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#### TOTAL MOMENT

POIN	T	G1 ③		<u>U.</u>	L. S		S.	L.S
		MOMENT		_	MOMENT OF DESIGN			MOMENT OF DESIGN
		M (kNm)	<u>7 fl  </u>	73	$Mu=M\gamma fL\gamma 3$ (kNm)	γfl	73	<u>Mu=MγfLγ3 (kNm)</u>
	AD							
L0	AD	717.9	1.20	1.15	990.7	1.00	1.00	717.9
LIVE	HA		1.50	1.10	0.0	1.20	1.00	0.0
LOAD	HA HB	1303.4	1.30	1.10	1863.9	1.10	1.00	1433.7
					990.7			117.9
TOT	AL	<u></u>			2854.6	<u> </u>		2151.6
DE	SIGN 1	KOMENT			2854.6			2151.6
REST	STANCI	E MOMENT			5012.7			2504. (

#### TOTAL MOMENT

POIN	T	G1 (5)		υ.	L. S		S.	L. S
		MOMENT			MOMENT OF DESIGN			MOMENT OF DESIGN
· · · ·		M (kNm)	_γ fL	<u> </u>	$Mu=M\gamma f L\gamma 3$ (kNm)	<u>γ fl</u>	<u> </u>	<u>Mu=Mγflγ3 (kNm)</u>
DE LO		-1549.9	1. 20	1, 15	-2138.9	1.00	1.00	-1549.9
LIVE	HA		1.50	1.10	0.0	1.20	1.00	0.0
LOAD	HA HB	-1483.7	1.30	1.10	-2121.7	1.10	1.00	-1632.1
<u>_</u>					-2138.9			1549.9
TOT	AL				-4260.6			-3182.0
DE	SIGN I	HOMENT			-4260. 6		······································	-3182.0
RESI	STANCI	E MOMENT			-5223.0	<u></u>		-3307.8

TOTAL MOMENT

POIN	Ť	G 2 (5)		υ.	L. S		<u>s.</u>	L. S
1010	•	MOMENT M (kNm)	γ fL	73	MOMENT OF DESIGN Mu=M7fl73 (kNm)	<u>γ fl</u>	73	MOMENT OF DESIGN Mu=M y fl y 3 (kNm)
DE LO		-1230.7	1. 20	1.15	-1698.4	1.00	1.00	-1230. '
LIVE	HA	-997 <u>.</u> 2	1.50	1.10	-1645.4_	1. 20	1.00	-1196. (
LOAD	HA HB	-1267.1	1.30	1,10	-1812.0	1.10	1.00	-1393, 1
TOT	Al.				-3343.7 -3510.3			-2427. -2624.
		10MENT			-3510.3			-2624.
1239	STANC	S MOMENT			-5223.0			-2870.

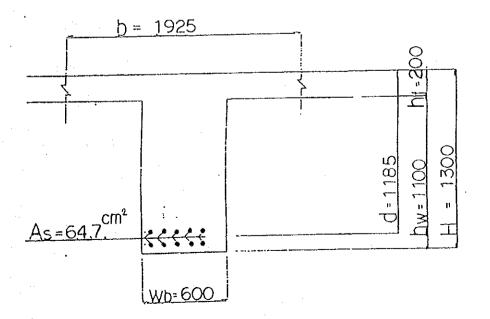
U.FLG b=	192.5 A S=	64.7
U.FLG hf=	20 fcu=	3000
WEB hw=	110 fy=	41000
d=	118.5	

#### Z=d-1/2*hf = 108.5

MRC=0.4*fcu*b*hf*Z= 5012.7

#### MRS=0.87*fy*AS*Z = 2504.0

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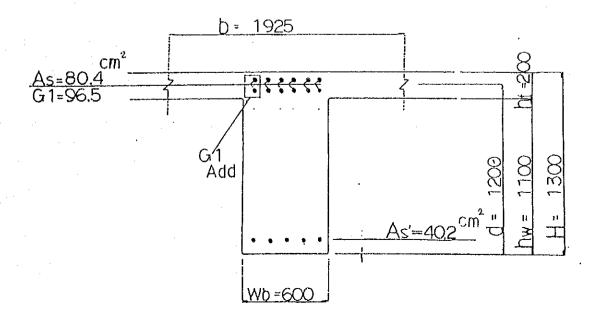
G1

U.FLG b= U.FLG hf= WEB hw= WEB Wb=	192.5 A S= 20 fcu= 120 fy=	96.5 3000 41000
d=	60 AS'= 120 d'= S/(0.4*fcu*Wb)	40.2 7.5 47.8
Z=d-1/2*X		96.1
MRC=0.15*fc	u*b*d^2+0.72*fy	* A S' * (d-d) 5223. 0
MRS=0. 87*fy	* A S*2 =	3307.8

G2

U.FLG b=	192.5 AS=	80.4
U.FLG hf=	20 fcu=	3000
WEB hw=	120 fy=	41000
WEB Wb=	60 AS'=	40.2
d=	120 d'=	7.5
X=0.87*fy*A	S∕(O.4*fcu*₩b)	39.8
Z=d-1/2¥X	· =	100.1
MRC=0.15*fc	u*b*d^2+0. 72*fy*AS	`*(d-d)

MRS=0. 87*fy*AS*Z = 2870.3



Calculation of deck slab for Main bridge (U.L.S)

1. Span and bending moment

a) Span  $\ell = 1.925 - 0.600 = 1.325 \Rightarrow 1.400^{m}$ 

b) moment of middle span

$$M \stackrel{\text{\tiny def}}{=} \left\{ \begin{array}{l} 0.80 \left( \begin{array}{c} 0.12 \,\ell + 0.07 \right) & P + \frac{1}{10} \,\omega \,d \,\,\ell^2 \end{array} \right\} \times 1.5 \times 1.1 \\ = \left\{ \begin{array}{c} 0.8 \left( 0.12 \times 1.40 + 0.07 \right) & \times 100 + \frac{1}{10} \times 4.72 \times 1.40^2 \end{array} \right\} \times 1.5 \times 1.1 = 33.0^{\text{KNM}/\text{m}} \end{array} \right\}$$

c) moment of each fulcrum

$$M \doteq \left\{ \left( \begin{array}{c} 0.15 \,\ell + 0.125 \right) P + \frac{1}{10} \omega d \ell^2 \right\} \times 1.5 \times 1.1 \\ = \left\{ \left( 0.15 \times 1.40 + 0.125 \right) \times 100 + \frac{1}{10} \times 4.72 \times 1.40^2 \right\} \times 1.5 \times 1.1 = 56.8^{K \times m/m}$$

where :

•

 $P = 100^{KN}$  : Single nominal wheel load  $\omega d =$  dead load of deck slab  $= 23.6 \times 0.20 = 4.720^{-KN/m}$ 

d) over hanging slab

Span  $\ell = 1.40 - 0.60 - 0.30 - 0.25 = 1.250^{m}$ 

moment 
$$M = \frac{P \cdot \ell}{1.30 \ \ell + 0.25} = \frac{100 \times 0.25}{1.30 \times 0.25 + 0.25} = 43.5^{KNm} < 56.8^{KNm}$$
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#### 2. Calculation of stress

a) middle span 
$$b = 100^{cm}$$
  $b = 20$   $d = 15.0$   $d' = 5.0$   
As  $= Y_{12} - 150^{ctc} = 1.131/0.150 = 7.540$  cm²  
P  $= \frac{7.540}{100 \times 15.0} \times 100 = 0.503 \%$   
 $\chi = \frac{0.87 \times 41000 \times 7.540}{0.40 \times 3000 \times 100} = 2.4^{cm}$   
Z  $= 15.0 - \frac{1}{2} \times 2.4 = 13.8^{cm} < 0.95 \times 15 = 14.25^{cm}$   
M_{RS} = 0.87 × 41000 × 7.54 × 13.8 × 10⁻⁵ = 37.1^{KNm} > 33.0^{KNm}  
M_{RS} = 0.40 × 3000 × 100 × 2.4 × 13.8 × 10⁻⁵ = 39.7^{KNm} > 33.0^{KNm}  
b) each fulcrum  $b = 100^{cm}$   $h = 20$   $d = 16.0$   $d' = 4.0$   
As  $= Y_{18} - 150^{ctc} = 2.011/0.150 = 13.407$  cm²  
P  $= \frac{13.407}{100 \times 16.0} \times 100 = 0.838 \%$   
 $\chi = \frac{0.87 \times 41000 \times 13.407}{0.4 \times 3000 \times 100} = 4.0^{cm}$   
Z  $= 16.0 - \frac{1}{2} \times 4.0 = 14.0^{cm} < 0.95 \times 16.0 = 15.2^{cm}$ 

 $M_{RS} = 0.87 \times 41000 \times 13.407 \times 14.0 \times 10^{-6} = 66.9^{KNm} > 56.8^{KNm}$  $M_{RC} = 0.40 \times 3000 \times 100 \times 4.0 \times 14.0 \times 10^{-5} = 67.2^{KNm} > 56.8^{KNm}$ 

0K

Calculation of deck slab (S.L.S) : Check

Span  $\ell = 1.40^{\text{m}}$  ... review of fulcrum for bending moment moment  $M = \left\{ (0.15 \times 1.40 + 0.125) \times 100 + \frac{1}{10} \times 4.720 \times 1.40^2 \right\} = 34.5^{\text{KNm}}$ 

Calculation of stress

 $b = 100^{\text{cm}} \quad h = 20 \qquad d = 16.0 \qquad d' = 4.0$   $A = Y_{18} - 150^{\text{ctc}} = 13.407 \text{ cm}^2$   $P = \frac{13.407}{100 \times 16.0} \times 100 = 0.838 \%$   $X = \frac{0.80 \times 41000 \times 13.407}{0.25 \times 3000 \times 100} = 6.3^{\text{cm}}$   $Z = 16.0 - \frac{1}{3} \times 6.3 = 12.9^{\text{cm}}$ 

 $M_{RS} = 0.80 \times 41000 \times 13.407 \times 12.9 \times 10^{-5} = 56.7^{KNm} > 34.5^{KNm}$  $M_{RC} = 0.25 \times 3000 \times 100 \times 6.3 \times 12.9 \times 10^{-5} = 60.9^{KNm} > 34.5^{KNm}$ 

0K

ucantity of expantion or shrinkage (maximum) for temperature : dt = $a \times T \times L = (I_0 \times i 0 \overset{5}{s} \times 5 \circ 0 \times L) = (0.15 \circ \times L)^{m}$ for shrinkage : ds = $a \times T \times L \times b = (J_0 \times i 0 \overset{5}{s} \times 20 \circ \times L \times 0.6) = (0.16 \circ \times L)^{m}$ for shrinkage : ds = $a \times T \times L \times b = (J_0 \times i 0 \overset{5}{s} \times 20 \circ \times L \times 0.6) = (0.16 \circ \times L)^{m}$ for creep : dc = $\frac{P}{e^{x}A} \times \Phi \times L \times b = \frac{750}{21\times(10^{6} \times 1.9)} \times L \times 0.60 = (0.16 \circ \times L)^{m}$ for other : do = $\frac{P}{e^{x}A} \times \Phi \times L \times b = \frac{750}{21\times(10^{6} \times 1.9)} \times L \times 0.60 = (0.16 \circ \times L)^{m}$ for other : do = $\frac{P}{e^{x}A} \times \Phi \times L \times b = \frac{750}{21\times(10^{6} \times 1.9)} \times L \times 0.60 = (0.16 \circ \times L)^{m}$ for other : do = $\frac{P}{e^{x}A} \times \Phi \times L \times b = \frac{750}{21\times(10^{6} \times 1.9)} \times L \times 0.60 = \frac{1}{600} = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00 = 0.00$
Ge ( maxin Ge ( maxin $f(0) \times 10^{5} \times 15.0$ $b = (10 \times 10^{5} \times 20)$ $b = (10 \times 10^{5} \times 20)$ $b = (10 \times 10^{5} \times 20)$ f thermal f emperatur f decrease flus $\times 300/2 = 1$

Calculation of Shoe 1) quantity of expantion between Girderedge and Parapet face of abutment

-134

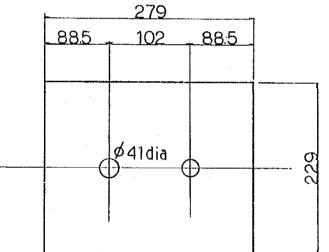
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	ation o	ofs	shoe	;				
edge	fulcru	n	Rd		1950.2/9 × 1.1	¥	238.4	KN⁄choe
( M	107)	F	?L1	=	1509.2/9 × 1.1	2	184.6	"
	(	or F	₹L 2	E	(472.4+373.4+206.8+105.8)/4	**	289.6	"
		Ru	nax	=		=	528.0	
		d	i L	z	$(0.80L+5) = (0.80 \times 28.7+5)$	Ħ	28 mm	
middle	fulcri	u m	Rd	=	6270.0 / 9 × 1.1	=	766.4	KN⁄choe
15	ix)	F	RL 1	=	2929. 2 / 9 × 1. 1	=	358.1	"
()	-							

Rmax =

= 1290.0 "

1) edge fulcrum (mov) = 279 mm  $\times$  229 $\times$ 65 (Al,A2)



vertical pressure

AS = 27.9×22.9 -  $\frac{\pi}{4}$  × 4.1²×2 = 612.5 cm² VC =  $\frac{Rmax}{AC}$  =  $\frac{528.0 \times 10^3}{612.5}$  = 800 N/cm² < Vca = 800 N/cm²

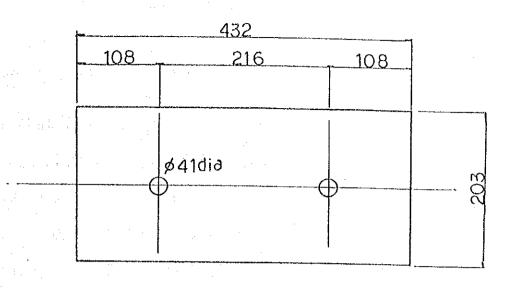
Dowel bar  $\rightarrow \phi 20 \text{ mm} \times 500 \text{ mm} \times 2 \text{ NO} / \text{shoe}$ 

Ab =  $\frac{\pi}{4}$  × 2.0² × 2 = 6.283 cm² Hd = 238.4 × 0.15 = 35.8 KN/choe --- temperature state shearing stress

 $\tau s = \frac{1.43 \text{ H d}}{\text{Ab}} = \frac{1.43 \times 35.8 \times 10^3}{6.283} = 8200 \text{ N/cm}^2 < 9000 \text{ N/cm}^2$ 

anchor cap -----  $\phi 80 \text{ mm} \times 250 \text{ mm} \times 2 \text{ NO} / \text{ shoe}$ 

2) middle fulcrum (Fix):432mm × 203 × 18 (P1, P2)



Vertical pressure

$$AS = 43.2 \times 20.3 - \frac{\pi}{4} \times 4.1^2 \times 2 = 850.5 \text{ cm}^2$$

$$VC = \frac{Rmax}{As} = \frac{1290.0 \times 10^3}{850.5} = 1520 \text{ N/cm}^2 \quad \langle \text{Vca} = 1600 \text{ N/cm}^2$$

Dowel bar  $\phi$  40 mm  $\times$  900 mm  $\times$  2 NO  $\checkmark$  shoe

Ab = 
$$\frac{\pi}{4}$$
 × 4.0² × 2 = 25.133 cm²

IId = 
$$(238.4 + 766.4) \times 0.1$$
 = 100.5 --- seismic state

shearing stress

$$\tau s = \frac{1.65 \text{ Hd}}{\text{Ab}} = \frac{1.65 \times 100.5 \times 10^3}{25.133} = 6.600 \text{ N/cm}^2 < 9000 \text{ N/cm}^2$$

anchor bar  $---- \phi$  50mm  $\times$  450mm  $\times$  2 NO/ shoe

#### MOMBASA ——— Substructure

Reaction from Superstructure

1) For Abut (Movable) ... S. L. S. For all width of Abut (B =  $18.300^{m}$ ) dead load : Rd =  $1950.2^{KN}$ live load : R & =  $1509.2^{KN}$ total : R =  $3459.4^{KN}$ 

For Unit width of Abut

(1) For Vertical load

$$R d = \frac{1950.2}{18.30} = 106.570^{KN/m}$$

$$R \ell = \frac{1509.2}{18.30} = 82.470^{KN/m}$$

$$R = 189.040^{KN/m}$$

(2) For Horizontal force for temperature of Seismic  $H_T = H_P = 106.570 \times 0.15 = 15.986^{KN/m}$ 

- 2) For Pier (Fixed) ... S.L.S.
  - (1) For Vertical load

dead load :  $Rd = 6270.0^{KN}$ live load :  $R \ell = 2929.2^{KN}$ total :  $R = 9199.2^{KN}$ 

- (2) For Horizontal load
- a) Longitudinal direction

Braking :  $H_B = 8.0 \times 39.25 + 200 = 514.0^{KN}$ Seismic :  $H_D = (1950.2 + 6270.0) \times 0.10 = 822.0^{KN}$ 

b) Crossing direction

Skidding :  $H_s = 250^{KN}$ Seismic :  $H_D = 6270.0 \times 0.10 = 627.0^{KN}$ 

34.300 (t/m²) 0.000 (t/m²) 23.600 (t/m^3) [9.600 (t/m³) 10.800 (t/m³) 18.600 (t/m³) 9.800 (t/m³) 9.800 (t/m³) 0.00 (t/m^2) 0.350 (m) 35.000 (*) 82:470 (t) 105:570 (t) 15.986 (t) 15.986 (t) 0.100 (m) 0.10 0.00 0.500 (UNDER WATER) GAMIS = GAM2 = (UNDERWATER) GAM2S = tanøB = KHS 님 ЧŽ 0g 8 КН GAMC Ê RX GAM1 H 긚 WATS INTERNAL FRICTIONANGLE FAI FOR FOUNDATION GROUND HORIZONTAL FORCE FOR SITUATION OF REACTION FOR ABOVE TOE SLAB AND HORIZONTAL FORCE LIVE LOAD REACTION OF DEAD LOAD TEMPERATURE.SEISMIC ALLOWABLE PRESSURE UNIT VOLUME WEIGHTS FOR CONCRETE FOR BACK FILL SEISMIC COEFFICIENT COHESIVE DOWER FRICTION FACTER FOR WATER DIMENSION (KN) INTO THIS CALCULATION NOTE : THE DIMENSION(1) BE EXCHANG TO 5.000 (m) (m) 001.1 1.500 (m) 0.300 (m) 2.400 (m) (m) 008.0 (m) 000.0 0.500 (m) 0.500 (m) HU2 = IOH BC E B2 83 B4 ខ្លួ B6 SHAPE AND SIZE MOMBASA-ABUT 9.000 (m) 1.400 (m) 0.000 (m) (m) 000.0 0.000 (m) 0.800 (m) 1.000 (m) (m) 000.1 6.800 (m) (m) 000 (m) (m) 000 (m) ÷, 11 ń BW2 0H BWJ IWH HW2 НI HZ щ Ŧ 12 ц Н (1)

350.00 (1/m²)

Q Q

CALCULATION OF WEIGHT AND FORCE OR LOAD

(1) CONCRETE

	V(t)	H(t)	X(m)		X(m) 4 V(m) 4 WX(t-m) 4 WX(t-m)	MY(t-m)
5  	9.912	166.0	2.450	8.300	24.284	8.227
4 00 	94.400	075 5	2.500	0.400	236.000	3.776
Σ 1 1 280	280.840 1	28.084 1			622.167	86.145

(2) EARTH

a) BACK FILLING

NO. I	V(t)	H(t) [	X(m)	Y(m)	MX(t·m)	MY(t·m)
	65.856 I	6.586	3.800	8.300 1	250.253 1	54.661
ч. 	319.872	31.987	3.800	4.200 ]	1215.510	134.346
2	385.728 1	38.573			1465.770 1	189.007

NO.	V(t)	H(t)	X(m) 1	Y(m)	MX(t·m) ]	MY(t·m)
9	27.900	0.000	0.750	1.300	20.925	0,000
5.3	27.900	0.000			20.925	0.000

1-140

Н = V*KHS МҮ = Н*Ү

V = X*Y*BW*GAM1 MX = V*X

(3) REACTION

•			· · · ·		• • •	0.3277 0.0000 1.0000
	(t-m)	0.000   123.052   123.092			SEISMIC	<u>оон</u>
	F RMY	122			EIS	0.3056 0.3007 0.9537
	RMX(t+m)   RMY(t+m)	368.628 368.628 207.811	X+*Y	0R		000
	I RMX	36	RH : RMY=	-AC1	TURE	52 52 53
•	RH(t)	0.000 5.986 5.986		ВE	NARY OR TEMPERATURE	0.2508 0.2022 0.9793
	HR -	• ••• •••		SSU	IARY MP	
2	RV Ct :	89.040 89.040 06.570	÷	I PRE	ORDINARY ( TEMPER	0.2497 0.5736 0.8192
2 2			RV : RMX= RV*X	RTH		22
	STATE	CRDINARY PERATURE SEISMIC	RV RM	(4) EARTH PRESSURE FACTOR		S I N (6) COS (6)
	•	ΤE				

(5) EARTH PRESSURE

		)))				
	[ V(t)	l = H(t) =	X(m) [	Y (m)	MX(t·m)	MX(t.m) MY(t.m)
•						
	1 44 215	62-147	5.000	4.500	1020-122	1701.482
	1 113.698	I 162.378 I	5.000 1	3.000	568.4911	487.1341
	1 83.224	1 118.857 1	5.000 1	3.867	416.1211	459.5791
	1 29.409	1 42.000 1	5.000 [	0.640	147.0441	26.896
					_	
÷.,	1 72.941	1 231.339 /	5.000 1	3.000	364.7051	694.0181
	1 53.391	1 169.335 1	5.000 1	3.867 1	266 9551	654.7511
	1 18.867	59.838	5.000	0.640	94 334	38.3181
				-	-	

(6) BUOYANCY

0.0001 0.0001 MY(t-m) -154.1281 -154.1281 338.027 MX(t-m) 0.0001 0.0001 Y(m) 2.4961 2.4963 3.6501 X(m) 0.0001 0.0001 H(t) -61.750 92.510 V(E)

TOTAL OF ACTION FORCE

CORDINARY FOR FOUNDATION

771.296	3605.080	225.525	1134.030	TOTAL
000 0	20.925	000.0		23
0.000	338.027 1	0.000 1	92.610.1	ſ
000 0	368.628	0.000	189.040	
487.134	568.491	162.378	13.698 1	· ·
284.162	221.080	63.147	44.216	• •••• •
0.000	622.167 1465.770	0.000	280.840 1 385.728 1	-01 
MY(t·m)	MX(t-m)	H(t)	V(t)	LOAD

2833.790 (t·m)  $Mo = \Sigma MX - \Sigma MY =$ 

# (2) ORDINARY ... FOR INVERSION OR SLIDE

	V ( t)	H(t)	MX(t-m) ]	MY(t·m)
SAME	280.840	0.000	622.167 1	0.000
11	1 827.685	000-0	1465.770	0.000
-	44.216	63.147	221.080	284.162
	113.698 ]	162.378	568.491	487.134
	106.570	0.000 4	207.811	000.00
	27.900	0.000	20.925	0.000
,	958.952	225.525	3106.240 1	771 296

2334.950 (t·m) ΣMY = ł SMX 11

ŝ

(3) TEMPERATURE-FOR FOUNDATION

	V(t)	H(t)	MX(t·m)	MY(t·m)
SAME	280.840	0.000	622.167 1465.770	0.000
1 (1)	44.216	63.147	221.080 568.491	284.162
	27.900	000.0	207.811	n o
	958.952	241.511	3106.240	894.388

2211.850 (t-m) II ΣMY I ΣMX Mo =

(5) SEISMIC

86.145 189.007 694.018 123.092 0.000 MY(t-m) 364.705 207.811 20.925 622.167 1465.770 MX(t-m) 28.084 38.573 231.339 15.986 0.000 H(t) 280.840 385.728 72.941 106.570 27.900 V(t) SAME 6

1589.110 (t·m) 11 ΣMY t XWX 11 o X

1092.260

2581.370

313.982

873.979

. |~|4|

(3) TEMPERATURE

2. INCLUDE BUOYANCY

		1111		I MI(L·B)
	280.840   385.728	0.000	622.167 1465.770	00000
•••	44.2	6	221.08	
	83.224	118.857	416.121	459.579
	40	~	47.04	26.
	4	•	68.62	6
	92.61	•	38.02	ം
	05	•	0.92	
	Ω` N	0.000	4 . L	
	1 1071.220	239.990	3445.630	893.729

2551.900 (t-m) 11  $Mo = \Sigma MX - \Sigma MY$ 

## (4) TEMPERATURE

<u> </u>	V(t)	R(t)	MX(t·m)	MY(t-m)
;	80.84	0.000	22.16	0.000
	385.728	0.000	1465.770	0.000
	4		21.0	84.
	83.224	118.857	416.121	459.579
-	4.6	~	47.0	ω,
	5.5	ن	07.8	
	7.9		0.92	
<b>-</b> 4	÷		.12	00000
l	896.137	239-990	2946.790	893.729

2674.990 (t·m) 0.000 284.162 4594.162 26.896 0.000 0.000 0.000 284.162 459.579 26.896 0.000 0.000 0.000 0.000 770.637 770.637 MY(t·m) MY(t·m) 622.167 1 1465.770 1 221.080 | 416.121 | 147.044 207.811 207.811 20.925 2946.790 622.167 1465.770 221.080 415.121 147.044 368.628 338.027 20.925 3445.630 -154.128MX(t-m) MX(t·m)  $\Sigma M X - \Sigma M Y =$ 224.004 | 0.000 224.004 0.000 63.147 18.857 42.000 63.147 18.857 12.000 H(t) H(t) Í. 280.840 385.728 896.137 44.216 83.224 29.409 106.570 27.900 27.900 44.216 83.224 29.409 189.040 92.610 92.610 -27.900 280.840 385.728 1071.220 Š V(t) V(E)**ORDINARY** (2) ORDINARY

2176.150 (t·m) ΣMY = I ΣMX П

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2053.060 (t·m)

11

ΣMY

ı

= 2MX

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TOTAL FORCE FOR UNDER

FONDATION CENTER

Mc(t-m)		523	. 43	124.385	. 52	95.83		3.049	4.4	26.14	187.284	96.35
e (B)		8	.06	0.110	51.	.68		00.	-0-	1	0.209	. 73
Mo(t·m)		833.7	334.9	2710.700 1	211.8	589.I		674.99	176.15	551.90	2053.060	432.51
H(t)		25-52	25.52	241.511	41.51	13.98	-	24.00	24.00	39.99	239.990 1	11.81
V(t)		34.03	58.95	1134.030	58.95	73.97		71.2	96.13	71.22	67	11 54
LOAD	4			ლ ლ	4		n 		- 2	- -	1 4	5

Mc = V * e e = B0/2 - Mo/V

### WHERE

A AND B: EXCLUDE OF BOUYANCY OR INCLUDE BOUYANCY

FOR INVERSION OR SLIDE

1. ORDINARY : FOR FOUNDATION

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3. TENPERATURE : STATE OF 1

5. SEISMIC

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(5) SEISMIC

MY(t·m)	86.145 189.007	654.761 38.318 123.092 0.000	1091.320
MX(t·m)	622.167 1465.770	266.955 94.334 207.811 20.925 -154.128	2523.830
H(t)	28.084 38.573	169.335 59.335 15.986 0.000	311.815
V(t)	280.840 1 385.728 1	53.391 18.867 106.570 27.900 27.900	811.546

1432.510 (t-m) – 2MY =  $Mo = \Sigma MX$ 

FOR CONTACT PRESSURE

UNDER FOUNDATION 5.000 873.979 313.982 595.835 0.682 317.796 31.796 Ð 1134.030 241.511 124.385 5.000 256.659 196.954 0.110 ŝ BOUYANCY 1134.030 225.525 1.293 5.000 227.117 226.496 0.001 Qmax(t/m^2)| Qmin(t/m^2)| EXCLUDE STATE ÊÊ êê N H K ۰ ۲ <u></u>м - 1

	r• •				-
	ഹ	5.000 1.000	811.546 311.815 596.356	, 0.735 5.000	305.435 19.184
CΥ	n	5.000 1.000	1071.220 235.990   126.141	0.118	244.517 183.970
BOUYANCY	-	5.000   1.000	1071.220   224.004   3.049	0.003   5.000	214.975 213.512
NCLUDE	STATE	ран (а) (а) (а)	V (t)   H (t)   Mc (t·m)	e (m) X (m)	Qmax(t/m^2)  Qmin(t/m^2)  r
		]			

= V/(B*L) + 6*Mc/(L*B^2) ø

Q = 2*V/(L*X) :X = 3*(B0/2-Mc/V)

FOR DIRECT FOUNDATION e (m) CALCULATION OF SECURITY Mc(t-m) INVERSION V(t) Н ОК

0.072 0.209 0.735 64.191 187.284 596.356 896.137 896.137 811.546 e = Mc/V

ψ¢

~~~~

B0/6 B0/3

0.833 0.833 1.667

0.833 0.833 1.667

0.065 0.193 0.682

62.435 185.528 595.835

958.952 958.952 873.979

∢

m

| | E S | <u>^</u> ^^ | |
|-------|---------|--|--------------------------|
| | | 2.13
1.99
1.39
2.00
1.30 | |
| | Hu(t) | 479.48
479.48
436.99
448.07
448.07
448.07 | 0.50 |
| | Hb(t) H | 225.525
241.511
313.982
224.004
239.990
311.815 | $an(\phi B) = 0$ |
| n | V(t) [| 958.952
958.952
873.979
896.137
896.137 | |
| SLIDE | A'(m^2) | 4 4 0 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 | 0.00 (t/m <sup>2</sup>) |
| FOR | ¥
 | NMNM | 0.0
II 0 |
| | | < 00 | |

5 5 6 1 1 7

500

 $Hu = C*A' + V*tan(\phi B)$

Fs = Hu/Hb

MOMBASA ---- ABUT

Notice : the Calculation of this Abut is abridge and the bar arengement of this vertical wall or footing slab is adapt Abut of Uhuru Bridge.

where

Vertical wall \cdots Y<sub>22</sub>-150<sup>ctc</sup> Footing slab \cdots Y<sub>25</sub>-150<sup>ctc</sup>

CALCULATION OF WINGWALL

Caluculation formula for wingwalls

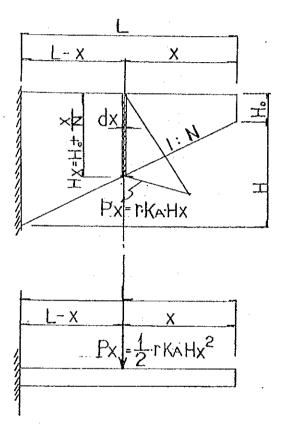
bending moment

$$Mx = \int_{0}^{L} Px (L-x) dx$$

= $\int_{0}^{L} \frac{1}{2} r K_{A} (H_{0} + \frac{x}{N})^{2} (L-x) dx$

shearing force

$$Sx = \int_{0}^{L} Px \cdot dx$$
$$= \int_{0}^{L} \frac{1}{2} rK_{A} (H_{0} + \frac{x}{N})^{2} dx$$

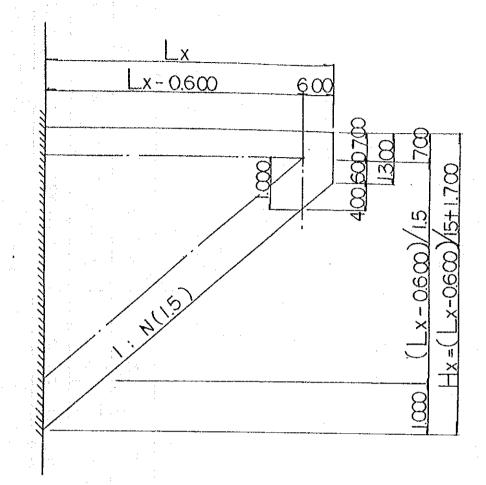


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. Analysis of upper formula for earth pressure

$$Mx = \frac{1}{2} r K_{A} \left(\frac{H_{o} \cdot x^{2}}{2} + \frac{H_{o} \cdot x^{3}}{3 \cdot N} + \frac{x^{4}}{12 \cdot N^{2}} \right)$$

$$Sx = \frac{1}{2} r K_{A} \left(H_{o}^{2} \cdot x + \frac{H_{o} \cdot x^{2}}{N} + \frac{x^{3}}{3 \cdot N^{2}} \right)$$



from general formula

where $r = 19.6 \quad \text{KN/m}^{2}$ $K_{A} = \tan^{2}(45^{\circ} - \frac{\phi}{2}) = \tan^{2}(45^{\circ} - \frac{30^{\circ}}{2}) = 0.5774^{2}$ $= 0.3334 \quad \Rightarrow \ 0.400$ $M_{X} = \frac{1}{2} \times 19.6 \quad \times 0.400 \left(\frac{1 \cdot 30^{2}}{2} x^{2} + \frac{1 \cdot 30}{3 \times 1.5} x^{3} + \frac{1}{12 \times 1.5^{2}} x^{4}\right) \not H_{X}$ $= (3.312 \cdot x^{2} + 1.132 \cdot x^{3} + 0.145 \cdot x^{4}) \not H_{X} \quad (\text{KNm/m})$ $S_{X} = \frac{1}{2} \times 19.6 \quad \times 0.400 \left(1 \cdot 30^{2} \cdot x^{2} + \frac{1 \cdot 30}{1 \cdot 5} x^{2} + \frac{1}{3 \times 1.5^{2}} x^{3}\right) \not H_{X}$ $= (6.628 \cdot x + 3.397 \cdot x^{2} + 0.581 \cdot x^{3}) \not H_{X} \quad (\text{KN/m})$

Calculation of bending moment and shearing force

menber ы С 34.034.0 44.0 54.053. 5 53.5 63.0 63.0 63.0 ഹ ഹ ണ്ണ <u>8</u>3. っ thickness of ដ **4**0 60 00 40 20 60 90 20 70 22 50 a Y<sub>25</sub>-300°t° Y<sub>32</sub>-300°t° $Y_{16} - 150$ ctc Y16-150ctc Y16-150°tc Y20-150ctc Y 25-150°tc Y<sub>25</sub>-150° tc Y32-300°tc Y25-150°tc Y25-150°tc Y20-300ctc Y32-150°tc reinforced Area of $S_X(K^N/m)$ 71.000 81.919 93.592 179.202 196.090 213.700 232.083 147.691 251.219 271.072 163.061 S U.L. $M_{\rm X} (^{\rm KNm}/_{\rm m})$ 146.283 360.312 418.782 630.760 714.538 903.322 179.552 483.200 553.792 805.330 117.257 85.618 41.159 47.489 54.256 94.528 123.884145.634 134.541 103.885 113.675 157.143 $S_{X}(^{KN}/_{m})$ S . دـــ . ທ $M_X (^{KNm}/_m)$ 67.975 84.802 365.658 414.225 466.858 208.876 104.088 242.772 280.116 321.039 523.665 H<sub>X</sub>(m) 4.633 5.3006.633 6.967 7.300 7.633 7.967 8.300 8.633 8.967 4.967 L X (m) ນ. ຄ. 6.0 8.0 %. ୨ 9.0 10.0 10.5 11.0 11.5ភ 5.0 പ്

| <pre>t and shearing intension for U.L.S and S.L.S
5.5<sup>m</sup>
= 40 d = 34.0 d' = 6.0
= 40 d = 34.1 cm<sup>2</sup>
/bd =13.41 cm<sup>2</sup>
/bd =13.41 ×100 /100 ×34.0=0.394 %
b)
/bd =13.41 ×100 /100 ×34.0=0.394 %
M<sub>RS</sub>
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/bd =13.41 ×100 *</pre> | • | 11 = 4.8 cm | - x/2= 34.0 - 4.8/3= 31.6 °m <0.95×34.0=32.3°m | r As Z | $= 0.87 \times 41000 \times 13.41 \times 31.6 \times 10^{-5}$ | <sup>кит</sup> > Mu =117.257 <sup>кит</sup> ог 146.283 <sup>кит</sup> | Zx-qn | =0.40×2500×100 ×4.8 ×31.6×10 <sup>-5</sup> | <sup>knm</sup> > Mu 0K |
|--|----------|--|--|-------------------------|---|---|-------------------------------|--|--------------------------|
| calculation of
esisting moment and shearing intension for
esisting moment and shearing intension for
$Lx = 5.0^{m}$ and 5.5 <sup>m</sup>
b = 100 <sup>cm</sup> h = 40 d = 34.0 d' =
b = 100 <sup>cm</sup> h = 40 d = 34.0 d' =
As = Y_1e-150 <sup>cte</sup> = 2.011/0.15 = 13.41 cm
b = As × 100/bd =13.41 × 100 /100 ×3
p = As × 100/bd =13.41 ×100 /100 ×3
s.L.S
S.L.S
s.L.S
= $\frac{0.84y.As}{0.25 \times 2500 \times 100} = 7$.
$n = \frac{0.84y.As}{0.25 \times 2500 \times 100} = 7$.
n = d - x/3 = 34.0 - 7.0/3 = 31.7 cm
n = d - x/3 = 34.0 - 7.0/3 = 31.7 cm
n = d - x/3 = 34.0 - 7.0/3 = 31.7 cm
n = d - x/3 = 34.0 - 7.0/3 = 31.7 cm
$n = 0.25fcub·K = Ms = 67.975 ^{Wm}or 84.8$
$n = 0.25fcub·K = Ms = 67.975 ^{Wm}or 84.8$
$n = 0.25fcub·K = 139.4 ^{Wm} > M$ | b) U.L.S | в
11
Х | ъ | $M_{Rs} = 0.87 fy As Z$ | $= 0.87 \times i$ | 02 <sup>кит</sup> — 151.1 <sup>кит</sup> | $M_{Rc} = 0.40 fcub \cdot xZ$ | $= 0.40 \times 2$ | = 151.7 <sup>KNm</sup> |
| Rech c
M <sub>i</sub> M Z X <sup>a</sup> | | $\frac{0.8 \text{fy} \cdot \text{As}}{0.25 \text{fcu} \cdot \text{b}} = \frac{0.8 \times 41000 \times 13.41}{0.25 \times 2500 \times 100} =$ | ฮ | $M_{rs} = 0.8 fy As Z$ | =0.8 ×41000 ×13.41 ×31.7×10 <sup>-5</sup> | $= 139.4 \text{ KNm} = \text{Ms} = 67.975 \text{ KNm}_{OT} 84.81$ | $M_{Rc} = 0.25 fcub \cdot xZ$ | =0.25×2500×100 ×7.0 ×31.7×10⁻⁵ | 138.7 <sup>кит</sup> > М |

Notice : the U.L.S is critical from result of both calculation of resisting moment and the other case is exclude calculation of resisting moment for S.L.S

Shearing intension (U.L.S)

2)

$$\tau = \frac{Su}{bd} = \frac{81.919 \times 10^3}{100 \times 34.0} = 24.1 \text{ N/cm}^2$$

$$< \tau a = 35.0 + \frac{15.00}{0.25} (0.394 - 0.25) = 43.6 \text{ N/cm}^2$$

Lx = 6.0<sup>m</sup> (for U.L.S)
b = 100<sup>cm</sup> h=50 d=44.0 d' = 6.0
As= Y<sub>16</sub>-150<sup>ctc</sup>= 2.011/0.15 = 13.41 <sup>cm</sup>
P = 13.41×100/100×44.0 = 0.305
X =
$$\frac{0.87 \times 41000 \times 13.41}{0.40 \times 2500 \times 100}$$
 = 4.8 <sup>cm</sup>
Z = 44.0 - 4.8/2 = 41.6<sup>cm</sup> < 0.95×44.0 = 41.8 <sup>cm</sup>
M<sub>Rs</sub> = 0.87 × 41000 × 13.41 × 41.6×10<sup>5</sup> = 199.0<sup>KNm</sup> > Mu = 179.552<sup>KNm</sup>
M<sub>Rc</sub> = 0.40 × 2500×100 × 4.8×41.6×10<sup>5</sup> = 199.7<sup>KNm</sup> > Mu = 179.552<sup>KNm</sup>
UK
 $\tau = \frac{93.592 \times 10^3}{100 \times 44.0}$ = 21.3 N/cm<sup>2</sup>
 $< \tau a = 35.0 + \frac{15.0}{0.25}$ (0.305-0.25) = 38.3 N/cm<sup>2</sup>

OK

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3) Lx = 8.0<sup>m</sup> (for U.L.S)
b = 100<sup>cm</sup> h=60 d=54.0 d'= 6.0
As= Y 20-150<sup>cxc</sup> = 3.141/0.15 = 20.94 cm<sup>2</sup>
P = 20.94×100/100×54.0 = 0.388

$$X = \frac{0.87 \times 41000 \times 20.94}{0.40 \times 2500 \times 100} = 7.4^{cm}$$

Z = 54.0 -7.4/2 = 50.3<sup>cm</sup> < 0.95×54.0 = 51.3 <sup>cm</sup> OK
Max = 0.87 ×41000 × 20.94 × 50.3×10<sup>-5</sup> = 375.7<sup>KMm</sup> > Mu = 360.312<sup>KMm</sup>
Mac = 0.40 × 2500×100 × 7.4×50.3×10<sup>-5</sup> = 372.2<sup>KMm</sup> > Mu = 360.312<sup>KMm</sup>
Mac = 0.40 × 2500×100 × 7.4×50.3×10<sup>-5</sup> = 372.2<sup>KMm</sup> > Mu = 360.312<sup>KMm</sup>
OK
 $x = \frac{147.691 \times 10^3}{100 \times 54.0} = 27.4$ N/cm<sup>2</sup>
 $< \tau a = 35.0 + \frac{15.0}{0.25}$ (0.388-0.25) = 43.3 N/cm<sup>2</sup>
4) Lx = 8.5<sup>m</sup> 9.0<sup>m</sup> and 9.5<sup>m</sup> (for U.L.S)
b = 100<sup>cm</sup> h=60 d=53.5 d'=6.5
As= Y 20 = 150<sup>cxc</sup> = 4.909/0.15 = 32.73 cm<sup>2</sup>
P = 32.73×100/100×53.5 = 0.612
 $X = \frac{0.87 \times 41000 \times 32.73}{0.40 \times 2500 \times 100} = 11.8^{cm}$
Z = 53.5 - 11.8/2 = 47.6<sup>cm</sup> < 0.95×53.5 = 50.8<sup>cm</sup> OK
Mmx = 0.87 × 41000 × 32.73 × 47.6×10<sup>-5</sup> = 561.7<sup>KMm</sup> > Mu = 418.782<sup>KMm</sup>
Mmc = 0.40 × 2500×100 × 11.8×47.6×10<sup>-5</sup> = 561.7<sup>KMm</sup> > Mu = 418.782<sup>KMm</sup>
Mx = $\frac{186.090 \times 10^2}{100 \times 53.5} = 36.7$ N/cm<sup>2</sup>
oK

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5)
$$Lx = 10.0^{m}$$
 (for U.L.S) i
 $b = 100^{cm}$ h=70 d=63.5 d'= 6.5
As= $Y_{xs} - 150^{ctc} = 4.909/0.15 = 32.73 \text{ cm}^{2}$
P = 32.73×100/100×63.5 = 0.515
 $X = \frac{0.87 \times 41000 \times 32.73}{0.40 \times 2500 \times 100} = 11.8$ cm
Z = 63.5 -11.8/2 = 57.6<sup>cm</sup> <0.95×63.5 = 60.3 cm OK
Mss = 0.87 ×41000 ×32.73 ×57.6×10<sup>-5</sup> = 672.5<sup>KHm</sup> > Mu = 630.760<sup>KHm</sup>
Msc = 0.40 ×2500×100 ×11.8×57.6×10<sup>-5</sup> = 679.7<sup>KHm</sup> > Mu = 630.760<sup>KHm</sup>
OK
 $\tau = \frac{263.700 \times 10^{5}}{100 \times 63.5} = 33.7$ N/cm<sup>2</sup>
 $< \tau a = 50 + \frac{15.0}{0.50}$ (0.515-0.50) = 50.4 N/cm<sup>2</sup>
6) $Lx = 10.5^{m}$ (for U.L.S)
 $b = 100^{cm}$ h=70 d=63.0 d'=7.0
As= $\frac{Y_{x0} - 300^{ctc}}{8.042/0.300} = 37.28$ cm<sup>2</sup>
 $P = 37.28 \times 100/100 \times 63.0 = 0.592$
 $X = \frac{0.87 \times 41000 \times 37.28}{0.40 \times 2500 \times 100} = 13.4$ cm
 $Z = 63.0 - 13.4/2 = 56.3^{cm} <0.95 \times 63.0 = 59.8$ cm OK
Mss = 0.87 ×41000 ×37.28 ×56.3×10<sup>-5</sup> = 748.6<sup>KMm</sup> > Mu =714.538<sup>KHm</sup>
Msc = 0.40 ×2500×100 × 13.4×56.3×10<sup>-5</sup> = 748.4<sup>KHm</sup> > Mu =714.538<sup>KHm</sup>
Msc = 0.40 ×2500×100 × 13.4×56.3×10<sup>-5</sup> = 748.4<sup>KHm</sup> > Mu =714.538<sup>KHm</sup>
OK
 $\tau = \frac{232.083 \times 10^{2}}{100 \times 63.0} = 36.8$ N/cm<sup>2</sup>
 $< \tau a = 50 + \frac{15.0}{0.50}$ (0.592-0.50) = 52.7 N/cm<sup>2</sup> OK

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7)
$$Lx = 11.0^{m}$$
 (for U.L.S)
 $b = 100^{cm}$ h=70 d=63.0 d' = 7.0
 $As = \frac{Y_{25} - 300^{ctc}}{Y_{32} - 300^{ctc}} \begin{bmatrix} 4.909/0.300 \\ 8.042/0.300 \end{bmatrix} = 43.17 \text{ cm}^{2}$
 $P = 43.17 \times 100/100 \times 63.5 = 0.685$
 $X = \frac{0.87 \times 41000 \times 43.17}{0.40 \times 2500 \times 100} = 15.4^{cm}$
 $Z = 63.0 - 15.4/2 = 55.3^{cm} < 0.95 \times 63.0 = 59.8^{cm}$ OK
 $M_{RS} = 0.87 \times 41000 \times 43.17 \times 55.3 \times 10^{-5} = 851.5^{KNm} > Mu = 805.330^{KNm}$
 $M_{Rc} = 0.40 \times 2500 \times 100 \times 15.4 \times 55.3 \times 10^{-5} = 851.6^{KNm} > Mu = 805.330^{KNm}$ OK
 $\tau = \frac{251.219 \times 10^{3}}{100 \times 63.0} = 39.9 \text{ N/cm}^{2}$

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$$(\tau_a = 50.0 + \frac{15.0}{0.50} (0.685 - 0.50) = 55.4 \text{ N/cm}^2$$

check of S.L.S

s,

$$X = \frac{0.80 \times 41000 \times 43.17}{0.25 \times 2500 \times 100} = 22.8 \text{ cm}$$

$$Z = 63.0 - 22.8/3 = 55.4^{cm}$$

$$\begin{split} M_{RS} &= 0.80 \times 41000 \times 43.17 \times 55.4 \times 10^{-5} = 784.4^{\text{KNm}} > \text{Mu} = 466.858^{\text{KNm}} \\ M_{RC} &= 0.25 \times 2500 \times 100 \times 22.8 \times 55.4 \times 10^{-5} = 789.4^{\text{KNm}} > \text{Mu} = 466.858^{\text{KNm}} \\ \text{OK} \end{split}$$

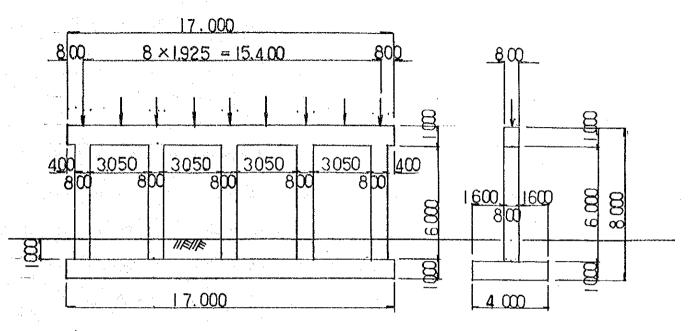
8) Lx = 11.5<sup>m</sup> (for U.L.S)
b = 100<sup>cm</sup> h=70 d=63.0 d' = 7.0
As=
$$Y_{32}-150^{ctc}= 8.042/0.15 = 53.61$$
 cm<sup>2</sup>
P = 53.61×100/100×63.0 = 0.851
X = $\frac{0.87 \times 41000 \times 53.61}{0.40 \times 2500 \times 100}$ = 19.2 cm
Z = 63.0 -19.2/2= 53.4<sup>cm</sup> <0.95×63.0= 59.8 cm OK
M<sub>RS</sub> = 0.87 ×41000 ×53.61 ×53.4×10<sup>-5</sup> = 1021.1<sup>KNm</sup> > Mu = 903.322<sup>KNm</sup>
M<sub>Rc</sub> = 0.40 ×2500×100 ×19.2×53.4×10<sup>-5</sup> = 1025.8<sup>KNm</sup> > Mu = 903.322<sup>KNm</sup>
C = $\frac{271.072 \times 10^3}{100 \times 63.0}$ = 43.1 N/cm<sup>2</sup>

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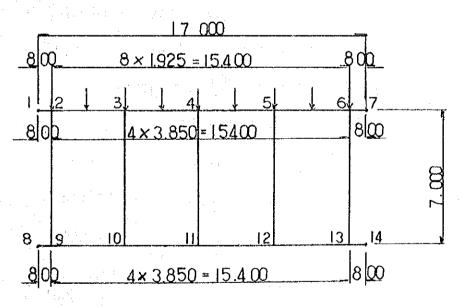
 $< \tau a = 50.0 + \frac{15.0}{0.50}$ (0.851-0.50) = 60.5 N/cm<sup>2</sup>

Crossing direction

1) Shape and Size



2) Frame



3) Factor of section

beam : $A = 0.80 \times 1.00$ = 0.80000 m<sup>2</sup> $I = \frac{0.80 \times 1.00^3}{12}$ = 0.06667 m<sup>4</sup> Pillar: $A = 0.80 \times 0.80$ = 0.64000 m<sup>2</sup> $I = \frac{0.80^4}{12}$ = 0.03413 m<sup>4</sup> footing : $A = 4.00 \times 1.00$ = 4.00000 m<sup>2</sup> $I = \frac{4.00 \times 1.00^3}{12}$ = 0.33333 m<sup>4</sup> E c = 2.5×10<sup>7</sup> KN/m<sup>2</sup> 4) Load

a) Reaction from superstructure Dead load : $Rd_1 \sim Rd_9 = 6270.0/9 = 696.670 \text{ KN/shoe}$ Live load : $R\ell_{1\sim3} = R\ell_{7\sim9} = 160.0 \text{ KN/shoe}$ $R\ell_4 = R\ell_5 = R\ell_6 = (2929.2 - 160.0 \times 6)/3 = 656.4 \text{ KN/shoe}$

b) Dead load of substructure

beam : $\omega d_1 = 23.6 \times 0.80 \times 1.00 = 18.880 \text{ KN/m}$ pillar : $\omega d_2 = 23.6 \times 0.80 \times 0.80 = 15.104 \text{ KN/m}$ footing : $\omega d_3 = 23.6 \times 4.00 \times 1.00 = 94.400 \text{ KN/m}$ surcharge: $\omega d_4 = 18.6 \times 4.00 \times 1.00 = 74.400 \text{ KN/m}$

c) Seismic state

Horizontal load

superstructure : $Hd = 6270.0 \times 0.10$ ----627.0 KN $: Md = 627.0 \times 1.20 / 17.00$ = 44.259 кмм/т beam $: Hd_1 = 18.880 \times 0.10$ 1.888 KNm/m == pillar : $Hd_2 = 15.104 \times 0.10$ 1.511 KNM/m ≕ footing $: Hd_{s} = 94.400 \times 0.10$ 9.440 KNm/m =

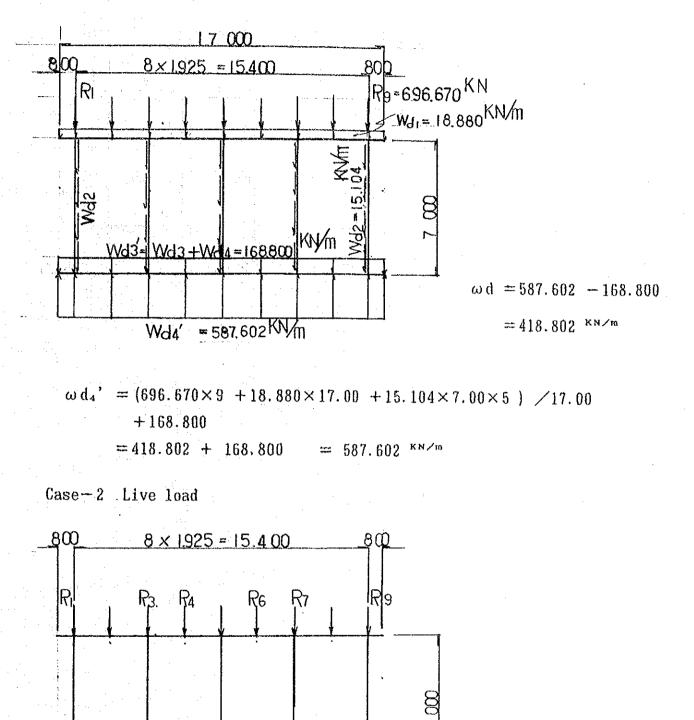
- d) Temperature
 - $T = \pm 12.5^{\circ}C$
 - $\alpha = 1.2 \times 10^{-5}$

 $Ec = 2.5 \times 10^{7} \text{ KN/m}^{2}$

MOMBASA — PIER

Loaded figure

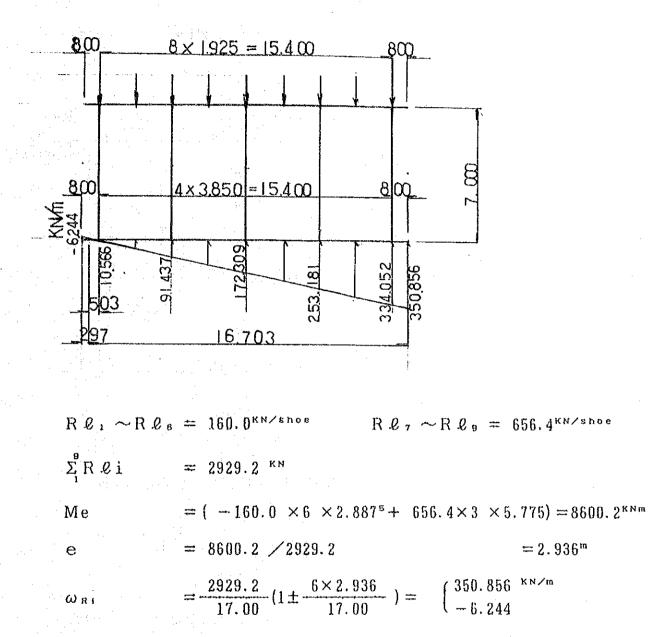
Case-1 Dead load

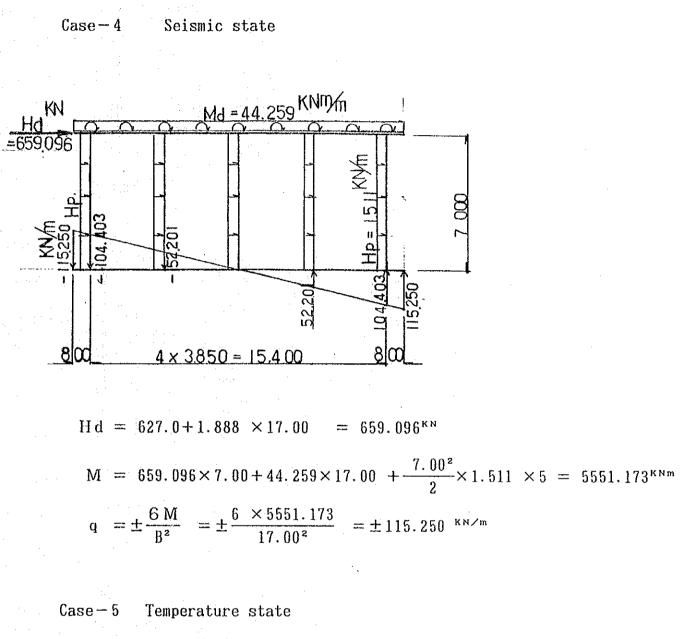


 $R \, \ell_{1 \sim 3} = R \, \ell_{7 \sim 9} = 160.0^{KN/shoe} \qquad R \, \ell_{4 \sim 6} = 656.4^{KN/shoe}$ $\omega_R = (160.0 \times 6 + 656.4 \times 3) / 17.00 = 172.306^{KN/m}$

WR = 172.306 KIVm

Case-3 Live load





 $t = 12.5 \ ^{\circ}C \qquad \alpha = 1.2 \times 10^{-5}$

 $Ec = 2.5 \times 10^7 \text{ KN/m}^2$

| | | • |
|-------------|----------|------------------------|
| NO | X
(m) | Y
(m) |
| | | C (11) y |
| 1 | 0.0000 | 7.0000 |
| 2 | 0.8000 | 7.0000 |
| 2
3
4 | 4.6500 | 7.0000 |
| | 8.5000 | 7.0000 |
| 5 | 12.3500 | 7.0000 |
| 6 | 16.2000 | 7.0000 |
| 7 | 17.0000 | 7,0000 |
| 8 | 0.0000 | 0.0000 |
| 9 | 0.8000 | 0.0000 |
| 10. | 4.6500 | 0.0000 |
| 11 | 8.5000 | 0.0000 |
| 12 | 12.3500 | 0.0000 |
| 13 | 16.2000 | 0.0000 |
| 14 | 17.0000 | 0.0000 |

NOTE: THE DIMENSION(t) BE EXCHANG TO DIMENSION(KN)INTO THIS CALCULATION

| 1 1 1 L | | | | | | | |
|---|--|--|--|---|---|--|---|
| No | I J | A (m2) | l (m4) | l ~ J | L (m) | E (t/m2) | EPS |
|
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4
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9
10
11
12
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15
16
17 | 1 - 2 $2 - 3$ $3 - 4$ $4 - 5$ $5 - 6$ $6 - 7$ $8 - 9$ $9 - 10$ $10 - 11$ $11 - 12$ $12 - 13$ $13 - 14$ $2 - 9$ $3 - 10$ $4 - 11$ $5 - 12$ $6 - 13$ | $\begin{array}{c} 0.80000\\ 0.80000\\ 0.80000\\ 0.80000\\ 0.80000\\ 4.00000\\ 4.00000\\ 4.00000\\ 4.00000\\ 4.00000\\ 4.00000\\ 4.00000\\ 0.64000\\ 0.64000\\ 0.64000\\ 0.64000\\ 0.64000\\ 0.64000\\ 0.64000\\ \end{array}$ | $\begin{array}{c} 0.066670\\ 0.066670\\ 0.066670\\ 0.066670\\ 0.066670\\ 0.33330\\ 0.33330\\ 0.33330\\ 0.33330\\ 0.333330\\ 0.333330\\ 0.333330\\ 0.333330\\ 0.333330\\ 0.334130\\ 0.0$ | $\begin{array}{rcrr} Fix & - & Fix\\ Fix & - & F$ | 0.800
3.850
3.850
3.850
0.800
3.850
3.850
3.850
3.850
0.800
7.000
7.000
7.000
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7.000 | 2.50E+07
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2.50E+07
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2.50E+07
2.50E+07
2.50E+07
2.50E+07 | 1.20E-05 |
| | | | | | | | |

| No | X (t/m) | Y (t∕m) | M(tm/Rad) |
|----|---------|---------|-----------|
| 8 | Fix | Fix | Free |
| 14 | Free | Fix | Free |

| | | dia kara | | | | | | | | | | |
|-------|-------------|----------------|-------|-------|-------|------|------|------|------|------|------|------|
| 1917 | No | | L-No | L-No | L-No | L-No | L-No | L-No | L-No | L-No | L-No | L-No |
| | | · · · · | . 1 | 2 | 3 | - 4 | 5 | 6 | 7 | 8 | 9 | 10 |
| | | | 11 | 12 | 13 | 14 | 15 | 16 | 17 | 18 | 19 | 20 |
| | 1
2
3 | . ° 0 · | | | | | | | | | | |
| . • | 2 | 3 🖯 | 0.963 | 1.925 | 2.888 | • | | | | | | |
| | 3 | 3 | 0.963 | 1.925 | 2.888 | | | | | | | |
| | 4 | 3 | 0.963 | 1.925 | 2.888 | | | | | | | |
| | 5 | 3 | 0.963 | 1.925 | 2.888 | | | | | | | |
| • | 6
7
8 | 0 | | | | | | | | | | |
| | 8 | 3 | 0.963 | 1.925 | 2.888 | | | | | | | |
| | 9 | 3 | 0.963 | 1.925 | 2.888 | | | | | | | |
| | 10 | 3 | 0.963 | 1,925 | 2.888 | | | | | | | |
| · · · | 11 | 3 | 0.963 | 1.925 | 2.888 | | | | | | | |
| | 12
13 | 0 | | | | | | | | | | |
| | 13 | 3 | 1.750 | 3.500 | 5.250 | | | | | | | |
| | 14 | .: 3 | 1.750 | 3.500 | 5.250 | - | | | | | | |
| | 15 | 3 | 1.750 | 3.500 | 5.250 | | | | | | | |
| | 16 | 3 | 1.750 | 3.500 | 5.250 | | | | | | | |
| ÷., | 17 | 3 | 1.750 | 3.500 | 5.250 | | | | | | | |

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| | : | Dead | Load |
|-----|---|------|------|
| No. | : | 1 | |
| No. | : | 1 | |

| No | i - j | | • | Li (m) | Lo (m) | Pi (t∕m) | Pj (t∕m) |
|-------------|---------|---|-----|-----------|--------|----------|----------|
| 2
2
3 | 2- 3 | | - Y | 0.001 | | -696,670 | |
| 2 | 2-3 | • | Y | 1,925 | | -696.670 | , |
| 3 | 3-4 | | - Y | 0.001 | | | |
| 3 | 3-4 | | ~ Y | 1.925 | | -696.670 | |
| 4 | 4~ 5 | | - Ÿ | 0.001 | | -696.670 | |
| 4 | 4- 5 | | - Ÿ | 1.925 | | -696.670 | |
| . 4 | 4- 5 | | - Ÿ | 3.849 | | -696.670 | |
| 5 | 5- 6 | | ~ Ŷ | 1.925 | | -696.670 | |
| 5 | 5~ 6 | | Ŷ | 3.849 | | -696.670 | |
| - 1 | 1~ 2 | | -Ŷ | | 0.800 | -696,670 | |
| 2
3 | 2- 3 | | -Ŷ | 0.000 | 3.850 | -18.880 | -18.880 |
| 3 | 3-4 | | - Ŷ | 0.000 | 3.850 | -18.880 | -18.880 |
| 4 | 4- 5 | | - Ŷ | 0.000 | 3.850 | ~18.880 | -18.880 |
| 5 | 5- 6 | | - Ŷ | 0.000 | 3.850 | -18.880 | -18.880 |
| 6 | 6-7 | | - Ý | 0.000 | 0.800 | -18.880 | -18.880 |
| 13 | 2-9 | | - Y | 0.000 | 7.000 | -18.880 | -18.880 |
| 14 | 3-10 | | ~ Ŷ | 0.000 | | -15.104 | -15.104 |
| 15 | 4- 11 | | ~Ŷ | 0.000 | 7.000 | -15.104 | -15.104 |
| 16 | 5-12 | | - Ŷ | 0.000 | 7.000 | -15.104 | -15.104 |
| . 17 | 6~ 13 | | -Ý | 0.000 | 7.000 | -15.104 | -15.104 |
| 7 | 8- 9 | | ·-Υ | 0.000 | 7.000 | -15.104 | -15.104 |
| 8 | 9-10 | | - Y | 0,000 | 0.800 | 418.802 | 418.802 |
| ğ | 10-11 | | -Y | 0.000 | 3.850 | 418.802 | 418.802 |
| 10 | 11- 12 | | Y | 0.000 | 3,850 | 418.802 | 418.802 |
| 11 | 12 - 13 | | - Y | | 3.850 | 418.802 | 418.802 |
| 12 | 13- 14 | 1. A. A. A. A. A. A. A. A. A. A. A. A. A. | - Y | 0.000 | 3.850 | 418.802 | 418.802 |
| | 10-14 | | - 1 | 0.000 | 0.800 | 418.802 | 418.802 |
| | | ΣV = | | 0.004 (t) | | | |
| | | Σ́н = | | 0.000(t) | | | |

MOMBASA-PIER(F)

: Live Load No. : 2 No. : 1

| + | 1917 - 19 | | | | | | | | |
|-------------|--|----------|---|-----|------------------|-----------|--------|----------|----------|
| 10 | i - | j. | | | | Li (m) | Lo (m) | Pi (t∕m) | Pj (l∕m) |
| 2
2
3 | 2- | : 3 | | | . - Y | 0.001 | | -160.000 | |
| 2 | 2- | 3 | | | Y | 1.925 | | -160.000 | |
| 3 | · 3- | 4 | | | - Y | 0.001 | | -160.000 | |
| 3 | - 3- | :4 | | | Y | 1.925 | | -656.400 | |
| 4 | 4- | 5 | | | - Y | 0.001 | | -656.400 | |
| 4 | 4- | 5 | | | - Y | 1.925 | | ~656,400 | |
| 4 | 4 | 5 | | | - Y | 3.849 | | -160.000 | |
| 5 | 5 | - 6 | | | - Y | 1.925 | | -160.000 | |
| - 6 | 5- | 6 | | | - Y | 3.849 | | -160.000 | |
| 7 | . 8- | . 9 | 1 | | - Y | 0.000 | 0.800 | 172.306 | 172.306 |
| 8 | · g- | 10 | | | ~ Ý | 0.000 | 3.850 | 172.306 | 172.306 |
| 9 | | | | | <sup>1</sup> – Y | 0.000 | 3.850 | 172.306 | 172.306 |
| 10 | | 12 | | | - Y | 0.000 | 3.850 | 172.306 | 172.306 |
| 11 | 2 | 13 | | | - Ÿ | 0.000 | 3.850 | 172.306 | 172.306 |
| 12 | 13- | | | | Y | 0.000 | 0.800 | 172.306 | 172.306 |
| | -
1911 - 201 | | | | | | | | |
| | | | | ΣΥ= | | 0.002 (1) |) | | |

0.002 (t) 0.000 (t)

| - , | | No.
No. | : Live
: 3
: 1 | Load | ÷., | | |
|---|--|------------|---|---|---|---|---|
| | | . • | · · · | | | | |
| No | ij | | | Li (m) | Lo (m) | Pi (t∕m) | Pj (t∕m) |
| 2
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10
11
12 | $\begin{array}{cccccccccccccccccccccccccccccccccccc$ | | -
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-
- | Y 0.001 Y 1.925 Y 0.001 Y 1.925 Y 0.001 Y 1.925 Y 0.001 Y 1.925 Y 0.001 Y 1.925 Y 0.001 Y 0.001 Y 0.000 >0.503
3.850
3.850
3.850
3.850
3.850
3.850
0.800 | $\begin{array}{c} -160.000\\ -160.000\\ -160.000\\ -160.000\\ -160.000\\ -656.400\\ -656.400\\ -656.400\\ -656.400\\ -656.400\\ -656.400\\ -6.244\\ 0.000\\ 10.566\\ 0.000\\ 91.437\\ 172.309\\ 253.181\\ 334.052\end{array}$ | $\begin{array}{c} 0.000\\ 10.566\\ 91.437\\ 0.000\\ 172.309\\ 253.181\\ 334.052\\ 350.856\end{array}$ |
|
 | | | V ≑
H ≂ | 0.052 (1 | | | |

MOMBASA-PIER(F)

1

: Seismic State : 4 : 1 No. No.

| X (t) | ¥ (t) | M (tm) |
|---------|-------|--------|
| 659.096 | 0.000 | 0.000 |

| No i -j | | Li (m) | Lo (m) | Pi (t∕m) | Pj (t∕m) |
|---|------------|--------|----------------|--------------|-----------------|
| 1 1- 2 | - M | 0.000 | 0.800 | 44.259 | 44.259 |
| 2 2 - 3 | - M | 0.000 | 3.850 | 44.259 | 44.259 |
| 2 2- 3
3 3- 4 | - M | 0.000 | 3.850 | 44.259 | 44.259 |
| 4 4-5 | - M | 0.000 | 3.850 | 44.259 | 44.259 |
| 5 5-6 | - M | 0.000 | 3,850 | 44.259 | 44.259 |
| 6 6- 7 | - M | 0.000 | 0.800 | 44.259 | 44.259 |
| 13 2-9 | ~X | 0.000 | 7.000 | 1.511 | 1.511 |
| 14 3-10 | -X | 0.000 | 7,000 | 1.511 | 1.511 |
| 15 4-11 | - X | 0.000 | 7.000 | 1.511 | 1.511 |
| 16 5-12 | -X | 0.000 | 7.000 | 1.511 | 1.511 |
| 17 6-13 | -X | 0.000 | 7.000 | 1.511 | 1.511 |
| 7 8- 9 | - Y | 0.000 | 0.800 | -115.250 | -104.403 |
| 8 9-10 | - Ÿ | 0.000 | 3.850 | -104.403 | -52.201 |
| | | 0.000 | 3.850 | -52,201 | 0.000 |
| | - Y | 0.000 | 3.850 | 0.000 | 52.201 |
| | - Y | 0.000 | 3.850 | 52.201 | 104.403 |
| 12 13-14 | - Y | 0.000 | 0.800 | 104.403 | 115.250 |
| 9 10- 11
10 11- 12
11 12- 13
12 13- 14 | - Y
- Y | 0.000 | 3.850
3.850 | 0.000 52.201 | $52.2 \\ 104.4$ |

ΣV = ΣH =

0.000 (t) 711.981 (t)

| | | No.
No. | : Tem)
: 5
: 1 | perature | |
|----|-------|------------|----------------------|----------------|------------|
| No | το Νο | т | (°C) | • | |
| 1 | - 6 | · ·] | 12.50 | | |
| | | | ΣV =
ΣH = | 0.000
0.000 | (1)
(1) |

| No | C-No 1 | C-No 2 | C-No 3 | C-No 4 | C-No 5 | C-No 6 | C-No 7 | C-No 8 |
|--------------------------------------|---|--|---|--|--|---|---|--|
| No | No 6 | No 7 | No 8 | No 9 | No10 | Noll | No12 | No13 |
| α | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 | 1.0000 |
| NO 1
NO 2
NO 3
NO 4
NO 5 | $\begin{array}{c} 1.3800\\ 1.6500\\ 0.0000\\ 0.0000\\ 0.0000\\ 0.0000\end{array}$ | 1.3800
0.0000
1.6500
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0.0000 | $\begin{array}{c} 1.3800 \\ 0.0000 \\ 0.0000 \\ 1.6500 \\ 0.0000 \end{array}$ | 1.3800
1.3800
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1.4300 | 1.3800
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1.3800
0.0000
1.4300 | $\begin{array}{c} 1.0000\\ 1.0000\\ 0.0000\\ 0.0000\\ 0.0000\\ 0.0000\end{array}$ | $\begin{array}{c} 1.0000\\ 0.0000\\ 1.0000\\ 0.0000\\ 0.0000\\ 0.0000\end{array}$ | 1.0000
0.0000
0.0000
1.0000
0.0000 |

| · No |) | C-No 9 | C-No10 |
|------|---|--------|--------|
| No |) | No14 | No15 |
| : | α | 1.0000 | 1.0000 |
| No | 1 | 1.0000 | 1.0000 |
| No | 2 | 1.0000 | 0.0000 |
| No | 3 | 0.0000 | 1.0000 |
| No | 4 | 0.0000 | 0.0000 |
| No | 5 | 1,0000 | 1.0000 |

No 1 : 6 7 8 9 10 No 2 : 11 12 13 14 15

| RM (tm) | 0.000 | RM (tm) | 0.000 | RM (tm) | 0000.0 | RM (tm) | 0.000 | RM (tm) | 0.000 |
|-------------------|-----------------|-------------------|------------------------------|-------------------|---------------------|--------------------|-----------------|--------------------|-------------------|
| 3
RY (t) | -0.136 | 6
RY (t) | -0.125
0.116 | 9
RY (t) | -0.114
0.106 | 12
RY (t) | -0.179
0.123 | 15
RY (t) | -0.179 |
| Case.
RX (t) | 0.000 | Case.
RX (t) | 0.000 | Case.
RX (t) | 0.000 | Case.
RX (t) | 0.000 | Case.
RX.(t) | 0.000 |
| RM (tm) | 0000.00 | RM (tm) | 0000.0 | RM (tm) | 0.000 | RM (tm) | 0.000 | RM (tm) | 0.000 |
| 2
RY (t) | -0.040 | 5
RY (t) | 0.000 | 8
RY (t) | 145.998
-146.003 | 11
RY (ţ) | -0.083
0.077 | 14
RY (t) | -0.083 |
| Case.
RX (t) | 0.000 | Case.
RX (t) | 000000 | Case.
RX (t) | -1174.770
0.000 | Case.
RX (t) | 0.000 | Case.
RX (t) | 0.000 |
| RM (tm) | 0.000 | RM (tm) | 0000.0 | RM (tm) | 0.000 | RM (tm) | 000.0 | RM (tm) | 0.000 |
| RY (t) | -0.043
0.039 | RY (t) | 88.519
188.519
188.519 | RY (t) | -0.283
0.192 | RY (t) | -0.247
0.169 | RY (t) | 88.476
-88.480 |
| Case. 1
RX (t) | 0.000 | Case. 4
RX (t) | -711.981 | Case. 7
RX (t) | 0.000 | Case. 10
RX (t) | 0.000 | Case. 13
RX (t) | -711.981
0.000 |
| N | 33
14
1 | No. | 7
7
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7 | . No. | н
8. | No. | | . o. | 14.
14. |

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1-164

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| ROTA. (mmRad) | 0.1065
0.1065
0.1682
0.1682
-0.0280
-0.0280
0.0720
0.0720 | 0.227 | ROTA (mmRad) | | ROTA. (mmRad)
-0.4369
-0.4365
-0.3165
-0.3167
0.4383
0.4383
0.4383
-0.007
-0.0323
-0.0323
-0.0323
-0.0323
-0.0323
-0.0325
-0.0325 | 040 |
|---------------------|--|--|---------------------|--|---|------------------|
| . 3
Y-DIS.(mm) | 0.02742
0.12742
0.87179
0.87179
1.265283
0.556283
-0.158693
-0.18872
-0.131111 | 1823 | . Y-DIS. (mm) | -0.48825
-1.02259
-1.02259
-4.32355
-4.32355
-1.02274
-2.5554
-1.02274
-1.5554
-1.5554
-1.55554
-1.25557
-0.09257
-0.09257 | Y-DIS.(mm)
-0.48489
-0.48489
-2.05971
-2.05971
-2.056538
-2.056538
-2.056538
-2.056638
-2.05971
-2.05971
-2.05971
-2.05071
-2.05094
-0.00000
-0.00000
-0.71978
-0.71978
-0.71978 | ~~~~ |
| Case
X-DIS.(mm) | -0.02525
-0.012525
-0.01266
-0.01266
-0.00508
-0.01146
0.01146 | 001300000000000000000000000000000000000 | Case
X-DIS.(mm) | 0.06391
0.05391
0.04612
0.01086
-0.04221
-0.04221
-0.04221
0.00000
0.00000
0.001061
0.01061
0.02122
0.02122 | <pre>Case.
X-DIS.(mm)
-1.75974
-1.75974
-0.78091
0.01077
0.80267
1.78127
0.00000
0.00000
0.00000
0.01055
0.01055
0.01055
0.01055</pre> | 4
4
3
2 |
| ROTA.(mmRad) | -0.27177
-0.27177
-0.37702
-0.37702
-0.37710
0.377179
0.27179
0.27179
0.27179 | 2897
2874
2874
2874
2898
2998
2916 | ROTA. (mmRad) | 0.10998
0.10998
0.05318
0.05318
0.05318
0.05318
0.08627
0.08627
0.05318
0.05318
0.05318
0.05318 | ROTA.(mmRad)
-0.66672
-0.66672
-0.668207
-0.01115
0.01115
-0.01115
-0.01115
-0.01115
-0.015283
-0.15743
-0.05743
-0.49558
-0.49558
-0.49537 | |
| 2
Y-DIS. (mm) | -0.13773
-0.35515
-1.71670
-2.65706
-1.71695
-0.35520
0.00000 | 2328
4525
4525
4527
4527
2329
0000 | 5
Ү-DIS.(mm) | -0.02365
0.06433
0.34783
0.34783
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0.34783
0.34783
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0.00000000000000000000000000000000000 | Y-DIS.(mm) F
Y-DIS.(mm) F
-0.07086
-0.0381456
-0.33163
-0.33163
-0.33163
-0.33163
-0.33163
-0.33163
-0.33163
-0.33163
-0.33163
-0.33163
-0.33163
-0.04160
0.39476
0.39476
0.00000 | |
| Case.
X-DIS.(mm) | 0.04244
0.04244
0.031394
0.00714
1.0.01711
1.0.02817
0.02817
0.0000 | .0000
.0022
.0119
.0141 | Case.
X-DIS.(mm) | -1.26727
-1.14727
-0.57241
0.00129
0.57499
1.14984
1.14984
1.26984
0.00129
0.00000
0.00000
0.000253
0.001253
0.001253
0.001253 | Case.
X-DIS.(mm)
8.90952
8.90952
8.6662
8.55663
8.49308
8.49308
8.49308
8.49308
8.49308
8.46054
0.00000
0.0070940
0.07308
0.09721
0.09721 | |
| ROTA.(mmRad) | -0.15881
-0.15978
-0.09259
-0.15988
-0.15988
-0.15988
-0.15883
-0.175883 | 0-1769
0.0637
-0.0637
-0.1769
-0.1769 | ROTA. (mmRad) | -10.27124
-0.27974
-0.07068
-0.01008
-0.05647
-0.05647
-0.05647
-0.05647
-0.017768
-0.17768
-0.01688
-0.01688
-0.01882
-0.01882
-0.01882
-15239
-15239 | HOTA. (mmRad)
-0.04337
-0.04470
0.40533
-0.65570
0.55470
0.55442
0.55423
0.55442
0.55540
-0.55540
-0.55562 | |
| γ-DIS.(mm) | | 57531
57531
57512
57512
13896 | (mm) | 0.11523
11523
0.11523
0.10403
0.31519
0.31519
0.31519
0.31519
0.25952
0.14009
0.14009
0.14147
0.33082
0.33082
0.33082
0.33082 | Y-DIS.(mm)
-0.21575
-0.21575
-0.221575
-0.221575
-0.25039
2214427
2214427
-0.47399
-0.47399
22.449098
3.34616
2.47519
0.525526
0.00000 | |
| Case
(mm) | -0.00443
-0.00443
-0.00443
0.00067
0.002067
0.00309 | 000000000000000000000000000000000000000 | Case
(mm) | 5.40341
5.37705
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0.000000 | X-DIS.(mm)
Case.
-0.04777
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-0.035477
-0.035477
-0.03317
0.01583
0.02317
0.02317
0.02317
0.0226
-0.00769
-0.01292
-0.01419
-0.01419 | |
| No. | | 201023 | . o <i>N</i> | - 0 0 4 0 0 0 0 0 0 0 0 7 0 0 7 0 0 0 0 0 | N
0 10040000010004 | |

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MOMBASA-PIER(F)

| ROTA. (mmRad) | -0.0523 | 0.2508 | -0.4010
0.2318 | 0.2308 | | • | -0.0024 | • | • • | | ROTA.(mmRad) | 0.0577 | 0 0567 | 0.3140 | -0.0283 | -0.4542 | 0.1218 | 0.1209 | 0.4869 | 0.4912 | 0.3017 | -0.0024 | -0.2792 | -0.5185 | -0.5107 |
|---------------------|----------------------------------|--------------------|--------------------|----------|---------|---------|----------|-------|----------|---------------|----------------------|----------|----------|-------------|----------|---------|----------|----------|---------|----------|----------|----------|----------|-----------------|------------------|
| 12
Y-DIS.(mm) | -0.16171 | 1.30674 | 0.39038 | -0.32028 | 0.32136 | 1.60383 | 2.14401 | | 0.00000 | | I5
Y-DIS.(mm) | | -0.13930 | | | | -0.44081 | | | | | | | | |
| Casc.
X-DIS.(mm) | -0.02968
-0.02968 | -0.00575 | 0.01455 | 0.01455 | 0,00000 | \circ | -0.00478 | | 0 | | Case.
X-DIS.(nm). | -1.29694 | -1.17694 | | | 0.58564 | 1,16439 | 1.28439 | 0.00000 | 0.00000 | -0,00085 | -0.00350 | -0.00602 | -0.00627 | -0.00627 |
| ROTA.(mmRad) | -0.43058 | | | | | | | | | • | ROTA.(mmRad) | .3206 | 3219 | 2312 | 000 | 2314 | 0.32161 | 3206 | 0325 | 0265 | 1705 | 0000 | 1705 | 0266 | 0327 |
| 11
Y-DIS.(mm) | -0.32686
-0.67152
-1.85208 | | 101 | | o' | | 40 | | • | , | Y-DIS.(mm) | | ۳, | ~; | 7 | | -0.60729 | | Υ. | Υ. | 0 | 0 | 5 | 0 | 0 |
| Case.
X-DIS.(mm) | 0.03801
0.03801
0.02728 | | | | | | | | | 9.3
2
2 | (HH) SIQ-X | -1.22926 | -1.10926 | -0.54514 | 0.00776 | 0.56065 | 1.12477 | 1.24477 | 00000 | 0.00000 | 0.00268 | 0.00760 | 0.01252 | 0 01519 | 0.01519 |
| ROTA. (mmRad) | 0.08513
0.08379
0.43596 | $-\infty$ or | 0 16262 | -1-0 | 0.68212 | ን ሮን | 200 | 19 | 60 | | ROTA.(mmRad) | 4 | 4 | 0 | 8 | 0.14 | ò, | 0.08 | 5,0 | 8 | 90 | 2 | 40. | .32 | -0.32726 |
| 10
Y-DIS. (mm) | -0.25698
-0.18914
1.51241 | 2.44307
1.03613 | -0.60511 | 000000 | 5421 | 5972 | o o | 5693 | 0 | 13 | Y-DIS. (mm) | 0738 | .4204 | -0.54440 | -0.01585 | s a | -0.22958 | 80862.0- | 0.0000 | -0.00245 | 0.15270 | 0.04945 | 0.90594 | .26 | 0.00000 |
| Case.
X-DIS.(mm) | -1.85314
-1.68154
~0.84998 | -0.00609 | 1.66435
1 23505 | | 0.00000 | • • | -0.00820 | .0085 | -0.00852 | Case. | | 5.39898 | • | • | • | • | • | ٠ | • | • | • | • | ٠ | • | • |
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۱ | 9.6 | | | | 12. | | 14. | | . oN | | | <u>،</u> در | สาม | . v | | - a | . a | n c | • | • | • • • | •
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70.55 | -70.553 | | -67.347 | 67.34
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| Load S (t) | õõ | 59 30 | | -160.691 | 6 | 7.54 | 7 54 | 37.54 | 4 | 70.87 | 10.87 | 10.87 | -49 126 | 1
 | 1075.855 | 0 4 0 7
7 0 7 | 236.94 | 36.94 | 656.400
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1.594 | 57.71 | 137.80 | 8.45 | 00 n
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04 65 | 32.079
188.120 | 1 C
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U | -29.539 | 65.40
80.01 | 13.85 |
| Case 3 Livc
M <tm></tm> | | 04 87 | 03.70 | -51.044 | | 37 | 44 06 | | 80.00 | 258.03 | 51:03 | 44 43 | - 30 005 | | -327.478 | | 2 2 2 | 24.51 | -0.656
0.000 | 0.000 | 4.15 | -9.70 | 24.90
02.02 | 194. | -65.67 | 327.91
486.49 | -523.007
-418.650 | 100 71 | -460.143 | 477.54
216.49 | 260.00 |
| N (E) | ŏŏ | 57 40 | 57.40 | -57.403 | >.
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• | 125,95 | 125.95 | -125 959
-175 959 | 125.95 | 125:98 | 125.98 | 125.98 | -125.984 | | -57,415 | | 57.41 | 57.41 | 000000 | 00000 | 7.40 | 7.40 | 57.403 | 7.40 | 25.95
25.95 | 25.95 | 125.959 | 9 E 0 E | 125.984 | 25.98
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-563 434 | 1 | 40.519 | | 119.48 | 9.48 | 0.000 | -0.040
137.805 | 1.67 | 24.25 | 355.947 | 21.70 | 2.07 | 49.61 | 415.541
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-83.59 | 82.16 |
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1 | | 83.65 | 8.75 | 0.000 | 0.000
55.106 | 147.99 | 04.53 | -101.468 | 83.56 | 54.53 | 39.30
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334.999 | 8.73 | 355.42 | 47.463 | 3.65 | 25.70 | 466.33 | 383.799
786.686 | 786 59 | -383.250 | 19.59
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| ast 1 Dead
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-263 841 | 363.58 | 416.81 | 84.14
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341.713 | 11 64 | -221.653 | 96.58
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| -1.651 -147.986 141.559 57.415 215.51 4 -988.000 0.000 0.000 110.54 4 -1088.002 198.717 -57.403 -279.477 -104.87 4 -1093.730 -203.106 -57.403 -279.477 -104.87 4 -1093.730 -203.106 -57.403 -279.477 -14.79 4 -1093.730 -203.106 -57.403 -279.477 -14.79 4 -1093.730 -203.106 -57.403 -279.477 -14.79 4 -1093.730 -203.106 -57.403 -279.477 -14.77 5 -1667.298 -100.913 -68.556 -603.785 -62.216 3 -16573.630 250.886 -603.785 -125.25 -11467.554 14.74 3 -1657.554 -10.913 -68.556 -603.785 -61.61 -63.274 3 -1657.554 -10.025 -11652.514 -125.25 -11652.514 -2.43 -1257.415 -1263.568 </td <td>24.52</td> <td>51 -204.47</td> <td>5 -1.651</td> | 24.52 | 51 -204.47 | 5 -1.651 |
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| 4 -988.002 198.717 -57.403 -279.477 -104.87 4 -1004.366 -2.194 -57.403 -279.477 -45.03 4 -1003.730 -203.106 -57.403 -279.477 -45.03 3 -1573.630 250.859 -68.556 -603.785 -102.22225 3 -1650.052 130.886 -68.556 -603.785 -1125.25 3 -1673.303 250.859 -68.556 -603.785 -124.47 3 -1652.328 -1209.033 -68.556 -603.785 -125.25 3 -1652.328 -68.556 -603.785 -125.25 3 -1679.358 0.1110 -0.025 -1162.514 14.34 6 -1467.554 0.110 -0.025 -1162.514 -2.43 -2.43 6 -1467.554 -0.025 -1162.514 -2.43 -2.43 6 -1467.554 -0.025 -1162.514 -2.43 $-2.63.447$ 6 -1573.282 -0.025 <td>.000 -0.03</td> <td>00 00 00</td> <td>00.00</td> | .000 -0.03 | 00 00 00 | 00.00 |
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| 4 -1067.298 -102.650 -57.403 -279.477 74.62 3 -1573.630 257.403 -279.477 134.46 3 -1600.052 130.986 -68.556 -603.785 $-61.61.61.61$ 3 -1652.926 -109.060 -68.556 -603.785 $-61.61.61.61$ 3 -1679.358 -229.033 -68.556 -603.785 $-65.65.65$ 3 -1652.926 -109.060 -68.556 -603.785 $-61.61.61.61$ 3 -1679.358 -229.033 -68.556 -603.785 -129.23 5 -1467.554 0.1110 -0.025 -1162.514 14.34 6 -1579.358 -0.025 -1162.514 -3.73 6 -1573.798 -0.025 -1162.514 -3.73 6 -1573.282 -0.025 -1162.514 -2.736 6 -1573.282 -0.025 -1162.514 -2.736 6 -1573.282 -1162.514 $-2.79.431$ $-2.79.431$ 6 | .194 -57.403 - | 66 -2.19 | 4 -1040.86 |
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0 10 | 2.45 | 2.45 | ୍ | 000 0 | 824.58 | 1861.05 | -1897.532 | 1934.00 | 1970.48 | 67.85 | -3204.331 | 40.80 | 77.28 | 3313.75 | 943.37 | 3979.84 | -4016.325 | 4052.80 | 089.27 | 168.34 | 3204.82 | -3241.300 | 3277.77 | 3314.25 | 1824.59 | 1861.06 | 1897.54 | -1970.496 |
|-------------------------|----------|-------------------|--------|----------------|---------|--------|--------|---------|-----------|---------|---------|--------|-----------|--------|-------|---------|--------|---------|-----------|---------|--------|--------|---------|-----------|---------|---------|-------------|------------------|-----------------------|---------------------|
| S (t) | 2039.0 | -1208.741 | 451.09 | 80.57 | - G | | 92.4 | 92.41 | 41 | 92.41 | 92.41 | 90.74 | -90.743 | 0.74 | 0.74 | 0.74 | 0.07 | 0.07 | -0.076 | 0.07 | 0.07 | 0.78 | 0.78 | 90.782 | 0.78 | 0.78 | 2.45 | 2.45 | ທີ່
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| Case 5
M (tm) | 90 | 2.10 | 827.51 | 5.42 | ୍ | 0.00 | 76.49 | 4.75 | ເມ | 108.70 | 70.43 | 30.75 | 171.955 | 13.15 | 5.64 | 04.44 | 34 | .20 | 0.075 | 0.0 | 0.19 | 330.92 | 72.05 | -13.191 | 45.67 | 04.54 | 376.60 | 14.80 | 53.00
53.00 | 270.590 |
| N (t) | 13.744 | - 1-
. m | 3.74 | 3.74 | 80. | 0000 | 10:71 | 10.71 | -10.715 | 10.71 | 10.71 | .72 | 9.726 | .72 | .72 | . 72 | .97 | .97 | 1.978 | .97 | 16 | .72 | . 72 | 9.726 | . 72 | .72 | 0.71 | 10.71 | 12.01 | -10.715 |
| rrature
S (t) | 10.715 | 12.0 | 0.71 | 0.71 | 00 | 0.000 | 74 | 13.74 | -13.744 | 13.74 | 13.74 | 5.99 | -5.990 | 5.99 | 5.99 | 5.99 | 00. | 8.00 | 0.000 | 80, | 80. | 66. | .99 | 5.990 | , 99 | 66. | 3.74 | 4 | 4 L
2 C | 13.744 |
| Case 5 Temper
M (tm) | 26 | 1.01 | 61.30 | 50.99 | | | 5.21 | 1.16 | -2.889 | 6.94 | 50.99 | 0.96 | 10.483 | 0.00 | .48 | 20.96 | 001 | 00. | 0.000 | . 00 | 00. | 20.96 | .48 | °. | .48 | 0.96 | 5.21 | .16 | 82.2 | 50.992 |
| N (t) (| 114.402 | 1 4
1 4
1 4 | 14.4 | 14.4 | 0 | 0 | 5.58 | 5 58 | 85.584 | 5.53 | 5.58 | 3.13 | 33.133 | 3.13 | 3.13 | 3.13 | 0.2 | 0.26 | -0.264 | 0.26 | 0.26 | 35.73 | 35.73 | -35.731 | 35.73 | 35.73 | 82.72 | 82.72 | 22.28 | -82.722 |
| ic State
S (t) | -218.083 | -92.47 | 10.78 | 83.38 | 0.65 | 88.519 | 07.49 | 10.14 | 112.785 | 15.42 | 18.07 | 44.26 | 146.911 | 49.55 | 52.20 | 54.84 | 60.84 | 63.48 | 166.129 | 68.77 | 71.41 | 42.66 | 45.31 | 147.955 | 50.60 | 53.24 | 03.82 | 06.46 | 1160 | 114.402 |
| ase 4 Seism
M (tm) | 111.3 | 18.28 | 469.01 | 435.10 | -35.093 | 00. | 375.86 | : 43 | 9.629 | 31 | 13.63 | 508.13 | -253.353 | 6.05 | .09 | 38.75 | 3.17 | 279.39 | | •06 | 99.72 | 2.53 | 250,55 | φ | 9.5 | 33.15 | 3.78 | 179.77 | 8.85
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L(m) | 0.000 | 200 | . 88 | 85 | 0.000 | .80 | 00. | . 73 | 3.500 | . 23 | 00. | .00 | 1.750 | . 50 | . 25 | 80. | .00 | . 75 | 3.500 | . 25 | 00. | 00. | . 75 | 3.500 | . 25 | 00. | .00 | 5
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| No | L(m) | Case 7
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218.517 | 293.005
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-272.590
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192.755
197.118 | 260.415
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M (tm) | 318.510
-851.322
-1385.110
-1264.003
-468.316 | 127.083
-571.083
-571.568
-253.334
72.534
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-462.039
479.955
962.404
-929.077
-460.895 | 98.00
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-2835.645
-2872.122
-2908.598
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-27470.700 | 2783.65
2820.12
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| S (t) | - 2021.512
- 1046.586
- 40.5886
- 40.581
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- 2068.782 | - 914.589
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-15.104 | 1132.206
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-414.965
-1129.798 | 1568.982
894.400
876.238
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-1209.846 | 671.504
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336.593 | +916,445
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-50.993
411.168
892.298 | -1145.293
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974.806 | -991.809
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| | Case 12
M (tm) | 0.000
-6.042 | 8331,052
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M (tm) | 0.000
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