REPUBLIC OF KENYA



MINISTRY OF PUBLIC WORKS

DETAILED DESIGN STUDY ON

THE NAIROBI BYPASS PROJECT

STRUCTURAL CALCULATIONS
COMPUTOR OUTPUT

VOL-1

SEPTEMBER 1992



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Japan International Cooperation Agency The Permanent Secretary Ministry of Public Works P.O.Box 30260 NAIROBI

The Chief Engineer (Roads) Ministry of Public Works P.O.Box 30260 NAIROBI 国際協力事業団 24837

DESIGN CALCULATION

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- 1. BRIDGES ON MAIN ROAD
 - 2. OVERBRIDGES
 - 3. BOX CULVERTS

CALCULATION OF

BRIDGES FOR MAIN ROAD

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1. General

This document has been prepared for the design of structures, which includes road bridges, a railway bridge, foot bridges, and box culverts, for the Nairobi Bypass project in the Republic of Kenya. All structures will be constructed of structural type RC Reinforced concrete having been planned on the basis of material availability analyses of procurement within Kenya.

2. Number and Positions of Structures Designed

2.1 Numbers of Structures Designed

Bridges: 2 road bridges, 2 road overbridges, 1 railway bridge, and 2 foot bridges.

Boxes: 7 road boxes, 4 pedestrian boxes, and 5 waterway boxes.

2.2 Locations

See the separate sheets (APPENDIX-1).

3. Scale and Configuration of Structures

3.1 Bridge Frames

a. Spans

Basic structure which is of reinforced concrete will have no fixed bridge span sizes. There sizes' will vary based on the design conditions of individual bridges. Longest spans of all bridges will however be less than 20.0 meters except the center span of the Mombassa Bridge which is 21.5 meters.

b. Configurations

Foot bridges:

Stair case loads will be supported by a cantilever arm or (Gerber beam) system. The super structure of the bridge will be comprised of a continuous floor slab, thus having no over hangs.

Other bridges:

In order to provide the best economic solution a continuous T beam construction method will be used effectively reducing loads on the substructure.

c. Substructure

Abutments:

Because abutment heights are less than 12.0m, abutment designs will be of the invert T type supported by individual spread footings.

Piers:

As the Mombassa and Uhuru Bridges are board, multistaged rigid-framed structures piers will be used in the substructure. For other bridge frame and wall types a rectangular single-pillar type support system will be for direct foundation. (However, the No. 2 road overbridge will be the only bridge to use a pile foundation of steel pipes in view of existing geologic conditions.)

3.2 Box Culverts

Box culverts will generally be placed on existing ground surfaces due to favorable hard ground therefore needing no pile foundation or other support. However, as the bottom ground for the No. 5 waterway box culvert is like a pond having silty clay and unproductive plant material the bottom ground will be replaced by quality materials such as crushed or broken stones having a depth of 0.3 to 0.5m.

4. Outline of Geologic Substructure and foundation

This section briefly describes specific geologic substructure and foundation support material types. For details, see the "Report of Geologic Grounds".

The geologic substructure for placing the Mombassa Bridge will accommodate a foundation directly supported on a geologic substruction. The initial reasoning behind this is a cotton soil layer of a 1.0m depth is distributed over a lower layer of weathered rock. Foundation footings can be placed directly on weathered rock layer off eliminating the need for support piers. In addition, the Uhuru Bridge, will also use a direct foundation on weathered rock which is convered by a 1.0m layer of red soil.

Further, foundations positions where railway bridges are placed will use the direct foundation supported on weather rock once as there is red soil distributed as a whole and the bottom for the foundation has N > 40. The placement of foundations for the No. 1 Main Road Overbridge uses the same direct foundation method as it's red soil layer as deep as 5.0m, but it can easily be set on the weathered rock as its bypass is cut to the weathered rock layer. The position for the No. 2 Main Road Overbridge uses a pile foundation for its support structures only as it has red soil distributed as deep as around 5.0m, and the lower layer has severely weathered rock distribution from 5.0m to 9.5m deep with N < 24. The position for the No. 1 Foot Bridge, such as the Mombassa Bridge also uses the direct supported foundation method. For the position for the No. 2 Foot Bridge, it is sufficient to use the direct foundation as it is similar to the positions for the railway bridges having hard red soil with N > 30.

All the box culverts use the direct foundation for the reason described in section 3.2.

5. Design Conditions

- 5.1 Unit Volume Weights of Materials Used (see the Design Agreement or APPENDIX-2)
- 5.2 Strength Characteristics of Concrete and Reinforcing Bar (see the Design Agreement or APPENDIX-3)
- 5.3 Design Stresses of Concrete and Reinforcing Bars

Design are made in view of two critical states, which include the final and working states, of which structures should be made in the critical states according to the BS5400 code.

Design Stresses

		S.L.S.	U.L.S.
Compression stress o		0.50 fcu	0.40 fcu
Stress of Steel	Tension side	0.80 fy	0.87 fy
Bar	Compression side	0.80 fy	0.72 fy

Where

SLS: serviceability limit state

ULS: ultimate limit state

fcu: Strength of Concrete

fy: Strength of Steel Bar

And

The shearing stresses of the concrete in the ultimate limit state are shown in Table below.

Shearing Stress of Concrete

<u>100 A.</u>	<u> </u>	Grade o	f Concrete	
bd	20	25	30	≥ 40
≤ 0.25	0.35	0.35	0.35	0.35
0.50	0.45	0.50	0.55	0.55
1.00	0.60	0.65	0.70	0.75
2.00	0.80	0.85	0.90	0.95
≥ 3.00	0.85	0.90	0.95	1.00

 (N/mm^2)

5.4 Earth Pressure Formulas and Earth Pressure Coefficients (see the Design Agreement)

a. Earth pressure formulas:

Two formulas, including the Ranking's formula and Coulomb's formula, are used, to calculate earth pressure. The Coulomb's earth pressure formula however is used in the design of movable walls with the walls with the wall surface friction angle of the wall surface in contact with soil being clearly seen. The static earth pressure formula is used for the design of fixed wall construction.

b. Earth pressure coefficients and internal friction angle of back filling soil.

Inclination of Back Soil	Internal Friction Angle	Earth Pressure Coefficient	Application
For Level Back Soil	ø = 35°	K=0.250 (Coulomb)	Abutment
For Incline Back	ø=30°	K=0.333	Retaining Wall
Soil		(Coulomb)	
	<u></u>	K=0.500	Box culvert
		(Static)	
	ø=30°	K=0.400	Wing wall
	φ-50	(Coulomb)	

NOTE 1: Abutments have seismic coefficient added to their design on the basis of the conditions given above.

NOTE 2: Wing walls at entrance points are made with use of the following equation.

$$K = \tan^2 (45^\circ - \phi/2) = \tan^2 30^\circ = 0.333 = 0.400$$

This equation is given for a "Design of Wing" section as seen in the Design Instruction published by the Japan Public Road Corporation. It also is used here in order to increase the sameness of the wing.

5.5 Earthquake Coefficient (design seismic coefficient): see the Design Agreement.

Railway bridges:

 $K_{\rm H} = 0.12$

Other bridges:

 $K_{\rm H} = 0.10$

5.6 Influence of Temperature Change: see the Design Agreement.

$$T = \pm 12.5$$
°C

6. Design Loads

a. Bridge frames: In general, these are designed according to Part 2, the Load Deviation, BS5400. As for railway bridges, these are further checked according to the (BS153) for sameness. b. Box culverts: In general, these are designed according to Part 2, the Load Deviation, BS5400.

7. Others

References are to the Design of Concrete Bridges section of the "New British Standard BS5400" L. A. Clark, National Science Co.

Type. Size and Location of Structure

(1) Bridges

Types of bridges, their location and their scale on Nairobi Bypass are as shown on the Table 9.1.

Table

Type	No.	Location	Length (m)	Width (m)
	1	Mombasa Road JC Bridge (Starting point of Bypass Road)	57.0	17. 0
For Road	2	Uhuru Konumeni JC Bridge (Crossing of C58 Road)	58.0	20. 5
	3	Railway Bridge Over Bypass in CH.27 + 20.0 m	56.85	11.4
Over	2	Over Bypass CH. 15 + 980.0 m	30. 1	6. 0
Bri-	4	- CH. 27 + 920. 0 m	28. 1	10.0
Foot	1	Over Bypass CH.1 + 180.0 m	38. 20	3. 0
Path	2	" CH. 27 + 220. 0 ₪	48.40	3. 0

(2) Box Culverts

Types, location and scale of Box Culverts are as follows:-

<u>Table</u>

Type	No.	Location	Length (m)	Width bxh (m)
	ı	Crossing of C60 Ngong Road CH. 15 + 540.0	32. 3	10. 0x5. 5
	2	Crossing of CH.19 + 500.0	37.5	8. 0x5. 5
F	3	Crossing of Rump for Dagoretti JC CH. 20 + 930.0	32. 0	10.0x5.5
r R	4	Crossing of D411 Thogoto Road CH. 23 + 193.0	26.7	8. 5x5. 5
o a d	5	Crossing of Public Road (E) CH. 23 + 169.4	26.7	8. 5x5. 5
	6	Crossing of Public Road CH. 24 + 980.0	25.5	8. 5x5. 5
	7	Crossing of D422 Ondiri Road CH.26 + 464.0	50. 2	8. 5x5. 5
F	1	Ruora River in Ngong Forest CH. 13 + 978.0	59.0	3. 0x3. 0
r ·	ş	Motoine River in Ngong Forest CH. 14 + 934.0	34.2	3. 0x2. 0
r a	3	Motolne River in Ngong Forest CH, 15 + 560.0	67.0	3.5x3.0
n 2	4	Motoine River in Ngong Forest CN.O + 157.0 (Ngong JC-Rump)	40.0	3.5x3.0
e e	5	Ondiri River CN. 26 + 355.0	132.0	3, 5x3, 5 (Double)
F	1	CH, 18 + 400.0	28. 0	3. 0x3. 0
0 P	2	CH, 20 + 200, 0	21.5	3. 0x3, 0
o a	3	CH. 22 + 880. 0	21.5	3. 0x3. 0
h i	4	CH. 26 + 220, 0	22. 1	3. 0x3. 0

APPENDIX - 2 Unit volume of materials

Dead load (Unterial wright of unit)

	Mierial	Meight of unit	Reportes
	Reinforced	23. 63H/US + 2. 40T/UP	
Concrete	· Soa-Rein- Forced	22. GRH/U3 - 2. 30 T/V*	
Pavement of asphalt	asphalt	22, 5xx/44 + 2, 30:7/4"	
Steel and Bar	11	76. 9KK/AB × 7. 85.E/M	
	Mores I	18. 61H/N ³ + 1. 90.T/M*	
e 1 1	Duck Filling	19. GTH/A3 + 2. 30/7/4*	Motern

APPENDIX - 3
STRENGTH CHARACTERISTICS OF CONCRETE AND STEEL BAR

1), Cancrete

.2) Steet Bar

Characteristic Strongth		 -	Hot Rolled		Wild Steel	•	N/ccs Xg/co	250 • 2550		•
Character Strongth	·		Not Rolled		High Yeild		N/ms Kg/cm ²	410 - 4180		
U255 per metre ren	kg 0.222	0.395	0, 516	0,888	1, 579	2, 465	3, 854	6,313	60 50 50 50 50 50 50 50 50 50 50 50 50 50	15, 413
Cross Sectional Area		56,3	78.5	113,1	201, 1	314.2	490, 9	804.2	1256, 6	1363.5
Mominal Size	6. (ain)	8	01	12	91	20	25	33	40	50 (max)
							-	•		

Super Structure (Casting of Concrete)

.Reinforced Concrete

25 . 255

Super Structure(Post-Tension)

* 07

9

Super Structure (Pre-Tension

017 - 09

9

Super Sicucture(Pre-Tension)

concrete

. 510

8

Sub Structure(Abutment)

Non-keinforced Base Concrete Concrete

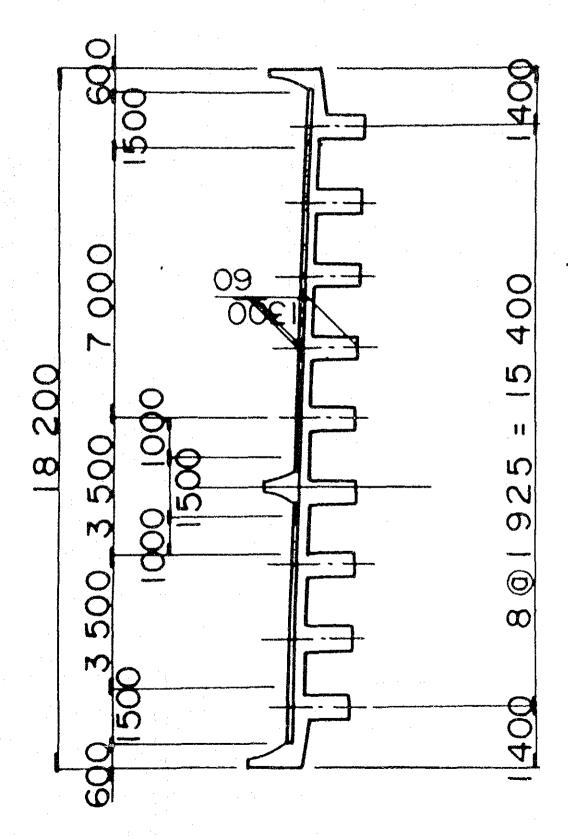
15 - 150

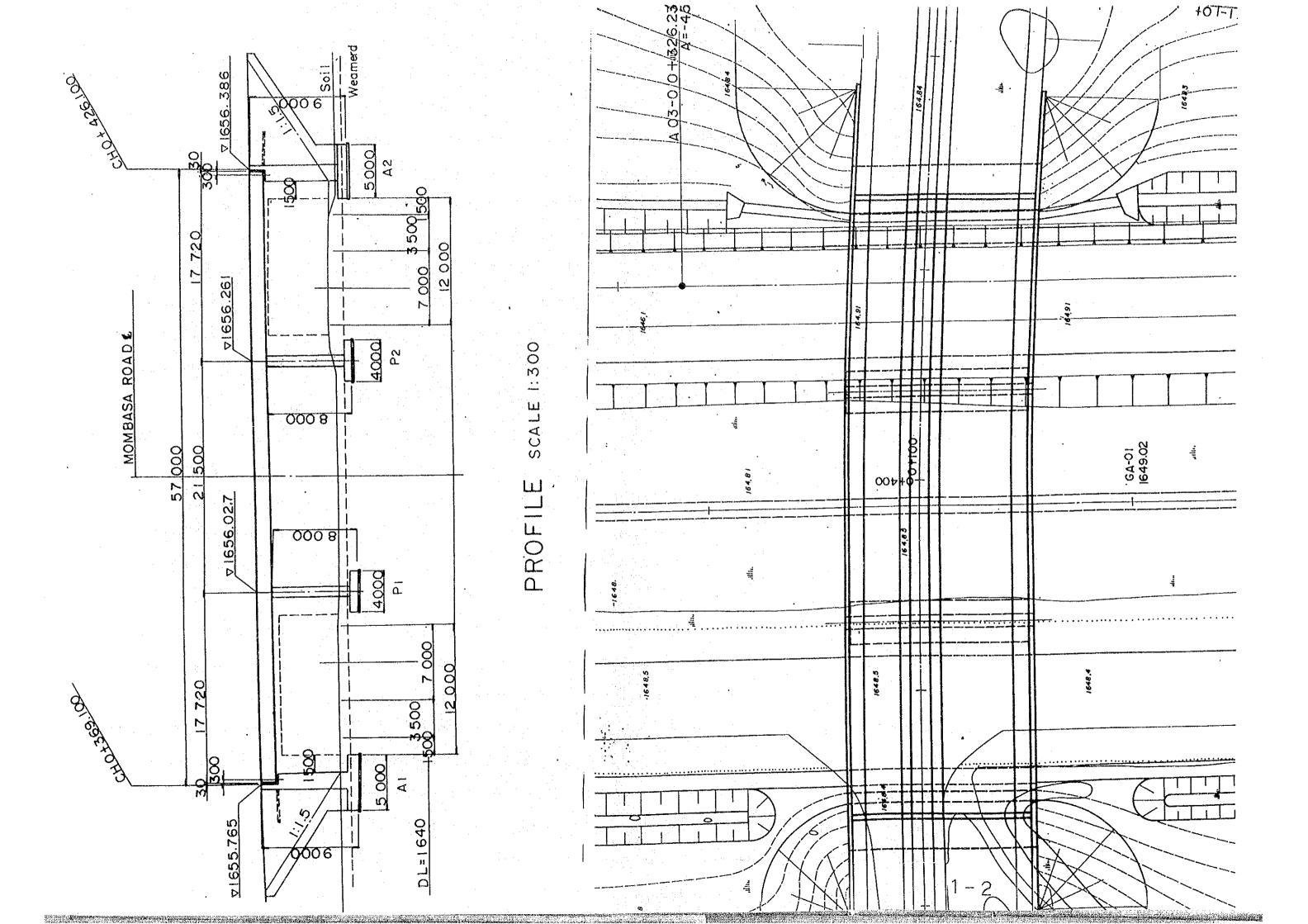
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Employment Diride

Characteristic Strength A/ma' (2/ca'

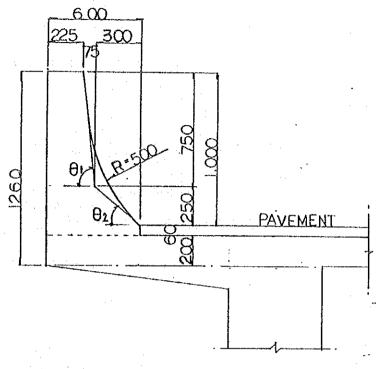
MCMBASA ROAD Ju. BRIDGE





SHAPE OF PARAPET OF MAIN ROAD

I) FOR VERGE



. Section arer.

$$A = 0.60 \times 1.06 - \frac{1}{2} \times 0.075 \times 0.75$$
$$- \frac{0.75 + 1.00}{2} \times 0.30 + 0.006$$
$$= 0.352 \text{ m}^2$$

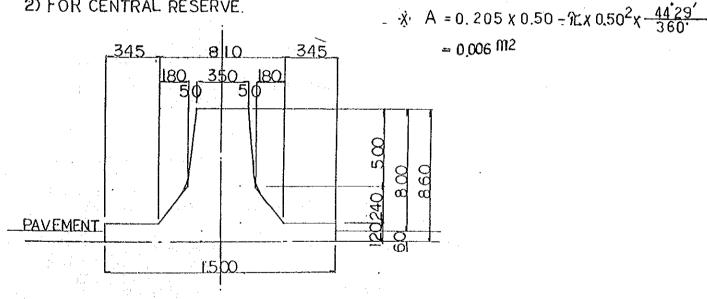
$$\theta_1 = \tan^{1} \frac{0.750}{0.075} = 84^{\circ} 17'22''$$

$$- |\theta_2 = \tan^{1} \frac{0.250}{0.300} = 39^{\circ} 48'20''$$

$$\theta = \pm 44^{\circ} 29'.$$

$$TL = R \cdot \tan \frac{\theta}{2} = 0.205^{\circ}$$

2) FOR CENTRAL RESERVE.



Section arer

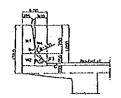
$$A = \frac{0.35 + 0.45}{2} \times 0.50 + \frac{0.45 + 0.81}{2} \times 0.24 + 1.50 \times 0.12$$
$$= 0.532 \, \text{m}^2$$

2. LOAD

2. 1 DEAD LOAD

Note Input data: unit=P', W' t, t/m unit≖P, W KN=P', W' *9.8m/s2

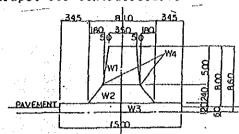
1) Parapet for verge



W1' = 1/2*(0.225+0.30)*0.75*2.4=0.473t/mW2' = 0.3*0.31*2.4\$W3' = 1/2 * (0.06 + 0.31) * 0.30 * 2.4 = 0.133=0.014W4' = 0.006 * 2.4 $\Sigma W' = 0.843 t/m$

W=0.843*9.8=8.26KN/m

Parapet for centralreserve



W1'=1/2*(0.350+0.45)*0.50*2.4=0.480t/m 42' = 1/2*(0.45+0.81)*0.24*2.4 = 0.363\(\frac{1}{3}\) = 1.5*0.12*2.4 =0.432=0.003W4' = 0.0007 * 2 * 2.4 $\Sigma W' = 1.278 t/m$

W=1.278*9.8=12.52KN/m

Payment

W=0.06*2.3*9.8=1.35KN/m2

W' = 0.06 * 2.3 = 0.138 t/m2

Main girder

end girder W1'=2.3625*0.20*2.4 W2'=1/2*1.10*0.15*2.4 =1.134 t/m=0.363 W3'=0.6*1.10*2.4 =1.534 $\Sigma W' = 2.916 t/m$

W=2.916*9.8=28.58KN/m

Middle gider W=(1.8*0.2+1.1*0.6)*2.4*9.8=24.58KN/m W' = 24.58/9.8 = 2.508 t/m

6) Cross girder End cross girder and sopportinggirder P1=0.6*0.95*1.325*2.4*9.8=17.76KN W = 17.76/9.8 = 1.813tP2=0. 35*0. 95*1. 325*2. 4*9. 8=10. 36KN

% W'=10.36/9.8=1.057t

2. 2 LIVE LOAD

1) H A 1 (center of span)

KEL main P1=40KN/m sub P2=12.630KN/m ※ P1'=40/9.8=4.082t/m P2'=12.63/9.8=1.289t/m

sub #2=3.16KN/m2

UDL main W1=10KN/m2 sub W2=3.16KN/m2 W1'=10/9.8=1.02t/m2 W2'=3.16/9.8=0.322t/m2

2) H A 2 (middle supprting point)

sub P2=12.630KN/m KEL main P1=40KN/m

 $\Re P1' = 40/9.8 = 4.082 t/m P2' = 12.63/9.8 = 1.289 t/m$

sub W2=2.79KN/m2 UDL main W1=8.84KN/m2

※ ¥1'=8.84/9.8=0.902t/m2
¥2'=2.79/9.8=0.285t/m2

L=17.425+21.5=38.925m

 $W=151*(1/L)^0.475=151*(1/38.925)^0.475=26.5KN/m$

W=26.5/3.0m=8.84KN/m2

```
3) H B 1 (center of span)
        HB P0=75KN \frac{1}{2} P0'=75/9.8=7.653t/m
        KEL main P1=40KN/m sub P2=12.630KN/m

    P1' =40/9.8=4.082t/m P2' =12.63/9.8=1.289t/m

        UDL main W1=10KN/m2 sub W2=3.16KN/m2
               W1'=10/9.8=1.02t/m2 W2'=3.16/9.8=0.322t/m2

※ P1'=40/9.8=4.082t/m P2'=12.63/9.8=1.289t/m
UDL main W1=8.84KN/m2 sub W2=2.79KN/m2

              ※ ₩1'=8.84/9.8=0.902t/m2
                                           W2' = 2.79/9.8 = 0.285 t/m^2
        COMBINATION OF LOADS
2.
    3
    1) Base Loads
        ① Dead loads1 (deck loads)
           Dead loads2 (own weight)
        ③ HA1E (HA1 for side spans)
        (HA1S (HA1 for middle span)
        (5) HA2
           HB (cosentration loads span L=6m)
           HB
                (cosentration loads span L=11m)
            HB
                (cosentration loads span L=16m)
            HB
                 (cosentration loads span L=21m)
                (cosentration loads span L=26m)
            HB.
            HB1E (HB1 distributed loads for side spans)
HB1E (HB1 distributed loads for middle spans)
HB2 (HB1 distributed loads)
    2) Combination Loads
           HB1 6+1
        (5)
            HB1
                 (1)+(1)
       ®
            HB1
                 (3)+(1)
            HB1
                 9+0
       ⑱
            HB1
                 @+@
       (9)
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                 (6)+(2)
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           HB1
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                 6)+(3)
       25
            HB2
                 (7)+(3)
       26
            HB2
                 ®+(3)
       27
           HB2
                 (9)+(3)
       28
           HB2
                 (10)+(13)
                    uр
     3) Pick
                           Cases
               (19), (15), (10), (18),
       (1)
               (9), (20), 21, 22, 23
24, 25, 26, 27, 28
       (2)
               24, 25,
③, (1)
       (3)
               ④,
                   (2)
       (5)
                   (3)
       (6)
```

EFFECTIVE WIDTH AND MODULAS 3.

3.1 EFFECTIVE WIDTH

2) Main girder

be=bw+1/5

be: effective width for flanges

1: length of moment zero

0.7*Ls or 0.85*Ls'

Ls: spans for connection girders of middle spans

Ls: spans for connection girders of end spans

end spans be=0.60+0.85*17.425/5=3.563m>b=1.925m(2.363m) middle spans be=0.60+0.70*17.425/5=3.040m>b=1.925m(2.363m)

2) Cross girder end cross girder $\lambda 1=1/8+bs=17.425*0.8/8+0=1.743m$ spporting cross girder $\lambda 2=1/8+bs=(21.5+17.425)*0.2/8+0=0.973m$ middle cross girder $\lambda 3 = (n-1)*(1b+1w)+bs=(9-1)/6*(1.925+0.60)+0=3.367m$

3. 2 MODULAS MONBASA

Main girder

1	В	H	A	Y	A*Y	A*y^2	le
End girder①	236.3	20	4726	10	47260	472600	157533
middle gir①'	192.5	20	3850	10	38500	385000	128333
	60	110	6600	75	495000	37125000	6655000
End girderΣ			11326		542260	37597600	6812533
middle girΣ			10450		533500	37510000	6783333

 $IY = \Sigma I c + \Sigma A * Y^2 - \Sigma A * (\Sigma A * Y / \Sigma A)^2$ = 18448107 = 0.184

 $IY = \Sigma I c + \Sigma A * Y^2 - \Sigma A * (\Sigma A * Y / \Sigma A)^2$ = 17056754 = 0.171

Cross girder							
	В	H	A	Y	A*Y	A*Y^2	lc
End cross ①	234.3	20	4686	10	46860	468600	156200
Sopprting ①'	254.6	20	5092	10	50920	509200	169733
2	60	95	5700	67.5	384750	25970625	4286875
End cross Σ			10386	•	431610	26439225	4443075
Soporting Σ	* .		10792		435670	26179825	4456608

 $1Y = \Sigma \ 1 \ c + \Sigma \ A * Y^2 - \Sigma \ A * (\Sigma \ A * Y / \Sigma \ A)^2$ = 12945925 = 0.129

 $1Y = \Sigma \ 1 \ c + \Sigma \ A * Y^2 - \Sigma \ A * (\Sigma \ A * Y / \Sigma \ A)^2$ = 13348558 = 0.133

Cross girder							
	В	H	A	Y	A*Y	A*Y^2	lc
Middle cro①	708.4	20	14168	10	141680	1416800	472267
2	35	95	3325	67.5	224438	15149531	2500677
Middle $cro\Sigma$			17493		366118	16566331	2972944

 $IY = \Sigma I c + \Sigma A * Y^2 - \Sigma A * (\Sigma A * Y / \Sigma A)^2$ = 11876666 = 0.119

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	Y (M) 5.9563 6.0321	-	Ö	DINATE
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** MONBASA JU BRIDGE **

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** MONBASA JU BRIDGE **