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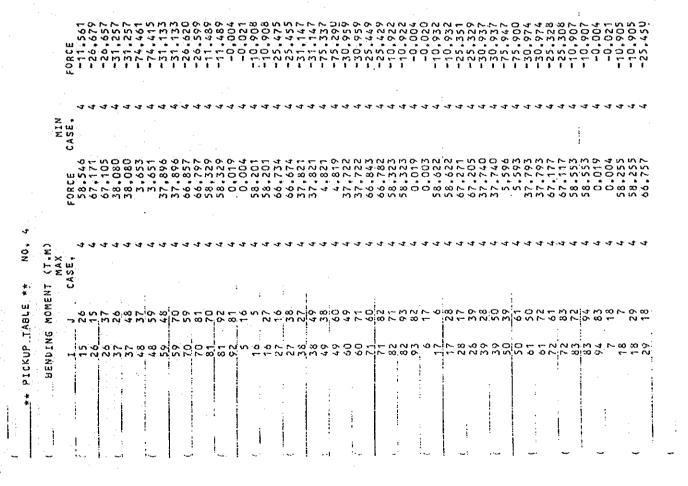
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7	(FORCE CASE.	13,114	5,609	25,435	17.569	15,526	7.837	19,371	10.949	13.053	5.553	25.339	17,472	15.664	7,962	19,649	11.294	12.807	5,414	25,461	17,646	15.692	8.070	20.252	12.061	4 11.667 4	
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** UHURU BRIDGE **

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UHURU BRIDGE **	PICKUP TABLE **
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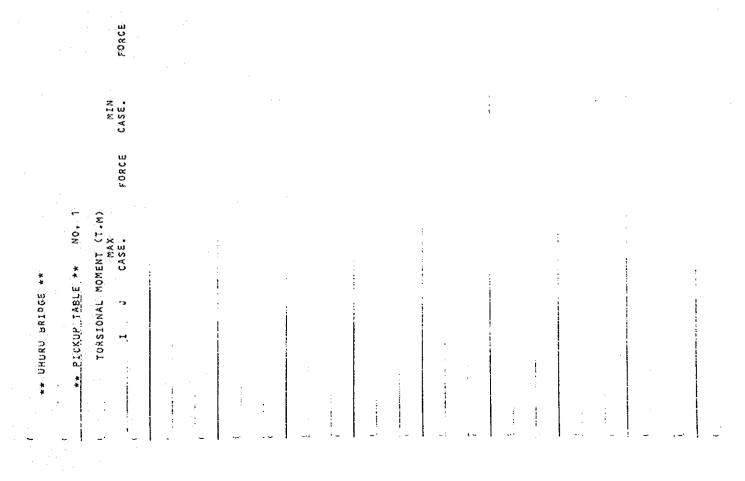
SHEARING FORCE (.T.)

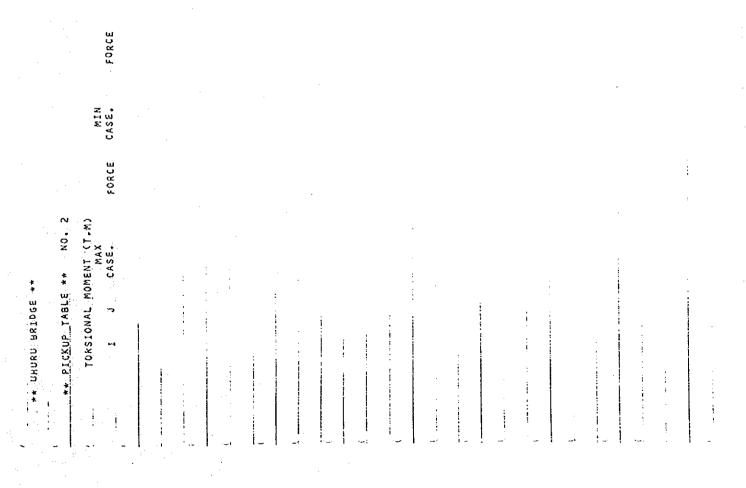
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SHEARING FORCE (.T.)

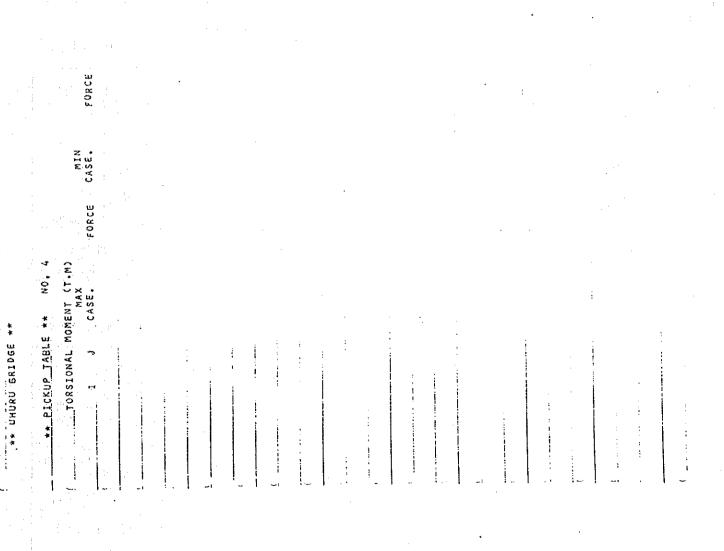
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** UHURU BRIDGE **





** PICKUP TABLE ** NO. 3			
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Uhuru Bridge
TOTAL MOMENT

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POIN	<u>T</u>	G1 ③		U.	L. S		S.	L. S
		MOMENT			MOMENT OF DESIGN		<u></u>	MOMENT OF DESIGN
		M (kNm)	γfl	γ 3	Mu=Mγflγ3 (kNm)	γfl	γ3	Mu=Mγ (Lγ 3 (kNm)
DE							· · · · · · · · · · · · · · · · · · ·	
L0	ΛD	694.8	1. 20	1.15	958.8	1.00	1.00	694.8
1 1110	11.4	1270 0						
LIVE	IIA	1373.0	1.50	1.10	2265. 5	1. 20	1.00	1647. 6
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	IIB	0.0	1. 30	1.10	0.0	1.10	1.00	0.0
		ļ	1		3224.3			2342.4
TOT	<u> </u>				958.8			694.8
DE	SIGN A	MOMENT			3224.3			2342.4
	·				į			
KESI	STANCE	E MOMENT			4947.6]	·		2504.0

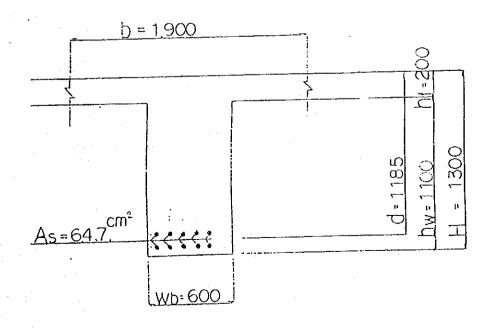
TOTAL MOMENT

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		MOMENT	İ		MOMENT OF DESIGN			MOMENT OF DESIGN
		M (kNm)	7 fL	γ 3	Mu=MγfLγ3 (kNm)	γfl,	γ 3	Mu=MγfLγ3 (kNm)
DE	AD							
1.0	ΛD	-1674.8	1. 20	1.15	-2311.2	1.00	1.00	-1674.8
LIVE	HA	-929.4	1.50	1.10	-1533.5	1.20	1.00	-1115.3
LOAD	IIA							
,	HB	-1239.6	1.30	1.10	-1772.6	1.10	1.00	-1363.6
					-3844.7			-2790.1
TOT	AL.				-4083.9			-3038.4
DE	SIGN 1	MOMENT	· • • • • • • • • • • • • • • • • • • •		-4083.9			-3038.4
REST	STANCI	E MOMENT	;		-5223.0			-3307.8

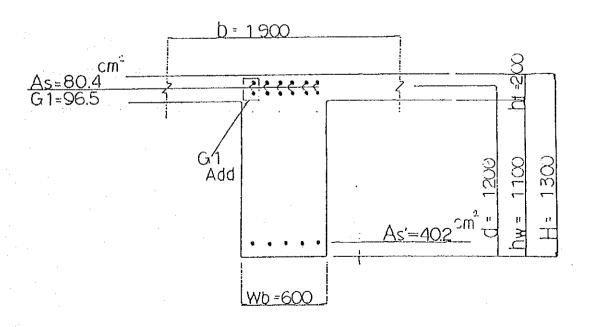
TOTAL MOMENT

	····						· /····	
POINT	ŗ	G 1 (5)		U.	L. S		S.	L. S
		MOMENT			MOMENT OF DESIGN			MOMENT OF DESIGN
		M (kNm)	γfl.	7 3	$Mu=M\gamma fl.\gamma 3$ (kNm)	γ f L	<u>γ3</u>	$Mu=M\gamma \Gamma L \gamma 3$ (kNm)
DE/	AD :							
LO	۸D	-1351.9	1. 20	1. 15	-1865. 6	1.00	1.00	-1351.9
LIVE	HΑ	-929.4	1.50	1. 10	-1533.5	1. 20	1.00	-1115.3
LOAD	HA							
	1113	-1021.6	1.30	1.10	-1460.9	1.10	1.00	-1123.8
					-3399.1			-2167.2
TOTA	۸L				-3326. 5	<u> </u>		-2475.7
					• •			
DES	SIGN N	IOMENT			-3399, 1			-2475.7
RESTS	STANCE	MOMENT			-5223.0	l		-2870.3

U. FLG b= 190 AS= 64.7 U. FLG hf= 20 fcu= 3000 WEB hw= 110 fy= 41000 d= 118.5 Z=d-1/2*hf = 108.5 MRC=0.4*fcu*b*hf*Z= 4947.6 MRS=0.87*fy* AS*Z = 2504.0



```
G1
 U. FLG b=
                  190 AS=
                                      98.5
 U. FLG hf=
                   20 fcu=
                                      3000
 WEB hw=
                  110 \text{ fy}=
                                     41000
 WEB Wb≈
                  60 AS'=
                                      40.2
    d=-
                  120
                       d' =
                                      7.5
 X=0.87*fy*AS/(0.4*fcu*Wb)
                                      47.8
 Z=d-1/2*X
                                      96.1
 MRC=0.15*fcu*b*d^2+0.72*fy*AS'*(d-d)
 MRS=0.87*fy*AS*Z =
                                    3307.8
                   G2
U. FLG b=
                  190 AS=
                                     80.4
U. FLG hf=
                  20 fcu=
                                     3000
WEB hw=
                 110 \text{ fy}=
                                    41000
WEB Wb=
                 60 AS =
                                     40.2
   d=
                 120
                        d' =
                                      7.5
X=0.87*fy*AS/(0.4*fcu*Wb)
                                     39.8
Z=d-1/2*X
                                    100.1
MRC=0.15*fcu*b*d^2+0.72*fy*AS'*(d-d)
                                   5223.0
MRS=0.87*fy*AS*Z =
                                   2870.3
```



Calculation of deck slab for Main bridge (U.L.S)

1. Span and bending moment

a) Span
$$\ell = 1.925 - 0.600 = 1.325 = 1.400^{m}$$

b) moment of middle span

$$\begin{aligned} \mathbf{M} &= \left\{ \begin{array}{l} 0.80 \, (\ 0.12 \, \ell + 0.07) & \mathrm{P} + \frac{1}{10} \omega \, \mathrm{d} \ \ell^2 \end{array} \right\} \times 1.5 \times 1.1 \\ &= \left\{ \begin{array}{l} 0.8 \, (0.12 \times 1.40 + 0.07) & \times 100 + \frac{1}{10} \times 4.72 \times 1.40^2 \end{array} \right\} \times 1.5 \times 1.1 = 33.0^{\mathrm{KNim/m}} \end{aligned}$$

c) moment of each fulcrum

$$\begin{split} \mathsf{M} & = \left\{ \; \left(\; 0.15\,\ell + 0.125 \right) \, \mathsf{P} + \frac{1}{10} \omega \, \mathrm{d} \; \ell^2 \; \right\} \times 1.5 \; \times 1.1 \\ & = \left\{ \; \left(0.15 \; \times 1.40 + 0.125 \right) \times 100 \; + \frac{1}{10} \times 4.72 \times 1.40^2 \; \; \right\} \times 1.5 \; \times 1.1 = 56.8^{\mathsf{KNm/m}} \end{split}$$

where:

$$P = 100^{KN}$$
: Single nominal wheel load $\omega d = \text{dead load of deck slab}$

$$= 23.6 \times 0.20 = 4.720^{KN/m}$$

d) over hanging slab

Span
$$\ell = 1.40 - 0.60 - 0.30 - 0.25 = 1.250^{m}$$

moment $M = \frac{P \cdot \ell}{1.30 \ \ell + 0.25} = \frac{100 \times 0.25}{1.30 \times 0.25 + 0.25} = 43.5^{\text{KNm}} < 56.8^{\text{KNm}}$ OK

2. Calculation of stress

a) middle span
$$b = 100^{cm}$$
 $b = 20$ $d = 15.0$ $d' = 5.0$

As $= Y_{12} - 150^{c+c} = 1.131/0.150 = 7.540$ cm²

$$P = \frac{7.540}{100 \times 15.0} \times 100 = 0.503 \%$$

$$\chi = \frac{0.87 \times 41000 \times 7.540}{0.40 \times 3000 \times 100} = 2.4^{cm}$$

$$Z = 15.0 - \frac{1}{2} \times 2.4 = 13.8^{cm} < 0.95 \times 15 = 14.25^{cm}$$

Mrs = $0.87 \times 41000 \times 7.54 \times 13.8 \times 10^{-5} = 37.1^{KNm} > 33.0^{KNm}$

 $M_{RC} = 0.40 \times 3000 \times 100 \times 2.4 \times 13.8 \times 10^{-6} = 39.7^{KNm} > 33.0^{KNm}$

OK

b) each fulcrum
$$b = 100^{cm}$$
 $h = 20$ $d = 16.0$ $d' = 4.0$

$$As = Y_{18} - 150^{cec} = 2.011/0.150 = 13.407 \text{ cm}^2$$

$$P = \frac{13.407}{100 \times 16.0} \times 100 = 0.838 \%$$

$$x = \frac{0.87 \times 41000 \times 13.407}{0.4 \times 3000 \times 100} = 4.0^{cm}$$

$$Z = 16.0 - \frac{1}{2} \times 4.0 = 14.0^{cm} < 0.95 \times 16.0 = 15.2^{cm}$$

$$M_{RS} = 0.87 \times 41000 \times 13.407 \times 14.0 \times 10^{-6} = 66.9^{KNm} > 56.8^{KNm}$$

$$M_{RC} = 0.40 \times 3000 \times 100 \times 4.0 \times 14.0 \times 10^{-5} = 67.2^{KNm} > 56.8^{KNm}$$

Calculation of deck slab (S.L.S) : Check

Span $\ell=1.40^m$ ··· review of fulcrum for bending moment $M=\left\{ \begin{array}{ll} (0.15 \times 1.40 + 0.125) \times 100 & +\frac{1}{10} \times 4.720 \times 1.40^2 \end{array} \right\} = 34.5^{\text{KNm}}$

Calculation of stress

b =
$$100^{\text{cm}}$$
 h = 20 d = 16.0 d' = 4.0
As = $Y_{16} - 150^{\text{cec}}$ = 13.407 cm^2
P = $\frac{13.407}{100 \times 16.0} \times 100$ = 0.838%
X = $\frac{0.80 \times 41000 \times 13.407}{0.25 \times 3000 \times 100}$ = 6.3^{cm}
Z = $16.0 - \frac{1}{3} \times 6.3$ = 12.9^{cm}

 $\begin{aligned} &M_{\text{RS}}\!=\!0.80\!\times\!41000 &\times\!13.407\!\times\!12.9\!\times\!10^{-5}\!=\!56.7^{\text{KNm}}\!\!>\!34.5^{\text{KNm}}\\ &M_{\text{RC}}\!=\!0.25\!\times\!3000\!\times\!100 &\times\!6.3 &\times\!12.9\!\times\!10^{-5}\!=\!60.9^{\text{KNm}}\!\!>\!34.5^{\text{KNm}}\\ &0\text{K} \end{aligned}$

Calculation of Shoe

GirderedSe and Parapet face of abutment 1) quantity of expantion between

```
=(0.80 [ +5.0) mit
                                =(.0.150×L)
                                                                                                                     dC = \frac{P}{E^{x}A} \times \Phi \times L \times b = \frac{750}{27 \times 10^{6}} \times 1.9 \times L \times 08 = (0.430 \times L)
                                                                              ds = axTxLxb=(10x105x200xLx0.8)=(0.160xL)
quantity of expantion or shrinkage (maximum)
                                        for temperature : dt = a \times T \times L = (1.0 \times 10^{5} \times 15.0 \times L)^{-1}
                                                                                  for shrinkage
                                                                                                                       for creep
                                                                                                                                                                  for other
total
```

-coefficient of thermal expantion or shrinkage -quantity of temperature variance - girder length fou = strength of concrete (30 Minm²) = coefficient of decrease =0.5 fcu/2 -0.5×300/2 sulubom sannov = - creep factor

```
UHURU - TU - Bridge
 calculation of shoe
  edge fulcrum Rd = 2508.8 \times 11 \times 1.1
                                                                ÷ 250.9 KN/choe
    (MOV) RL1 = 1634.6/11 \times 1.1
                                                                or RL2 = (465.5 + 372.4 + 215.6 + 126.4 + 83.3) / 5 \div 252.6
     Rmax =
                                                                \div 503.5
             \therefore dL = (0.80L + 5) = (0.80 \times 19.0 + 5)
                                                                = 21 mm
  middle fulcrum Rd = 8084.0 / 11 \times 1.1
                                                                ÷ 808.5 KN/choe
    (Fix) RL1 = 2978.2/11 \times 1.1
                                                                ÷ 297.9
            or RL2 = (690.9 + 599.8 + 400.8 + 280.3 + 178.2) / 5 = 432.0
                Rmax =
                                                                = 1240.5
             dL = 0
```

Notice : this case is abridge and apply MOMBASA-Br

where

edge fulcrum :
$$279 \text{ mm} \times 229 \times 46 --- (A_1.A_2)$$
 (mov)

middle fulcrum :
$$432 \text{mm} \times 203 \times 18 --- (P_1)$$
(Fix)

<u>UHULU</u> — Substructure

Reaction from Superstructure

1) For Abut (Movable) ... S.L.S.

For all width of Abut ($B = 22.700^{m}$)

dead load: Rd = 2508.8 KN

live load: $R \ell = 1634.6$ KN

total : $R = 4143.4^{KN}$

For Unit width of Abut

(1) For Vertical load

$$Rd = \frac{2508.8}{22.70} = 110.520 \text{ KN/m}$$

$$R \mathcal{L} = \frac{1634.6}{22.70} = 72.009$$
 KN/m

$$R = 182.529 \text{ KN/m}$$

(2) For Horizontal force for temperature or seismic

$$H_T = H_D = 110.520 \times 0.15 = 16.578 \text{ KN/m}$$

- 2) For Pier (Fixed) ··· S.L.S.
 - (1) For Vertical load

dead load: Rd = 8084.0 KN

live load: $R Q = 2978.2^{KN}$

total : $R = 11062, 2^{KN}$

- (2) For Horizontal load
 - a) Longitudinal direction

Braking: $H_B = 8.0 \times 38.0 + 200 = 504.0^{KN}$

Seismic: $H_D = (2 \times 2508.8 + 8084.0) \times 0.10 = 1310.2$ KN

b) Crossing direction

Skidding: $H_s = 250^{\kappa N}$

Seismic : $H_D = 8084.0 \times 0.10 = 808.4^{KN}$

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$\widehat{\mathbb{G}}$	SHA	Щ	(1) SHAPE AND SIZE	SIZE				
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	£	. 41	000.0	Ê	12	н	1.100	Ë
	H H	B	0.000	(E)	e E	u	002:0	Ę
	I	u	6.300	(e)	13 14	11	2.400	E
	H	n	0.000	(E)	35	n	0.800	Ę
	H9	u ju	0.800	(E)	BS	11	0.000	5
	B¥1	21	1.000	(E)	HU.1	Ħ	0.500	Ę
	BW2	11	1.000	ê	HU2	a	0.500	Ē
	H&I	Ħ	1.000	(m)				
	HW2	н	1.000	(E)				

NOTE: THE DIMENSION (1) BE EXCHANG TO

DIMENSION (KM) INTO THIS CALCULATION

DEAD LOAD RL = 72,009 (t)	LIVE LOAD RD = 110.520 (t)	ORCE FOR HT = 16.578 (t)	R QH	REACTION RX = 0.350 (m)	TAL FORCE RY = 1.200 (m)	QL = 34.300 (t/m^2)	QD = 0.000 (t/m^2)	FIGIENT KH = 0.10	XHS = 0.00	/EIGHTS	VORETE GAMC = 23.600 (t/m^3)	GAMI	<pre>(UNDERWATER) GAMIS = 10.800 (t/m^3)</pre>	H AT	II 61	(4/m/3)	1
REACTION OF	LIVE LOAD	HORIZONTAL FORCE FOR	TEMPERATURE . SEISMIC	SITUATION OF REACTION	AND HORIZONTAL FORCE			SEISMIC COEFFICIENT		UNITVOLUME WEIGHTS	FOR CONCRETE	FOR BACK FILL	(a) »	IN TERNAL FRICTION ANGLE	FOR ABOY		

0.00 (t/m^2) 0.500 tanøB = FOR FOUNDATION GROUND COHESIVE DOWER FRICTION FACTER ALLOWABLE PRESSURE

= 350.00 (t/m^2)

CALCULATION OF WEIGHT AND FORCE OR LOAD

CONCRETE TO THE PROPERTY OF T

MX(t·m) MY(t·m)	24.284 8.227 361.882 74.142 236.000 3.776	622.167 86.145
Х(m) ј Y(m)	2.450 8.300 2.000 2.500 0.400	
H(t)	0.991 17.653 19.440 1	28.084
۷۲٤	9.912 1 176.528 1 94.400 1	280.840
No.		21

V = X*Y*BW*GAMI NX = V*X

(2) EARTH
3) BACK FILLING

No	V(t)	H(t)	(a) X	ζŒ).	MX(t-m)	MY(t·m)
	65.856 (6.586	3.800	8.300	250.253	54.861
4	319.872	31.987	3.800	4.200	1215.510	134.346
22	385.728 1	38.573			1465.770	189.007

* SURCHAGE OF TOE SLAB

-	- (1)	()	, E	(*) >	((6.4) 2%	(E.+/AN
; }		,,,,,,,,	(m)	1 (111)	/# - 1 \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \
- -	27,900 [0.000	0.750	1.300	20.925	0.000
23	27.900	0.000			20.925	000.0

V n X*Y*BW*GAM1 MX n V*X

(3) REACTION

1				
VIAIE	RV(t)	RH(t)	RMX(t-m)	RMY(t.m)
OBOINARY	407			400
	.62c 28I	000	355 933	000
した アドドスター つくり	182.		355.932	1 145.886
CINCIENT	C 1	16.578	215.514	145.886
֖֓֝֝֞֝֝֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֡֓֓֓֓֓֡֓֓֓֡֓֡֓֡֓֡				

RV : RMX= RV*X

(4) EARTH PRESSURE FACTOR

_]	
<u> </u>	0.3277
SEISMIC	0.3056 1
Y OR RATURE	0.2508 0.2022 0.9793
ORDINARY TEMPER	0.2497 0.5736 0.8192
	S1N(8) COS(8)

(5) EARTH PRESSURE

ا ل ــــ ،	(1) N	H(t)	X(m) 1	Y(m)	MX(t·m)	MY(t·m)
	· —					
-	.44.216		5.000 1	4.500	221.0801	284.16
	113,698 1		5.000 4	3,000 /	568.491	487.13
_	83.224	1 118.857 !	5.000	3.867 1	416.1211	459.579
	1 29.409	1 42.000	5.000 [0.640	147.044	26.89
	72.941	1 231.339	5.000 1	3.000 1	364.705	694.01
	1 53.391	1 169.335 1	5.000 }	3.867	266.955	654.761
	18.867	59.838	5.000 /	0.640	94.334	38.31

(6) BUOYANCY

 V(t)	H(t)	X(m) X	Y(m)	MX(t·m)	MY(t-m)
 52.610	0.0001	3.650	0.0001	338.0271	0.000
 -61.750	0.000	2.4961	0.0001	154.128	0.000

TOTAL OF ACTION FORCE

1. EXCLUDE BUOYANCY

(1) ORDINARY...FOR FOUNDATION

העאם האט	(1)	H(1)	MX(t-m) F MY(t-m)	MY(t·m)
Σ 1	280.840	0.000	622 167	000
Σ 5	385.728	0.000	1465.770	0000
	44.216	63.147	221.080	284.162
	113.698	162.378	568.491	487,134
	182.529	00000	355.932	0.00
i	92.610	000.0	338.027	000.0
2.3	27.900	0.000	20.925	00000
TOTAL	1127.520	225.525	3502 200 5	200 150

 $Mo = \Sigma MX - \Sigma MY = 2821.090 (t·m)$

(2) ORDINARY...FOR INVERSION OR SLIDE

	1 V(E)	H(t)	MX(t-m)	MY(t-m)
SAME	280.840	0.000	622.167	0.000
	44.216	63.147	221.080	284.162
	113.698	162.378 1	568.491	487.134
	110.520	0.000	215.514	000.0
		0.00.0	20.925	000.0
•	962.902	225.525	3113.940	771.296

2342.650 (t-m) - 2MY = Mo = SMX

(3) TEMPERATURE ... FOR FOUNDATION

622.167 1465.770 221.080 568.491 355.932 148		1 V(t)	3(1)	MX(t·m)	MY(t.m)
113.698 162.378 568.491 182.529 16.578 355.932 92.610 0.000 338.027	SAME	385.728	0.000	622.167	0.000
16.578 355.491	-	44.216	63,147	221.080	284.162
0.000 338.027	-	182.529	16.578	355.932	487.134
		27.900	0000	338,027	0000

2675.210 (t·m) $Mo = \Sigma MX - \Sigma MY =$

(4) TEMPERATURE...INVERSION OR SLIDE

	(V(t)	H(t)	MX(t-m)	MY(t-m)
SAME	280.840	0.000	622.167 1	0.000
5	44.216 113.698 110.520	63.147 162.378 16.578 0.000	221.080 568.491 215.514 20.925	284.162 487.134 145.886 0.000
	962.902	242.103	3113.940	917.182

2196.760 (t·m) - 2MY = Mo = SMX

(5) SEISMIC

•	V(t)	H(t)	MX(t·m)	MY(t·m)
SAME	280.840 385.728	28.084	622.167 1	86.145
(1)	72.941 110.520 27.900	231.339 16.578 0.000	364.705 215.514 20.925	694.018 145.886 0.000
	877.929	314.574	2689,080	1115.060

1574.020 (t·m) SMY = Mo # 2MX -

2. INCLUDE BUOYANCY
(1) ORDINARY

1 V(t)	H(t)	MX(t·m)	MY(t·m)
280.840 385.728	0.000	622.167	0.000
2.1	63.147		
1 83.224	œ	416.121	459.879
.40			56.
ເຄ	0.000	5.93	0
.61	0.000	φ.	
6	0.000	0.92	
١. ١	00000	4.12	0.000
1 1064.710	224.004	3432.930	770.637

 $Mo = \Sigma MX - \Sigma MY = 2662.300 (t·m)$

(2) ORDINARY

V(t)	HCt)	MX(t-m)	MY(t·m)
280.840 385.728	0.000.0	622.167 1	0.000
44.216	63.147	221.080	-1 -1
83.224	118.857 }	416.121	459.579
29.409	42.000		26.
110.520	0.000	'n	Ö
27.900	0.000		0.000
-61.750	0.000	-154.128	0.000
900.087	224.004	2954.490	770.637

(3) TEMPERATURE

	1	H(t)	MX(t·m)	MY(t·m)
; ;	280.840 1 385.728	0.000	622.167	0.000
-		က	221 080	
	i 83.224	118.857)	450 F 10 C
		1 42.000 1	,	26.896
	•	16.578	ທ	145 886
		000.0	8.02	000.0
	•	0.00.0	0.32	000.0
	1 -61.750	000-0	-154.128	0.000
	1064.710	240,582	3432.930	916.523

 $Mo = \Sigma MX - \Sigma MY = 2516.410 (t-m)$

(4) TEMPERATURE

	l V(t)	H(t)	MX(t·m)	MY(t-m)
· _ ·	280.840	0.000	1 ~4	0.000
	~	000.0	1465.770	0.000
	•	63.147	221.080	284.162
	83.224	118.857	416.121	٠,
	29.409	42.000	147.044	
	ö	•		
	•	0.000	20.92	000
	1 -61.750 1	0.000	4.12	000.0
	1 900.087	240.582	2954.490	916.523

 $Mo = \Sigma MX - \Sigma MY = 2037.970 (t·m)$

2183.850 (t·m)

Mo = SMX + SMY =

(3) SEISMIC

	V(t)	H(t)	MX(t·m)	MY(t.m)
	∞ ∞	28.084	2.16	86.145
:	385.728	8.57	1465.770	189.007
	3.39	169.335	6.9	.1
	18.867	59.838	G	38.318
7.	0.52	Ġ	S.	ຜ
	06	ં	20.9	Ö
	1.75	0.000	1.1	•
*	815.496	312.407	2531.530	1114.120

 $Mo = \Sigma MX - \Sigma MY = 1417.420 (t.m)$

TOTAL FORCE FOR UNDER FOUNDATION CENTER

Γ.	
Mc(t-m)	-2.289 64.608 143.598 210.494 620.801 -0.533 66.354 212.353
e(m)	0.000000000000000000000000000000000000
Mo(t-m)	2821.090 2342.650 2675.210 1574.020 1574.020 2662.300 2183.850 2516.410
H(t)	225.525 225.525 242.103 314.574 224.004 2240.582 240.582
v(t)	1127.520 962.902 1127.520 962.902 877.929 1064.710 1064.710 900.087 815.496
LOAD	. и помать чомать

WHERE

A AND B:EXCLUDE OF BOUYANCY OR INCLUDE BOUYANCY

1. ORDINARY : FOR FOUNDATION

2. . FOR INVERSION OR SLIDE

3, TENPERATURE: STATE OF 1

5. SEISMIC

CALCULATION OF SECURITY FOUNDATION

	F :	т		_			<u>-</u>		
			n	833	Ö		c	m	667
	("		0	0	~		0	0	
	e(m)		~	V	Y		٧.	Y	~
			067	213	707		074	236	762
•			0	Ö	0	:	ó	ò	Ö
								-	
	Ē		608	494	801		9	250	C1
NO	Mc(t-m)		Ó	210.	S			213.	61
S	-				<u> </u>		_		
α	_		903	900	929		087	087	196
INVERSION	V(t)		Ó	962.			900.	900.	815.
FOR			- -	_ ~	m 	·	(~ N	m
ŀ		٠.							

" Mc/V

B0/6 B0/3

			_	_		-	-		
	l "		^	^	^	-	^	^	
	Ŗ	 	2.13	1.99	1.40		1.87	1.31	
	Hu(t)		1.4	481.45	438.971	C	000	07.	C i
	Hb(t)		25.52	242.1031	14.57	4	240.582	12	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \
	V(t)		962.9021	962.9021	۲.	900.087	900.0871	815.496	1
SLIDE	I A'(m^2)		1 4.871	4.56	166.6	- 00	4.53	4	(6,4/4) 00 0
TOR			, 	(Vi	m	-	N	m	ا ا
[_]	⋖	<u>.</u>	_		n 			

 $Hu = C*A' + V*tan(\phi B)$

Fs = Hu/Hb

FOR CONTACT PRESSURE UNDER FOUNDATION

EXCLUDE BOUYANCY STATE 1 3 5 5 5 B (m) 5.000 5.000 1.000 L (m) 1.27.520 1.27.520 877.929 H (t) 225.525 242.103 314.574 Mc (t·m) -0.002 0.127 0.707 X (m) 5.000 5.000 5.000 Qmax(t/m^2) 226.054 259.958 324.578 Qmin(t/m^2) 224.955 191.041 26.594
(m) 5.000 5. (m) 1.27.520 1127. (t) 225.525 242. (t·m) -0.002 0. (m) 5.000 5. (m) 5.000 5. (m) 5.000 5. (m) 5.000 5. (t/m^2) 226.054 259. n(t/m^2) 224.955 191.
(m) (t) (c) (m) (c) (m) (d) (d) (d) (d) (d) (d) (d) (d) (d) (d
(#) (#) (#) (#) (#) (#) (#) (#)
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5.000 5.000 1.000 1.000	34.710 815.496 10.582 312.407 15.354 621.323	0.137 0.762 5.000 5.000	826 312.217 056 13.982
5.000	71.58	137	(3) (2)
	1064 240 145	0.0	247.8
1.000	1064.710 224.004 -0.533	-0.001	213.069 212.813
(a)	V (t) H (t) Mc (t·m)	e (m) X	Qmax(t/m^2) Qmin(t/m^2)
	(E)	(m) (m) (t) 106 (t) 22 (t·m) 1	(m) (t) 106 (t) 22 (t·m) 1 –

 $Q = V/(B*L) + 6*Mc/(L*B^2)$

Q = 2*V/(L*X) : X = 3*(BO/2-Mc/V)

<u>UHURU</u> ABUT

Calculation for vertical wall. U.L.S

1. Action force

(1) state of normal load ... only earth pressure

$$M_{1} = \frac{1}{6} \times 19.6 \times 0.251 \times 8.20^{3} \times 1.5 \times 1.1 = 746.0^{\text{KN} \cdot \text{m}}$$

$$M_{2} = \frac{1}{2} \times 34.3 \times 0.251 \times 8.20^{2} \times 1.5 \times 1.1 = 477.6^{\text{KN} \cdot \text{m}}$$

$$Mu = = 1223.6^{\text{KN} \cdot \text{m}}$$

$$S_{1} = \frac{1}{2} \times 19.6 \times 0.251 \times 8.20^{2} \times 1.5 \times 1.1 = 272.9^{\text{KN}}$$

$$S_{2} = 34.3 \times 0.251 \times 8.20 \times 1.5 \times 1.1 = 116.5^{\text{KN}}$$

$$S_{3} = 34.3 \times 0.251 \times 8.20 \times 1.5 \times 1.1 = 389.4^{\text{KN}}$$

(2) State of Temperature and normal load

$$Mu = 1223.6 + 16.578 \times 8.00 \times 1.30 \times 1.1 = 1413.3^{KN-m}$$

$$Su = 389.4 + 16.578 \times 1.3 \times 1.1 = 413.1^{KN}$$

(3) State of seismic

$$Mu = (\frac{1}{6} \times 19.6 \times 0.328 \times 8.20^{3} + 16.578 \times 8.00) \times 1.35 \times 1.1 = 1000.4^{\text{KN} \cdot \text{m}}$$

$$Su = (\frac{1}{2} \times 19.6 \times 0.328 \times 8.20^{2} + 16.578) \times 1.35 \times 1.1 = 345.6^{\text{KN}}$$

2. Calculation of stress for U.L.S.

section b =
$$100^{\text{cm}}$$
 h = 110 d = 103.0 d' = 7.0

As = $Y_{32} - 150^{\text{ctc}} = 8.042 / 0.15 = 53.613$ cm²

P = $\frac{\text{As}}{\text{bd}} \times 100$ = $\frac{53.613}{100 \times 103.0} \times 100$ = 0.520%
 $\chi = \frac{0.87 \text{fy} \cdot \text{As}}{0.40 \text{fcu} \cdot \text{b}} = \frac{0.87 \times 41000 \times 53.613}{0.40 \times 2500 \times 100}$ = 19.2^{cm}

Z = $d - \frac{\chi}{2} = 103.0 - \frac{19.2}{2} = 93.4^{\text{cm}} < 0.95 d = 0.95 \times 103.0 = 97.8^{\text{cm}}$ OK

Mrs = $0.87 \text{fy} \cdot \text{As} \cdot \text{Z} = 0.87 \times 41000 \times 53.613 \times 93.4 \times 10^{-5}$ = $1786.1^{\text{KNm}} > \text{Mu} = 1413.3^{\text{KNm}}$

Mrc = $0.40 \text{fcubx} \text{Z} = 0.40 \times 2500 \times 100 \times 19.2 \times 93.4 \times 10^{-5}$ = $1793.3^{\text{KNm}} > \text{Mu} = 1413.3^{\text{KNm}}$ OK

Vc = $\frac{\text{Su}}{\text{bd}} = \frac{413.1 \times 10^3}{100 \times 103.0} = 40.1 \text{ N/cm}^2$
 $< \text{V ca} = 50.0 + 15.0 \quad (\frac{0.520 - 0.50}{0.50}) = 50.6 \text{ N/cm}^2$ OK

<u>UHURU</u> — ABUT

Calculation of stability for S.L.S.

1) action force for bottom of Foundation

(from output of computer)

load state	N _{KN}	Нки	MKNm	
Normal	1127.6	225.6	$-2.3 \ \ \div \ \ 0$	
Temperature	1127.6	242.1	143.6	
Seismic	878.0	314.6×0.80 = 251.7	* 398.0	

$$\times \left\{ \frac{5.00}{2} - (2689.1 - 1115.1 \times 0.80) / 878.0 \right\} \times 878.0 = 398.0^{\text{KNm}}$$

2) stability for Foundation

a) Normal state

$$e = \frac{M}{N} = 0$$
 $q = \frac{N}{B} = \frac{1127.6}{5.0} = 225.6 \text{ KN/m}^2 < qa =$
 $Fs = \frac{N \cdot \mu}{H} = \frac{1127.6 \times 0.50}{225.6} = 2.5 > 1.5$ OK

b) Temperature state

$$e = \frac{M}{N} = \frac{143.6}{1127.6} = 0.128^{m} < \frac{B}{6} = \frac{5.00}{6} = 0.833^{m}$$

$$q = \frac{N}{B} \left(1 \pm \frac{6e}{B} \right) = \frac{1127.6}{5.00} \left(1 \pm \frac{6 \times 0.128}{5.00} \right) = \left(\frac{260.2 \text{ KN/m}^{2}}{190.9 \text{ KN/m}^{2}} \right) = \frac{N \cdot \mu}{H} = \frac{1127.6 \times 0.50}{242.1} = 2.3 > 1.5$$

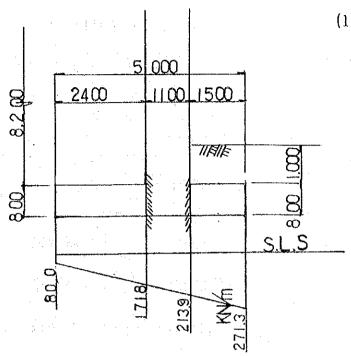
$$0 \times \mu = \frac{N \cdot \mu}{H} = \frac{1127.6 \times 0.50}{242.1} = 2.3 > 1.5$$

c) seismic state

$$e = \frac{M}{N} = \frac{398 \cdot 0}{878 \cdot 0} = 0.454^{m} < \frac{B}{6} = 0.833^{m}$$

$$q = \frac{N}{B} \left(1 \pm \frac{6e}{B} \right) = \frac{878 \cdot 0}{5.00} \left(1 \pm \frac{6 \times 0.454}{5.00} \right) = \left(\frac{271 \cdot 3}{80 \cdot 0} \frac{\text{KN/m}^{2}}{\text{KN/m}^{2}} \leq qa = \frac{878 \cdot 0}{80 \cdot 0} \right)$$

3) Calculation of action force for each section ... Seismic state



- (1) Surcharge load
 - a) toe footing slab

$$\omega = 23.6 \times 0.80 + 18.6 \times 1.00 = 37.480^{\text{KN/m}}$$

b) heel footing slab

$$\omega = 23.6 \times 0.80 + 19.6 \times 8.20 = 179.6^{\text{KN/m}}$$

(2) Calculation of bending moment and shearing force

a) toe footing slab

$$M = \frac{1.50^{2}}{6} (2 \times 271.3 + 213.9) - \frac{1.50^{2}}{2} \times 37.480 = 241.6^{\text{KNm}}$$

$$S = \frac{1.50}{2} (271.3 + 213.9) - 1.50 \times 37.480 = 307.7^{\text{KN}}$$

b) heel footing slab

$$M = \frac{2.40^{2}}{2} \times 179.6 - \frac{2.40^{2}}{6} (2 \times 80.0 + 171.8) = 198.8^{\text{KNm}}$$

$$S = 2.40 \times 179.6 - \frac{2.40}{2} (80.0 + 171.8) = 128.9^{\text{KN}}$$

Calculation of stability for U.L.S.

1) action force for bottom of Foundation

load state	Ики	Н ки	M KNm
Normal	$ \begin{array}{c} 1127.6 \times 1.2 \times 1.15 \\ = 1556.1 \end{array} $	$\begin{array}{c} 225.6 \times 1.5 \times 1.1 \\ = 372.3 \end{array}$	※ 1 205.4
Temperature	1556.1	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	※2 446.2
Seismic	$\begin{array}{c} 878.0 \times 1.2 \times 1.15 \\ = 1211.7 \end{array}$	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	※ 3 974.3

2) Stability of Foundation

a) Normal state

$$e = \frac{205.4}{1556.1} = 0.132^{m} < \frac{B}{6} = \frac{5.00}{6} = 0.833^{m}$$

$$q = \frac{1556.1}{5.00} (1 \pm \frac{6 \times 0.132}{5.00}) = \begin{pmatrix} 360.6 & \text{KN/m}^{2} \\ 262.0 & \text{KN/m}^{2} \end{pmatrix} < qa = Fs = \frac{1556.1 \times 0.50}{372.3} = 2.1 > 1.1$$

b) Temperature state

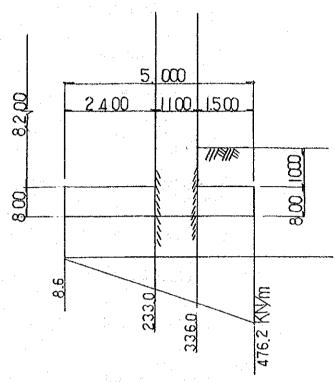
$$e = \frac{446.2}{1556.1} = 0.287^{m} < \frac{B}{6} = 0.833^{m}$$

$$q = \frac{1556.1}{5.00} (1 \pm \frac{6 \times 0.287}{5.00}) = \begin{pmatrix} 418.4 \text{ KN/m}^{2} \\ 204.1 \text{ KN/m}^{2} \end{pmatrix} < qa = Fs = \frac{1556.1 \times 0.50}{399.5} = 1.9 > 1.1$$

c) Seismic state

e =
$$\frac{974.3}{1211.7}$$
 = 0.804^m < $\frac{B}{6}$ = 0.833^m
q = $\frac{1211.7}{5.00}$ (1 ± $\frac{6 \times 0.804}{5.00}$) = $\begin{pmatrix} 476.2 \text{ KN/m}^2 \\ 8.6 \text{ KN/m}^2 \end{pmatrix}$ = qa = Fs = $\frac{1211.7 \times 0.50}{467.2}$ = 1.3 > 1.1 0K

3) Calculation of action force for each section ... Seismic state



- (1) Surcharge load
 - a) toe footing slab

$$\omega = (23.6 \times 0.80 + 18.6 \times 1.00) \times 1.380$$

b) heel footing slab

$$\omega = (23.6 \times 0.80 + 19.6 \times 8.20) \times 1.38$$

 $=247.848^{KN/in}$

 $=51.722^{KN/m}$

- (2) Calculation of bending moment and shearing force
 - a) toe footing slab

$$M = \frac{1.50^{2}}{6} (2 \times 476.2 + 336.0) - \frac{1.50^{2}}{2} \times 51.722 = 425.0^{\text{knm}}$$

$$S = \frac{1.50}{2} (476.2 + 336.0) - 1.50 \times 51.722 = 531.6^{\text{kn}}$$

b) heel footing slab

$$M = \frac{2.40^{2}}{2} \times 247.848 - \frac{2.40^{2}}{6} (2 \times 8.6 + 233.0) = 473.7^{\text{KNm}}$$

$$S = 2.40 \times 247.848 - \frac{2.40}{2} (8.6 + 233.0) = 305.0^{\text{KN}}$$

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Calculation of stress for footing slab (U.L.S)

section
$$b = 100^{cm}$$
 $h = 80$ $d = 73.5$ $d' = 6.5$

$$As = Y_{25} - 150^{c+c} = 4.909 / 0.150 = 32.727 \text{ cm}^2$$

$$P = \frac{As}{bd} \times 100 = \frac{32.727}{100 \times 73.5} \times 100 = 0.445 \%$$

$$x = \frac{0.87 \text{ fy} \cdot \text{As}}{0.40 \text{ fcu} \cdot \text{b}} = \frac{0.87 \times 41000 \times 32.727}{0.40 \times 2500 \times 100} = 11.8^{cm}$$

$$Z = d - \frac{x}{2} = 73.5 - \frac{11.8}{2} = 67.6^{m} < 0.95 d = 0.95 \times 73.5 = 69.8^{cm}$$

$$M_{RS} = 0.87 \text{ fyAs} \cdot Z = 0.87 \times 41000 \times 32.727 \times 67.6 \times 10^{-5}$$

$$= 789.1^{KNm} > Mu = 473.7^{KNm}$$

$$M_{RC} = 0.40 \text{ fcu} \cdot \text{bxZ} = 0.40 \times 2500 \times 100 \times 11.8 \times 67.6 \times 10^{-5}$$

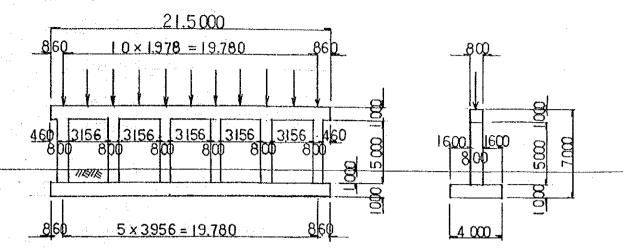
$$= 797.6^{KNm} > Mu = 473.7^{KNm} \text{ oK}$$

$$Vc = \frac{Su}{bd} = \frac{531.6 \times 10^{3}}{100 \times 73.5} = 72.4 \text{ N/cm}^{2}$$

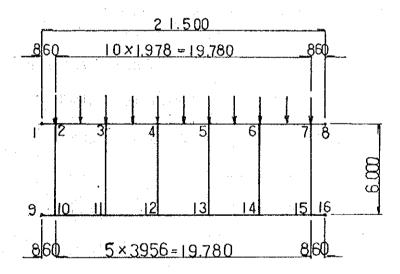
$$Vca = 35.0 + 15.0 \quad (\frac{0.445 - 0.25}{0.25}) \times 2 = 93.4 \text{ N/cm}^{2} \text{ OK}$$

Crossing direction

1) Shape and size



2) Frame



3) factor of section

beam
$$A = 0.80 \times 1.00 = 0.8000 \text{ m}^2$$

$$I = \frac{0.80 \times 1.00^3}{12} = 0.06667 \text{ m}^4$$
pillar $A = 0.80 \times 0.80 = 0.6400 \text{ m}^2$

$$I = \frac{0.80^4}{12} = 0.03413 \text{ m}^4$$
footing $A = 4.00 \times 1.00 = 4.0000 \text{ m}^2$

$$I = \frac{4.00 \times 1.00^3}{12} = 0.33333 \text{ m}^4$$
Ec = $2.5 \times 10^7 \text{ KN/m}^2$

4) load

a) Reaction from Superstructure

Dead load

$$Rd_1 \sim d_{11} = 8084.0 / 11 \text{ Girder} = 734.910 \text{ kN/shoe}$$

Live load

 $R\ell_{1\sim 4} = R\ell_{8\sim 11} = = 140.0 \text{ kN/shoe}$
 $R\ell_5 = R\ell_6 = R\ell_7 = (2978.2 - 8 \times 140.0) / 3 = 619.4 \text{ kN/shoe}$

b) Dead load of Substructure

beam	$\omega d_i = 23.6 \times 1$	$1.00\times0.80 =$	18.880 KN/m
	$\omega d_2 = 23.6 \times 0$	$0.80 \times 0.80 =$	15.104 KN/m
footing	$\omega d_3 = 23.6 \times 1$	= 1.00×4.00	94.400 KN/m
surcharge	$\omega d_4 = 18.6 \times 1$	1.00×4.00	74.400 KN/m

c) Seismic state

Horizontal load

Superstructure	$Hd = 8084.0 \times 0.10$	=	808.4	KN
	$Md = 808.4 \times 1.20 / 21.50$	=	45.120	KNm/m
beam	$Hd_1 = 18.880 \times 0.10$	=	1.888	KN/m
pillar	$H d_z = 15.101 \times 0.10$	==	1.511	KN/m
footing	$Hd_3 = 94.400 \times 0.10$	===	9.440	K N / m

d) Temperature

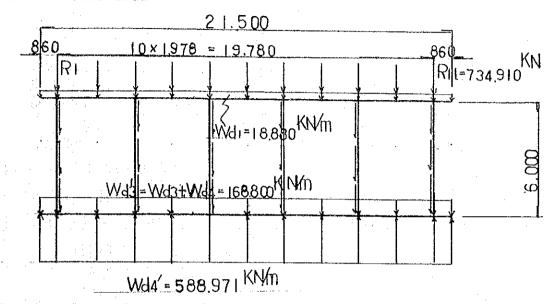
T = 12.5 °C

$$\alpha$$
 = 1.2×10⁻⁵
E c = 2.5×10⁷ KN/m²

UHURU — PIER

Loaded figure

Case-1 Dead load



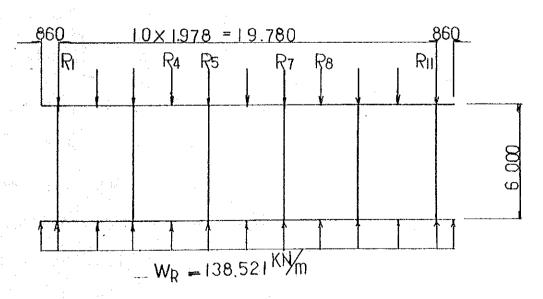
$$\omega d = 588.971 - 168.800$$

= 420.171 KN/m

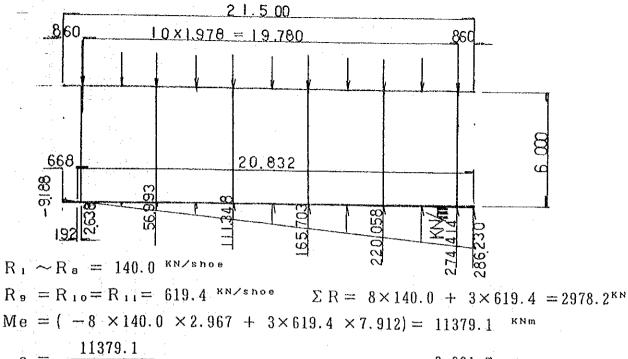
$$\omega d_4' = (734.910 \times 11 + 18.880 \times 21.50 + 15.104 \times 6.00 \times 6) / 21.50 + 168.800$$

= 420.171 + 168.800 = 588.971 KN/m

Case-2 Live load



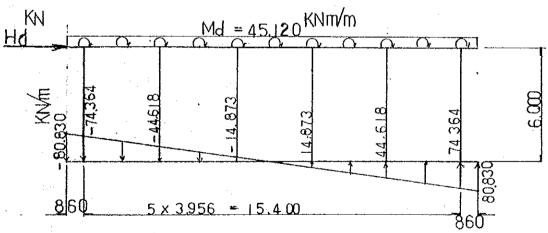
$$R_{1\sim4} = R_{8\sim11} = 140.0 \text{ KN/shoe}$$
 $R_{5} = R_{6} = R_{7} = 619.4 \text{ KN/shoe}$
 $\omega_{R} = (140.0 \times 8 + 619.4 \times 3) / 21.50 = 138.521 \text{ KN/m}$



$$e = \frac{11379.1}{2978.2} = 3.821^{m}$$

$$\omega_{Ri} = \frac{2978.2}{21.50} (1 \pm \frac{6 \times 3.821}{21.50}) = \begin{pmatrix} \omega_{Ri} = 286.230^{\text{KN/m}} \\ \omega_{R2} = -9.188^{\text{KN/m}} \end{pmatrix}$$

Case-4 Seismic state



load : Hd =
$$808.4 + 21.50 \times 1.888 = 848.992$$
 KN

$$M = 45.120 \times 21.50 + \frac{6.00^{2}}{2} \times 1.511 \times 6 + 848.992 \times 6.00 = 6227.220^{\text{KNm}}$$

$$q = \pm \frac{6 M}{B^{2}} = \pm \frac{6 \times 6227.220}{21.50^{2}} = \pm 80.830^{\text{KN/m}}$$

Case-5

Temperature

$$t = 12.5 \text{ °C}$$
 $\alpha = 1.2 \times 10^{-5}$
 $Ec = 2.5 \times 10^{7} \text{ Kg/m}^{2}$

	X	Υ
	(m)	(m)
1	0.0000	6.0000
2	0.8600	6.0000
- 3	4.8160	6.0000
4	8.7720	6.0000
5	12.7280	6.0000
5 6	16.6840	6,0000
7	20.6400	6.0000
8	21.5000	6,0000
9	0.0000	0.0000
10	0.8600	0.0000
11	4.8160	0.0000
12	8.7720	0.0000
13	12.7280	0.0000
14	16.6840	0.0000
15	20.6400	0.0000
16	21.5000	0.0000

NOTE: THE DIMENSION(L) BE EXCHANG TO DIMENSION(KN) INTO THIS CALCULATION

			-				
No	I J	A (m2)	1 (m4)	I J	L (m)	E (t/m2)	EPS
1 2	1 - 2 2 - 3	0.80000	0.066670	Fix - Fix	0.860	2.50E+07	1.20E-05
		0.80000	0.066670	Fix - Fix	3.956	2.50E+07	1.20E-05
3	3 - 1	0.80000	0.066670	Fix - Fix	3.956	2.50E+07	1.20È-05
4.	4 - 5	0.80000	0.066670	Fix - Fix	3 956	2.50E+07	1.20E-05
5	5 - 6	0.80000	0.066670	Fix - Fix	3 956	2.50E+07	1.20E-05
. 6	6 - 7	0.80000	0.066670	Fix - Fix	3.956	2.50E+07	1.20E-05
7	7 - 8	0.80000	0.066670	Fix - Fix	0.860	2.50E+07	
. 8	9 - 10	4.00000	0.333330	Fix - Fix	0.860		1.20E-05
g	10 - 11	4.00000	0.333330			2.50E+07	1.20E-05
. 10	11 - 12	4.00000			3.956	2.50E+07	1.20E-05
			0.333330	Fix - Fix	3.956	2.50E+07	1.20E-05.
1 t	12 - 13	4.00000	0.333330	Fix - Fix	3.956	2.50E+07	1.20E-05
12	13 - 14	4.00000	0.333330	Fix - Fix	3.956	2.50E+07	1.20E-05
-13	14 - 15	4.00000	0.333330	Fix - Fix	3.956	2.50E+07	1.20E-05
14	15 - 16	4.00000	0.333330	Fix - Fix	0.860	2.50E+07	1.20E-05
15	2 - 10	0.64000	0.034130	Fix ~ Fix	6.000	2.50E+07	
16	3 - 11	0.64000	0.034130	Fix - Fix			1.20E-05
17	4 - 12				6.000	2.50E+07	1.20E-05
		0.64000	0.034130	Fix ~ Fix	6.000	2.50E+07	1.20E-05
18	5 - 13	0.64000	0.034130	Fix - Fix	6.000	2.50E+07	1.20E-05
19	6 - 14	0.61000	0.034130	Fix - Fix	6.000	2.50E+07	1.20E-05
20	7 - 15	0.64000	0.034130	Fix - Fix	6.000	2.50E+07	1.20E-05

No			
	X (1/m)	Y (t/m)	M(lm/Rad)
9	Fix	Fix	Free
16	Free	Fix	Free

				•	*						
No		L-No 1	L-No 2	L-No 3	L-No 4	L-No 5	L-No 6	L-No	L-No 8	L-No	L-No 10
		11	12	13	14	15	16	17	18	19	20
	7										
1 2	0	•									
2	3	0.989	1.978	2.967						•	
3	3	0.989	1.978	2.967							
4	3	0.989	1.978	2.967							
5	3	0.989	1.978	2.967							
6 7	3∷	0.989	1.978	2,967							
7	0		term of the								
8	0 -										
. 9	3	0.989	1.978	2.967		*					
10	3	0.989	1.978	2.967							
11,	3	0.989	1.978	2.967							
12	3	0.989	1.978	2.967							
13	3	0.989	1.978	2.967							
14	0				•						
15	3	1.500	3.000	4.500							
16 :	3 '	1.500	3.000	4.500		1 1					
17	3	1.500	3.000	4.500							
18	3	1.500	3.000	4.500							
19	3	1.500	3.000	4.500							
20	3	1.500	3,000	4.500							

: Dead Load : 1 : 1

No. No.

No	i - j		Li (m)	Lo (m)	Pi_(t/m)	Pj (t/m)
2	2- 3	- Y	0.001			
2	2- 3		0.001		-734.910	
.3	3- 4	, I	1.978		-734.910	
3	3- 4	- Y			-734.910	
.4	4- 5	- у			-734.910	
4	-1 · 5	- Y	0.001		-734.910	
4	4- 5	' Y	1.978		-734.910	
- 5		- Y	3.955		-734.910	
-5					-734.910	
		- Y	3.955		-734.910	
- 6	6- 7	Ү	1.978		~734.910	
. 6	6- 7	~ Y	3.955		-734.910	
1	1- 2	- Y	0.000	0.860	-18.880	-18.880
3	2-3	- ү	0.000	3.956	-18.880	-18.880
ં	3- 4	- Y	0.000	3.956	-18.880	~18.880
4	4- 5	Y	0.000	3.956	-18.880	-18.880
5	5- 6	-Y	0.000	3.956	-18.880	-18.880
6	6- 7	~ Y	0.000	3.956	-18.880	-18.880
7	7- 8	- Y	0.000	0.860	-18.880	-18.880
8 9	9- 10	~Y		0.860	420.171	420.171
	10- 11	· ~ү		3.956	420.171	420,171
10	11- 12	. – Y	0.000	3.956	420.171	420.171
11	12- 13	-Y	0.000	3.956	420.171	420.171
12	13- 14	- ү	0.000	3.956	420,171	420.171
13	14- 15	-Y	0.000	3.956	420.171	420.171
14	15- 16	. – ү	0.000	0.860	420.171	420.171
15	2- 10	- Y	0.000	6.000	-15.104	-15.104
16	3- 11	~ Y	0.000	6.000	-15.104	-15.104
17	4- 12	- Y	0.000	6.000	-15.104	-15.104
18	5- 13	- Y	0.000	6.000	-15.104	-15.104
19	6-14	Y	0.000	6.000	-15.104	-15.104
20	7- 15	- Y	0.000	6.000	-15.104	-15.104
		•			23.104	131104

Σ V = Σ H = 0.002 (t) 0.000 (t)

UHURU PIER(F)

: Live Load : 2 : 1

No. No.

No	i - j		Li (m)	Lo (m)	Pi (t/m)	Pj (t/m)
2	2- 3	- Y	0.001		-140.000	
2	2- 3	- Y	1.978		-140.000	
3	3- 4	- Y	0.001		-140,000	
	3- 4	- Y	1.978		-140.000	•
4	4- 5	- Y	0.001		-619.400	
3 4 4 4 5 5	4 5	- Y	1.978		-619.400	
4	4- 5	 γ	3.955		-619.400	
5	5- 6	- Y	1.978		-140.000	
6	5- 6	~ Y	3.955		-140.000	
6	6- 7	~ Y	1.978		-140.000	
6	6- 7	- Y	3.955		-140.000	
8	9- 10	- Y	0.000	0.860	138.521	138.521
8 9	10- 11	-Y	0.000	3.956	138.521	138.521
10	11- 12	-Y	0.000	3.956	138.521	138.521
11	12- 13	- Y	0.000	3.956	138.521	138.521
12	13- 14	- Y	0.000	3.956	138.521	138.521
13	14- 15	- Y	0.000	3.956	138.521	138.521
14	15- 16	~Y	0.000	0.860	138.521	138.521

0.001 (t) 0.000 (t) Σν = ΣΗ =

: Live Load : 3 : 1

. No	i -j		Li (m)	Lo (m)	Pi (t/m)	Pj (t/m)
2	2- 3	· - Y	0.001		140.000	
2	2- 3	- Ý			-140.000	
3	3- 4		1.978		-140.000	
3	3- 4	~ Y	0.001		-140.000	
4	4- 5	- Y	1.978		-140.000	
		~ Y	0.001		-140.000	
4 5	4~ 5	- Y	1.978		~140.000	
5	5- 6	Y	0.001		-140.000	
5	5- 6	- Y	1.978		-140.000	
6	6- 7	- Y	0.001		-619,400	
6	6~ 7	- Y	1.978		-619.400	
6	6-7	· - Y	3.955			
8	9- 10	- Ÿ	0.000	0.000	-619.400	
8	9- 10	- Ŷ		0.668	-9.188	0.000
9	10- 11		0.668	0.192	0.000	2.638
		- Y	0.000	3.956	2.638	56.993
10	11- 12	~ Y	0.000	3.956	56.993	111.348
11	12- 13	- Y	0.000	3.956	111.348	165.703
12	13- 14	- Y	0.000	3.956	165.703	220.058
13	14- 15	~ Y	0.000	3.956	220.058	274.414
14	15- 16	- Y	0.000	0.860	274.414	286.230
		and the second s				

0.098 (t) 0.000 (t)

UHURU PIER(F)

: Seismic State : 4 : 1

No.

No X (t) Y (t) M (tm) 848.992 0.000 0.000

No	i ~j		Li (m)	Lo (m)	Pi (t/m)	Pj (t∕m)
1	1- 2	- M	0.000	0.860	45.120	45.120
	2- 3	-M	0.000	3.956	45.120	45.120
- 3	3- 4	-M	0.000	3.956	45.120	45.120
4	4- 5	- M	0.000	3.956	45.120	45.120
5	5~ 6	-M	0.000	3.956	45.120	45.120
2 3 4 5 6 7	6- 7	-M	0.000	3.956	45.120	45.120
7	7- 8	- M	0.000	0.860	45.120	45.120
8	9- 10	- Y	0,000	0.860	-80.830	-74.364
8 9	10- 11	~ Ŷ	0.000	3.956	-74.364	-44.618
10	11- 12	- Ÿ	0.000	3.956	-44.618	-14.873
11	12- 13	- Y	0.000	1.978	-14.873	0.000
11	12-13	-Ÿ	1.978	1.978	0.000	14.873
12	13- 14	- Ŷ	0.000	3.956	14.873	44.818
13	14- 15	- Ÿ	0.000	3.956	44.818	74.364
14	15- 16	-Ŷ	0.000	0.860	74.364	80.830
15	2- 10	- x	0.000	6.000	1.511	1.511
16	3-11	- X	0.000	6.000	1.511	1.511
17	4- 12	- X	0.000	6.000	1.511	1.511
18	5- 13	-X	0.000	6.000	1.511	1.511
19	6- 14	- X	0.000	6,000	1.511	1.511
20	7- 15	~ X	0.000	6.000	1.511	1.511

0.791 (t) 903.388 (t)

UHURU PIER(F)

: Temperature
No. : 5
No. : 1

No TO No T (°C)

 $\Sigma V = 0.000 (t)$ $\Sigma H = 0.000 (t)$

UHURU PIER(F)

No C-No 1 C-No 2 C-No 4 No 9 C-No 3 C-No 5 C=No 6. C-No 7 C-No 8 No No 6 No 7 No 8 No 10 No 11 No12 No13 α 1.0000 1.0000 1.0000 1.0000 1.0000 1.0000 1.0000 1.0000 No 1.3800 1.3800 1.3800 1.3800 1.3800 1.0000 1.0000 1.0000 3 No 1.6500 0.0000 0.0000 1.3800 0.0000 1.0000 0.0000 0.0000 0.0000 No 1.6500 0.0000 0.0000 1.3800 1.0000 0.0000 0.0000 0.0000 Nο 4 0.0000 1.6500 0.0000 0.0000 0.0000 0.0000 1.0000 No 5 0.0000 0.0000 0.0000 1.4300 1.4300 0.0000 0.0000 0.0000 No C-No 9 C-No10 No No 14 No15 α 1.0000 1.0000 1.0000 No 1 2 3 1.0000 No 0.0000 No 0.0000 1.0000 No 0.0000 0.0000

UHURU PIER(F)

No

No 1 : 6 7 8 9 10 No 2 : 11 12 13 14 15

1.0000

1.0000

No.	Case, 1 RX (t)	RY (t)	RM (tm)	Case.	2 RY (t)	RM (tm)	Case. RX (t)	3 RY (t)	RW (tm)
6 P	0.000	-0.001	0.000	0.000	-0.001	00000	0.000	-0.058	0.000
No.	Case. 4 RX (t)	RY (t)	RM (tm)	Case.	5 RY (t)	RM (tm)	Case. RX (t)	6 RY (t)	RM (tm)
16.	-903.388	90.066	0.000	0.000	0.000	0.000	000.0	-0.003	0.000
No.	Case. 7 RX (t)	RY (t)	RM (tm)	Case. RX (t)	8 RY (t)	RM (tm)	Case.	9 RY (t)	RM (tm)
 	0.00.0	-0.097	0.000	-1490.590	148.607	0.00.0	000.0	-0.003	0.000
o Z	Case. 10 RX (t)	RY (t)	RM (tm)	Case. RX (t)	11 RY (t)	RM (tm)	Case. RX (t)	12 RY (t)	RM (tm)
96.	00000	-0.082	0000.0	0.000	-0.002	0.000	0.00.0	-0.059	0.000
	Case. 13 RX (t)	RY (t)	RM (tm)	Case. RX (t)	14 RY (t)	RM (tm)	Case. RX (t)	15 RY (t)	RM (tm)
. 9. 16.	-903.388	90.065	00000	0.000	-0.002	0.000	0.000	-0.059	0.000

1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	mmnad)	30	330	182	105	69	56		80	141	141	60	.66	05	. 90	18	82			mmRad)	78	95	1493	45	45	93	9.5	78	89	86	62	71 .	7.1	62	86	68
i c	200	0.1630	0.16	0.26	0.14	-0.13	-0.4]	-0.02	-0.02	0.25	0.25	0.30	0.16	-0.12	-0.30	-0.38	-0.35			ROTA. (mmRad	-0.707	-0.709	٧.	4.	4.	Υ.			64	C1	w	e.	0.30	ഗ	N	C1
8 2 3	2	0.07028									0.25294								₉	Y-DIS.(mm)		o,	-3.18212	u,	u,	7	o,		O	-0.25483	-2.10722	-4.16284	-4.16284	.1072	.2548	0.0000.0
Case.	o o	-0.07019	-0.07019	-0.05799	-0.03031	0.00200	0.03563	0.04575	0.04575	0.0000	0.0000	-0.00244	0079	-0.01522	0233	0231	ಟ	•		X-DIS.(mm)	0.14254	0.14254	0.11580	0.06153	-0.01402	-0,06829	-0.09502	-0.09502	0.00000	0,00000	0.00535	0.01620	0.03131	0.04216	0.04751	0.04751
, F.O.		~	ď		٠.	٠,	(1)	"	۲,	67	٠,	4		0.20966	4					ROTA. (mmRad)	0.18644	0.18644	0.11215	0.04046	-0.04048	-0.11215	-0.18644	-0.18644	0.15636	0.15636	0.11195	0.03913	-0.03913	-0.11195	-0.15636	-0.15636
2 (, , , , , , , , , , , , , , , , , ,		-0.13521			٠	:	Ċ	٠											ນາ	Y-DIS.(mm)	-		0.67800		٠.	٦.		•	-		~.	٠:	•	~.		0.00000
Case.	(mm) . C 101-V	0.09489	0.09489	0.07849	0.04138	-0.00975	-0.04686	-0.06328	-0.06326	0.0000	0.0000	0.00328	0.01070	0.02093	0.02835	0.03163	0.03163		Case.	X-DIS.(mm)	-1.59636	-1.46736	-0.87808	-0.29073	0.29611	0.88346	1.47274	1.60174	0.0000	0.0000	0.00082	0.00203	0.00335	0.00456	0.00538	0.00538
	ACIA: Immedia	-0.16470	-0.16591	0.10350	0.02128	-0.02128	-0.10350	0.16591	0.16470	0 17211	0.17745	0.07091	0.02814	-0.02814	-0.07091	-0.17745	-0.17211			ROTA. (mmRad)	-0.28635	-0.29636	-0.11219	0.01350	0.02720	-0.07434	-0.23299	-0.22298	-0.22417	-0.22118	-0.04700	0.10523	0.11846	-0.00927	-0.16855	-0.17158
7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7		171	590	338	0.2188	188	3336	5590	171	0000	0.1491	.6041	.8157	.8157	.6041	1491	000.		7	Y-DIS.(mm)	0.07727	.17	-0.67534	4		44	. 12	.06	8	139	.68	.45	~ .	. 46	₹.	00.
Case.		.0101	-0.01017		-	0.00150	0.00655	0.00678	0.00678	0.0000	0.0000	-0.00005	-0.00105	-0.00233	-0.00334		-0.00339		Case.	X-DIS.(mm)	8	83	2	800	.50	4.5	43	4.	00.	00.	03	90.	80.	60.	е С.	60.
2		1.	63	ю	4	, ()	9		8	.0	10.	11.	12.	13.	14.	13.	16.			No.	.		ю 6	4	ທ	6.	7.	∞	о	10.	11.	13.	13,	14.	15.	16.

ROTA.(mmRad)	00000000000000000000000000000000000000	ROTA, (mmRad)	-0.00000000000000000000000000000000000	ROTA.(mmRad)	00000000000000000000000000000000000000
9 Y-DIS.(mm)	100.0000000000000000000000000000000000	12 Y-DIS.(mm)	00111111000000000000000000000000000000	15 Y-DIS.(mm)	0.000000000000000000000000000000000000
Case. X-Dis.(mm)	-2.16588 -0.16588 -0.36539 -0.36539 -0.36539 -0.00000 -0.000000 -0.00000 -0.00000 -0.00000 -0.00000 -0.00000 -0.00000 -0.000000 -0.00000 -0.00000 -0.00000 -0.00000 -0.00000 -0.00000 -0.000000 -0.0000 -0.000000 -0.00000 -0.00000 -0.00000 -0.00000 -0.00000 -0.00000 -0.000000 -0.00000 -0.00000 -0.00000 -0.00000 -0.00000 -0.00000 -0.000000 -0.00000 -0.00000 -0.00000 -0.00000 -0.00000 -0.00000 -0.000000 -0.00000 -0.00000 -0.00000 -0.00000 -0.00000 -0.00000 -0.000000 -0.00000 -0.00000 -0.00000 -0.00000 -0.00000 -0.00000 -0.000000 -0.00000 -0.00000 -0.00000 -0.00000 -0.00000 -0.00000 -0.000000 -0.00000 -0.00000 -0.00000 -0.00000 -0.00000 -0.00000 -0.0000000 -0.0000000 -0.000000 -0.000000 -0.0000000 -0.00000000	Case. X-DIS.(mm)	00000000000000000000000000000000000000	Case. X-DIS.(mm)	11.647672 11.547672 10.94601 10.32593 0.32593 11.52564 11.52564 11.65426 0.00000 0.000000 10.00100 10.0120 10.0120
ROTA (mmRad)	0.059978 0.071795 0.07166 0.05166 0.05550 0.05550 0.05550 0.05550 0.05550 0.05560 0.05660 0.0560 0.0560 0.0560 0.0560	ROTA.(mmRad)	0.01	ROTA.(mmRad)	0.2569 0.23763 0.23763 0.23763 0.23763 0.0023763 0.002328 0.011960 0.14240 0.11960 0.01049
8 Y-DIS.(mm)	-0.03417 -0.154106 -0.54306 -0.54306 -0.54306 -0.00000 -0.0000000000000000000000000	11 Y-D(S.(mm)	0.111111 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	14 Y-DIS.(mm)	0 0 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
Case. X-DIS.(mm)	6. 34 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Case. X-DIS.(mm)	00000000000000000000000000000000000000	Case. X-DIS.(mm)	-1.51164 -1.388264 -0.269524 -0.287364 -0.287364 -0.00000 -0.000000 -0.000000 -0.000000 -0.000000 -0.000000 -0.000000 -0.00000000
ROTA.(mmRad)	0.04171 0.04006 0.04006 0.058541 0.022218 0.19469 0.19469 0.722284 0.732284 0.73284 0.	ROTA, (mmRad)	0.000000000000000000000000000000000000	ROTA.(mmRad)	-0.046527 -0.00869 -0.00869 -0.00869 -0.00891 -0.00809 -0.00828 -0.00839 -0
7 Y-DIS.(mm)	0.00 10.	10 Y-DIS.(mm)	100000 110000 110000 110000 1000000	13 Y-D[\$;(mm)	- 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
Case. X-DIS.(mm)	1 1 0 0 1 1 2 9 8 8 4 8 9 8 9 8 9 8 9 8 9 8 9 8 9 9 9 9	Case. X-DIS.(mm)		Case. X-DIS. (mm)	3.88125 3.88474 3.69766 3.58441 3.46658 3.44665 0.0000 0.0000 0.003898 0.0038980 0.003899
. NO.	- 00 0 4 00 0 0 0 0 0 0 0 0 0 0 0 0 0 0	o N		No.	- 4667 8600 H 26 4 8 8

N (t)	0.000	61.676 61.676 61.676 61.676	139,975 139,975 139,975 139,975	183.052 183.052 183.052 183.052	150,295 150,295 150,295 150,295 150,295		000000000000000000000000000000000000000	-61.676 -61.676 -61.676 -61.676	-139.975 -139.975 -139.975 -139.975
Load S (t)	0.000	113.213 -26.787 -26.787 -166.787	140.697 0.697 0.697 -139.303	188. 48.396 48.396 -91.604	272. 132. 133. 133. 17. 867.	00000	0.000	-116.087 -106.758 -83.990 -47.783	-305.620 -242.534 -166.009 -76.045 27.358
Case 3 Live M (tm)	0000.0	166.394 140.042 113.550 -51.402	1135,4728 1122,4128 125,4138	135.067 187.064 139.200 129.796	1318,140 156,641 172,222	0.00 4.00 4.00 6.00	0.000	201.671 90.367 -5.066 -71.335 -95.150	144.393 -127.777 -330.910 -451.713 -476.896
N (t)	0.000	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	-187.600 -187.600 -187.600 -187.600		-187.600 -187.600 -187.600 -187.600	დ თ თ თ თ თ	0000.0	82.896 82.896 82.896 82.896	187.600 187.600 187.600 187.600
Load S (t)	0.000	211 844 6000 6000 60000	318.738 178.738 178.738 38.738	929.100 309.700 309.700 -309.700	111111111111111111111111111111111111111	44444	0.000	-164.814 -27.816 109.181 246.178 383.175	68.379 205.376 342.373 479.371 616.368
Case 2 Live M (tm)	0.000	-244.125 -101.627 40.730 44.628	-259.386 -82.475 94.297 132.608 170.920	252.268 558.3668 252.268 252.268 152.368	170.920 132.608 94.297 - 182.475	1000	0.000	-202.028 -297.284 -257.049 -81.324 -259.891	-90.420 44.951 315.814 722.166 1264.009
N (t)	0.000	1.160 1.160 1.160	25. 25. 25. 25. 26. 26. 26. 26. 26. 26. 26. 26. 26. 26	332.33 322.33 332.34 3341 441 441 441	25.495 25.495 25.495 26.495	99999	000000	111111111111111111111111111111111111111	1 1 1 1 1 20 5 1 20 5 1 20 5 20 5 20 5 5 5 5 5 5 5 5 5 5 5 5 5
Load S (t)	0.000	1027,004 273,422 254,749 1498,833	1137.893 384.311 365.638 -387.944 -406.616	1139.710 386.127 367.455 -386.127 -1139.710	406.616 387.944 389.272 -384.311	000440	+0.001 361.346	- 356.519 - 356.519 58.579 - 474.128 889.677	-856.345 -440.796 -25.247 390.302 805.852
se l Dead M (tm)	0.000		-419.818 -29.767 341.083 -33.360 -426.270	-408.706 -14.887 357.789 -114.857	-426.270 -33.360 341.083 -29.767	4 H 4 O 5 C C C C C C C C C C C C C C C C C C	0.000	10507. 1050.868 1058.4001 1034.9077	\$07.796 -133.640 -364.098 -183.578 407.920
Ca L(m)	0.000.0	0.000 0.989 1.978 2.967 3.956	0.000 0.989 2.978 3.967	0,000 0,989 2,989 3,984	0.000 0.989 1.978 3.967	00000	0.0000000000000000000000000000000000000	000 000 000 000 000 000 000 000 000 00	0.000 0.000 1.000 3.000 3.000
N	11	m → 0 m o	0 4 1 * * * 1 1 ↔ 01000	4 N (***) N~6104	0 ~ 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	1 * * * 1	8-7 8-7 10-10	1 * * * !	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1

N (E)	1183.052 1183.052 1183.052 1183.052	1150.295 1150.295 1150.295 1150.295	dentity of	1111 0000 1113.2213 1113.2213 1123.2213 113.2213	1307.4884 1307.4884 1307.4884 1307.4884	1 327,699 1 327,699 1 327,699 1 327,699	1 963,737 1 963,737 1 963,737 1 963,737	1061.068 -1061.068 -1061.068 -1061.068 -1061.068	-804,999 -804,999 -804,999 -804,999
Load S (t)	-300.341 -183.498 -53.216 90.505 247.666	-116.071 54.529 238.568 435.047 645.964	288	,	78.2599 78.2599 78.2599	43.077 43.077 43.077 43.077	1 1 32 2 7 35 7 1 1 32 2 7 7 35 7 1 1 3 2 2 7 7 35 7 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	1999.158 1991.158 1991.158 1991.158	181 181 181 181 181 183 183 183 183 183
Case 3 Live M (tm)	-343,493 -583,858 -702,021 -684,689	-617.365 -648.905 -505.076 -172.587 361.855	130		-230.254 -112.805 -122.094 239.543	-125,057 -60,442 4,173 68,789 133,404	97.747 48.612 -0.523 -49.659	313. 165.091 16.355 -132.382 -281.118	104.913 28.207 -48.499 -125.205
N (t)	225 225 225 225 225 235 235 235 235 235	187.600 187.600 187.600 187.600	്കാര്യ്യ് വ	1.28833 1.28833 1.28833 1.39844 1.39843 1.39843 1.39843 1.39843 1.39843 1.39843 1.39843 1.39844 1.3984	-314.796 -314.796 -314.796 -314.796	1 8 8 9 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	- 8900.362 - 8900.362 - 890.362 - 890.362	-314.796 -314.796 -314.796 -314.796	1 1 2 2 8 3 3 5 2 4 1 1 1 2 2 8 3 3 5 2 4 1 1 1 2 8 3 3 5 5 4 1 1 1 4 4 4 4 4 4 4 4 4 4 4 4 4 4
Load S (t)	-273.995 -136.997 0.000 136.997 273.995	-616.368 -479.371 -342.373 -205.376 -68.379	83.17 46.17 09.18 27.81	1 82 896 1 82 896 1 82 896 1 82 896 1 82 896 1 82 896	-104.704 -104.704 -104.704 -104.704	0.071111 0.00000 0.00000 0.00000	70.935 70.935 70.935 70.935	104.704 104.704 104.704 104.704 104.704	88.22.88.89.06.22.88.89.06.89.06.89.06.89.06.89.06.08.09.06.09.06.09.06.09.06.09.06.09.06.09.06.09.06.09.06.09
Case 2 Live M (tm)	1063.960 860.725 792.980 860.725 1063.960	1264.009 722.166 315.814 44.951	8 1.77 8 1.07 1.08 1.08 1.08 1.08	11.25 11.25 11.25 11.35 11.35 11.25	307.912 150.856 -163.255 -320.311	225. 119.161 12.758 193.645	-119.161 -119.161 -12.758 -93.645 200.048	-307.912 -150.856 -200 163.255 320.311	110-110-110-110-110-110-110-110-110-110
N (E)	1111 5000	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		-1043.241 -1065.897 -1088.553 -1111.209	-1655.398 -1678.054 -1700.710 -1723.366 -1746.022	-11546.326 -1591.638 -1591.638 -1614.294	-1546.026 -1568.982 -1591.638 -1614.294 -1636.950	-1655.398 -1678.054 -1700.710 -1723.365 -1746.022	-1043.241 -1065.897 -1088.553 -1111.209
Load S (t)	-831,098 -415,549 0,000 415,549 831,098	-805.852 -390.302 25.247 440.796 856,345	89,67 74,12 58,57 72,51	1,160 1,160 1,160 1,160 1,160 1,160	4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	6 6 8 4 6 6 8 8 8 6 6 8 8 8 8 8 8 8 8 8	1 1 1 1 1 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	-1,160 -1,160 -1,160 -1,160
ase 1 Dead M (tm)	-129.433 -187.034 -392.523 -187.034 429.433	1183.578 1384.098 1133.640	4 2 4 5 6 7 6 7 6 7 6 7 6 7 6 7 6 7 6 7 6 7 6	4 4 4 4 6 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	-77.640 -41.137 -4.635 31.868 68.371	119,564 0,295 0,2995 111,244 211,544	19.564 9.295 -0.274 -11.244	77, 640 41, 137 4, 638 -31, 868 -68, 371	1 1 4 4 5 5 4 5 5 4 5 5 6 5 6 8 8 6 5 6 5 6 8 8 6 5 6 5 6 6 8 6 6 6 6
ນ (m) ໄ	0.000 0.989 1.978 2.967	0.000 0.989 1.978 2.967	0 8 6 9 8 9	000000000000000000000000000000000000000	0.000	0.000 1.500 3.000 6.500	0.000 3.000 6.500	0.000 3.000 6.000 6.000	0,000 1,500 3,000 4,500 6,000
N O	1 * * * 1 5 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	13 14 14 14 14 14 15 16 16 16 16 16 16 16 16 16 16 16 16 16	1 * * * 1 6	D 10 O I O I O I O I O I O I O I O I O I O	1 * * * 1	4 6 1 * * * 1 0 - 0 0 0 4		1 * * * 1) + * + ! + * + !

N (E)	0.000	5.178 5.178 5.178 5.178	4.357 4.357 4.357 4.357	1.953 1.953 1.953 1.953	4.357 4.357 4.357 4.357	, 85, 178 85, 178 178 178 178 178 178	8 88	5.178 5.178 5.178 5.178	4.357 4.357 4.357 357 357
			44444		00000	66666666666666666666666666666666666666		8 8 8 8 8 8 8 8 8 8 8 8	00000
(+) s	0.000	1885.758 614.825 589.057 -681.887	2096.209 825.266 799.498 -471.446	3105.814 1043.861 1018.093 -1043.861 -3105.814	497.213 471.446 445.678 -2096.209	707.654 681.887 656.119 -614.825 -1885.768	9 9 %	-1338.019 -538.516 250.988 1050.491	-1068.931 -269.428 530.076 1329.579 2129.082
Case 6 M (tm)	0000	-475.022 147.027 742.347 80.703 -606.425	-1007.336 -177.161 626.285 172.767 -306.235	-651.417 -651.417 1415.376 -651.417	-306.235 172.787 626.285 -177.161 -1007.336	606. 806. 742. 742. 147. 19.66. 19.66. 18.66. 19.66.	0 0 0	-46,740 -974.686 -1111.924 -458.453	551.565 -110.253 18.637 938.237 2648.544
N (E)	0.000	20.807 20.807 20.807 20.807	000000000000000000000000000000000000000		000000000000000000000000000000000000000	120.8807 120.8807 120.8807 120.8807 120.8807	8 88	20.807 20.807 20.807 20.807	30.600
erature S (t)	0.000	10.00 11.00 10.00 11.00 11.00 11.00	000000	000000		13.571 13.571 13.571 13.571 13.571	3 88		
Case 5 Temp	0.000	- 58.144 - 31.722 - 17.878 - 17.878	-33.804 -32.005 -30.206 -28.406	1 1 1 1 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	-26.607 -28.406 -30.206 -32.005	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	80.	-66.701 -80.123 -93.545 -106.967	-149,796 -151.695 -153.395 -155.194
N (t)	-848.992 -848.992	1744.759 1744.759 1744.759 1744.759	000000000000000000000000000000000000000	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	1 2 2 40 . 0 4 9	1 1 1 1 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		790.089 790.089 790.089 790.089	634.294 634.294 634.294 634.294 634.294
ic State S (t)	0.000	00000	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	1112. 1112. 112. 14.4. 112. 12. 14. 14. 14. 14. 14. 14. 14. 14. 14. 14	000,000 000,000 000,000 000,000 000,000 000,000	2 1 1 1 1 1 4 8 8 8 8 8 8 8 8 8 8 8 8 8 8	. 33	76.633 6.764 -55.750 -110.909	-120.660 -161.110 -194.206 -219.947 -238.334
Case 4 Seism M (tm)	0.000-	272.271 174.933 77.596 -19.741	322,903 187,903 52,959 - 82,013	317.405 161.587 5.770 -150.048	225.794 91.506 -42.782 -177.071	118 007 25 582 - 665 842 - 159 267 - 251 692 38 803	.38	389.888 405.691 322.692 188.7693	656,328 516,386 340,076 134,671
C(m) 7	0.000	00000000000000000000000000000000000000	0.000 0.989 1.978 3.967	0.000 0.989 3.9967 3.9967	0.000 0.989 1.978 2.967	00000000000000000000000000000000000000	00.	0.000 0.989 1.978 2.967 3.956	0.000 0.989 1.978 3.957 3.956
S.N.	2-1	n ← o n c o	u 4 * * * 1	4 m 1 * * * 1 m = 440 4	n α γι 1 * * * 1 Φ ~ νι οι	1 * * * 1 1 C		100000000000000000000000000000000000000	12 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1

N CE	381.953 381.953 381.953 381.953	274.357 274.357 274.357 274.357 274.357	135.178 135.178 135.178 135.178	0000.0	-1908.175 -1939.440 -1970.706 -2001.971	-2803.864 -2865.129 -2866.394 -2897.660 -2928.925	-3603.028 -3634.293 -3665.558 -3696.823 -3728.089	-3603.028 -3634.293 -3655.558 -3696.823 -3728.089	12803; 864 128835; 123 123866; 394 12897, 660	-1908.175 -1939.440 -1970.706 -2001.971
\$ (1)	-1599.007 -799.503 0.000 799.503 1599.007	-2129.082 -1329.579 -530.076 269.428 1068.931	559,99 60,49 60,98 38,51	-695.217 0.003	-135.178 -135.178 -135.178 -135.178	-139.179 -139.179 -139.179 -139.179	-107.596 -107.596 -107.596 -107.596	107.596 107.596 107.596 107.596	139,179 139,179 139,179 139,179	135,178 135,178 135,178 135,178
Case 6 M (tm)	2348.153 1162.089 766.735 1162.089 2348.153	2648.544 938.237 18.637 -110.253 551.565	85.72 58.45 11.92 74.68	298.942	465.387 262.620 59.853 -142.915	400.911 192,143 -16.625 -225.393 -434,162	345.1183.77889 122.3995 108.9988	1345.182 1183.789 122.395 138.998	-192.143 -192.143 16.625 225.393 434.162	1 2 4 6 6 5 5 6 5 6 5 6 5 6 5 6 5 6 5 6 5 6
N (E)		000000000000000000000000000000000000000	00000	0.000	-13.571 -13.571 -13.571 -13.571	11.755211.755211.755211.7552		1.819 1.819 1.819 1.819	11.752 11.752 11.752 11.752 11.752	111111111111111111111111111111111111111
erature S (t)	00000	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	7777	00000	-20.807 -20.807 -20.807 -20.807 -20.807	19,793	ភាភាភភភភភភភភភភភភភភភភភភភភភភភភភភភភភភភភភ	១០០០០០ ១១១១១១ ១១១១១១ ១១១១១១	9.793 9.793 9.793 9.793	20.807 20.807 20.807 20.807 20.807
Case 5 Temp M (tm)	1164.860 1164.860 1164.860 1164.860	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	800.440	0.000	58.144 26.933 -4.278 -35.489 -66.701	29.349 14.660 -0.029 -14.718	7.488 3.649 -0.189 -7.028	-7 488 -3.649 0.189 7.866	-29.349 -114.660 0.029 14.718	-58.144 -26.933 -4.278 35.489 66,701
N (3)	448.771 445.771 445.771 445.771		00000	000.0	88888888888888888888888888888888888888	38.0053 38.0053 38.0053 38.0053	21.077 21.077 21.077 21.077	121.769 121.769 121.769 121.769	1 1 1 4 4 4 4 4 4 4	1 1 1 1 1 1 1 4 4 4 4 4 4 4 4 4 4 4 4 4
mic State S (t)	-217.256 -228.288 -231.966 -228.288	-239.025 -220.614 -194.798 -161.579	2000 2000 2000 2000 2000 2000	4.12	104.233 106.500 108.766 111.033	146.729 148.995 151.262 153.528 155.795	179,456 181,723 183,989 186,256 188,525	178.524 180.791 183.057 185.324 187.590	143.188 145.452 147.718 149.986	96.864 99.131 101.397 103.664 105.930
Case 4 Seis M (tm)	476.991 256.064 27.862 -200.340 -421.258	1. 1. 2. 8. 2. 2. 2. 2. 2. 2. 2. 2. 2. 2. 2. 2. 2.	484400 444400 4440000000000000000000000	0.0	-311.074 -153.024 8.426 173.276 341.525	-439.982 -218.189 7.004 235.597	-534.390 1263.506 10.778 288.462 569.546	-531.660 -262.174 -10.712 286.997 566.683	1 429 . 367 - 212 . 889 - 6 . 988 - 56 . 942 - 942	-290,496 -143,499 6,896 160,692 317,887
L(m)	0.000 0.989 1.978 3.967	0.000 0.939 1.978 3.967	0000	86.	0.000 1.800 0.000 0.000	0.000 3.000 4.500 6.000	0.000 3.000 6.000 6.000	0.000 1.500 2.000 6.000	0 4 8 4 8 0 0 0 0 0 0 0 0 0 0	0.000 1.5000 3.0000 4.5000
No	13- 13 13- 12	1 * * * 1 T	1 * * * 1	~	21 10 10 10 10 10 10 10 10 10 10 10 10 10	3327	1 * * * 1	# * * * 1 0 = 0 = 0 = 0	0 4 4 4 1 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	1 * * * 1 0 ~ 0 ~ 0

N (t)	0.000	114422 114422 114422 114422 114422 114422 114422 114422 1144	-267.463 -267.463 -267.463 -267.463 -267.463		1 267.463 1 267.463 1 267.463 1 267.463	-11422 -11422 -11422 -1422 -1531 -11422 -1531 -11422 -1531	0000.0	0 ខេត្តសំខេត្ត	267.463 267.463 267.463 267.463 267.463
S (t)	0.000	1828, 512 595, 368 859, 660 -669, 543 -669, 311	2012.752 779.608 753.841 -479.303	2854.957 960.242 934.474 -960.242	505.071 479.303 453.535 -779.608	689.311 663.543 637.776 637.776	22.407	20.92 20.92 4.61 1.13	-1089.995 -327.481 435.033 1197.547 1960.061
Case 9 M (tm)	0.000	110.254 686.551 435.081 645.091 686.891	- 985. - 200.667 - 557.657 - 96.342	-685.419 278.871 1215.808 278.871 -685.419	- 390.431 96.342 557.630 - 200.660	-625.898 43.088 686.591 110.514 -492.254	900	A ROOM 44	361.770 -339.171 -285.987 521.324 2082.762
N (E)	-1400.837	-1227.251 -1227.251 -1227.251 -1227.251	000000 000000 000000 11110 00000	. 646.015 - 646.015 - 646.015 - 646.015	8868 860 860 860 860 860 860 860 860 860	1115882225	0.000	302.04 302.04 302.04 302.04	1011.401 1011.401 1011.401 1011.401
S (£)	0.000	1329.321 289.377 263.609 -776.334	1419 379.559 353.848 -686.096	1387.289 347.289 321.345 -718.366	411.538 385.770 360.002 -679.941 -1719.885	634.408 608.640 582.873 -457.071	22.407 0.000	39 39 31 31 31 31 31 31 31 31 31 31 31 31 31	-1380,846 -874,130 -355,280 175,705
Case 8 M (tm)	0.000	377,031 603,352 803,175 -25,507 -879,673	146,559 269,008 558,077 1181,358	246.116 503.269 11665.083	1215.691 100.104 100.104 1033.245	-491.781 49.278 564.852 51.921 -487.507	40 0	0 0 4 4 6	1783.700 667.614 58.670 -31.130 410.215
N Ct)	0.000	103.366 103.366 103.366	2666.142 2666.1442 2666.1442 442442442	346.667 346.667 346.667 346.667	2883.170 2883.170 2883.170 2883.170	88 88 88 88 88 88 88 88 88 88 88 88 88	000000	9 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6	1266.142 1266.142 1266.142 1266.142
S	0.000	2604,067 333,124 307,356 -963,588	1802.442 531.499 505.731 795.213	1883,653 612,709 586,941 -684,002	1010.150 753.382 727.614 -5543.330	2451.939 1404.161 1378.393 -683.560	20 00	2	-1686.029 -1008.480 -308.756 413.143
ase 7 M (tm)	0.000	202.335 545.781 862.499 -77.747	1556.416 496.134 1247.920 1017.487	-784.115 -164.158 -129.069 -924.901	-1113.183 -355.115 -377.237 -147.374	-1323.443 79.037 1455.010 791.711		00.104	939.007 -1048.255 -2998.664
C(m)	0.000	0.000 0.000 0.000 0.000 0.000 0.000 0.000	0.000 0.989 1.978 3.967	0.000 0.989 1.978 2.967	0.000 0.989 1.978 3.957 3.956	0.000 0.989 1.978 2.967 3.956	0.000	\$ 0.00 0.00 0.00 0.00 0.00	0.000 0.989 1.978 3.967
S.	2 2	0 + 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	1 * * * †	n → 01 02 ± 1	n	0 7 * * * 1 to 0 to	8 - 1		1 * * * 1

(t) N	359 359 359 359 359 359 359 359 359 359	267.463 267.463 267.463 267.463	142.551 142.551 142.551 142.551	0.000	-1882.184 -1913.449 -1944.714	-2702.063 -2733.328 -2764.594 -2795.859	13360 13360 134631 134631 1366 1366 1366 1366 1366	-3360.028 -3391.293 -3422.559 -3453.824 -3485.089	-2702.063 -2733.328 -2764.594 -2755.859 -2827.124	-1850.918 -1882.184 -1913.449 -1944.714
s (t)	1525.028 -762.514 762.514 1525.028	-11960.061 -1197.547 -435.033 327.481 1089.995	-1737.130 -974.616 -212.102. 550.413	-663.053	-142.551 -142.551 -142.551 -142.551	111111111111111111111111111111111111111	11.10000	92.103 92.103 92.103 92.103 92.103	124.912 124.912 124.912 124.912 124.912	142,551 142,551 142,551 142,551 142,551
Case 9 M (tm)	1825.134 693.945 316.881 693.945 1826.134	2082.762 521.324 -285.987 -339.171 361.770	751.499 -589.459 -1176.290 -1008.995	285.112	482.620 268.793 54.967 -158.859	359,744 172,376 -14,993 -202,361 -389,729	294.988 1156.834 119.6834 257.6834	156.834 -156.834 -18.680 119.474 257.628	-359.744 -172.376 14.993 202.361 389.729	11 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
N (E)	690.892 690.892 690.892 690.892 690.892	390.816 390.816 390.816 390.816	173.184 173.184 173.184 173.184	0000	-1351.727 -1382.993 -1414.258 -1445.523 -1476.788	12221.662 12252.927 12284.192 12315.458	1 1 1 2 2 0 9 9 1 1 2 2 1 2 2 1 3 2 2 2 2 2 2 3 3 3 3 3	-2169.848 -2201.113 -2232.379 -2263.644 -2294.909	123354,293 123385,558 122416,823 12448,089	-1519,421 -1550,687 -1581,952 -1613,217
S (†)	1505.388 1950.1338 195.743 782.7443	1506.466 1902.630 1286.577 341.693	-1497.176 -844.555 -179.880 -496.848	149,916	173.586 177.326 181.065 184.805 188.545	275,685 279,425 283,165 286,904	305.551 509.290 313.030 316.770 320.510	285.117 388.857 292.597 296.336	202.673 206.413 210.163 213.892 217.632	158.225 161.965 165.704 173.184
Case 8 M (tm)	1379.654 164.398 -495.709 -588.667	802.865 19389.440 1958.509 1298.61	340.988 +797.991 -11305.567 -1149.820	133,500	-450.691 -187.508 81.286 355.688 635.701	-833,114 -416,781 5,161 432,713 865,875	1908.743 1447.6123 19.129 491.479 969.438	-850.241 -419.760 16.330 458.030 905.339	-601.311 -294.497 17.926 335.959 659.602	-541.898 -301.755 -56.004 195.358
N (F)	1.0346.067	-283.170 -283.170 -283.170 -283.170	85.977 85.977 85.977 777 85.977 779	000.0	-1626.474 -1657.740 -1689.005 -1720.270	-2791.798 -28823.063 -28854.328 -28885.893 -2916.8859	12674.633 12705.899 12737.164 12768.429	-2734.095 -2765.361 -2796.626 -2827.891 -2859.157	-4035.212 -4066.477 -4097.743 -4129.008	-2767.921 -2799.186 -2830.451 -2861.717
(t) S	-1642 478 -876 230 -876 230 -187 306 1555 565	-1303.592 -448.644 -428.478 1327.775 2249.247	-1911.026 -967.379 -1.557 986.441 1996.614	-896.368 0.068	103.366 103.366 103.366 103.366	162,777 162,777 162,777 162,777	88 88 88 88 88 88 88 88 88 88 88 88 88	1 1 1 1 1 6 0 3 4 4 0 0 0 7 4 0 0 0 7 4 0 0 0 7 4 0 0 0 7 4 0 0 0 7 4 0 0 0 7 4 0 0 0 7 4 0 0 0 7 4 0 0 0 7 4 0 0 0 7 4 0 0 0 7 7 0 0 0 7 7 0 0 0 7 7 0 0 0 7 7 0 0 0 7 7 0 0 0 7 7 0 0 0 7 7 0 0 0 7 0 0 0 7 0 0 0 7 0 0 0 7 0 0 0 7 0 0 0 7 0 0 0 0 7 0	-197,193 -197,193 -197,193 -197,193	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
Case 7 (tm)	25.855 -1221.473 -1700.017 -1387.844 +263.024	-1455.723 -1335.031 -1469.191 1297.819		386.611	+211.969 -56.921 98.128 253.176 408.224	1 2 4 8 7	-233.346 -112.356 -120.017 249.804	188.282 93.037 -12.208 -197.453	6224 229.170 233.170 1.253.170 1.253.170 1.253.170	110.526 -18.440 -1.17.405 -276.371
(m)	0.000 0.989 1.978 2.967 3.956	0.000 0.989 1.978 2.967 3.956	0.000 0.989 1.978 2.967 3.956	0.000	00000	0.000 0.000 0.000 0.000	0000.0000.0000.0000.0000.0000.0000.0000.0000	0.000 1.5000 3.000 6.000	0.00. 1.500 3.000 6.000	0.1.0.000000000000000000000000000000000
N _o	1 * * # 1	1 * * * 1	1	15- 16	3 + * * 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	1 * * * 1	1 * * * 1	5 + + + 13	0 4 1 * * * 1 4 12 to 0	7 * * * 1

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N (E)	0.000	622.836 622.836 622.836 62.836 62.836	165.470 165.470 165.470 165.470	215.393 215.393 215.393 215.393 215.393	175.790 175.790 175.790 175.790	52.297 52.297 52.297 52.297 52.297	000.00	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	-165.470 -165.470 -165.470 -165.470
S (t)	0.000	1140.217 246.635 227.963 -665.620	1278.590 385.008 366.335 -527.247	1328.106 434.233 415.851 -477.731	678.749 520.077 501.404 -392.178 -1145.760	1570.706 932.634 913.962 -459.021 -1832.003	16.237 0.000 -0.059 358.472	1 888 606 1 463 7 411 425 411 891 1 551 411	-1161.965 -683.330 -191.256 314.257 833.210
Case 12 M (tm)	0.000	114.064 368.094 602.783 -46.282 -713.813	115.038 715.038 75.038 155.713 1686.394	- 541.773 - 101.921 - 318.589 - 144.654 - 627.098	-744.410 -220.580 -284.442 -194.188	-883,489 48,739 961,881 517,142 52,583	-6.982 0.000 0.000 153.389	409.357 -2660.480 -503.466 -306.312 344.275	652.189 -261.417 -695.008 -635.291 -68.976
N Ct	0.000	- 81.736 - 81.736 - 81.736 - 81.736	11622.105 11622.105 11622.105 11622.105	1226.194 1226.194 1226.194 1226.194	1162.105 1162.105 1162.105 1166.105	1 81.736 1 81.736 1 81.736 1 81.736	00 00 00 00 00 00 00 00 00 00 00 00 00	81.736 81.736 81.736 81.736	162.105 162.105 162.105 162.105
s (t)	0.000	1310,948 417,363 398,690 -494,892 -513,564	1456.631 563.048 544.376 -349.206	2068.810 695.827 677.155 -695.827 -2068.810	367,879 349,879 330,534 1563,048	8487 888 882 882 884	16.237 0.000 -0.002 480.473	-937.333 -384.786 167.760 720.306	-787,966 -235,420 317,127 869,673 1422,219
Case 11 M (tm)	0.000	1296, 455 1206, 455 520, 963 449, 749 933	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	237.411 916.351 237.411 237.411 -461.350	1255.350 499.249 435.380 112.241 679.204	ಬ4ದಲಾಬ	-6,982 0,000 0,000 206,603	5.658 -648.130 -755.450 -316.301 669.316	417.376 -88.688 -48.284 538.588 1671.929
(2) Z	0.000	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	1884 1884 1884 1884 1884 1884 1890 1890 1890 1890	6449 6449 6449 6449 6449 6449 6449 6449	11998.888	24442224222422422424242424242424242424	000000000000000000000000000000000000000	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	-184.591 -184.591 -184.591 -184.591
(1) S	0.000	1592.9007 359.763 359.763 1899.148	1767.056 533.912 508.144 1724.999	1832.786 599.642 573.874 1659.269	934.072 715.104 689.337 543.807		22.407 0.000 -0.082 494.692	-1245, 683 -659, 352 -54, 474 -568, 949 1210, 919	-1606,113 -945,597 -266,535 431,074
ase 10 M (tm)	0,000	74,262 444,018 787,081 189,434 1991,433	166.509 166.509 1255.509	1796, 403 1189, 407 390, 897 1248, 378 1914, 151	1066.334 1345.160 349.336 1757.747	-1225.586 41.695 1282.636 649.704	-9.635 0.000 0.000 211.677	4469.531 - 474.038 - 828.553 - 575.674 302.943	685.812 -577.537 -1178.465 -1098.629
ິບ (ພ)ໆ	0.000	00.00 00.00 00.00	0.000 0.989 1.978 2.967	0.000 0.989 1.978 2.967 3.956	0.000 0.989 1.978 2.967 3.956	0.000 0.989 1.978 2.967 3.956	0.000 0.860 0.000 0.860	0.000 0.989 1.978 3.987	0.000 0.989 1.978 3.967
No.		0 → 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 4 1 * * * 1 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	1 * * * 1 N ~ 0 00 4	n + * * 1 n − 0 m m	0	8 - 10 m	1 * * * 1 1 1 2 2 2 2 2 2 2 2 2 2 2 2 2	1 * * * 1

N (E)	- 215 - 215 - 215 - 215 - 25 - 25 - 25 - 25 - 25 - 25 - 25 - 2	-175.790 -175.790 -175.790 -175.790	जंबन्धन ०	0.000 -1156.454 -1179.110 -1224.422 -1247.078	-1962.882 -1985.538 -2008.194 -2030.850 -2053.506	-1874.025 -1896.681 -1919.337 -1941.993	-1910,063 -1932,378 -1955,378 -1978,031 -2000,687	-2716.466 -2739.122 -2761.778 -2784.434	1848.240 -1870.896 -1893.552 -1916.208
s (t)	-1131.439 -599.047 -53.216 506.055 1078.754	-921.922 -335.773 263.815 876.843 1503.309	2222	622.836 622.836 622.836 622.836 62.836	102.635 102.635 102.635 102.635 102.635	49.923 49.923 49.923 49.923	09,600 009,600 009,600 009,600	11233.493	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
Case 12 M (tm)	85.941 -770.892 -1094.544 -871.723 -89.138	-209.445 -832.483 -869.174 -306.227 869.651	កាលលុខ ។	0.000 -121.046 -26.7046 -67.461 161.715 255.968	-307.894 -153.942 0.010 153.962 307.914	144.621 -69.737 5.148 80.032 154.917	117.311 57.907 -1.498 -60.903 -120.307	391.468 206.229 1.620.990 1.164.250 1.349.489	59.565 -18.881 -197.327 -175.773
K (t)	226.194 226.194 226.194 226.194 226.194	162.105 162.105 162.105 162.105 162.105	22.22	-1327 -1349 -1372 -1395 -1417 -806	-1970.195 -1992.851 -2015.507 -2038.163 -2060.819	-2456.688 -2459.344 -2482.000 -2504.656 -3527.312	12436.688 12489.344 12482.000 12504.656	-1992.351 -1992.351 -2015.507 -2060.819	-1327.182 -1345.838 -1372.494 -1395.150
\$ (t)	-1105.093 -552.546 0.000 552.546 1105.093	-1422.219 -869.673 -317.127 235.420 787.966	800 F F W 4	0.002 -81.736 -81.736 -81.736 -81.736	1 1 1 1 8 9 9 8 9 8 9 8 9 8 9 8 9 8 9 8	1644.089 1644.089 164.089	64.089 64.089 64.089 64.089	80.369 80.369 80.369 80.369	81.736 81.736 81.736 81.736 81.736
Case 11 M (tm)	1493.394 673.691 400.457 673.691 1493.394	1671,929 538.588 -48.284 -88,688 417.376	6.64400 0	0 6 9 4 6 0	230.272 109.719 -10.834 -131.387 -251.940	206,000 109,866 13,732 -82,402	-206.000 -109.866 -13.732 82.402 178,535	-230.272 -109.719 10.834 131.387 251.940	-289,473 -166,869 -44,264 -78,340 200,945
N (t)	1 2 4 9 . 8 2 5 5 4 4 9 . 8 2 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	111111111111111111111111111111111111111	4444	0.000 -1615.314 -1647.8579 -1709.110	-2691.971 -2723.237 -2754.502 -2785.767	12583.553 12614.818 12646.084 12677.349 13677.49	12664.5550 12664.5550 12727.081 12727.081	-3731.918 -3763.183 -3764.449 -3825.714 -3856.979	2569.978 2601.244 2632.509 -2663,774
s (t)	-1561.386 -826.685 -73.438 698.355 1488.695	-1269.651 -460.766 366.666 1212.644 2077.169	0 0 0 C C	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	127.632 127.632 127.632 127.632	88888888888888888888888888888888888888	20000000000000000000000000000000000000	11111 111111 1111111 11111111111111111	1 1 2 2 4 1 5 2 2 4 1 5
Case 10 M (Lm)	-117.151 -1299.581 -1746.220 -1438.727 -358.759	-1370.754 -1418.815 -639.074 985.910	857.00		-3822 -191.476 -0.028 382.869	-188.869 -91.018 6.833 104.685	151,182 71,796 -78,286	498,257 263,632 29,007 -205,618 -440,243	-0.947 -64.570 -128.193 -191.816
L(m)	0.000 0.989 1.978 3.967	0.000 0.989 1.978 2.967 3.956	0 2 0 0 0	0.860 1.5000 6.50000 6.000000000000000000000000	0.000 1.800 4.5000	0.000 1.500 3.000 4.500	0.000 1.500 3.000 6.500	00000 00000 00000	0.000
No.	13 + * * 1 13 + * * 1 13 + 1 + 1	- 0 - * * * + 4 - 1 - 2 - 2 - 2 - 2 - 2 - 2 - 2 - 2 - 2	1 * * * 1 I	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	1 * * * 1	4 + * * - 21		1 * * * 1 1 * * * 1	1 * * * 1

S C	0.000	42.028 42.028 42.028 42.028 42.028	134.870 134.870 134.870 134.870	182.234 182.234 182.234 182.234 182.234	145.190 145.190 145.190 145.190 145.190	31.490 31.490 31.490 31.490	000000000000000000000000000000000000000	-42.028 -42.028 -42.028 -42.028	-134.870 -134.870 -134.870 -134.870
\$ (1)	0.000	1153.789 240.206 241.534 -652.048 -670.720	1280.409 388.155 368.155 -525.428	1328,106 434,523 415,851 -477,731	676.930 518.257 499.585 -393.997	1557.135 910.390 900.390 -472.592	16.237 0.000 -0.059 358.472	- 902.177 - 477.299 - 38.982 - 412.774 877.969	-1163,784 -1685,149 -193,075 312,438
Case 15 M (tm)	0.000	55.920 323.372 571.483 -64.159	-439.723 -47.043 326.235 -184.120	-575.868 -136.016 -284.494 -178.749	-771.017 -254.236 -126.193	-887.944 30.861 930.581 472.420 -5.561	-6.982 0.000 0.000 153.389	342.656 -340.602 -597.011 -413.279 -223.886	502.393 -413.012 -848.402 -790.485
N (C)	0.00.0	1102.544 1102.544 1102.544 1102.544	1192.705 1192.705 1192.705		192.705 -192.705 -192.705 -192.705	11002.544 11002.544 11002.544 11002.544	000.00	102.544 102.544 102.544 102.544	192.705 192.705 192.705 192.705
S (t)	0.000	1324.517 430.934 412.262 -481.320	1458, 450 5461,868 -347,387	2068.810 695.827 677.155 -695.827	365.059 347.387 328.715 1564.868	499.993 481.320 462.648 -430.934	16.237 0.000 -0.002 480.473	1950.904 1398.358 154.188 706.735	-789.785 -237.239 315.307 867.854 1420.400
Case 14 M (tm)	0.000	-354. 498.7099 498.664 493.861	-713.009 -114.246 405.174 70.842	203.316 882.316 203.316 203.316 495.316	-281.957 70.842 405.174 -144.246 -713.009	-453.388 31.871 498.664 81.703 -354.599	16.982 0.000 0.000 206.603	1 1 6 1 . 0 4 3	267.580 -240.284 -201.679 383.394 1514.936
N (t)	-848,992 -848,992	1742 1743 1743 1743 1743 1743 1743 1743 1743	1 1 1 1 1 1 1 1 1 2 2 2 2 3 3 4 1 2 3 3 3 4 2 3 3 3 4 3 3 3 4 3 3 3 4 3 3 3 4 3 3 3 4 3 3 4 3 3 4 3 3 4 3 3 4 3 3 4 3 3 4 3 3 4 3 3 4 3 3 4 3 3 4 3 3 4 3 3 4 3 3 4 3 3 4 3 3 4 3 3 4 3 3 4 3 4 3 4 3 3 4	00000000000000000000000000000000000000	1 1 1 1 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	- 95.704 - 95.704 - 95.704 - 95.704	0.000 0.000 0.000 903.388	788.929 788.929 788.929 788.929	608.798 608.798 608.798 608.798 608.798
s (t)	0.000	973.704 220.122 201.449 -552.133 -570.805	1046.540 274.285 479.297	1027.279 273.097 255.024 -498.558 -1252.140	315.954 297.282 278.610 -474.973	469.172 450.500 431.828 -321.755 -1075.337	16.237 0.000 90.065 384.679	1695.886 1350.206 2.829 363.219 730.964	-6017,005 -6019,006 -219,453 170,356
Case 13 M. (tm)	0.000	219,941 402,986 566,829 -14,621	-96.915 158.164 394.042 -115.373	-89.301 148.303 363.559 -164.905	2000.475 58.146 298.301 -206.838	-379,451 30,703 422,391 68,785	31.821 0.000 0.000 203.741	597.574 79.675 -92.709 87.696 628.163	1164.124 382.746 -24.022 -48.906 315.366
C(m)	0.000	0.000 0.989 1.978 2.967	0.000 0.989 1.978 3.957	0.000 0.989 1.978 3.967	0.000 0.989 1.978 3.967	3.948 3.954 3.954 3.954	0.000	0.000 0.989 1.948 3.964	0.000 0.989 1.978 3.967
97	1- 2 2- 1	0 - 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 4 1 * * * 1 4 ↔ 01 € €	1 * * * 1 1 * * * 1	0 # # # I	Q 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	7- 8 8- 7 9- 10	100111	11. 12. 12. 12. 13. 13. 14. 17. 17. 17. 17. 17. 17. 17. 17. 17. 17

N (E)	111882 11882 118	-145.190 -145.190 -145.190 -145.190	44444 000000	00.0	-1170.026 -1192.682 -1215.338 -1237.994 -1260.650	-1951,130 -1973,786 -1996,442 -2019,098 -2041,754	-1872.206 -1894.862 -1917.513 -1940.174 -1962.830	11908.243 11930.243 11953.555 11976.211	-2704.714 -2727.370 -2750.026 -2772.682 -2795.338	-1861.811 -1884.467 -1907.123 -1929.779 -1952.435
S (t)	-1131.439 -539.047 -53.216 506.055	1.930 255.954 878.634 1505.129	50.21 3.04 50.30 50.30	8.0.	42.028 42.028 42.028 42.028 42.028	92.842 92.842 92.842 92.842	47,364 47,364 47,364 47,364 47,364	1007.004 1007.004 1007.004 1007.004 1007.004	-113.700 -113.700 -113.700 -113.700	-31,490 -31,490 -31,490 -31,490 -31,490
Case 15 M (tm)	- 78.919 - 935.752 + 1259.404 - 1036.583 - 253.997	-366.438 -987.677 -1022.569 -457.822 719.855	59.77 82.09 50.47 47.25	0.00	-62.901 0.141 63.183 126.225 189.268	-278.545 -139.282 -0.019 139.244 278.507	-137.133 -66.087 -4.959 76.005 147.050	109,2824 1136,834 1126,874 1127	362.119 191.569 21.018 -149.532 -320.082	1.421 -45.814 -53.048 -140.283
N (#)	2000 2000 2000 2000 2000 2000 2000 200	192.705 192.705 192.705 192.705	000000 0	000	-1340.753 -1363.409 -1386.065 -1408.721 -1431.377	-1958.443 -1981.099 -2003.755 -2026.411	-2434.869 -2457.525 -2480.181 -2502.837	12434,869 12487,528 12480,181 12502,837 12525,493	1958,443 1981,099 12003,755 12026,411 12049,067	-1340.753 -1385.469 -1386.065 -1408.721
S C	-1105.093 -582.546 0.000 582.546 1105.093	-1420,400 -867,854 -315,307 237,239 789,785	800.00 800.00 800.00	0 0	-102.544 -102.544 -102.544 -102.544	190,161 190,161 190,161 190,161	166.648 166.648 166.648 166.648	66.06.000 66.06.000 64.00.000 84.00.000	90.161 90.161 90.161 90.161 90.161	102.544 102.544 102.544 102.544 102.544
Case 14 M (tm)	1328.534 508.832 235.597 508.832	1814.936 383.394 -201.679 -240.284	24229 86899 96899 9699	06.60	347.618 193.802 39.986 -113.830	259.621 124.379 -10.863 -146.105	213.488 113.516 13.543 -86.129	1213 1133,516 113,545 186,129 186,129	-259.621 -124.379 10.863 146.105 281.347	-347,618 -193.802 -39.986 113.830
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(2) S	1048.354 1643.837 1831.966 187.261	-1044.876 -610.916 -169.552 279.217	882.98 89.43 89.63 44.97	0.82	105.393 107.660 109.926 112.193	171.064 173.331 175.597 177.864 180.130	186.303 188.569 190.836 193.102 195.369	171.678 173.945 176.211 178.478	118,850 121,116 123,383 125,649 127,916	95.704 97.971 100.237 102.504 104.770
Case 13 M (tm)	906.125 69.030 7354.851 7387.374 8.166	1 266.036 1 652.036 1 698.995 1 97.888	001720	8.0 0.0	-265.726 -105.936 -57.254 223.843 393.833	-517.622 -259.326 2.370 267.465 535.961	272.801 11.753 299.706 591.059	-512.096 -252.879 9.737 275.754 5.45.170	-351,726 -171,752 11,623 198,397 388,571	-335.843 -190.587 -41.932 110:124
L(m)	0.000 0.989 1.978 3.967	0.000 0.989 2.978 3.987	0 8 4 9 8	0.8	0 4 8 4 8 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	011848 0000000 000000	0.00 0.00 0.00 0.00 0.00 0.00	0.000 1.500 3.000 6.500	0.000 1.500 3.000 6.000	0.000 1.500 3.000 4.500 6.000
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Σ	(t)	-1400.837 -1400.837	-142.551 -142.551 -142.551 163.551 103.356	1274.357 1267.463 184.591 266.142	249.825 249.825 249.825 	283.170 198.832 -360.898	85. 42.415 -158.225 -158.225	000000000000000000000000000000000000000		267,463 -1184,591 -1184,591 -1184,591 -1384,591
MINIMU	S (t)	0.000	18228 5525 5525 6852 6852 1853 1853 1853 1853 1853 1853 1853 1853	2096.209 779.608 508.144 -724.999 -790.981	1832,786 599,642 573,874 -718,366 -1758,310	1010,150 753,382 689,337 -679,941	2451.939 1267.628 582.873 -457.071	22.407 0.000 -0.003 -0.003	12.92 50.41 12.10 74.61	-1089.995 -945.597 -266.535 -431.074
Σ	M (tm)	0.000	-492.254 110.514 686.591 -89.434	-1007,336 -200,660 -448,777 -255,505 -1017,457	-795,403 -189,407 390,897 -268,083	-1113,183 -355,115 -333,245 -1093,092	-1323.443 41.695 564.852 51.921 -492.254	0.000	87.57 08.99 76.29 89.45	361.770 -577.537 -1178.465 -1098.629 -319.687
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MAXIMU	S (+)	0.000	1329.321 289.377 307.356 -681.887	1419.559 379.616 799.498 -471.446	1387.289 1013.861 1018.093 -1043.861 -3105.814	411.538 471.446 475.678 -543.330	634.408 681.887 1378.393 -683.560	22.407	39,63 81,45 11,114 71,29	1380.846 1355.280 1329.0879
Σ	M (tm)	0.000	377.031 503.352 862.499 80.703 -606.425	1.46.55 269.56 626.2008 1.72.765 2007 1.0008	-37.536 395.740 1415.376 395.740 -651.417	-215.691 172.767 626.285 -147.374	-491.781 80.703 1455.010 791.711	54.390 0.000 0.000 298.942	85.140 85.140 7240 7240	1783,700 667,614 58,610 938,237 2648,544
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MINIMU	s (t)	-1561.386 -826.685 -73.438 698.355 1.188.695	-1269.651 -460.765 366.666 341.693	-1497.176 -844.555 -179.880 496.848	-458.853 149.916	173.586 177.326 -142.551 -142.551	275,685 279,425 -139,179 -139,179	305.551 309.290 65.234 -107.596	285.117 288.857 107.596 -63.497	202.673 206.413 124.912 -197.193	158.125 161.955 -85.977 -85.977
×	M (tm)	-117.151 -1299.581 -1746.220 -1438.727 -358.759	-513,534 -1370,754 -1318,815 -952,261 -298,614	360,988 -797.991 -1305.567 -1149.820 -318.828	133.500	1450.691 1847.508 154.867 1372.686	-833.114 -416.781 -16.625 -225.393 -434.162	-908.743 -447.612 -138.998 -300.392	-850.241 -419.760 -22.395 -97.453	-601,311 -294,497 14,993 -262,408 -558,197	-551. -301.7898 -1301.785 -147.405 -276.337
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MAXIMU	S (t)	-1599,007 -799,503 0,000 799,503	-13129,082 -1329,579 -530,076 -269,428	11859. 110859. 11080. 1260. 12	-896.368 0.003	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	-139.179 -139.179 283.165 286.904 290.644	-107.596 -107.596 -107.596 316.770	163,497 2093,5497 2063,5497 300,076	-197.193 -107.193 -197.193 213.892 217.632	-85.977 -85.977 142.551 169.444
×	M (tm)	2348, 153 1162, 089 766, 735 1162, 089	12548,544 938,237 18,637 -110,253 1297,819	985,727 1111,924 1681,345	386.511	2682.620 268.793 98.128 355.688 635.701	400,911 192,143 5,161 432,713 865,875	345.182 183.789 22.395 491.479 969.438	188.282 93.037 16.330 458.030 905.339	624.960 329.170 33.381 336.959 659.602	110,526 -18,440 -54,967 195,358 452,329
	Case	စစ္စေဇ ဦးဦးဦးဦးဦး	တ္တေလာင္ ၂၂၂၂၂ ၂၂၂၂၂	44000	0 0 - 7	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	000000	000000 11111	00000 66000	00000 	00000 11111 11000
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* * *		97.97	11111	007.33 177.16 626.28 172.76 306.23	96.20 25.26 99.49 71.44	274.35 274.35 274.35 274.35		46.55 69.00 58.07 47.92	419.55 379.61 353.84 765.21	51.56 51.56 51.56 56.14 56.14
1 * * * 1 1 * * * 1		0.000 0.989 1.978 2.967	00000	-651.417 395.740 1415.376 -248.378	3105.814 1043.861 1018.093 -659.269	.381.953 -381.953 -381.953 249.825 249.825	ဂုပ္ဂ်ပ္ဂ်ပ္ ထမ္မာ့စုစု	-37.536 246.116 503.269 395.740 -651.417	1387,289 347,345 321,577 -1043,861 -3105,814	-646.015 -646.015 -646.015 -381.953
u n n n n n n n n n n n n n n n n n n n		0.000 0.989 1.978 2.967 3.956	00000	-1113.183 -355.115 377.237 -147.374 -698.483	1010.150 753.382 727.614 -543.330 -1583.273	283.170 283.170 283.170 283.170 283.170	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	-215.691 104.948 400.104 -177.161 -1007.336	411,538 385,770 360,002 -825,266 -2096,209	1360.898 1360.898 1360.898 1274.357
1 * * * 1		0.000 0.989 1.978 2.967 3.956	00000 11111 666	-1323.443 79.037 1455.010 51.921	2451.939 1404.161 1378.393 -457.071	85.977 85.977 -158.977 -158.225	44888	-491,781 49.278 564,852 791,711 100.891	634,408 608,640 582,873 -683,560 -2745,514	-158.225 -158.225 -158.225 -158.225 -158.225 -158.225
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1 * * * 1	H-1000	0.000 0.989 1.978 2.967 3.956	00000 00000	929.922 226.192 -1111.924 -458.453 985.727	- 939.632 - 481.458 256.988 1060.491 1859.994	1302.045 1302.045 135.178 135.178 135.178	00000 00000	-46.740 -335.063 -696.151 208.142 917.825	-1338.019 -668.769 -57.744 471.297 965.877	135.178 -103.266 -103.366 1302.045 1302.045
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Σ	N (t)	1346.667 690.892 690.892 690.892	274.357 274.357 274.357 390.816	135.178 135.178 175.178 177.178	0,000	1882.184 1913.449 1944.714	-2835,129 -2835,129 -2866,394 -2897,660	1 3603 1 3663 1 3665 1 3665 1 3665 1 3728 1 3728 1 3728	-2734.095 -2765.361 -2796.626 -2857.891 -2859.157	-4035.212 -4065.477 -4097.743 -4129.008 -4160.273	-2767,921 -2799,186 -2830,451 -2861,717
MINIMU	(1) 8	-1642,478 -950,133 -382,743 196,782 788,443	-2129.082 -1329.082 -530.076 269.428 982.178	-1911.026 -1060.491 -260.988 496.848	-896.368 0.003	1142.551 1142.551 1142.551 1142.551	139,179 139,179 139,179 139,179	-107.596 -107.596 -107.596 -107.596	-63.497 -63.497 -63.497 -63.497	1197.1193 1197.1193 1197.193 1197.193	1 855, 977 1 855, 977 1 855, 977
'n	M (tm)	25.855 164.398 -495.709 -588.567	2648.544 938.537 110.253 -298.614	739,622 1111,924 11149,820 -318,828	386.611	268.793 268.793 54.967 158.859	1900.911 1900.911 116.605 1225.3393 1.000	345.182 183.789 22.395 -138.998	188.282 93.037 -2.208 -192.698	624.960 329.170 33.381 -262.408 -558.197	110.526 -18.440 -147.405 -276.371
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MAXIMO	s (t)	-1505.388 -762.514 0.000 759.503 1559.007	-1269.651 -448.644 428.478 1327.775	-1497.176 -844.555 -877 986.441	149.916	173,586 177,326 181,065 184,805	275.685 279.426 286.904 290.644	305.551 309.290 313.030 316.770 320.510	285,117 288,857 292,597 296,336 300,076	202.673 206.413 210.152 213.892 217.632	158 151.965 155.704 169.444
S	X (tm)	1379,654 693,945 766,735 1162,089 2348,153	1335,834 1335,831 1469,191 1297,819	360.988 -797.991 -1221.971 -681.345 791.948	133,500	1450,691 187,508 81,286 355,688 635,701	1803 1114 1503 1114 1503 161 161 161 163 173 173	-908,743 -447 19,129 491,479 969,438	-850,241 -419,760 16,330 458,030 905,339	1.601 1.294.1.431 1.7.926 3.35.959 6.59.602	-541,898 -301.755 -56,004
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. MINIM	s (t)	0.000	29.32	289.37 263.60	-776.334	200	19.55	353.848	86.09	711.86	87.28	47.34	718.366	58.31	11.53	22.72	20.000	-1719.885	34.40	08.54	582.87	-457.071 -1497.015	.40	0	-0.003 695.217	57,61	568,76	7, 70	1230.830	586.02	1008.48	413,143	
×	M (tm)	0.000	77.03	03.35	-25		000	558.077	81.35	946.27	37.53	46.11	-268,083	65.93	-215,691	4 P . C C	333.24	93.09	1.78	49.27	 	51.921	. 63	0.000	0.000	619.36	30.000	24.00	449.4	939.00	048.45	-998.664 -223.949	
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N	N (t)	0.000	03.35	03.36	103.366	•	66 I.4	266.142	56.14	66.14	45.65	46.66	346.667	46.66	283.170	77.90	7.00	83.17	5.97	5.97	5.07	85.977	.00	0.000	1490,590	1302.045	302.04	302.04	302.04	011.40	011.40	1011,401	
MAXIMU	S (t)	0.000	04.06	33.12	1963.588		27.44	505.731	65.21	790.98	83.65	12.70	1684.002	23.94	1010.150	00.00	543.33	83.27	451.93	404.16	378,39	-2745.514	.40	0	148.607	39	481.45	71.00	5.87	80.84	355.28	175.705 718.825	
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MAXIMU	\$ (1)	1505.088 1382.743 196.782 788.443	11506,466 -902,630 -286,577 341,693	-11497.176 -844.555 -179.880 496.848 1185.630		275 275 275 283 285 295 296 296 296 296 297 297 297 297 297 297	305.551 309.290 313.030 316.770 320.510	285.117 288.857 292.897 300.076	202.673 206.413 210.152 213.892 217.632	158.225 161.965 165.704 169.444 173.184
Z.	M (ta)	1379.654 164.398 -495.709 -588.667 -102.474	802.865 -389.440 -978.509 -952.251 -298.614	360.988 -797.991 -1149.8567 -318.828 298.942	. 60 . 60 . 70 . 70	-833,114 -416,781 5,161 432,713 865,875	-908.743 -447.612 19.129 491.479 969.438	1850.241 1419.760 16.330 458.030 905.339	-601.311 -294.497 17.926 335.959 659.602	-541.898 -301.755 -56.004 195.358
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MAXIMUM	()	0.000	2273,704 2227,963 227,963 -513,564	1046.540 292.957 544.376 -349.206	1027.279 595.827 677.155 -695.827 -2068.810	315.954 339.206 330.534 1145.760	469.172 491.172 913.962 1832.003	0.03	480.473	1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	-977.005 -601.906 -219.453 -869.673
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DMINIMO	S (t)	-1131.439 -599.047 -53.216 506.055 1078.764	11 200 200 200 200 200 200 200 200 200 2	-1052,963 -650,303 3.042 359,533 844,976	-337,222	105.393 107.660 102.544 102.544	171.064 173.331 -90.161 -90.161	1886. 1886. 1886. 1886. 1886. 1866. 1866. 1866. 1866. 1866.	171.678 173.945 64.089 -39.603	118.850 121.116 80.369 -123.493	102:544 -522:294 -522:297 -52:2997
Σ.	M (tm)	178,919 11259,401 11036,583	2.00011 2.00011 2.00012 2.0000 2.0000 2.0000	290.687 -560.919 -882.097 -754.272	.106.335	105.936 1105.936 1113.830	1 259.326 1 110.863 1 146.105	1 2753, 9 201 1 2 2 2 2 3 3 4 4 5 8 5 4 2 3 9 5 5 4 2 3 9 5 5 4 2 3 9 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	1 1 2512 2 8 9 9 8 9 9 9 9 9 9 9 9 9 9 9 9 9 9	-351.726 -171.752 10.834 -164.250	-347.618 -193.802 -97.327 -175.773
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Σ	N (t)	226.194 226.194 226.194 226.194	162,105 162,105 162,105 162,105 -175,790	81,736 81,736 81,736 -52,297	0.000	-1340.753 -1363.409 -1201.766 -1057.909	-1958,443 -1981,099 -1662,657 -1685,313 -1707,969	-2434.869 -2457.525 -2482.000 -1593.216 -1615.872	11910.063 11932.719 11613.407 11636.063	-2716.466 -2739.122 -2750.026 -1765.596 -1788.352	-1848.240 -1870.896 -1386.065 -1408.721
MAXIMU	s (t)	-1105.093 -552.546 0.000 552.546 1105.093	-1422.219 -869.673 -317.127 235.420 1503.309	-1272.853 -720.306 -167.760 656.256	-602.383	-102.544 -102.544 -12.836 112.193	-90.161 -90.161 175.597 177.864 180.130	165,548 165,648 193,102 195,369	139.603 176.211 178.478 180.744	-123.493 -123.493 -113.700 125.649 127.916	-52.297 -52.297 102.514 102.544
· · · · · · · · · · · · · · · · · · ·	% (tm)	1493.394 673.691 400.457 673.691 1493.394	1671,929 538,588 -48,284 -88,688 869,651	669.316 -316.301 -755.450 -470.347	259.735	347,618 1997,618 67,461 223,843 393,833	259.621 124.379 2.370 267.465 535.961	213,488 113,516 13,732 299,706 591,059	117,311 57,907 9,737 275,754 545,170	391.468 205.229 21.018 198.397 388.571	59.065 113.086 111.830
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N N	1215 1215 1215 1215 1215 1399 1215 1399	-175.790 -175.790 -175.790 -175.790	1.522.2937 1.522.2937 1.522.2937 0.000	-1340,753 -1363,409 -1386,065 -1408,721 -1431,377	-1970.195 -1992.851 -2015.507 -2038.153 -2060.819	-2436,688 -2459.344 -2482.000 -2504,656	-2436.688 -2482.000 -2564.656 -257.312	-2716.466 -2739.122 -2761.778 -2784.434 -2807.090	-1861.811 -1884.467 -1907.123
G) S	-1131.439 -539.047 -53.216 506.055 1078.764	-921 922 -335.773 263.815 876.843 1503.309	-1303,781 -663.875 -10.529 656.256 1336.481 -480.473	102.544 102.544 102.544 102.544	96 8 8 8 8 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	64.089 64.089 64.089 64.089	1123 1123 123 123 123 123 123 123 123 12	-31.490 -31.490 -31.490
M (tm)	85.941 -770.892 -1094.544 -871.723	1809.445 1869.174 1306.227 869.651	520.162 1788.552 1770.347 13.954 206.603	347.618 193.802 39.986 -113.830	230.272 109.719 -10.834 -131.387	206.000 109.866 13.732 -82.402	-206.000 -109.866 -13.732 82.402 178.535	206,229 206,229 20,990 1164,250 1349,489	1,421 -45,814 -93,048 -140,283
Case	00000 00000	00000	00000 11	00000		55555	00000	00000 00000	C C - 15 C - 15 C - 15 C - 15
G N	413.430 413.430 413.430 415.430 415.430	232 232 232 232 232 232 232 686 232 686	104.770 104.770 104.770 104.770 0.000	-989.941 -1012.597 -1035.253 -1057.909	-1617.345 -1640.001 -1662.657 -1685.313	-1525,248 -1547,904 -1570,560 -1593,216	-1568.095 -1590.751 -1513.407 -1636.063 -1658.719	-1697.728 -1720.384 -1743.040 -1765.696 -1788.352	11091.574 -1114.230 -1136.886
s (t)	-1048.3834 -231.966 -231.966 -187.261	-1044.876 -1610.916 -169.832 -169.532 -735.383	11052.2563 11082.2563 1118.504 359.533 844.976	105.393 107.660 109.926 112.193 114.459	171.064 173.331 175.597 177.864 180.130	186.303 188.569 190.836 193.102	171.678 173.945 176.211 178.478 180.744	118.850 121.116 123.383 125.649 127.915	95.704 97.971 100.237 102.504
M (tm)	906,425 69,030 -364,661 -387,374 8,166	1 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	290.687 -8522.081 -872.809 -754.272 -159.244 206.603	1265.726 1105.936 57.254 223.843 393.833	-517.622 -259.326 2.370 267.465 535.961	-553.954 -272.801 11.753 299.706 591.059	-512.096 -252.879 9.737 275.754 545.170	1 1 051 771 725 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	-335.843 -190.587 -41.932
Case	10000 22222	20000	000000	00000	00000	00000	00000	00000	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2
(m)	0.000 0.989 1.978 2.967	0.000 0.989 1.978 3.967	00.00000000000000000000000000000000000	0.000 11.000 0.000 0.000 0.000	0.000.0000.000000000000000000000000000		0.000	0.000 1.5000 1.0000 6.000	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
ON.	13.22.13	m	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	0=0000 et 1 * * * 1 0 0	1 * * * 1	4 0 1 * * * 1 0 = 0 to 4	## ## # ## ## ## ## ## ## ## ## ## ## #	0 4 ! * * * A ~ 6100	12 ← 61 € 1 * * *

UHURU - PIER

Longitudinal direction
Calculation of pillar
action force for bottom of pillar (S.L.S)

		N KN	Нки	y ^m	M = H · у кыш
Super	Rd	8084.0		-ù-	
structure	R Q	2978.2			
Braking			504.0	6.100	3074.4
Seismic		· · · · · · · · · · · · · · · · · · ·	1310.2	6.100	7992.3
beam		$23.6 \times 1.00 \times 0.80$ $\times 21.5 = 406.0$	40.6	5.500	223.3
Pillar		$23.6 \times 0.80 \times 0.80 \times 5.00 \times 6 = 453.2$	45.4	2.500	113.5
Braking		11921.4	504.0		3074.4
Seismic		8943.2	1396.2	_	8329.1

action force for bottom of pillar (U.L.S)

	Ики	Нки	Мкиш
 Braking	$ \begin{array}{c} 11921.4 \times 1.2 \\ \times 1.15 = 16451.6 \end{array} $	$504.0 \times 1.25 \times 1.1$ = 693.0	$3074.4 \times 1.25 \times 1.1$ = 4227.3
Seismic	8943.2×1.2 ×1.15 = 12341.7	$1396.2 \times 1.5 \times 1.1 = 2303.8$	$8329.1 \times 1.5 \times 1.1 = 13743.1$

Calculation of stress for U.L.S.

for action force of one pillar

$$N = 12341.7/6 = 2057.0$$
 KN

$$H = 2303.8/6 = 384.0$$
 KN

$$M = 13743.1/6 = 2290.5$$
 KNm

section
$$b = 80^{cm}$$
 $h = 80$ $d = 67.5$ $d' = 12.1$

Ma = M + N (
$$d - \frac{h}{2}$$
) = 2290.5 + 2057.0 (67.5 - $\frac{8.00}{2}$) $\times 10^{-3} = 2856.2^{KNm}$

$$As = As' = Y_{32} - 16^{NO} = 8.042 \times 18^{NO} = 144.76 \text{ cm}^2$$

$$\chi = \frac{(0.87 - 0.72) \times 41000 \times 144.76}{0.40 \times 2500 \times 80.0} = 11.4^{\text{cm}}$$

$$Z = 67.5 - \frac{11.4}{2} = 61.8^{m} < 0.95 \times 67.5 = 64.1^{cm}$$
 OK

$$M_{RS} = 0.87 \times 41000 \times 144.76 \times 61.8 \times 10^{-5} = 3191.1^{KNm} > Ma = 2856.2^{KNm}$$

$$M_{RC} = (0.72 \times 41000 \times 144.76 \times 55.0)$$

$$+0.40 \times 2500 \times 80.0 \times 11.4 \times 61.8$$
) $\times 10^{-5} = 2913.9^{\text{KNm}} > \text{Ma} = 2856.2^{\text{KNm}}$ OK

Asn=A'sn = As
$$-\frac{N}{0.87 \text{fy}}$$
= 144.76 $-\frac{2057.0 \times 10^3}{0.87 \times 41000}$ = 87.1 cm²

$$<$$
 A su = A'su = Y₃₂-6^{NO} ×2 = 8.042×12^{NO} = 96.50 cm² OK

$$P = \frac{A su}{bd} \times 100 = \frac{96.50}{80 \times 67.5} \times 100 = 1.787 \%$$

$$Vc = \frac{N}{bd} = \frac{384.0 \times 10^3}{80 \times 67.5} = 71.2 \text{ N/cm}^2$$

$$< V ca = 65.0 + 20.0 \left(\frac{1.787 - 1.00}{1.00}\right) = 80.7 \text{ N/cm}^2$$
 OK

Calculation of stability for Foundation

- 1) action force for bottom of Foundation
 - (1) Longitudinal derection for S.L.S.

State	load	Nĸn	Нки	y ^m	М= Н ∘ у ^{кми}
Super	Rd	8084.0		_	
structure	R &	2978.2		_	
beam		$23.6 \times 21.50 \times 1.00$ $\times 0.80 = 406.0$	40.6	6.50	263.9
Pillar		$23.6 \times 0.80^{2} \times 5.00$ $\times 6 = 453.2$	45.4	3.500	158.9
footing		$\begin{array}{c} 23.6 \times 21.50 \times 4.00 \\ \times 1.00 = 2029.6 \end{array}$	203.0	0.500	101.5
surcharge		$ \begin{array}{c} 18.6 \times 21.50 \times 4.00 \\ \times 1.00 = 1599.6 \end{array} $		_	<u></u>
Braking		_	504.0	7.100	3578.4
Seismic			1310.2	7.100	9302.5
Braking		15550.6	504.0		3578.4
Seismic		12572.4	1599.2	<u>-</u> -	9826.8

(2) Longitudinal load for U.L.S.

load State	Nĸn	Нки	Мкив
Braking	15550.6×1.2 $\times 1.15 = 21460.0$	$504.0 \times 1.25 \times 1.1$ = 693.0	$3578.4 \times 1.25 \times 1.1$ = 4920.3
Seismic	$12572.4 \times 1.2 \\ \times 1.15 = 17350.0$	$1599.2 \times 1.5 \times 1.1 = 2638.7$	$ 9826.8 \times 1.5 \times 1.1 \\ = 16214.3 $

2) Stability of Foundation for Longitudinal direction.

(a) Braking state

$$e = \frac{M}{N} = \frac{3578.4}{15550.6} = 0.231^{m} < \frac{B}{6} = \frac{4.00}{6} = 0.667^{m}$$

$$q = \frac{N}{B \cdot L} (1 \pm \frac{6e}{B}) = \frac{15550.6}{4.00 \times 21.50} (1 \pm \frac{6 \times 0.231}{4.00}) = {243.5 \text{KN/m}^{2} < \text{qa=350 KN/m}^{2}}$$

$$F = \frac{N \cdot \mu}{H} = \frac{15550.6 \times 0.50}{504.0} = 15.4 > 1.5 \qquad 0 \text{K}$$

(b) Seismic state

$$e = \frac{M}{N} = \frac{9826.8}{12572.4} = 0.782^{m} > \frac{B}{6} = 0.667^{m}$$

$$x = \frac{B}{2} - e = \frac{4.00}{2} - 0.782 = 1.218^{m}$$

$$q_{max} = \frac{2 \cdot N}{3 \cdot x \cdot L} = \frac{2 \times 12572.4}{3 \times 1.218 \times 21.50} = 320.1 \text{ KN/m}^{2} < qa=350 \text{ KN/m}^{2}$$

$$Fs = \frac{N \cdot \mu}{H} = \frac{12572.4 \times 0.50}{1599.2} = 3.9 > 1.5 \text{ OK}$$

(2) For U.L.S

a) Braking state

$$\begin{split} \mathrm{e} &= \frac{\mathrm{M}}{\mathrm{N}} = \frac{4920.3}{21460.0} = \ 0.230^{\mathrm{m}} < \frac{\mathrm{B}}{\mathrm{6}} = \ 0.667^{\mathrm{m}} \\ \mathrm{q} &= \frac{\mathrm{N}}{\mathrm{B} \cdot \mathrm{L}} \left(1 \pm \frac{\mathrm{6e}}{\mathrm{B}} \right) = \frac{21460.0}{4.00 \times 21.50} \left(1 \pm \frac{6 \times 0.230}{4.00} \right) = \left(\frac{335.7 \, \mathrm{KN/m^2}}{163.4 \, \mathrm{KN/m^2}} < \, \mathrm{qa} = 525 \, \mathrm{KN/m^2} \right) \\ \mathrm{Fs} &= \frac{\mathrm{N} \cdot \mu}{\mathrm{H}} = \frac{21460.0 \times 0.50}{693.0} = 15.5 > 1.1 \quad \mathrm{OK} \end{split}$$

b) Seismic state

$$e = \frac{M}{N} = \frac{16214.3}{17350.0} = 0.935^{m} > \frac{B}{6} = 0.667^{m}$$

$$\chi = \frac{B}{2} - e = \frac{4.00}{2} - 0.935 = 1.065^{m}$$

$$q_{\text{max}} = \frac{2 \cdot N}{3 \cdot \chi \cdot L} = \frac{2 \times 17350.0}{3 \times 1.065 \times 21.50} = 505.2 \text{ KN/m}^{2} < q_{\text{a}=525 \text{ KN/m}^{2}}$$

$$Fs = \frac{N \cdot \mu}{H} = \frac{17350 \times 0.50}{2638.7} = 3.3 > 1.1 \text{ OK}$$