REPUBLIC OF KENYA



MINISTRY OF PUBLIC WORKS

DETAILED DESIGN STUDY ON

THE NAIROBI BYPASS PROJECT

STRUCTURAL CALCULATIONS
COMPUTOR OUTPUT

VOL-2

SEPTEMBER 1992

JAPAN INTERNATIONAL COOPERATION AGENCY

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Japan International Cooperation Agency The Permanent Secretary Ministry of Public Works P.O.Box 30260 NAIROBI

The Chief Engineer (Roads) Ministry of Public Works P.O.Box 30260 NAIROBI 国際協力事業団 24833

CALCULATION OF

OVER BRIDGE

PEDESTRIAN OVER BRIDGE(1) | - | --- | -70

PEDESTRIAN OVER BRIDGE(2) 2-1--2-79

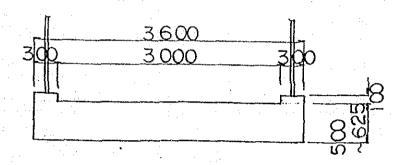
OVER BRIDGE (1) W = 6m 3 - 1 - -3 - 46

OVER BRIDGE(2) W=10m 4-1--4-87

PEDESTRIAN OVER BRIDGE (1)

No. (1) PEDESTRIAN BRIDGE OF MAIN

- 1) Shape and factors for calculation of stress
- (1) Superstructure

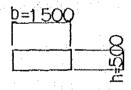


$$\begin{cases} A_1 = 3.60 \times 0.50 &= 1.800 \text{ m}^2 \\ I_1 = \frac{1}{12} \times 3.60 \times 0.50^3 &= 0.03750 \text{ m}^4 \end{cases}$$

$$\begin{cases} A_2 = 3.60 \times 0.625 &= 2.250 \text{ m}^2 \\ I_2 = \frac{1}{12} \times 3.60 \times 0.625^3 &= 0.07324 \text{ m}^4 \end{cases}$$

$$Ec_1 = 27 \text{ KN/mm}^2 = 2.7 \times 10^7 \text{ KN/m}^2 \text{ (} \therefore \text{ fcu} = 30 \text{ N/mm}^2 \text{)}$$

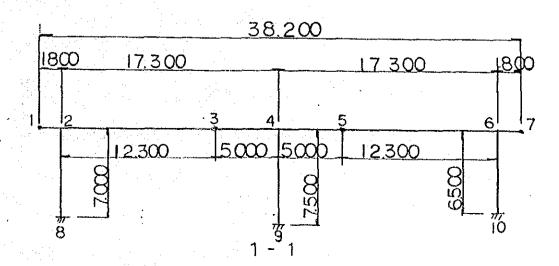
(2) Substructure



$$\begin{cases}
A = 1.50 \times 0.50 & = 0.750 \text{ m}^3 \\
I = \frac{1.50 \times 0.50^3}{12} & = 0.01562 \text{ m}^4
\end{cases}$$

$$Ec_2 = 25 \text{ KN/mm}^2 = 2.5 \times 10^7 \text{ KN/m}^2 \text{ (fcu} = 25 \text{ N/mm}^2\text{)}$$

(3) Frame



2) Load

(1) Dead load

Parapet = 23.6 (2 × 0.30 × 0.10) + 0.294 × 2 = 2.004 KN/m

 Slab (1) = 23.6 × 1.800 = 42.480 KN/m

$$\omega d_1$$
 = = 44.484 KN/m

 Parapet = = 2.004 KN/m

 Slab (2) = 23.6 × 2.250 = 53.100 KN/m

 ωd_2 = 55.104 KN/m

pillar $\omega d_3 = 23.6 \times 0.750$

= 17.700 KN/m

(2) Dead load form staircase $\omega_R = 98.116/3.60$

= 27.255 KN/m

- (3) Live load : width = 3.0^{m}
 - a) Intensity of load

small than 2.0^{m} width $:\omega \,\ell$, $= 5.0 \,\text{KN/m}^2 \times 2.0 \,\times 1.00 = 10.000 \,\text{KN/m}^2$

large than 2.0° width : $\omega \mathcal{L}_2 = 5.0 \text{ KN/m}^2 \times 1.0 \times 0.85 = 4.250 \text{ KN/m}^2$

$$\omega \ell_0 = = 14.250 \text{ KN/m}^2$$

b) For first span

loaded length ...
$$\ell = 17.30^{\text{m}} < 30.0^{\text{m}}$$
 ... $K = 1.0$
... $\omega \ell = \omega \ell_0 \cdot K = 14.250 \times 1.00 = 14.250 \text{ KN/m}$

c) For middle fulcrum

loaded length \cdots $\ell = 2 \times 17.30 = 34.60$ m

reduced factor : K

$$K = 151 \left(\frac{1}{\ell}\right)^{0.475} / 30.0 = 151 \left(\frac{1}{34.60}\right)^{0.475} / 30.0 = 0.935$$

$$\omega \ell = \omega \ell_0 \cdot K = 14.250 \times 0.935 = 13.324 \text{ KN/m}$$

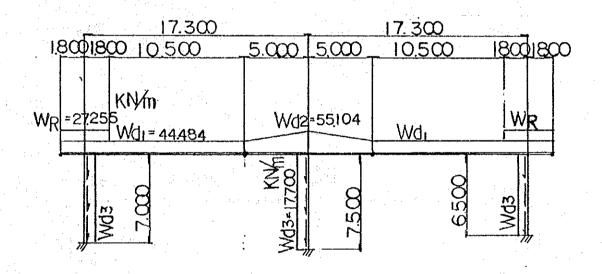
- (4) Others load
 - a) Temperature

Point, ② to ⑥
$$T=+12.5$$
 °C Coefficient of thermal expansion : $\alpha=12\times10^{-6}$ /°C

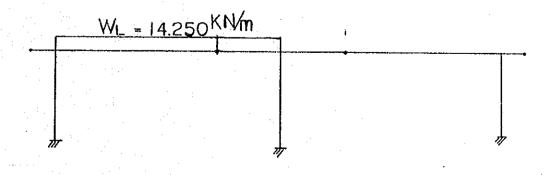
b) Seismic

Coefficient of seismic $k_H = 0.10$

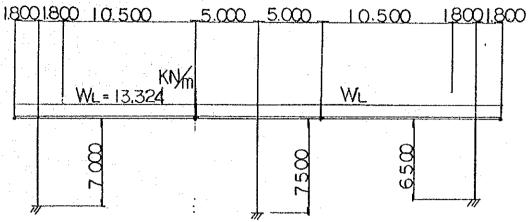
- 13) Loaded figure
- (1) Dead load ... case-1



- (2) Live load
 - a) for first span ··· case-2



b) for middle fulcrum ··· case-3

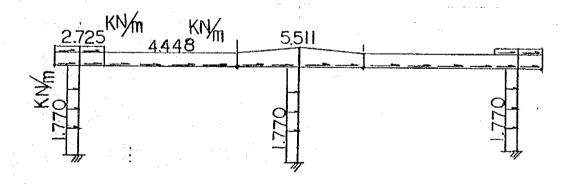


- (3) Others load
 - a) Temperature ... case-4

$$T = +12.5^{\circ}\mathbb{C}$$

$$\alpha = 12 \times 10^{-6} / ^{\circ}\mathbb{C}$$

b) Seismic $(k_H = 0.10)$ ··· case-5



PEDE BY NO-1 SUPERSTRUCTURE (MAIN BRIDGE)

NOTE: THE DIMENSION(1) BE EXCHANG TO DIMENSION(KN) INTO THIS CALCULATION

(面)	(m)
0.0000	7.5000
1.8000	7.5000
14.1000	7.5000
19.1000	7.5000
24.1000	7.5000
36.4000	7.5000
38.2000	7.5000
1.8000	0.5000
19.1000	0.0000
36.4000	1.0000
	0.0000 1.8000 14.1000 19.1000 24.1000 36.4000 38.2000 1.8000 19.1000

	1.4					•	:	
No	I	J	A (m2)	I (m4)	i - J	L (m)	E (t/m2)	EFS
1	1 -	2	1.80000	0.037500	Fix - Fix	1.800	2.70E+07	1 20E-05
2	2 -	3	1.80000	0.037500	Fix - Fix	12.300	2.70E+07	1.20E-05
3	3 -	4	2.02500	0.055370	Fix - Fix	5.000	2.70E+07	1.202-05
4	4 -	5	2.02500	0.055370	Fix - Fix	5,000	2.70E+07	1.20E-05
5	5 -	6	1.80000	0.037500	Fix - Fix	12.300	2.70E+07	1.20E-05
6	6	7	1.80000	0.037500	Fix - Fix	1.800	2.70E+07	1.20E-05
7	2 -	8	0.75000	0.015620	Fix - Fix	7.000	2.50E+07	1.20E-05
8	4 -	9	0.75000	0.015620	Fix - Fix	7.500	2.50E+07	1.208-05
9	6	10	0.75000	0.015620	Fix - Fix	6.500	2.50E+07	1.20E-05

No	X (t/m)	Y (t/m)	M(tm/Rad)
8	Fix	Fix	Fix
9 10	Fix Fix	Fix Fix	Fix Fix

	100											
		L-:	io 1	L-No	L-No 3	L-No	L-No	L-No	L-No	L-No	L-No	L-No 10
:	• •	1	1	12	13	14	15	16	17	18	19	20
1	0				•						* .	
2	. 9	1.23	30 2	460	3.690	4.920	6.150	7.380	8.610	9.840	11.070	
3		1.00			3.000	4.000		٠.				
5	4	$\frac{1.00}{1.23}$.000	3.000 3.690	4.000	6.150	7.380	8.610	0 040	11 050	
6	. 0				1	41320	0.100	7.500	0.010	9.840	11.070	
7	4	1.40	0 2	.800	4.200	5.600						
8	4	1.50		.000	4.500	6.000						
9	4	1.30	00 2	.600	3.900	5.200				•		

: DEAD LOAD No. : 1 No. : 1

No	1 -1		Li (m)	Lo (m)	Pi (t/m)	Pj (t/m)
. 1	1- 2	-γ	0,000	1.800	-27.255	-27.255
2	2~ 3	- Y	0.000	1.800	-27.255	~27,255
-1	1- 2	- Y	0.000	1.800	-44.484	-44.484
2	2- 3	∽ Υ	0.000	12.300	-44.484	-44.484
. 3	3- 4	- Y	0.000	5.000	-44.484	-55.104
4	4- 5	Y	0.000	5.000	-55.104	-41.184
5	5- 6	- Y	0.000	12.300	-44.484	-44,484
6	6- 7	- Y	0.000	1.800	-44.484	-44.484
5	5 - 6	- Y	10.500	1.800	-27.255	-27,255
6	6- 7	- Y	0.000	1.800	-27.255	-27.255
7	2 - 8	Y	0.000	7.000	-17.700	-17.700
8	4- 9	: -Y	0.000	7.500	-17.700	-17.700
9	6-10	- Y	0.000	6.500	-17.700	-17,700

 $\Sigma V = -2320.325 (t)$ $\Sigma II = 0.000 (t)$

: LIVE LOAD : 2 : 1

No. No.

Li (m) Lo (m) Pi (t/m) Pj (t/m)

2 2- 3
3 3- 4
0.000 12.300 -14.250 -14.250
0.000 5.000 -14.250 -14.250

 $\Sigma V = -246.525 (t)$ $\Sigma H = 0.000 (t)$

: LIVE LOAD
No. : 3
No. : 1

Li (m) Lo (m) Pi (t/m) Pj (t/m) 2-3-4-5-2 3 0.000 0.000 3 4 5 6 12.300 -13.324 -13.324 5.000 5.000 12.300 -13.324 -13.324 -13.324 -13.324 4 5 0.000 -13.324 0.000 -13.324

> $\Sigma V = -461.010 (t)$ $\Sigma II = 0.000 (t)$

: TEMPERATURE : 4 : 1

No TO No T (°C)

2 -- 5 12.50

No. No.

> $\Sigma V = 0.000 (t)$ $\Sigma II = 0.000 (t)$

: SEISMIC : 5 : 1

No	i j			Li (m)	Lo (m)	Pi (t/m)	Pj (t/m)
1	1- 2		- X	0.000	1.800	2.725	2.725
2	2- 3		X	0.000	1.800	2.725	2.725
i	1 - 2		- X	0.000	1.800	4.448	4.448
2	2- 3	•	~ X	0.000	12.300	4.448	4.448
3	3- 4		- X	0.000	5.000	4.448	5.511
4	4- 5		~ X	0.000	5.000	5.511	4,448
5	5- 6		- X	0.000	12.300	4.448	4,448
6	6- 7		- X	0.000	1.800	4.448	4.448
. 5	5- 6		- X	10.500	1.800	2.725	2,725
. 6	6- 7		-X	0.000	1.800	2.725	2.725
7	2- 8		-X	0.000	7.000	1.770	1.770
8	4- 9		- X	0.000	7.500	1.770	1.770
. 9	6- 10		- X	0.000	6.500	1.770	1.770

 $\Sigma V = 0.000 (t)$ $\Sigma II = 232.019 (t)$

PEDE Br NO-1

					•				
		C-No 1	C-No 2 C	-No 3	C-No 4	C-No 5	C-No 6	C-No 7	C-No 8
		No 6	No 7	No 8	No 9	No 10	No 1.1	No 12	No.1.3
		1.0000	1.0000 1	.0000	1.0000	1.0000	1.0000	1.0000	1.0000
No	1	1.3800	1.3800 1	.3800	1.3800	1.0000	1.0000	170000	1.0000
No	2	1.6500	0.0000 0	.0000	0.0000	1 0000	0.0000	0.0000	0.0000
No	3	0.0000	1.6500 1	.3800		0.0000			0.0000
No	4	0.0000	0.0000 -1	.4300	0.0000	0.0000	0.0000	-1,0000	0.0000
No	5	0.0000	0.0000 0	.0000	1.3200	0.0000	0.0000		0.8000

No 1: 6 7 8 9
No 2: 10 11 12 13

No. X-D15 (mm) Y-D15 (mm) ROTA (mm)	í	(mmRad)	2		F*,	ıç	**	6	F*	0	0	0		(mmRad)	*1	<u>ر</u> د	1 C-1	i co	, o	ဖွာ	<u>.</u>	o (
X-DIS.(mm) Y-DIS.(mm)		OTA.	3.61	3.70	9.	0.09	2.65	.00	7.3	00.	00.	00.		⋖.	s c	100	100	70.	2.84	.98	F4 (38	30.			i											
Case. 7 X. 75 (mm) NoTA. (mmRad) XDIS. (mm) ROTA. (mmRad) XDIS. (mm) PDIS. (mm) NoTA. (mmRad) XDIS. (mm) XDIS. (mm) NoTA. (mmRad) XDIS. (mm) XDIS. (mm) NoTA. (mmRad) XDIS. (mm) NoTA. (mmRad) XDIS. (mm) NoTA. (mmRad) XDIS. (mm) XDIS. (mm) NoTA. (mmRad) XDIS. (mm) X.		-DIS. (.2608	0.2838	11.6566	0.5536	0.5350	0.2777	8833	0000	0000	. 0000		-DIS.	0.00	0.2456	10.7926	0.5138	0.6869	0.2262	1000		0000.														
X-DIS.(mm) Y-DIS.(mm) ROTA.(mmRad) X-DIS.(mm) Y-DIS.(mm) ROTA.(mmRad) C-67991 C-679991 C-679991 C-679991 C-679991 C-679991 C-679991 C-679991 C-679991 C-679992		Cas -DIS.(mm	.6963	,6958	.6639	6476	6358	. 5899	. 5903	.0000	0000	. 0000	į	-DIS. (mm	.2836	2836	4104	0.3498	1.1103	2.9843	2000		0000														
X-DIS.(mm) Y-DIS.(mm) ROTA.(mmRad) X-DIS.(mm) Y-DIS.(mm) P-DIS.(mm) POTA.(mmRad) X-DIS.(mm) Y-DIS.(mm) Y-DIS.(mm) POTA.(mmRad) X-DIS.(mm) Y-DIS.(mm) P-DIS.(mm) P-DIS		FA. (mmRa	4.0426	4.1376	. 9387	.0248	3.9275	1381	0431	0000	0000.	0000.		OTA. (mmRa	2.6402	2.7091	7711	.0268	7595	2607.	4 C		.0000													:	
X-DIS.(mm) Y-DIS.(mm) ROTA.(mmRad) X-DIS.(mm) -0.67991	•	-DIS.(m	9866	0.3389	4.5002	0.7094	4.7618	0.3121	.0082	0000.				-DIS.	5354	0.2480	0.5636	0.5088	0.4098	2222.0		0000	.0000	٠.	*												
X-DIS.(mm) Y-DIS.(mm) ROTA.(mmRa -0.67991 6.57020 -3.8219 -0.72966 -15.23507 3.99816 -0.74764 -0.73113 0.038816 -0.74764 -15.01277 -3.9816 -0.81767 -0.52419 3.9171 -0.81767 -0.52419 3.9171 -0.81767 -0.52419 3.9171 -0.0000 0.00000 0.0000 0.00000 0.00000 0.0000 0.00000 0.00000 0.0000 0.00000 0.00000 0.0000 0.53174 -0.25450 -2.9539 0.53174 -0.25450 -3.0628 0.48578 -0.19112 1.7491 0.44773 -0.19112 1.7491 0.44773 -0.19112 1.7491 0.44773 -0.19112 1.7491 0.00000 0.00000 0.00000 0.00000 0.00000 0.00000		DIS. (.2891	.2891	0.0121	0.4745	1.5612	4.2395	4.2353	0000			Case	-DIS. (0.4702	0.4702	0.5047	0.5171	0.5299	0		0000	.0000	*.				-									
Case. 7 -0.67991 -0.57020 -0.67991 -0.72966 -0.72966 -0.73115 -0.74764 -0.73115 -0.81767 -0.81767 -0.81767 -0.81767 -0.81767 -0.81767 -0.81767 -0.81767 -0.81767 -0.81767 -0.81767 -0.81767 -0.81767 -0.81767 -0.00000		OTA.(mmRa	3.8219	3.9170	000	0.0000	0.00	1/16.	0228	0000		•		. (mmRa	2.9939	3.0628	.0405	0.7004	1 8938	5802	0000	0000.	.0000			. (ກກກຄື	2.5131	2.5820	2.2685	のずのつ・つ	ついのか・1	3761	0000	0000	.0000		
X-DIS. (mm) X-DIS	<i>L</i>	Ď1S. (6.5702	100000	10.400	410.01	7770.01	40000	0000					-DIS.(mm)	1656	0.2545	12.8472	0.4591	0.8864	2 8643	0000	0000	.0000			-DIS.(mm)	3479	0.2066	8.3949	0.40.0 0.40.0	0.000	2.3078	.0000	.0000	0000.		
	. U	-DIS.(mm	0.6799	7005	2077	0.446	0 0 0 0	0110	0000		0000	3 3 3	s S	-Dis.(mm	.5317	.5317	4965	4837	7,44	4477	0000	.0000	.0000		S	-D1S.(mm	.2121	2119	1 0 0 C -	0001	0.000	.1362	.0000	.0000	0000		•
			· ·											No.	· • • • •						∞	O	.0.		:	No.	क इन्ते :	. 63	, m s	.	့် ထ	1/	<u>«</u>	.	10.		

		Case 1 DEAD	LOAN		Case 2 LIVE	1045	
٧o	L(m)	M (tm)	S (t)	N (t)	y (tm)	S (1)	N (t)
	12 (11.7)		5 (1)	3 (4)	.4 (411)	0 (1)	
1 - 2	0.000	0.000	0.000	0.000	0.000	0.000	0,000
2 - 1	1.800	-116.217	-129.130	0.000	0.000	0.000	0.000
2 – 3	0.000	-590.328	376.480	-102.810	-183.914	114.146	-36.379
* 1	1.230	-181.525	288.241	-102.810	-54.294	96,619	-36.379
* 2	2.460		217.990	-102.810	53.768	79.091	-36.379
* 3	3.690	359.158	163.275	-102.810	140.270	61.564	-36.379
* 4	4.920	526.336	108.559	-102.810	205.214	44.036	-36.379
× 5	6.150	626.214	53.844	-102.810	248.599	26.509	-36.379
* 5	7.380	658.792	-0.871	-102.810	270.425	8.981	-36.379
* 7	8.610	624.071	-55.587	-102.810	270.693	~8.546	-36.379
* 8	9.840	522.049	-110.302	-102.810	249.401	-26.074	-36.379 -36.379
	11.070	352.728 116.107	-165.017 -219.733	-102.810 -102.810	206,551 142,142	-43.601 -61.129	-36.379
35 Z	12.300	110.107	-219.733	~102.010	142.143	-01.129	-40.319
3- 4	0.000	116.107	-219.733	-102.810	142.142	-61.129	-36.379
* 1	1.000	-126.221	-265.279	-102.810	73.888	-75.379	-36:379
* 2	2 000	-415,158	-312.949	-102.810	-8.616	-89.629	-36.379
* 3	3.000	-752.826	-362.743	-102.810	-105,370	-103.879	-36.379
* 4	4.000	-1141.351	-414.661	-102.810	-216.374	-118 129	-36.379
4- 3	5.000	-1582.855	-468.703	-102.810	-341.628	-132.379	-36.379
4- 5	0.000	-1594.911	469.719	-106.308	-163,542	10.807	1.668
* 1	1.000	-1152.390	415.677	-106.308	-152.736	10.807	1.668
* 2	2.000	-762.848	363.759	-106.308	-141.929	10.807	1.668
* 3	3.000	-424.163	313.965	-106.308	-131.122	10.807	1.668
* 1	4.000	-134.210	266.295	-106.308	-120.316		1.668
5- 4	5.000	109.136	220.749	-106.308	-109.509	10.807	1.668
c c	0.000	109.136	220.749	-106.308	-109.509	10.807	1.668
5- 6 * 1	1.230	347.007	166.034	-106.308	-96.217	10.807	1.668
* 2	2.460	517.579	111.319	-106.308	-82.925	10.807	1.668
* 3	3.690	620.851	56.603	-106.308	-69.633	10.807	1.668
* 4	4.920		1.888	-106.308	-56.340	10.807	1.668
* 5	6.150	625.496	-52.827	-106.308	-43.048	10.807	1.668
* 6	7.380	526.868	-107.543	-106.308	-29.756	10.807	1.668
	8.610	360.941	-162.258	-106.308	-16.464	10.807	1.668
* 8	9.840	127.714	-216.973	-106.308	-3.172	10.807	1.668
* 9	11.070	-177.241	-287.224	-106.308	10.121	10.807	1.668
6- 5	12.300	-584.793	-375.463	-106.308	23.413	10.807	1.668
35 July 2			122.424		2.222		
6- 7	0.000	-116.217	129.130	0.000	0.000	0.000	0.000
7- 6	1.800	0.000	0.000	0.000	0.000	0.000	0.000
3 o	0.000	174 111	-102 910	-505 610	183.914	-36.379	-114.146
2- 8	0.000	474.111 330.177	-102.810 -102.810	-505.610 -530.390	132.984	-36.379	-114.146
* 1 * 2	2.800	330.177 186.242	-102.810	-555.170	82.053	-36.379	-114.146
* 3	4.200	42.307	-102.810	-579.950	31.123	-36.379	-114.146
* 4	5,600	-101.627	-102.810	-694.730	-19.808	-36.379	-114.146
8- 2	7.000	-245.562	-102,810	-629.510	-70.738	-36.379	-114 146
		210.000			. •		•
1.00	The same of		0.405	-938.422	-178.085	38.047	-143.186
4- 9	0.000	12.055	-3.497	-964.972	-121.015	38.047	-143.186
* 1	1.500	6.809	-3.497	-991.522	-63.945	38.047	-143.186
* 2	3.000	1.563	-3,497 -3,497	-1018.072	-6.875	38.047	-143.186
* 3	4.500	-3.683	-3,497	-1044.622	50.196	38.047	-143.186
and the second second	6.000	-8.929 -14.175	-3.497	-1071.172	107.266	38.047	-143.186
9- 4	7.500	-14.170	0.1201	*		1 11	
6- 10	0.000	-468.576	106.308	-504.593	23.413	-1.668	10.807
* 1	1.300	-330.376	106.308	-527.603	21.244	-1.668	10.807
* 2	2.600	-192.176	106.308	-550.613	19.076	-1.668	10.807
* 3	3.900	-53.975	106.308	-573.623	16.908	-1.668	10.807 10.807
* 4	5.200	84.225	106.308	-596,633	14.739	-1.668 -1.668	10.807
10- 6	6.500	222.425	106.308	-619,643	12.571	-1,000	10.001

	÷.	Case 3 LIVE	LOAD		Case 4 TEMPER	ATURE	
No	L(m)	M (tm)	S (t)	N (t)	M (tm)	S (t)	N (t)
1 - 2		0.000	0.000	0.000	0,000	0.000	0.000
2~ 1	1.800	0.000	0.000	0.000	0.000	0.000	0.000
					•		
2- 3	1	-152.905	96,967	-33.156	-69.414	6.480	-24.281
* 1		-43.714	80.579	-33.156	-61.444	6.480	-24.281
* 2	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	45.319	64.190	-33.156	-53.473	6.480	-24.281
* 3	3.690	114.194	47.802	-33,156	-45.503	6.480	-24.281
* 4	4.920	162.912	31.413	-33.156	-37.532	6.480	-24.281
* 5	6.150	191.472	15.025	-33.150	-29,562	6.480	-24.281
* 6	7.380	199.873	-1.364	-33.156	-21.591	6.480	-24.281
* 7	8.610	188.117	-17.752	-33.156	-13.620	6.480	-24.281
* 8	9.840	156.203	-34,141	-33.156	-5.650	6.480	-24,281
* 9	11.070	104.131	-50.529	-33.156	2.321	6.480	-24.281
3- 2	4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	31.901	-66.918	-33.156	10.291	6.480	-24.281
an in the second				00.100			
3- 4	0.000	31.901	-66.918	-33.156	10.291	6.480	-24.281
* 1	1.000	-41.679	-80.242	-33.156	16.772	6.480	-24.281
* 2	2.000	-128.582	-93.566	-33.186	23.252	6.480	-24.281
* 3	4.5	-228.810	-106.890	-33.156	29.732	6.480	-24.281
* 4	4.000	-342.362	-120.214	-33.156	36.212	6.480	-24.281
4- 3	5.000	-469.237	-133.538	-33.156	42.692	6.480	-24.281
		-403.231	-100.000	-99.190			
4- 5	0.000	-473.131	133.866	-34.285	37.501	-6.041	-25.783
* 1	1.000	-345.927	120.542	-34.285	31.460	-6.041	-25.783
* 2		-232.046	107.218	-34.285	25.418	-6.041	-25.783
* 3		-131.490	93.894	-34,285	19.377	-6.041	-25.783
* 4	and the second second	-44.258	80.570	-34.285	13.336	-6.041	-25.783
5- 4	5.000		67.246	-34.285	7.294	-6.041	-25.783
• •	*	29.651	077240	-34.260		0.012	20.,00
5- 6	0.000	29.651	67.246	-34,285	7.294	-6.041	-25.783
* 1	and the second of the second	102.285	50.858	-34.285	-0.137	-6.041	-25.783
* 2	2.460	154.761	34.469	-34.285	-7.568	-6.041	-25.783
* 3	3.690	187.079	18.081	-34.285	-14.999	-6.041	-25.783
* 4			1.692	-34,285	-22.430	-6.041	-25.783
* 5		199.240	-14.696	-34.285	-29.861	-6.041	-25.783
* 6	The state of the s	191.242			-37.292	-6.041	-25.783
* 7	the state of the s	163.087	-31.085	-34.285	-44.723	-6.041	-25.783
* 8	70.00	114.774	-47,473	-34,285:	-52.154	-6.041	-25.783
* 9	and the second second second second	46.302	-63.862	-34.285	-59.585	-6.041	-25.783
6- 5	12.300	-42.327	-80.250	-34.285	-67.016	-6.041	-25.783
0 0	12.300	-151.113	-96.639	-34,285 ·	07.010	0.041	\$0.760
6- 7	0.000	0.000	0.000	0.000	0.000	0.000	0.000
7- 6	1.800	0.000	0.000	0.000	0,000	0.000	0.000
, ,	11000	0.000	0.000	0.000	••••		
2- 8	0.000	152.905	-33.156	-96.967	69.414	-24.281	-6.480
* 1	1.400	106.486	-33.156	-96.967	35.422	-24.281	-6.480
* 2	2.800	60.067	-33.156	-96.967	1,429	-24.281	~6.480
* 3			the state of the s	-96.967	-32.564	-24.281	-6.480
* 4	4 7 7 4	13.648	-33.156		-66.557	-24.281	-6.480
8- 2		-32.771	-33.156	-96.967	-100.550	-24.281	-6.480
0- 2	, ,,,,,,,,,	-79.190	-33.156	-96.967	100.000	24.201	0.400
			1 120	-267.404	5.191	-1.503	12.522
4- 9	0.000	3.894	-1.128	· ·	2.936	-1.503	12.522
* 1	1.500	2.201	-1.128	-267.404	0.682	-1.503	12.522
* 2		0.508	-1.128	-267.404	-1.572	-1.503	12.522
* 3		-1.184	-1.128	-267. <u>4</u> 04	-3.826	~1.503	12.522
* 4	and the second of the	-2.877	-1.128	-267.404	-6.080	-1.503	12.522
9- 4	7.500	-4.569	-1.128	-267.404	0.000	1.000	10,000
				-96.639	-67.016	25.783	-6.041
6-10	0.000	-151.113	34.285		-33.498	25.783	-6.041
* 1	the state of the state of the state of the	-106.543	34.285	-96.639	0.021	25.783	-6.041
* 2		-61.973	34.285	-96.639	33.539	25.783	-6.041
* 3		-17.403	34.285	-96.639	67.057	25.783	-6.041
* 4	and the same of	27.168	34.285	-96.639	100.576	25.783	-6.041 -6.041
10- 6		71.738	34.285	-96.639	1001010	201700	0.041
	+ + * * * *						The second secon

		1.	Case 5 SEISMIC			Case 6		-
	No	L(m)	M (tm)	S (t)	N (t)	M (tm)	S (1)	· N (t)
V-1	1.						0 (1)	
1 -	2	0.000	0.000	0.000	0.000	0.000	0.000	0.000
2 -	l	1.800	0.000	0.000	-12.911	-160.380	-178.200	0.000
			~ , , , , ,	0.000	12.911	-100.380	-176.200	0.000
2 -	3	0.000	176.733	-17.428	44.084	1110 110	505 000	001 001
*		1.230	155.297			-1118.112	707.883	-201.904
		The Control of the Co		-17.428	35.261	-340.090	557 193	-201,904
	2	2.460	133.861	-17.428	28.237		43),327	-201.904
疹		3.690	112.424	-17.428	22.766	727.084	326.899	-201.904
>:	_	4.920	90.988	-17.428	17.295	1064.946	222.472	-201.904
*		6.150	69.552	-17.428	11.824	1274.364	118.044	-201.904
*	6	7.380	48,115	-17.428	0.353	1355.335	13.637	-201.904
*	7	8.610	26.679	-17.428	0.882	1307.860	-90.811	-201.904
*	- 8	9.840	5.243	-17.428	-4.589	1131.940	-195.238	-201.904
*	.9	11.070	-16.194	-17.428	-10.060	827.574	-299.666	-201.904
3-	2	12.300	-37.530	-17.428	-15.531	394.762	-404.094	-201.904
-		10.0			10.001	0.5 7 1.1 9 2	2042032	201.304
3-	4	0.000	-37,630	-17.428	-15.531	394.762	-404.094	-201.904
*		and the second second	-55.058	-17.428	-20.085	-52.271	·	
*		2.000		-17.428			-490-459	-201.904
and the second	3		the contract of the contract o	· ·	-24.852	-587.135	-579.757	-201.904
*		3.000		-17.428	-29.832	-1212.761	** *	-201.904
*	4	4.000		-17,428	-35.024	-1932.081	-767.144	-201.904
4-	3	5.000	-124.769	-17.428	-40.429	-2748.026	-865.235	-201.904
	-					1 10 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		
. 4 -	4.00	0.000	137.357	-19.393	26.346	-2470.822	666.044	-143.953
*		1.000	117.964	-19.393	20.942	-1842.312	591.466	-143.953
*	2	2.000		-19.393	15.750	-1286.914	519.819	-143.953
*	3	3.000	79,179	-19.393	10.770	-801.697	451.103	-143.953
*	4	4.000	59.786	-19.393	6.003	-383.730	385.318	-143.953
5⊤	4	5.000	40.393	-19.393	1.449	-30.083	322.465	-143.953
	1.	-				•		
5-	6	0.000	40,393	-19.393	1.449	-30.083	322.465	-143.953
*	1	1.230	16.540	-19.393	-4.022	320.112	246.958	-143.953
*	2	2.460	-7.313	-19.393	-9.493	577.433	171.451	-143.953
*	3	3.690	-31.166	-19.393	-14.964	741.881	95.944	
\$	4	4.920	-55.019	-19.393	-20.435	813.455	20.436	-143.953
*	5	6.150		-19.393	-25.906	792.155	-55.071	-143.953
*	6	7.380	-102.725	-19.393	-31.377	677.981	-130.578	-143.953
*	7	8.610		-19.393	-36.848	470.933	-206.085	-143.953
*	8	9.840	-150.432	-19.393	-42.319	171.012	-281,592	
*	9	11.070		and the second of the second o				-143.953
6-	5		-174.285 -198.138	-19.393	-49.344	-227.894 -768.384	-378.538	-143.953
0-	. 0	12.300	-190.196	-19.393	-58.166	~ 708.364	-500.308	-143.953
6-	7	0.000	0.000	0.000	12.911	-160.380	178.200	0.000
7-					0.000			0.000
	U	1.800	0.000	0.000	. 0.000	0.000	0.000	0.000
9	0	0.000	-176 722	56.996	17.428	957.732	-201 004	_000 000
	•		-176.733				-201.904	-886.083
*	1	1.400	-95.205	59.474	17.428	675.067	-201.904	-920,279
*	2	2.800	-10.207	61.952	17.428	392.402	-201.904	-954.476
*	3	4.200	78.260	64.430	17.428	109.737	-201.904	-988.672
*	4	5.600	170.196	66.908	17.428	-172.928	-201.904	-1022.868
8-	2	7.000	265.601	69.386	17.428	-455.593	-201.904	-1057.065
						055.00	***	
4-	9	0.000	-262.127	66.775	1.965	-277.204	57.951	~1531.278
*	1	1.500	-159.973	69.430	1.965	-190.278	57.951	-1567.917
*		3.000	-53.836	72.085	1.965	-103.352	57.951	-1604.556
*	3	4.500	56.282	74.740	1.965	-16.426	57.951	-1641.195
*	4	6.000	170.384	77.395	1.965	70.500	57.95)	-1677.834
9-	4	7.500	288.468	80.050	1.965	157.427	57.951	-1714.473
	. i	. 1 7	Committee and the second					
6	10	0.000	-198.138	71.078	-19,393	-608.004	143.953	-678.508
*	1	1.300	-104.241	73.379	-19.393	-420.865	143.953	-710.261
*	3	2.600	-7.353	75.680	-19.393	-233.727	143.953	-742.015
×	3	3.900	92.527	77.981	-19.393	-46.588	143.953	~773.769
*	4	5.200	195.397	80.282	-19.393	140.550	143.953	-805.523
10-	6	6.500	301.259	82.583	-19.393	327.689	143.953	-837,277

PEDE	Br :	NO-1						·
			Case 7			Case 8		•
		No L(m)	M (tm)	s (t)	N (t)	M (tm)	S (t)	8 (1)
		of Paris		÷		· · ·		
	1 -	2 0.000		0.000	0.000	0.000	0.000	0.000
	2,-	1 1.800	-160.380	-178.200	0.000	-160.380	-178.200	0.000
	2-	3 0.000	-1066.947	679.538	-196.587	-926.400	644.091	-152.913
	- Z	1 1.230		530.728	-196.587	-222.966	499.705	-152.913
	ંજ્ર			406.741	-196.587	311.065	380.142	-152,913
100	3,0	3 3.690		304.192	-196.587	718.295	282.019	-152.913
	27	The state of the s		201.644	-196.587	1004.833	183.896	-152.913
	4	5 6.150		99.096	-196.587	1170.679	85.773	-152.913
	**	6 7.380		-3.452	-196.587	1215.833	-12.351	-152.913
**	*	7 8.610		-106.000	-196.587	1140.296	-110.474	-152.913
**	*	8 9.840		-208.549	-196.587	944.067	-208.597	-152.913
:	*	9 11.070		-311 097	-196.587	627.147	-306.721	-152.913
• •	3-	2 12.300	212.865	-413.645	-196.587	189.535	-404.844	-152.913
	3-	4 0.000	212.865	-413.645	-196.587	189.535	-404.844	-152.913
	*	1 1.000		-498.483	-196.587	-255.685	-486.085	-152.913
	*	2 2.000		-586.252	-196.587	-783.612	-570.256	-152.913
4 4 4 4 4 4	*	3 3.000		-676.953	-196.587	-1397.175	-657.359	-152.913
11.1	*	4 4.000		-770.584	-196.587	-2099.307	-747.393	-152.913
	4-	3 5.000	-2958.582	-867.147	-196.587	-2892.938	-840.358	-152.913
		5 0 000	2001 649	060 000	000 075	2007 525	041 507	-157.148
	4 *	5 0.000 1 1.000		869.092 772.530	-203.275 -203.275	-2907.525 -2112.664	841.587 748.622	-157.148
	*	2 2.000			-203.275	-1409.303	658.588	-157.148
	*	3 3.000		588.198	-203,275	-794.510	571.486	-157.148
	*	4 4.000		500.429	-203.275	-265.355	487.314	-157.148
	5-	4 5.000		415.590	-203.275	181.095	406.073	-157.148
					The state of the			
	5 -	6 0.000		415.590	-203.275	181.095	406.073	-157.148
	*	1 1.230		313.042	-203.275	620.219	307.950	-157.148
	×	2 2.460		210.494	-203.275	938.652	209.827	-157.148
	*	3 3.690 4 4.920		107.946 5.398	-203.275 -203.275	1136.393 1213.442	111.703 13.580	-157.148 -157.148
	*	5 6.150		-97.151	-203.275	1169.800	-84,543	-157.148
1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	*	6 7.380		-199.699	-203.275	1005.466	-182.667	-157.148
	*	7 8.610		-302.247	-203.275	720.440	-280,790	-157.148
	\$:	8 9.840		-404.795	-203.275	314.723	-378.913	-157.148
7.8%	*	9 11.070		-528.782	-203.275	-217.796	-498.475	-157.148
	6-	5 12.300	-1056.352	-677.593	-203.275	-919.718	-642.861	-157.148
1				150 000	0.000	160 200	170 000	0.000
	6-	7 0.000		178.200	0.000	~160.380 0.000	178.200	0.000
•	7-	6 1.800	0.000	0.000	0.000	0.000	0.000	0.000
	. 2 -	8 0.000	906.567	-196.587	-857.738	766.020	-152.913	-822.290
	*	1 1.400	and the grade of the first term of the contract of the contrac	-196.587	-891.934	551.942	-152.913	-856.487
	25	2 2.800		-196.587	-926.131	337.863	-152.913	-890.683
	\$.	3 4.200		-196.587	-960.327	123.785	-152.913	-924.879
	*	4 5.600	-194.318	-196.587	-994.524	-90.293	-152.913	-959.076
	8-	2 7.000	-469.539	-196.587	-1028.720	304.372	-152.913	-993.272
			00.001	C C00	1776 970	14.587	-4.235	-1681.946
	4-	9 0.000		-6.688	-1736.239 -1772.878	8.235	-4.235	-1718.585
	*	1 1.500		-6.688 -6.688	-1809.517	1.883	-4.235	-1755.224
	*	2 3.000 3 4.500		-6.688	-1846.156	-4.469	-4.235	-1791.863
	*	4 6.000		-6.688	-1882.795	-10.821	-4.235	-1828.502
	9-			-6.638	-1919.434	-17.173	-4.235	-1865.141
							•	
	6-	10 0.000		203.275	~855.793	-759.338	157.148	-821.061
	*	1 1.300		203.275	-887.546	-555.046	157.148	-852.815
	*	2 2.600		203.275	-919.300	-350.754	157.148	-884.568
	*			203.275	-951.054 -982.808	-146.463 57.829	157.148 157.148	-916.322 -948.076
and the second	10			203.275 203.275	-1014.562		157.148	-979.830
	10-	6 6.500	425.314	200.510	1014.004	- 500,101	10,1140	0,0,000

			Case 9	·		Case 10		
	No	L(m).	M (tm)	S (1)	N (t)	M (tm)	s (t)	N (1)
			3 (111)	9, (()		14 (C III)	0 (()	., , , , ,
1 -	2	0.000	0.000	0.000	0.000	0.000	0.000	0,000
2 -	1	1.800	-160.380	-178.200	-17.043	-116.217	-129.130	0.000
			,00.000	1.0.400	27.0740		1401100	****
2 ~	: 3	0.000	-581.366	496.537	-83.687	-774,243	490.626	-139.189
*	-1	1.230	-45.513	374.767	-95.333	-235,819	384.859	-139.189
*	. 2	2.460	348.754	277.821	-104.605	178.447	297.081	-139.189
*	-3	3.690	644.038	202.314	-111.827	499.428	224.838	-139.189
*	4	4.920	846.447	126.807	-119.049	731.550	152.596	-139.189
*	5	6.150	955.983	51.300	-126.271	874.813	80.353	-139.189
*	- 6	7.380	972.6.15	-24.207	-133.492	929.217	8.110	-139,189
×	7	8.610	896.434	-99.714	-140.714	894.763	-64.133	-139.189
×	8	9.840	727.348	~175.221	-147.936	771.450	-136.376	-139.189
*	9	11.070	405.389	-250.729		559.279	-208.619	-139.189
3-	2	12.300		· ·	-155.158	258.249	-280.861	-139.189
-	_	7 7 7 7 7 7	110.556	-326.236	-162.380	200.249	-200.001	-100/100
3-	4	0.000	110.556	-326.236	-162.380	258.249	-280.861	-139.189
*	1	1.000	-246.862	-389.089	-168.391	-52.334	-340.657	-139.189
s k t	2	2.000	-668.599	-454.874	-174,684	-423.774	-402.577	-139.189
*	3	3.000	-1157.586	-523.589	-181,256	-858.196	-466.621	-139.189
:k	. 4	4.000	-1716.755	-595.236	-188.110	-1357.725	-532.789	-139.189
4	3	5.000	-2349.036			-1924.483	-601.081	-139.189
•		0.000	-5949:090	-669,814	-195.244	1964.400	-001.001	-109-109
4-	5	0.000	-2019.665	622.614	-111.928	-1758.453	480.526	-104.640
*	ī	1.000	-1434.584	548.036	-119.062	-1305.125	426.484	-104,640
*	2	2.000	-922.616	476.389	-125.915	-904.777	374.566	-104.640
*	ä	3.000	-480.829	407.674	-132.488	-555,285	324.772	-104.640
*	· 4	4.000				-254.525	277.102	-104.640
5-	4	5.000	-106.292	341.889	-138.781	-0.374	231.556	-104.640
	•		203.926	279.036	-144.792	-0.374	231.330	-104.040
5-	6	0.000	203.926	279.036	-144.792	-0.374	231.556	-104.640
*	1	1,230	500.703	203.528	-152.014	250.790	176.841	-104.640
*	2	2.460	704.606	128.021	-159.236	434.654	122.125	-104.640
**	3	3.690	815.635	52.514	-166.458	551.219	67.410	-104.640
*	4	4.920		-22.993	-173.679	600.483	12.695	-104.640
*	5	6.150	833.791		-180.901	582.448	-42.021	-104.640
*	6	7.380	759.073	-98.500: -174.007	-188.123	497.112	-96.736	-104.640
*	7	8.610	591.481			344.477	-151.451	-104.640
*	8	9.840	331.015	-249:514	-195.345	124.542	A Company of the Comp	-104.640
- > *c	9	11.070	-22.325	-325.022	-202.566	-167.120	-276.417	-104.640
6-	5	12.300	-474.649	-421.968	-211.839	-561.381	-364.656	-104.640
U T	J	12.500	-1068.557	-543.737	-228.485	-201.301	-304.030	-104.040
6-	7	0.000	-160.380	178.200	17.043	-116.217	129.130	0.000
7-	6	1.800	0.000	0.000	0.000	0.000	0.000	0.000
15.1			v. v.v	V. VVO	0.000	0.000	0.000	.01000
2-	8	0.000	420.986	-66.644	-674.737	658.026	-139.189	-619.756
*	1	1.400	329.974	-63.373	-708.933	463.160	-139.139	-644.536
*	2	2.800	243.541	-60.102	-743.130	268.295	-139.189	-669.316
k	3	4.200	161.687	-56.831	-777.326	73.430	-139.189	-694.096
*	4	5,600	84.413	-53.560	-831.522	-121.435	-139.189	-718.876
8-	2	7.000	11.718	-50,289	-845.719	-316.300	-139.189	-743.656
	7		11.720					
			-329.371	83.317	-1292.428	-166.030	34.549	-1081.607
4	9	0.000	-201 767	86.821	-1329.067	-114.206	34.549	-1108.157
*	1	1.500	-68.907	90,326	-1365.706	-62.382	34.549	-1134.707
*	2	3.000	69.210	93.830	-1402.345	-10.558	34.549	-1161.257
*	3	4.500	212.584	97.335	~1438.984	41.266	34.549	-1187.807
*	4	6,000	361.215	100.840	-1475.623	93.091	34.549	-1214.357
9 -	4	7.500	301.310	Coom-o		W		
٠.	1 /	0.000	-908.177	240.528	-721.937	-445.163	104.540	-493.786
	10	0.000	-593.517	243,565	-753.691	-309.131	104.640	-516.796
*		1.300	-271.908	246.602	-785.445	-173.100	104.640	~539.806
Ąŧ		2,600	47.649	249.640	-817.198	-37.068	104.640	-562 816
*	3	3,900	374.155	252.677	-848.952	98.964	104.640	-585.826
* 10-	- 6	5,200 6,500	704,609	255.714_	-880.706	234.996	104.640	-608.836

			Case 11			Case 12		
	No	L(m)	M (tm)	. S (t)	N (t)	M (tm)	S (t)	N (t)
			1	0 (1)	х (т)	.4 (() ()	5 (()	., (()
1 -	2	0.000	0.000	0.000	0.000	0.000	0,000	0.000
2	1.	1.800	-116.217	-129,130	0.000	-116.217	-129.130	0.000
				100,100	0.000	110.51,	1551100	0.000
2	3	0.000	-743.234	473.447	-135.967	-673.819	466.967	-111.686
*	1	1.230	-225.239	368.820	-135.967	-163.796	362.340	-111.686
*	_ 2	2.460	169,999	282.181	-135.967	223.472	275.700	-111.686
*	. 3	3.690	473.352	211.077	-135.967	518.855	204.597	-111.686
*	4	4.920	689,248	139.973	-135.967	726.780	133.493	-111.686
*	5.	6.150	817.685	68.869	-135.967	847.247	62.389	-111.686
×.	6	7.380	858.665	-2.235	-135.967	880.256	-8.715	-111.686
*	7	8.610	812.188	-73.339	-135.967	825.808	-79.819	-111.686
*	8	9.840	678,252	-144.443	-135.967	683.902	-150.923	-111.686
*	9	11.070	456.859	-215.546	-135.967	454.538	the state of the s	-111.686
3	2	12.300	148.008	-286.650		137.717	-293.130	-111.686
			140.000	200.000	-135.967	101.111	290.100	-111.000
3-	4	0.000	148.008	-286.650	-135.967	137.717	-293.130	-111.686
*	1	1.000	-167.900	-345.520	-135.967	-184.672	-352.000	-111.686
*	2	2.000	-543.740	-406.514	-135.967	-566.992	-412.994	-111.686
×	- 3	3.000	-981.636	-469.632	-135.967	-1011.368	-476.112	-111.686
*	4.	4.000	-1483.713	-534.874	-135.967	-1519.925	-541.354	-111.686
:4-	3	5.000	-2052.093	-602.240	-135.967	-2094.785	-608.720	-111.686
·	-		2002.000	-002.240	-199.901	207,100	050.720	~ / 2 J + WOO
4	. 5	0.000	-2068.042	603.586	-140.593	-2105.543	609.627	-114.809
*	1	1.000	-1498.316	536.220	-140.593	-1529.776	542.261	-114.809
*	2	2.000	-994.895	470.978	-140.593	-1020.313	477.019	-114.809
*	3	3.000	-555.653	407.860		-575.030	413.901	-114.809
*	4	4.000	-178.467	346.866	-140.593	-191.803	352.907	-114.609
5 -	4	5.000	138.786	287.996	-140.593	131.492	294.037	-114.809
-				201.990	-140.593	10)1.432	235.001	114.005
: 5-	6	0.000	138.786	287.996	-140.593	131.492	294.037	-114.809
*	1	1.230	449.292	216.892	-140.593	449.429	222.933	-114.809
*	2 -	2.460	672.340	145.788	-140.593	679.908	151.829	-114.809
*	3	3.690	807.930	74.684	-140.593	822.929	80.726	-114.809
*	4	4.920	856.063	3.580		878.493	9.622	-114.809
*	5	6.150	816.738	-67.524	-140.593 -140.593	846.599	-61.482	-114.809
*	6	7.380				727.247	-132.586	-114.809
*	7.		689.955 475.714	-138.627	-140.593	520.438	-203.690	-114.809
*	8	1. 1	174.016	-209.731	-140.593	226.170	-274.794	-114.809
*		11.070		-280.835	-140.593	-159.982	-361.433	-114.809
6-	. 5	12.300	-219.567	-367,474	-140.593	-668.891	-466.060	
•		12.000	-735.907	-472.102	-140.593	. 000.031	400.000	-114.809
6~	7	0.000	-116.217	129.130	0.000	-116.217	129.130	0.000
7-	. 6	1.800	0.000	0.000	0.000	0.000	0.000	0.000
			0.000	0.000	0.000			
2-	8	0.000	627.016	-135.967	-602.577	557.602	-111.686	-596.097
*	1	1 400	136.663	-135.967	-627.357	401.241	-111.686	-620.877
*	2	2.800	246.309	-135.967	-652.137	244.880	-111,686	-645,657
*	3	4.200	55.955	-135.967	-676.917	88.519	-111.686	-670.437
*	4	5.600	-134,398	-135.967	-701.697	-67.841	-111.686	-695.217
8-	2	7.000	-324.752	-135.967	-726.477	-224.202	-111.686	-719.997
	- T.		324.733	-100.007	720.411			, (5.557
			15.949	-4.626	-1205.826	10.758	-3.123	-1218.347
4-	9	0.000	9.010	-4.626	-1232.376	6.074	-3.123	-1244.897
*	1		2.072	-4.626	-1258.926	1.389	-3.123	-1271.447
*	2	3.000	-4.867	-4.626	-1285.476	-3.295	-3.123	-1297.997
*	3	4.500	-11.806	-4.626	-1312.026	-7.980	-3.123	-1324.547
*	4	6.000	-18.745	-4.626	-1338.576	-12.664	-3.123	-1351.097
9-	4	7.500	10.740	4.020	1000.070			
		المناسية الم	-619.690	140.593	-601.232	-552.673	114.809	-595.190
6-		0.000	-436.919	140.593	-624.242	-403.421	114.809	-618.200
*	: 1	1.300	-254.149	140.593	-647.252	-254.169	114.809	-641.210
»į:	. 2	2.600	-71.378	140.593	-670.262	-104.917	114.809	-664.220
*	3	3.900	111.392	140.593	-693.272	44.335	114.809	-687.230
**	4	5.200		140.593	-716.282	193.587	114.809	-710.240
10-	6	6.500	294.163	140.030	110.202		1171003	110.640

PEDE Br	NO-1			
	No L(m)	Case 13 M (tm)	\$ (t)	N (t)
1-2-		0.000 -116.217	0.000 -129.130	0.000 -10.329
2-	4 1 1.230 2 2.460 3 3.690 4 4.920 5 6.150	-448.942 -57.288 231.768 449.097 599.126 681.855 697.284	362.537 274.298 204.048 149.332 94.617 39.902	-67.543 -74.601 -80.221 -84.598 -88.974 -93.351 -97.728
* * 3-	7 8.610 8 9.840 9 11.070	645.414 526.243 339.773 86.003	-69.529 -124.244 -178.959 -233.675	-102.105 -106.482 -110.859 -115.235
3- * * * 4-	1 1.000 2 2.000 3 3.000	86.003 -170.268 -473.146 -824.757 -1227.224 -1682.671	-233.675 -279.221 -326.891 -376.685 -428.603 -482.645	-115.235 -118.879 -122.692 -126.676 -130.830 -135.153
4- * * * *	2 2.000 3 3.000	-1485.025 -1058.018 -683.991 -360.820 -86.381 141.450	454.205 400.163 348.245 298.451 250.781 205.235	-85.231 -89.555 -93.708 -97.692 -101.505 -105.149
5- * * * * * * * *	2 2.460 3 3.690 4 4.920 5 6.150 6 7.380 7 8.610 8 9.840 9 11.070	141.450 360.239 511.729 595.918 612.808 562.398 444.688 259.678 7.368	205.235 150.520 95.804 41.089 -13.626 -68.342 -123.057 -177.772 -232.487 -302.738	-105.149 -109.526 -113.902 -118.279 -122.656 -127.033 -131.410 -135.787 -140.163 -145.783
6 - 7 -		-743.304 -116.217 0.000	-390.977 129.130 0.000	-152.841 10.329 0.000
2- * * * * * *	8 0.000 1 1.400 2 2.800 3 4.200 4 5.600 2 7.000	332.725 254.013 178.076 104.915 34.530 -33.081	-57.214 -55.232 -53.249 -51.267 -49.284 -47.302	-491.668 -516.448 -541.228 -566.008 -590.788 -615.568
4- * * * * *	9 0.000 1 1.500 2 3.000 3 4.500 4 6.000 4 7.500	-197.646 -121.169 -41.506 41.343 127.378 216.599	49.923 52.047 54.171 56.295 58.419 60.543	-936.850 -963.400 -989.950 -1016.500 -1043.050 -1069.600
6- * * * 10-	10 0.000 1 1.300 2 2.600 3 3.900 4 5.200 6 6.500	-627.087 -413.769 -198.058 20.046 240.543 463.433	163.170 165.011 166.852 168.693 170.533 172.374	-520.107 -543.117 -566.127 -589.137 -612.147 -635.157

PICK-UP NO 1 (ULS)

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	N Ct	0.00	201.50	196.58	119.04	126.27	140.71	200	162.38	-162.380	9	တ် မ	9	203.27	203.27	203.27	-143.953	143.95	143.95 143.95	143.95	80.95	188.12	195.34	-211.839	00.0		67.4		960	250		329	1604.556	38.5	6.6	3	-887.546	ο α (γ C	9 00 5 00 .
X C X	æ	200	88 55 68 55 68 55	741	.807	207	714	725	982.	236	252	553	147	60	000	198	45.8 55.8	463	0) 10 10 10 10	9 6	500	007	017	968	200	000	. 00	(; ••	10 (587	317 -	\$21	(36)	688	oc cc	2.8	275	00 00 17 17	000
NI W.	W	0 -178	707 8							-326	-580	-676	-867	∞ •	772	880	383	~			~~			1421.9		o.	13	99	10	-196.	83.	86.	524	9	Ġ	41	100	60 50	100
Σ	M (tm)		118.112	46.833	4.0	72.645	96.434	65.389	9	110,556	783	39.	958.	981.6	151.076	802.303	383.730	80.	4	88	2 4 5	48	0 8	474.649	160.380	0	S	ດເລ	0.0	-154.318	60	101.767	103.352	17.069	27.101	28.17	367.458	46 46 83	
	5.0	ထတ		0 ج	o c n	ກທ	о л (ስርኮር		ا ص	•	(† C)		2.	7 -		ບທ							5 0 0		 On				- (-	"	'	1 10 to			• [•	
	Cas	្រក្	ያሳ						-	ပ်ပ				ပံ၊	აბ	. ს	ပ်ပံ	ů	აა	ას	ე ე	ሪ	ပ်ပ်	99	ა .	ပ	ს	ပ်ပ	ပ်ဖ	79	ပ်	Ç	ბბ	ւ .	ს ՝	ចំ ថ	50	ა	, ს
	N C	0.000	-83.687	-104.605	-201.504	-201.904	-201.904	201.904	406.107	-201.904	50	88	8	111.92	125.00	32.48	-138.781	44.79	03.27	03.27	03.27	57.14	57.14	-157.148	000:0	•	98	့် ကို	777.	- us	736.	772.	1809.517	300	475	678.	-710.26	8.17	88
X O X	3	200	537	ы с	. 03	.	C			094	737	388	4.	14		74	8 C							308	200	000	25	4.4	31	280	88	. 88	688 -1) in			ກຕຸ		
- X + X	S	-178	374.	ri d		i ni	å.		:	. 404 -	٠.,			22	9 9	6	241.	79		6		182.	280 378	-498.	178	•	-1.		φ.	; ;	9	Ġ	9.0		ol.		2 11		
Σ	M (tm)	0.000	81.366							94,762	() (200	ē	95	9.0	82	105,292 203,926	. 6	2.0	4.	35	46	7 7	217.796	60.380	۹,	4	25	1.68	7.7	90.	6	2.986	10		8	13.727	7 64	9
	6		ις I Ο σι	m 1~	2:	1 6	3.		,	რ i დ დ	10 :	7 64	-53	20		•				м.		-		5 2	17		9	90			,,	_	r 0	- CI	"	'		:	1
	Cas	44	ე ე	ပ်ပ	ბ (ას	δt	. 6	: }	ሳ ሳ	ბ	រប់	ሪ	ბი	5 6	ပ်	56	٠ د	56	٠ د	აა	δ.	ს ს	<u>ა</u> ს	٠,	- 5	ပ်ဖ	5 <u>0</u>	ပ ပ	სბ	S	ပ	ბძ	აბა	٥	ბ	5 t)	ბბ	ას
	J E	0.000	1.236	4.0	0.		٠, a	96	,	1.000				0			5.000	0,0	14	9.	" ~	e, i	200	11.070	000.0	•	•	2.800	•			•	3.000			•	200		
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MINIME	(t) S	0.000	456.537	200		110			8	676.95	-770.584	**	50	10	341.889	0	סניו	. 10	u ra	199.6	302.2	-528.782	0.0	201.90	201.90	201.90	-201.904	6.68	80.0	3	-6.688	44.000000000000000000000000000000000000	63.63 59.63	0.00	
S	X (tm)	0.000	-581.366 -45.513 348.754	4.4	55.	Ç,	2 20 1		~	1416.	-2139.961 -2958.582	2019.66	ຮິດ	. 83	-106.292 203.926	6	- 6		. 6	7	. 4	-314.431	-160.380		200	3 5	-172.928	23.061	(a)	-	-17.069	-608,004 -420,865	233.72	20.02	
	Case	ပ် ပီ -	ပ်ပ်ပ်	, ,	4.7					۰۲ د د	66	-	_	-	ပ်ပ် ၈၈	ပ်း	56					66	က် စ ဧ				ဂ္ဂ ဇ					ဖ ဖ ဖ ပ ပ ပ			
×.	N C	0.000	-201.904 -201.904 -201.904	201.90	201.90	201.90	155.15	62.38	68.39	1.25	-188.110	03.27	03.27	03.27	-203.275	3.275	3.27	7.0	3.0	3.95		-143.953	0.000	4.73	86.5	7.32	-811.522	1292.42	200	1402.34	-1438.584	-721.937	5:7	848.	
. XAXING	S (t)	-178.200	707.883 557.193 431.327	26.89	13.61	90.83	323	5.23	385.08	3.58	1669.814	60.69	3 8	88.19	500.429 415.590	5.59	2 0	2.70	55.07	30.57	200	-378.538	178.200	66.6	ი ⊶	56.8	-53.360	3.3	90.326	80	0.64	243.528	49.6	52.0 50.0	
'n	M (tm)	0.000	-1118.112 -340.090 260.775	κ	* 10		300	10.55	20.00 00.00 00.00	-1157.586	0.03	2981.	9 69	8	195.531	99.5	9.696	6.4	92.1	9.07		-227.894	-160.380	20.		61.	84.413	oi.	5 5	69.21	2 2 3	-908.177	44.64	74.15	
	Case	ပ် ပဲ က	ပ်ပုံပဲ							ပ်ပ	3 1		. c						ï	, ,		မှ မ ပ ပ	7 7 6 6				ဂု ဂု စ စ					0 0 0 0 0 0			
		0.000	0.000 1.230 2.460							3.000		-	2.000	•			• •			• 1		11.070	0.000	٠,	<u></u>	٠;	7.000	•	3.000	•	7.500	0000	90,	es no	
	.0X	11	N N (ગ વ ા * ૐ		•		ч. 	* *	m ₹ 1	4. Ε.Ι 1. ω	4. เมษ	+ e4	en •	ε 1 + 1 2 + 2	κ κ φ						ω αιο	7 - 7	00 r	7 77		. 60 ∔ 1	1 t		eo • * •	7 √ 7 * I O	0 H C 1 ★ 1	v (3		

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_ PICK-UP NO 2

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No. ① PEDESTRIAN BRIDGE - Superstructure

- 1. Calculation of bending moment for U.L.S.
- 1) For middle point of first span \cdots Mu.max = 1355.4 $^{\text{KNm}}$ section b = 360 $^{\text{cm}}$ h = 50 d = 43.0 d' = 7.0

 As = $Y_{25} 29^{\text{NO}}$ = 4.909×29 = 142.361 cm² $x = \frac{0.87 \times 41000 \times 142.361}{0.40 \times 3000 \times 360} = 11.8^{\text{cm}}$ $Z = 43.0 \frac{11.8}{2} = 37.1^{\text{cm}} < 0.95 \times 43.0 = 40.8^{\text{cm}} \text{ OK}$ $M_{\text{RS}} = 0.87 \times 41000 \times 142.361 \times 37.1 \times 10^{-5} = 1884.0^{\text{KNm}} > 1355.4^{\text{KNm}}$

 $M_{RC} = 0.40 \times 3000 \times 360 \times 11.8 \times 37.1 \times 10^{-5} = 1891.2^{KNm} > 1355.4^{KNm}$ OK

2) For middle fulcrum ... Mu.min = -2981.7^{KNm} section b = 360^{cm} h = 62.5 d= 56.0 d' = 6.5As = $Y_{32} - 29^{\text{NO}}$ = 8.042×29 = 233.218 cm² $x = \frac{0.87 \times 41000 \times 233.218}{0.40 \times 3000 \times 360}$ = 19.4^{cm} $Z = 56.0 - \frac{19.4}{2} = 46.3^{\text{cm}} < 0.95 \times 56.0 = 53.2^{\text{cm}}$ OK $M_{\text{RS}} = 0.87 \times 41000 \times 233.218 \times 46.3 \times 10^{-5} = 3851.6^{\text{KNm}} > M=2981.7^{\text{KNm}}$

 $M_{RC} = 0.40 \times 3000 \times 360 \times 19.4 \times 46.3 \times 10^{-5} = 3880.3^{KNm} > M = 2981.7^{KNm}$ OK

- Calculation of bending moment for S.L.S.
- 1) For middle point of first span ... Ms. max = 929.3 KNM

$$b = 360^{cm}$$
 $h = 50$ $d = 43.0$ $d' = 7.0$

$$h = 50$$

$$1 = 43.0$$

$$1' = 7.0$$

$$As = Y_{25} - 29^{NO} = 4.909 \times 29 = 142.361 \text{ cm}^2$$

$$x = \frac{2 \times 0.80 \times 41000 \times 142.361}{0.50 \times 3000 \times 360} = 17.4^{\text{cm}}$$

$$Z = 43.0 - \frac{17.4}{3} = 37.2^{\text{cm}}$$

$$M_{RS} = 0.80 \times 41000 \times 142.361 \times 37.2 \times 10^{-5} = 1737.0^{KNm} > M = 929.3^{KNm}$$

$$M_{Rc} = \frac{1}{2} \times 0.50 \times 3000 \times 360 \times 17.4 \times 37.2 \times 10^{-5}$$

$$=1738.3^{\text{KNm}} > M = 929.3^{\text{KNm}} \text{ OK}$$

2) For middle fulcrum \cdots Ms.min = -2105.6 KNM

$$b = 360$$
 cm

$$b = 360^{cm} \quad h = 62.5$$

$$d = 56.0$$

$$d = 56.0$$
 $d' = 6.5$

$$As = Y_{32} - 29^{NO} = 8.042 \times 29 = 233.218 \text{ cm}^2$$

$$\chi = \frac{2 \times 0.80 \times 41000 \times 233.218}{0.50 \times 3000 \times 360} = 28.5^{\text{cm}}$$

$$Z = 56.0 - \frac{28.5}{3} = 46.5^{\text{cm}}$$

$$M_{\,\text{R}\,\text{S}} = 0.\,80 \times 41000 \ \times 233.\,218 \ \times 46.\,5 \times 10^{-5} = 3557.\,0^{\,\text{K}\,\text{Nm}} \ > \ M = 2105.\,6^{\,\text{K}\,\text{Nm}}$$

$$M_{RC} = \frac{1}{2} \times 0.50 \times 3000 \times 360 \times 28.5 \times 46.5 \times 10^{-5}$$

$$=3578.2^{\text{knm}} > M=2105.6^{\text{knm}} \text{ OK}$$

Note: U.L.S is critical state than S.L.S.

No. ① PEDESTRIAN BRIDGE -Superstructure

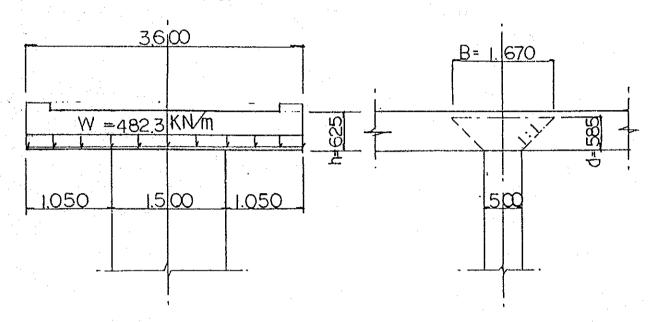
- 3. Calculation of shearing force for U.L.S.
- 1) For first fulcrum Su.max = 707.9^{KNM} section b = 360^{cm} h = 50 d = 43.0 d' = 7.0As = $Y_{25} - 29^{\text{NO}}$ = 4.909×29 = 142.361 cm^2 $P = \frac{142.361}{360 \times 43.0} \times 100 = 0.920 \%$ $Vc = \frac{707.9 \times 10^3}{360 \times 43.0} = 45.8 \text{ N/cm}^2$ $< Vca = 55.0 + 15.0 \left(\frac{0.920 - 0.500}{0.500} \right) = 67.6 \text{ N/cm}^2$
 - 2) For second fulcrum Su.max= 869.1 KN section b = 360^{cm} h = 62.5 d= 56.0 d'=6.5 As = $Y_{32} 29^{\text{NO}}$ = 8.042×29 = 233.218 cm² $P = \frac{233.218}{360 \times 56.0} \times 100 = 1.157 \%$ $Vc = \frac{869.1 \times 10^{3}}{360 \times 56.0} = 43.1 \text{ N/cm}^{2}$

$$< V ca = 70.0 + 20.0 \left(\frac{1.157 - 1.00}{1.00} \right) = 73.1 \text{ N/cm}^2$$

No. ① PEDESTRIAN BRIDGE -Superstructure

- 1. Calculation of pedestal for U.S.L.
 - 1) For middle fulcrum
 - a) Reaction of middle fulcrum $Rmax = 1736.3^{KN}$

b) load ···
$$\omega = \frac{R}{b} = \frac{1736.3}{3.60} = 482.3 \text{ KN/m}$$



c) bending moment and shearing force.

$$M = \frac{1}{2} \times 482.3 \times 1.050^{2} = 265.9^{\text{ KNm}}$$

$$S = 482.3 \times 1.050 = 506.5^{\text{ KN}}$$

d) For bending moment

As =
$$Y_{20} - 150^{\text{ctc}} (11^{\text{NO}}) = 3.1416 \times 11 = 34.558 \text{ cm}^2$$

b = 167.0^{cm} h = 62.5 d = 58.5 d' = 4.0

$$X = \frac{0.87 \times 41000 \times 34.558}{0.40 \times 3000 \times 167.0} = 6.2^{\text{cm}}$$

$$Z = 58.5 - \frac{6.2}{2} = 55.4^{\text{cm}} < 0.95 \times 58.5 = 55.6^{\text{cm}}$$

 $M_{RS} = 0.87 \times 41000 \times 34.558 \times 55.4 \times 10^{-5} = 682.9^{KNm} > M = 265.9^{KNm}$ $M_{RC} = 0.40 \times 3000 \times 167.0 \times 6.2 \times 55.4 \times 10^{-5} = 688.3^{KNm} > M = 265.9^{KNm}$ 1 - 27

e) For shearing force

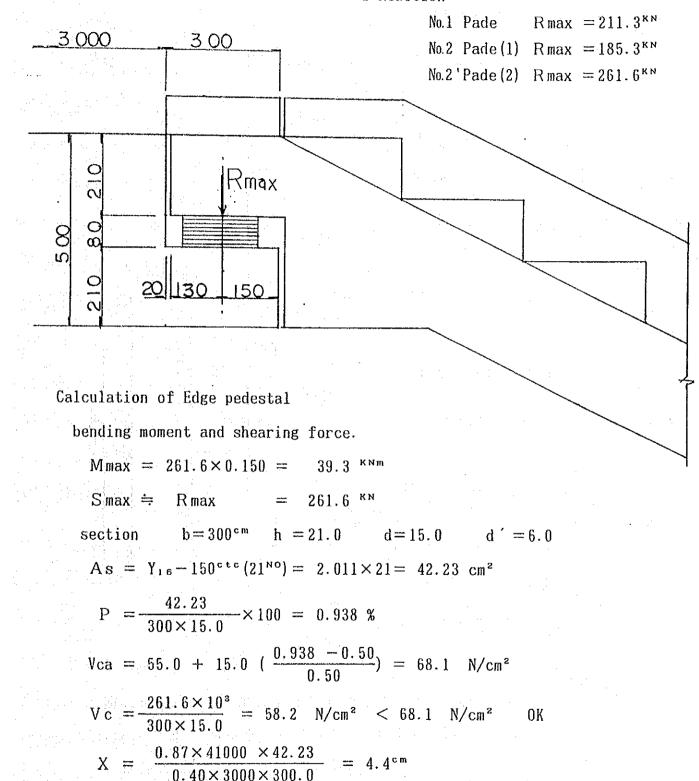
$$Vc = \frac{506.5 \times 10^{3}}{2 \times 167.0 \times 58.5} = 30.0 \text{ N/cm}^{2}$$

$$P = \frac{34.558}{167.0 \times 58.5} \times 100 = 0.353 \%$$

$$< V ca = 35.0 + 20.0 \left(\frac{0.353 - 0.25}{0.25} \right) = 43.2 \text{ N/cm}^2$$
 OK

1. Calculation of pedestal of joint for U.L.S.

1. Reaction



$$Z = 15.0 - \frac{4.4}{2} = 12.8^{\text{cm}} < 0.95 \times 15.0 = 14.3^{\text{cm}}$$
 OK
 $M_{\text{RS}} = 0.87 \times 41000 \times 42.23 \times 12.8 \times 10^{-5} = 192.8^{\text{KNm}} > M = 39.3^{\text{KNm}}$

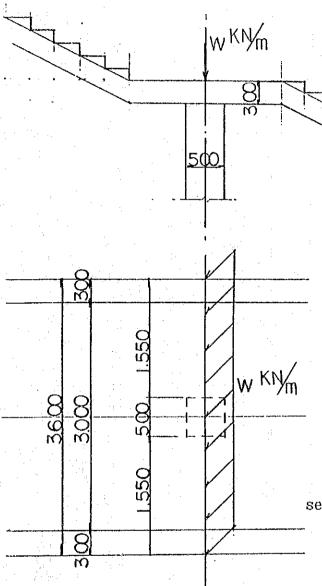
$$M_{RC} = 0.40 \times 3000 \times 300 \times 4.4 \times 12.8 \times 10^{-5} = 202.7^{KNm} > M = 39.3^{KNm}$$

$$M_{RC} = 0.40 \times 3000 \times 300 \times 4.4 \times 12.8 \times 10^{-5} = 202.7^{KNm} > M = 39.3^{KNm}$$

Reaction

No.1) Pedestrian-Rmax=709.1 KN

No.② Pedestrian-Rmax=677.1KN



load and bending moment

$$\omega = \frac{709.1}{3.60} = 197.0 \text{ KN/m}$$

$$Su = 197.0 \times 1.55 = 305.4^{KN}$$

$$Mu = \frac{1}{2} \times 197.0 \times 1.55^2 = 236.7 \text{ KNm}$$

section
$$h=30.0$$
 $d=24.5$ $d'=5.5$

$$b = 50.0 + 2 \times 24.5 = 99.0$$
 cm

$$As = Y_{25} - 8(150^{ctc}) = 39.27 \text{ cm}^2$$

$$\chi = \frac{0.87 \times 41000 \times 39.27}{0.40 \times 3000 \times 99.0} = 12.0^{\text{cm}}$$

$$Z = 24.5 - \frac{12.0}{2} = 18.5^{\text{cm}} < 0.95 \times 24.5 = 23.3^{\text{cm}}$$

$$M_{RS} = 0.87 \times 41000 \times 39.27 \times 18.5 \times 10^{-5} = 259.1^{KNm} > Mu = 236.7^{KNm}$$

$$M_{RC} = 0.40 \times 3000 \times 99 \times 12.0 \times 18.5 \times 10^{-5} = 263.7^{KNm} > Mu = 236.7^{KNm}$$

$$P = \frac{39.27}{99.0 \times 24.5} \times 100 = 1.619 \%$$

$$Vc = \frac{305.4 \times 10^3}{99.0 \times 24.5} = 126.0 \text{ N/cm}^2$$

$$< V ca = 70.0 + 20.0 \left(\frac{1.619 - 1.0}{1.0} \right) \times 2 = 164.8 \text{ N/cm}^2 \text{ OK}$$

Calculation of Shoe

Girderedge and Parapet face of abutment 1) quantity of expantion between.

```
for temperature : dt = a \times T \times L = (.0 \times 10^5 \times 15.0 \times L)^{11} = (0.150 \times L)^{11111}
for shrinkage : ds = a \times T \times L \times D = (.0 \times 10^5 \times 20.0 \times L \times 0.8) = (0.160 \times L)^{11}
for creep : dc = \frac{P}{E \times A} \times \Phi \times L \times D = \frac{750}{27 \times 10^6} \times 1.9 \times L \times 0.8 = (0.430 \times L)^{11111}
quantity of expantion or shrinkase (maximum)
                                                                                                                                                                                                                                                                                                                                               for other
total
```

```
where a - coefficient of thermal expantion or shrinkage
T - quantity of temperature variance
L - girder length
b - coefficient of decrease
                                                                                                                                                                                                       fou =strength of concrete (30 Minm²)
                                                                                                                E = young's modulus
P/A = 0.5 fcu/2 - 0.5 x 300/2
                                                                                                                                                                               - creep.tactor
```

Pedestrian bridge calculation of shoe

No@ Pedestrian, bridge

$$Rmax = 144.0 KN$$

$$dL = (0.80L + 5) mm$$

$$= (0.80 \times 17.30 + 5)$$

$$R = \frac{144.0}{2}$$

=
$$72.0 \text{ KN/shoe}$$

No② Pedestrian, bridge

$$dL = (0.80L + 5) mm$$

$$= (0.80 \times 22.80 + 5)$$

$$R = \frac{126.0}{2} = 63.0 \text{ KN}^{\text{shoe}}$$

No② Pedestrian, bridge

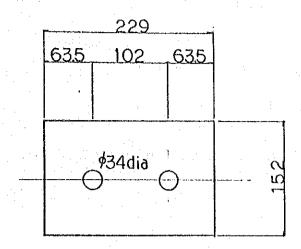
stair(2)

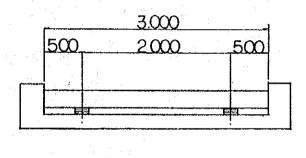
$$Rmax = 206.4 KN (Rd=156.1KN+Rd=50.3KN)$$

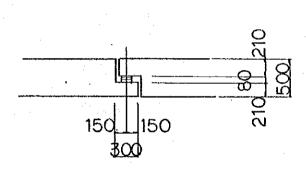
$$dL = (0.80L + 5) mm$$

$$= (0.80 \times 23.80 + 5)$$

$$R = \frac{206.4}{100}$$







this case is calculate for stair (2)

Dowel bar

Hd =
$$156.1 \times 0.15$$
 = 23.5 KN/shoe ---- temperature

anchor bar As = ϕ 20mm × 2NO × 400mm = $\frac{\pi}{4}$ × 2.0² × 2 = 6.283 cm² shearing stress

$$\tau s = \frac{1.43 \text{ Hd}}{\text{As}} = \frac{1.43 \times 23.5 \times 10^3}{6.283} = 54.00 \text{ N/cm}^2 < 9000 \text{ N/cm}^2 \text{ OK}$$

anchor cap ϕ (20+30+30) × 160 mm × 2 = ϕ 80 × 160 mm × 2NO/ shoe vertical pressure:

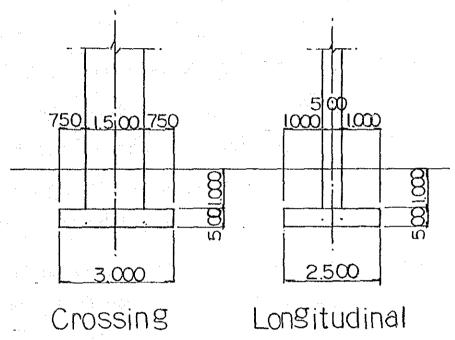
As = 22.9 × 15.2 -
$$\frac{\pi}{4}$$
 × 3.4² × 2 = 330 cm²
· VC = $\frac{R max}{As}$ = $\frac{104 \times 10^3}{330}$ = 320 N/cm² < Vca = 800 N/cm²

No. (1) PEDESTRIAN BRIDGE - Substructure

Calculation of stability for Longitudinal direction.

1. Footing and surcharge

1) Shape and size of foundation



2) load of foundation

Footing =
$$23.6 \times 3.00 \times 2.50 \times 0.50$$
 = 88.5^{KN}
Surcharge = $18.6 \times 3.00 \times 2.50 \times 1.00$ = 139.5^{KN}
Total = 228.0^{KN}

No. ① PEDESTRIAN BRIDGE - Substructure

- 2. Calculation of stability of foundation for S.L.S.
 - 1) action force for bottom slab from case-13
 - a) For pillar @~ (pillar @~ 2)

$$N = 635.2 + 228.0 = 863.2^{KR}$$

$$H = 172.4 = 172.4^{KN}$$

$$M = 463.5 + 172.4 + 0.50 = 549.7$$
^{KNm}

b) For pillar 9~4

$$N = 1069.6 + 228.0 = 1297.6^{KN}$$

$$H = 60.6$$
 = 60.6 KN

$$M = 216.6 + 60.6 + 0.50 = 246.9^{KNm}$$

- 2) stability for foundation
- a) For pillar ®~6

$$e = \frac{M}{N} = \frac{549.7}{863.2} = 0.637^{cm}$$

$$x = \frac{B}{2} - e = \frac{2.50}{2} - 0.637 = 0.613$$
 m

$$q_{\text{max}} = \frac{2 \cdot N}{3 \cdot \chi \cdot L} = \frac{2 \times 863.2}{3 \times 0.613 \times 3.00} = 313.0 \text{ KN/m}^2 < qa=350.0 \text{ KN/m}^2 \text{ OK}$$

$$Fs = \frac{863.2 \times 0.50}{172.4} = 2.5 > 1.5$$
 OK

b) For pillar⊕∼④

$$e = \frac{M}{N} = \frac{246.9}{1297.6} = 0.191$$
°m

$$q = \frac{N}{B \cdot L}$$
 $(1 \pm \frac{6e}{B}) = \frac{1297.6}{2.50 \times 3.00} (1 \pm \frac{6 \times 0.191}{2.50})$

$$= \left(\frac{252.3 \text{ KN/m}^2}{93.7 \text{ KN/m}^2} < qa=350.0 \text{ KN/m}^2 \right) \text{ OK}$$

Fs =
$$\frac{N \cdot \mu}{H} = \frac{1297.6 \times 0.50}{60.6} = 10.7 > 1.5$$
 OK

- 3. Calculation of stability of foundation for U.L.S.
 - 1) action force for bottom slab from case-9
 - a) For pillar $^{\circ}$ (pillar $^{\circ}$ $^{\circ}$)

$$N = 880.7 + 228.0 \times 1.380 = 1195.4^{KN}$$

$$H = 255.8 = 255.8^{KN}$$

$$M = 704.6 + 255.8 + 0.50 = 832.5^{KNm}$$

b) For pillar 9~4

$$N = 1475.7 + 228.0 \times 1.380 = 1790.4^{KN}$$

$$H = 100.9 = 100.9^{KI}$$

$$M = 361.3 + 100.9 + 0.50 = 411.8^{KNm}$$

- 2) stability for foundation
 - a) For pillar ®~6

$$e = \frac{M}{N} = \frac{832.5}{1195.4} = 0.697^{cm}$$

$$\chi = \frac{B}{2} - e = \frac{2.50}{2} - 0.697 = 0.553$$
 m

$$q_{\text{max}} = \frac{2N}{3 \cdot \chi \cdot L} = \frac{2 \times 1195.4}{3 \times 0.553 \times 3.00} = 480.4 \text{ KN/m}^2 < qa=525.0 \text{ KN/m}^2$$
 OK

Fs =
$$\frac{N \cdot \mu}{H} = \frac{1195.4 \times 0.50}{255.8} = 2.3 > 1.1$$

b) For pillar 9~4

$$e = \frac{M}{N} = \frac{411.8}{1790.4} = 0.231^{cm}$$

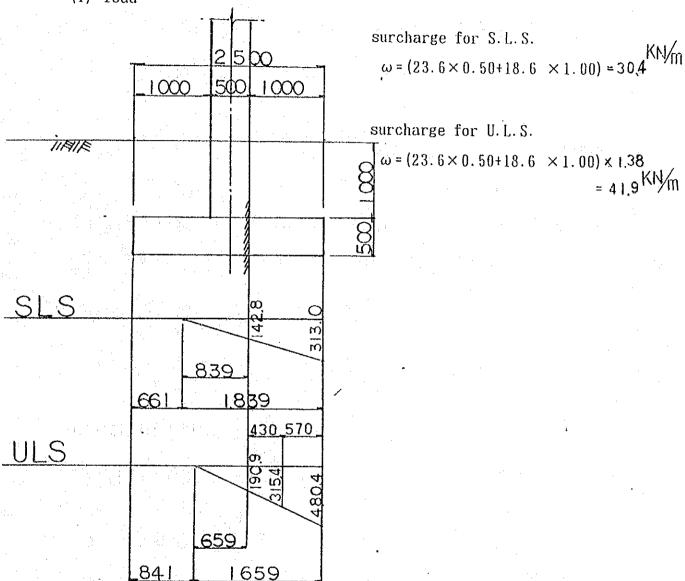
$$q = \frac{N}{BL} (1 \pm \frac{6e}{B}) = \frac{1790.4}{2.50 \times 3.00} (1 \pm \frac{6 \times 0.231}{2.50})$$

$$= \begin{pmatrix} 371.1 & \text{KN/m}^2 \\ 106.4 & \text{KN/m}^2 \end{pmatrix} = 525.0 & \text{KN/m}^2$$
 OK

$$Fs = \frac{N \cdot \mu}{H} = \frac{1790.4 \times 0.50}{100.9} = 8.8 > 1.1$$

Calculation of stress for Longitudinal direction

- 1. For pillar ®~© (pillar ®~②)
 - (1) load



- (2) bending moment and shearing force.
 - a) for S.L.S.

$$M = \left[\frac{1.00^{2}}{6}(2 \times 313.0 + 142.8) - \frac{1.00^{2}}{2} \times 30.4\right] \times 3.00 = 338.8 \text{ KNm}$$

$$S = \left(\frac{1.00}{2}(313.0 + 142.8) - 1.00 \times 30.4\right) \times 3.00 = 592.5 \text{ KN}$$

a) for U.L.S.

$$M = \left(\frac{1.00^{2}}{6}(2 \times 480.4 + 190.9) - \frac{1.00^{2}}{2} \times 41.9\right) \times 3.00 = 513.0 \text{ KNm}$$

$$S = \left(\frac{1.00}{2}(480.4 + 190.9) - 1.00 \times 41.9\right) \times 3.00 = 881.3 \text{ KN}$$

$$1 - 37$$

(3) Calculation of stress for U.L.S.

section b=15.00+ 2×43.0=236.0°m d=43.0 d'=7.0

As =
$$Y_{20}-160^{\text{ctc}}(15^{\text{NO}}) = 3.142 \times 15 = 47.13$$
 cm²

$$P = \frac{47.13}{236.0 \times 43.0} \times 100 = 0.464.\% > 0.15 \text{ cm²}$$

$$X = \frac{0.87 \times 41000 \times 47.13}{0.40 \times 2500 \times 236.0} = 7.2 \text{ cm}$$

$$Z = 43.0 - \frac{7.2}{2} = 39.4 \text{ cm} < 0.95 \times 43.0 = 40.8 \text{ cm}$$

$$M_{\text{RS}} = 0.87 \times 41000 \times 47.13 \times 39.4 \times 10^{-5} = 662.4 \text{KNM} > M = 513.0$$

$$M_{\text{RC}} = 0.40 \times 2500 \times 236.0 \times 7.2 \times 39.4 \times 10^{-5} = 669.5 \% > \%$$

$$V_{\text{C}} = \frac{881.3 \times 10^3}{2360 \times 43.0} = .86.9 \text{ cm²}$$

$$V_{\text{C}} = \frac{881.3 \times 10^3}{2360 \times 43.0} = .86.9 \text{ cm²}$$

$$V_{\text{C}} = \frac{831.3 \times 10^3}{2360 \times 43.0} = .86.9 \text{ cm²}$$

Check of Critical Section

$$S = \left(\frac{0.57}{2} \left(480.4 + 315.4\right) - 0.57 \times 41.9\right) \times 3.00 = 608.8^{\text{KN}}$$

$$V_{\text{C}} = \frac{608.8 \times 10^{3}}{236.0 \times 43.0} = 60.0 \text{ V/cm}^{2}$$

$$V_{\text{C}} = \left(35.0 + 15.0\left(\frac{0.464 - 0.25}{0.25}\right)\right) \frac{2 \times 43.0}{50.0} = 82.3 \text{ V/cm}^{2} \text{ OK}$$

No.① PEDESTRIAN BRIDGE -Substructure

Calculation for pillar for Longitudinal direction.

$$\begin{array}{l} M=957.8^{\text{KMm}} \\ H=201.9^{\text{KN}} \\ N=886.1^{\text{KN}} \\ \end{array} \end{array} \end{array} \end{array} = \begin{array}{l} \text{for pillar } \textcircled{2} \sim \textcircled{\$} \text{ from case-6 (U.L.S)} \\ N=886.1^{\text{KN}} \\ \end{array} = \begin{array}{l} \text{for pillar } \textcircled{2} \sim \textcircled{\$} \text{ from case-6 (U.L.S)} \\ \end{array} = \begin{array}{l} \text{section} \\ \text{b} = 150^{\text{cm}} \\ \text{h} = 50 \\ \end{array} = \begin{array}{l} \text{d} = 44.5 \\ \text{d}' = 5.5 \\ \end{array} = \begin{array}{l} \text{Ma} = \text{M} + \text{N (d} - \frac{\text{h}}{2}) \\ = 957.8 + 886.1 (0.445 - \frac{0.50}{2}) \\ \text{2} = 1130.6^{\text{KNM}} \\ \end{array} = \begin{array}{l} \text{As} = \text{As'} = \text{Y}_{32} - 10^{\text{NO}} = 8.042 \times 10 \\ \text{2} = 80.42 \text{ cm}^2 \\ \end{array} = \begin{array}{l} \text{2} \\ \text{2} = \frac{(0.87 - 0.72) \times 41000 \times 80.42}{0.40 \times 2500 \times 150} \\ \Rightarrow 4.4^{\text{cm}} \\ \end{array} = \begin{array}{l} \text{2} \\ \text{2} = \frac{(0.87 - 0.72) \times 41000 \times 80.42}{0.40 \times 2500 \times 150} \\ \Rightarrow 4.4^{\text{cm}} \\ \end{array} = \begin{array}{l} \text{2} \\ \text{2} = 44.5 - \frac{4.4}{2} \\ = 42.3^{\text{cm}} \\ \Rightarrow 0.95 \times 44.5 = 42.3^{\text{cm}} \\ \text{OK} \\ \end{array} = \begin{array}{l} \text{Ma} = 1130.6^{\text{KNm}} \\ \text{Ma} = 1130.6^{\text{KNm}} \\ \text{Ma} = 1130.6^{\text{KNm}} \\ \text{Ma} = (0.72 \times 41000 \times 80.42 \times 39.0 + 0.40 \times 2500 \times 150 \times 4.4 \times 42.3) \\ \times 10^{-5} = 1205.0^{\text{KNm}} > \text{Ma} = 1130.6^{\text{KNm}} \\ \text{Asn} = \text{A'sn} = \text{As} - \frac{\text{N}}{0.87 \text{fy}} = 80.42 - \frac{886.1 \times 10^3}{0.87 \times 41000} \\ \Rightarrow 54.0 \text{ cm}^2 \\ \text{2} \leq \text{Asu} = \text{A'su} = \text{Y}_{25} - 11^{\text{NO}} = 54.0 \text{ cm}^2 \\ \text{OK} \\ \text{P} = \frac{54.0}{150 \times 44.5} \times 100 = 0.809 \% \\ \text{Vc} = \frac{201.9 \times 10^3}{150 \times 44.5} = 30.3 \text{ N/cm}^2 \\ \text{3} = 30.3 \text{ N/cm}^2 \\ \text{3} = 59.3 \text{ N/cm}^2 \\ \text{3} = 59.3 \text{ N/cm}^2 \\ \text{3} = \frac{201.9 \times 10^3}{150 \times 44.5} = 30.3 \text{ N/cm}^2 \\ \text{3} = \frac{201.9 \times 10^3}{150 \times 44.5} = 30.3 \text{ N/cm}^2 \\ \text{3} = \frac{201.9 \times 10^3}{150 \times 44.5} = 30.3 \text{ N/cm}^2 \\ \text{3} = \frac{201.9 \times 10^3}{150 \times 44.5} = 30.3 \text{ N/cm}^2 \\ \text{3} = \frac{201.9 \times 10^3}{150 \times 44.5} = 30.3 \text{ N/cm}^2 \\ \text{3} = \frac{201.9 \times 10^3}{150 \times 44.5} = \frac{20.00 \times 10^3}{150 \times 10^3} = \frac{20.00 \times 10^$$

Note: U.L.S is critical stress than S.L.S.

section
$$b = 150^{cm} h = 50$$
 $d = 44.5$ $d' = 5.5$
 $Ma = M + N (d - \frac{h}{2}) = 658.1 + 619.8 (0.445 - \frac{0.50}{2}) = 779.0^{KNM}$
 $As = As' = Y_{32} - 8^{NO} = 8.042 \times 8 = 64.336 \text{ cm}^2$
 $x = \frac{(0.80 - 0.72) \times 41000 \times 64.336}{\frac{1}{2} \times 0.50 \times 2500 \times 150} = 4.4^{cm}$
 $Z = 44.5 - \frac{4.4}{2} = 42.3^{cm} = 0.95 \times 44.5 = 42.3 \text{ cm}^2$ OK

 $M_{RS} = 0.80 \times 41000 \times 64.336 \times 42.3 \times 10^{-5} = 892.6^{KNm} > Ma = 779.0^{KNm}$
 $M_{RC} = (0.72 \times 41000 \times 64.336 \times 39.0 + \frac{1}{2} \times 0.50 \times 2500 \times 150 \times 44.4 \times 42.3) \times 10^{-6} = 915.2^{KNm} > Ma = 779.0^{KNm}$ OK

 $Asn = A'sn = 64.336 - \frac{619.8 \times 10^3}{0.80 \times 41000} = 45.5 \text{ cm}^2$

 $A su = A'su = Y_{25} - 11^{NO} = 54.0 \text{ cm}^2$

Calculation for bottom of pillar for Crossing direction. (U.L.S)

action force and stress.

$$M = (67.6 \times 7.75 + 10.7 \times 3.75) \times 1.650 = 930.7^{KNR}$$

$$H = (67.6 + 10.7) \times 1.650 = 129.2^{KN}$$

$$N = (938.5 + 132.8) \times 1.380 = 1478.4^{KN}$$

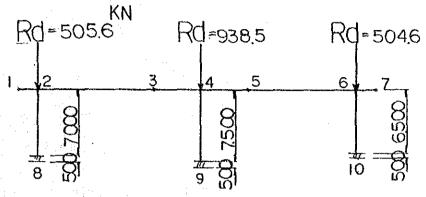
section
$$b = 50^{cm}$$
 $h = 150$ $d = 143.0$ $d' = 7.0$
 $Ma = 930.7 + 1478.4 (1.430 - \frac{1.50}{2}) = 1936.0^{KNm}$
 $As = As' = Y_{32} - 6^{NO} = 8.042 \times 6 = 48.252 \text{ cm}^2$
 $x = \frac{(0.87 - 0.72) \times 41000 \times 48.252}{0.40 \times 2500 \times 50.0} = 14.4^{cm}$
 $Z = 143.0 - \frac{14.4}{2} = 135.8^{cm} = 0.95 \times 143.0 = 135.8^{cm}$ OK

$$M_{RS} = 0.87 \times 41000 \times 48.252 \times 135.8 \times 10^{-5} = 2337.3^{\text{KNm}} > \text{Ma} = 1936.0^{\text{KNm}}$$
 $M_{RC} = (0.40 \times 2500 \times 50.0 \times 15.0 \times 135.8 + 0.72 \times 41000 \times 48.252 \times 136.0) \times 10^{-6} = 2955.6^{\text{KNm}} > \text{Ma} = 1936.0^{\text{KNm}}$

Asn=A'sn=48.252
$$-\frac{1478.4 \times 10^3}{0.87 \times 41000}$$
 = 6.9 cm²

$$<$$
 Asu=A'su = $Y_{25}-4^{NO}$ = 19.636 cm² OK

- 1. Calculation of stability of Crossing direction
 - 1) Shape and Reaction



- 2) Action force and stability
 - a) For bottom slab for S.L.S.

action force for pillar 9~4 (seismic state)

	Nĸn	Н ки	У ^m	$M = H \cdot y^{RNm}$
Reaction from superstructure	938.5	67.6	8. 250	557.7
Pillar	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	10.7	4.250	45.5
Footing	228.0			- .
Total	1299.3	78.3		603.2

Stability

$$e = \frac{M}{N} = \frac{603.2}{1299.3} = 0.465^{m} < \frac{B}{6} = \frac{3.00}{6} = 0.500^{m}$$

$$q = \frac{N}{BL} (1 \pm \frac{6e}{B}) = \frac{1299.3}{3.00 \times 2.50} (1 \pm \frac{6 \times 0.465}{3.00})$$

$$= {334.4 \text{ KN/m}^{2} \over 12.2 \text{ KN/m}^{2}} < qa = 350 \text{ KN/m}^{2}$$

$$F_s = \frac{N \cdot \mu}{H} = \frac{1299.3 \times 0.50}{78.3} = 8.3 > 1.5$$

b) For bottom slab for U.L.S.

action force for pillar 9~4 (seismic state)

$$N = 1299.3 \times 1.38 = 1793.1^{KN}$$
 $H = 78.3 \times 1.65 = 129.2^{KN}$

$$M = 603.2 \times 1.65 = 995.3^{KNm}$$

Stability

$$e = \frac{M}{N} = \frac{995.3}{1793.1} = 0.555^{m}$$

$$\chi = \frac{B}{2} - e = \frac{3.00}{2} - 0.555 = 0.945^{m}$$

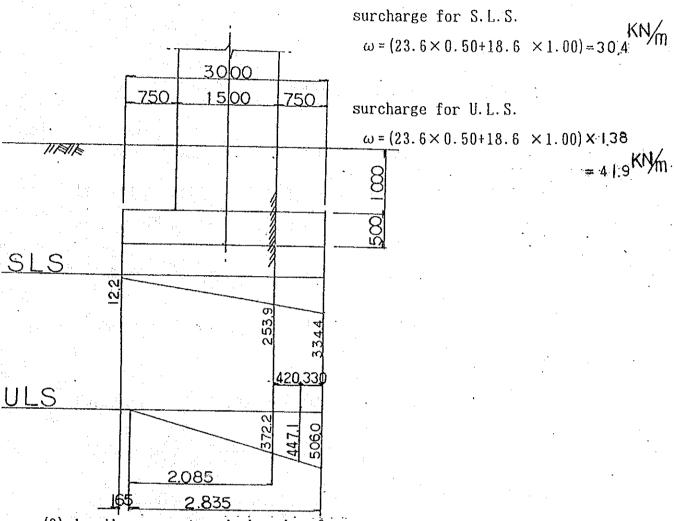
$$q_{\text{max}} = \frac{2 \cdot N}{3 \cdot \chi \cdot L} = \frac{2 \times 1793.1}{3 \times 0.945 \times 2.50} = 506.0 \text{ KN/m}^{2} < qa=525 \text{ KN/m}^{2}$$

$$Fs = \frac{N \cdot \mu}{H} = \frac{1793.1 \times 0.50}{129.2} = 6.9 > 1.1$$

Calculation of stress for Crossing direction.

1. For pillar 9~4

(1) load



(2) bending moment and shearing force.

a) for S.L.S.

$$M = \left(\frac{0.75^{2}}{6}(2 \times 334.4 + 253.9) - \frac{0.75^{2}}{2} \times 30.4\right) \times 250 = 194.9 \text{ KNm}$$

$$S = \left(\frac{0.75}{2}(334.4 + 253.9) - 0.75 \times 30.4\right) \times 250 = 494.6 \text{ KN}$$

a) for U.L.S.

$$M = \left[\frac{0.75^{2}}{6}(2 \times 506.0 + 372.2) - \frac{0.75^{2}}{2} \times 41.9\right] \times 2.50 = 2950^{\text{KNm}}$$

$$S = \left[\frac{0.75}{2}(506.0 + 372.2) - 0.75 \times 41.9\right] \times 2.50 = 744.8^{\text{KN}}$$

$$-Sc = \left[\frac{0.33}{2}(506.0 + 447.1) - 0.33 \times 41.9\right] \times 2.50 = 358.6^{\text{KN}}$$

(3) Calculation of stress for U.L.S.

section
$$b = 50.0 + 2 \times 42.0 = 134.0^{\circ m}$$
 $h = 50$ $d = 42.0$ $d' = 8.0$ As $= Y_{20} - 130^{\circ t \circ} (11^{No}) = 3.142 \times 11 = 34.562$ cm^2

$$P = \frac{34.562}{134.0 \times 42.0} \times 100 = 0.614 \% > 0.15 \%$$

$$X = \frac{0.87 \times 41000 \times 34.562}{0.40 \times 2500 \times 134.0} = 9.2^{\circ m}$$

$$Z = 42.0 - \frac{9.2}{2} = 37.4^{\circ m} < 0.95 \times 42.0 = 39.9^{\circ m}$$

$$M_{RS} = 0.87 \times 41000 \times 34.562 \times 37.4 \times 10^{-5} = 461.0 \times 10^{-5} = 461.1 \times 10^{-$$

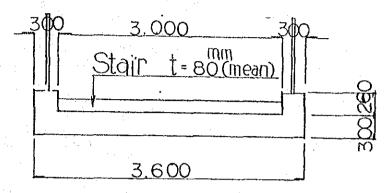
Check of Critical Section

$$V_{C} = \frac{358.6 \times 10^{3}}{134.0 \times 42.0} = 63.7 \text{ M/cm}^{2}$$

$$< V_{CQ} = \left[50.0 + 15.0 \left(\frac{0.614 - 0.50}{0.50}\right)\right] \times \frac{2 \times 42.0}{375} = 119.6 \text{ N/cm}^{2} \text{ OK}$$

No. ① PEDESTRIAN BRIDGE OF STAIRCASE

- 1) Shape and factor for Calculation of stress.
 - (1) Superstructure



$$A = 3.60 \times 0.30 = 1.080 \text{ m}^2$$

$$I = \frac{3.60 \times 0.30^3}{12} = 0.00810 \text{ m}^4$$

$$E c_1 = 2.7 \text{ KN/mm}^2 = 2.7 \times 10^7 \text{ KN/m}^3 \text{ (fcu=30 N/mm}^2)$$

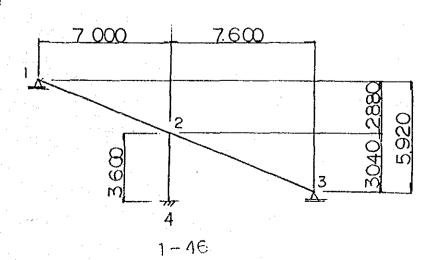
(2) Substructure

$$A = 0.50 \times 0.50 = 0.250 \text{ m}^2$$

$$I = \frac{0.50^4}{12} = 0.00521 \text{ m}^4$$

$$E c_1 = 2.5 \text{ KN/mm}^2 = 2.5 \times 10^7 \text{ KN/m}^3 \text{ (fcu=25 N/mm}^2)$$

(3) Frame



2) Load

(1) Dead load

parapet =
$$(23.6 \times 0.30 \times 0.26 + 0.294) \times 2 = 4.270^{\text{KN/m}}$$

slab = 23.6×1.080 = $25.488^{\text{KN/m}}$
stair = $22.6 \times 3.00 \times 0.08$ = $5.424^{\text{KN/m}}$
 ωd_1 = $35.182^{\text{KN/m}}$
pillar ω_2 = 23.6×0.250 = $5.900^{\text{KN/m}}$

(2) Live load

loaded length < 30.0^m

$$\omega \ell = 5.0 (2.00 + 1.00 \times 0.85)$$
 = 14.250 KN/m

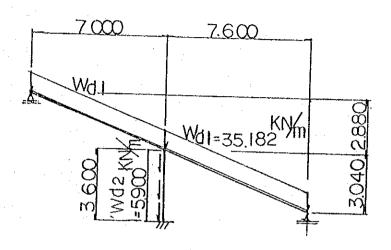
(3) Other load

seismic

Coefficient of seismic \cdots $k_H = 0.100$

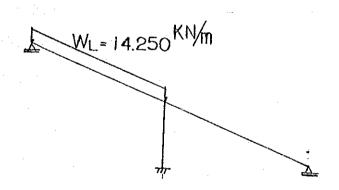
3) Loaded figure

(1) Dead load ... case-1

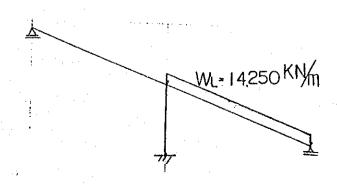


(2) Live load

a) for first span ··· case-2

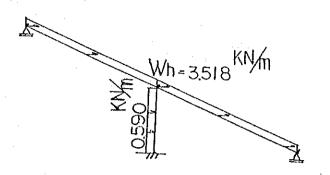


b) for second span \cdots case-3



3) seismic

··· case-4



NOTE: THE DIMENSION(t) BE EXCHANG TO DIMENSION(KN) INTO THIS CALCULATION

No	X	V
	(m)	Y (m)
1	0.0000	10.0000
. 2	7.0000	7.1200
3	14,6000	4.0800
4	7.0000	3 5200

PEDE NOI STAIR

No	1	J	A (m2)	I (m4)	1 - J ·	L (m)	E (t/m2)	EPS
2	2 - 3	3	1.08000 1.08000 0.25000	0.008100	Fix - Fix Fix - Fix Fix - Fix	8.185	2.70E+07	

PEDE NOI STAIR

	No. 1	*	
No	X (t/m)	Y (t/m)	M(tm/Rad)
1	Free	Fix	Free
3	Free	Fix	Free
4	Fix	Fix	Fix

PEDE NOI STAIR

No	,	L-No	L-No	L-No	L-No	L-No	L-No	L-No	L-No	L-No	L-No
		1	2	3	4	5	6	7	8	9	10
		11	12	13	14	15	. 16	17	18	19	20
			rain e				_				
- 1	9	0.757	1.514	2.271	3.028	3.785	4.542	5.299	6.055	6.812	
2	9	0.819	1.637	2.456	3.274	4.093	4.911	5.730	6.548	7.367	
3	4	0.720	1.440	2.160	2.880						

PEDE NOI STAIR

No. : Dead load

No Li (m) Lo (m) Pi (t/m) Pj (t/m) 1 2 Y Y Y 0.000 -35.182 -35.182 -5.900 -35.182 -35.182 -5.900 7.569 0.000 8.185 3 0.000 3.600

> $\Sigma V = -575.497 (t)$ $\Sigma II = 0.000 (t)$

PEDE NOI STAIR

: Live load No. : 2 No. : 1

No i -j Li (m) Lo (m) Pi (t/m) Pj (t/m)

1 1- 2 -Y 0.000 7.569 -14.250 -14.250 $\sum V = -107.858 (t)$ $\sum H = 0.000 (t)$

PEDE NOI STAIR

: Live load No. : 3 No. : 1

No i -J Li (m) Lo (m) Pi (t/m) Pj (t/m)

2 2- 3 -Y 0.000 8.185 -14.250 -14.250

 $\Sigma V = -116.636 (t)$ $\Sigma H = 0.000 (t)$

PEDE NOI STAIR

: SEISMIC No. : 4 No. : 1

No	i	- j			Li (m)	Lo (m)	Гi	(t/m)	Рj	(t/m)
1 2 3		l ~ 2 ~ 2 ~		- X - X - X	0.000 0.000 0.000	7.569 8.185 3.600		3.518 3.518 0.590		3.518 3.518 0.590

 $\Sigma V = 0.000 (t)$ $\Sigma H = 57.547 (t)$

PEDE NOI STAIR

	α	No 5	No 6	No 7	No 8	C-No 5 No 9 1.0000	No 10	No 1 1	No 1 2
No	1	1.3800	1.3800	1.3800	1.3800	1.0000	1.0000	1.0000	1.0000
No	2	1.6500	0.0000	1.6500	0.0000	1.0000	0.0000	1.0000	0.0000
	3					0.0000			
No	4	0.0000	0.0000	0.0000	1.3200	0.0000	0.0000	0.0000	0.8000

PEDE NOI STAIR

No 1: 5 6 7 8

No.		Case. 1 RX (t)	RY (t)	RM (tm)	Case. RX (t)	2 RY (t)	RM (tm)	Case. RX (t)	3 RY (t)	RM (tm)	
r:d	. :	0.000	97.732	00000	0.000	46.259	0.000	0.000	-6.688	0.000	
ന		000.0	110.391	000.0	0.000	-4.846	0.000	0.00	49.555	00000	
4		000.0	367.374	-7.297	00000	66.435	16.807	0.000	73.770	-19.762	
		Case. 4			Case.	ശ		Case.	ø		
No.	. :	RX (t)	RY (t)	RM (tm)	RX (t)	11 (t)	RM (tm)	nx (t)	RY (t)	nM (tm)	
		0.000	-10.177	0.000	0.000	211.214	0.000	0.000	123.835	0.000	
ო		0.000	0.870	0.000	000.0	144.344	000.0	000.0	234.105	0.000	
4	: · ·	-57.547	9.307	-120.077	0.000	616.594	17.662	0000	628.696	-42.678	
									·.		
		Case. 7			Case.	κο	:	Case.		- 4	
9		RX (t)	RY (t)	RM (tm)	RX (t)	RY (t)	RM (tm)	RX (t)	RY (t)	KM (tm)	
m		0.000	200.179	00000	000.0	121.437	0.000	0.000	144.003	00000	
С		0.000	226.109	000.0	0.000	153.487	0.000	000.0	105.545	000.0	
4		000.0	738.314	-14.946	-75.961	519:262	-168.571	000.0	433.809	9.510	
					11					:	
٠		Case. 10			Case.			Case.	12		
No	-	RX (t)	RY (t)	RM (tm)	ПХ (t)	NY (t)	RM (tm)	RX (t)	RY (t)	RM (tm)	
m		0.000	91.044	00000	0.000	137.313	0.000	0.000	89.591	0.000	
e5		0.000	159.946	0.000	0000	155.100	000.0	000.0	111.087	0.000	
₹7		0000	441.144	-27.059	000.0	507.579	-10.252	-46.037	374.820	-103.359	

ROTA. (mmRad)	0.2640 -0.5462 0.9728 0.0000	EOTA.(mmRad) -1.3414 -1.1796 4.1557 0.0000	ROTA.(mmRad) -2.0726 0.2629 1.6236 0.0000	ROTA.(mmRad) -1.0481 -0.5946 1.9811
Y-DIS.(mm)	0.00000	Y-DIS.(mm) 0.00000 -0.35369 0.00000	9 Y-DIS.(mm) 0.00000 -0.24376 0.00000	12 Y-DIS.(mm) 0.00000 -0.20978 0.00000
Case. X-DIS.(mm)	1.00137 0.98318 1.00116 0.00000	Case. 2.27516 2.12324 2.27154 0.00000	Case. X-DIS.(mm) -0.36824 -0.47313 -0.37131	Case. X-DIS.(mm) 2.51278 2.41905 2.50985 0.00000
ROTA.(mmRad)	-0.78495 0.46454 -0.22468 0.0000	ROTA. (mmRad) -3.07206 0.48816 2.17986 0.00000	ROTA.(mmRad) -1.38176 -0.92655 2.78634 0.00000	ROTA.(mmRad) -1.80861 -0.28336 2.59636 0.00000
2. Y-DIS.(mm)	0.00000	5 Y-DIS.(mm) 0.00000 -0.34672 0.00000	8 Y-DIS.(mm) 0.00000 -0.29065 0.00000	11 Y-DIS.(mm) 0.00000 -0.28625 0.00000
Case. X-DIS.(mm)	-0.81961 -0.83617 -0.82032 0.00000	Case. X-DIS.(mm) -0.72946 -0.87868 -0.73389	Case. X-DIS.(mm) 4.02421 3.89341 4.02008 0.00000	Case. X-DIS.(mm) 0.63314 0.51005 0.62985
ROTA.(mmRad)	-1.28760 -0.20169 1.84825 0.00000	OTA. (mmRad) 0.29934 -0.49107 0.17860 0.00000	ROTA. (mmRad) -2.63655 -0.41309 3.78497 0.00000	ROTA.(mmRad) -1.02366 -0.74790 2.82104 0.00000
1 Y-DIS.(mm) I	0.00000 0.00000 0.00000	Y-DIS.(mm) ROTA.(mmRad 0.00000 0.29934 -0.00536 -0.49107 0.00000 0.17860	Y-DIS.(mm) 0.00000 -0.41683 0.00000	10 Y-DIS.(mm) 0.00000 -0.24798 0.00000
Case. X-DIS, (mm)	0.45137 0.48901 0.00000	Case. X-DIS.(mm) 2.57676 2.57609 0.00000	Case. X-DIS.(mm) 0.92280 0.74356 0.91802	Case. X-DIS.(mm) 1.45274 1.34622 1.45017 0.00000
STAIR.	- N O ₹	N 4 9 6 4	δ • Η 9 ω 4 • • • • • • • • • • • • • • • • • • •	S - 9.6.4

		• . • .										
	3 *	10 10 10 14 14 44	20.01	12.01 12.01 12.01 13.01 14.01 14.01	0.00	(2.0) (2.0)	4.91	25.	3.23	-1.077	8.40	-73.770 -73.770 -73.770 -73.770
	load S (t)	6.138	900	A GA GA A	66.138	8 6 6	2.28	0 62 9 78	8 8 8 9 5 5 6 1 3 8	0 10 10 0 10 10 0 11 10 10	0.00	00000
	Case 3 Live M (tm)	0.00	14.04	123.410	32.77	42.	6.87	7.65	6.42 7.52 2.23	40.5	800	19.762 19.762 19.762 19.762
	N (‡)	.50	0 0 0 0 0 0	401	11.12	22.	808.	.80	80.89	008. 0008.	888	1 1 1 1 1 1 66 6 6 6 6 6 6 6 6 6 6 6 6
	load S (t)	2.2	22.83	-7.09 -7.09	27.04	6.98	4 4 9	49	0.4	\$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$	244	000000000000000000000000000000000000000
	Case 2 Live M (tm)	0.00	9.67	67.55 67.55 67.55 67.55	7.51	3.63	36.82	29.46 25.77	8.41	114.733	3.68	16.807 16.807 16.807 16.807
	N (E)	1.8	6.91 6.78	1.1.0.00 1.2.0.00 1.2.0.00 1.2.000 1.2.00 1.2.00 1.2.00 1.2.00 1.2.00 1.2.00 1.2.00 1.2.00 1.2.000 1.2.00 1.2.00 1.2.00 1.2.00 1.2.00 1.2.00 1.2.00 1.2.00 1.2.0000 1.2.000 1.0000 1.0000 1.0000 1.0000 1.0000 1.0000 1.0000 1.0000 1.0000 1.0000 1.0000 1.0000 1.0000 1	3.7.8	54.00	5.0 40.0	3.85	.46	~ Ø Ø	931	1 1 3 4 6 5 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
	load S (t)	5.75		-32.767	82.02	131.25	4.87	11.39 84.64	7.92		7.7.	00000
	Case 1 Dead M (tm)	9.09	23.35	109.036	22.13	9.20	55.23	29.10 51.17	09.48 45.97	160.543	72.98	7.297 7.297 7.297 7.297 7.297 7.297 7.297
STAIR	No L(m)	0.00	20.5	5 3.785 5.785 5.785	6.29	6.81	0.00	1.63 2.45	3.27	6 4.911 7 5.730 8 8.730	8 1 8	4 4 6 6 4 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6
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	(;)	7	(n (: :	Ġ	3	2.5	evi (S	0	74	7	100	លើ	ı,	ფ	co.		O)	7	73	¥		7	හ	2	4	৩
		r s	60 P	اسم آيا ه (13 ج	8.8	22.8	36.7	0.7	64.3	8.7	52.7	32.1	0.1	88.30	8,3	4.4	ei ei	ි. ව	21,2	43.2	5.0	86.9	599.3	605.2	-611.10	6.919	622.3	628.6
	\$ (t)	4.52	80.032	5 0	21.43	3.45	-88.43	23.40	157.34	191.33	225.32	30.29	75.49	220.762	65 96	11.23	6.43	1.70	53.09	107.83	2.62	217.36	00	ွ.	0	0	ွ	00.
	ase 6 M (tm)	0	73.82	4, 29	40.53	11.84	37.02	-23.52	29.65	61.62	419.39	462.0	14.00	03	47.32	60.69	29.35	53.13	32.08	66.26	55.51	00.	2.67	2.67	42.678	2.67	2.67	2.67
	N (t)	0.36		18.09	2.66	23.41	44.17	4.03	85.65	06.41	127.17	3.97	9.21	64.462	69.6	4.94	0.17	. 42	9.34	24.09	85	53.60	587.28	593.14	-599.008	604.87	610.73	616.59
	3 (1)	: co :	44.87	3.98	-6.46	6.9	107.36	57.81	208.19	258.64	309.09	34.94	98.02	161.155	24.23	7.35	0.44	3.56	23,35	60.22	7.14	34.02	•		0.000		٠	. •
	Case 5 M (tm)	0.00	128.768	71.73	85.93	61.94	99.76	99.38	38.86	5.66	430.65	412.9	5.68	88.78	8:08	14.63	1.06	97.24	93.23	59.05	94.60	00.0	17.66	17.66	-17.662	17.66	17.66	17.66
	N (t)	3.87	-6.335	11.26	3.72	16.18	18.64	21.11	23.57	26.03	28.49	6.41	3.73	21.065	8.39	5.71	3.04	0.37	.69	.02	34	.32	30	9.30	-0.307	9.30	9.30	9.30
	c s (t)	9.4	1 8 30 8	6.37	5.35	4.34	es.		30	Ci Ci	. 72	50	10.43	-9.363	3.29	7.22	3.15	5.08	1.01	2.94	1.87	08.0	5.42	5.84	56.272	6.63	7.12	7.54
	ase 4 SEISMIC M (tm)	0.00	9 6	7.92	22.36	26.03	28.94	1.07	32.44	33.05	32.89	0.37	1.39	33.298	90.9	9.72	4.24	9.64	.92	.07	60.	00.	3.26	3.21	-2.848	7.82	8.79	0.07
æ	Ca L(m)	8	0.757	2	.02	. 78	10.	ල :	.05	8	က်	. 00	8	1.637	45	27	00	6	73	n A	.36	1.8	00.	72	1.440	1.6	88	.60
STAIR	No	۰,	N	(C)	য	'n	တ	t~ (æ	o	~ √	က	,-4	જ	ო	ਚਾ [*]	w	Q	<u>-</u> -	0)	Φ	63	4	Н	67	თ	শ	C)
NOI		; ·	* *	*	*	*	*	*	*		Ç.	<i>'</i>	*	*	*	*	*	*	*	*	*	ဗ	2	*	*	*	*	4
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bace								•		٠								1	-		5. ⁽	<u> </u>						
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	٠	ပ	Case 7			Case s			Case S		
r-	No	L(m)	M (tm)	S (t)	N (£)		s (t)	N CF		\$ (1)	(3) N
i H	63	00.	00.	85.12	5.16	00.	2.30	6.20	00.	3.17	7.79
*	, 4 ,	.75	21.04	4.67	5.40	2.65	9.65	8.97	7.71	8.56	0.55
*	8	1.514	203.896	84.225	34.652	120.593	47.000	11.735	49.2	63.959	26.335
*	m	.27	48.55	3.77	3.89	3.81	4.34	5.50	4.54	9.35	2.07
×	4,	.02	55.03	6.67	6.86	42.31	8.30	22.73	93.66	5.25	2.16
*	ស	.78	23.31	7.12	27.61	16.10	0.95	39.97	76.59	9.85	6:39
*	\$	ις. 4	53.40	17.57	8.37	65.17	3.60	57.20	33.32	74.46	0.63
*	~	. 29	5.31	8.02	69.12	10.47	16.25	74.44	63.85	90.60	4.87
*	ø	0.5	00.75	18.40	9.85	10.69	48.86	91.65	31.66	143.62	50.55
*	6	.81	5.18	68.85	10.61	235.73	181.51	108.88	53.48	178.23	3.33
į,	-	TO.	507.89	19.30	1.37	85.57	4.16	6.12	301.57	2.84	87.56
	•		6	į	, ,	1			(•	
:	'n	3	77.84	37:11	32.03	82.72	12.34	78.67	32.06	69.37	, ,
*		8	68.69	82.91	3.16	36	6,83	7.57	4.30	2.6	7.04
*	c	.63	59.65	28.18	1.27	7.8	41.37	9.29	58,56	15,89	6.35
*	n	. 45	04.79	73.38	9.35	05.03	05.86	1.00	5.39	89.14	5.65
¥	4	.27	24.23	. 63	.46	77.12	0.40	2.72	7.38	. 42	.97
*	ĸ)	.09	98.97	63.85	5.04	20.24	4.89	4.42	27.56	5.67	4.26
*	vo.	.91	28.82	3.12	3.65	34.28	0.56	6.14	45.81	8.95	3.53
*	7	73	13.85	5.67	8.27	19.27	6.07	2.15	87	7.80	7 . 2
*	œ	ŝ	54.11	00.40	40.16	75.26	71.53	0.42	16.69	44.52	7.80
*	σ	7:367	149.438	-155.205	62	102.132	-107.046	-38.727	ധ	-71.276	-28.510
ပ္ပ	N	. 25	0.00	9.93	3.97	0.00	42.51	7.00	00.	97,99	51.6
	7	Č	0		00	7 a a a	ئ ۳۰ ۲۰	0 0	u u		e C
	۴,	2 1		•			•) ·	•	•	
*	٦,	 !?	4.94		14.86	96,	3.71	495.81	9.51	٥,	16.81
*	ċ 1	44	4.94	•	20.72	6.31	4.27	501.67	9.51	٥,	21.06
¥	ന	. 16	.94	•	26.59	9.99	4.84	507.53	.51	٥.	5,31
*	4	2.880	4	0.000	-732,452	114.081	75.401	-513,400	o	000.0	-429.561
4-	~1	.60	.94	•	38.31	68.57	5.96	519.26	.51	٥.	3.80

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N (t)	4.0	21.984	8	2.22	14.32	26.43	38.53	50.63	72	74.82	86.53	7.07	2.23	61.412	8.57	5.74	2.90	0.07	2.76	5.58	28.43	41.25	53.58	57.82	-362,076	66.32	70.57	74.82
s (t)	2.85	59.033	5.21	1.39	2.42	36.24	0.06	83.88	7.66	131.48	55.30	7.00 7.00	29:77	103.909	78.01	2.14	6.24	0.38	25.51	51.37	77.27	3.14	4.33	4.67	45.018	5.35	5.69	6.03
ase 12 M (tm)	00		89.37	07.01	6.62	88.20	7.7	2.72	75.13	65,65	274.25	14.93	-98.04	2	2.03	25.26	57:36	68.26	57.97	6.52	73.83	00	59.31	7.27	5 019	7 55	33	3.35
N (t) N	2 4	38.008	3 77	533	4.70	18.94	33.18	47 41	63	75.87	90.11	. 66	7.62	62.609	7:57	2,55	7.52	2.50	12.53	55	42.58	57.60	86.33	90.58	-494.835	499.08	03.33	07.57
S (t)	26.98	8	57:77	23,16	11.43	46:04	80.64	15,25	149.81	184.41	219.0	6. 6.	94.05	156.524	18.93	81,39	3.80	.25	31,33	.87	06.46	144.00	.00	00.	00000	00.	8	00.
ase 11 M (tm)	8	3.0	39.86	70.50	4.94	53 18	05.23	31.08	69.11	95.61	8.38	5.64	184.30	40	71.88	53.81	05.08	25.55	15.29	74.30	02.50	0.00	0.25	0,25		0.25	0.25	0.25
N CE O	4.64	80	4 37	4.24	5 89	16.02	26.15	36.29	41	56.54	99	0.86	5.83		5.77	0.75	5.72	0.70	14.33	8	44.38	59.40	19.90	424.15		432.64	436.89	41.14
S (t)	4	56	4.93	10.30	14.32	38:95	63.58	88:21	12:80	137.43	162.06	27.1	80.08	152.024	14.43	76.89	9.30	1.75	35.83	.37	110.96	48.50	•	٠	0.000	•	•	0.000
ase 10 M (tm)	00	7	90.18	07.30	5 79	85.62	6.81	10.63	6.62	81.34	294.75	321.81	51.16	-11.452	97.66	75.91	23.49	40.29	26.33	1.67	06.18	00.0	7.05	7.05	7.05	7.05	7.05	0
Ca L(m)	00.	0.757	5	. 27	. 02	78	5.5	23	0.3	8	. 56	00	8	1.637	45	.27	0.0	.91	.73	ις 14	.36	. 18	00.	72	7	9.7	8	9
No		m		٠.	1.1	4	.21							. (1)		4	ß											٨
	1	*	*	*	*	*	*	*	*	*	7	S .		*	*	*	*	×	*	*	*	က	4		*	*	*	4-

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	No.	
	PICK-UP	

No. L (m) Case M (tm) S (t) N (t) Case M (tm) S (t) N (t) N (t) N (tm) S (t				×	MAXIMUM	5		×	MINIMUM	فجرام
	No.	\smile	S.	±		\smile	25	M (tm)	V	.
** 1 0.757 C-5 128.768 144.879 59.607 C-8 72.655 79.652 79.652 79.652 79.652 79.652 79.652 79.652 79.652 79.652 79.652 79.652 79.652 79.652 79.652 79.652 79.652 78.993 76.29 76.703 <t< td=""><td>ī</td><td></td><td>:</td><td></td><td>12.30</td><td>6.20</td><td></td><td>00</td><td>85.12</td><td>6.36</td></t<>	ī		:		12.30	6.20		00	85.12	6.36
*** **	¥	•		8	44.87	9.60		2,65	9,65	8.97
** **<				G	94.43	8.85	3	20.59	7.00	1.73
** 4 ** 5 ** 6 ** 6 ** 6 ** 7 ** 6 ** 7 ** 7 ** 7 ** 8 ** 7 ** 8 ** 7 ** 8 ** 6 ** 7 ** 8 ** 7 ** 8 ** 6 ** 7 ** 8 ** 6 ** 7 ** 8 ** 6 ** 7 ** 8 ** 6 ** 7 ** 8 ** 6 ** 7 ** 8 ** 6 ** 7 ** 8 ** 6 ** 7 ** 8 ** 6 ** 7 ** 7 ** 8 ** 6 ** 7 ** 7 ** 8 ** 6 ** 7 ** 8 ** 6 ** 7 ** 6 ** 7 ** 6 ** 7 ** 7 ** 7 ** 7 ** 7 ** 8 ** 7 ** 8 ** 8 ** 6 ** 7 ** 6 ** 7 ** 7 ** 7 ** 7 ** 7 ** 7 ** 7 ** 7 ** 7 ** 7 ** 7 ** 8 ** 8 ** 8 ** 8 ** 8 ** 8 ** 8 ** 8 ** 8 ** 8 ** 8 ** 8 <th< td=""><td></td><td></td><td>٠.</td><td></td><td>3.98</td><td>8.09</td><td>1</td><td>43.81</td><td>4.34</td><td>5.50</td></th<>			٠.		3.98	8.09	1	43.81	4.34	5.50
7.785 C-5 76.9178 -56.918 -23.418 C-6 11.843 -55.423 -52.80 ** 6.055 C-5 199.786 -107.387 -44.174 C-6 -23.527 -123.412 -52.78 ** 8 6.055 C-5 -38.966 -157.387 -106.415 C-6 -23.55.184 -157.345 -157.345 ** 9 6.055 C-7 -285.184 -268.73 -110.611 -157.345 -157		• 1		82	6.45	2.66	· , ´	40.93	21.43	8.8
** 6 54.2 C5 199.760 -107.367 -44.174 C6 67.502 -23.527 -123.401 -56.78 ** 7 5.299 C5 -99.388 -106.415 C6 -123.401 -157.3401 -50.77 ** 9 6.812 C5 -28.648 -106.415 C6 -235.184 -26.853 -10.710 ** 9 6.812 C5 -286.728 -10.617 -26.853 -10.617 -26.853 -110.617 ** 1 0.000 C8 -286.728 -10.732 -10.732 -10.732 -10.733 -26.385 -27.732 -110.617 -10.617 -10.617 -10.617 -10.617 -10.617 -10.617 -10.617 -10.617 -10.733 -10.733 -10.733 -10.733 -10.733 -10.733 -10.733 -10.733 -10.733 -10.733 -10.733 -10.733 -10.733 -10.733 -10.733 -10.733 -10.733 -10.733 -10.733 -10				61	56.91	23.41	ì	11.84	55.42	2.80
* 7 5.299 C-5 5.299 C-6 -129,657 -127.345 -507.345 -64.73 -64.73 -64.73 -64.73 -64.73 -64.73 -64.73 -64.73 -64.73 -64.73 -64.73 -64.73 -64.73 -64.73 -67.84 -64.73 -67.84		• •		66	107.36	44.17	4	57.02	89.41	36.78
** 6.056 C-5 -157345 -157345 -167345 ** 6.056 C-5 -208.199 -85.659 C-7 -285.184 -167345 -16.473 ** 9 6.312 C-7 -285.184 -215.345 -10.6110 -10.6110 -10.617.899 -110.617				6	157.81	64.93	i	23.52	123.40	50.77
* 9 6.812 C- 5				80	208.19	85.65	ï	129.65	157.34	64.73
- 1 7.569		•		215	258.64	106.41	ı	285.18	268.85	110.61
* 3 0.000 C- 8 -285.728 125.874 C- 7 -522.845 337.717 135.08 * 2 1.26.360 176.836 107.575 C- 7 -268.695 282.918 113.16 * 46.381	· i			385	214.16	126.12	1	507.89	319.30	31.37
** 1 0.000 C-8 -288.728 212.343 123.674 C-7 -268.635 232.918 133.16 ** 1 0.000 C-8 -126.360 141.372 123.64 14.493 C-7 -268.780 161.155 64.46 ** 2 -457 C-5 -88.780 161.155 64.46 ** 3 2.456 C-6 260.699 111.231 44.493 C-5 28.087 124.236 49.69 ** 4 -93 C-6 56.433 22.573 C-5 144.630 87.361 24.24 ** 5 4.093 C-7 50.807 C-17.001 50.17 50.17 ** 6 548 C-7 50.807 C-7 50.242 20.17 ** 5 7.367 C-7 50.007 150.051 -23.36 -24.09 ** 6 548 C-7 50.007 -134.020 -134.020 -24.09				; ;	(t t		0	20	i u
** 1 0.819 C-8 -126.360 176.836 107.975 C-7 -208.093 282.91 282.91 282.91 282.91 282.91 282.91 282.91 282.91 282.91 282.91 282.91 282.91 282.91 282.91 282.91 282.92 282.93	ŧ.	٥,	4	782	12.34	20.07		40.220	1 0	•
* 2 1.637 C- 8 141.372 89.299 C- 5 -88.780 161.155 64.46 * 3 2.456 C- 6 147.326 165.963 66.385 C- 5 28.087 124.236 49.69 * 4 3.274 C- 6 329.357 56.433 22.573 C- 5 171.061 50.442 20.17 * 5 4.913 C- 6 329.357 56.433 22.573 C- 5 197.241 13.563 5.42 * 7 30 C- 6 325.134 1.701 0.680 C- 5 197.241 13.563 5.42 * 8 6.548 C- 6 266.267 -107.830 -43.132 C- 5 159.051 -97.145 -38.85 * 9 7.367 C- 6 155.514 -162.629 -65.052 C- 5 94.607 -97.145 -38.85 * 9 7.367 C- 6 42.678 0.000 -589.385 C- 8 -46.968 73.153 -485.55 * 1 0.720 C- 6 42.678 0.000 -665.247 C- 8 -46.968 73.719 -495.81 * 2 1.440 C- 6 42.678 0.000 -605.247 C- 8 -46.968 73.719 -495.81 * 2 2.160 C- 8 154.840 -513.400 C- 5 -17.662 0.000 -610.73 * 2 2.160 C- 8 11.491 75.961 -513.400 C- 5 -17.662 0.000 -610.59		8	ì	126.36	76.83	07.57		268.69	85.8	01.51
* 3 2.456 C- 6 147.326 165.963 66.385 C- 5 28.087 124.236 49.59 * 4 3.274 C- 6 260.699 111.231 44.493 C- 5 114.630 87.361 34.949 * 5 4.093 C- 6 329.357 56.433 22.573 C- 5 177.061 50.442 20.17 * 6 4.911 C- 6 322.036 -53.098 -21.239 C- 5 197.241 13.568 -9.34 * 7 5.730 C- 6 266.599 -43.132 C- 5 159.051 -60.226 -24.09 * 8 6.548 C- 6 155.514 -162.629 -65.052 C- 5 94.607 -97.145 -38.85 - 2 8.185 C- 6 42.678 0.000 -599.385 C- 8 -46.967 -97.146 * 7 0.000 C- 6 42.678 0.000 -659.385 C- 8 -46.968 77.719 -499.83 * 2 1.440 C- 6 42.678 0.000 -611.109 C- 5 -17.662 0.000 -604.87 * 3 2.160 C- 8 59.994 74.840 -513.400 C- 5 -17.662 0.000 -610.73 * 4 2.880 C- 8 114.081 75.961 -513.400 C- 5 -17.662 0.000 -610.73		63		7.8	41.37	9.29	ı	88.78	61.15	4.46
* 4 3.274 C- 6 260.699 111.231		4		47.32	65.96	6.38	ī	28.08	24.23	9.69
* 5 4.093 C-6 329.357 56.433 22.573 C-5 171.061 50.442 20.17 * 6 4.911 C-6 352.134 1.701 0.680 C-5 197.241 13.568 5.42 * 7 5.730 C-6 352.086 -53.098 -21.239 C-5 159.051 -23.352 -23.352 * 8 6.548 C-6 266.267 -107.830 -43.132 C-5 159.051 -23.352 -24.09 * 9 7.367 C-6 10.000 -217.361 -86.944 C-5 94.607 -97.145 -38.85 - 4 0.000 -559.385 C-7 94.607 -97.145 -489.95 * 1 0.720 C-6 94.607 -97.145 -73.158 -489.95 * 1 0.000 -559.385 C-8 -99.843 73.158 -489.95 * 1 0.000 -605.247 C-8 -46.968 73.158 -495.81 * 2 1440 C-8 50.994 74.840 -507.537 C-8 -17.662 0.000 -610.73 * 2 3.80 <		27		69.09	11,23	4.49	ı	14.63	7.36	4.94
* 6 4.911 C- 6 353.134 1.701 0.680 C- 5 197.241 13.568 5.42 * 7 5.730 C- 6 332.086 -53.098 -21.239 C- 5 159.051 -23.352 -9.34 * 8 6.548 C- 6 266.267 -107.830 -43.132 C- 5 159.051 -23.352 -24.09 * 9 7.367 C- 6 162.629 -65.052 C- 5 94.607 -97.145 -24.09 * 9 7.367 C- 6 94.607 -97.145 -38.85 - 2 8.185 C- 6 94.607 -97.145 -38.85 - 4 0.000 -217.361 -86.944 C- 5 94.607 -97.145 -38.85 - 4 0.000 -65.247 C- 8 -46.948 73.158 -489.95 * 1 0.000 -61.07.537 C- 8 -17.662 0.000 -59.90 * 2 3.80 C- 8 14.00 -65.93 74.840 -513.400 -65.00		C	,	29.35	56.43	2.57		71.06	0.44	0.17
* 7 5.730 C- 6 332.086 -53.098 -21.239 C- 5 159.051 -23.352 -54.09 * 8 6.548 C- 6 266.267 -107.830 -43.132 C- 5 159.051 -60.226 -24.09 * 9 7.367 C- 6 155.514 -162.629 -65.052 C- 5 94.607 -97.145 -38.85 - 2 8.185 C- 6 0.000 -217.361 -86.944 C- 5 0.000 -134.020 -53.60 * 42.678		G	•	53.13	1.70	0.68		97.24	3.56	42
* 8 6.548		1	ı	32.08	53.09	21.23	1	93.23	23.35	9.34
* 9 7.367 C- 6 155.514 -162.629 -65.052 C- 5 94.607 -97.145 -38.85 -38.85 C- 6 0.000 -217.361 -86.944 C- 5 0.000 -134.020 -53.60 -53.60 -53.60 -599.385 C- 8 -99.843 73.158 -489.95		T.	i	66.26	07.83	43.13	1.	59.05	60.22	24.09
- 2 8.185 C- 6 0.000 -217.361 -86.944 C- 5 0.000 -134.020 -53.60 - 4 0.000 C- 6 42.678 0.000 -559.385 C- 8 -99.843 73.158 -485.95 * 1 0.720 C- 6 42.678 0.000 -605.247 C- 8 -46.968 73.719 -495.81 * 2 1.440 C- 6 42.678 0.000 -611.109 C- 5 -17.662 0.000 -604.87 * 3 2.160 C- 8 5.94 74.840 -507.537 C- 5 -17.662 0.000 -604.87 * 4 2.860 C- 8 114.081 75.401 -513.400 C- 5 -17.662 0.000 -610.73		C		55.51	162.62	65.05	1	4.60	97.14	38.85
4 0.000 C-6 42.678 0.000 -599.385 C-8 -99.843 73.158 -489.85 * 1 0.720 C-6 42.678 0.000 -605.247 C-8 -46.968 73.719 -495.81 * 2 1.440 C-6 42.678 0.000 -611.109 C-5 -17.662 0.000 -595.00 * 3 2.160 C-8 59.994 74.840 -507.537 C-5 -17.662 0.000 -604.87 * 4 2.380 C-8 114.081 75.401 -513.400 C-5 -17.662 0.000 -610.73 * 4 2.360 C-8 168.571 75.961 -519.262 C-5 -17.662 0.000 -616.59	1	7		0.00	217.36	86.94	ŧ	8	134.02	53.60
* 1 0.720 C- 6 42.678 0.000 -605.247 C- 8 -46.968 73.719 -495.81 * 2 1.440 C- 6 42.678 0.000 -611.109 C- 5 -17.662 0.000 -595.00 * 3 2.160 C- 8 59.994 74.840 -507.537 C- 5 -17.662 0.000 -604.87 * 4 2.380 C- 8 114.081 75.401 -513.400 C- 5 -17.662 0.000 -610.73 * 5 2.80 C- 8 168.571 75.961 -519.262 C- 5 -17.662 0.000 -616.59	,	S		5.67	0	599.38		99.84	3	489.95
* 1 1.40 C- 6 42.678 0.000 -611.109 C- 5 -17.662 0.000 -599.00	•			5 67	C	605.24	ł	46.96	3	495.81
* 3 2.160 C- 8 59.994 74.840 -507.537 C- 5 -17.662 0.000 -604.87 * 4 2.380 C- 8 114.081 75.401 -513.400 C- 5 -17.662 0.000 -610.73 - 2 3.600 C- 8 168.571 75.961 -519.262 C- 5 -17.662 0.000 -616.59		1 T		5.0		611.10	,	17.66	0	599.00
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	F. 1	9 6		8.57	S S	519.26	1	7.66	Ġ	16.59

PICK-UP No. 1 * ULS

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	N (E)	6.20	6.	9.34	5.16	œ	7.6:	48.37	9.12	89.85	110	3.37	25.87	7.57	89.29	1.00	2.72	34.424	6.14	21.23	43.13	5.00	86.94	87.28	593.14	599.00	04.87	-610.732	16.59
MINIMUM	s (t)	2.30	79.65	6.54	2.55	23.43	67.12	17.57	168.02	218.40	-268.853	19.30	12.34	6.83	41.37	05.86	70.40	34.895	0.56	53.09	07.83	62.62	217.36	•	•	•	•	000.0	•
S	M (tm)	0	2.65	21.92	44.29	0.93	23.31	53.40	45.31	00.75	-285.184	07.89	285.72	.36	3,78	05.03	77.12	220.244	34.28	32.08	66.26	55.51	0.00	17.66	17.66	17.66	17.66	-17.662	17.66
	Case									٠,	C. 7			.1	ı	'n.		ထ		ı:	r		ì		ŧ		ı	က ပ	
	N (‡)	0.36	9.6	8.85	8.09	99.	39.97	7.20	74.44	91.65	108.88	26	35.08	3.16	91.27	9.35	7.46	25.543	5.42	9.34	4.09	38.85	53.60	9.95	495.81	501.67	07.53	-513.400	19.26
MAXIMUM	(E)	95,32	.87	94.43	93	46	50,95	83.60	16.25	48.86	-181.516	14.16	37.71	2.91	28.18	73.38	18.65	63.85	3.56	23.35	0.22	97,14	•	с. 	ω 	4	æ	75.401	S.(
Ø	M (tm)	8	28.76	19.34	71.73	5.93	16.10	65.17	10.47	110.69	-235.739	385.57	522.84	68.69	59.65	04.79	24.23	298.975	97.24	93.23	59.05	4.60	8	9.84	46.96	6 31	9.99	114.081	68.57
	Case						ì				8 - O			ı		1	ı	C- 7	ŧ	ſ	ı	ı	ဖ ပ်	1	ï	i	1	ر 8	
	(m)	00	75	153 153	.27	. 02	78	. 54	29	0.5	6.812	36	00	0	.63	4.55	.27	4.093	16	.73	4	36	. 18	00	72	44	16	2.880	.60
	No.					*	1				o *		(r)					ю *					1 61	1.4			ლ *	*	C1 -

PICK-UP No. 1 * ULS

	(3) N	6.20	8.97	7.3	5.50	2.73	9.97	7.20	14	91.60	10.61	131.37	3.97	9.21	4.46	9.69	.94	0.17	.68	1.23	3.13	-65.052	6.94	00.60	14.86	720.72	26.59	732	38.31
MINIMUM	s (t)	2.30	9.65	7.00	4.34	8.30	50.95	83.60		48.86	68.85	30	34.94	98.02	61.15	4.23	87.36	0.44	.70	53.09	07.83	-162.629	17.36	•	•	•		0.000	•
N.	M (tm)	00.	2.65	20.59	43.81	42.31	6.10	65.17	10.47	0.69	285.18	07.8	12.99	5.68	88.78	8.08	14.63	71.06	53.13	32,08	6.26		8	4.94	4.94	4.94	4.94	14.946	4.94
	Case	í	ř	1	1	ŀ		í	ر ا ا	F	ı.			ì	· t	ı	ı	ï			E	9 -5	j		ı	ŧ	j	C- 7	1 .
	(t) %	0.36	9.60	8.85	60.	2.66	2.80	6.78	00	4.73	8.72	2.70	35.08	3.16	91.27	1.00	2.72	4.42	6.14	2.15	0.42	-38.727	53.60	89.95	495.81	501.67	507.53	-513.400	519.26
MAXIMUM	S (t)	95.32	4.87	94.43	3.98	6.46	5.42	89.41	-123.401	57.34	191.33	25.32	37.71	82.91	8.18	05.86	0.40	4.89	0.56	6.07	71.53	-107.046	34.02	3.15	3.71	4.27	4.84	75.401	5.96
7	M (tm)	00	28.76	19.34	71.73	85.53	11.84	57.02	-23.527	129.65	61.62	419 39	22.84	268.69	59.65	05.03	77.12	20.24	34.28	19.27	75.26	102.132	0.00	99.84	6.96	6.31	9.99	114.081	8.57
	Case	i	1	i	Ċ					1	1	9.)										& - -	ر اد	i			ı	۵ -	,
	(m)	8	7.5	5	2.7	9	7.	5.4		0.5	8	56	0	8	53	4,	27	0.0	.91	.73	3.4	ശ		00.	775	4.4	91	2.880	.60
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MINIMO	s (t)	6.98	9:03	5.21	39	14.32	-38.952	63.58	88.21	12.80	84.41	219.02	3.1 83.		4.00	15.39	5.14	2.42	35.672	8.95	17.80	. 52	71.27	7.99	33	.67	0.00	00.	0.000	00.
Ä.	M. (tm)	9.	3.70	9.37	07.01	5.79	in co	6.81	10.63	6.62	95.61	48.38	er er		184.30	58.56	5,39	87.38	127.560	45.81	42.18	16.69	69.27	8	59.31	7.27	9.51	9.5	-9.510	5
	Case		H	7	امنو ا	7	C-10	1	ï	7	7	H	- 5			ı	1	ı	ი ე	3	ı	,	ì	ì	1	1	'n	1	ာ ပ	1
	N (t)	4.08	0.55	6.33	2.07	2.16	16.39	0.63	44.87	9.03	73.33	-86.932	7.07	÷ (4.23	1.41	5.77	0.75	.72	0.70	4.33	9.34	44.38	59.4	19.90	24.15	428.40	66.32	-370.572	74.82
MAXIMUM	S (t)	8	8.56	3.05	9.35	5.25	9.85	74.46	109.06	43.62	178.23	155.30	. u		29.77	03.90	4.43	76.89	30	1.75	35.83	73.37	10.96	-148.506	٥.				45.697	0.9
×	M (tm)	0.000	1	6	77	t:	۵	5	63		i th	274.	7	* •	8	તં		73		0	26	8	90	000.0	7.05	, C	7.05	, r	70.334	800
	Case	-	1				,	ı	·	·	. T	C-12	٠	7	4	7	7	, , 1	<u>ټ</u>	7	ï	<u>ب. </u>	1	C-10	, I	· ~	۲,	ب ۱	16	+ + i
	L (m)	00	(~		2.7	S	000	L)		0.0	00	7.569		3	ä	63	4	27	Ö	0.		r.	6	8.185	ç	, ,			. «	2000
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No.	L (m)	Case	M (tm)	(1) S	N (t)	Case	M (tm)	s (t)	
1 - 2	00.	ì	00	3.17	4.79	rel Į.	00	2.85	4.
*	0.757	6 - 5	87.712	98.5	40.552	C-12	Ė	59.033	
ري *	5.	3 .	49.22	3.95	6.31	겉	8.	4.93	
	27	·	4.54	9.35	2.07	4	07.30	0.30	
	0.	. 1	93.66	5.25	2.16	7	05.79	4.32	
	.78	Ħ	8.20	6.24	26.43	7	53.18	46.04	
	Ţ,	7	1.75	90.0	8,53	7	5.23	0.64	
	.29	7	2.72	3.88	50,63	7	31.08	115.25	
	9	7	75.13	07.66	62.72	774	69.11	149.81	
თ *	8	***	5,65	1.48	4.82	H	195.61	84.41	-
7 - 2	3	ĭ	274.25	155.30	86.93	Ħ	48.38	219.02	
3	00	9~4 1	58.64	31.65	2.66	7	14.93	55.67	
*	0.819	C-11	-184.308	194.067	77.627	C-12	-98.044	129.775	
	63	بہ i	40.91	56.52	2.60	ï	2.46	08.80	
	4	Π	71.88	18.93	7.57	무	2.03	78.01	
	.27	급	53.81	1.39	2.55	ï	25.26	2.14	
	0.0	7	05.08	3.80	7.52	Ħ	57.36	6.24	
φ *	91	,	5.81	8.95	3.58	7	8.26	0.38	
	73	ı	42.18	7.80	7.12	급	26.33	5.83	
	5.4	ı	16.69	4.52	7.80	۲۳I ا	81.67	73.37	
	36	1	69.27	2.7	28.51	Ħ	06.18	36.	
23 1 8		1	00	97.99	9.19	-	9	48.50	
2 - 4	00.	7	9.31	4.33	353.58	1	9.51	•	J
*	72	H	7.27	4.67	357.82	ı	9.51	•	1
	44	7	5.01	5.01	362.07	ı	9.51	•	•
ო *	2,160	C-12	37.554	45.358	-366.324	ი -ე	-9.510	0.000	•
	88	ï	0.33	5.69	370.57	-1	9.51	•	•
4 - 2	.60	1	3.35	6.03	74.82	1	9.51	٠	1

87.079 87.079 88.574.079 88.574.070 88.594.074 86.0073 86.0073 86.0073 86.0073 -412.569 -416.817 -421.065 -425.313 -429.561

PICK-UP NO. 2 * SLS

				Z	MAXIMU	Σ.	:	Z	MINIMO	Ž
Z	No.	L (m)	Case	M (tm)	s (t)	N (t)	Case	M (tm)	s (t)	N (t)
	67	00	1	8	3.17	4.79	C-12	00	2.85	4
*		.75	í	87.71	8.56	0.55	7	3.70	9.03	98
	2	5	1	49.22	3.95	6.31	7	9.37	5 21	88
	ന	.27	 1	184.54	9.35	2.07	Ħ	07.01	1.39	22
	4	.02	· .	3.66	5.25	2.16	Η	6.62	2.42	4.32
	LO.	78	7	85.62	38.95	6.02	T	8.20	36.24	6.43
	့်	4.542	C-10	46.817	-63.581	-26.159	C-12	51.757	60	-38.534
*		29	7	10.63	88.21	36.29	7	2:72	83.88	0.63
*	00	.05	7	6.62	12.80	6.41	귺	75.13	7.66	2.72
*	Or	8	74	181.34	37.43	56.54	H	5.61	84.41	75.87
1 8	-	. 36	0-10	94.75	62.06	6.67	Ŧ	48.38	18.02	1.O
. 1	ເສ	00		358.64	31.65	2.66		292.06	69.37	7.74
*	· end	8	· ~	84.30	94.06	7.62	1	4.30	42.61	7.04
*	~	1.637	C-11	-40.916	156.524	62.609	6 - 0	28	115.899	46.359
*	ෆ	45	7	72.03	78.01	8.57	,	5.39	89.14	5
*	4	27	7	25,26	2.14	5.74	r	7.38	2.42	4.97
*	ເດ	.09	7	57,36	6.24	2.90	4	27.56	5.67	4.26
	9	. 91	7	68,26	0.38	0.07	-1	40.25	.75	.70
*	7	. 73	7	.97	25.53	2.76	H	6.33	35,83	4.33
*	æ	2.	r-4	26.52	1.37	5.58	,-1	81:67	.37	ς. 1
*	6	.36	7	73.83	77.27	28.43	77	06.18	10.96	44.38
၊ ဗ	Ø	œ ***	1	00.	7.99	61.6		00.	48.50	9.40
1		00.	<u></u> 4	9.31	4.33	353.58	7-1	0.25		86,33
*	r-4	72	T	27:27	4.67	357.82	ř	0.25		0.58
*	2	44	7	5.01	5.01	362.07	7	0.25		54.83
*	် က	. 16	7	7.55	. 35	66.32	7	0.25	0.000	80.66
*	4	2.880	C-12	70.334	45.697	370	C-11	10.252	0000	-503.333
1	8	.60	~~	3.35	.03	74.82	7	0.25	0.000	07.57

No. ① PEDESTRIAN BRIDGE-Staircase - Superstructure

- 1. Calculation of bending moment for U.L.S.
 - 1) For middle point of second span ... Mu. max = 353.2^{KNm} (For middle point of first span ... Mu. max = 286.0^{KNm}) section b = 360^{cm} h = 30 d = 23.5 d = 6.5 As = $1.6 25^{\text{NO}}$ = 2.011×25 = 50.275 cm² $x = \frac{0.87 \times 41000 \times 50.275}{0.40 \times 3000 \times 360} = 4.2^{\text{cm}}$ $Z = 23.5 \frac{4.2}{2} = 21.4^{\text{cm}} < 0.95 \times 23.5 = 22.4^{\text{cm}}$ OK Mrs = $0.87 \times 41000 \times 50.275 \times 21.4 \times 10^{-5} = 383.8^{\text{KNm}} > \text{Mu} = 353.2^{\text{KNm}}$ Mrs = $0.40 \times 3000 \times 360 \times 4.2 \times 21.4 \times 10^{-5} = 388.3^{\text{KNm}} > \text{Mu} = 353.2^{\text{KNm}}$ OK
- 2) For middle fulcrum \cdots Mu.min= $-522.9^{\rm KNm}$

section $b = 360^{\text{cm}}$ h = 30 d = 24.0 d' = 6.0 $As = Y_{20} - 25^{\text{NO}} = 3.1416 \times 25 = 78.54 \text{ cm}^2$ $\chi = \frac{0.87 \times 41000 \times 78.54}{0.40 \times 3000 \times 360} = 6.6^{\text{cm}}$

$$Z = 24.0 - \frac{6.6}{2} = 20.7^{\text{cm}} < 0.95 \times 24.0 = 22.8^{\text{cm}}$$

 $M_{RS} = 0.87 \times 41000 \times 78.54 \times 20.7 \times 10^{-5} = 580.0^{\text{knm}} > \text{Mu} = 522.9^{\text{knm}}$ $M_{RC} = 0.40 \times 3000 \times 360 \times 6.6 \times 20.7 \times 10^{-5} = 590.2^{\text{knm}} > \text{Mu} = 522.9^{\text{knm}}$

- 2. Calculation of bending moment for S.L.S.
 - 1) For middle point of second span ··· Ms. $max = 240.3^{KNm}$

(For middle point of first span \cdots Ms. max=193.7^{KNm})

section
$$b = 360^{cm} h = 30 d = 23.5$$

$$h = 30$$

$$d = 23.5$$

$$d' = 6.5$$

$$As = Y_{18} - 25^{NO} = 50.275 \text{ cm}^2$$

$$x = \frac{0.80 \times 41000 \times 50.275}{\frac{1}{2} \times 0.50 \times 3000 \times 360} = 6.3^{\text{cm}}$$

$$Z = 23.5 - \frac{6.3}{3} = 21.4^{\text{cm}}$$

$$M_{RS} = 0.80 \times 41000 \times 50.275 \times 21.4 \times 10^{-5}$$
 = 352.9^{KNm}>M=240.3^{KNm}

$$=352.9^{KNm}>M=240.3^{KNm}$$

$$M_{RC} = \frac{1}{2} \times 0.50 \times 3000 \times 360 \times 6.3 \times 21.4 \times 10^{-5} = 364.0^{RNm} > M = 240.3^{KNm} OK$$

For middle fulcrum \cdots Ms. min = -358.7 KN m

section

$$b = 360^{cm}$$
 $h = 30$ $d = 24.0$ $d' = 6.0$

$$h = 30$$

$$d = 24.0$$

$$d'=6.0$$

$$A_{S} = Y_{20} - 25^{NO} = 78.54 \text{ cm}^2$$

$$\chi = \frac{0.80 \times 41000 \times 78.54}{\frac{1}{2} \times 0.50 \times 3000 \times 360} = 9.6^{cm}$$

$$Z = 24.0 - \frac{9.6}{3} = 20.8^{\text{cm}}$$

$$M_{RS} = 0.80 \times 41000 \times 78.54 \times 20.8 \times 10^{-5}$$
 = 535.8 KNm > M=358.7 KNm

$$=535.8^{\text{KNm}} > \text{M} = 358.7^{\text{KNm}}$$

$$M_{RC} = \frac{1}{2} \times 0.50 \times 3000 \times 360 \times 9.6 \times 20.8 \times 10^{-5} = 539.1^{\text{KNm}}$$

 $> M = 358.7^{KNm}$ OK

Note: U.L.S is critical state than S.L.S.

No.(1) PEDESTRIAN BRIDGE-Staircase - Superstructure

3. Calculation of shearing stress for U.L.S.

1) For middle fulcrum Su. max=337.8 knm section b=360 cm h=30 d=24.0 d'=6.0 As =
$$Y_{20}-25^{NO}=3.1416 \times 25=78.54$$
 cm²

$$P = \frac{78.54}{360 \times 24.0} \times 100 = 0.909 \%$$

$$Vc = \frac{337.8 \times 10^{3}}{360 \times 24.0} = 39.1 \text{ N/cm}^{2}$$

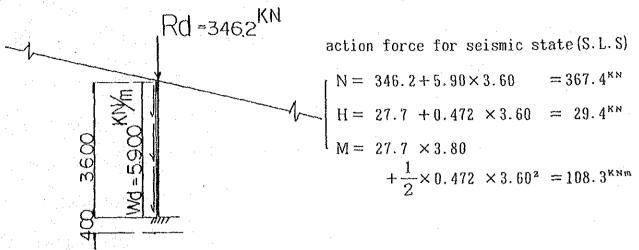
$$< V ca = 55.0 + 15.0 \left(\frac{0.909 - 0.50}{0.50} \right) = 67.3 \text{ N/cm}^{2}$$
 OK

No.① PEDESTRIAN BRIDGE-Staircase - Substructure

Calculation for bottom of pillar for Longitudinal direction.

 $< V ca = 50.0 + 15.0 \quad (\frac{0.903 - 0.50}{0.50}) = 62.1 \text{ N/cm}^2$

Calculation for bottom of pillar for crossing direction.



action force for seismic state for U.L.S

$$\begin{cases}
N = 367.4 + 1.38 & = 507.0^{KN} \\
H = 29.4 + 1.65 & = 48.5^{KN} \\
M = 108.3 \times 1.65 & = 178.7^{KNm}
\end{cases}$$

section b =
$$50^{\text{cm}}$$
 h = 50 d = 43.5 d' = 6.5
As = As' = $Y_{32} - 3^{\text{N}}$ = 8.042×3 = 24.126 cm²
Ma = $178.7 + 507.0$ ($0.435 - \frac{0.50}{2}$) = 272.5^{KNm}
 $x = \frac{(0.87 - 0.72) \times 41000 \times 24.126}{0.40 \times 2500 \times 50}$ = 4.4^{cm}
 $Z = 43.5 - \frac{4.4}{2} = 41.3^{\text{cm}}$ < $0.95 \times 43.5 = 41.3^{\text{cm}}$

 $M_{RS} = 0.87 \times 41000 \times 24.126 \times 41.3 \times 10^{-5} = 355.4^{\text{KNm}} > \text{Ma} = 272.5^{\text{KNm}}$ $M_{RC} = (0.72 \times 41000 \times 24.126 \times 37.0 + 0.40 \times 2500 \times 50 \times 4.4 \times 41.3) \times 10^{-6} = 354.4^{\text{KNm}} > \text{Ma} = 272.5^{\text{KNm}}$

A sn = A'sn = 24.126
$$-\frac{507.0 \times 10^3}{0.87 \times 41000}$$
 = 10.0 cm²

$$<$$
 Asu=A'su = $Y_{25}-4^{NO}=4.909 \times 4=19.636 \text{ cm}^2$ OK

Calculation of stability of foundation

Notice: this case is abridge and the bar arrengement is apply substructure of No.2 pedestrian bridge.