1	92.2
July	1st	88.3	33.1	91.0	34.1	91.8	23.0	90.2
· · ·	2nd	87.1	32.7	89.7	33.6	93.6	23.4	89.7
·	3rd	93.3	35.0	97.1	36.4	98.1	24.5	95.9
August	1st	51.8	19.4	80.6	30.2	78.6	19.7	69.3
0	2nd	24,9	9.3	66.7	24.8	74.7	18.7	52.8
	3rd	-	-	46.8	17.6	67.2	16.8	34.4
Sep.	1st			21.1	7.9	37.8	9.5	17.4
<b>r</b> ·	2nd		-			21.1	5.3	5.3
То	tal	936.3	351.1	1,001.3	375.4	1,229.5	307.6	1,034.1

Note:

(1) BWR **Basic Water Requirement** ≏

CVR ----

Crop Variety Ratio Weighted Water Requirement WWR =

(2) Whole land preparation period within the same irrigation zone is expected 50 days instead of 40 days for the period of each varieties.

(3) Harvesting time is expected at 10 days after irrigation water was stopped (maturing period of paddy rice).

TABLE B. 1. 2 - 2 MONTHLY EFFECTIVE RAINFALL FOR RICE

							- Unit: mm)
Year	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sep.
1951	35.7	14.1	23.4	21.6	12.5	25.7	73.9
1952	68.3	48.1	27.6	24.1	26.8	37.6	29.2
1953	76.9	32.3	34.5	14.0	44.6	14.3	118.1
1954	35.2	29.3	23.0	11.4	62.2	15.0	64.2
1955	50.7	25.4	13.9	32.5	13.4	43.4	45.5
1956	60.2	29.4	16.4	11.4	39.2	16.3	82.1
1957	58.9	19.6	23.3	73.5	46.4	67.1	21.8
1958	31.6	38.0	11.7	31.7	72.8	31.5	29.9
1959	37.4	22.2	33.5	17.0	15.7	44.3	59.0
1960	73.7	20.3	12.8	14.9	21.2	28.2	78.3
1961	44.0	33.9	20.3	14.0	37.8	39.5	103.5
1962	29.6	31.4	15.0	64.4	13.0	41.2	29.2
1963	57.5	16.4	30.4	29.8	18.4	49.4	20.2
1964	31.7	107.3	5.9	11.7	7.3	27.3	46.1
1965	43.7	31.9	18.0	8.6	24.8	7.1	46.4
1966	70.5	23.9	20.5	14.2	17.5	14.9	31.3
1967	52.5	36.5	9.5	36.3	5.7	16.0	75.7
1968	62.7	87.8	<b>21.7</b>	19.8	40.1	50.9	6.3
1969	72.2	58.4	19.2	16.7	18.8	23.1	113.3
1970	76.7	19.8	13.8	13.4	21.5	49.4	86.9
1971	42.6	36.6	11.9	13.4	0.0	2.7	22.4
1972	53.6	17.5	35.9	23.6	20.9	59.2	33.5
1973	36.5	9.3	13.1	22.3	3.1	52.1	55.1
1974	31.7	34.9	4.4	23.2	104.5	16.4	59.3
1975	39.2	31.1	36.5	13.6	3.2	84.3	25.6
1976	61.4	27.3	17.9	17.2	15.6	1.1	57.2
1977	27.2	14.8	23.6	8.2	17.2	55.8	20.0
1978	42.2	32.9	55.3	35.4	11.6	38.6	19.9
1979	40.8	22.0	22.1	11.2	10.6	17.0	48.0
1980	45.7	12.8	6.2	3.8	1.1	45.7	66.5
1981	44.0	59.2	35.6	3.2	52.1	18.4	133.4
1982	69.9	10.4	35.1	31.1	13.9	32.9	29.3
1983	49.3	12.5	18.5	35.0	4.5	64.4	37.7
1984	26.6	7.7	24.3	21.1	1.6	62.9	6.0
1985	48.7	14.9	13.3	14.6	16.7	49.6	22.1
1986	63.1	17.6	12.5	32.9	20.1	19.0	63.8
1987	44.4	34.8	13.2	11.9	17.4	117.8	32.2
1988	39.2	43.4	21.8	16.7	69.1	90.9	27.8
1989	75.2	13.7	12.1	12.5	11.4	46.4	166.3
1990	48.5	21.8	18,9	13.4	17.6	32.9	21.2

Note: 1990, Drought Year (1/10)

B1-30

TABLE B. 1. 2 - 3 RELEASE RECORD AT LAR DAM FOR HARAZ RIVER

(Unit: m³/sec)

Therefore, applied value shall be 1.5 times of the average values indicated above. Total volumes of averaged discharge during irrigation period are (163.81 MCM) × 1.5 ± 245.7 MCM Applied values shall be almost same as committed volume (about 240 MCM)to be released to Haraz River such as: 21.60 31.55 19.34 Applied 4.55 19.91 25.14 30.27 36.51 37.IL 19.29 15.41 10.34 4.01 2.34 2.30 1.41 1.38. 18.52 12.89 2.75 0.68 0.46 0.281.96 71.01 0.74 0.44 0.46 0.340.380.272.71 Min. 29.89 25.10 31.09 27.92 26.75 12.27 2.71 9.48 22.02 35.94 42.2130.26 31.49 30.024.84 4.85 Max. Average 20.18 0.943.03 13.27 16.76 24.34 24.74 21.03 12.89 14.40 12.8610.276.89 1.56 1.53 2.67 163.81 17.70 20.00 1.79 0.270.2819.04 19.17 14.94 6.43 16.07 0.460.460.340.40165.00 19.31 8.31 1661 0.39 13.22 25.70 20,62 23.75 27.88 0.740.440.68 0.55 0.48 0.45 0.38 0.282.42 11.87 0.320.31 0.29 0.270.29 0.26 0.26 0.220.170.17 119.53 1990 20.66 35.94 29.22 20.9416.75 19.10 15.726.17 4.00 1.020.95 0.920.90 0.80 0.68 198.54 2.71 3.86 42.216.210.540.58 0.260.200.221.30 1989 0.21222.72* N.A 4.48 4.85 18.52 30.26 31.49 25.10 31.09 30.02 27.92 26.75 4.75 Ň.A 3.10 2.19 N.A 4.57 12.27 4.84 4.85 4.80 Ň.A 4.05 3.323.77 2.621988 154.63* 23.6024.09 25.140.50 1.38 1.96 29.09 21.30 24.14 21.32 N.A N.A Ň.A ΥZ ΝA 0.90 2.73 ΝZ N.A N.A N.A N.A N.A N.A N.A N.A 1987 24.28 24.18 13.16 1.19 1.15 9.48 20.08 21.63 29.43 21.91 5.288.28 1.251.25 1.09 1.05 185.84 0.41 5.97 5.241.09 0.99 0.91 0.820.47 0.420.47 1986 1.29 18.86 28.96 29.89 28.97 18.25 11.254,60 1.53 1.69 1.57 1.361.29 1.17 1.221.14 146.96 0.77 16.07 1.41 1.11 0.94 0.86 0.45 0.361.21 0.611985 12.73 20.63 1.49 1.16 5.09 22.0220.17 27.06 12.89 2.7513.54 19.14 18.827.45 1.79 1.761.67 1.64 1.56 1.45 1.33 0.63 166.91 1.62 0.38 1.01 0.41 1984 ••• Irri. Period Non. I.P Total (Irrigation Period: MCM) I.P `. ` ŧ . ŧ ŧ × ۲ ŧ (Monthly) (Monthly) (Monthly) (Monthly) (Monthly) Decade 2 ന -3 က -61 က -2 က ----3 ŝ **7--**4 67 ŝ ----01  $\mathbf{c}$ Month Aug. Nov. Jan. May Dec. Mar. Feb. Jun. Apr. Jul. Sep. Oct.

B1-31

Item	Order	ODS	ADS
1. Case (I)			
	1	311	332
	2	285	313
	3	280	305_
2 ¹	4	279	304
	5	269	301
2. Case (II)			
<b>D</b> , 0(000 (11)	1	336	417
	2	328	368
	3	323	362
	4	321	359
	5	296	349

## TABLE B. 1.2-4 RESULT OF WATER BALANCE SIMULATION (REQUIRED STORAGE LIVE VOLUME)

Note :

Summary of water balance simulation for each cases Mentioned above is tabulated in Table B. 1. 2-5, -6, -7 and -8, respectively. Besides this, more detailed outputs were compiled in the Data Book. TABLE B. 1. 2 - 5 SUMMARY OF WATER BALANCE (CASE-I, ODS)

+inflow+--Haraz H.W--+--Auol H.W---+demand+---Mangol DAM---+ 27,855(ha) | 26,023(ha) |53,878| demand defici[demand defici[total |shorta resorvoir] (MOM) ***** Mangol DAM Operation Summary ***** LAR DAM OPERATION RULE CASE = 1 Dam Site of Mangol Dam : ODS Lar DAM OPERATION RULE CASE = 1 DRINKING WATER RATE = 1 RELEASE WATER RATE = 1 start of print table No.=1 end of print table No.=1 LARDAM DRINKING WATER RATE =1 LARDAM RELEASE WATER RATE =1 Dam Site of Mangol Dam = 1 1,341 912 867 671 776 776 818 818 879 879 691 year down { (8) (MOM) itrigation area(ha) = 53878 Vangol DAM reservoir (1000+CU.M)=0 Vangol DAM initial vol (1000+CU.M)=0 Lar DAM reservoir (1000+CU.M)=300000 Lar DAM reservoir (1000+CU.M)=300000 **** LAR DAM operation rule (when enpty)**** * case 1 .... drinking:release=50:50 * ******** drkn release excess deficit over reserv(1)-(2)-(3) flow oir (2) flow oir (6) (7 output device printer or screen (p/s)?p pperation start year =1956 operation end year =1982 ***** 0000000 ***** Lar DAM Operation Summary ---- LAR DAM ----છે 1111 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 1222 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12552\\ 12552\\ 12552\\ 12552\\ 12552\\ 12552\\ 12552\\ 12552\\ 12552\\ 12552\\ 12552\\ 12552\\ 12552\\ 12552\\ 12552\\ 12552\\ 1255$ Dam Site of Mangol Dam : ODS Lar DAM OPERATION RULE CASE = 1 DRINKING WATER RATE = 1 RELEASE WATER RATE = 1  $\widehat{\mathbb{C}}$ 1956 - 1982 ઉ [inflow] ਰ year  TABLE B. 1. 2 - 6 SUMMARY OF WATER BALANCE (CASE-I, ADS)

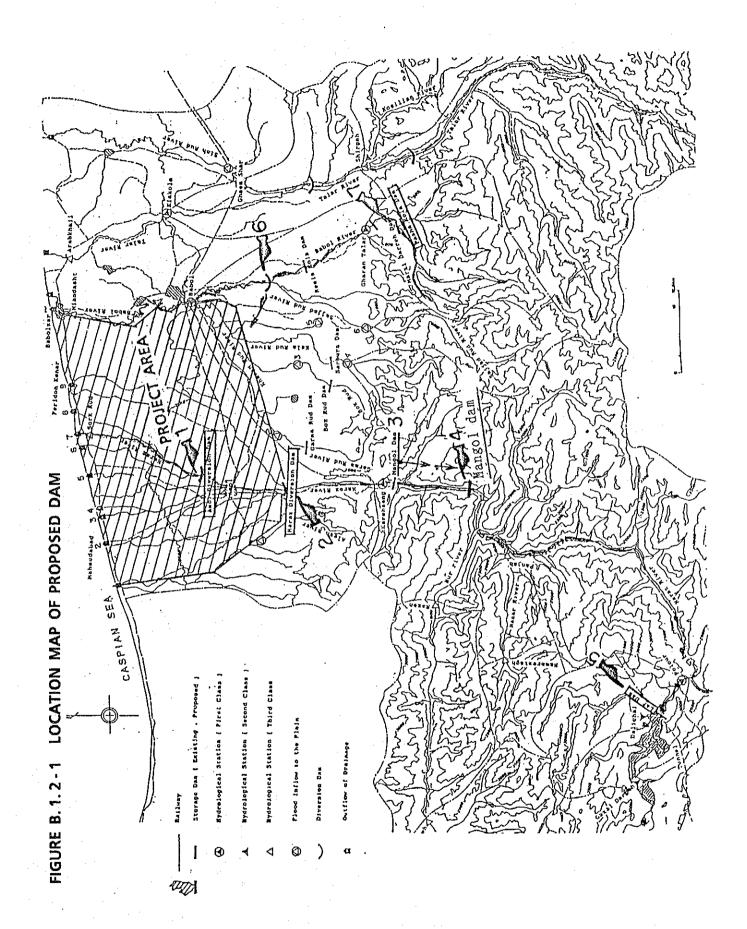
shorta resorvoir (MOM) +inflow+--Haraz H.W--+--Amol H.W---+demand+---Mangol DAM---27,855(ha) 26,023(ha) 53,878 demand defici demand defici total shorta resorvoir 112 ***** Mangol DAM Operation Summary ***** 6695 643 643 Dam Site CA=4,061km2) Dam Site CA=3,773km2) 111.3 ***** Dam Site of Mangol Dam ****** Dam Site of Mangol Dam : ADS Lar DAM OPERATION RULE CASE = DRINKING WATER RATE = RELEASE WATER RATE = ۲ ۴ --i 6 ß AR DAM OPERATION RULE CASE = start of print table No. =1 end of print table No. =1 LARDAM DRINKING WATER RATE = LARDAM RELEASE WATER RATE = .... ADS(Alternative Dam Dam Site of Mangol Dam = 2 .... ODS(Original 1,237 1,248 845 1,002 834 617 755 808 1,402 719 831 1,067 1,181 667 915 938 902 755 7755 6425 6425 6425 666 635 939 year --- **~**1 excess down ar DAM reservoir (1000*CU.M)=300000 **** LAR DAM operation rule (when enpty)**** * case 1 .... drinking:release=50:50 ******* (MOM) witput device printer or screen (p/s)?p peration start year =1956 peration end year =1982 deficit over reserv 269 255 1127 0 00000 5 [ango] DAM reservoir (1000+CU.M)=0
[ango] DAM initial vol (1000+CU.M)=0 110 ************************** flow **** છ 131 53 53 0 0000 1400 400 00 00 00 00 00 00 00 00 0 0 0 000 53878 drinking only ***** Lar DAM Operation Summary 3 Ŗ 222224452892452 22244528925282452 2224452892582452 I 230 (1) - (2) - (3)(4) ummary only (y/n)?y rrigation area(ha) ---- LAR DAM drkn release excess  $\begin{array}{c} 1.131\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.000\\ 0.$ 1956 - 1982 ----11 3 μ 10 248 248 239⁻2239 248 248 237 Site of Mangol Dam ; ADS DAM OPERATION RULE CASE DRINKING WATER RATE RELEASE WATER RATE case case (3)L41 L46 L28 Dam Site of Mangol 1nflow (1) year 11955 11955 11955 11955 11955 11955 11955 11957 11955 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 119577 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 11957 119577 11957 11957 11957 11957 11957 11957 11957 11957 11957 1195 Гаг

TABLE B. 1. 2 - 7 SUMMARY OF WATER BALANCE (CASE-II, ODS)

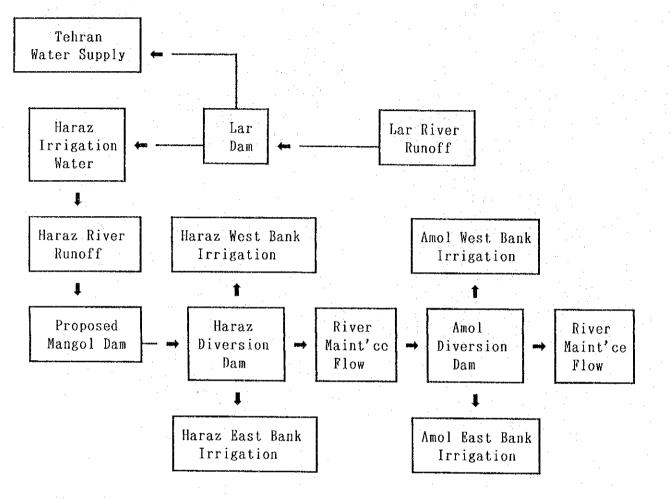
+inflow+--Haraz H.W--+--Amol H.W---+demand+---Mangol DAM---+
27.855(ha) 26,023(ha) 53.878
demand defici demand defici total shorta resorvoir 107 (MOM) 107 43 43 188 1166 336 336 336 265 265 295 295 295 240 Dam Site of Mangol Dam : ODS Lar DAM OPERATION RULE CASE = 1 DRINKING WATER RATE = 1.3 RELEASE WATER RATE = .75 ***** Mangol DAM Operation Summary ***** 1 ..... ODS(Original Dam Site CA=4,061km2)
2 ..... ADS(Alternative Dam Site CA=3,773km2) 105 177 н Ц AR DAM OPERATION RULE CASE Dam Site of Mangol Dam = 1 1,295. 1,295. 1,295. 8359 650 714 756 666 969 ,123 908 839 684 844 762 735 856 856 ,511 year L955 excess (8) 403 down 1556 - 1982 1956 - 1982 wummary only (y/n)?y rrigation area(ha) = 53878 langol DAM reservoir (1000+CU.M)=0 langol DAM initial vol (1000+CU.M)=0 ar DAM reservoir (1000+CU.M)=300000 .*** LAR DAM operation rule (when enpty)**** **************************** (MCM) utput device printer or screen (p/s)?p pperation start year =1956 pperation end year =1982 236 133 128 0 0 0 0 00 251 case 1 .... drinking:release=50:50
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TABLE B. 1. 2 - 8 SUMMARY OF WATER BALANCE (CASE-II, ADS)

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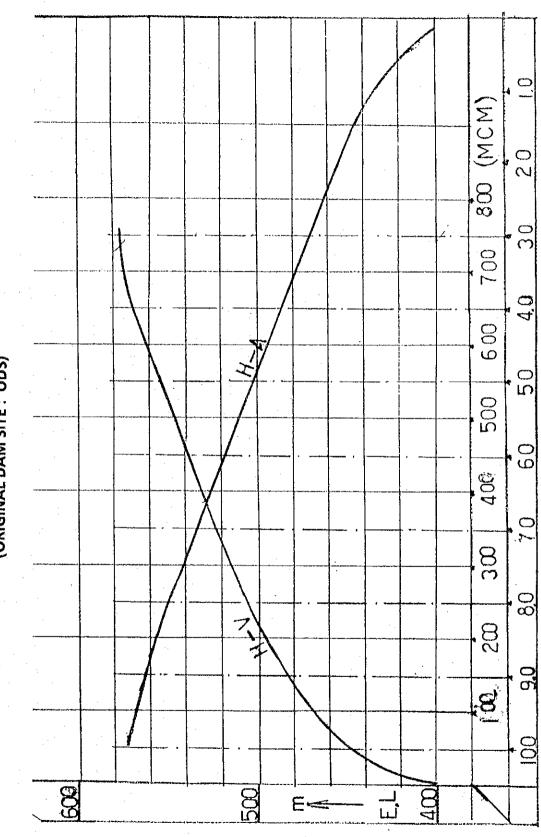
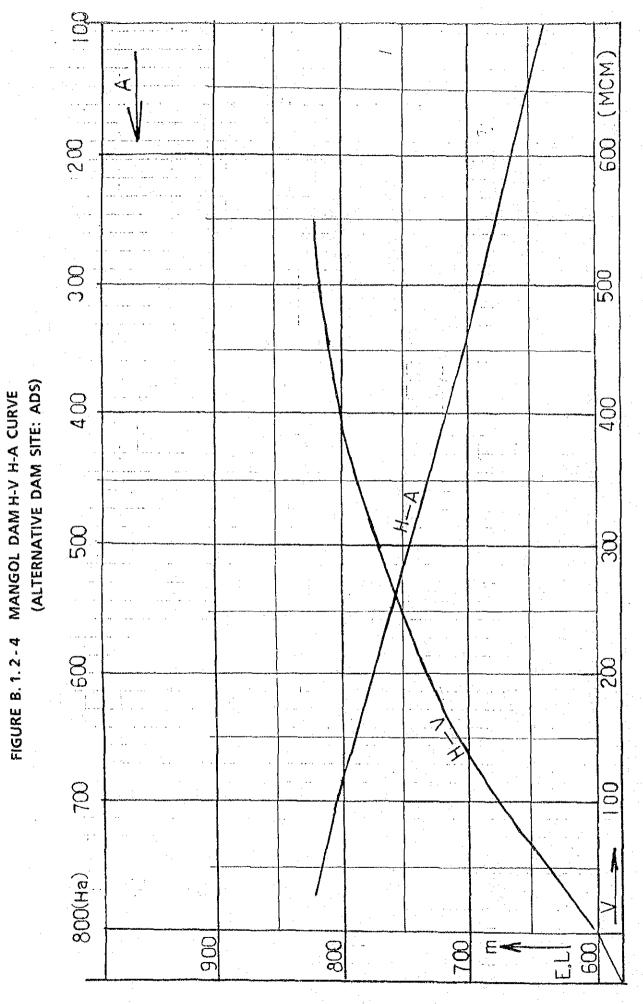


FIGURE B. 1. 2 - 3 MANGOL DAM H-V H-A CURVE (ORIGINAL DAM SITE : ODS)



# B. 1.3 Outline of Geology in Mangol Damsite

Table B. 1. 3 - 1 shows geologic stratigraphy in the proposed damsite, which is referred to geological map attached in Fig B. 1. 3 - 2.

Age	Symbol	Lithofacies and facies
Quaternary	Qa 1	Loose alluvium (river deposits)
	Qs	Talus, fans
	Qv 3	Lava, tuff,
	Qv 2	
Mio-Pliocene	Ple	Conglomerate, with sands, sandy tuff
Eocene	Mm.s.l	Marl, calcareous sandstone sandy limestone
	Pe (FAJAN F.)	Marl, sandstone, gypsum, limestone, locally
	· .	may include undifferentiated younger deposit.
Cretaceous	K2v	Basalt, andesitic tuff, pyroclasstics.
	K21	Limestone
	K2pl.m	Limestone, marl, Marly limestone, silty marl, : locally at base may include indifferent deposits
	Klt (TIZ KUH F.)	Orbitolina limestone.
	Kly	Diabase, basalt, pyroclasstics.
	Klg	Gypsum, dolomitic limestone and shale intercalations
Jurassic	J1 (Lar limestone)	Limestone : locally dolomitic in the upper part.
	Jd (Dalichai F.)	Ammonite bearing marl and limestone, calcareous sandstone.
	Js (Shemshak F.)	Shale, sandstone, siltstone, quartzite, conglomerate
Triassic	Re 2 (Elika F.)	Thick bedded to massive dolomite, dolomitic limestone, limestone
· ·	Re 1 (Elika F.)	Thin bedded limestone with worm trace, calcareous shale.
and a second	Pn (Nesen F.)	Limestone, marly and sandy shales.
	Pr (Ruteh F.)	Fusulina limestone, dolomitic limestone
	Pd (Dorud F.)	Shale, sandstone, limestone, siltstone, quartzite.

TABLE B. 1. 3 - 1 GEOLOGIC STRATIGRAPHY IN THE DAMSITE

Lithofacies in the area along Haraz river mainly consists of talus, river deposits and sedimentary rocks before Miocene. Most of them are calcareous and partially intercalated basaltic lava in Quaternary and basic pyroclassics in Cretaceous. The eminent facies of them repeatedly appears with hundreds or thousands meter cycle. In the conditions of rock outcrop, they were crashed and cracks and corrosion holes are prominent.

It is confirmed that pyroclasstics outcrops at two places, that is, one is a section between 50 km and 60 km of the downstream side, and another point is near Lar damsite. The rock body in the downstream area is able to be correlative with basalt, andesitic lithic tuff and pyroclasstics in Cretaceous. Existence of open cracks at welded time and breakable layers are remarkable in the basaltic lava and its circumference. And then the old basalt probably has the open cracks and very weak part as same as the basaltic lava by reason of the origin.

Geological structure along Haraz river is restricted by the anticline structure which runs in the suburbs of Emarat. The structure has an axis of ENE-WSW in direction, and the axis and its environs make an anticlinal valley by differential erosion. The oldest stratum such as calcareous rocks outcrops in the circumference of the anticline axis near Emarat by the reason of the structure. Newer stratum appears from the axis to the wing of fold, that is, from the suburb of Emarat to the upstream and down stream sides of Haraz river in the geological section. The fault system in the area is eminent in the direction of NW - SE in view of the structure and tectonic movement.

Table B. 1. 3 - 2 shows geography, typical stratum and recommendations in 4 geologic areas.

Based on the geological and geographical recommendation mentioned above, it can be stated that;

- 1. Economic damsite is not found out in Area 1 because of geographical condition, that is, large width of river bed.
- 2. Distribution of high permeable strata consisting of limestone and lava is big obstruction for dam construction in Area 4.
- 3. Area 2 and 3 except Area 1 and 4 mainly consists of calcareous stratum, and volcanic rocks or pyroclassics respectively. It is delicate matter to say that these strata have good geological condition for dam

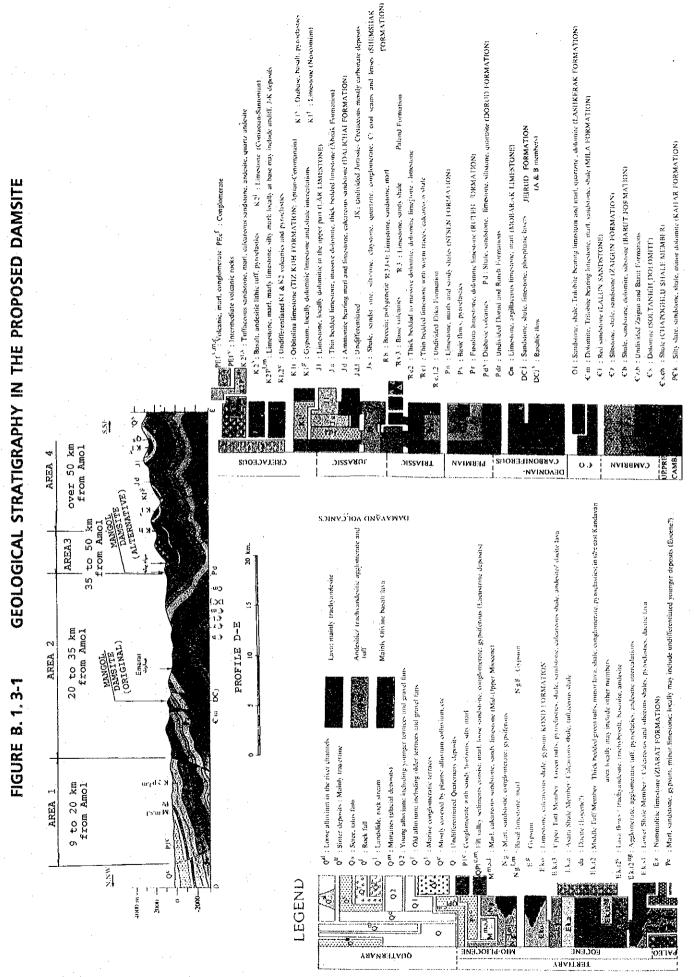
foundation in all cases. Comparing with both Areas, some points of difference can be described as follows;

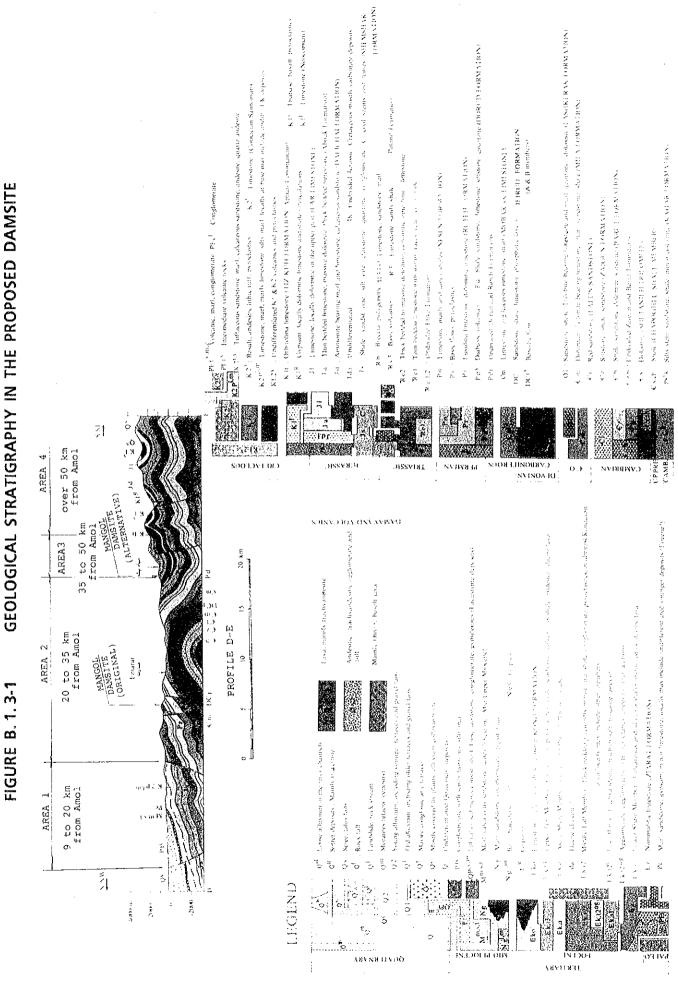
- a) Area 2 is much nearer to the anticline axis and the fault system is more remarkable.
- b) Composed Strata in Area 2 is mainly limestone and solution holes and underground channels, are therefore distinguished.
- c) Limestone in area 2 spreads in the most part.
- d) On the contrary, distribution off calcareous stratum in Area 3 is set a limit or small area in the upstream side of the area.
- e) Volcanic rocks in Area 3 are old and there is some possibility to differ from younger lava appeared surrounding Lar dam.

At present, details of lithofacies and structure in Area 2 and 3 are not enough but based on the study of the geological maps and primary reconnaissance, it is more feasible to propose a damsite at the upstream side in Area 2 than in Area 3.

# TABLE B. 1. 3 - 2 COMPARISON TABLE OF GEOLOGIC AREA

Geologic Area	Geography	Typical Stratum	Recommendations
Area 1	Hilly area spreads out with 700 to 800 meters in fixed height. Height and width of river bed area 200 to 350 meters and 400 to 800 meters respectively. Cross-profile of valley is flat-floored type.	Marl, Conglomerate, Sandstone, and	The scale of dam will be large because of vast river bed. this area is geologically not appropriate as damsite because dam foundation is soft rock in Tertiary. The dam foundation may be permeable due to existing of conglomerate and limestone in the composed strata.
Area 2	The mountains surrounding Area 2, having 2,000 meter height at the highest top, makes many U-shaped valley with narrow river bed and a few V-shaped valleys in part. The height of river bed is 350 to 600 meters. There are some appropriate sites with large pocket and narrow river bed for dam construction in the downstream area.	limestone, marl, dolomite, shale, sandstone, silt stone, conglomerate.	Typical stratum consists of bedded limestone, dolomite and marl which are highly permeable. These permeable strata exist in widely in Area 2. It is, therefore, necessary that water leak may be occurred not in dam foundation but also from reservoir. Since this area locates nearby the fold axis, fault system can be concentrated.
Area 3	The mountain slope in Area 3 is steep and the top of mountains reaches 3,000 meters in height. Haraz river makes U-shapes valley in an indentation of the river. Height of river bed varies from 600 to 700 meters. River slope is steep and forms 1/75 in gradient.	andesite, tuff, lisic tuff, pyroclasstics, limestone, marl, silty marl	The Area 3 mainly consists of volcanic rocks such as basalt and andesite, and pyroclasstics Key question for dam foundation is generally extent of cracks and existence of high permeable layers in accordance with the cracks extended. But it is not judged at present whether the Area is appropriate or not, because the origin and characteristic of the rock body in Mesozoic is not clear. As limestone outcrops somewhere in all the area, it is need to study water leaks from reservoir.
Area 4	The mountain slope is steep as same as Area 3 and the river makes V- shaped valley in the Area 4. The slope of river bed is also perpendicular with gradient of 1/25. There is not an efficient damsite in the downstream area.	agglomerate, andesitic tuff, pyroclasstics, limestone, marl, silty marl,	Typical stratum extended in Area 4 is limestone or marl, and trachyandesitic lava. Both was leakage strata in Lar dam located at more upstream area. At present, definite countermeasure for cut-off treatment of water leakage from the strata is not found yet out. The treatment of the high permeable strata is indispensable for a plan of dam construction in the Area, which makes big obstruction to future plan.



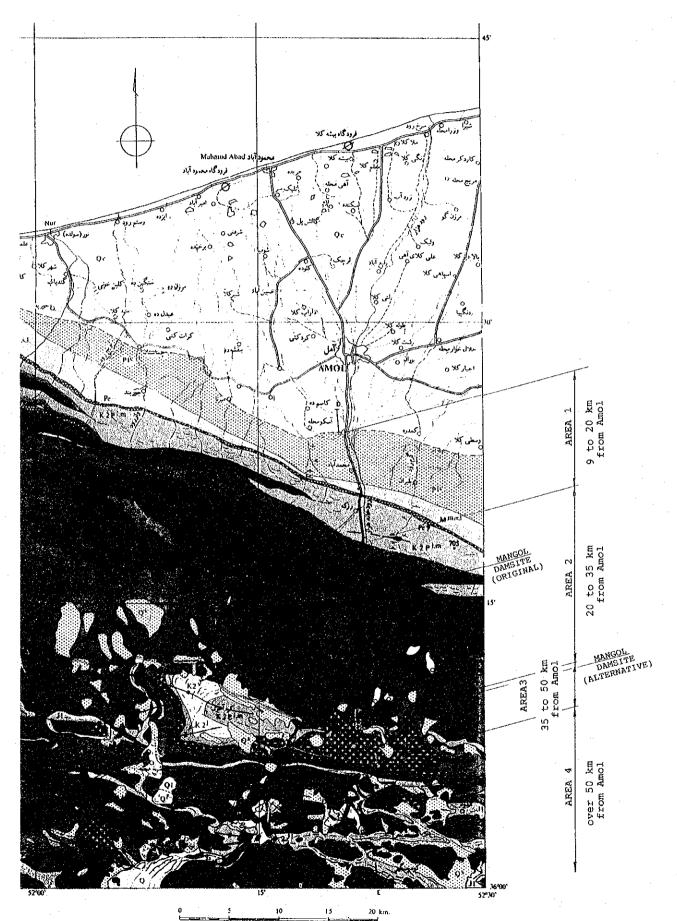


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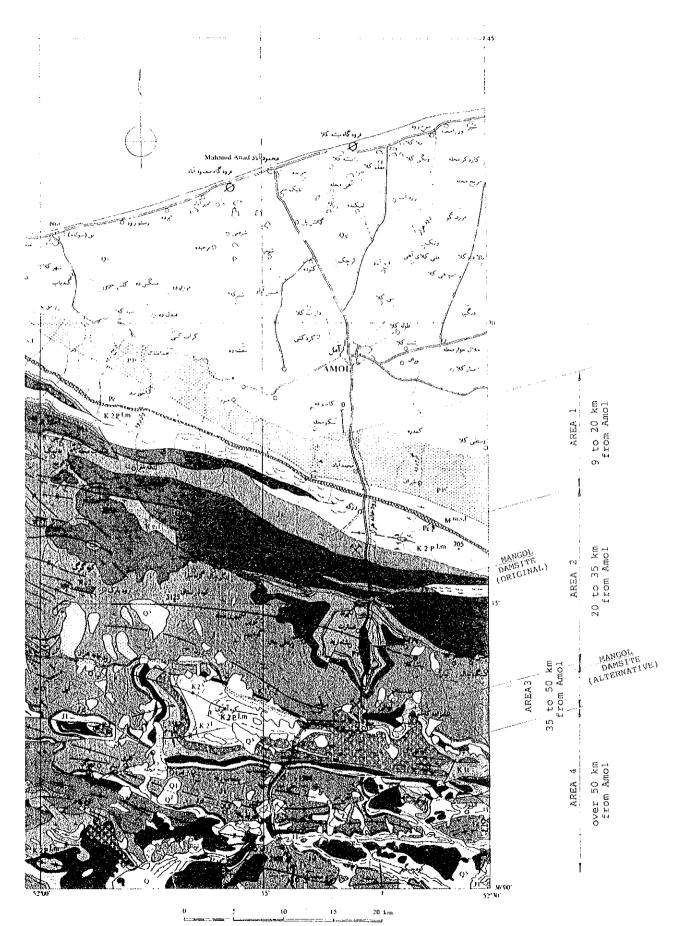
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FIGURE B. 1. 3-2

# SHOWS A GEOLOGICAL ALONG HARAZ RIVER FOR THE GEOLOGICAL STUDY OF DAM CONSTRUCTION



# SHOWS A GEOLOGICAL ALONG HARAZ RIVER FOR THE GEOLOGICAL STUDY OF DAM CONSTRUCTION



# **B. 1.4** Preliminary Water Balance Study in Case of Lar Dam Only

# 1. Introduction

As studied in Paragraph 4. 4. 5 "Availability of Water Resources" in the Main Report, water shortage will not be avoidable not only in a drought year but in a normal year even operated with the Lar dam. The shortage amount of irrigation water is estimated at 133 MCM in a normal year and at 317 MCM in a drought year respectively. For ensuring the irrigation water, as mentioned in Paragraph B. 1. 2 "Preliminary Study of New Water Resources Development", it is necessary to construct the Mangol dam, of which live storage capacity is 305 MCM, for ensuring the necessary irrigation supply to irrigate the project area without water shortage.

In this Paragraph B. 1. 4, additional study has been given to the said Paragraph 4. 4. 5 for examining the water balance for the conditions at Present Stage and at after-land-consolidation (referred as Proposed Stage).

Major differences between Present Stage and Proposed Stage are (1) increase of abbandan storage capacity from 36 MCM to 50 MCM, (2) decrease of irrigation area from 82,834 ha to 78,850 ha due to land consolidation, and (3) increase of return-flow rate from 8% to 12%. Although unit water requirement increases in Proposed Stage due to increase of percolation rate from 2 mm/day at Present Stage to 3 mm/day at Proposed Stage by drainage improvement, total irrigation demand decreases slightly in Proposed Stage due to decrease of irrigation area and change of cropping pattern of rice. Water requirement of rice is discussed in Paragraph B. 2. 5 "Irrigation Water Requirement".

Water supply condition will be improved slightly in Proposed Stage due to slight decrease of total irrigation demand and increase of abbandan capacity. However, improvement is limited and water supply condition is almost same as Present Stage.

On the other hand, there is unconfirmed groundwater of about 23 MCM in present groundwater utilization. (see Table A. 3. 8) Most of this amount is considered to be for irrigation. It is necessary to evaluate this effect in water balance.

In this Paragraph, the study has been carried out to evaluate water shortage in case of provision of the Lar dam only. The aims of this study are as follows;

(1) Aims of the Study

In Paragraph 4. 4. 5 in the Main Report, availability of water resources and the shortage are examined in monthly basis. However, in this Paragraph, the study has been carried out in 10-day basis to clarify the necessary operation when water shortage. The aims of this study are as follows;

clarification of amount and occurrence period of water shortage.

- clarification and evaluation of yield decrease by water shortage.
- optimum operation of the Lar dam and the abbandans under water shortage.
- clarification of effect of te unconfirmed groundwater amounting to 23 MCM.
- clarification of differences on crop yield and water supply conditions between Present Stage and Proposed Stage.

(2) Relation between Water Shortage and Yield of Rice

This relation is mentioned in Paragraph B. 2. 7 "Relation between Water Shortage and Yield of Rice". In this study, as an index of water shortage, water shortage causing 10% yield decrease (equivalent to 90% Yield Level) is selected as below;

Dec	adal Day	Water Saving	Growing Stage		
Apr	III	10%	Vegetative		
May	Ι	11	· · · · · · · · · · · · · · · · · · ·		
	П	4	4		
	ĪĪI	4	11		
Jun	Ī	11	11		
-	ĪI	4	4		
	ĪĪI	4	11		
Jul	Ī	11	11		
	ĨI	5%	Flowering		
	m	4	4		
Aug	I	10%	Yield Formation		
0	ĪT	11	4		
	ÎN -	4	4		
Sep	I	4	11		
~~₽	п	11	4		

From the results of calculation of water saving amount at 90% Yield Level for 27 years from 1956 to 1982 (see Table B. 1. 4-1 and B. 1. 4-2), water saving amount is around at 90 MCM both for present and proposed stages. If another 90 MCM of water saving is conducted, yield will decrease to 80% Yield Level.

2. Operation Case and Rule of Water Resources Facility

(1) Preconditions of Water Resources Allocation

Allocation of water resources is preconditioned as below;

1) Maintenance Flow of the Haraz River

Maintenance flow of the Haraz river is environmentally necessary and it is prior to irrigation demand. The amount of this flow is set as below;

Jan	I	0.30 cms
	II III	0.30 0.30
Feb	I	0.60
ren	Î	0.60
	ÎII	0.60
Mar	Î	0.60
	ÎI -	0.60
	III	0.60
Apr	I	0.30
· -	II	0.30
	III	0.30
May	I	0.30
	II	0.30
	III	0.30
Jun	I	0.30
	II	0.30
т. 1	III	0.30
Jul	I	0.30
	II III	0.30
A 110	I	0.30 0.30
Aug	II	0.30
	Î	0.30
Sep	I	0.30
юср	Î	0.30
	ÎÎI	0.30
Oct	I	0.60
	II	0.60
	III	0.60
Nov	Ι	0.60
	II_	0.60
	ĨĦ	0.60
Dec	I	0.60
	II	0.60
	III	0.60

# 2) Irrigation Requirement

Irrigation requirement is calculated based on water requirement (Table B. 2. 5-5 to 7 for present and Table B. 2. 5-8 to 10 for proposed) and effective rainfall (Table B. 1. 2-2), and using the overall irrigation efficiency 0.7 for surface water (Paragraph B. 2. 5. 6). Irrigation requirement of 10-day basis is summarized as below;

Water Requirement

					•	•				(Unit	: mm)
Mo	nth		Present Stage				Proposed Stage				
		EMV	MMV	LMV	Ave.	Month	EMV	MMV	LMV	Ave.	Month
Apr	I	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	
	Н	0.0	0.0	0.0	0.0	68.9	0.0	0.0	0.0	0.0	47.3
	Ш	101.3	0.0	152.0	68.9		60.2	0.0	98.9	47.3	
May	Ι	101.3	101.3	152.0	109.4		64.9	64.9	129.8	81.1	:
	II	101.3	101.3	68.4	96.0		91.9	64.9	78.4	78.1	253.3
	III	76.0	101.3	75.2	86.0		97.1	96.3	86.2	94.1	
Jun	I	83.3	80.5		81.7		92.3	99.1	90.5	94.4	
	11	84.2	81.4	80.5	82.5	247.2	93,3	91.4	90.5	91.9	278.5
	III	82.9	83.3	82.3	83.0		92.0	92.3	92.3	92.2	1. A.
Jul	Ι	77.1	81.8	81.8	79.7		88.3	91.0	91.8	90.2	
	II	74.8	80.6	83.6	78.5		87.1	89.7	93.6	89.7	275.8
	Ш	51.8	84.8	87.1	70.6		93.3	97.1	98.1	95.9	
Aug	I	28.0	68.6	68.6	50.7		51.8	80.6	78.6	69.3	
	II	10.8	55.9	64.7	37.5	113.1	24.9	66.1	74.7	52.8	156.5
	III	0.0	38.8	58.6	24.9		0.0	46.8	67.2	34.4	
Sep	I	0.0	21,1	32.0	13.6		0.0	21.1	37.8	17,4	
	П	0.0	0.0	17.8	2.8	16.4	0.0	0.0	21.1	5.3	22.7
	III	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	
То	tal	872.8	980.7	1,185.1	965.8		936.3	1,001.3	1,229.5	1,034.1	

(Note) Average is calculated based on the weight of cropping intensity.

1) Present Stage : EMV = 0.44, MMV = 0.40, LMV = 0.16

2) Proposed Stage : EMV = 0.375, MMV = 0.375, LMV = 0.250

# 3) Irrigation Area

Out of the total irrigation areas of 82,834 ha for present and of 78,850 ha for proposed, water balance study has been carried out for the irrigation areas which are irrigated by surface water from the Haraz river and abbandans. Subject study areas are 65,892 ha for present and 60,056 ha for proposed respectively. Irrigation areas commanded by groundwater and return-flow are constant, and excluded from the study of water balance of surface water. Irrigation areas by water resources are as follows;

	Presen	it Stage	Proposed Stage		
Water Resources	Available Water (MCM)	Irrigation Area (ha)	Available Water (MCM)	Irrigation Area (ha)	
Groundwater	143	12,394	143	12,346	
Return-flow	51	4,548	77	6,448	
Surface Water	678	65,892	692	60,056	
(Haraz River)	(642)	(62,681)	(642)	(55,869)	
(Abbandans)	(36)	(3,211)	(50)	(4,187)	
Total	872	82,834	912	78,850	

# Irrigation Areas by Water Resources

(Note) based on Table in Paragraph B. 1. 1. 6

# (2) Operation Cases

Study has been made on three cases both for present stage and proposed stage, that are for the averaged year, the normal year and the drought year. Those years are as follows;

Averaged Year	:	averaged from 1956 to 1982 (27 years)
Normal Year	•	1977 (Runoff of the residual basin during irrigation period is closest to the averaged year)
Drought Year	•	1970 (Runoff of the residual basin during irrigation period is close to 1/10-year probable value. see Paragraph. A. 2. 2. 1)

# (3) Operation Rule

Water balance study was carried out under following operation rule;

- Maintenance flow of the Haraz river is prior to irrigation water.
- Storage water of abbandans is utilized prior to storage water of the Lar dam.

- Abbandans will store excess water when river flow is more than irrigation demand and abbandans have a room to store excess water.

Lar dam will release irrigation water only when irrigation water is deficient and abbandans are empty.

# 3. Runoff Discharge of the Haraz River

Runoff discharge of the Haraz river has been given on decadal day (10day basis) of Gregorian calendar converted from the daily basis data of Iranian calendar, which were given from MahabGhodss Consulting Engineer. The study used the data of a period from 1956 to 1982, in which the Lar dam was not yet constructed and runoff was virgin flow.

Runoff discharge of the residual basin below the Lar dam was calculated subtracting the runoff discharge of Polour from that of Karehsang. Runoff discharge of the residual basin is shown in Table B. 1. 4-9.

# 4. Storage Capacity of Abbandan and Available Release of Lar Dam

Storage capacity of abbandans is totally 36 MCM at present stage and 50 MCM after excavation at proposed stage. Abbandans will be filled before irrigation, and when excess water is available during irrigation period.

Release of Lar dam is maximumly 240 MCM for irrigation during an irrigation period. Release will be made when irrigation water is deficient and abbandans are empty.

# 5. Calculation of Water Balance

Water balance study has been made for three cases both for present stage and proposed stage as mentioned above. Other than three cases, all years from 1956 to 1982 have been studied for clarifying excess water utilization by abbandans and water shortage with and without the unconfirmed groundwater.

The results of water balance of three cases are presented in Table B. 1. 4-3 to 5 for present stage and Table B. 1. 4-6 to 8, and water balance is illustrated in Figure B. 1. 4-1 for proposed stage.

## Summary of Water Balance of Three Cases

(Unit: MCM)

					(01	110.141()141)
	Pi	esent Stag	(e	Pr	oposed Sta	ge
	Averaged	Normal	Drought	Averaged	Normal	Drought
	year	year	year	year	year	year
	(1956-82)	(1977)	(1970)	(1956-82)	(1977)	(1970)
Irrigation Demand	791	797	795	775	783	778
Water Saving (90% Yield) [1] Irrigation Demand	93	76	97	<u>99</u>	91	104
after Water Saving	<u>698</u>	<u>721</u>	<u>698</u>	<u>676</u>	<u>691</u>	<u>674</u>
Intake during Irrigation Period Irrigated	363	359	172	<u>369</u>	372	173
by Abbandans	36	36	. 36	<u>50</u>	50	50
Irrigated by Lar Dam Total	240	240	240	<u>240</u>	240	240
of Irrigation	<u>639</u>	<u>635</u>	<u>448</u>	<u>659</u>	<u>662</u>	<u>463</u>
Deficiency [2] Deficiency	59	. 86	250	17	29	211
with 23 MCM Deficiency W/O Saving	36 152	63 162	227 347	0 116	6 120	188 315

(Note) 1. 23 MCM: Unconfirmed groundwater

2. Deficiency W/O saving = [1] + [2]

As seeing above table, when yield is allowed at 90% level, water shortage will be 86 MCM for present stage and 29 MCM for proposed stage in normal year. When unconfirmed groundwater of 23 MCM is considered for irrigation use, 90% Yield Level can be retained in proposed stage and slightly lower in present stage. However, extensive water shortage will be caused in drought year, and its shortage will be estimated at 188 MCM in proposed stage.

On the other hand, water shortages of proposed stage are 120 MCM and 315 MCM in a normal year and in a drought year respectively from an aspect of 100% yield level.

# 6. Conclusions

(1) Yield Level

Under water supply only by the Lar dam, same yield level can be retained in proposed stage as in present stage as mentioned above.

(2) Utilization of Excess Water by Abbandans

Availability and utilization of excess water of the Haraz river have been examined for both stages based on the residual flow and irrigation demand on 10-day basis for 27 years from 1956 to 1982. The results are shown in Table B. 1. 4-1 for present stage and Table B. 1. 4-2 for proposed stage. The results are summarized as below;

Chance of availability and utilization of excess water is not frequent for both stages in the irrigation period, because residual flow is generally smaller than irrigation demand through irrigation period. Available years are only 3 years in 27 years, and those years are 1956, 1965 and 1975.

(3) Water Shortage

Frequency of water shortage and yield level of both stages are calculated in Table B. 1. 4-1 and B. 1. 4-2. Yield levels of both stages are almost same, but yield level of proposed stage is slightly higher as compared as follows;

	Frequency of Yield Level								
	Present Stage				Proposed Stage				
	in 27 years		Frequency		in 27 years		Frequency		
Vithout Unconfirmed Grou	ndwate	er 23 MC	M						
100% Yield	5	(5)	1/5	(1/5)	6	(6)	1/5	(1/5)	
$90 = \langle \text{Yield} \langle 100\% \rangle$	2	(7)	1/13	(1/4)	2	(8)	1/13	(1/3)	
80 = < Yield $< 90%$	7	(14)	1/4	(1/2)	8	(16)	1/3	(2/3)	
Yield $< 80\%$	13	(27)	1/2	(1/1)	11	(27)	1/2	(1/1)	
Vith Unconfirmed Ground	vater 2	3 MCM							
100% Yield	6	(6)	1/5	(1/5)	7	(7)	1/4	(1/4)	
$90 = \langle \text{Yield} \langle 100\% \rangle$	2	(8)	1/13	(1/3)	2	(9)	1/13	(1/3)	
80 = <Yield $< 90%$	7	(15)	1/4	(1/2)	9	(18)	1/3	(2/3)	
Yield < 80%	12	(27)	1/2	(1/1)	9	(27)	1/3	(1/1)	

## Yield Levels of Both Stages due to Water Shortage

(Note) ( ): Shown in accumulation.

# (4) Effect of Unconfirmed Groundwater

As mentioned in Table A. 3. 8 "Groundwater utilization Quantity for Project Area", unconfirmed groundwater is estimated totally at 23 MCM. The purpose of its utilization is not identified, but it is considered mostly for agriculture. Especially in drought year, it will be able to be utilized for agriculture. When it is utilized fully for agriculture, yield level can be improved at certain level.

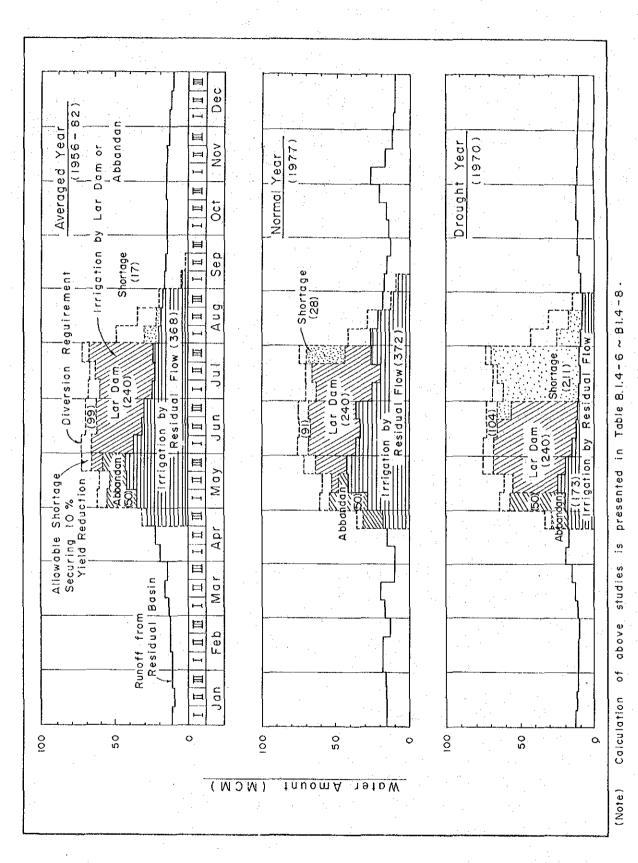
# (5) Effect of Water Saving

In case water saving is introduced beforehand, yield reduction can be minimized. Yield more than 80% can be expected twice in three years, and 90% and 100% yields are once in three and four years respectively at proposed stage. If water saving is not introduced, water shortage will cause sever damage in late stage of rice growing.

# (6) Prediction of Water Saving

If snow depth is less in the Alborz and sever water shortage is predicted beforehand, it is recommended to operate surface water irrigation at 90 % or 80%-yield level. In this case, yield of 80% or more can be ensured in most years. ILLUSTRATION OF PRELIMINARY WATER BALANCE STUDY IN CASE FIGURE B.1.4-1

OF LAR DAM ONLY (PROPOSED)



$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$	÷			IT: MCM)			: •			
Year         10% Yield         Water         Runoff         Irrigation Abbandan Lar Dam         23 HCH         23 HCH <td>·</td> <td></td> <td></td> <td></td> <td></td> <td>ual flow</td> <td>Irrigat</td> <td>ion by</td> <td>Water S</td> <td>shortage</td>	·					ual flow	Irrigat	ion by	Water S	shortage
$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$	· ·	Demand	(securing	Demand after	Annual	Intake in		T	W/0	WZ I
$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$			10% Yield	Water	Runoff	Irrigation	Abbandar	Lar Dam	i	
1956 $605.84$ $60.29$ $745.55$ $801.59$ $415.66$ $39.49$ $240.00$ $50.40$ $27.40$ 1957 $695.21$ $38.23$ $656.98$ $953.97$ $439.09$ $36.00$ $181.89$ $0.00$ $0.00$ 1958 $745.81$ $97.81$ $648.00$ $603.65$ $300.05$ $36.00$ $181.69$ $0.00$ $0.00$ 1959 $742.47$ $81.02$ $701.45$ $827.23$ $509.79$ $36.00$ $185.66$ $0.00$ $0.00$ 1960 $814.29$ $96.62$ $717.67$ $663.94$ $285.07$ $36.00$ $240.00$ $156.60$ $133.60$ 1961 $777.71$ $95.49$ $682.22$ $523.12$ $238.52$ $36.00$ $240.00$ $167.70$ $144.70$ 1962 $761.78$ $97.47$ $664.31$ $510.21$ $283.96$ $36.00$ $240.00$ $115.70$ $92.70$ 1964 $810.80$ $98.07$ $712.73$ $562.67$ $321.03$ $36.00$ $240.00$ $115.70$ $92.70$ 1965 $828.32$ $103.65$ $724.67$ $711.86$ $425.69$ $36.00$ $240.00$ $15.52$ $92.70$ 1966 $826.72$ $103.65$ $724.67$ $711.86$ $425.69$ $36.00$ $240.00$ $15.52$ $92.70$ 1966 $826.72$ $103.65$ $724.67$ $711.86$ $425.69$ $36.00$ $240.00$ $15.52$ $10.52$ 1967 $818.65$ $103.65$ $715.00$ $511.50$ $285.48$ $36.00$ $240.00$ $153.52$ <	Year		level)	Saving		Period			23 нсн	23 MCM
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$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$		(		745.55	881.59		39.49	240.00	50.40	27.40
$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$				1	953.97	439.09	36.00	181.89	0.00	0.00
$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$					603.65	300.05	36.00	240.00	71.95	48.95
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1977         797.10         76.07         721.03         728.04         359.17         36.00         240.00         85.86         62.86           1978         757.23         103.36         653.87         566.94         196.65         36.00         240.00         85.86         62.86           1979         829.50         100.84         728.66         584.18         174.92         36.00         240.00         181.22         158.22           1979         829.50         100.84         728.66         584.18         174.92         36.00         240.00         277.74         254.74           1980         836.11         85.76         750.35         540.72         205.81         36.00         240.00         285.55         62.55           1981         772.11         101.16         670.95         661.86         309.40         36.00         240.00         85.55         62.55           1982         787.53         96.62         690.91         492.10         210.11         36.00         240.00         20.80         181.80           Average         790.29         87.53         702.76         677.83         345.36         37.77         217.52         102.11         85.07 <td></td>										
1978         757.23         103.36         653.87         566.94         196.65         36.00         240.00         181.22         158.22           1979         829.50         100.84         728.66         584.18         174.92         36.00         240.00         181.22         158.22           1979         829.50         100.84         728.66         584.18         174.92         36.00         240.00         277.74         254.74           1980         836.11         85.76         750.35         540.72         205.81         36.00         240.00         268.54         245.54           1981         772.11         101.16         670.95         661.86         309.40         36.00         240.00         85.55         62.55           1982         787.53         96.62         690.91         492.10         210.11         36.00         240.00         204.80         181.80           Average         790.29         87.53         702.76         677.83         345.36         37.77         217.52         102.11         85.07										
1979         829.50         100.84         728.66         584.18         174.92         36.00         240.00         277.74         254.74           1980         836.11         85.76         750.35         540.72         205.81         36.00         240.00         268.54         245.54           1981         772.11         101.16         670.95         661.86         309.40         36.00         240.00         85.55         62.55           1982         787.53         96.62         690.91         492.10         210.11         36.00         240.00         204.80         181.80           Average         790.29         87.53         702.76         677.83         345.36         37.77         217.52         102.11         85.07	1 1									
1980         836.11         85.76         750.35         540.72         205.81         36.00         240.00         268.54         245.54           1981         772.11         101.16         670.95         661.86         309.40         36.00         240.00         85.55         62.55           1982         787.53         96.62         690.91         492.10         210.11         36.00         240.00         20.80         181.80           Average         790.29         87.53         702.76         677.83         345.36         37.77         217.52         102.11         85.07										1
1981         772.11         101.16         670.95         661.86         309.40         36.00         240.00         85.55         62.55           1982         787.53         96.62         690.91         492.10         210.11         36.00         240.00         85.55         62.55           1982         787.53         96.62         690.91         492.10         210.11         36.00         240.00         204.80         181.80           Average         790.29         87.53         702.76         677.83         345.36         37.77         217.52         102.11         85.07										
1982         787.53         96.62         690.91         492.10         210.11         36.00         240.00         204.80         181.80           Average         790.29         87.53         702.76         677.83         345.36         37.77         217.52         102.11         85.07										
Average 790.29 87.53 702.76 677.83 345.36 37.77 217.52 102.11 85.07										
										the second se
INDUA TIENU LEVEN ; YEARS NOT REQUIRING WATER SAVING (740-1715=171) 6 1 1 1							71 > = (21)	5 5		
Nore than 90% Yield Level : without 23MCH 7	More tha	n 90% Yiel	d Level :	without 23MC	M			J	7	<u>├</u> ─────
Nore than 90% Yield Level : with 23MCM 8						· · · · · · · · · · · · · · · · · · ·			· · · ·	8
Nore than 80% Yield Level : [8] <= 90 MCH without 23MCM 14	More tha	n 80% Yiel	d Level :	[8] <= 90 MC	M withou	t 23MCM			14	
Nore than 80% Vield Level : (9) <= 90 MCN with 23MCM 15	More tha	n 80% Yiel	d Level :	[9] (= 90 MC	M with 2	3 M C M				15

TABLE B.1.4-1 SUMMARY OF WATER BALANCE (PRESENT) IN CASE OF LAR DAM

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(Note)

Annual: Jan - Dec , Irrigation Period: Apr III - Sep II
 23 MC: Unconfirmed Groundwater (probably for Agricultural Use)
 W/O: without, W/: with
 [8]=[3]-([5]+[6]+[7]) [9]=[8]-23

TABLE B, 1.4-2	SUMMARY OF	WATER	BALANCE	(PROPOSED)	IN	CASE	OF	LAR	DAM	
		11 July 200	en dia serie		e			1.8.8		
· .	ONLY (UNIT : I	MCM) 👘								

	ridation								
		Water Save	Irrigation	Resid	ual Flow	Irrigat	ion by	Water S	hortage
	Demand	(securing	Demand after	Annual	Intake in			₩/0	₩7.
- A		10% Yield	Water	Runoff	Irrigation	Abbandani			
Year		tevel)	Saving		Period		. 1	23 MCM	
	(1)	[2]	[3]=[1]-[2]	[4]	[5]		[7]	[8]	[9]
1956	787.75	49.27	738.48	881.59	461 67		223.57	0.00	0.00
1957	690.10	36.51	653.59	953.97	506.04	50:22	97.33		0.00
1958	736.24	100.87	635.37	603.65	295.06	50.00		50.31	27.31
1959	766.44	78.94	687.50	827.23	489.31	50.00		0.00	0.00
1960	795.50	101.44	694.06	663.94	287.32	50.00		116.74	93,74
1961	761.99	102.91	659.08	523.12	239.62	50.00		129.46	106.46
1962	750.83	104.90	645.93	510.21	286.90			69.03	46.03
1963	762.48	96.81	665.67	521.67		50.00		79.90	56.90
1964	793.86	100.39	693,47	562.67		50.00		91.41	68.41
1965	809.80	94.90	714.90	711.86	419.64	53 11	240.00	2.15	0.00
1966	809.50	94.64	714.86	648.84		50.00	240.00	41.23	18.23
1967	799.34	105.66	693.68	511.50	285.48	50.00	240.00	118.20	
1968	744.75	91.79	652,96	770.45	465.27	50.00			0,00
1969	784.04	83.14		1090.85		50.00		0.00	0.00
1970	777.72	104.02	673.70	460.60				17	188.14
1971	841.50	101.60	739.90	648.51.	318.24				108.66
1972	748.19	64.28	683.91	905.27	549.50	50.00	84.41	0.00	0.00
1973	787.28	64.09	723,19	970.10	515.12	50.00		0.00	0.00
1974	730.33	105.24	625.09	467.69	224 87			110.22	
1975	748.31	72.54	675.77	730.55	402.84		186.07		0.00
1976	815.45	103.36	712.09	763.40	382.82		240.00		16.27
1977	782.95	91.39	691.56	728.04	372.65		240.00		5.91
1978	746.62	109.12		566.94	196.65			150.85	
1979	810.45	105.66	704.79	584.18				238.67	215 67
1980	815.12	97.73	717 39	540.72	210.66			216.73	
1981	756.82	101.60	655.22	661.86	302.98		240.00		39.24
1982	774.27	103.14	671.13		213.72			167.41	
Average	775.10	91.33	683.77	677.83			209.62	76.13	60.72
100% Yield	d Level :	Years not	requiring *	later sav	/ing (240-	[7]>≈[2])	6	1	
Hore than	90% Yiel	d Level :	without 23MC	M .				8	
More than	90% Yiel	d Level :	with 23MCM		· · · · · ·			<u> </u>	9
More thân			[8] <= 90 MC	H withou	JL 23MCM	<u> </u>		16	
Nore than	80% Yie	d Level :	[9] <= 90 MC	M with 2	2 3 M C M		L <u>.                                    </u>	<u> </u>	18

÷ N

(Note)

Annual: Jan - Dec , Irrigation Period: Apr III - Sep II
 23 MC: Unconfirmed Groundwater (probably for Agricultural Use)
 W/0: without, W/: with
 [8]=[3]-([5]+(6]+(7]) [9]=[8]-23

# TABLE B. 1. 4-3 PRELIMINARY WATER BALANCE STUDY (PRESENT) FOR AVERAGED

# YEAR IN CASE OF LAR DAM ONLY

Year: Averaged Year 1956-1982

						) Year De ôf L		1 101 0	trigat	ion:		240	ксн .	· ·					·					- 1 - L																
	<u> </u>		Jar	1			feb				181			ADT			Hay			Jun	 		JUI			AUg		1	Şep			OCT		l	Nov		· · · ·	Dec		Tolal
(CMS	) []	11.86	11.2	20 11	. 81	12.36	13.1	belov t 6 13.4 7 11.5	0 14.	69 1	7.36 1 5.00 1	15.78 13.64	21.14 18.26	25.91 22.38	31.71 27.40	38.95 33.66	47.86 41.35	49.40 42.68	43.95 37.98	36,49 31,53	33.65 29.07	28.05 24.24	25.33 21.88	26.67 23.04	23.50 20.30	20.64 17.84	19.78 17.09	18.67 16.13	18,28 15,79	15.68 13.55	15.41 13.31	15.69 13.55	16.22 14.02	16,62 14,36	16.67 14.41	15,23 13,16	14.65 12.66	14.17 12.24	12.56 10.85	21.79 677.82
(cas	5	0.3	0.	3	0.3	0.6	: 0,1	Mainten 6 0. 2 0.5	6 0	.6	0.6	0.6	0.3	0.3 0.26	0.3	0.3 0.26	0.3 0.26	0.3 0.26	0.3 0.26	0.3	0.3 0.26	0,3 0.26	0.3 0.26	0.3 0.26	0.3 0.26	0.3 0.26	0.3 0.26	0.3 0.26	0.3 0.26	0.3 0.26	0.6 0.52	13.26								
								the Res 5 11.0					ion (HCP 18		27.14	33.4	41.09	42.42	37.72	31.27	28.81	23.98	21.62	22.78	20.04	17.58	16.83	15.87	15,53	13.29	12.79	13.03	13.5	13.84	13.89	12.64	12.14	11.72	10.33	664.56
		. 0		0	0	0		ement + D	Perco 0	latio Q	n] 0	0	0	0	58.9	109.4	96	- 86	81.7	82.5	83	79.7	78.5	70.6	50.7	37.5	24.9	13.6	2.8	0	0	0	0	0	0	0	0	. 0	0	965.8
Mean	Nel 1	ltriga 0	tion	Requi O	гемел   О	it (#ma) .0	(	D	0	0	0	·	· O	Û	58	102.4	89	79	74.5	75 3	75 8	71.6	70 Å	62 5	39.2	26	13 5	2.5	•	0	0	ó			٥			٥	•	820 1
1		0		0	0		i	0	0	0	0	0		0							108.3					37.1			0	0	0	0	0	0	Û	0	0	0	0 0	839.7 1,199.7
65.8	92 92	пніса			Tyate	u by o	VFIAG	e water	(12)	[2]	ee vobe	rna i X	B.1.2.1]	I																							1			
19VIO	sion	Requi O	16861	nt to O	Surfa 0	ce Wat 0	er (M)	CM) 9	0	0	0	0	. 0	0.	54.62	96.4	83.75	74.39	70.11	70.9	71.36	67.41	66.29	58.84	36.9	24.45	12.72	2.37	0	0	0	0	. 0	0	0	0	0	0	0	790.51
eirri 🦷	atio	n by R	esidu	ia L.º FI	อก (ห	ich)			, ·																	· .											[			
Accu		0		0	• 0	0	(	0	0	0	0	0	0	0	27.14	33,4 60.54	41.09	42.42 144.05	37.72 181.77	31.27	28.81	23.98	21.62 287.45	22.78 310.23	20.04 330.27	17.58 347.85 3	12.72	2.37 362.94	0 362.94	0 362.94	0 362.94 3	0 362.94 3	0 362.94	0 362.94	0 362.94	0 362.94	0 362.94	0 362.94 (	0 362.94	362,94
0efic	ienc	у (нсн 0	I) [D.	R AV	ailáb n I	le Wat	(19	D	0	n	n	0	n	0	27.48	63	12 86	21 07	22 20	10 61	42.55	12 12	41 67	26 06	16 66	6 97														
Accu Exces	s (İl	•	• • •	0	0	0	10.01	0 5 11.0	0	0	0 ·	Ŏ	0	0	27.48	90.48	133.14	165.11	197.5	237.13	279.68	323.11	367.78	403.84	420.7	427.57 4														
							•					ŀ		22.12	. 0		U	. 0	0	U	. 0	. <b>O</b>	0	0	0	0	4.11	13.5	15.53	13.29	12.79	13.03	13.5	13.84	13.89	12.64	12.14	11.72	10.33	301.6
ALIO	vabl		tage	Rate	to Ir			r secur quireme			id (10%	Yield	d Reduct	ion)	- 10 6.49	10. 10.30	10 9,04	10 8.10	10 7,69	10 7.77		10 7.50			30 14-, 32	30 10.59	30 7.03	30 3.84	30 0.79											108.28
Water	Sav	ing Am	ount	secur	 190/9	OX Yie	19 (H(	CH)																																
Accu	<b>n</b> I .	0 0		0 0	0 0	0 0	(	) i ) i	0	0 0	0	0	0	0		10.3 16.79				7.77 49.39	7.81 57.2	7.5 64.7	3.69 68.39	3.32 71.71	14.32 86.03	6.87 97.9	0 92.9	92.90												
Neces Accu		ei 111 0 0	ation	Wate O	r to 0	be sup O	plied (	from Li	ar Dam O	and i	Abbanda O O	ins (Ni O	CH) [Def O O	0	20.99	52.7	33.62	23.87	24.7	31.86	34.74	35.93	40.98	32.74	2.54	0	0	0	0	0	0	0	0	0	0	0	0	0	0	334,67
	1	•		•	°	U	,	, ,	Ĭ.	v	U	v	U	U	20.99	13.68	107.31	131.10	193.00	107.74	222.48	258.41	299.39	332.13	334.67	334.67 3	34.67	334.67	334.67	334.67	334.67	334.67	334.67	334.67	334.87	334.67	334.67	334.67 3	334.67	
Abba Lar	ndan	2	Stor 3 24 27	6 0	in Ab	bandan	s and	Lar Dai	н (ИСИ	)																											·			
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Stor	896	36	3	6	36	36	36	3 3	6	36	36	36	. 36	36	15.01	0	0	0	0	0	0	0	0	Ó	0		4.11		33.14	36	36	36	36	36	36	36	36	36	36	
Short Accu		after 0 0		ise fr O O	OB AD O O	bandan O O	s (MCH (	4) D	0	0	0 0	0 0	0,0	0. 0	0	37.69 37.69	33.62 71.31	23.87 95.18	24,7 119.88	- 31.86 151.74	34.74 186.48	35.93 222.41	40.98 263.39	32.74 296.13	2.54 298.67	0 298.67 2	0 98.67	. 0 298.67	0 298.67	0 298.67	0 298.67	0 298.67	0 298.67	0 298.67	0 298.67	0 298.67	0 298.67	0 298.67 1	0 298.67	298.67
Opera Outf Stor	lov	of ia 0 240		0	0 240	0 240	240	D 24	0 2	0 40	0 240	0 240	0 240	0 240	. 0	37.69	33.62	23.87	24.7	31.86	34.74 53.52	35,93	17.59	0	0	0 0	0	0	0	0 0	0 D	0 0	0	0	0	0 0	0	0 0		240.00
Final Accur	ł	rtage O O	(KCH)	0	0	0	(	0	0	0	0	0	0	0	. 0 0	0	0	0	0	0	0	0	23.39	32.74	2.54	0 58.67	0	0	0	0	0	0	0	0	0	0	0	0	0	58.67
						·····				·····		I	,	<u> </u>		<u> </u>			L	0	0	<u>_</u>	10.03	30.13	30.07	10.01	30.01	1 30:07	30.01	30,07	30.01	30.01	30.0/	1 30.0/	30.07	00.01	1 20.01	30.01	30.07	L]

#### TABLE B. 1, 4-4 PRELIMINARY WATER BALANCE STUDY (PRESENT) FOR NORMAL

YEAR IN CASE OF LAR DAM ONLY Year: Normal Year (1977) Avaiable Storage of Lar Dam for Irrigation: 240 HCH

	·····	Aval	able St	99810	ofila	r Dam Feb	111 101	igatio		240	нсн	<u></u>		r	Hay	· · · · · · · · · · · · · · · · · · ·	· · · ·	JAU	·			<u> </u>	· · · · · ·	AUG		: T	Sep			Oct	r		Nov			Dec		Total
Runoff	) i scha	50 11 901	u ona the	Residu	Jal Ba	sin he	lov the	lar D	Har			Apr													·	l ·		·	·									
(cne)	1 17 7	7 17	AA 18.	99 20	0.78	20.90	15.62	20:16	22.73	11.84 10.23	12.44 10.75	17.18 14.84	21.04 18.18	32.35 27.95	48.41 41.83	48.56 41,96	41.85 36.16	38.10 32.92	40.56 35.04	29.49 25.48	23.27 20.11	30.11 26.02	30.25 26.14	24.56 21.22	21.42 18.51	21.22 18.33	17.21 14.87	15.33 13.25	16.51 14.26	18.60 16.07	24.61 21.26	31.31 27.05	20.22 17.47	14.39 12.43	12.70 10.97	12.02 10.39	12.23 10.57	23.41 728.04
	l ∩ :	3 0	3 0	31	0.6	0.6	0.6	a 0	0.6	e Haraz 0.6 0.52	1 0.3	0.3 0.26	0.3	0.3 0.26	0.3 0,26	0.3 0.26	0.3 0.26	0.3 0.26	0.3 0.26	0.3 0.26	0.3 0.26	0.3 0.26	0.3	0.3	0.3 0.26	0.3 0.26	0.3 0.26	0.3 0.26	0.6	0,6 0.52	0.6 0.52	0.6 0.52	0.6 0.52	0.6 0.52	0.6	0.6 0.52	0.6 0.52	13.26
Availab (NCH)	le Wat 15.0	er of 9 15,	Runoff 19 16.	0 i scha 15   1	arge f 7.43	rom th 17.54	e Resid 12.98	iual Ba 16.9	sin for 19.12	Irriga 9.71	tion (M) 10.49	:H) 14.58	17.92	27.69	41.57	41.7	35.9	32.66	34.78	25.22	19.85	25.76	25.88	20.96	18.25	18.07	14.61	12.99	13.74	15.55	20.74	26.53	16.95	11.91	10.45	9.67	10.05	714.78
Water Re	1	0	0	0	0	avirem O	ent + P O.	ercola 0	tion} 0	0	. 0	0	68.9	109.4	96	86	81.7	82.5	83	<b>7</b> 9.7	78.5	70.6	50.7	37 5	24.9	13.6	2.8	0	0	0	0	0	0	0	0	0	0	965.8
Hean Net		gatiun O	0		(nn) 0	0	0	0	0	o	0	0	64	101.5	88.1	78.1	79	79.8	80.3	74	72.8	64.9	32.1	18.9	6.3	6.9	0	0	0	0	0	0	0	0	0	0	0	846.7
Diversio	1	0	0	0]	0	0	0	0	. 0	0 Na kana kana kana kana kana kana kana ka			91.4	14.5	125.9	111.8	112.9	114	114.7	.105.7	104	92.7	45.9	27	9	9.9	0	0	0	0	0	0	0	0	0	0	0	1,209.7
ltrigat 65,892	010 10 10	ea 10	of the	yateu	uy au	1.1976	HALEF (		1266 H	ppendix	B. I. Z.									-																	1	
Diversi		uire≊e 0	nt to S O	urfaci 0	e Wate O	UL (HCH	) 0	0	. 0	0	0	0	60.23	95.54	82.96	73.54	74.39	75,12	75.58	69,65	68.53	61.08	30.24	17.79	5.93	6.52	0	0	0	0	0	0	0	0	0	0	0	797.10
lrrigat	ion by	Resid	ual Flo	ง (หต ง ไ	H) 0	0	0	0	. 0	0	. 0	. 0	17.92	27.69	41:57	41.7	35.9	32.66	34.78	25.22	19,85	25.76	25.88	17.79	5.93	6.52	Ó	. 0	. 0	0	0	0	. 0	0	. 0	0	0	359.17
Accumit		0	0	0	Ō	0	0	0	0	0	0										277.29							359.17	359.17	359.17	359.17	359.17	359.17	359.17	359.17	359.17	359.17	
Oeficie	nсу (н ]	CH) [[ 0	Rİ-AVA O	ilablı 0	e Wate O	(1) 0	. 0	0	0	0	0	0	42.31	67.85	41.39	31,84	38.49	42.46	40.8	.44.43	48.68	35.32	4.36	0	0	0	0	0	0	0	0	0	. 0	0	0	0	0	437.9
ACCURI. Excess	(нсп)	0	Û	0	0	0	. 0	0	0	. 0	0			110.16	151.55	183.39	221.88	264.34	305.14	349.57	398.25	433.57					:											
	15.0	9 15.	19 16.	15 1	7,43	17.54	12.98	16.9	19.12	9.71	10.49	14.58	0	0	0	0	0	0	0	0	0	0	0	3.17	12.32	11.55	14,61	12.99	13.74	15.55	20.74	26.53	16.95	11.91	10.45	9.87	10.05	355.6
Aliovab Aliovai Aliovai	le Sh	ortage	e Rate 1						Yield (	10% ¥le	lđ Redu	tion)	10 6.49	10 10.30	10 9.04					10 7.50	5 3.69	5 3.32	30 14.32	30 10.59			30 0.79											108.28
Mater Sa	aving (	Asount O	securi 0	ng 90: 0	¥ Yiel O	н (нсн 0	) 0	0	0	0	0		6.49					7.77			3.69 68.39						0	0	0	0	0	0	76 07	0	0	0		76.07
Actual		v isstic	U Nator		V 0 5400	U Liod f	V 1011 192	0.98 3	u ad Abba	ndans (I		0 Afiria					41.62	49.09	97.2	04.1	60.39	71.71	10.01	10.01	10.01	10.01	10.01	10.07	10.01	10.01	10.07	10.01	10.01	10.07	10.01	10.07	10.01	
Accual	1	0	0	0	0	0	0			0		0	35.82	57.55	32.35	23.74					44.99 329.86				. 0 361.86		0 361.86	0 361.86	0 361.86	0 361.86	0 361.86	0 361.86	0 361.86	0 361.86	0 361.86	0 361.86		361.86
Availab Abband Lar Dai Tota	ាន ៖	2	rages i 36 40 76	n Abb.	andans	and L	ar Dam	(нсн)																												· .		
Operation Outflow Inflow Storage	1	0 0	0 0	0 0 36	0 0 36	0 0 36	0 0 36	0 0 36	) 0 . 0 . 36		0	0 0 36	35.82 0 0.18	0.18 0 0	0 0 0	0	0 0 0	0 0 0	0 0 0	0 0 0	0 0 0	0 0 0		0 3.17 3.17			0 8.96 36		0 0 36									
Shortag Accumi	e afte		ase fro 0 0			6 (MCK) 0 0	0 0	0	) () ) ()	0	0	0	0	57.37 57.37	32.35 89.72	23.74 113.46	-30.8 144.26	34.69 178.95	32.99 211.94	36.93 248.87	44.99 293.86	32 325.86	0 325.86	0 325.86	0 325.86	0 325.86	0 325.86	0 325.86	0 325.86	0 325.86	0 325.86			0 325.86		0 325.86		325.86
Operati Outflo Storag	4	0	0	0	0 240	. 0 240	0 240	0 240	) 0 ) 240	1 0 240	-	0 240			32.35 150.28			34.69 61.05		28.06 0	0	0 0	0	0	0 0	0	0 0	0 0	0	0 0	0 0	0	0 0	0 0	0	0 0	0 0	240.00
Final S Accumit	hortag	0 0	1) 0 0	0 0	0 0	0	0 0	0	) (	0	0	0	0 0	0	0	0	0	0	0 0	8.87	44.99 53.86	32 85.86	0 85.86	0 85.86	0 85.86	0	0 85.86	0 85.86	0 85.86	0 85.85	0 85.86							

#### TABLE B. 1. 4-5 PRELIMINARY WATER BALANCE STUDY (PRESENT) FOR DROUGHT

YEAR IN CASE OF LAR DAM ONLY Year: Drought Year (1970)

		A	iva la	ble Sti			IT Dan	for Ir	rigat	ion:		240	нсн	· · · ·						<u>.</u>																				
			Jan				feb				16			19A		1	May			Jun			្រៃ			AUg			Sep			0c t			Nov			Dec		Total
	11	5.06	14.5	5 13, !	50   12	2.75	13.27		13.0	00 14							28.03																							
	1	0.3	0.	30.	3	0.6	0.6	0.6 0.52	0	. 6	0.6	0.6			0.3 0.26		0.3				0.3 0.26										0.6				0.6 0.52			0.6 0.52	0.6 0.52	13.26
Availab (MCH)	ble (   12	Nater 2,75	0f R 12.3	inoff <u>(</u> 11.	) i scha 4	1 901 10.5	rom ti 10.95	he Resi 10.55	dual     10.1	Basin 71 12	for 11 . 15	rrigat 14.6	ion (XC) 19.49	i) 17.18	17.3	19,07	23.96	18.89	15.42	12.24	10.83	9.99	9.23	10.28	8.92	8.27	8.16	8.76	10.57	11.79	11.33	11.85	12	11.45	11.34	10.8	10,96	10.35	10.96	447.34
Water R Mean Ne	1	0		)	0	0	quires O	ient + O	1	lation 0	0	0	. 0	0.	68.9	109.4	96	85	81.7	82.5	83	79.7	78.5	70.6	50.7	37.5	24.9	13.6	2.8	0	0	0	. 0	0	0	0	Q	. 0	0	965.8
0 i versi	     	0 Nater 0	Requ	irement	0  t (112) 0		0 . R. /0. 0	7) 0		0 0	0	0 0	0 0				91.4 130.6										8.4 12	0. D	0	0	0	0	0 0	0 0	0 0	0 0	0 0	0 0	0	844.4
lrrigat 65,892 Diversi	2								(ha) 	[ 56	e App(	endix	B.1.2.1]																				-							
irrigat	Lion	0 by Re	Šidua		0   / (HCP	· 0 1),	0	0		0	0	0	.0	. 0.	58.64	98.64	86.05	76.63	72.68	73,4	73.86	68.26	67.14	59.7	32.22	19.77	7.91	0	0	0	0	0	0	0	0	0	0	0	0	794.90
ACCURI	1	0 0	. (	) }	0	0. 0	0	0 0		0 0	0 0	0 0	0	0			23.96													0 172.31	0 172.31 1	0 72,31	0 172.31	-	0 172.31		0 172.31	0 172.31		172.31
0eficie Accumi		0	(0.) ( )	{Avai } )	ab l e 0   0	e Wate O O	(1 0 0	0 0		0 0	0	0	· 0 0				62.09										0 522.59	0 622.59	0 622.59	0. 622.59	0 622.39 (	0	0 622.59	0 622.59	0 622.59	0 622.59	0 522.59 (	0 622.59		622.6
Excess	11	.75											19.49		0	0	0	0	0	Ó	Ö	0	0	0	0	0	0.25	8.76	10.57	11,79	11.33	11.85	12	11.45	11.34	10.8	10.96	10.35	10.96	275.0
	ible	Short	age I	late to				Securi Sirenen			d (101	¥ Yiei	d Reduci	ion)	10 6.49	10 10,30	10 9.04	10 8.10			10 7.81	10 7.50	5 3.69	5 3.32	30 14,32	30 10.59	30 7.03	30 3.84	30 0.79											108.28
Mater S Accumi		ng A∎o 0 0	iunt s ( (	iecurin H	0 90 <b>%</b> 0 0	Yiel 0 0	d (MCH 0 0	1) 0 0		0	0	0 0	0		6.49 6.49	1	9.04 25.83									10.59 96.62			0 96.62		0 96.62	0 96.62	0 96.62	0 96.62	0 96.62	0 96.62	0 96.62	0 96.62		96.62
Necessa		sşi 111 0 0	tion (	Water	to be	supp O	lied f 0	ros tai 0	r Dam	and A O	bbanda O	 ans (H 0   0	CH) (Dei D	0	34.85	69.27	Shortage 53.05	49.64									0	0	0	0	0	0	0	0	0	0	0	0		525.97
	bie Jans Jans		Stora 36 240 276	). 	й     Абра	ndans	and i	ar Dam	(NCN)	)	v	v	U			108.12	157,17	200.01	230.30	309.11	304,35	413.70	409.30	510.00	323.06	323.97 1	929.97		323.97	323.97	92 <b>9.</b> 97 :	125.97	323.97	929.97	323.97		323.97		323.97	
Doerati Outflo Inflow Storag	1	0 0 0 36	andar ( ( 36	) )	0 0 6	0 0 36	0 0 36	0 0 36		0 0 36	0 0 36	0 0 36	0 0 36	0 0 36	34.85 0 1.15	1.15 0 0	0 0 0 0	0 0 0	0 0 0	0 0 0	0 0 0	0 _0 0	0 0 0	0 0 0	- 0 0 0	0 0 0	0 0.25 0.25		0 10.57 19.58	0 11.79 31.37	0 4.63 36	0 0 36	0 0 36		0 0 36	0 0 36	0 0 36	0 0 36	0 0 36	36.00
Shortag Accumi	1	iter R O O	eleas (	e from I	0   0   0	ndans O O	( KC K) 0 0	0		0 0	0 0	0 0	0	0 0			53.05 121.17																		0 489.97			0 489.97		489,97
Operati Outflo Storag	M	of Lar 0 240	Dan ( 24(	24	0	0 240	0 240	0 240	24	0	0 240	D 240	0 240	0 240			53.05 118.83				0	0	0 0	0 0	0 0	0	0 0	0	0 0	0 0	0	0 0	0	0	0 0	0 0	0	0 0	0	240.00
Final S Accumi		lage ( 0 0	KCH)	)	0	0	0	0		0	0	0 D	0	0	0	C C	0	0			55.22 88.99					0.91 249.97 1	0 249.97	0 249.97	0 249.97	0 249.97	0 249.97	0 249.97	0 249.97	0 249.97	0 249.97	0 249.97	0 249.97	0 249.97		249.97

# TABLE B.1.4-6 PRELIMINARY WATER BALANCE STUDY (PROPOSED) FOR AVERAGED

# YEAR IN CASE OF LAR DAM ONLY Year: Averaged Year 1956-1982

			1: AV Table :				-1982 ana for 1		ation		240	нсн		1.1.1		1												1.1											
<b></b>			an			ſe				Mar	240		104			Hay		<b>_</b>	Jun			Jul		[	Aug			Sep			001		1	Nov			Dec		10131
RUNOFF D (CES) (NCM)	11118	11 36	20 1	1.81 Ì	12.36	13	16 13 4	0 1 1	8 6 9	17 36	15.78 13.64	21.14 18.26	25.91 22.38	31.71 27.40	38.95 33.66	47.86 41.35	49,40 42.68	43.95 37.98	36.49 31.53	33.65 29.07	28.06 24.24	25.33 21.88	26.67 23.04	23.50 20.30	20.64 17.84	19.78 17.09	18.67 16.13	18.28 15.79	15.68 13.55	15.41 13.31	15.69 13.55	16.22 14.02	16.62 14.36	16.67 14.41	15.23 13.16	14.65 12.66	14.17 12.24	12.56 10.85	21.79 677.82
Required (CBS) (NCM)	0.	3	0.3	0.3	0.6	0	.6 0.	61	0.6	0.6	0.6	0.3	0.3	0,3 0,26	0.3	0.3 0.26	0.3 0.26	0.3 0.26	0.3 0.26	0.3 0.26	0.3 0.26	0.3 0.26	0.3 0.26	0.3	0.3 0.26	0.3 0.26	0.3 0.26	0,3 0,26	0.3 0.26	0.6 0.52	0.6 0.52	0.6 0.52	0.6 0.52	0.6		0.6 0.52	0.6 0.52		13.26
Availabl (HCM)	e Wat 9.9	ler of 9 9	RUNO (	DIS 9.94	charge 10.16	from 10.1	the Res 85 11.0	sidua )6   1:	Basi 2.17	n for 14,48	Irriga1 13.12	ion (MC 18	N) 22.12	27.14	33.4	41.09	42.42	37.72	31.27	28.81	23.98	21,62	22.78	20.04	17.58	16.83	15.87	15.53	13.29	12.79	13.03	13.5	13.84	13.89	12.64	12.14	11.72	10.33	664.56
Hater Re	en lup	ent O	(##) (( 0	   qo1:   0				Per 0	colati O	on] 0	0	. 0	0	47.3	81.1	78.1	94.1	94.4	91.9	92.2	90.2	89.7	95.9	69.3	52.8	34.4	17.4	5.3	0	0	0	0	c	, 0	0	0	Û	0	1,034.1
Hean Net		0	0	0	0		0	0	0	0	0	. 0	0	36.4	74.1	71.1	87.1	87.2	84.7	85	82.1	81.6	87.8	57.8	41.3	22.9	4.4	0.1	0 [°]	0	0	0	0	0	0	0	0	0	903.6
Diversio Irrigati		Q	0	0	0		0	0   (ha	0	0 0 992	0 Vihoso			52	105.9	101.6	124.4	124.6	121	121.4	117.3	116.6	125.4	82.6	59	32.7	8.3	0.1	0	0	0	0	6	) 0	0	0	0	0	1,290.9
60,056 Diversio										300 Mp	penarx																												
		0	0	0	-		0	0	0	0	0	. 0	0	31.23	63.6	61,02	74.71	74.83	72.67	72.91	70.45	70.03	75.31	49:61	35.43	19.64	. 3.78	0.06	0	0	0	0	0	0	0	0	0	0	775.28
Accuml.	on by	O O O	ovali O O	04 (1	ncn) 0 0		0	0	0	0	· 0 0	0													17.58 347.85 3					0 368.52	0 368.52	0 368.52	0 368.52	) 0 2 368.52	0 368.52	0 368.52	0 368.52		368.52
Deficien	су (Ж	ICH) [	D. R A1	iailai ailai	ble Wa	ter]										••	20.00	97 11			15 17	40.41	50 51	20.57	17.85	2 81	0		0	0	0	0			0		0	0	406.8
Accumi. Excess (		0	0	0	0		U Q.	0	. 0	0	0	0	0														406.76	406.76	406.76	406.76	406.76	406.76	406.76	5 406.78	406.76	406.76	406.76	406.75	
				· · [			85 11.0					]	22.12	0	0	0	0	0	0	0	0	0	0	0	0	0	12.09	15.47	13.29	12.79	13.03	13.5	13.84	13.89	12.64	12.14	11.72	10.33	296.0
Allovabi Allovab Allovab	le Sh	nortag	e Rate	to 1				-		eld (1	OX Yiel	lq ¥eqnc	tion)	10 4.06	10 6.96	10 6.70	10 8.07	10 8.10	10 7.88		10 7,74			. 30 17.84	30 13.59	30 8,85	30 4.48	30 1.36											:111.50
Water Sa Accumi.	ving	Amour O O	t secu O O	'ing ' 0 0	90% ¥i 0 0	eld (I	HCH) 0 0	0 0	0	0	0 0	0	0 0		6.96 11.02										13.59 96.81			0 99.62	0 99.62	0 99.62	0 99.62	0 99.62	0 99.61	0 0 2 99.62	0 99.62	0 99.62	0 99.62	0 99.62	99.62
Necessar	y lir V lir	igati 0	on Wate	rto 1	be su	pplie	d from L	ar D o I	am and	Abban	dans ()	  CM) (De 						29.01	33.52	36.19	38.73	44.56	48.42	11.73	4.26	0	0	0	0	0	n	0	r	) 0	0	0	0	0	307.14
Accuml.		0	õ	ő	Ő		0	0	Ö	Õ	ů	ő	Ũ													307.14	307.14	307.14	307.14	307.14	307.14	307.14	307.14	307.14	307.14	307.14	307.14		
Availabi Abbanda <u>tar Dam</u> Tota	n s		orages 50 240 290	in A	bband <b>a</b>	NS 200	d Lar Da	18 (H	CM)		•						·														·								
Operatio Outfloy Infloy Storage		0 0	dans 0 0 50	0 0 50	0 0 50		0 0 50 5	0	0 0 50	0 0 50	0 0 50	0 0 50	0 0 50	0.03 0 49.97	23.24 0 26.73	0	13.5 0 0	. 0 . 0	.0 0 0	0 0 0	0 0 0	0 0 0	0 0	0 0 0	0 0 0	0 0 0		0 15.47 27.56	0 13.29 40.85	0 9.15 50	0 0 50	0 0 50		; 0 ; 0 ; 50	0 0 50	0 0 50	0 0 50	0 0 50	50.00
Shortage Accumi.	afte	er Rei O O	ease f O O	A 4601 0 0	bbanda 0 0 0	ns (M	CH) 0 0	0	0	0 0	0	0 0	0	. 0 0	0 D										4.26 257.14 2						0 257.14			D 0 4 257.14					257.14
Operatio OutFlow Storage		0	an 0 240	0 240	0 240		0 40 24	0	0 240	0 240	0 240	0 240	0 240	0 240	-					36.19 130.56			47.27 0	0 0	0 0	0	0	0 0	0	0 0	0	0 0	0	) O	0	0	0 0		240.00
Final Sh Accuml.	ortag	)e (MC 0 0	H) 0 0	0	0	 	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0 0	1,15	11.73 12.88	4.26 17.14	0 17.14	0 17.14	0 17.14	0 17. <u>14</u>	0 17.14	0	0 17.14	17.14	D 0 4 17.14	0 17.14	0 17.14	0 17.14	0 17.14	17.14

#### PRELIMINARY WATER BALANCE STUDY (PROPOSED) FOR NORMAL TABLE B. 1. 4-7

# YEAR IN CASE OF LAR DAM STUDY Year: Normal Year (1977)

			: Norma able Sto				for Iri	ridati	on: (		240 1	сн								1.1																			
[		Jai				Teb		1	На	11	The second second second second second second second second second second second second second second second se		Apr			Hay			Jun			Jul			Aug	·	· · · · · · · · · · · · · · · · · · ·	Sep			Oct			Nov			Dec	-	Total
RUNOIT ( (cms) (HCH)	17 77	17	RA 1A 9	9 20	78	20.90	15.62	1 20.1	6 22	73 1 64 1	1.84 0.23	12.44 10.75	17.18 14.84	21.04 18.18	32.35 27.95	48.41 41:83	48.56 41,96	41.85 36.16	38.10 32.92	40.56 35.04	29,49 25.48	23.27 20.11	30.11 26.02	30.25 26.14	24.56 21.22	21.42 18.51	21.22	17.21 14.87	15.33 13.25	16.51 14.26	18.60 16.07	24.61 21.26	31.31 27.05	20.22 17.47	14.39 12.43	12.70 10.97	12.02 10.39	12.23 10.57	23,41 728,04
Required (cms) (HCH)	6.3	0	3 0	3	<u> </u>	<b>a</b> 0	0.6	1 0	6 0	6.	1 a o	03	0.3 0.26	0.3 0.26	0.3 0.26	0.3 0.26	0.3 0.26	0.3	0.3 0.25	0,3	0.3 0.26	0.3 0.26	0.3	0.3 0.26	0.3 0.26	0,3 0,26	0,3 0,26	0.3 0.26	0.3 0.26	0.6 0.52	0.6 0.52	0.6 0.52	0.6 0.52	0.8 0.52	0,6 0.52	0.6 0.52	0,6 0.52	0.6 0.52	13.26
Availabi (NCH)	e Wate 15.09	r of 1 15.	Runoff D 19 16.1	ischa 5   17	rge f ,43	ron the 17.54	e Resid 12.98	jual 8   16.	asin 1 9 19.	for Ir 12	rigati 9.71	ion (HCM) 10,49	) 14,58	17.92	27.69	41.57	41.7	35.9	32.66	34.78	25.22	19.85	25.76	25.88	20.96	18.25	18.07	14.61	12.99	13.74	15.55	20,74	26.53	16.95	11.91	10.45	9.87	10.05	714.78
Hater Re	eniup 0			p Wat		quirene O	ent+A O		ation] 0	0	0	. 0	0	47.3	81.1	78.1	94.1	94.4	91.9	92.2	90.2	89.7	95.9	69.3	52.8	34.4	17,4	5.3	0	0	0	0	0	0	0	0	0	0	1,034.1
Hean Net	0		0	0	0	0	0		0	0	-0	0	0	42.4	73.2	70.2	86.2	91.7	89.2	89.5	84.5	84	90.2	50.7	34.2	15.8	10.7	0	0	0	0	0	0	0	. 0	0	0	0	912.5
0iversio Irrigali	0		0	0	0	0	0	(ba)	0	0. Anne	0 Nivin	0	O	60.6	104.6	100.3	123.1	131	127.4	127.9	120.7	120	128.9	72.4	48.9	22.6	15.3	0	. 0	· 0	0	0	0	0	0	o	0	0	1,303.7
60,056 Diversio	· .							1.								. *														1									
		•		<u>0</u>	0	0	0		0	0	0	0	0	36.39	62.82	60.24	73.93	78.67	76.51	76.81	72.49	72.07	77.41	43.88	29.37	13.57	9.19	0	0	0	0	0	0	0	0	0	0	0	782.95
Accumt.	on by O		0 0 0	0 0 0	) 0 0	0 0	0		0	0 0	0	0															9.19 372.65 3		0 372.65	0 372.65	0 372.85	0 372.65	0 372.65	0 372.65	0 372.65	0 372.65	0 372.65		372.65
Deficier Accuml,	су (НС 0 0	M) [D		1ab1e 0   0	Wate O O	(1 0 0	0		0	0	0	0													8.41 410.3		0	0 410,3	0 410,3	0 410.3	0 410.3	0 410.3	0 410.3	0 410.3	0 410.3	0 410.3	0 410.3		410.3
Excess (		15.	19 15.1	5 17	. 43	17.54	12.98	16.	9 19.	12	9.71	10.49	14.58	0	0	. 0	Û	0	0	0	0	0	0	· 0	0	4.68	8.88	14.61	12.99	13.74	15.55	20.74	26.53	16:95	11.91	10.45	9.87	10.05	342.1
Allovabi Allovat Allovat	le Sho	rtage	Rate to						; Yield	1 (10%	Yield	i Reduct	ion)	10 4.06	10 6.96	10 6.70	10 8.07	10 8.10						30 17.84	30 13.59		30 4.48												111.50
Water Sa Accumi.	ving A 0 0		securin O O	9 90 <b>x</b> 0 0	Yiel O O	d (NCH) 0 0	) 0 0		0	0 0	0 0	0 0													8.41 91.39			0 91,39		0 91.39	0 91.39		0 91.39	0 91.39		0 91.39	0 91.39		91.39
Necessai	y Irri 0 0	gatio	n Water O	to be o	SUPP O	Lied fi Q	ron Lai O	r Dam 	and At O	obanda 0	ns (MC O	CH) (Def O						34.67	35.97	36.12	39.53	48.37	47.54	0	0	. 0	0	0	0	0	. 0	0	0	0	0	0	0	0	318.91
ACCURI.	C	I	0	0	0	0	0		0	0	0	0	0	14.41	42.58	54,55	78.71	113.38	149.35	183.47	223	271.37	318.91	318.91	318.91	318.91	318.91	318.91	318.91	318.91	318.91	318.91	3,18.91	318.91	318.91	318.91	318.91	318.91	
Availabl Abbanda <u>Lar Dan</u> Tota	ns .	2	rages in 50 40 90	Абба	ndans	and La	ar Dam	(нси)	I																													1	
Operatio Outflow Inflow Storage		) 	0 0	0 0 0	0 0 50	0 0 50	0 0 50	1	0 0 10	0 0 50	0 0 50	0 0 50	0	14.41 0 35.59	28.17 0 7.42	7.42 0 0	0 0 0	0 0	0 0 0	0 0	0 0 0	0 0 0	0 0 0	0 0 0	0 0 0	0 4.68 4.68		0 14.61 28.17		0 8.84 50	0 0 50	-	· ·	0 . 0 50	0 0 50	0050	0 0 50	0 0 50	50.00
Shorlage Accumi.	after G	Rele	ase from O O	Abba O O	ndans O O	(NCK) 0 0	0		0 0	0 0	0 0	0 0	0	0 0	0	4.55 4.55	24.16 28.71	34.67 63.38	35.97 99.35	34.12 133.47	39.53 173	48.37 221.37	47.54 268.91	0 268.91	0 268.91	0 268.91	0 268.91	0 268.91	0 268.91	0 268.91	0 268.91	0 268.91	0 268.91	0 268.91					268.91
Operatio Outfloy Storage	1 (	ł	0 .	0	0 240	0 240	0 240		0 10 :	0 240	0 240	0 240	0 240	0 240						34.12 106.53		48.37 18.63	18.63 0	0	0 0	0 0	0	0	0 0	0	0	0	0	0	0	0	0 0		240.00
Final St Accuml.	ortage C	(HCH	) 0 0	0 0	0	0	0		0 0	0	0	0	0 0	0	0	0	0	0	0	0	0	0	28.91	0 28.91	0 28.91	0 28,91	0 28.91	0 28.91	0 28.91	0 28.91	0	0 28.91	0	28.91	0 28.91	0 28.91	0 28.91		28.91

# TABLE B. 1. 4-8 PRELIMINARY WATER BALANCE STUDY (PROPOSED) FOR DROUGHT

# YEAR IN CASE OF LAR DAM ONLY

Year: Drought Year (1970)

				age of			ir Trt	igati	on:		240 1	ich													•														
		Jan				eb		<u> </u>		16			19A		[	May			ู่ ปมก			Jul			AUG			Sep			001		<u> </u>	Nov.		(	Dec		Total
	15.06	14.58	13.50	12.7	5 13	3.27 1	2.81	13.0	0 14.																									13.73					
Required (čæs) (HCK)	0.3	0.3	0.3	0.	6	0.6	0.6	0.	6 (	5.6	Q.6	0.3								0.3 0.26				0.3 0.26							0.6 0.52			0.6 0.52				0.6 0.52	
Available (MCM)														17.3	19.07	23.96	18.89	15.42	12.24	10.83	9.99	9.23	10.28	8.92	8,27	8,16	8.76	10.57	11.79	11.33	11.85	12	11.45	11.34	10.8	10.96	10.35	10.96	447.34
Water Red Mean Ket	0 Irriga	0 tion R	) Asriups	ent (mi	0	nemenil Q	t + ₽ ; 0		ation] 0 0	0	0	0			}						90.2		•				17.4	5.3	0	0	0	0	. 0	0	0	0	0		1,034.1
Diversion	. 0	Requi O	rement C		0	0	0	ha)	0 [see	0 0 9 Appe	0 0 ndix 6	0	0							877 1253	83			52.8 75.4			0	0	0	0	0	0	0	ບ 0	0	0	0 · 0		906.4 1,295.0
60,035 Diversio	n Requis O	rement O		face N	ater 0	(HCH) 0	0		0	0	0	. 0	0	34.89	65.64	63.06	76.81	77.11	75.01	75.25	71.23	70.81	76,09	45.28	31,17	15.37	0	0	0	0	0	0	0	, O	0	0	0	o	111.72
Lrrigatio Accumf.	on by Re O O	esidua O O	E FLOV - C C	(HCM)	0	0 0	0 0		0	0	.0 • • 0	0 0									9.99 127,7								0 172.56	0 172.56	0 172.56	0 172.56	0	0	0 172.56	0 172.56	0 - 172.56		172.56
Deficien	0	) (D.R . 0	Avail Q	able W	ater) 0	0	0		0 ·	0.	0	.0									61.24							0	-	. 0	0	0	0	, 0	0	0	0		605.2
ACCURI Excess (1	ICN)	12.34	11.4	10.	5 10	0 9.95 1	U 0.55	10.7	1 12.	. 15	U 14.6	U 19.49	5 N	0	ļ	103.26	0	0	285.64	350.05 0	411.3	<b>4</b> 72.88 0	538.69 0	575.05 0	0 241.92 (	05.16				Į			1	605.16 11.34		l			274.8
Aliovable Aliovabl Aliovabl	lé Shor	lage R	ate to						Yield	i (10%	Yield	Reduct	ion)	10 4.06	10 6.96	10 6.70	10 8.07	10 8.10			10 7.74			30 . 17. 84	30 13,59	30 8,85	30 4.48	30 1.36										j	111.50
Water Sav Accumi.	/ing Ami O O	ounts O O	ecuring C C	90* Y	ield O O	(HCH) 0 0	0 0		0	0 0	0 0	0 0									7.74 57.42							0			0 104.02		-	) 0 104.02	0 104.02	0 104.02	0 104.02		104.02
Necessary Accumi	v Trriga O O	ation V O	Water 1 f	o be s	uppli O O	ed fro 0 0	n lar O O	Dam i	and At O O	obanda: D O	ns (HC 0 0	អ) (Def ប ប	0	13.53	39.61	32.4	43.85				53.5 353.88							0 501.14	0 501,14	0 501.14	0 501.14	0 501.14	501.14	) Ö 1. 501, 14	0 501.14	0 501.14	0 501.14		501.14
hväilabin Abbandan Lar Dam Jota	ns	Stora 50 240 290		Abband 	ans a	ind Lar	Dan	(NCH)		·																								·	-				
Dperation Outflow Inflow Storage	0 0	bandan O SO	(	5	0 0 0	0 0 50	0 0 50	5	0 0 0	0 0 50	0 0 50	0 0 50	0	13.53 0 36.47	36.47 0 0	0 0 0	0 0 0	0 0 0	0 0 0	0 0 0	0	0 0 0	0 0 0	0- 0 0	0 0 0	0 0 0		0 10.57 19.33		0 11.33 42.45		0 0 50	· ·	) 0 ) 50	0 0 50	0 0 50	0 0 50	0 0 50	50.00
Shortage Accumi.	after 0 0	Releas O O	e froma ( ,	Abband	апз ( 0 0	(MCH) 0 0	0 0		0 0	0 0	0 0	0 0	0 0	0 0	3.14 3.14	32.4 35.54	49.85 85.39	53.59 138.98	54.89 193.87	56,51 250.38	53,5 303.88	57.73 361.61	61.7 423,31	18.52 441.63	9.31 451.14	0 451.14	0 451,14	0 451.14	0 451.14	0 451.14	0 451,14	Ú 451.14	0 451.14	0 0 4 451,14				0 451.14	451.14
Operatio Outflow Storage	0	0	240	24	0	0 240	0 240	24	0	0 240	0 240	0 240	0 240	0 240	3.14 236.86	32.4 204.46	49.85 154.61	53.59 101.02	54.89 46.13	46.13 0	0	0 0	0 0	0 0	0 0	0 0	0	0 0	0	0	0 0	. 0	0	) O	0 0	0	0 0	0 0	240.00
Final Shi Accuml	ortage 0 0	(нсн) 0 0	 		0	0	0 0		0	0 0	0 0	0	0	0	0	0	0	0	0	10.38	53.5 63.88	57.73	61.7 183.31	18.52	9.31 211.14	0 211.14		0	0 211.14	0	0 211.14	0 211.14	0	0 0 <u>1 211.14</u>	0 211.14	0	0 211.14		211.14

TABLE B. 1.4-9 10-DAY RUNOFF	DISCHARGE OF	<b>RESIDUAL BASIN</b>	(CMS)
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}T		Jan		· .	Feb			Mar			ADT			May			JUN			Jul		<u> </u>	Aug			Sep			0 ¢ l			Nov		i	Dec	
1956	9.57	8.94	11.81	12.41	13.61	14.96	. 15.44	14.74	2.22	6.95	46.85	19.11	29.91	69.51	92.05	27.4	9.1	28.41	24.41	19.71	101.9	63.43	61.23	67.16	80.6	65.1	31.88	13.38	10.68	10.06	10.06	10.06	10.56	11.56	10.76	4.84
1957	4.57	4.57	6.33	6.24	6.24	7.35	8.1	8.1	. 0	1	16.7	0	5.7	33	30.64	79.14	68.64	122.45	65.25	70.95	79.74	66.66	57.16	50.44	41.44	34.44	24.4	22.1	28.1	23.83	24.32	30.82	24.17	18.87	16.07	16.62
1958	16,62	16.33	12.77	.11.9	11.45	13.85	14.59	15.64	17.83	29.6	28.3	43.9	34.6	34	34.38	29.6	28.1	26	24.4	22.27	18.38	19.59	17.5	14.61	14.95	13.59	13.9	14.28	15.8	13.47	14.78	13.4	12.79	14.68	13.18	7.66
1939		4.61		12.64	11.18	9.53	12.59	11.21	21 03	19,96	13 3	65.9	72.1	70.6	70.27	65.8	52.8	46.4	35.7	28.8	26 73	25.09	22.49	20.33	20.91	15.79	11.53	18.49	17.25	17.61	18.71	19.43	19.6	17.39	17.63	18.11
1960		•			· · ·		17.92	· .						41,9													17.91					17.03	,		13.64	
1961			1	1.	· .		15.29			1				32.8		}						l					15.36					11.61		}	10.92	1
1962				1. T						.*							•																	1		
1963	10.05		10.5		12,47						13.97			36.3		}						13.28		1			12.66		13.6	. 1		13.52		}	10.29	9.11
1964	-		9.9				12.08			l.				29.9	30			28.9													ł	17.3		1		Q
1965	0	8.06	0	14.26	13.89	13.93	10.46	12.15	9.52	26.9	30.8	24.1	46.8	68.8	36.54	}-						1					10.54					11.4	11.01	9.7	9.7	8.97
1966	9.3	10.48	9.62	9.62	10.5	9.59	12.9	18.2	19.33	21.12	27.95	42	53.1	75,5	65.18	54.2	42.3	36.2	32 8	24.11	22.3	20.71	14,11	14,08	15.12	12.28	12.54	17.33	13.57	14.62	15.86	16.4	15.46	14	12.19	11.13
1967	11.63	11.01	10.83	14.64	10.26	7.74	8.45	18,96	16.5	20.26	34.8	42.7	74.5	44.4	51.73.	45.4	39.8	31.6	25.2	21.7	22.85	18.76	14.07	12.12	12.51	12.14	11.63	14.51	13.69	11.76	11.88	12.2	11.07	9,92	9.31	10.45
1968	10.75	9.72	9.95	9.75	10.22	10.94	11.81	11.67	12.44	13.28	17.81	30.4	35	50.3	44.46	33	28.7	23.4	20.37	17.33	14.27	13.33	12.92	10.85	10.09	10.74	11.81	13.21	11.54	10.63	10.87	11.41	11.26	10.6	8.98	8.22
1969	8.09	7.85	6.39	· 0	8.48	11.76	12.48	22.52	18.38	31.97	49.87	51.2	44.8	48	60.63	80.1	55.9	47.4	43.6	36.9	28.82	22.9	20.41	16.81	15.1	13.31	10.99	11.78	11,99	13	11.77	14.22	13.86	12.8	14.23	13.4
1970	8.17	7.56	12.62	10.19	9.65	13.88	24.55	45.4	37.96	72.7	39.1	41.9	84.2	98.8	70.09	79.1	53.4	53.8	65.8	59.1	44.73	35.2	25.5	22.1	19.36	25.04	21.3	19,23	21.57	24.4	23.88	21.95	19.41	18.5	16.95	15.52
1971	15.06	14.58	13.5	12.75	13.27	12.81	13	14.67	17.5	22.86	20.18	20.32	22.37	28.03	22.16	18.15	14.47	12.83	11,86	10.98	12.2	10.62	9.87	9.74	10.44	12.53	13.95	13.72	14,32	14,49	13.85	13.73	13.1	13.29	12.58	13.29
	13.74	12.67	12.63	13.1	11.77	11.6	15.41	22.73	23.65	31.04	29.36	38.76	44.94	43.64	33.37	23.4	34.6	25.9	22.13	19.8	20.48	20.49	19.16	19.42	18.63	18.14	14.35	13.79	16.28	16	15,99	16.44	14.74	13.79	14.58	13.08
1972	7.44	0.08	0	5:52	11.06	9.5	14.96	16.92	19.13	25.39	34.12	30.79	59.37	47.9	72.35	78.78	67.36	50.69	46.69	43.9	38.89	48.91	44.86	43.6	39.28	37.95	17.41	13.05	12.6	13.42	15.77	15.06	14,05	17,09	17.72	16.1
	12.41	15.14	15.05	15.35	17.14	19.58	19.45	18.12	21.77	20.91	36.51	79.98	93:99	105.68	150.46	85.29	58.92	41.2	20,77	23.53	25:36	24.56	20.6	14.98	15.25	14.3	14.4	16.07	14.75	14.13	13.08	14.04	14.06	11.68	10.65	10.64
1974	8.09	7.85	10.22	10.43	10.68	15.17	15.78	21.63	25.94	19.19	28.58	31.54	12.12	27.05	37.52	20.6	15.82	12.89	29.14	29.44	17.64	11.78	7.79	10.83	9.94	12.23	10,56	7.07	7.07	8.06	8.35	9.71	9,21	7.86	6.91	6.61
1975	6.72	5.28	6.28	8.36	9.11	12.69	12,24	11.1	1.06	11.66	33.89	19.07	31, 15	118.08	120,35	96.81	71,89	38.14	16.75	8.09	11.81	10.27	10.73	11.45	9.23	8.06	8.82	10.94	10.24	14,19	16.98	16.03	16.59	16.93	16.98	17.62
1976	14.42	13.64	13.05	10.9	12.61	12.8	14,48	13,66	13.68	16.08	33.92	36.21	33.2	28.5	53.4	47.12	48.57	40.14	40.37	29.85	27.01	22.43	18.8	21.4	21.54	22.65	23.08	21.97	20.07	22.82	22.47	21.15	21.97	23.94	26.67	19.02
1977	17.77	17.88	18.99	20.78	20.9	15.62	20,16	22.73	11.84	12.44	17.18	21.04	32.35	48.41	48,56	41.85	38.1	40.56	29.49	23.27	30.11	30.25	24.56	21.42	21.22	17.21	15.33	16.51	18.6	24.61	31.31	20.22	14.39	12.7	12.02	12.23
1978	11.59		7.5		13.69		ł									20.83																			27.94	28.8
1979																21.42										1 - C	18.31								14.41	
1980															-							1.													:	
1981		14.65																				· ·													16.91	
1982				•																															15.54	
اا	11.16	11.16	12.01	12.01	12.01	12.67	12.67	12.67	12.96	12.96	12.96	24.14	24.14	24.14	15 52	15.52	15.52	19.95	19.95	19.95	16.21	16.21	16.21	16.19	16.19	16.19	16.58	16.58	16.58	16.12	16.12	16.12	15.9	15.9	15.9	12.39

TABLE B. 1. 4 - 10 10-DAY RUNOFF DISCHARGE AT KAREHSANG (CMS)

Year 1		Jan			feb	<u> </u>	1	Mar			APT		1	May	· · · · · · · · · · · · · · · · · · ·	<u></u>	ากบ		· · · · · ·	JUI			AUg .		<u> </u>	Sep	<u> </u>		οςι		· · · · · · · · · · · · · · · · · · ·	NOV			0ec	1
1956	18 63	18.00	18 00	18 60		20 22	20.70		20 27	25 00	<del></del>	76 80	87 60	127.20	150.45	85 80	·····	55 00	51 00	46.30	115 27	76.80		76.36	69,80	74.30	38.50	20,10	17.40	16.00	16.00	16.00	15.80	16.80	16.00	13 27
1957															1.1		÷.,								1.1											
1958		13.00											]					1.1																		
1959	23.20	21.00	18.45	18.00	18,10	18.25	19.60	21.70	27.82	47.80	71.90	83.90	68.20	63.20	58.00	52.50	48.00	42.20	37.80	31.80	26.00	26.30	23.60	20.00	19.50	18.00	18.00	18.10	19.60	17.09	18.20	16.60	16.70	18.20	16.80	14.64
	15.00	16.50	16.18	14.90	13.50	14.38	14.70	15.50	26.82	40.10	52.60	112.60	110.30	106.60	110.09	98.20	86.30	71.40	53.10	41.80	38.00	34.30	29.90	26.91	27.10	21.70	16.90	23.50	21.90	21.91	22.50	23.00	23.00	21.00	21.70	22.45
1960	22.20	21.70	21.82	20.80	20.80	21.33	22.30	22.60	24.45	42.60	33.30	51.20	53.70	71.00	57.09	45.80	36.40	31.50	29.00	27,50	28.45	22.90	23.30	24.73	25.70	22.70	22.10	22.00	22.00	21.73	21.00	20.50	19.00	19,20	18.30	18.91
1961	18.40	19.00	18.91	18.20	18.00	18.00	17.70	17.00	17.91	21.50	27.70	43.90	68,90	60.00	55.55	43.60	32.10	21.60	29.20	25.80	22.82	23.70	21.70	19.73	21.90	21.70	19.80	15.80	14.80	18.00	15.00	15.30	15.00	14.60	14.60	13.73
1962		14.00										· .															1			· ·						
1963															÷.,																					
1964	13.00	12.60	13.18	12.70	12.40	12.50	14,70	14.50	17.73	25.40	41.70	56.50	58.40	70.00	13.36	106.10	86.30	58.70	.44.30	39.00	29.73	32.90	23.90	29,18	20.00	17.50	19.20	18.80	20.00	16.17	17.90	20.90	19.20	17.30	16.20	15.91
1965	13.60	16.10	13.36	17.50	16.20	17.00	14.50	17.10	23.18	47.90	49.50	52.60	95.70	114.90	69.36	58.50	46.20	34.70	31.10	26.10	23.55	21.50	20.00	21.18	19.60	20.10	14.90	14.90	14.60	15.09	15.40	14.50	14.30	13.00	13.00	12.00
1966	12.20	13.20	12.82	12.50	13.30	12.50	15.70	20.40	25.55	28.60	40.70	78.30	104.70	130.00	108.18	86.20	65.20	53.40	45.50	34.00	30.55	27.90	20.40	19.82	20.30	17.00	17.00	21.90	17.80	18.82	20.40	20.70	19.50	17.60	15.90	15.09
	15.00	14.80	14.09	18.20	13.60	10.88	12.00	25.60	21,27	27.80	56.50	70.90	113.70	81.20	93.00	78.80	63.50	49.50	38.70	33.10	32.73	26.30	20.50	17.73	17.50	16.50	15.90	19.40	18.70	16.91	16.50	16.60	15.10	14.00	13.20	14.09
1967	14.00	13.00	13.00	12.80	13.20	13.75	14.70	14:60	15.91	17.00	24.80	46.20	61.50	100.10	81.73	58.10	45.60	36.70	30.90	25.90	21.36	19.80	18.50	15.73	14.60	15.00	16.00	17.40	15.30	14.00	14.20	14.70	14.00	13.60	13.10	11.27
1968	11.00	11.40	11.36	10.80	11.90	14.00	15.10	26.60	21.55	37.40	57.00	92.70	94.90	98.90	146.27	151.80	105.50	83.20	71.20	53,90	41.55	33.90	30.30	26.36	23 90	21.30	17.90	18.00	17.60	17.91	17.90	20.30	18.70	17.20	18.30	17.73
1969		16.00															1	2																		
1970				i									1.1			1 · · · ·																				
1971		19.75												· · · ·	÷				ļ																	
1972	16.38	15.33	15.27	15.42	15.30	14.44	19.08	26.78	30.02	38.70	48.21	75.10	94.31	113.00	94.40	66.74	66.49	47.22	36.50	32,03	29.61	28.60	26.00	25.38	24.06	23.17	19.27	18.49	20.60	20.24	20.00	20.43	18.41	17.14	18.04	17.35
1973	16.72	16.66	15,98	16.24	17.09	16.87	17.68	20.22	22.35	30.04	47.37	79.7 <b>9</b>	118.92	94.60	133.09	141.20	119.99	96.86	80.55	66.97	55.16	64.99	55.47	52.65	46.67	44.53	24.76	20.29	19.46	19.35	21.38	19.48	18.40	21.60	21.84	19.80
	18.02	18.67	18.71	18.94	21.02	23.90	24.66	24.07	28.83	35.52	65.02	123.32	143.96	164110	205.00	128.49	88.09	62.47	36.69	35.35	34.65	32.39	27.71	21.22	20.87	19.61	19.44	20.83	19.36	18.45	17.06	17.96	18.00	15.42	14,77	14.'07
1974	11.58	11.34	13.37	13.58	13.83	18.34	18.95	24.80	36.94	30.19	39.58	63.02	43.60	58.53	62.97	46.05	41.27	28.09	44.34	44.64	26.52	20.66	16.67	17.77	16.88	19.17	16.13	12.64	12.64	11.91	12.20	13.56	12.30	10.95	10.00	9.39
1975	9.50	8.06	8.47	10.55	11.30	14.84	14.39	13.25	19.85	30.45	52.68	57.05	69.13	156.06	158.64	135.10	110.18	51.56	30.17	21.51	18.70	17.16	17.62	16.44	14.22	13.05	12.72	14.84	14.14	17.74	20.53	19.58	19.35	19.69	19.74	20.44
1976	17.39	16.33	15 29	13 36	14 98	15 25	16 34	15 92	16, 17	19 30	45 AG	69 73	89 11	97 B.L	115 75	99 34	92 19	71 17	65 11	47 46	12 38	32.60	27 19	28.84	27.98	28 59	27.87	26.76	24.86	27.19	25.84	25.52	25.40	27.37	30.10	21.95
1977																															1					
1978		20.81		· ·			1									1						ł														
1979	15.22	13.61	10.31	9.70	17.33	15.00	12.43	27.21	33.17	56.57	60.22	54.13	56.66	65.67	62.19	51.67	55.81	47.71	36.59	33.35	25.37	19.96	14.17	13.81	12.93	13,19	21.14	28.41	30.17	30.35	30.52	30.73	30.93	31.13	31.33	31.58
1980	31.79	31.99	32.20	32.30	32.37	32.40	32.40	32.40	32.50	32.60	41:58	52.80	57.10	64.09	62.66	64.14	70.75	53.10	28.58	26.82	26.04	25.18	23.87	22.94	22.86	22.80	23.03	22.44	21.72	21.65	21.30	20.46	18.12	16.72	17.44	17.67
1981	17,92	17.44	16.51	17.04	17.36	17.64	17.84	17.32	19.27	\$3.82	60,87	61.07	53.85	44.27	46.14	46.14	45.14	31.20	31.20	31.20	31.20	31.20	31.20	21.70	21.70	21.70	23.77	23.77	23.77	20.60	20.60	20.60	21.04	21.04	21.04	20.85
	20.85	20.85	21.50	21.50	21.50	21.70	21.70	21.70	25.04	25.04	25.04	74.48	74.48	74.48	63.48	63.48	63.48	53.27	53.27	53.27	30.73	30.73	30.73	21.83	21.83	21.83	23.13	23.13	23.13	31.32	31,32	31.32	18.56	18.56	18.56	16.02
1982	16.02	16.02	15.33	15.33	15.33	15.49	15.49	15.49	22.65	22.65	22.65	55.12	55.12	55.12	46.88	46.88	46.88	34.23	34.23	34.23	23.39	23.39	23.39	21.13	21.13	21.13	22.47	22.47	22.47	21.33	21.33	21.33	20.49	20.49	20.49	19.78
h													4						1			1			<u> </u>						1			• • • • • • • • •		ليتنششه

TABLE B. 1. 4 - 11 10-DAY RUNOFF DISCHARGE AT POLOUR (CMS)

Year T		Jan			feb	······		Mar	1	Γ	Apr		1	May		r	Jun			Jul			Aug	j		Sep	i		0ct			Nov	······································		Dec	}
1956	9.06		6.19	6,19	6.19	5.26	5.26	5.26	18.05	18.05		57.69	57,69		58.40	58.40		26.59	26.59		13.37	13.37		9.20	9.20		6.72	6.72		5.94	5.94	5.94	5 24	5.24	5,24	A 43
1957		8.43			5.76					16.80			1.11									1.1				8.56				5.08		5.08				
1958.										18.20				· · ·		1.								1		4.41									5.13	
1959		4,67				4.40				]			).	1.1		)	1.1												3.80			3.20	1		3.62	
1960		11.89			2.32			•							:	l ·																3.57				
1961		8,08	÷ .		2.65				3.68				19.60									5.67				4.39	ļ		4,10			3.47	ļ		4.66	
1962					2.29				3.05	·			28.30		:		1.1									5.03			4,30			3.69			3.68	
1963		4.06			3.53								33.30			]								· · ·					5.20			4.08			3.11	1
1964		4.57		]	2.59					11.16		1	1.1													6.56			4.84			3.60			624	18,49
1965	46.30	8.04	26.17	3.34	2.31	3.07	4.04	4.95	13.66	21.00																5.30			3.78	3.65	3.35	3.10	3.29	3.30	3.30	3.03
1966	2.90	2.72	3.00	2.88	2.80	2.91	2.80	4.20	6.22	7.48	12.75	36.30	51.60	54.50	43.00	32.00	22.90	17.20	12.70	9.89	8.25	7.19	6.29	5.74	5.18	4.72	4.46	4.57	4.23	8.20	4.54	4.30	4.04	3.60	3.71	3.95
1967	3.37	3.79	3.26	3.56	3.34	3.14	3.55	6.64	4.77	7.54	21.70	28.20	39.20	36.80	41.27	33.40	23.70	17.90	13.50	11.40	9.88	7.54	6.43	5.81	4,99	4,36	4.27	4.89	5.01	5.15	4.62	4.40	4.03	4.08	3.89	3.64
1968	3.25	3.28	3.05	3.05	2.98	2.81	2.89	2.93	3.47	3.72	6,99	15.80	28.50	49.80	37.27	25.10	16.90	13.30	10.53	8.57	7.09	6.47	5.58	4.88	4.51	4.26	4,19	4.19	3.75	3.37	3.33	3.29	2.74	3.00	4,12	3.05
1969	2.91	3.55	4.97	15.67	3.42	2.24	2.62	4.08	3.17	5.43	17,13	41.50	50.10	50.90	85.64	71.70	49.60	35.80	27.60	17.00	12.73	11.00	9.89	9.55	8.80	7.99	6.91	6.22	5.61	4.91	6.13	6.08	4.84	4.40	4.07	4.33
1970	8 23	8.44	4.56	6.91	6.65	4.12	6.55	13.60	11.86	41.90	28.70	53.10	98.90	107.00	96.55	67.10	69.30	46.40	33.30	28,00	24.27	17.30	13.30	11.45	10.34	8.96	7.62	6.79	7.33	10.87	8.26	7.65	7.02	6.52	5.65	5.59
1971	5.65	5,17	4.87	4.76	4.56	4.34	4.31	4.72	5.97	15.06	23.88	18.42	18.09	22.28	21:45	16.20	11.86	9.87	8.88	7.67	7.14	6.00	5.48	5.03	4,64	4.50	4.18	3.83	3.75	3.53	3.38	3.14	3.03	3.03	3.23	2.64
1972	2.64	2.66	2.64	2.32	3.53	2.84	2.67	4.05	6.37	7.66	18.85	36.34	49.37	69.36	61.03	43.34	31.89	21.32	14.37	12.23	9.13	8.11	6.84	5.96	5.43	5.03	4.92	4.70	4.32	4.24	4.01	3.99	3.67	3.35	3.46	4.27
1973	9.28	16.58	36.90	10.72	6.03	7.37	2.72	3.30	3.22	4.65	13.25	49.00	59.55	46.70	60,74	62.42	52.63	46,17	33.86	23.07	16.27	16.08	10.61	9.05	7.39	6.58	7.35	7.24	6.86	5.93	5.61	4.42	4.35	4.51	4,12	3.70
1974	5.61	3.53	3.66	3.59	3.88	4.32	5.21	5.95	7.06	14,61	28.51	43.34	49.97	58.42	54.54	40.20	29.17	21.27	15.92	11.82	9.29	7.83	7.11	6.24	5.62	5.31	5.04	4.76	4.61	4.32	3.98	3.92	3.94	3.74	4,12	3.43
1975	3.49	3.49	3.15	3.15	3.15	3.17	3.17	3.17	11.00	11.00	11.00	31.48	31.48	31.48	25.45	25.45	25.45	15.20	15.20	15.20	8.88	8.88	8.88	6.94	6 94	6.94	5.57	5.57	5.57	3.85	3.85	3.85	3.09	3.09	3.09	2.78
1976	2.78	2,78	2.19	2.19	2.19	2.15	2.15	2.15	18,79	18.79	18.79	37.98	37.98	37.98	38.29	38.29	38,29	13.42	13.42	13.42	6.89	6.89	6.89	4.99	4,99	4.99	3.90	3.90	3.90	3.55	3.55	3.55	2.78	2.76	2,76	2.82
1977	2.97	2.69	2.24	2,46	2.37	2.45	1.86	2.26	2.49	3.22	11.48	33.52	55.91	69.34	62.35	52.22	43.62	31.03	24.74	17.61	15,37	10.17	8.39	7.44	6.44	5.94	4.79	4,79	4.79	4.37	4.37	4.37	3.43	3.43	3,43	2.93
1978	2.93	2.93	2.88	2,88	2.88	3.93	3.93	3.93	9.88	.9.88	9.88	27.22	27.22	27.22	32.79	32.79	32.79	13.31	13.31	13.31	7.36	7.36	7.36	5.67	5.67	5.67	5.95	5.95	5.95	5.60	5.60	5.60	5.06	5.06	5.06	3.78
1979	3.63	3.77	2.81	3.39	3.64	3.86	3.78	4.74	7.05	29.26	45.67	37.58	34.06	47.38	38.26	30.84	30.04	25.51	18.90	13.11	10.84	9,96	8.44	7 34	5.93	5.44	4.39	4,39	4.39	4.23	\$.23	4.23	3.39	3.39	3.39	2.78
1980	2.78	2.78	2.95	2.95	2.95	2.87	2.87	2.87	21.98	21.98	21.98	46,85	46.85	46.85	42.72	42.72	42.72	21,82	21.82	21.82	10.31	10.31	10.31	6.63	6.63	6.63	4.72	4.72	4.72	4.16	4.16	4.18	3.03	3.03	3.03	3.00
1981	3.06	2.79	2.60	2.54	2.50	2.71	2.49	2.78	4.57	22.18	40.38	32.52	26.93	27.90	32.90	32.90	32.90	14.98	14.98	14.98	7.53	7.53	7.53	5.18	5.18	5.18	4, 93	4,93	4.93	4.36	4.36	4.36	4.13	4.13	4.13	6.18
	6.18	8.18	4.22	4.22	4.22	3.59	3.59	3.59	12.31	12.31	12.31	39.37	39.37	39.37	39.85	39.85	39.85	18.14	18.14	18.14	9,12	9.12	9.12	6.27	6.27	6.27	3.88	3.88	3.88	3.43	3.43	3.43	3.02	3.02	3.02	4.86
1982	4.86	4.86	3.32	3.32	3.32	2.82	2.82	2.82	9.69	9.69	9.69	30.98	30.98	30.98	31.36	31.36	31.36	14.28	14.28	14.28	7.18	7.18	7 18	4.94	4.94	4.94	5.89	5.89	5.89	5.21	5.21	5.21	4.59	4.59	4.59	7.39

# APPENDIX B. 2 IRRIGATION

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# B. 2.1 TIB Survey on Irrigation

# 1. General of Terminal Irrigation Block (TIB) Survey

The TIB survey has been carried out for clarifying the present condition on irrigation and water management in the Project Area. The survey was conducted both for the zone mirabs and the village mirabs using the questionnaires (see DATA BOOK II, B2. Irrigation (TIB Survey)). The Survey was carried out for the entire zone mirabs and the sampled village mirabs.

# Surveyed Mirabs

Zone Mirab 116 mirabs (conducted by entire survey) Village Mirab 67 mirabs (conducted by sampling survey)

Village mirabs were compiled district-wise, and the zone mirabs were compiled river-system-wise. Because the command areas of zone mirabs are following the existing canal system and its boundary are not correspond to the districts which are arranged in accordance with the proposed canal network.

The arrangements of the village mirabs and the zone mirabs are as

# follows;

### **Arrangement of Zone Mirab**

District	Zone Mirab					
District	Code	Number	Percent			
Alesh Rud	1 - 7	7	6.0%			
Haraz Left	8 - 29	22	19.0			
Haraz Right	30 - 44	15	12.9			
Kari Left	45 - 96	52	44.8			
Kari Right	97 - 110	14	12.1			
Garma Rud	111 - 115	5	4.3			
Kharan Rud	116	. 1	0.9			
Total		116	100.0%			

(Note) Location of zone mirab allocation is shown in Figure B. 2. 1-1.

#### Arrangement of Village Mirab

District	Zone Mirab					
DIStrict	Code		Number	Percent		
Haraz West	1 -	17	17	25.4%		
Amol West	18 -	31	14	20.9		
Haraz East	32 -	52	21	31.3		
Amol East	53 -	67	15	22.4		
Total		*****	67	100.0%		

(Note) Location of selected village mirabs is shown in Figure B. 2. 1-2.

# 2. Number of Mirabs in the Project Area

There are 624 mirabs (including a part of irrigation area of the Garma Rud and the Kharan Rud) in the Project Area. Out of 624 mirabs, 116 are zone mirabs and 508 are village mirabs.

Distribution of zone mirabs and village mirabs are as follows;

River System		Mirabs iber (%)	Village Mirabs Number (%)		Total Mirabs Number (%)	
Alesh Rud	7	( 6.0)	6	(1.2)	13	(2.1)
Haraz Left	<b>22</b>	(19.0)	135	(26.6)	157	(25.2)
Haraz Right	15	(12.9)	61	(12.0)	76	(12.2)
Kari Left	52	(44.8)	224	(44.1)	276	(44.2)
Kari Right	14	(12.1)	53	(10.4)	67	(10.7)
Sub-Total	110	(94.8)	479	(94.3)	589	(94.4)
Garma Rud	5	(4.3)	10	(2.0)	15	(2.4)
Kharan Rud	1	( 0.9)	19	( 3.7)	20	( 3.2)
Total	116	(100)	508	(100)	624	(100)

# Distribution of Mirabs

Note: 1. see details in Table B. 2. 1-1.

As seeing above table, Kari Left Area has the largest number of mirabs for irrigation.

# 3. Ages and Experiences of Mirabs

Average age of mirab is 49 years old for the zone mirabs and 47 years old for the village mirabs. Average experienced years are 11 years for the zone mirabs and 8 years for the village mirabs.

# 4. Selection Method of Mirabs

Mirabs are selected generally every year and maximumly once in every three years, but same experienced men are selected in many cases. The village mirab is selected by farmers themselves, but the zone mirab is selected generally by the irrigation office.