(G) N	20.24	2.24	20.24	20.24	-20.242		3.5	000.0	147.31	1173.74	1200.17	226.60	-1253.038	931.86	1958,29	984.72	2011.15	2037.	860.88	1887.31	r-	1940.18	1966.61	698.78	2725.21	2751.64	778.07	2804	1881.35	1907.78	-1934.214	1960.64	1987.07
S (t)	364.86	-708.00	32.37	63.42	1377.951		3.16	-0.123	ა. გე	5,85	50.00	5.85	35.855	2.57	2.57		2.57	2.57	3.23	3.23	-3.231	3.23	3.23	4.95	64.9	64.95	4.95	64.95	20.24	20.24	-20.242	20.24	20.24
Case 12 M (tm)	7.	528.57	86.24	583.96	396.382	, , ,	n n	000.0	9.64	-6.90	5.84	8.59	181.336	85.5	3.50	49	0.50	2.51	.46	8.80	3.151	2.50	. 15	48.67	135.002	21.32	92.34	06.02	10.09	45.51	-80.940	116.36	51.78
N (t)	5.76	55.764	5.76	5.76	Ø	Ç	2	000.0	1267.47	1293.91	1320.34	346.	1373.20	177.41	2203.84	30.	2256.71	2283.14	2630.06	2656.50	-2682.932	2709.36	2735.79	2177.74	-2204.173	2230.60	2257.03	2283.46	1267.48	1293.91	-1320.351	1346.78	1373.21
\$ (‡)	375.51	6.3	237.63	31.60	00.2	0	0000	-0.077	55.73	55.73	55.73		53	2.34	52.34	-52.343	52.34	52.34	0.05	0.05	-0.051	0.05	0.05	2.36		2.36	2.36	2.36	5.76	5.76	55 764	5.76	5.76
Case 11 M (tm)	47.76	102	04.88	559.64	32.88	ď	7	0	33.94	36.39	8.85	ŝ	6.23	0.5	98.99	7.39	84.20	5.80	23	. 13	0.050	0.03	12	0.71	99	7.42	4.22	5.86	4.01	136.43	33	58.74	6.33
£ . z	.28	-8.280	28	8.28	8.28			8	598.61	1635.08	1671.56	-1708.039	1744.51	2652.06	2688.53	ß	2761.49	2797.96	2565.19	2601.67	-2638.147	2674.62	2711.10	3710.40	-3746.884	3783.36	3819.83	3856.31	2611.58	2648.06	-2684.538	2721.01	2757.49
\$ (£)	68.18	-961.719	29.35	930.84	16.89	0 V C V X		0.16	9.82	9.82	9.82	29.827	9.82	63.988	3.98	3.98	3.98	3.98	. 45	4.45	-4.459	4.45	4.45	81.07	-81.075	81.07	81.07	81.07	8.28	8.28	-8.280	8.28	8.28
Case 10 M (tm)	18.17	846	25.43	893.52	74.08	u t) · · · · · · · · · · · · · · · · · · ·	0.00	1.46	0.73	72.93	125.128	77.32	-226.025	114.04	-2.06	16.6	21.88	•	2.15	4.34	. 45	1.25	3.19	171.312	29.43	112.45	54.33	78.58	93.07	-107.566	22.05	136.54
L(m)	00.	0.963	92	88	8	0		8.	00.	.75	.50	5.250	8	0.000	. 78	.50	വ	90.	000.0	7.	00.	23.53	8	00	1.750	. 50	. 25	8.	00.	7.3	3.500	.2	00.
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s (t)	0.000	1142.921 268.070 249.907 -624.944 -643.107	1279.030 404.179 386.016 -488.835 -506.998	1351.911 477.060 458.897 -415.954 -1130.787	1558.267 883.686 865.523 -505.728	671.504 0.000 -0.179 336.593	-927.160 -503.939 -61.708 400.453 881.583	-1146.282 -645.182 -125.152 414.888 973.817	-990.820 -411.840 185.989 803.908 1440.636
Case 15 M (tm)	0.000	18.394 286.187 535.304 -57.763 -67.763	-503.150 -104.315 -175.769 -186.225 -665.221	-679.682 -210.663 -239.532 -152.278 -561.859	-789,570 70,830 912,200 433,938 -62,006	-6.698 0.000 0.000 133.669	264.013 -426.624 -700.259 -538.716	237.934 -626.219 -998.308 -860.363	-202,113 -879,057 -989,250 -514,178 563,890
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s (t)	0.000	1263.090 388.238 370.076 -504.776	1644.751 759.737 751.737 -619.514 -637.677	1990.413 619.161 500.999 -770.253 -1645.085	522.930 504.749 486.586 -388.265	15.104 0.000 -0.083 472.804	-911.118 -341.881 226.765 796.002	-908.769 -339.532 229,113 798.350 1366.996	-1366.821 -797.584 -228.939 340.298 908.944
Case 14 M (tm)	0.000	-285. + 988. + 63.039 -114.305 -368. -368.	-720.199 730.825 762.733 174.895 -429.814	-430.041 176.319 763.216 30.217 -720.359	-508.683 -13.856 462.977 97.832 -285.272	-6.042 0.000 0.000 189.088	-18.136 -621.455 -676.826 -184.364 854.909	658.140 57.083 3.971 498.695 1540.227	1540.100 497.938 4.181 57.801 658.686
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s (£)	0.000	887.313 172.462 154.299 -560.552 -578.715	961.782 246.931 228.768 -486.083	963.573 248.721 230.559 -484.293 -1199.125	410.405 392.223 374.061 -340.791	15.104 0.000 88.476 335.657	-672.488 -363.485 -42.148 292.041 638.437	-1007.789 -648.465 -276.960 107.499	-1069.434 -659.841 -238.119 196.610 643.441
Case 13 M (tm)	41.44	299.192 432.102 546.697 -26.982 -617.546	-49.151 155.474 341.708 -160.257 -679.182	-116.125 90.224 278.180 -222.061 -739.961	-297,631 46,213 372,218 10,169 -369,682	29.366 0.000 0.000 169.075	629. 1229. 129. 156.317 152.997	1091.536 293.041 -153.094 -235.699 57.480	657.143 -176.512 -609.436 -630.432
C (m)	0.000	0.000 0.963 1.925 2.888 3.850	0.000 0.963 1.925 3.888	0.000 0.963 1.925 3.858	0.000 0.963 0.888 858	0.0000.0000.000000000000000000000000000	0.000 0.000 0.000 0.000 0.000 0.000	00.00 00.00 00.00 00.00 00.00 00.00 00.00	32 H O O O O O O O O O O O O O O O O O O
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s (t)		697.28	21.65	74.14	88.66		9	,	4.5	1 .	77.0	22.111	6.58	6.58	6.58	46.585	6.58	3.23	3.23	-3.231	3.23	3.23	58.96	60	58.96	58.96	58.96	6.49	6.49	6.49	. 49	tO.
Case 15 M (tm)	378.	10.49	957.86	645.26	345.39	S.	0	*) () () () ()) i	7.0	130.344	4.54	83.02	1.49	8	1.54	.46	8.80	3.151	2.50	13	27.71	124.519	21.32	81.86	5.05	5.30	66.67	8.05	89.4	0.79
N (t)	69.508	9.50	9.50	9.50	9.50	8	0.000	978 10	1200110	1991.06	357.40	1383	167.68	2194.12	2220.55	-2246.985	2273.41	2628.09	2654.52	Ó	2707.38	2733.81	168.01	-2194.448	2220.88	247 31	2273 74	1278.20	1304.63	1331.06	-1357.498	1383.93
(÷) s	7.9	795.56	226.91	42.32	10.96	2.96	9	ď	0 4 A	07.00	169.482	69.48	33	58.33	58.33	-58,333	58.33	0.05	0.05	-0.051	0.05	0.05	8.35	58.358	8.35	8.35	8.35	9.50	9.50	9.50	69.508	9.50
Case 14 M (tm)	85.5	84,69	676.50	620.94	18.10	2	0	79.1	7	30.00	9 20	7.22	11.56	.48	7.39	94	6.76	2	.13	0.050	0.03	. 12	11.67	-109.548	-7.42	94.70	6.83	9.23	157.59	35.95	85.686	7.32
N (t)	112.750	12.75	12.75	12.75	12.75	00	000 0	902 41	428 84	0.000	-981.713	008.14	1540.49	1566.92	1593.36	-1619.793	1646.22	1467.81	1494.25	-1520.682	1547.11	1573.54	1609.53	-1635.962	1662.39	1688.82	1715.25	1070.72	1097.15	1123.59	-1150.023	1176.45
s (t)	071.81	611.95	40.01	44.98	42.03	4.42	88	09.16	11.80	34.44	117.094	19.73	(3)	63.12	65.76	ð	71.05	60.81	63.45	166,103	68.74	71.39	25.46	129.111	31.75	34.40	37,04	102.173	04.81	07.46	10.10	12.75
Case 13 M (tm)	252	-559.16	921.86	824.18	254.23	0	00.00	340 64	47.29	50.67	253.278	60.50	ო	285.24	2.07	₹,	91.98	3.05	279.31		.04	99.66	42.32	-218.699	9.55	. 44	79.95	0	217.93	32.18	58.18	53.18
C(m) C	0.000	ο. Ο (7	8	က တ	.00	0.800	.00	7	C.	5.230	8	000.0	73	. 50	. 25	00.	00.	. 75	3.500	. 25	00.	00.	1.750	. 50	.2	00.		75	. 50	.25	00.
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PICK-UP No. 1 *

M	(t) N	-1087.508	196.573	58.82	-183.162 93.814 93.814 141.083	47.42	135.758 135.758 89.355 -382.076	32.953 -96.608 -169.032 -169.032	00000	1174.769	96.573 96.573 -29.827 -29.827	1 1 1 1 1 6 9 9 1 4 4 6 9 9 9 1 4 4 9 9 9 9 1 4 4 9 9 9 9 9 9 1 4 4 9 9 9 9	1 89
MINIMU	S (t)	0.000	536.305	45.68	2420.472 557.816 532.752 -709.021	727.91	1940.138 689.643 633.228 -700.306	2455.676 696.018 493.869 -492.626	1103.904	145.998	-1257.879 -472.332 -85.692 552.089 1216.049	-1581.918 -890.400 -172.759 572.496 1343.818	-1367.282 -568.289 256.714 210.222 838.713
Σ	M (tm)	0.000	-395.829 133.895 637.765	20.4	-1039.780 -145.154 379.410 -260.521	210	-978.485 -301.052 329.403 -356.530 -1113.495	-1172.558 490.997 -41.646 -598.822	-9.420	0.000	-27.577 -860.673 -969.938 -747.525	322.688 -869.891 -1383.421 -1193.105 -273.508	-284.767 -1218.902 -1370.921 -990.661 -487.784
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MAXIMUI	s (t)	0.000	1201.384 214.889 329.899	22.31	1295.206 308.711 1144.913 -924.642)))	1297.748 924.111 899.047 -1170.508	544,023 1347.525 1322.461 -747.094	20.844	-0.125	-904.748 -503.703 -82.359 360.159	-1439.912 -955.920 -384.936 -1215.285 -2044.771	-2044.506 -1214.158 -384.672 445.676
X	M (tm)	0.000	504.809 654.464 800.649	-2.09 02.11	266.319 266.325 335.677	00.00	-83.462 337.467 1214.406 99.288 -776.611	-367.396 138.273 1422.536 715.166	50.084	0.000	989.976 310.142 26.586 158.682 1464.634	1688.494 533.236 303.597 1074.104 2642.190	2641.998 1072.951 303.914 333.288 1161.011
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	(±)	537.99	88.10	218.18	473.1	žą. Πλ		0701104-	-0.19	79.66	84.02	96.57	-96.573	96.57	60.41	١.	63.98	0.74	90.74	65.35	9.71	-0.07	-5.325	5.32	3.04		0.78	102.80	2.80	9.03	73.39	32.95	-32.953	32.95
	M (tm)	18.51	51.32	85.11	1264.0	68.31	. 6	200.121	0.00	571.56	3.33	49.48	119.5	8.51	921.58	-462.039	2.06	45.64	304.44	929.07	60.89	0.06	-4.126	44	746.09	-369.453	13.19	45.66	325.57	48.90	349.28	22.47	-180.137	37,80
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	N (t)	3	2.45	2.45	4.5	2.95	. 5		3	764.44	1800.91	1699.25	55	136	167.85	3204	3240.80	226.36	2262.84	2674.22	2710.70	2098,61	0	2171.56	4028.06	-4064.537	4101.01	2340.22	2376.70	2837.46	2873.94	1572.89	-1609.367	1645.84
	(F)	39.0	208.74	379.25	451.09	68.78	, r		7	96.57	96.57	58.71	192.755	97.11	90.74	-90.743	90.74	73.50	77.86	5.32	5.32	74.07	278.440	82.80	102.80	-102.805	102.80	26.13	30.49	32.95	32.95	77.75	182.121	86.48
1	M (tm)	1465.556	-98.27	62.10	827.51	05.19	ç		5	7.49	18.48	1.05	0	17	30.75	171.955	3,15	79.95	2.40	ω		0.7	98	9.4	4.06	214,153	34.24	6.74	6.29	.13	54.80	42.02	S	95.40
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n o 1 * * * 1 0 ~ ∪ ∪ n	0.000 0.000 1.000 3.888 858	00000 66600	-1172.558 138.273 1422.536 -41.646 -598.822	2455.676 1347.525 1322.461 -492.626 -1479.095	32.953 32.953 32.953 -169.032	000000	-367.396 40.992 490.997 715.166 -384.944	544.023 518.933 493.869 -747.094 -1803.748	-169.032 -169.032 -169.032 -32.953 -92.457
0 / 8 6 1 () 1 1 0 0 8	000000000000000000000000000000000000000	77 77 6 % % %	-9.420 0.000 0.000 275.821	1103.904 0.000 145.998 689.677	0.000 0.000 1174.769 0.000	00 00 11 11 88 88	50.084 0.000 0.000 242.798	20.844 0.000 -0.283 463.384	0.000 0.000 1174.769
0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		989.976 -822.223 -862.583 -97.768 1464.634	-904.748 -450.460 3209.375 2038.861	977.650 92.418 92.418 92.418	00000	470.928 -506.753 -901.247 158.682 726.090	-1307.278 -717.855 -96.953 360.159 822.933	158.711 158.711 977.650 977.650
10 11 11 11 11 11 11 11 11 11 11 11 11 1	0.00 0.00 0.00 1.90 2.80 8.80 8.80 8.80	00000	902.573 332.271 303.597 1074.104 2642.190	-1254.151 -444.550 384.936 1215.285 2044.771	177.372 183.162 183.162 183.162	00000 11111	466.845 533.236 145.498 -329.155 2.579	-1666.793 -955.920 -451.717 73.747 619.380	699.783 699.783 699.783 699.783 699.783
11 13 11 11 11 11 11 11 11 11 11 11 11 1	0.000 0.000 1.000 3.000 3.000 3.000	00000	-284.767 -1218.502 -1335.246 -610.476	-1367.282 -568.289 299.958 1210.633	-89.355 -89.355 -135.758 -135.758	00000 11111 00000	2641.998 1072.951 -898.491 -990.661	-2044.506 -1214.158 -398.187 210.222 838.713	183.238 183.238 415.980 416.980

			MAXIMU	M		W	MINIMUM	M
(m)	Case	M (tm)	S (t)	N (t)	Case	M (tm)	S (t)	N (t)
0.000	ა დ ქექ	318.510	-1537,990	186-484	0 6	1465.556	-2039.090	92.457
92	H	1325.43	-29.3	-8.28		862.10	379 25	6.4 0.4 0.10 0.10
88		867.64	998.5	32.95	ī	27.51	51.09	2.45
es CS	1	05.19	8.7	2 95	1	468.31	84.51	6.48
0.00.0	ر د 8	127.083	61.	0.000	ပ	3	55	00
80	ı	00.	6.00	00.	C- 7		0	0.000
00		571.56	79.66	1222.22		87.49	96.57	764.44
75		3.33	84.02	1258.70		8 48	96.57	1800.91
200	& -	72.534	188.392	-1295.180	၈ -ပ	6	-96.573	-1837.396
23	ī	6.03	92.75	1331.65		119.51	96.57	1873.87
8	ŧ	7.17	97.11	1368.13		88.51	96.57	1910.34
0.00.0	۵ دا ۵	8	60.41	2116.94		30.75	90.74	3167.85
7.5	ı	462.03	64.77	2153.41	ı	1.95	90.74	3204.33
. 50	ı	5.1		-2189.893	C- 6	33	٢	0.80
2.55	ı	.95	73.50	2226.36	ï	145.64	90.74	3277.28
90	i	2.40	77.86	2262.84	1	04.44	90.74	3313.7
00.	,	9.07	65.35	2025.66	,	3.82	5.32	674.22
1.750	ů ů	-460.895	269.714	-2062.137	C- 7	14.511	-5.325	2710
.50	1	4.92	74.07	2098.61	1	5.19	5.32	747.17
.25	ı	8.37	78.44	2135.08	ı	4.12	5.32	2783.65
80.	ı	9.46	82.8	2171.56		. 44	5.32	2820.12
00	1	746.09	13.04	2230.79	ì	94.06	102.80	4028.06
.750	۲- ۵-	-369.453	217.407	-2267.275	C- 7	214.153	020	064.53
.50	ı	4.82	21.77	2303.75	i	34.24	102.80	4101.01
25	ı	06.74	26.13	2340.22	ı.	145.66	102.80	4137.48
00.	i	6.29	30.49	2376,70	ī	.57	102.80	4.1
00	ı	648.90	69.03	1499.93	1	7.13	32.95	837.46
.75	ŧ	9.28	73.39	1536.41	,	64.80	32.95	2873.94
.500	C- 8	-42.024	177.758	-1572.891	C- 7	-122.470	-32,953	٠,-
. 25	ı	2.87	82.12	1609.36	ı	80.13	32.95	2946.89
00.	ı	95.40	86.48	1645.84		237.80	32.95	2983.3

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PICK-UP No. 1 *

N. MAXIMUM

N. MINIMUM

									,
N (F)	-1087.508 -1087.508	-907.842 -907.842 -907.842 -907.842	-647.427 -647.427 -647.427 -647.427	-382.076 -382.076 -382.076 -382.075	-169.032 -169.032 -169.032 -169.032	000.0	55888888888888888888888888888888888888	1141. 1141.0833 1141.0833 1141.0833	-135.758 -135.758 -135.758 -135.758 -135.758
s (t)	0.000	1201.384 214.889 189.825 -796.670 -821.734	1295.206 308.711 283.647 -702.848 -727.913	1297.748 311.253 286.189 -700.306 -1686.775	544.023 518.933 493.869 -492.626	20.844 0.000 -0.125 689,677	-1307.278 -717.855 -96.953 556.719 1241.804	-1666.793 -948.871 -199.602 582.572 1396.025	-1424.104 -577.679 299.958 1210.633 2152.454
M (tm)	0.000	504.809 654.464 778.879 - 46.558	26.319 266.325 480.997 -254.090 -1012.538	-83.462 158.992 376.109 -356.530	1367.396 74.092 490.997 141.646	-8.337 0.000 0.000 275.821	. 506.753 -901.247 -682.449 180.069	466.845 -795.177 -1350.163 -1168.343 -219.209	-232.653 -1199.091 -1335.246 -610.476 1004.597
Case	& & - ∪ ∪ - ∪ ∪		0 0 0 0 0, 0 0 0 0 0	0 0 0 0 0 0 1 1 1 1 1 1 1 1 1 1	00000 111111	99 99 11 11 00 00	66666	00000	4444
N (t)	000.0	588.7111 588.7111 588.7111	141.083 141.083 141.083 141.083	11338 1338 1338 1338 1338 1338 1338 133	8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	0.000 0.000 1174.769 1174.769	977.650 977.650 977.650 977.650	699.783 699.783 699.783 699.783	416.980 416.980 416.980 416.980 416.980
s (t)	0.000	1605,458 354,963 329,899 -920,596 -945,660	1817.033 566.538 541.474 -709.021 -734.086	1940.138 689.643 664.579 -585.916	2455.676 1347.525 1322.461 -747.094 -1733.563	20.844 0.000 145.998 463.384	- 504.748 - 803.703 - 802.359 360.159	-1439.912 -955.920 -451.717 73.747 619.380	-1552.185 -985.249 -398.187 210.222 838.713
M (tm)	0.000	116.094 471.230 800.649 -73.804 -971.473	-681.650 -122.768 410.186 -260.521 -954.655	-978.485 -301.052 350.329 -201.828	-1172.558 138.273 1422.536 715.166	-8.337 0.000 242.798	989.976 310.142 26.586 158.682 726.090	1688.494 533.236 -145.498 -329.155	992.041 -231.399 -898.491 -990.661
Case	1.	99999 2222	66666	66666	66666	ဖေဖ ဆဆ ပေပ ပေပံ	0 0 0 0 0 0 0 0 0 0 0 0	~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~	**************************************
L (m)	0.000	0.000 0.963 2.925 3.888 3.50	0.00 0.000 0.963 2.888 50	0.000 0.963 1.925 3.888	0.000 0.963 1.928 3.888 3.850	000000000000000000000000000000000000000	0.000 0.000 0.963 0.8885 0.8885	22.000 0.000 0.000 0.000 0.000 0.000 0.000	3.5.000
No.	0 H	0 0 0 0 0 0 0 0 0 0	w 4 1 * * * 1 4 → 0 w w	4 n !***I n⊶0w4	n n	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	1	1 * * * 1

PICK-UP No. 1 *

		:							
			N.	MAXIMU	UM		N	MINIMU	Σ
No.	[(B)	Case	M (tm)	S (£)	N (t)	Case	A (tm)	s (t)	.: '
. F		1.0	8.51	37.99	86.48	1	79.02	2021.51	
-	96.		851.32	888.10	86.48		800.84	046.58	
	.92		385.11	218.18	86.48	1	26.34	40.58	
	٠		1264.00	473	48	C- 7	-867.	998.59	
12	(3)	ပ	468.3	84.51	86.4		05.19	68.	
. 막 금	0.000	C- 6	9	6	80.		.01	9	
	.80		0.00	-0-11	0.000	G- 6			
o,	00.		1.56	79.66	1222.22		76.49	92.41	1
	.75		253.33	84.02	1258.70	1	4.75	92.41	ŀ
C-3	3.500	& -	72.534	188.392	-1295.180	Q0	53.026	-92.418	ŧ
က	. 25		6.03	92.75	1331.65	ï	8.70	92.41	ï
61	0	ထ ပ်	47.17	97.11	1368.13		270.43	92.41	ï
01	00.	-	1.58	60.41	2116.94	1	30.75	0.74	ï
	. 75		462.03	64.77	2153.41		1.95	0 74	ï
67 (3.500	ပ ဗ	5.140	269.141	-2189.893	c 5	13.154	-90.743	ï
က	. 23	•	9.93	73.50	2226.36	1	145.64	0.74	
ന	00.	ဂ္	62.40	77.86	2262.84	. 1	04.44	0.74	ì.
H H		-	29.07	65.35	2025.66	1	.34	0.07	1
,	. 75		460.89	69.71	2062:13	i	.20	0.07	1
7	. 50	ထ	4	274.077	-2098:613	C- 6	0.075	-0.076	1
မ်ာ	.25		8.37	78.44	2135.08	τ	.05	0.07	1
4,	8		89.46	82.80	2171.56	1	0.19	0.07	1
12	00.		746.09	13.04	2230.79	,	94.06	102.80	1
r-4	1.750	∞ -¦	-369.453	217.407	-2267.275	C- 7.	214.153	-102.805	i
~	.50		14.82	21.77	2303.75	ŧ	34.24	102.80	4
m ı	. 25		06.74	26.13	2340.22	1	5.66	102.80	1
o.	8		06.29	30.49	2376.70		325.57	102.80	
13	00		48.90	69.03	1499.93	1	-7.13	32.95	13
۰, د	٠. د د		349.28	73.39	1536.41	į	-64.80	32.95	12
v1 (r)	ຸ ນີ້ດ ກີດ		42.04.04.04.04.04.04.04.04.04.04.04.04.04.	77.75	1572.89		122.47	32.95	100
o c	7.000	ပ ပ	595.400	186.484	-1645.843	\ \ \ \ \	1237.805	132.953	2 5
,	•				•				1

-1824.580 -1861.056 -1897.532 -1934.008

-3167.854 -3204.331 -3240.807 -3277.283 -3313.759

-3943.372 -3979.849 -4016.325 -4052.801 -4089.277

-4028.061 -4064.537 -4101.013 -4137.489

-2837.466 -2873.943 -2910.419 -2946.895 -2983.371

0.000

PICK-UP No. 2 *

iei i	N (t)	-659.096		169.482	9.48	2.11	2.11	7.81	8.59	8,69	68.696 -389.455	5.46	5.46	00.40	-228.640	3	ייני מייני	102.17	-102.173	102.17	0.000	00.	711.981	9.48	9.48	20 c	122.111	68.69	68.69	68.69	168.695		65.46	-65,465 -65,465	49.79	49.79
MINIMUM	s (t)	0.000	9 6	388.238	70.07	24.94	643.10	44.75	404.17	86.01	-488.835 -504.246	51.91	77.06	458.89	-1199.125	 	0000	74.06	-340.791	055.62	671.504	8	88.476 336.593	911.11	41.88	61.70	881.583	146.28	45.18	125.15	414.888	, c. o.	90.82	-411.840 185.989	96.61	43.44
Σ.	M (tm)	0.000		98.290	63.03	7.76	67.69	20.19	104.31	75.76	-186.225	79.68	210.66	239.53	-739.961	7 U O O O	10 00	2.2	10.169	9.68	969.9	0	0.000	18.13	621.45	0.25		37.93	626.21	998.30	1860.353	C 70 . C 70 T	202.11	1879.057	630.43	227 37
	Case	000	4 P) 	1	H	H	. H	,,,,	Ţ	ပ် - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1	1	7	Τ,	555		₹ #	1	C-13		C-12	r~t f	C-13 C-15	- es	7	Ţ.	C-13		۳۲ ا	7	C C C C C C C C C C C C C C C C C C C	ત્રં L	۳ ۱ ا	2 C	H	۲4 ا
	(3) N	00000		1540.935	35.85	5.73	55.73	389.45	389.45	108.08	-108.082 -108.082	228.64	8.13	108.13		. 60	7.0		20.242	0.24	0.000	8	0.000	92.24	92.24	2.24	55.739	21.18	21.18	08.08	108.082	00.00	08.13	108.132	08.13	08.13
MAXIMUM	s (#)	0 -) i	172.462	39.19	15.49	533.65	61.78	46.93	50.74	-620.503 -638.666	 63.57	20.15	601.98	-1129.798	: · ·	24.0	76.23	-495.014	209.84	15.104	0	-0.083	672.48	63.43	42.14	1375.363	007.78	48.46	30.10	799	Ø8./0	367.81	-798.573	39.30	07.95
Σ.	M (tm)	0.000		432,102	59.89	-0.03	4.67	49.15	55.47	5.75	196.966 -408.695	16.12	8.39	86.23	-536.931	9		787.9	68	6.79	29,366	8	0.000	9.58	9.77	6.3	947.153	91.53	293.04	9.08		7.7.	7.11	614.003 119.294	1.96	1.89
	Case	0-13		2 C	7	4	1	~ ~	7	7	- - - - - - - - -	74	~! !	Ϊ,	C-11 C-12	-	7 7	1 -	C-112	- 1	C-13		C-14 C-11	극	77	7	-1-0 -1-1-0	7	ï	겉	C-11	⊣ ‡	Ę,	-5 -11 -11	7	7
	(m)	000.0		0.000	8	.83		00	.96	.92	2.888 3.850	00.	96	92	3.888	9	2 0	9 6	2.888	.85	0.000	80	0.000	00	96.	. 92	3.850	00	.96	.92	2.888	8	00.	0.000 0.000 0.000	88	. 85
	No.	. H c	!.	n *	* *	რ *		1 (m			4. ¥ i თთ				ιυ * Ι ω 4.				າ ເວ	ល រ ម	6 - 7	ŧ	o ∞ ∞ o	01 - 6	*	*	ი ი 10 - *	10-	*	* 61		Н	77	* *		-

PICK-UP No. 2 *

PICK-UP No. 2 *

X	N (t)	0.000	949	1549.935	35.85	389.45	339.45	-108.082	228.64	228.64 228.64	-127.866	20.	17.	102.17	6.458	0.000	0.000	22.11	22.11	592.111	92.24	68.69	21,18	421.186 421.186	08.13	49.79	249.794 249.794
MINIMUN	(1) S	0.000	87.31	154.299	653.82	61.78	228.76	-620.503 -638.666	63.57	48.72	-770.253 -1645.085	0 0	10.40	374.06	-505.728 -1263.098	15.104	-0.179	927.16	03.93	-51.708 292.041	38.43	46.28	276.96	107.499	367.81	38.11	196.610 643.441
Ś	M (tm)	0.000	32.1	546.697	63.73	49.15	41.70	196.966 -408.695	6.12	90.22 78.18	30.217		97.03 46.21	72.21	433.938	29.366	0.000 169.075	64.01	6.62		9.55	7.93	53.09		57.11	609.43	-630.432 -227.374
	Case			1 1	1	. H	-4 }-4 	-5-0, 11-0,	1		000 111 144	√ ,		7	C-15	0 113 0-13	0-15 0-13	7	-4	C-13	1 p==1	77	·	0-13 0-13	⊢ +		0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
	N (t)	-659,096 -659,096	69		69.48	127.81	127.81	-389.455 -389.455	108.13	80 00 1.3 1.3			0.24 4.44	20.24	-102.173 -102.173	0.000	711.981	2.24	55.73	55.739 55.739	5.73	08.08	08.08	108.082	65.46	5.40	-65.465 -65.465
MAXIMUM	(3) S	0.000	63.09 88.23	370.076	22.93	44.75	751.73	-486.083 -504.246	91.40	20.15	-414.965 -3130 708	7.60 . 10	58.98 94.40	876.23	-340.791 -1055.623	671.504	88.476	72.48	31.16	237.480	75.36	07.78	230.10	799.340 1367.985	90.82	411.84 185.98	803.908 1440.636
w	M (tm)	0.000	285.19	€0 ~	08.63	20.19	50.02 62.73	-160.257 -679.182	08.92	98.39	128,302	0.000	85.50	36.78	10.169 -369.682	0.000	0.000	29.58	560.14	-605.208	947.15	71.34	19.08	614.761 1657.244	202.11	89.25	-514.178 563.890
	Case	0-13 0-13	~	000 1000 1000 1000 1000 1000 1000 1000	4 17		-	C-13 C-13		75	127	٠ ا	7 7	•	C+13 C-13	C-12 C-11	C-13 C-11	i I	7	55	1		7	0-11 0-11	7		C-15 C-15
	L (m)	0.000	00.	1.925	. 85	80.	9 0	3.888	00.	96	4 C1 C	3	၁ ဗ	. 6.	2.888 3.850	0.000	0.000.0	8	96.	1.020 0.020 0.000 0.000	, w	00.	200	2.888 3.850	86	929	2.888
	No.	01 FE	.က က	, co c		٠.		ლ ლ ლ ლ			. * I				* I დო	L 0	΄ σ φ Ι Ι	+-1		* *		1		* ·			හ ස # 1

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PICK-UP No. 2 *

			w	MAXIMU	×		Ś	MINIMU	Σ
No.	L (m)	Case	M (tm)	S (t)	N (t)	Case	M (tm)	(4) S	£
12 - 13	00	1	52.58	71.81		C-11	47	-1375.514	
*	96	س ا	-559.16	611.95	12.75	넊	102.76	806.27	5.76
·*	o;	Ħ	57.86	21.65	6.49	7	04.88	237.63	5.76
	88	Ė	645.26	74.14	6.49	껕	559.64	31.60	5.76
13 - 12	85	C-15	345.390	9	6.49		254.23	42.03	2.75
; (7)	00		9	4.42	.00	7-4	ც	.12	00.
14 - 13	0.800	C-13	0.000	88	00000	0-18	0	Ŷ	000.0
					:				
6 1 2	00.	ï	340.6	60	902.	41.5	279.153	-69.482	-1278.194
	7.5	7	47.29	11.80	928.84	ij	57.55	69.48	1304.62
*	.50	7	50.67	14.44	955.28	7	35.96	69.48	1331.05
₩ *	5.250	1	53.27	17.09	81.71	Ţ	85.63	9.48	1357.49
1 5	8	-	0.50	.73	008.14	 i	7.22	69.48	1383.92
3 - 10	00	77	568.39	60.48	1540.49	Ţ	11.56	58.33	2167.68
*	7.5	H	2.4	63.12	1566.92	겉	8	58.33	2194.12
*	3.500	7	6	65	-1593.361	C-14	7.397	-58,333	-2220.553
	.25	1	94.94	68.41	1619.79	77	94.68	58.33	2246.9
	00.	C-13	591.985	05	1646.22	7	9.19	58.33	2273.41
4 - 11	Õ.	1	563.05	60.81	1467.81	<u>بر</u>	4.46	3.23	1860.88
	75		65	63.45	1494.25	겉	8.80	3.23	1887.31
*	3.500	0-13	ı	166,103	-1520.682	C-12	3.151	-3.231	-1913.751
	23	7	02.04	68.74	1547.11	퍁	2.50	3.23	1940.18
•	00.	ĭ	99.	71.3	1573.54	幵	8.15	3.23	1966.61
5 - 12	00	7	442.32	26.46	1609.53	Ψť	48.67	64.95	2698.78
*	7.3	7	8.69	29.11	1635.96	Ħ	35.00	64.95	2725.21
*	50	- 1	9.5	(2)	9	C-12	21.327	-64.957	-2751.644
	25	7	42.44	34.40	1688.82	7	92.34	64.95	2778.07
12 - 5	7.000	_	. 95	.04	1715.25	Ħ	0.90	64.95	2804.50
	00	7	99.04	02.17	1070.7		10.09	20.2	1881
	. 7	Ţ	217.93	04.81	1097.15	- , 	20.00	20.24	27.7051
	က်	<u>.</u>	32.18	07.46	1123.59	7	180.94	20.24	1934.2I
	5.250	e e	158.187	110,106	-1150.023		0 0 %	2.0	900.04 987.07
	00.	٦ ١	o	12.13	11.0.40	4	07.707	****	70.7007

PICK-UP No. 2 *

M	N (t)	-659.096	549	549.93	49.93	389.45	389.45	-389.455	0	228.64	228.64	-228.640	102.17	102.17	-102.173	102.17	0.000	0000	3	35.85	35. 20. 20. 20.	135.855	35.85	88.43	188.430	88.43	88.43	85.19	1 000 1 000 1 000 1 000 1 000 1	85.19	85.19
MINIMU	s (t)	0.000	887.313	154.29	60.55 78.71	61.78	28 93 28 46	-486.083	i c	48.72	230.55	-484.293 -1199.125	10.40	92.25	374.061	55.62	15.104	-0.083	00.7	16.44	493.22	411.168	92.29	145.29	-544.192	415.87	74.80	91.80	12.829	02.91	39.64
N	M (tm)	0.000	0.6	46.69	o r. 8 4	49.15	55 47 41 70	-160.257		0.22	278.18	-222.061 -739.961	7.63	46.21	372.218	69.68	.6.042	000	9	15.00	365.31	-456.780	68.62	351.14	-512.057 -883 195	744.29	76.93	85.09	-762.992	400.03	77.09
	Case	0-13 0-13	1.1	-	C - 13 - 13 - 13	7	, , , , , , , , , , , , , , , , , , ,	,000,	•	7 7	Η.	0 -1 -1 -1 -1 -1 -1 -1 -1 -1 -1 -1 -1 -1	7	7	0-13 1-13 1-13	→	000	G-11	~{ 	-1	Ξ,	0112	- 1	~~i ·	C-12	4 pm	1	***	0-12	4 +-4	1
	N (t)	000.0	ທູດ ໝູດ ໝູດ	.8 85	3.00 3.00 3.00 3.00 3.00 3.00 3.00 3.00	8.43	80 a 4. d 8. d	88.430 88.430	•	 	5.19	85.199 85.199	0.24	0.24	20.242	0.24	0.0000	711.981		92.24	92.24	592.243	92.24	21.18	421.186	21.13	21.18	49.79	249.794	49.79	49.79
MAXIMUM	s (t)	0.000	32.20	39.19	-635,659 -653,822	78.04	03.19	-489.824	1	52.90 78.04	459.88	-414.965 -1129.798	68.98	894.40	876.238	99.60	15.104	80 t	5.65 C	72.48	363.43	292.043	38.43	007.78	CO U	107.49	0	069.43	-659.843	196.61	43.44
Z	M (tm)	0.000	63.60	59.89	-43.494	78.22	000	-164.154 -644.102	1	ος ος ος ος ος	262.55	-128,302	85.50	85.11	936.787	16.79	-6.042		. 07	29.58	29.77	55.317	99.55	91.53		30. 30. 30.	57.48	57.14	-176.512	30.40	227.37
	Case	0-14 0-14	m m	4 2	C-12 C-12			505		77	4	C-12 C-12	, ,	r	C-13	-1 +-1 } I	C - 1	C-13	⊢ i	7	Ļ	C-13	1 7	1	7,	77	3 C	7	C-13	7 7	1
	L (m)	0.000	000	20.	2.888 3.888	90.	96.6	2.888 3.888 850		000	92	2.888 3.850	00	.96	1.925	. 80	0.000	0.000	80	00.	96		S CO	00.	96.	00 0	3.850	00	0.963	2. α	
	No.	11	***		ന റു * I ന	დ 4		4.* 1 1000				ო ქ∗ I დ 4.	ري ب		* ÷		9 - 2	တ (t ထ (ı	9 - 10		* *	0 6 1 Or	10 - 01			11 10	11 - 12	*	* ;	m

PICK-UP No. 2

			:	N	MAXIMU	X		Z	MINIMU	X
Ñ	0	L (m)	Case	M (tm)	s (t)	N <t< th=""><th>Case</th><th>M (tm)</th><th>s (t)</th><th>N (£)</th></t<>	Case	M (tm)	s (t)	N (£)
12 - 1	ო	0	7	252.58	! ~	112.750	C-12	471.076	-1364.861	-20.242
*	Fri	.963	7	-559.16	611.95	12.750		528.57	708.00	20.24
	. (1)	92	61	-921.86	140.01	12.75	7	86.24	32.37	20.24
*		888	E [-	824.18	44.98	12,750	-	583,96	63.42	20.24
Н	8	3.850	C-13	-254.231	2:03	12.750	7	96.38	77.95	20.24
۳. ا ش		00	, , , , ,	.21	2.96		C-11	189.216	-472.963	000.0
14 - 1	(13)	0.800	C-11	000.0	0	8	بر ا	8	0.07	00
•	(. 6	7	4 0 0	31.00	902 41	-	79.15	69.48	1278.19
ı ;		, 9 C	-, - (0.0	200	428.84	•	7.55	69.48	1304.62
e 3	٦,		بم ہے ا	7.00	14.44	955.28	-	35.96	69.48	1331.05
+ ·X	a es) C	1 7	3.27	17.09	981	C-14	-85.630	-69.482	-1357.490
e i	o ~	7.000	C-13	460.505	119.738	08.14	C-14	07.22	69.48	1383.92
(*) (*)		00	- 7	568.39	60.48	540.49	~~	0 59	52.34	2177.41
*		7 5	1	85.24	63.12	1566.92	.~	98.99	52.34	2203.84
· *	. 0	50	; , ,	2.54	65.76	1593.36	ř	7,39	52.34	2230.27
*	3 62	2	4 ≓ 1	6	168.413	-1619.793	C-11	-84.203	-52.343	-2256.711
10 -	· 62	8	C-13	1.98	71.05	1646.22	mi	5.80	52.34	2283.14
	,-	8	7	563.05	60.81	1467.81	7	. 22	0.05	2630.06
*			1 7	0.0	63.45	1494.25	~		0.05	2656.50
÷ -9	٠, ٥		7	40.0	56.10	1520.68	H	0.5	0.05	2682.93
÷ *	ı co	, c	4	02.04	68.74		G-11	-0.039	-0.051	-2709.364
+ 1 ++1 ++1) ''	7.000	0-13	599.663	171.391	1573.54	r~(0.12	0.05	2735.79
		00	Ţ	442.32	26.46	1609.53	7	48.67	64.95	2698.78
7		ָ יע י	5	9	29.11	1635.96	7	5.00	64.95	2725.21
+ 4	۰,		ς,	1 C	31, 75	1662.39	7	21.32	64.95	2751.64
ę -y	1 C) (C	1 7	42.44	34.40	688.82		-92.348	-64.957	-2778.076
12 -	സ	7.000	C-13	479.958	137.044	1715	C-12	6.02	64.95	2804.50
, (eri T	00	7	399.04	02.17	1070.72	***t	55.30	6,49	1892.06
-)) L	٠ <u>-</u>	17.93	04.81	1097.15	77	56.67	6.49	1918,49
÷ ×	٦ ،	- u	- 1	325	107.462	-1123.591	C-15	-78.051	-6.498	-1944.929
*	1 63	25.5	۱ ۲۲ ا	58.18	10.10	1150.02	Ħ.	89.42	6.49	1971.36
ဗ	φ	7.000		53.18	12.75	1176.45	7	00.79	6.49	1997.79

MOMBASA - PIER

Longitudinal derection

Calculation of pillar

action force for bottom of pillar (S.L.S)

State	load	Ики	Нки	У ^m	$M = H \cdot y^{KNm}$
Super	Rd	6270.0		.	<u>—</u>
structure	R &	2929.2	j.e .		·
Braking		_	514.0	7.100	3649.4
Seismic		h	822.0	7.100	5836.2
beam		$\begin{array}{c} 23.6 \times 1.00 \times 0.80 \\ \times 17.00 = 321.0 \end{array}$	32.1	6.500	208.7
Pillar		23.6 x 0.80 x 6.00 x 5 = 453.3	45.4	3.000	136.2
Braking		9973.5	514.0		3649.4
Seismic		7044.3	899.5		6181.1

action force for bottom of pillar (U.L.S)

load State	N _{KN}	Нки	Мкиш
Braking	$ 9973.5 \times 1.2 \times 1.15 \\ = 13763.5 $	$514.0 \times 1.25 \times 1.1$ = 706.8	$3649.4 \times 1.25 \times 1.1$ = 5018.0
Seismic	$7044.3 \times 1.2 \times 1.15 \\ = 9721.2$	$899.5 \times 1.5 \times 1.1$ = 1484.1	$6181.1 \times 1.5 \times 1.1 \\ = 10198.8$

Calculation of stress for U.L.S.

for action force of one pillar

$$N = 9721.2/5 = 1944.3$$
 KN

$$H = 1484.1/5 = 296.9$$
 KN

$$M = 10198.8/5 = 2039.8$$
 KNm

Notice, this case is abridge for small action force or near than the pillar of Uhulu bridge and the Bar arrengement is arrange following below.

$$A_{S} = A_{S}' = Y_{32} - 6^{NO} \times 2$$

Calculation of stability for Foundation

- 1) action force for bottom of foundation
- (1) Longitudinal direction for S.L.S

State	load	Ики	Нки	y ^m	М = H • у ^{кыт}
Super	Rd	6270.0		-	
structure	R @	2929.2		· 	
Braking			514.0	8.100	4163.4
Seismic			822.0	8.100	6658.2
beam		$\begin{array}{c} 23.6 \times 1.00 \times 0.80 \\ \times 17.00 = 321.0 \end{array}$	32.1	7.500	240.8
Pillar		$\begin{array}{c} 23.6 \times 0.80^{2} \times 6.00 \\ \times 5 = 453.3 \end{array}$	45.4	4.000	181.6
footing	<u> </u>	23.6×17.00 ×4.00 ×1.00=1604.8	160.5	0.500	80.4
surchar	·ge	$ \begin{array}{c} 18.6 \times 17.00 \times 4.00 \\ \times 1.00 = 1264.8 \end{array} $			
Braking	7	12843.1	514.0	Lames a	4163.4
Seismic)	9913.9	1060.0		7161.0

(2) Longitudinal direction for U.L.S

	load State	Nĸn	Нки	MKNW
	Braking	$ \begin{array}{c} 12843.1 \times 1.2 \\ \times 1.15 = 17723.5 \end{array} $	$514.0 \times 1.25 \times 1.1$ = 706.8	$\begin{array}{r} 4163.4 \times 1.25 \times 1.1 \\ = 5724.7 \end{array}$
-	Seismic	9913.9×1.2 $\times 1.15 = 13681.2$	$ \begin{array}{c} 1060.0 \times 1.5 \times 1.1 \\ = 1749.0 \end{array} $	$7161.0 \times 1.5 \times 1.1 = 11815.7$

- 2) Stability of Foundation for Longitudinal direction.
 - (1) For S.L.S
 - (a) Braking state

$$e = \frac{M}{N} = \frac{4163.4}{12843.1} = 0.325^{m} < \frac{B}{6} = \frac{4.00}{6} = 0.667^{m}$$

$$q = \frac{N}{B \cdot L} (1 \pm \frac{6e}{B}) = \frac{12843.1}{4.00 \times 17.00} = (1 \pm \frac{6 \times 0.325}{4.00}) = \begin{pmatrix} 281.0 \text{KN/m}^{2} \\ 96.8 \text{KN/m}^{2} \end{pmatrix}$$

$$Fs = \frac{N \cdot \mu}{H} = \frac{12843.1 \times 0.50}{514.0} = 12.5 > 1.5$$
 OK

(b) Seismic state

$$e = \frac{M}{N} = \frac{7161.0}{9913.9} = 0.723^{cm} > \frac{B}{6} = 0.667^{m}$$

$$\chi = \frac{B}{2} - e = \frac{4.00}{2} - 0.723 = 1.277^{m}$$

$$q_{max} = \frac{2 \cdot N}{3 \cdot \chi \cdot L} = \frac{2 \times 9913.9}{3 \times 1.277 \times 17.00} = 304.5 \text{ KN/m}^{2} < qa=350 \text{ KN/m}^{2}$$

$$Fs = \frac{N \cdot \mu}{H} = \frac{9913.9 \times 0.50}{1060.0} = 4.6 > 1.5 \text{ OK}$$

- (2) For U.L.S
 - a) Braking state

$$\begin{split} \mathrm{e} = & \frac{\mathrm{M}}{\mathrm{N}} = \frac{5724.7}{17723.5} = 0.323^{\mathrm{m}} < \frac{\mathrm{B}}{\mathrm{6}} = 0.667^{\mathrm{m}} \\ \mathrm{q} = & \frac{\mathrm{N}}{\mathrm{B \cdot L}} \cdot (1 \pm \frac{\mathrm{6e}}{\mathrm{B}}) = \frac{17723.5}{4.00 \times 17.00} = (1 \pm \frac{6 \times 0.323}{4.00}) = \left(\frac{387.0 \ \mathrm{KN/m^2}}{134.4 \ \mathrm{KN/m^2}} < \mathrm{qa=525.0KN/m^2} \right) \\ \mathrm{Fs} = & \frac{\mathrm{N} \cdot \mu}{\mathrm{H}} = \frac{17723.5 \times 0.50}{706.8} = 12.5 > 1.1 \end{split}$$

b) Seismic state

$$e = \frac{M}{N} = \frac{11815.7}{13681.2} = 0.864^{cm} > \frac{B}{6} = 0.667^{m}$$

$$\chi = \frac{B}{2} - e = \frac{4.00}{2} - 0.864 = 1.136^{m}$$

$$q_{max} = \frac{2 \cdot N}{3 \cdot \chi \cdot L} = \frac{2 \times 13681.2}{3 \times 1.136 \times 17.00} = 472.3 \text{ KN/m}^{2} < qa=525.0 \text{ KN/m}^{2}$$

$$Fs = \frac{N \cdot \mu}{H} = \frac{13681.2 \times 0.50}{1749.0} = 3.9 > 1.1 \text{ OK}$$

Notice: this case is follow the footing of the pillar of Uhuru bridge.

$$As = Y_{20} - 150^{\text{ctc}}$$

MOMBASA - PIER

Crossing direction

- 1) Calculation for Beam
- a) middle span ... U.L.S

Mu.max =
$$1422.6^{\text{KNm}}$$

Su.max = 1322.5^{KN} from out put of Computer

section b =
$$80^{\text{cm}}$$
 h = 100 d = 86.5 d' = 13.5
As = $Y_{25} - 6^{\text{NO}} \times 2$ = $4.909 \times 12^{\text{NO}} = 58.90$ cm²
P = $\frac{\text{As}}{\text{b}} \times 100$ = $\frac{58.90}{80 \times 86.5} \times 100$ = 0.851%
 $\chi = \frac{0.87 \text{ fy As}}{0.40 \text{ fcu b}} = \frac{0.87 \times 41000 \times 58.90}{0.40 \times 2500 \times 80}$ = 26.4^{cm}
Z = $d - \frac{\chi}{2} = 86.5 - \frac{26.4}{2} = 73.3^{\text{cm}} < 0.95 d = 0.95 \times 86.5 = 82.2^{\text{cm}}$ OK
M_{RS} = $0.87 \text{ fy As} \cdot \text{Z}$ = $0.87 \times 41000 \times 58.90 \times 73.3 \times 10^{-5}$ = $1540.0^{\text{KNm}} > \text{Mu} = 1422.6^{\text{KNm}}$
M_{RC} = $0.40 \text{ fcubx} \cdot \text{Z}$ = $0.40 \times 2500 \times 80 \times 26.4 \times 73.3 \times 10^{-5}$ = $1548.1^{\text{KNm}} > \text{Mu} = 1422.6^{\text{KNm}}$ OK
V c = $\frac{\text{Su}}{\text{bd}} = \frac{1322.5 \times 10^3}{80 \times 86.5} = 191.1 \text{ N/cm}^2$
V ca = $50.0 + 15.0 \text{ (}\frac{0.851 - 0.50}{0.500}\text{)}$ = 60.6 N/cm^2

shering bar

$$Asv = \frac{b (Vc - Vca) \cdot Sv}{0.87 fyv}$$

$$= \frac{80 (191.1 - 60.6) \times 15.0}{0.87 \times 41000} = 4.0 cm^{2}$$

$$\Rightarrow Asu = Y_{16} - 2^{NO} = 4.0 cm^{2}$$
 OK

b) middle fulcrum

$$Mu.mix = -1172.6^{KNm} < Mu.max = 1422.6^{KNm}$$

Notice: this case is abridge.

MOMBASA — PIER

Crossing direction

2) Calculation for Pillar - U.L.S.

section
$$b = 80^{cm} h = 80 d = 72.5 d' = 7.5$$

$$As = As' = Y_{32} - 10^{NO} = 8.042 \times 10^{NO} = 80.42 \text{ cm}^2$$

Ma = M + N (
$$d - \frac{h}{2}$$
) = 989.5 + 2171.5 (72.5 $-\frac{80}{2}$) $\times 10^{-2} = 1695.3^{\text{KN} \cdot \text{m}}$

$$\chi = \frac{(0.87 - 0.72) \text{ fy As}}{0.40 \text{ fcu b}} = \frac{(0.87 - 0.72) \times 41000 \times 80.42}{0.40 \times 2500 \times 80} = 7.2^{\text{cm}}$$

$$Z = d - \frac{\chi}{2} = 72.5 - \frac{7.2}{2} = 68.9^{cm} \le 0.95d = 0.95 \times 72.5 = 68.9^{cm}$$
 OK

$$M_{RS} = 0.87 \text{ fyAsZ} = 0.87 \times 41000 \times 80.42 \times 68.9 \times 10^{-5}$$

= 1976.4 KNm > Ma = 1695.3 KNm

$$M_{RC} = \{0.72 \text{ fyAs'} (d-d') + 0.40 \text{ fcubxZ} \}$$

= $(0.72 \times 41000 \times 80.42 \times 65.0)$

$$+0.40 \times 2500 \times 80 \times 7.2 \times 68.9$$
) $\times 10^{-5} = 1940.0^{\text{KNm}} > \text{Ma}$ OK

Asn = Asn' = As
$$-\frac{N}{0.87 \text{fy}}$$
 = 80.42 $-\frac{2171.5 \times 10^3}{0.87 \times 41000}$ = 19.6 cm²

$$<$$
 Asu = Asu' = Y₃₂-6^{NO} = 8.042×6^{NO} = 48.25 cm²

$$P = \frac{A su}{bd} \times 100 = \frac{48.25}{80 \times 72.5} \times 100 = 0.832 \%$$

$$V_{c} = \frac{N}{bd} = \frac{282.8 \times 10^{3}}{80 \times 72.5} = 48.8 \text{ N/cm}^{2}$$

$$< V ca = 50.0 + 15.0 \quad (\frac{0.832 - 0.50}{0.50}) = 60.0 \text{ N/cm}^2 \text{ OK}$$

- 3) Calculation for footing -U.L.S.
- a) middle fulcrum

b) middle span

Mu.min =
$$-1385.2^{\text{KNm}}$$
 ... from output of Computer section b= $80 \pm 94.5 = 174.5$ cm h= 100^{cm} d= 94.5 d'= 5.5

As = $Y_{25} - 150^{\text{ctc}} (12^{\text{NO}}) = 4.909 \times 12^{\text{NO}} = 58.91$ cm²

$$\chi = \frac{0.87 \times 41000 \times 58.91}{0.40 \times 2500 \times 174.5} = 12.2^{\text{cm}}$$

Z = $94.5 - \frac{12.2}{2} = 88.5^{\text{cm}} < 0.95 \times 94.5 = 89.8^{\text{cm}}$ OK

M_{RS} = $0.87 \times 41000 \times 58.91 \times 88.5 \times 10^{-6}$ = $1859.6^{\text{KNm}} > \text{Mu} = 1385.2^{\text{KNm}}$

M_{RC} = $0.40 \times 2500 \times 174.5 \times 12.2 \times 88.5 \times 10^{-5}$ = $1884.0^{\text{KNm}} > \text{Mu} = 1385.2^{\text{KNm}}$ OK

c) middle span ... S.L.S (for check)

 $Ms.max = 936.8^{KNm}$

$$x = \frac{0.80 \text{ fy As}}{\frac{1}{2} \times 0.5 \text{ fcub}} = \frac{0.80 \times 41000 \times 58.90}{\frac{1}{2} \times 0.50 \times 2500 \times 80} = 38.7^{\text{cm}}$$

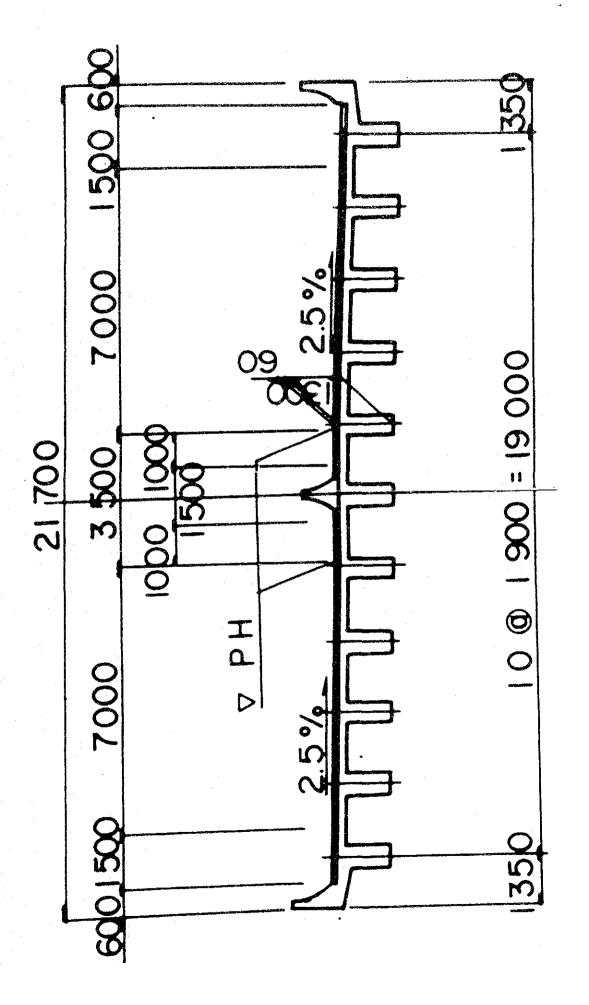
$$Z = d - \frac{\chi}{3} = 86.5 - \frac{38.7}{3} = 73.6^{\text{cm}} < 0.95 \times 86.5 = 82.2^{\text{cm}}$$

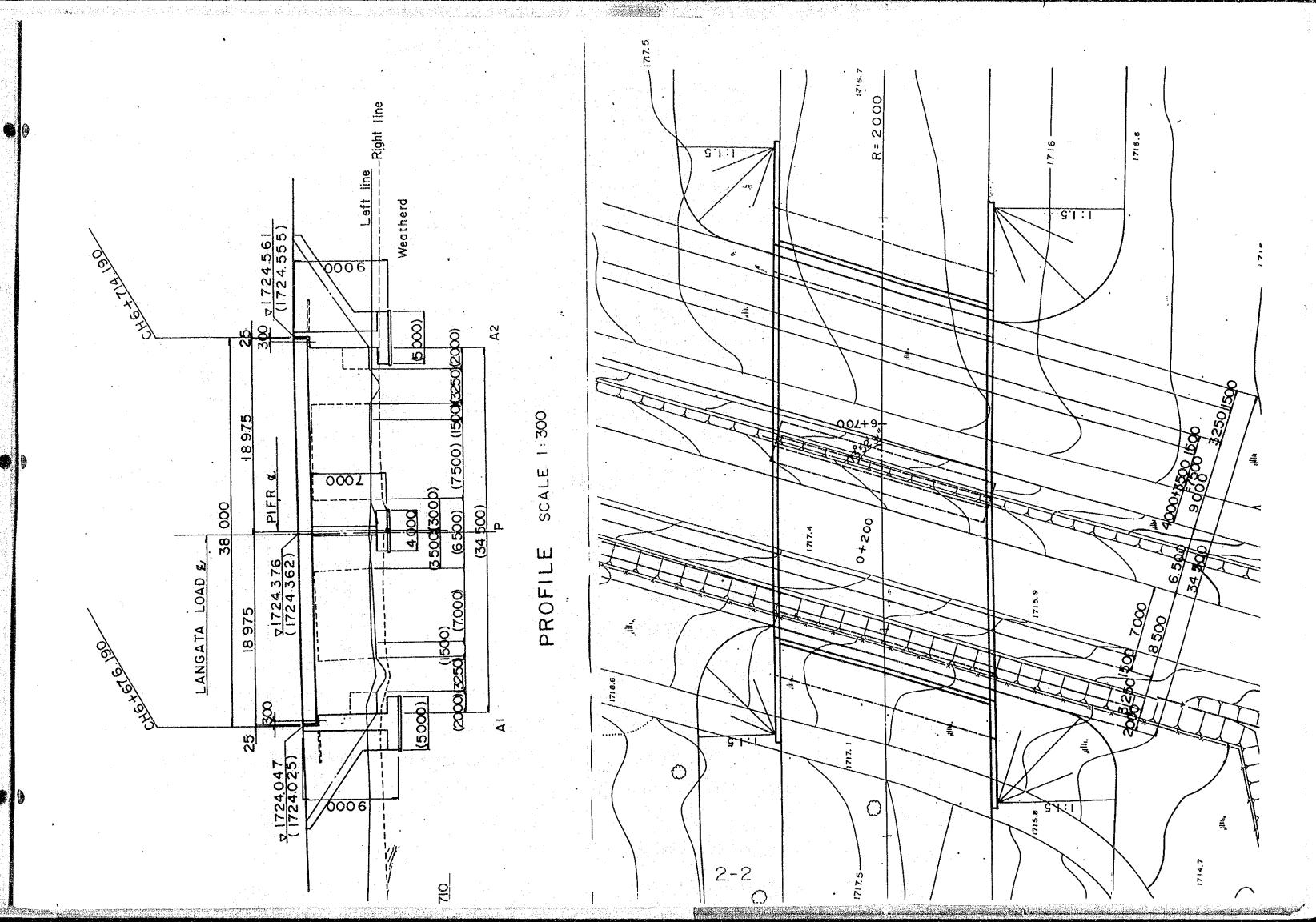
$$M_{RS} = 0.80 \text{ fyAsZ} = 0.80 \times 41000 \times 58.90 \times 73.6 \times 10^{-5}$$

$$=1421.9^{KNm} > Ms = 936.8^{KNm}$$

$$M_{RC} = \frac{1}{2} \times 0.50 \text{ fcubx} Z = \frac{1}{2} \times 0.50 \times 2500 \times 80 \times 38.7 \times 73.6 \times 10^{-5}$$

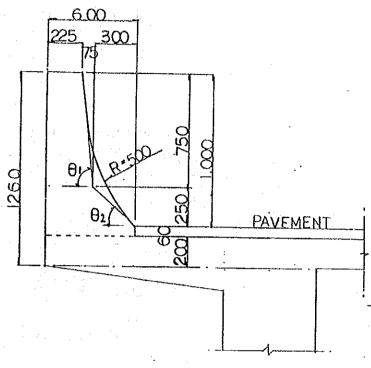
= 1424. 1^{KNm} > Mu = 936.8^{KNm} 0K





SHAPE OF PARAPET OF MAIN ROAD

I) FOR VERGE



Section arer

A =
$$0.60 \times 1.06 - \frac{1}{2} \times 0.075 \times 0.75$$

- $\frac{0.75 + 1.00}{2} \times 0.30 + 0.006$
= 0.352 m^2

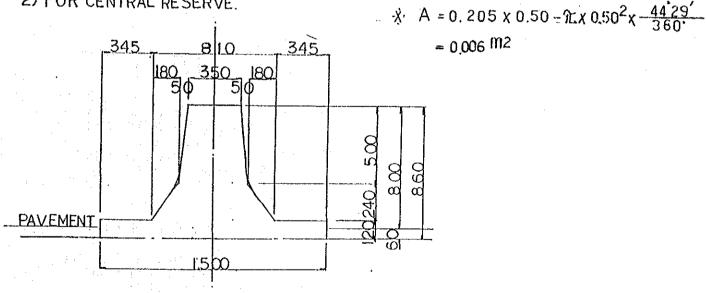
$$\theta_{1} = \tan^{1} \frac{0.750}{0.075} = 84^{\circ} 17'22''$$

$$- \left| \theta_{2} = \tan^{1} \frac{0.250}{0.300} = 39^{\circ} 48'20'' \right|$$

$$\theta = \frac{44^{\circ} 29'}{2} = 10.205^{\circ}$$

$$TL = R \cdot \tan \frac{\theta}{2} = 0.205^{\circ}$$

2) FOR CENTRAL RESERVE.



Section arer

$$A = \frac{0.35 + 0.45}{2} \times 0.50 + \frac{0.45 + 0.81}{2} \times 0.24 + 1.50 \times 0.12$$
$$= 0.532 \,\text{m}^2$$

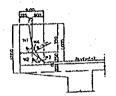
2. LOAD

2. 1 DEAD LOAD

Note Input data: unit=P', W' t, t/m

unit=P. W KN=P', W'*9.8m/s2

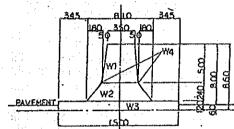
1) Parapet for verge



W1'=1/2*(0.225+0.30)*0.75*2.4=0.473t/m
W2'=0.3*0.31*2.4 =0.223
W3'=1/2*(0.06+0.31)*0.30*2.4 =0.133
W4'=0.006*2.4 =0.014
\(\Sigma W' = 0.843t/m\)

W=0.843*9.8=8.26KN/m

2) Parapet for central reserve



W1'=1/2*(0.350+0.45)*0.50*2.4=0.480t/m W2'=1/2*(0.45+0.81)*0.24*2.4=0.363 W3'=1.5*0.12*2.4 =0.432 W4'=0.0007*2*2.4 =0.003 \(\Sigma W'=1.278t/m\)

W=1.278*9.8=12.52KN/m

3) Payment

W=0.06*2.3*9.8=1.35KN/m2

W' = 0.06 * 2.3 = 0.138 t/m2

4) Main girder

end girder
W1'=2.3*0.20*2.4 =1.104t/m
W2'=1/2*1.05*0.15*2.4 =0.189
W3'=0.6*1.10*2.4 =1.584
\(\Sigmu W=2.877*9.8=28.19KN/m\)

Middle gider W=(1.9*0.2+1.1*0.6)*2.4*9.8=24.46KN/m % W'=24.46/9.8=2.496t/m

6) Cross girder

2. 2 LIVE LOAD

1) H A I (center of span)

KEL main P1=38.10KN/m sub P2=12.70KN/m

※ P1'=38.10/9.8=3.887t/m P2'=12.70/9.8=1.296t/m

UDL main W1=9.52KN/m2 sub W2=3.17KN/m2

% W1'=9.52/9.8=0.972t/m2 W2'=3.17/9.8=0.324t/m2

2) H A 2 (middle supprting point)

KEL main P1=38.10KN/m sub P2=12.70KN/m

※ P1'=38.10/9.8=3.887t/m P2'=12.70/9.8=1.296t/m

UDL main W1=8.51KN/m2 sub W2=2.84KN/m2

3% W1'=8.51/9.8=0.868t/m2 W2'=2.84/9.8=0.289t/m2

L=18.975+18.975=37.95m

 $W=151*(1/L)^0.475=151*(1/37.95)^0.475=26.8KN/m$

W=26.8/3.15m=8.51KN/m2

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3) H B 1 (center of span)
   sub P2=12.70KN/m

※ P1'=38.10/9.8=3.887t/m P2'=12.70/9.8=1.296t/m

   UDL main W1=10KN/m2 sub W2=3.33KN/m2

₩ 1'=10/9.8=1.02t/m2  ₩2'=3.33/9.8=0.340t/m2

3) H B 2 (middle supprting point)
                  ※ PO'=75/9.8=7.653t/m
   HB P0=75KN
   KEL main P1=38.1KN/m
                           sub P2=12.70KN/m
         ※ P1'=38.10/9.8=3.887t/m
                                  P2'=12.70/9.8=1.296t/m
   UDL main W1=8.51KN/m2
                           sub W2=2.84KN/m2
         ₩ W1'=8.51/9.8=0.868t/m2
                                   W2' = 2.84/9.8 = 0.289 t/m2
    COMBINATION OF
3
                               LOADS
1) Base Loads
       Dead loads1 (deck loads)
Dead loads2 (own weight)
      HA1E (HA1 for side spans)
       HA2
           (cosentration loads span L=6m)
       HB
           (cosentration loads span L=11m)
      HB
      HB
           (cosentration loads span L=16m)
   <u>®</u>
      B
           (cosentration loads span L=21m)
   <u>(</u>
       HB
           (cosentration loads span L=26m)
       HB1 (HB1 distributed loads for side spans)
       HB2 (HB1 distributed loads)
2) Combination
                         Loads
       HB1
            (5)+(10)
       HB1
            6+0
       HB1
            (7)+(0)
   9999
       HB1
            (8)+(0)
       HB1
            9+0
       HB2
            (5)+(1)
   (8)
       HB2
            (6)+(1)
   Ϣ
       HB2
            ⑦+①
       HB2
            ®+0
       HB2
            9+0
 3) Pick up
                     Cases
         12, 13, 19, 15, 16
   (1)
   (2)
         (f), (g),
(g), (1)
                 (19), (20), 21
   (3)
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(4)

3. EFFECTIVE WIDTH AND MODULAS

3. 1 EFFECTIVE WIDTH

2) Main girder

be=bw+1/5

be: effective width for flanges

l : length of moment zero

0.7*Ls or 0.85*Ls'

is: spans for connection girders of middle spans

Ls: spans for connection girders of end spans

end spans

be=0.60+0.85*18.925/5=3.817m > b=1.90m(2.30m)

2) Cross girder

end cross girder

λ 1=1/8+bs=18. 925±0. 8/8+0=1. 893m

spporting cross girder

 $\lambda 2=1/8+bs=(18.925+2)*0.2/8+0=0.946m$

middle cross girder

 $\lambda 3 = (n-1)*(1b+1w)+bs=(11-1)/6*(1.9+0.60)+0=4.167m$

3.2 MODULAS UHURU

Main girder							
	В	H	A	Y	A*Y	A*y^2	.1c
End girder①	230	20	4600	10	46000	460000	153333
middle gir①'	190	20	3800	10	38000	380000	126667
2	60	110	8600	75	495000	37125000	6655000
End girderΣ			11200		541000	37585000	6808333
middle girΣ			10400		533000	37505000	6781667

 $IY = \Sigma I c + \Sigma A * Y^2 - \Sigma A * (\Sigma A * Y / \Sigma A)^2$ = 18261101 = 0.183

 $1Y=\Sigma \ I \ c+\Sigma \ A*Y^2-\Sigma \ A* (\Sigma \ A*Y/\Sigma \ A)^2$ = 16970417 = 0.170

Cross girder		•					
	В	H	A	Y	A*Y	A*Y^2	lc
End cross ①	249.3	20	4986	10	49860	498600	166200
Sopprting ①'	249. 2	20	4984	10	49840	498400	166133
2	60	95	5700	67.5	384750	25970625	4286875
End cross Σ			10686		434610	26469225	4453075
Sopprting Σ			10684		434590	26469025	4453008

IY= Σ I c+ Σ A *Y^2- Σ A * (Σ A *Y/ Σ A) ^2 = 13246289 = 0.132

 $IY = \Sigma I c + \Sigma A * Y^2 - \Sigma A * (\Sigma A * Y / \Sigma A)^2$ = 13244341 = 0.132

Cross girder			-		•		
	В	H	A	Y	A*Y	A*Y^2	Īс
Middle cro①	868.4	20	17368	10	173680	1736800	578933
2	35	95	3325	67.5	224438	15149531	2500677
Middle $cro\Sigma$			20693		398118	16886331	3079610

 $IY=\Sigma I c+\Sigma A*Y^2-\Sigma A* (\Sigma A*Y/\Sigma A)^2$ = 12306465 = 0.123

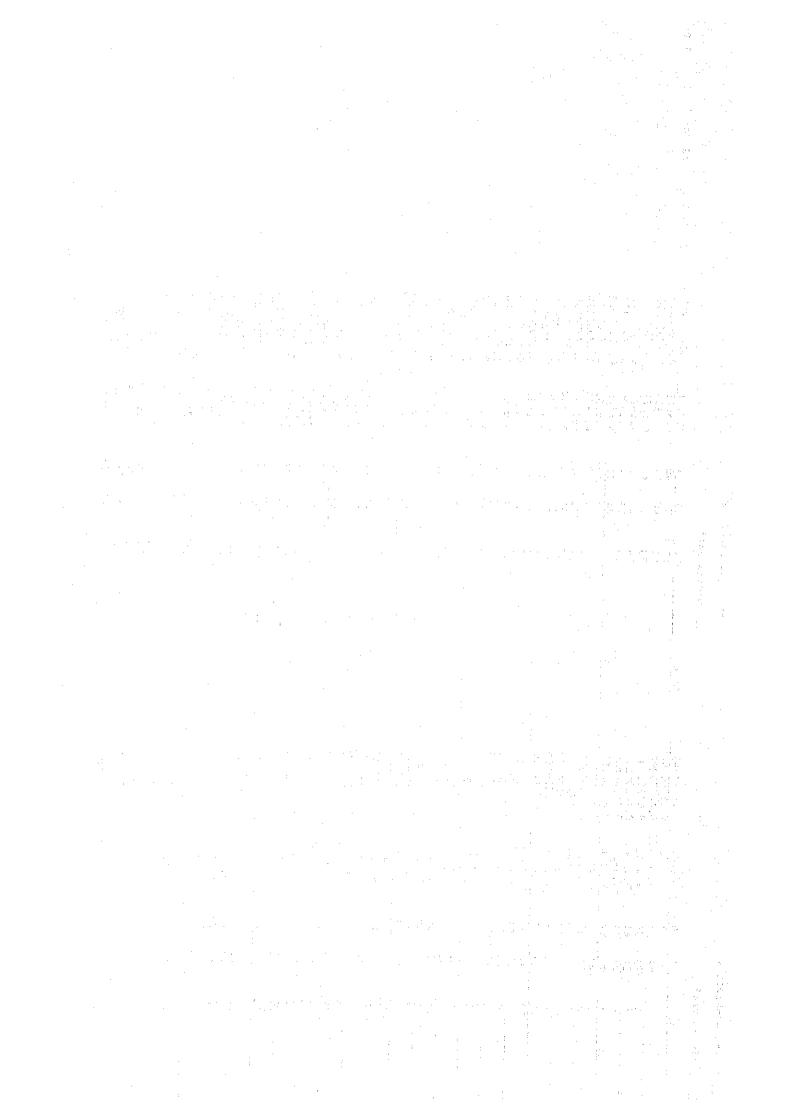
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NUMBER OF MEMBERS	140			
NUMBER OF MAIN GIRDERS	4-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1			
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SHEAR MODULUS	1.2176+06			
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(M+4)	0.00000000 37.285 18.642 18.643	0.00000000 37.288 18.644 18.644	0,00000000 37,291 18,645 18,646	0.0000000 37.294 18.647	0.00000000 37.297 18.648 18.649	
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** UHURU SKIDGE



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ALL UNITS ARE METER AND TON.

NO. OF POINTS
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NO. OF MAIN GIRDER
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SKEW ANGLE MATERIAL

** UNURD BRIDGE **

LOAD POINT DIAGRAM

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** UHURU BRIDGE **

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* LOAD DATA

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PAGE 18	JIP-GRIDD					** UHURU BRIDGE **

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		M-Y(T.M)	20,920	58,140	7,867	70.424	-28,826	84,432	67.203	-19,221	63.785	-18,786	-21,719	63.785	-23.146	11.472	75,123	-35,605	27.854 27.854	54.477	-28,095	24.477	-31,027	757.05-	95,903	-14.826	78.674	75.257	-7.315	75,257	-10-247	-11.674				
	WE UHURU BRIDGE **	NO. LOAD NAME			HAT	(+))	(+) (9 ≡ T) S	(+) (+) (+) (+)	· ·	(+) (F=19) L	(#) (*C#!) o	:	(L=26) (+)		- N 1 1 N 1 N 1 N 1 N 1 N 1 N 1 N 1 N 1	2 T(L= 6) + HB1 (13 T(L=11) + HB1 (+)	14 T(L=16) + HB1 (+)		15 T(L=21) + HB1 (+)		., ., - D	17 T(L= 6) + HB2 (+)	`	18 T(L=11) + HB2 (+)	(+) [2H + 171 H + 182]	(+)	20 T (L=21) + HB2 (+)) (E)	(-)				

PAGE 49	7.8) 8-2(T) 25 14 - 25	725	0.00	206	137	504	227 7,218	112	244 -2.98	നെ	395	362 -2.90	395	707	746 1.85	9,78	60,	7.47	7.47	178 -0.29	7.97	27.04 27.04 27.04	178 -0.29	573 9.04	15	100	541 6.72	788	7 22	17.	384 -1.04			
JIP-GRIDD	14 - 3 M-YC1	1.121	242	1,920	.052	1.294	3,543	2.980	756	2.905	2,905	.368	2,905	107.4	859	0,938	.789	0.375	1007	336	2,299	2000	336	1.684 6	,043	1,171 6,615	270	. 590	1,045	027	0000			
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** UHURU BRIDGE **	NO. LOAD NAME 50		7 187		(+) 2×4 7	(40)	(+) (9 =1, 5	. ~		7 (L=16) (+)					10 xxx	T(L= 6) + HB1 (+)	(-)	~		(-)		(+) ton + (*C+1/**)			(1)		19 T(L=16) + HB2 (+)	(*)	0.0	(+) Can + (7C=17+ 7	(1)			

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** USOING DAUXU 4*			•		ATT	-GRIDD PAGE	50	
NO. LOAD NAME	M-YCT_M) 25 - 14	0-2(T)	M-Y(T,M)	2-2(T)	M-Y(T+M)	36 - 25	M-Y(T.M)	
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15 T(L=21) + HB1 (+)	65.302	1	200	2 6 2 5	7	-	. ⁴.	
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16 T(L=26) + HB1 (+)	65.302	-7	5,22	8 39	7.7	74	4,	
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17 1(L= 0) + MB2 (+)	1 0 × 0 × 1	♪ て	0 4	- r	26.	9 4	3 14	
19 T(1=11) + HB2 (+)	77. X5	· ~		75	, (-)	7	. 10	}
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19 T(L=16) + HB2 (+)	58.347	٥,	8,28	6.40		32	Ψ-	
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