BILL OF QUANTITIES

BOX CULVERTS

VEHICL		X CUL	VER	тѕ —		1
		CULVE				47
9 9	* .					
PEDEST	RIAN	$R \cap X$	CHL	VERTS		84-100

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14800 x 35.300

```
Length=16.150m(mean) skew angle=80'10'56"~90'.00'
 (B1) BOX
                                           \alpha = 1/\sin 80^{\circ} 10^{\circ} 56^{\circ} = 1.0149
1) concrete (grade: 25)
   bottom slab V = 11.80 \times 1.00 \times 16.150
                                                                                       =171.513m^3
            slab V = " \times 0.90 \times
   top
                                                                                       =190.570 ~
            slab V=5.80\times0.90\times " \times 2
   side
                                                                                       =168,606 "
                    V = (29.976 \times 7.361 - 6.00 \times 9.00 - 7.090 \times 11.976) \times 0.60
            wall
   wing
                    . = (81, 743) \times 0.60
                                                                                       =49.046m<sup>3</sup>
   total
                     V = -
                                                                                       =579.735m^3
2) formwork
                    A = (11.80 \times 7.70 - 10.0 \times 5.80) (1.0 + 1.0149)
  Section
                                                                                       =66. 209 m<sup>2</sup>
  Out side
                    A = 7.70(16.150-0.60 \times 1.0149) \times 2
                                                                                       =239.332 "
   // (wing)
                    A = 7,70 \times 0,60 \times 1,0149 \times 2
                                                                                       =9.377 "
                                                                                                     l=493.017
                    A = \times 81.743 \times 2
   wing:side
                                                                                       =163,486 "
        : section A = 0.60(1.361+\sqrt{6.0^2+9.0^2}) × 2
                                                                                       =14.613 //
          side
                    A = 5.80 \times 16.150 \times 2
  i n
                                                                                       =187.340 //
  top
          slab
                    A = 10.00 \times
                                                                                       =161.500 //
  total
                    A =
                                                                                       =841.857 "
  (B2) BOX
               length=16.150m(mean) skew angle =90'00' \sim76'.00'
                                           \alpha = 1/\sin 76^{\circ} = 1.0307

    concrete(grade:25)

    bottom slab V =
                                                                                       =171.513m^3
             slab
    top
                     V=
                                                                                       =190.570 "
    side
             slab
                     V =
                                                                                       =168.606 ~
             wall
                     V = (29, 976 \times 7, 361 - 6, 0 \times 9, 0 - 7, 090 \times 12, 162) \times 0, 60
    wing
                        =(80, 425) \times 0, 60
                                                                                       =48.255 "
                      V=
    total
                                                                                       =578,944 "
2) formwork
    section
                      A = (11.80 \times 7, 7 - 10.0 \times 5, 80) \times 1.0307
                                                                                       =33, 868 m<sup>2</sup>
    out side
                                                                                       =239.186 "
                     A = 7.70(16.150-0.60 \times 1.0307) \times 2
                                                                                                      -458.040
     // (wing)
                     A = 7,70 \times 0,60 \times 1,0307 \times 2
                                                                                       =9.523 "
    wing:side
                     A = 380.425 \times 2
                                                                                       =160.850 //
                                                                                       =14.613 "
         :section A =
                                                                                       =187.340 "
    i n
          side
                     A =
                                                                                       =161,500 "
    top slab
                     A =
                                                                                       =806.880 //
    total
                                                      3) excavation (Carth)
                                                           V = 1.060/6 \{14.80 \times 35.30 + 15.860 \times 36.360\}
                                                               +(14.80+15.86)(35.30+36.360) = 582.329m<sup>3</sup>/box
                                                           V = 39.005 + 78.010 + 0.76 \times 11.80 \times 32.30 = 406.681 \text{m}^3/\text{box}
                                                          back fill
                                                                                                          =175.648 \text{m}^3/\text{box}
                                                           V = 582.329 - 406.681
              11.800 x 32.300
```

A) base concrete (grade:15) CONCRETE V=0.10×12.00× {32.30+0.10(1.0149+1.0307)} $\approx 39.005 \,\mathrm{m}^3/\mathrm{box}$ $A = 0.10 \{32.30 \times 2 + 12.00(1.0149 + 1.0307)\}$ $=8.914 \,\text{m}^2/\text{box}$ 5) joint filler A=11.80 \times 7.70-10.0 \times 5.80 $=32.860 \,\mathrm{m}^2/\mathrm{box}$ 6) water stop $L=(10, 90+6, 75) \times 2$ =35.300 m/box7)base(crucherran) $V = 0.20 \times 12.00 \{32.30 + 0.10(1.0149 + 1.0307)\}$ $=78.010m^3/box$ 8) support: top slab (B1) $V=10.0\times(5.80-0.10)\times16.150=920.550m^3$ $=920.550m^3$ V= $V = 9.0(6, 0-0, 10) \times 0.60$:wing =31.860 " ٧= =31.860 // $=952.410m^3$ total =952, 410m³ VB2[™] 9)scaffold:main " $V = 1.20 \times 7.70 \times 16.150 \times 2$ $=298.452m^3$ V = =298.452 // :wing " $V = 1.20 \times 7.70 \times 9.00 \times 2 \times 2$ =332,640 ~ =332.640 // V= =631.092 // =631.092 " total [™]B2¯ Asphalt A 1=0.074 \times 10.0+1/2 \times 0.064 \times 1.75 =0.796 m² in) Pavement --- into Box ----A 2=0.052 \times 10.0+1/2 \times 0.053 \times 1.75 =0.566 " A $3=0.050 \times 10.0$ =0.500 " $V = 1/2 \{(0.796+0.566) \times 16.10 + (0.566+0.500) \times 16.2$ 500 3500 3500 15Q $=19.598m^3/box$ mm 202 $(A 1=0.214\times1.50+1/2(0.214+0.128)\times3.50)$ $+1/2(0.128+0.064)\times 1.750$ $A = 0.101 \times 1.50 + 1/2(0.101 + 0.106) \times 3.50$ $+1/2(0.106+0.053) \times 1.75$ =0.653 m² A 3=0. $095 \times 1.50 \times 2+(0.095+0.182) \times 3.50=1.254 \text{ m}^2$ 158 153 \therefore V = 1/2 { (1.087+0.653) × 16.10+(0.653+1.254) × 16.20} $=29.453m^3/box$ 1750 1750 232.

```
BOX ROAD
               BOX
 1) concrete(grade: 25) 1=13.750m \theta =90°
   bottom slab V=9.40\times0.70\times13.750
                                                                                      =90.475m^3
                   V = " \times 0.80 \times
            slab
   top
                                                                                      =103.400 //
                   V = 5.60 \times 0.70 \times
            wall
   side
                                         " × 2
                                                                                      =107.800 "
                   V = \{27.0(7.464-0.267)-1/2(5.733\times8.60+6.00\times9.0)-9.40\times(7.10-0.267)\} \times 0.6
   wing
                       = \{78, 437\} \times 0.60
                                                                                      =47.062
                     V=
   total
                                                                                      =348, 737
2) form-section A = (9.40 \times 7.10 - 8.0 \times 5.60) \times 2
                     =(21.940)\times 2
                                                                                      =43.880 \,\mathrm{m}^2(21.940)
            side A = 7.10(13.750-0.60) \times 2
   out
                                                                                      =186.730 //
                                                                                                               (388,352)
          (wing) A = 7, 10 \times 0, 60 \times 2
                                                                                      =8.520 "
                                                                                                               410,292
                    A = \times 78.437 \times 2
   wing:side
                                                                                      =156.874 "
         : section A = 0.60(1.464+1.197+\sqrt{5.733^2+8.60^2}+\sqrt{6.0^2+9.0^2})
                                                                                      =14.288 "
         in side A=5.60\times13.750\times2
                                                                                      =154.000 //
         top slab A=8.0\times13.750\times2
                                                                                      =110.000 //
   total
                                                                                      =674.292 \% (652.352)
                                                                                         (B_1) (B_2)
3) base concrete(grade:15)
   concrete
                   V = 9.60 \times 27.70 \times 0.10
                                                                                      =26.592m^3/box
   form Work
                   A = (9.60+27.70) \times 0.10 \times 2
                                                                                      =7.460 \, \text{m}^2/\text{box}
4) base
                   V = 9.60 \times 27.70 \times 0.20
                                                                                      =53.184 \text{ m}^3/\text{box}
  (crusherran)
5) suport: topslab V = 8.0 (5.60-0.10) \times 13.75
                                                                                      =605.000m<sup>3</sup>
    :wing V=1/2 \{(5.733-0.10) \times 8.60+(6.0-0.10) \times 9.0\} \times 0.60=30.463m^3 = 635.463
6) scaffold: main V=1.20\times7.10\times13.750\times2
                                                                                      =234.300 \,\mathrm{m}^3
                                                                                      =299.904 \,\mathrm{m}^3 = 534.204
            :wing V = 1.20(8.60+9.00) \times 7.10 \times 2
7) joint filler A = 9.40 \times 7.10 - 8.0 \times 5.60
                                                                                      =21.940 \,\mathrm{m}^2/\mathrm{box}
8) water stop L=(6.350+8.70)\times 2
                                                                                      =30.100 \text{m/box}
                                                                                      =14.712m^3/box
                    V = \{0.05 \times 1.00 \times 2 + (0.05 + 0.095) \times 3.00\} \times 27.50
   pavement
      asphalt
                                                     (0) ecavation (earth)
                                                            V = 1.120/6 {12.40 × 30.50+13.520 × 31.620
                                                                 +(12.40+13.520)(30.50+31.620) = 450.959m<sup>3</sup>/box
                                                         remain
                                                            V = 9.60 \times 27.70 \times 0.30 + 9.40 \times 27.50 \times 0.020
                                                                                                            =84.946m^3/box
                                                          back fill
                                                                                                            =366.013m^3/box
                                                            V = 450.959 - 84.946
```

9400 x 27500 12400 x 30500

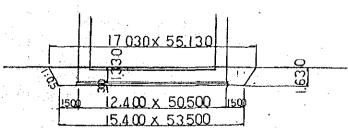
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(B1) L=16.420m (mean) skew angle = 60^{\circ} \sim 90^{\circ}
i) concrete
                                                    \alpha = 1/\sin 60^{\circ} = 1.1547
      bottm slab
                       V = 16.420 \times 12.40 \times 1.30
                                                                                          =264.690m<sup>3</sup>
                       V =
                               " × " ×1,20
      top
                                                                                          =244.329 "
                      V = " \times 6.00 \times 1.20 \times 2
      side
               wall
                                                                                          =236,448 "
                       V = \{8.30 \times 8.496 - 1/2 \times 8.30(1.714 + 5.533) + 14.318 \times 0.30\} \times 0.60
      wing
                          = \{44,737\} \times 0.60
                                                                                          =26.842 "
                       V =
      total
                                                                                          =772.309 "
2) formwork
     section
                       A = (12.40 \times 8.50 - 10.00 \times 6.00) \times 1.1547
                                                                                          =52.423 m
     out
               side A = 8. 50 \times 16. 420 \times 2 - 0. 693 \times 8. 20
                                                                                          =273.457 "
                                                                                          =5.682m"
               wing A = 0.693 \times 8.20
                                                                                                       - 427,978
               side A = \frac{3}{4}4.737 \times 2
     wing
                                                                                          =89.474 "
                       A = 0.60 \times 1.1547 \times 0.30 + 0.60 (1.249 + \sqrt{5.533^2 + 8.30^2})
      section
                                                                                          =6.942"
          in sade A = 6.00 \times 16.420 \times 2
                                                                                          =197.040 "
          top slab A = 10.0 \times 16.420
                                                                                          =164.200 "
      total
                                                                                          =789. 218 m
3) support: topslab V = (6.00-0.10) \times 10.00 \times 16.420
                                                                                          =968.780m<sup>3</sup>
                                                                                          =13.528 //
            wing
                      V = 1/2(5.533-0.10) \times 8.30 \times 0.60
4) scaffold:main V = 1.20 \times 8.50 \times 16.420 \times 2
                                                                                          =334.968 "
          :wing
                                                                                          =169.320 //
                       V = 1.20 \times 8.50 \times 8.30 \times 2
```

NO3 BOX FOR ROAD

(B2) L=18. 224m $\theta = 90^{\circ}$

	b $V = 18.224 \times 12.40 \times 1.30$	=293.770m ³
top sla	b V = " × " × 1.20	=271.173 "
side wal	$1 V = \text{``} \times 6.00 \times 1.20 \times 2$	=262. 425 "
total	V =	=827.368 //
)form work		•
section	$A = (12.40 \times 8.50 - 10.00 \times 6.00) \times 2$	=90.800 m ² =309.808 "]= 400.608
out sid	$A = 8.50 \times 18.224 \times 2$	=309.808 <i>"</i> ∫ * 400.600
in side	$A=6.00\times w\times 2$	=218.688 "
top slab	A=10.00× "	=182. 240 "
total	A =	=801.536 //
support:tops	$1ab V = (6.00-0.10) \times 10.00 \times 18.224$	=1075.216m ³
	$V = 1.20 \times 8.50 \times 18.224 \times 2$	=371.769m ³

```
(B3) L=15.856m(mean) skew angle = 90^{\circ} \sim 56^{\circ}
   1) concrete
            bottom slab V = 15.856 \times 12.40 \times 1.30
                                                                                                                                                                                                                 =255.598m<sup>3</sup>
            top
                                  "
                                                      V = " \times " \times 1.20
                                                                                                                                                                                                                 =235, 937 "
            side
                                  wall V =  \times 6.00 \times 1.20 \times 2
                                                                                                                                                                                                                 =228, 326 "
                                                      V = 0.70 \{ (14.966 \times 0.30) + 1/2 (1.405 + 8.50) \times 9.90 + 1/2 (1.416 + 8.50) \times 11.50 \}
                                                           =0.70 \{110.536\}
                                                      V=
            total
                                                                                                                                                                                                                 =797. 236 "
  2)formwork
           section
                                                     A = (12.40 \times 8.50 - 10.00 \times 6.00) \times 1.2063
                                                                                                                                                                                                                 =54.766 m²
                                 side A = 8.50 \times 15.856 \times 2 - 0.70 \times 1.2063 \times 8.20
           out
                                                                                                                                                                                                                 =262.627 "
                               (wing) A = 0.70 \times 1.2063 \times 8.20
                                                                                                                                                                                                                                                               =565.367
                                                                                                                                                                                                                 =6.924 "
           wing.
                                 side A = \times 110.536 \times 2
                                                                                                                                                                                                                 =221.072 "
                                                    A = 0.70 {1.405+1.416+\sqrt{6.60^2+9.90^2}} +\sqrt{7.667^2+11.50^2}} = 19.978 \frac{1}{2}
                        section
           in
                                 side A = 6.00 \times 15.856 \times 2
                                                                                                                                                                                                                =190.272 "
                                 slab
                                                     A = 10.0 \times
           top
                                                                                                                                                                                                                 =158.560 "
           total
                                                                                                                                                                                                                =914.199 "
3) support: topslab V = (6.00-0.10) \times 10.00 \times 15.856
                                                                                                                                                                                                                 =935.504m^3
                                                                                                                                                                                                                                                             988.483
                             wing
                                                    V = 1/2 \{ (6.60-0.10) \times 9.90 + (7.667-0.10) \times 11.50 \} \times 0.70 = 52.979 \times 0.70
4) scaffold: main V=1.20\times8.50\times15.856\times2
                                                                                                                                                                                                                =323.462 //
                             wing V = 1.20 \times 8.50(9.90+11.50) \times 2
                                                                                                                                                                                                                                                        = 760.022
                                                                                                                                                                                                                =436.560 "
5) joint filler A = (12.40 \times 8.50 - 10.00 \times 6.00) \times 2
                                                                                                                                                                                                                =90.800 \,\text{m}^2/\text{box}
           (t=20 \,\mathrm{mm})
6) water stop
                                                      L=(11.20+7.25)\times 2\times 2
                                                                                                                                                                                                                =73.800m/box
 7) base concrete(grade:15)
          concrete
                                                    V = \{50.50+0.10(1.1547+1.2063)\} \times 12.60 \times 0.10
                                                          = \{50.736\} \times 12.60 \times 0.10
                                                                                                                                                                                                                =63.927m^3/box
           formwork
                                                    A = \% \{50, 736 \times 2 + 14, 318 + 14, 966\} \times 0.10
                                                                                                                                                                                                                =13.075 m²/box
8) base (crusherran) V = 350.736 \times 12.60 \times 0.20
                                                                                                                                                                                                                =127.854m^3/box
 9) excavation (earth)
                                    V = 1.630/6 \{15.40 \times 53.50 + 17.030 \times 55.130 + (15.40 + 17.030)(53.50 + 55.130)\} = 1435.930
                                   V = 350.736 \times 12.60 \times 0.30 + 12.40 \times 50.50 \times 1.330
                                                                                                                                                                                                                =1024.628 \,\mathrm{m}^3/\mathrm{box}
       backfill: V = 1435. 930-1024. 628
                                                                                                                                                                                                                =411.302m^3/box
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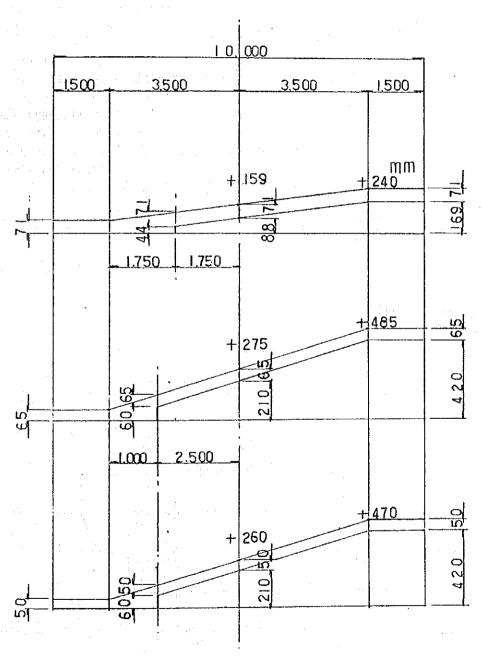


10) pavement Asphalt

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A1=0.071×10.0+1/2×0.044×1.75 =0.748 m<sup>t</sup>
A2=0.065× "+1/2×0.060×1.00 =0.680 "
A3=0.050× "+ " =0.530 "
∴ V=1/2 {(0.748+0.680)×25.690+(0.680+0.530)×24.810} =33.352 m<sup>3</sup>
```

base(crusherran)

usher ran/	
$A1=1/2(0.044+0.169) \times 5.25+0.169 \times 1.50$	=0.812 m²
$A2=1/2(0.060+0.420) \times 6.00+0.420 \times 1.50$	=2.070 "
A3= " + "	= // //
$V = \{1/2(0.812+2.070) \times 25.690+2.070 \times 24.810\}$	=88.375m ³



WALL

 $\alpha = 1/\sin 60^{\circ} = 1.1547$

 $V = 1/2(3.70+1.20) \times 16.10 \times 0.20$

I)		de: 25) $1 \text{ V} = 1/2 (6.90+0.40) \times 16.00 \times 0.60$ $\text{ V} = 1/2 (3.50+1.00) \times " \times " +0.50^2 \times 8.00$	=35.040m ³ =23.600 w		
	total	V =	=58.640 //		
2)	form work vertical wal	A = $1/2$ (6. 90+0. 40) × 16. 00 A = $1/2$ (6. 90+0. 40) × 0. 60+1/2 (6. 90+0. 40) × 16. 00 A = 0. 60 (3. 50+1. 00+16. 00 × 2)	=58.400 m ² =60.590 " =21.900 "] = 82.490		
	total	V =	=140.890 //		
4) 5)	joint filler	$V = 1.20(6.90+1.00) \times 14.50$ $A = 6.90 \times 0.60 \times 1.1547$ L = (6.90-0.30)	=137.460m ³ =4.780m ³ =6.600m		
	concrete	$V = 1/2 (3.70+1.20)+16.10 \times 2)0.10$ $A = (3.70+1.20+16.10 \times 2) \times 0.10$	=3.944m ³ =3.710 "		

8) excavation.

excavation (Carth)

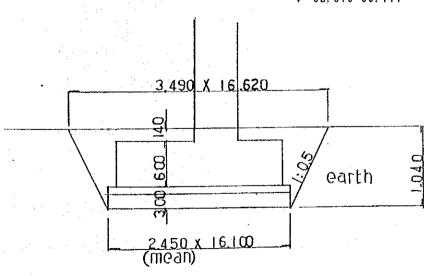
V=1.040/6 {2.45×16.10+3.490×16.620 +(2.45+3.49)(16.10+16.620)} +0.50²×8.00 =52.579 m^3

 $=7.889m^3$

remain $V = 1/2 (3.70+1.20) \times 16.10 \times 0.30+1/2 (3.50+1.00)$ $\times '16.0 \times 0.60+0.60 \times 16.0 \times 0.14+0.50^2 \times 8.00$ $= 36.777m^3$

backfill V=52,579-36.777

=15.802m³

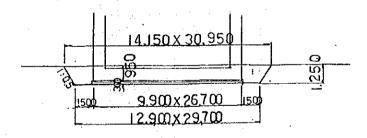


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(B1)(B2)B0X length=13.350m(mean) skew angle = 84° 22'16"
                                         \alpha = 1/\sin 84^{\circ} 22'16'' = 1.0048
) concrete(grade:25)
    bottom slab V=9.90\times0.70\times13.350
                                                                                    =92.515m^3
            slab V = " \times 0.80 \times "
    top
                                                                                    =105.732 "
            slab V = 5.75 \times 0.70 \times " \times 2
    side
                                                                                    ±153.525 "
                   V = \{27.848 \times 7.330 - 1/2(8.60 \times 5.733 + 9.30 \times 7.60) - 9.948 \times 6.98\} \times 0.60
            = \{74.697\} \times 0.60
                                                                                    =44.818 "
                    V =
    total
                                                                                    =396.590m<sup>3</sup>
2) fomwork
                    A = (9.90 \times 7.25 - 8.50 \times 5.75)(1+1.0048)
    section
                                                                                    =45.909 \,\mathrm{m}^{2}(23.009)
                                                                                                           (380.521)
            side A = (13.350-0.60 \times 1.0048) \times 7.25 \times 2
                                                                                    =184.833 #
            (wing) A = 0.60 \times 1.0048 \times 7.25 \times 2
                                                                                    =8.741 "
                                                                                                          = 403.421
            side A = \times 74.697 \times 2
    wing
                                                                                    =149.394 "
    section
                    A=0.60(1.597+1.130+\sqrt{8.60^2+5.733^2}+\sqrt{9.30^2+6.20^2})
                                                                                      =14.544 "
             side
                    A = 13.350 \times 5.75 \times 2
                                                                                    =153.525 "
    top slab
                    A = " \times 8.50
                                                                                    =113.475 "
                 A =
    total
                                                                                    =670.421 / (647.521)
                                                                                      (B_t)
                                                         excavation
                                                           V = 1.250/6 {12.90 × 29.70+14.15 × 30.95
                                                              +(12.90+14.15)(29.70+30.95) =512.844m<sup>3</sup>/box
```

_3) excavation (earth)

 $V = 0.30 \times 10.10 \times (26.70 + 0.20 \times 1.0048)$ $=332.623m^3/box$ $+0.95 \times 9.90 \times 26.70$ backfill V = 512.844 - 332.623

 $=180.221 \text{ m}^3/\text{box}$



4) base concrete (grade: 15)

 $V = 0.10 \times 10.10 \times \{26.70 + 0.10 \times 1.0048 \times 2\}$ concrete $=27.169 \,\mathrm{m}^3/\mathrm{box}$ $A = 0.10 \{(26.70+0.20 \times 1.0048) + 10.10 \times 1.0048\} \times 2 = 7.409 \text{ m}^2/\text{box}$ formwork 5) base (crucherran) $V = 0.20 \times 10.10 \{26.70 + 0.10 \times 1.0048 \times 2\}$ $=54.339m^3/box$ 6) support B1=B2 :top slab V = 8. $50 \times (5.75 - 0.10) \times 13.350$ =641.133m³ :wing $V = 1/2 \{8.60(5.733-0.10) + 9.30(6.20-0.10)\} \times 0.60$ $=31.552m^3$ 7) scaffold: main $V = 1.20 \times 7.25 \times 13.35 \times 2$ =232, 290m³ : wing V = 1, 20×7 , $25(8, 60+9, 30) \times 2$ =311.460m³ 8) joint filler $A = 9.90 \times 7.25 - 8.50 \times 5.750$ $=22.900 \,\mathrm{m}^2/\mathrm{box}$ 9) water stop $L=(9.20+6.50)\times 2$ =31.400m/box (0) pavement Asphalt: $A1=0.05\times 8.50+1/2\times 0.049\times 1.50$ $=0.461 \, \text{m}^2$ $A2=0.059 \times "+1/2 \times 0.045 \times "$ =0.535 " $A3=0.05\times1.25+1/2(0.05+0.07)\times3.0$ $+0.07 \times 4.25 + 1/2 \times 0.039 \times 1.50$ =0.569 " $V = 1/2 \{ (0.461+0.535) \times 12.00+(0.535+0.569) \times 14.70 \}$

base: $A1=0.170\times1.25+1/2(0.170+0.099)\times3.0$ $+1/2(0.099+0.049) \times 1.50$ $=0.727 \, \text{m}^3$

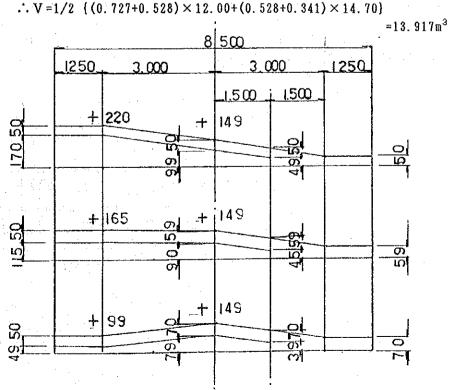
 $=14.090 \,\mathrm{m}^3$

 $A2=0.106\times1.25+1/2(0.106+0.09)\times3.0$

 $\pm 1/2(0.09+0.045) \times 1.50$ =0.528 "

 $A3=0.049\times1.25+1/2(0.049+0.079)\times3.0$

 $+1/2(0.079+0.039) \times 1.50$ =0.341 "



(B)(B2) BOX

```
i) concrete (grade:25)
     bottom slab V=9, 90\times0, 70\times13, 350
                                                                                           =92.515m^3
     top
                " V = " \times 0.80 \times "
                                                                                           =105.732 "
     side
              wall V=5.60\times0.70\times \times 2
                                                                                           =104.664 "
                     V = \{27, 30 \times 7, 158 - 1/2 (5, 60 \times 8, 40 + 6, 00 \times 9, 0) - 9, 90 \times (7, 10 - 0, 267)\} \times 0, 60
     wing
                        = \{77.247\} \times 0.60
                                                                                           =46.348 "
                     V =
     total
                                                                                           =349.259m^3
2) form. section A = (9.90 \times 7.10 - 8.50 \times 5.60) \times 2(22.690 \times 2)
                                                                                           =45.380 \,\mathrm{m}^2 (22.690)
              side A = 7.10(13.350-0.60) \times 2
     out
                                                                                           =181.050 //
                                                                                                                     (380930)
             (wing) A = 7.10 \times 0.60 \times 2
                                                                                           =8.520 //
                                                                                                                     ± 403,620
            side A = \times 77.247 \times 2
     wing
                                                                                           =154.494 "
          : section A = 0.60(1.158+1.558+\sqrt{9.00^2+6.00^2}+\sqrt{8.40^2+5.60^2})
                                                                                           =14.176 // -
              side A = 5.60 \times 13.350 \times 2
                                                                                           =149.520 //
     top
              slab
                     A = 8.5 \times 13.350
                                                                                           =113.475 //
     total.
                       A =
                                                                                           =666.615 " (643.925 ")
3) base concrete (grade:15)
                                                                                           \approx 27.169 \,\mathrm{m}^3/\mathrm{box}
                      V = 10.10 \times 26.90 \times 0.10
   concrete
   form
                       A = (10.10+26.90) \times 0.10 \times 2
                                                                                           =7.400 \,\mathrm{m}^2/\mathrm{box}
4) base (crusherran)
                                                                                           =54.338m^3/box
                       V = 10.10 \times 26.90 \times 0.20
5) suport: topslab V = 8.50 (5.60-0.10) \times 13.35
                                                                                            =624.112m<sup>3</sup>
                     V = 1/2 \{ (5.60-0.10) \times 8.40 + (6.0-0.10) \times 9.0 \} \times 0.60 = 29.790 
                                                                                            =227.484m<sup>3</sup>
6) scaffold:main V=1.20\times7.10\times13.350\times2
              :wing V = 1.20(8.40+9.00) \times 7.10 \times 2
                                                                                            =296.496m<sup>3</sup>
                                                                                            =22.690 \,\mathrm{m}^2/\mathrm{box}
   joint filler A = 9.90 \times 7.10 - 8.50 \times 5.60
                                                                                            =31.100m/box
    water stop
                       L=(6,350+9,20)\times 2
                                                                                            =14.952m^3/box
                       V = \{0.05 \times 1.25 \times 2 + (0.05 + 0.095) \times 3.00\} \times 26.70
    pavement
      asphalt
                                                               excavation (earth)
10) excavation
                                                               V = 1.10/6 {12.90 × 29.70+14.00 × 30.80
                                                                 +(12.90+14.00)(29.70+30.80) = 447.659m^3/box
                                                               remain
                                                                 V = 10.10 \times 26.90 \times 0.30 + 9.90 \times 26.70 \times 0.80
                                                                                                       =292.971 \,\mathrm{m}^3/\mathrm{box}
                                                              back fill
                                                                V = 447.659 - 292.971
                                                                                                       =154.688m^3/box
                          9.900 x 26.700
```

(B) (B2)

```
1) concrete(grade: 25)
    bottom slab V = 9.90 \times 0.70 \times 12.750
                                                                                         =88.357m^3
    tob
                      V = " \times 0.80 \times "
                                                                                         =100.980 "
    side
              wall
                     V = 5,60 \times 0,70 \times " \times 2
                                                                                         =99.960 W
                      V = \{27, 20 \times 7, 139 - 1/2 (5, 467 \times 8, 20 + 6, 067 \times 9, 10) - 9, 90 \times (7, 10 - 0, 267)\} \times 0.60
    wing
                         = \{76.514\} \times 0.60
                                                                                         =45.908 W
                       V=
    total
                                                                                         =335.205m^3
2) form
        section A = (9.90 \times 7.10 - 8.50 \times 5.60) \times 2
                         =(22.690)\times 2
                                                                                         =45, 380 m² (22, 690)
             side A = 7.10(12.750-0.60) \times 2
                                                                                         =172,530 "
                                                                                                                 (370.889)
            (wing) A = 7.10 \times 0.60 \times 2
      "
                                                                                         =8.520 "
                                                                                                                 =393.579
                      A = \times 76.514 \times 2
             side
                                                                                         =153.028 "
    wing
     section A=0.60(1.672+1.072+\sqrt{5.467^2+8.20^2}+\sqrt{6.067^2+9.10^2})=14.121 "
             side A = 5.60 \times 12.750 \times 2
                                                                                         ≈142, 800 m²
    in:
    top
              slab
                     A = 8.50 \times \%
                                                                                         =108.375 "
                                                                                         =644.754 m² (622.064 m²)
    total
                    ···· A =
                                                                                             (Bi)
                                                                                                          (B2)
3) base concrete (grade:15)
                      V = 10.10 \times 25.70 \times 0.10 = 25.957 \text{m}^3/\text{box}
    concrete
                       A = (10.10+25.70) \times 0.10 \times 2=7.160 \,\text{m}^2/\text{box}
    form
4) base (crusherran)
                                                                                         =51.914m^3/box
                       V = 10.10 \times 25.70 \times 0.20
5) suport
                                                                                         =596.062m^3
                     V = 8.50(5.60-0.10) \times 12.750
       :top slab
              wing V = 1/2 \{ (5.467-0.10) \times 8.20 + (6.067-0.10) \times 9.10 \} \times 0.60 = 29.492 m^3
                                                                                          =217.260m^3
6) scaffold:main V = 1. 20 \times 7. 10 \times 12. 75 \times 2
            :wing V = 1.20(8.20+9.10) \times 7.10 \times 2
                                                                                          =294.792 "
                                                                                          =22.690 \,\mathrm{m}^2/\mathrm{box}
7) joint filler A=9.90\times7.10-8.50\times5.60
                                                                                          =31.100m/box
                      L=(6.350+9.20)\times 2
8) water stop
                       V = \{0.05 \times 1.25 \times 2 + (0.05 + 0.095) \times 3.00\} \times 25.50
                                                                                        =14.280m<sup>3</sup>/box
9) pavement
        asphalt
                                                             excavation(earth)
10) excavation.
                                                              V = 1.460/6 {12.90 × 28.50+14.360 × 29.960
                                                                  +(12.90+14.360)(28.50+29.960)}
                                                                                                          =581.930m^3/box
                                                             V = 10.10 \times 25.70 \times 0.30 + 9.90 \times 25.50 \times 1.160
                                                                                                          =370.713m<sup>3</sup>/box
                                                             backfill
                                                             V = 581.930 - 370.713
                                                                                                          =211.217m^3/box
                            9900 x 25,500
```

```
length=13.100m(mean) skew angle = 60^{\circ} \sim 90^{\circ}
                                             \alpha = 1/\sin 60^{\circ} = 1.1547
 1) concrete (grade: 25)
    bottom slab V = 9.90 \times 0.70 \times 13.10
                                                                                               =90.783 \,\mathrm{m}^3
    top
                        V = n \times 0.80 \times n
                                                                                               =103.752 //
                      V = 5.65 \times 0.70 \times " \times 2
    side
              wall
                                                                                               =103.621 //
                       V = \{8.70 \times 7.828 - 1/2(8.45 \times 5.633 + 8.70 \times 0.877)\} \times 0.60
              wall
    wing
                             (11.432 \times 0.685 + 1/2 \times 0.515 \times 7.828) \times 0.57
                          = (9.846) \times 0.57 + \{40.489\} \times 0.60
                                                                                               =29.905m^3
    total
                                                                                               =328,061m<sup>3</sup>
2) formwork
                       A = (9.90 \times 7.15 - 8.50 \times 5.65) \times 1.1547
    section
                                                                                               =26. 280 m² ¯
                       A = 7.15 \times 13.10 \times 2 - 0.659 \times 7.828
    out
              side
                                                                                               =182.171 "
                                                                                                              = 321,553
      . 11
             (wing) A = 0.659 \times 7.828
                                                                                               =5.158 "
                       A = \% (9.846 + \% 40.489) \times 2
              side
    wing
                                                                                               =100.670 //
                        A = 0.57 × 0.685+0.60(1.318+\sqrt{5.633^2+8.45^2})
    section
                                                                                               =7.274 //
    in
           side A=5.65\times13.10\times2
                                                                                               =148.030 ~
    top slab
                      A = 8.50 \times 13.10
                                                                                               =111.350 "
    total
                        A =
                                                                                               =580.933 "
3) support
                        V = (5.65-0.10) \times 8.50 \times 13.10
                                                                                               =617.992m<sup>3</sup>
    main:
                        V = (5.633-0.10) \times 8.45/2 \times 0.60
                                                                                               =14.026m<sup>3</sup>
    wing:
                        V =
                                                                                               =632.016 "
    total
4) scaffold
    main:
                        V = 1.20 \times 7.15 \times 13.10 \times 2
                                                                                               =224.796m^3
                                                                                               =149.292 "
                        V = " \times " \times 8.70 \times 2
    wing:
                                                                                               =374.088m<sup>3</sup>
    total
```

length=12,000m

1) concrete bottom slab $V=9, 90\times0, 70\times12, 00$ $=83.160 \,\mathrm{m}^3$ top slab $V = " \times 0.80 \times "$ $=95.040m^3$ side wall $V = 5.65 \times 0.70 \times " \times 2$ =94.920 " total $=273.120m^3$ 2) formwork =45.520 m (22.760) (194,360) $A = (9.90 \times 7.15 - 8.50 \times 5.65) \times 2 = 22.760 \times 2$ section ±217.120 out side $A = 7.150 \times 12.00 \times 2$ =171.600 " in side A = 5, $650 \times " \times 2$ =135.600 W $A = 8.50 \times "$ =102.000 // top slab total A = =454.720 m² (431.960 ") (B₂) (B_3) 3) support $=566.100 \,\mathrm{m}^3$ main: $V = (5.65-0.10) \times 8.50 \times 12.00$ 4) scaffold =205, 920m³ main: $V = 1.20 \times 7.15 \times 12.00 \times 2$ 5) base concrete(grade:15) $=50.935m^3/box$ concrete $V = 0.10 \times (50.20 + 0.20 \times 1.1547) \times 10.10$ $A = 0.10 \times \{(50, 20+0, 20 \times 1, 1547) + 10, 10 \times 1, 1547\} \times 2 = 12, 418 \text{ m}^2/\text{box}$ form 6) base(crucherran) $=101.870 \,\mathrm{m}^3/\mathrm{box}$ $V = 0.20 \times (50, 20 + 0.20 \times 1.1547) \times 10.10$ 7) joint filler $A = (9.90 \times 7.15 - 8.50 \times 5.65) \times 3$ =68.280 m²/box (t=20mm) 8) water stop $L=(9.20+6.40)\times 2\times 3$ =93.600m/box 9) pavement

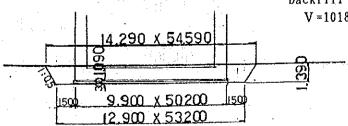
10) excavation (earth)

asphalt

excavation

 $V = \{1.250 \times 0.05 \times 2 + (0.05 + 0.110) \times 3.00\} \times 50.20 = 30.371 \text{ m}^3/\text{box}$

V = 1.390/6 {12.90×53.20+14.290×54.590 +(12.90+14.290)(53.20+54.590) =1018.680m³/box remain $V = 10.10(50.20+0.20 \times 1.1547 \times 0.30+$ $=694.513m^3/box$ $9.90 \times 50.20 \times 1.090$ backfill $=324.167m^3/box$ V = 1018, 680 - 694, 513



```
wall \alpha = 1/\sin 60^{\circ} = 1.1547
  (v1) Retainins wall
 () concrete (grade: 25)
    vertical wall V = 1/2(7.235+0.400) \times 0.60 \times 13.50
                                                                                      \approx 30.921 \, \text{m}^3
    footing slab V = 1/2(3.50+1.00) \times 0.60 \times 13.50+0.50^2 \times 5.00
                                                                                      =19.725m
                     ٧=
    total.
                                                                                      =50, 646 mt
2) formwork
    vertical wall A = 1/2(7.235+0.40) \times 13.50
                                                                                      =51, 536 m
    vertical wall A = 0. 60 \times 0. 40 + 1/2(7.235 + 0.40) \times 13.50
                                                                                      =51.776 //
    footing slab A = (13.50 \times 2 + 3.50 + 1.00) \times 0.60
                                                                                      =18.900 "
                     A =
                                                                                      =122. 212 m²
    total
3) scaffold
                     V = 1.20 \times (7.235 + 1.00) \times 12.30
                                                                                      =121.548m<sup>3</sup>
 4) joint filler A = 7.235 \times 0.60 \times 1.1547
                                                                                      =5.012 \,\mathrm{m}^2
 5) water stop L=(7.235-0.300)
                                                                                      =6.935m
6)base concrete
                                                                                      =3:332m^3
                     V = 1/2(3.70+1.20) \times 0.10 \times 13.60
    concrete
                                                                                      =3, 210 m²
    form
                     A = (3.70+1.20+13.60\times2)\times0.10
7) base(crusherran)
                                                                                      =6.664m^3
                     V = 1/2(3.70+1.20) \times 0.20 \times 13.60
8) excavation (earth)
                                                           excavation
                                                             V = 1.180/6 {2.45×13.60+3.63×14.190
                                                                  +(2, 45+3.63)(13.60+14.190)+0.50^2 \times 6.00
                                                                                                                  =51.412m<sup>3</sup>
                                                           remain
                                                             V = 1/2(3.70+1.20) \times 13.60 \times 0.30
                                                                +1/2(3.50+1.00) \times 13.50 \times 0.60+0.60 \times 13.50 \times 0.280
                                                                                                                  =31.989m<sup>3</sup>
                                                                +0.50^2 \times 6.00
                                                           backfill
                                                                                                                  =19.423m^3
                                                             V = 51.412 - 31,989
                                 3.630 X 14.
                                 N
                                 89
                                                                    earth
                                  2.450 x 13.600
```

(mean)

Retaining wall

) concrete(grade: 25) vertical wall V = $1/2$ (6.85+0.400) × 0.60 × 9.50 footing slab V = $1/2$ (3.50+1.00) × 0.60 × 9.50+0.50 ² × 4.00	=20.662m ³ =13.825m
total V=	=34. 487 m²
2) foem work vertical wall A = $1/2(6.85+0.40) \times 9.50$ " A = $0.60 \times 0.40+1/2$ { $(6.85+0.40) \times 9.50$ } footing slab A = $(9.50 \times 2+3.50+1.00) \times 0.60$	=34. 437 m ² =34. 677 " =14. 100 "
total A=	=83.215 "
3) scaffold $V=1.20\times(6.85+1.00)\times8.60$ 4) joint filler $A=6.85\times0.60\times1.1547$ 5) water stop $L=(6.85-0.300)$ 6) base concrete	=81.012m ³ =4.745 m² =6.550m
concrete $V = 1/2(3.70+1.20) \times 0.10 \times 9.6$ form $A = (3.7+1.20+9.60 \times 2) \times 0.10$	= 2. 352m ³ = 2. 410 m ^t
7) base (crusherran) $V = 1/2(3.70+1.20) \times 0.20 \times 9.6$	=4.704m ³
+(2.45+ remain V=1/2(3.7	$5 \{2.45 \times 9.60 + 3.63 \times 10.190$ $+3.63\} (9.60 + 10.190) + 0.50^2 \times 4.00\}$ $= 36.563 \text{m}^3$ $70 + 1.20) \times 9.60 \times 0.30 + 1/2 (3.50 + 1.00)$ $\times 0.60 + 0.60 \times 9.50 \times 0.280 + 0.50^2 \times 4.00$ $= 22.477 \text{m}^3$ $= 14.086 \text{m}^3$
3.630 X 10. 190	080

Retaining wall

	de: 25) $1 V = 1/2 (6.85+0.400) \times 0.60 \times 16.$ $V = 1/2 (3.50+1.00) \times 0.60 \times 16.$		$=35.887m^3$ =24.275m	
total	V =		=60.162m ³	
2) formwork		•		
and the second s	$1 A = 1/2 (6.85+0.40) \times 16.50$		=59.812m ³	
	$A = 0.60 \times 0.40 + 1/2(6.85 + 0.40)$ $A = (16.50 \times 2 + 3.50 + 1.00) \times 0.60$		=60.052 m²	
TOOLING STAD	A = (10. 50 × 2/5. 50/1. 50) × 0. 50	J	=22.500 "	
total	A = 10 m		=142. 365 m²	
3) scaffold	$V = 1.20 \times (6.85 + 1.00) \times 15.00$		=141.300m ³	
	$A = 6.85 \times 0.60 \times 1.1547$	4	=4.745 m²	
~,	L=(6.85-0.300)		=6.550m	
6) base concrete concrete	$V = 1/2(3.70+1.20) \times 0.10 \times 16.10$	¢Λ	=4.067m ³	
form	$A = (3.7+1.20+16.60 \times 2) \times 0.10$		=3.810 m²	
7) base (crusherr	an) V=1/2(3.70+1.20)×0.20×16.	60 	=8.134m ³	
8)excavatio	on (earth)		<16.60+3.18×16.96 16.60+16.965)} +0.	
•		remain	×16.60×0.30+1/2(ያ 50±1 በበ\
•		$\times 16.50 \times 0.430$		=30.164m ³
		backfill		
		V = 36.504 - 30.164		=6.339m³
·	3.180 X 16.965			
		_ ;	,	
	\	(9/	0	
	7 4	⊥ÿ earth	13	
	\- 8	\exists /		

BOXCULVERT FOR ROAD - NO (1)

LIST OF REINFORCED BAR --- BI

100	DIAMETER	LENGTH	NUMBAR	UNITWEIGHT	PIECEMEIGHT	REIGHT	REMARK
<u> </u>		<u> </u>					
S 1	Y25	21800	51	3,854	84.02	1285	
S	"	12560	51	41	48.41	2469	
3	Y32	12800	50	6,313	80.81	4040	
4	"	6210	100	11	39.20	3920	
5	Y20	16730	72	2.466	41.26	2971	
6	Y32	2460	51	6.313	15.53	792	
7	n	2460	51	н	п	792	
8	Y12	2180	417	0,888	1.94	809	
9	Y32	19000	7	6.313	119.95	840	
01	"	17886	6		112.88	677	
11	41	12970	13	"	81.88	1064	
12	14	2480	7	u	15.66	110	
13	Y12	2180	18	0.888	1.94	35	
				-		22804	kg
W 1	Y25	15890	19	3.854	61.24	1164	· .
2	Y16	15710	. 19	1.579	24.81	471	
3	11	17470	19	0	27.59	524	
4	Y25	17920	19	3.854	69.06	1312	
5	> t	7570	51	16	29.17	1488	
6	4	7570	51	ıı .	"	1488	
7	,,	7570	7	11	п	204	
8	11	7570	1	11	"	29	
9	Y12	1030	221	0.888	0.91	201	
10		1030	221	н	11	201	
11	41	1040	23	и	0.92	21	
		· · · · · · · · · · · · · · · · · · ·				7103	kg
· · · · · · · · · · · · · · · · · · ·							
F 1	Y32	21410	51	6.313	135.16	6893	
F 1	Y32	21410 12800	51	6.313	135.16 80.81	6893 8162	
	Y32			6.313			
2		12800	101	II.	80.81	8162	
3	11	12800 6300	101 50) II	80.81 39.77	8162 1988	
2 3 4	" "	12800 6300 6300	101 50 50	1E 11	80.81	8162 1988 1988	
2 3 4 5	" " Y25	12800 6300 6300 16900	101 50 50 72	3.854	80.81 39.77 " 65.13	8162 1988 1988 4689	
2 3 4 5 6	" " " "Y25 "Y32	12800 6300 6300 16900 2590	101 50 50 72 51	3.854 6.313	80.81 39.77 " 65.13 16.35	8162 1988 1988 4689 834	
2 3 4 5 6 7	" " " " " " " " " " " " " " " " " " "	12800 6300 6300 16900 2590	101 50 50 72 51 51	3.854 6.313	80.81 39.77 " 65.13 16.35	8162 1988 1988 4689 834	
2 3 4 5 6 7	" " " " " " " " " " " " " " " " " " "	12800 6300 6300 16900 2590 2590 2380	101 50 50 72 51 51 417	3.854 6.313 0.888	80.81 39.77 " 65.13 16.35	8162 1988 1988 4689 834 834	
2 3 4 5 6 7 8	" Y25 Y32 " Y12 Y32	12800 6300 6300 16900 2590 2590 2380 17970	101 50 50 72 51 51 417	3,854 6,313 0,888 6,313	80.81 39.77 " 65.13 16.35 " 2.11 113.44	8162 1988 1988 4689 834 834 880 794	
2 3 4 5 6 7 8 9	" " " " " " " " " " " " " " " " " " "	12800 6300 6300 16900 2590 2590 2380 17970 19090	101 50 50 72 51 51 417 7	3.854 6.313 " 0.888 6.313	80.81 39.77 " 65.13 16.35 " 2.11 113.44 120.52	8162 1988 1988 4689 834 834 880 794	
2 3 4 5 6 7 8 9 10	" " " " " " " " " " " " " " " " " " "	12800 6300 6300 16900 2590 2590 2380 17970 19090	101 50 50 72 51 51 417 7 6	3.854 6.313 " 0.888 6.313	80.81 39.77 " 65.13 16.35 " 2.11 113.44 120.52 81.88	8162 1988 1988 4689 834 834 880 794 723	
2 3 4 5 6 7 8 9 10 11	" " " " " " " " " " " " "	12800 6300 6300 16900 2590 2380 17970 19090 12970 2600	101 50 50 72 51 51 417 7 6 13	" 3.854 6.313 " 0.888 6.313 "	80.81 39.77 " 65.13 16.35 " 2.11 113.44 120.52 81.88 16.41	8162 1988 1988 4689 834 834 880 794 723 1064 115	kg
2 3 4 5 6 7 8 9 10 11	" " " " " " " " " " " " "	12800 6300 6300 16900 2590 2380 17970 19090 12970 2600	101 50 50 72 51 51 417 7 6 13	" 3.854 6.313 " 0.888 6.313 "	80.81 39.77 " 65.13 16.35 " 2.11 113.44 120.52 81.88 16.41	8162 1988 1988 4689 834 834 880 794 723 1064 115	kg
2 3 4 5 6 7 8 9 10 11	" " " " " " " " " " " " "	12800 6300 6300 16900 2590 2380 17970 19090 12970 2600	101 50 50 72 51 51 417 7 6 13	" 3.854 6.313 " 0.888 6.313 "	80.81 39.77 " 65.13 16.35 " 2.11 113.44 120.52 81.88 16.41 2.10	8162 1988 1988 4689 834 834 880 794 723 1064 115	kg
2 3 4 5 6 7 8 9 10 11	" " " " " " " " " " " " "	12800 6300 6300 16900 2590 2380 17970 19090 12970 2600 2370	101 50 50 72 51 51 417 7 6 13	3.854 6.313 0.888 6.313 "	80.81 39.77 65.13 16.35 2.11 113.44 120.52 81.88 16.41 2.10	8162 1988 1988 4689 834 834 880 794 723 1064 115	kg
2 3 4 5 6 7 8 9 10 11 12 13	" " " " " " " " " " " " "	12800 6300 6300 16900 2590 2380 17970 19090 12970 2600 2370	101 50 50 72 51 51 417 7 6 13	3.854 6.313 0.888 6.313 " 0.888	80.81 39.77 " 65.13 16.35 " 2.11 113.44 120.52 81.88 16.41 2.10	8162 1988 1988 4689 834 834 880 794 723 1064 115	kg
2 3 4 5 6 7 8 9 10 11 12 13	" " " " " " " " " " " " " " " " " " "	12800 6300 6300 16900 2590 2380 17970 19090 12970 2600 2370	101 50 50 72 51 51 417 7 6 13	3.854 6.313 0.888 6.313 " 0.888	80.81 39.77 " 65.13 16.35 " 2.11 113.44 120.52 81.88 16.41 2.10	8162 1988 1988 4689 834 834 880 794 723 1064 115	kg
2 3 4 5 6 7 8 9 10 11 12 13	" " " " " " " " " " " " " " " " " " "	12800 6300 6300 16900 2590 2380 17970 19090 12970 2600 2370 Y32 Y25	101 50 50 72 51 51 417 7 6 13	3.854 6.313 0.888 6.313 " 0.888 35630 k	80.81 39.77 65.13 16.35 2.11 113.44 120.52 81.88 16.41 2.10	8162 1988 1988 4689 834 834 880 794 723 1064 115	kg

LIST OF REINFORCED BAR --- B2

L J	ST O	REINE	ORCE	D BAR	B 2		
MARK	DIAMETER	LENGTH	NUMBAR	UNITWEIGHT	PIECEMEIGHT	WEIGHT	REMARK
					·	······································	<u></u>
ន 1	Y25	21800	50	3.854	84.02	400.	
2		12560	51	01034		4201	
3	Y32	12800	50	6.313	48,41 80.81	2469	
4	n ·	6210	. 98	"	39.20	1040	
5	Y20	16730	72	2.466	41.26	3842	
6	Y32	2460	50	6.313	15.53	2971	
7		2460	50	"	"	776	
8	Y12	2180	400	0.888	1.94		
9	1	19190	10	6.313	121.15	776	· · · ·
10		18070	9	",010		1212	-
11		13160	19	. "	114.08	1027	
12		2500	10	!!	83.08	1579	
13	1	2180	28	0.888	15.78	158	
-				0,000	1.94	54	
?	~					23881	K g
₩ 1	Y25	18360	19	3.854	70.76	1044	·
2	J	17850	19			1344	
			19	1.579	28.19	536	
4	Y25	15330			24.21	460	
<u>-</u> 5		15450	19	3.854	59.54	1131	<u></u>
6	11	7570	50	"	29.17	1459	
- 7		7570	50		11	1459	
		7570	. 1.	<u>"</u>	. "	29	·
8		7570	10			292	
9	Y12	1030	216	0.888	0.91	197	· :
10	"	1030	216	,, :		197	
11	<u> </u>	1050	36		0.93	33 [<u> </u>
						7137	kg
	1	00.48				7074	
F 1		22410	50	6.313	141.47	7074	
2		12800	99		80.81	8000	
3	 	6300	49	ļ	39.77	1949	
4	- 11	6300	49	11		1949	
5	2.0	16910	72	3.854	65.17	4692	
6	Y32	2590	50	6.313	16.35	817	
7	"	2590	50		11	817	
8	Y12	2380	400	0.888	2 11	844	
9	Y32	18160	10	6.313	114.64	1146	·
10		19280	9	· · · · · · · · · · · · · · · · · · ·	121.71	1095	
11	"	13160	19	. 11	83.08	1579	· .
12	"	2620	10		16.54	165	
13	Y12	2380	32	0.888	2.11	68	
						30195	kg
<u> </u>							
		Y32		38001 1	g		
· · · · · ·		Y25		17076 k	g		
		Y20		2971 1	g		
	·	Y16		996 k	g		
	· · · · · · · · · · · · · · · · · · ·	Y12		2169 k	g		
				61213 k	g		

LIST OF REINFORCED BAR --- WING 1=2

XRAK	DIAMETER	LENGTH	RABKUK	THEIGHT	PIECEMEIGHT	WEIGHT	REMARK
						Water and the second se	
v 1	Y25	11190	3	3.854	43.13	129	
2	13	5670	21	·u	21.85	459	
3	pt	4000	17	11	15.42	262	
4	H.	2990	5	n ·	11.52	58	
5	Y20	4370	30	2.466	10.78	323	<u> </u>
6	Y16	13490	1	1.579	21.30	21	
7	Y25	10920	3	3.854	42.09	126	
8	- <u>n</u>	5400	21	n	20.81	437	
9	13	4000	17	н	15.42	262	·
10		2720	5	u	10.48	52	
11	Y20	4270	29	2.466	10.53	305	
12	Y16	13450	1	1.579	21.24	21	
13	Y20	10850	. 3	2.466	26.76	80	
14	n .	5510	20	n	13.59	272	
15	Y12	4370	. 30	0.888	3.88	116	
16	Y16	13640	1	1.579	21.54	22	
17	Y20	10820	3	2.466	26.68	80	
18	11	5480	20	ti .	13.51	270	·
19	Y12	4270	29	0.888	3.79	110	
20	Y16	13600	1	1.579	21.47	21	
21	Y25	2840	21	3.854	10.95	230	
22	Y12	680	23	0.888	0.60	14	
23	Y25	2410	21	3.854	9.29	195	
24	Y12	680	23	0.888	0.60	14	·
25	п.	650	- 30	11	0.58	17	
26	11	650	29	0	п	17	
27	11	2550	41	"	2.26	93	-
28	£1	31300	2	n	27.79	56	· :
29	11	31300	2	н	11	56	·
30	11	680	2	17	0.60	1	
31	17	680	2	н	11	1	
32	Y25	3380	21	3.854	13.03	274	
33	,,	3380	21	ti .	"	274	
34	Y12	720	46	0.888	0.64	29	
35	n	720	41	ч	n .	26	
						4723	kg
					· · · · · · · · · · · · · · · · · · ·		-,
		Y25		2758 1			
		Y20		1330 l			
·		Y16		85 1		· · · · · · · · · · · · · · · · · · ·	<u> </u>
	<u></u>	Y12		550 1			
				4723 I	(g		

LIST OF REINFORCED BAR---B1 = B2

					-		
WARK	RETERATO	LENGTH	NUMBAR	UNITWEIGHT	PIECEWEIGHT	WEIGHT	REMARK
5 1	Y32	18110	47	6.313	114.33	5374	
2	.,	9280	46	"	58,58	2695	
3	Y25	9280	47	3.854	35.77	1681	
4	Ý32	5230	92	6.313	33.02	3038	
5	Y25	14510	60	3.854	55.92	3355	
6	Y32	1890	47	6.313	11.93	561	L
7	ju ju	1890	47	**	11	561	
8	Y12	1800	286	0.888	1.60	458	
						17723	kg
· · · · · · · · · · · · · · · · · · ·		<u> </u>		:			
W 1	Y25	14510	19	3.854	55.92	1062	
2	Y16	14190	19	1.579	22.41	426	
3	.,,	14190	19		н	426	
4	Y25	14510	19	3.854	55.92	1062	
5	.,,	6970	47	н	26.86	1262	
6	н	6970	47	"	"	1262	
7	Y12	820	203	0.888	0.73	148	
8		820	203	11		148	
			·			5796	kg
		·	· .	1 - 1		· · · · · · · · · · · · · · · · · · ·	
F 1	Y32	18290	47	6.313	115.46	5427	,
2	* 111	9280	93	12	58.58	5448	:
. 3	"	5320	46	"	33.59	1545	
4	1,9	5320	46	n	**	1545	
5	Y25	14510	60	3.854	55.92	3355	
6	Y32	2020	47	6.313	12.75	599	
7	"	2020	47	17	. 11	599	
8	Y12	1980	286	0,888	1.76	503	Ĺ <u> </u>
· · · · ·					<u> </u>	19021	kg
		Y32		27392 1	(g		
	<u> </u>	Y25		13039	cg .		
		Y16		852 1	(g	<u></u>	
		Y12		1257 }	(g		

LIST OF REINFORCED BAR---WING1=2

MA.	KK	DIAMETER	LENGTH	NUMBAR	UNITWEIGHT	PIECEWEIGHT	WEIGHT	REMARK
		· · · · · · · · · · · · · · · · · · ·				***************************************		
4	1_	Y25	9560	4	3.854	36.84	147	
	_2		4940	19		19.04	362	
	3		4000	16	11	15.42	247	
	4	.,	2610	6	11	10.06	60	
	5	Y20	4270	28	2.466	10.53	. 295	
	6	Y16	12760	2	1.579	20.15	40	
	7	Y25	9960	4	3.854	38.39	154	
	8	5)	5540	20	1)	21.35	427	
	9	11	4000	16	п	15.42	247	
	10	,,	2610	6	11	10.06	60	
·:	11	Y20	4110	29	2.466	10.14	294	
	12	Y16	13050	2	1.579	20.61	41	
	13	Y20	9480	. 4.	2.466	23.38	94	
	14	31	5240	19	n	12.92	245	
	15	Y12	4270	28	0.888	3.79	106	
<u></u>	16	Y20	9880	4	2.466	24.36	97	
	17	11	5460	20	н .	13.46	269	
	18	Y12	4110	29	0.888	3.65	106	
	19	Y25	2300	20	3.854	8.86	177	
	20	Y12	680	24	0.888	0.60	14	
	21	Y25	2300	20	3.854	8.86	177	
:	22	Y12	680	24	0.888	0.60	14	
	23	11	650	28		0.58	16	
	24	. 11	650	29	!!	11	17	- :
	25	"	2330	32	.**	2.07	66	
	26	"	27720	2		24.62	49	
	27	Y25	3380	20	3.854	13.03	261	
	28	,,	3380	20	u ·	'n	261	
	29	Y12	720	39	0.888	0.64	25	
· ·	30		720	42	.,	<u> </u>	27	:
							4395	kg
					<u> </u>	· · · · · · · · · · · · · · · · · · ·	· · · · · · · · · · · · · · · · · · ·	
			Y25		2580 J	(g		
		20	Y20		1294 }	(g		***************************************
	:		Y16		81 }	κg		
			Y12		440 1	(g		
		·			4395 l	rg		

LIST OF REINFORCED BAR---BI

\$ 1	L I		REINF	T	***************************************			
1	MARK	DIAMETER	LENGTH	NUMBAR	UNITWEIGHT	THOLOWSOSIG	YEIGHT	REMARK
1	1.	19.5			!			
1	S 1	Y32	23630	5.2	6 210			
3 Y25								
4 Y32	***	Y25			3 954			
S Y25								
6 " 21270 3 " 81.97 246 7 Y32 3300 52 6.313 20.83 1083 8 " 3300 52 " " 1083 8 " 3300 52 " " 1083 9 Y12 2750 475 0.888 2.44 1159 10 Y32 22850 29 6.313 144.25 4183 11 " 21730 28 " 137.18 3841 12 " 15300 29 " 96.59 2801 13 Y25 15060 28 3.854 58.04 1625 14 Y32 3550 26 6.313 22.41 583 15 Y12 2750 121 0.888 2.44 295 15 Y12 2750 121 0.888 2.44 295 2 Y16 13790 25 3.854 52.49 1312 2 Y16 13790 25 3.854 52.49 1312 2 Y16 13790 25 3.854 83.21 2080 5 " 8370 52 " 31.00 775 6 " 8370 52 " 32.26 1678 8 " 8370 1 " " 32.26 1678 8 " 8370 1 " " 32.26 1678 8 " 8370 1 " " 32 9 Y12 1320 12 0.888 1.17 14 10 " 1320 275 " " 322 11 " 1490 132 " 1.32 174 11 " 1490 132 " 1.32 174 11 " 1490 132 " 1.32 174 11 " 1490 132 " 1.32 174 11 " 1490 132 " 1.32 174 11 " 1490 132 " 1.32 174 11 " 1490 132 " 1.32 174 11 " 1490 132 " 1.32 174 11 " 1490 132 " 1.32 174 11 " 1270 3 " 81.97 246 11 Y32 6990 102 6.313 144.13 4501 15 Y25 15300 29 " 81.97 246 17 Y32 13400 52 " 81.97 246 18 " 3430 52 " " 1126 19 Y12 1270 3 " 81.97 246 10 Y32 12820 29 6.313 137.75 3996 11 " 22940 28 " 144.82 4055 12 " 15300 29 " 96.59 2801 13 Y25 15300 29 " 96.59 2801 14 Y32 3680 26 6.313 137.75 3996 15 Y12 2930 121 0.888 2.60 1235 11 Y32 2360 28 3.854 58.04 1625 12 " 15300 29 " 96.59 2801 13 Y25 27549 kg Y16 1319 kg Y12 3514 kg			·					·
Y Y Y Y Y Y Y Y Y Y				F				
8 " 3300 52 " " 1083 9 Y12 2750 475 0.888 2.44 1159 10 Y32 22850 29 6.313 144.25 4183 11 " 21730 28 " 137.18 3841 12 " 15300 29 " 96.59 2801 13 Y25 15060 28 3.854 58.04 1625 14 Y32 3550 26 6.313 22.41 583 15 Y12 2750 121 0.888 2.44 295 11 Y25 13600 25 121 0.888 2.44 295 11 Y25 13600 25 1.579 21.77 544 1592 kg 11 Y25 13600 25 3.854 52.49 1312 2 Y18 13790 25 1.579 21.77 544 13 " 19630 25 " 31.00 775 14 Y25 21590 25 3.854 83.21 2080 5 " 8370 52 " 31.00 775 6 " 8370 52 " " 32.26 1678 6 " 8370 52 " " 32.26 1678 8 " 8370 1 " " 32 9 Y12 1320 12 0.888 1.17 14 10 " 1320 275 " " 322 11 " 1490 132 " 1.32 174 9 Y12 13400 52 " 84.69 4399 3 Y25 13160 61 3.654 50.72 2587 4 Y32 6990 102 6.313 150.31 7816 5 Y25 16810 85 3.854 64.79 5507 6 " 21270 3 " " 1126 9 Y12 2930 475 0.888 2.60 1235 10 Y32 21820 29 6.313 137.75 3996 11 " 22940 28 " 144.82 11 Y32 23810 52 6.313 21.65 1126 11 " 22940 28 " 144.82 11 " 22940 28 " 144.82 11 " 22940 28 " 144.82 11 " 22940 28 " 144.82 11 Y32 3544 kg		Y32			6.313			
9 Y12 2750 475 0.888 2.44 1159 10 Y32 22850 29 6.313 144.25 4183 11 " 21730 28 " 137.18 3841 12 " 15300 29 " 96.59 2801 13 Y25 15060 28 3.854 58.04 1625 14 Y32 3550 26 6.313 22.41 583 15 Y12 2750 121 0.888 2.44 295								
10		Y12						
11		-						
12 " 15300 29 " 96.59 2801 13 Y25 15060 28 3.854 58.04 1625 14 Y32 3650 26 6.313 22.41 583 15 Y12 2750 121 0.888 2.44 295		at .	11.5					
13 Y25 15060 28 3.854 58.04 1625 14 Y32 3550 26 6.313 22.41 583 15 Y12 2750 121 0.888 2.44 295					"			·
14 Y32 3550 26 6.313 22.41 583 15 Y12 2750 121 0.888 2.44 295 41592 kg 7 1 Y25 13620 25 3.854 52.49 1312 2 Y16 13790 25 1.579 21.77 544 3 " 19630 25 " 31.00 775 4 Y25 21590 25 3.854 83.21 2080 5 " 8370 52 " 32.26 1678 6 " 8370 52 " " 1678 7 " 8370 26 " " 32.26 1678 8 " 8370 1 " " 32 9 Y12 1320 12 0.888 1.17 14 10 " 1320 275 " " 322 11 " 1490 132 " 1.32 174 9448 kg 1 Y32 23810 52 6.313 150.31 7816 2 " 13400 52 " 84.59 4399 3 Y25 13160 61 3.854 50.72 2587 4 Y32 6990 102 6.313 44.13 4501 5 Y25 16810 85 3.854 64.79 5507 6 " 21270 3 " 81.99 246 6 " 3430 52 " " 1126 9 Y12 2330 475 0.888 2.60 1235 10 Y32 21820 29 6.313 137.75 3996 11 " 22940 28 " 144.82 4055 12 " 15300 29 " 96.59 2801 13 Y25 15660 28 3.854 58.04 1625 14 Y32 2930 121 0.888 2.60 315 15 Y12 2930 121 0.888 2.60 315 17 Y32 3680 26 6.313 23.23 604 15 Y12 2930 121 0.888 2.60 315 17 Y32 3680 26 6.313 23.23 604 17 Y32 3680 26 6.313 23.23 604 18 Y32 60596 kg Y25 27549 kg Y16 1319 kg Y12 1319 kg Y12 1319 kg Y12 3514 kg		Y25				-		
15 Y12 2750 121 0.888 2.44 295								· · · · · · · · · · · · · · · · · · ·
1			3,00		0.000	2.44		le or
2 Y16 13790 25 1.579 21.77 544 3 " 19630 25 " 31.00 775 4 Y25 21590 25 3.854 83.21 2080 5 " 8370 52 " 32.26 1678 6 " 8370 52 " " 1678 7 " 8370 26 " " 32.26 1678 8 " 8370 1 " " 32 9 Y12 1320 12 0.888 1.17 14 10 " 1320 275 " " 322 11 " 1490 132 " 1.32 174 9448 kg 1 Y32 23810 52 6.313 150.31 7816 2 " 13400 52 " 84.59 4399 3 Y25 13160 51 3.854 50.72 2587 4 Y32 6990 102 6.313 44.13 4501 5 Y25 16810 85 3.854 64.79 5507 6 " 21270 3 " 81.97 246 8 " 3430 52 " " 1126 9 Y12 2930 475 0.888 2.60 1235 10 Y32 21820 29 6.313 137.75 3995 11 " 22940 28 " 144.82 4055 12 " 15300 29 " 96.59 2801 13 Y25 15060 28 3.854 58.04 1625 14 Y32 3680 26 6.313 23.23 604 15 Y12 2930 121 0.888 2.60 315 14 Y32 3680 26 6.313 23.23 604 15 Y12 2930 121 0.888 2.60 315 173 Y25 15060 28 3.854 58.04 1625 14 Y32 3680 26 6.313 23.23 604 15 Y12 2930 121 0.888 2.60 315 41938 kg							41592	кв
2 Y16 13790 25 1.579 21.77 544 3 " 19630 25 " 31.00 775 4 Y25 21590 25 3.854 83.21 2080 5 " 8370 52 " 32.26 1678 6 " 8370 52 " " 1678 7 " 8370 26 " " 32.26 1678 8 " 8370 1 " " 32 9 Y12 1320 12 0.888 1.17 14 10 " 1320 275 " " 322 11 " 1490 132 " 1.32 174 9448 kg 1 Y32 23810 52 6.313 150.31 7816 2 " 13400 52 " 84.59 4399 3 Y25 13160 51 3.854 50.72 2587 4 Y32 6990 102 6.313 44.13 4501 5 Y25 16810 85 3.854 64.79 5507 6 " 21270 3 " 81.97 246 8 " 3430 52 " " 1126 9 Y12 2930 475 0.888 2.60 1235 10 Y32 21820 29 6.313 137.75 3995 11 " 22940 28 " 144.82 4055 12 " 15300 29 " 96.59 2801 13 Y25 15060 28 3.854 58.04 1625 14 Y32 3680 26 6.313 23.23 604 15 Y12 2930 121 0.888 2.60 315 14 Y32 3680 26 6.313 23.23 604 15 Y12 2930 121 0.888 2.60 315 173 Y25 15060 28 3.854 58.04 1625 14 Y32 3680 26 6.313 23.23 604 15 Y12 2930 121 0.888 2.60 315 41938 kg	W 1	Y25	13620	25	3.854	52 40	: 1212	
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4 Y25 21590 25 3.854 83.21 2080 5 " 8370 52 " 32.26 1678 6 " 8370 52 " " 1678 7 " 8370 1 " " 32 9 Y12 1320 12 0.888 1.17 14 10 " 1320 275 " " 322 11 " 1490 132 " 1.32 174 2 " 13400 52 " 84.59 4399 3 Y25 13160 51 3.854 50.72 2587 4 Y32 6990 102 6.313 44.13 4501 5 Y25 16810 85 3.854 64.79 5507 6 " 21270 3 " 81.97 246 7 Y32 3430 52 6.313 21.65 1126 8 " 3430 5								
5 " 8370 52 " 32.26 1678 6 " 8370 52 " " 1678 7 " 8370 26 " " 839 8 " 8370 1 " " 32 9 Y12 1320 12 0.888 1.17 14 10 " 1320 275 " " 322 11 " 1490 132 " 1.32 174 9448 kg 1 Y32 23810 52 6.313 150.31 7816 2 " 13400 52 " 84.59 4399 3 Y25 13160 51 3.854 50.72 2587 4 Y32 6990 102 6.313 44.13 4501 5 Y25 16810 85 3.854 64.79 5507 6 " 21270 3 " 81.97 246 7		Y25			3.854			·
6 " 8370 52 " " 1678 7 " 8399 8 " 8370 1 " " 8399 8 " 8370 1 " " 32 2 9 9 Y12 1320 12 0.888 1.17 14 10 " 1320 275 " " 322 11 " 1490 132 " 1.32 174 9448 kg 944								
7 " 8370 26 " " 839 88 8 " 8370 1 " " 32 9 Y12 1320 12 0.888 1.17 14 14 10 " 1320 275 " " 322 174 11 " 1490 132 " 1.32 174 11					11			
8 " 8370 1 " " 32 9 Y12 1320 12 0.888 1.17 14 10 " 1320 275 " " 322 11 " 1490 132 " 1.32 174					*1			•
9 Y12 1320 12 0.888 1.17 14 10 " 1320 275 " " 322 11 " 1490 132 " 1.32 174				ļ	,,	,,		
10 " 1320 275 " " 322 11 " 1490 132 " 1.32 174 9448 kg 7 1 Y32 23810 52 6.313 150.31 7816 2 " 13400 52 " 84.59 4399 3 Y25 13160 61 3.854 50.72 2587 4 Y32 6990 102 6.313 44.13 4501 5 Y25 16810 85 3.854 64.79 5507 6 " 21270 3 " 81.97 246 7 Y32 3430 52 6.313 21.65 1126 8 " 3430 52 " " 1126 8 " 3430 52 " " 1126 9 Y12 2930 475 0.888 2.60 1235 10 Y32 21820 29 6.313 137.75 3995 11 " 22940 28 " 144.82 4055 12 " 15300 29 " 96.59 2801 13 Y25 15060 28 3.854 58.04 1625 14 Y32 3680 26 6.313 23.23 604 15 Y12 2930 121 0.888 2.60 315	9	Y12			0.888	1,17		
11 " 1490 132 " 1.32 174 9448 kg 2 1 Y32 23810 52 6.313 150.31 7816 2 " 13400 52 " 84.59 4399 3 Y25 13160 61 3.654 50.72 2587 4 Y32 6990 102 6.313 44.13 4501 5 Y25 16810 85 3.854 64.79 5507 6 " 21270 3 " 81.97 246 7 Y32 3430 52 6.313 21.65 1126 8 " 3430 52 " " 1126 9 Y12 2930 475 0.888 2.60 1235 10 Y32 21820 29 6.313 137.75 3995 11 " 22940 28 " 144.82 4055 12 " 15300 29 " 96.59 2801 13 Y25 15060 28 3.854 58.04 1625 14 Y32 3680 26 6.313 23.23 604 15 Y12 2930 121 0.888 2.60 315		"						
9448 kg 1	7.7	**			11	1.32		
2 " 13400 52 " 84.59 4399 3 Y25 13160 51 3.854 50.72 2587 4 Y32 6990 102 6.313 44.13 4501 5 Y25 16810 85 3.854 64.79 5507 6 " 21270 3 " 81.97 246 7 Y32 3430 52 6.313 21.65 1126 8 " 3430 52 " " 1126 9 Y12 2930 475 0.888 2.60 1235 10 Y32 21820 29 6.313 137.75 3995 11 " 22940 28 " 144.82 4055 12 " 15300 29 " 96.59 2801 13 Y25 15060 28 3.854 58.04 1625 14 Y32 3680 26 6.313 23.23 604 15 Y12				L				kg
2 " 13400 52 " 84.59 4399 3 Y25 13160 51 3.854 50.72 2587 4 Y32 6990 102 6.313 44.13 4501 5 Y25 16810 85 3.854 64.79 5507 6 " 21270 3 " 81.97 246 7 Y32 3430 52 6.313 21.65 1126 8 " 3430 52 " " 1126 9 Y12 2930 475 0.888 2.60 1235 10 Y32 21820 29 6.313 137.75 3995 11 " 22940 28 " 144.82 4055 12 " 15300 29 " 96.59 2801 13 Y25 15060 28 3.854 58.04 1625 14 Y32 3680 26 6.313 23.23 604 15 Y12								
2 " 13400 52 " 84.59 4399 3 Y25 13160 51 3.854 50.72 2587 4 Y32 6990 102 6.313 44.13 4501 5 Y25 16810 85 3.854 64.79 5507 6 " 21270 3 " 81.97 246 7 Y32 3430 52 6.313 21.65 1126 8 " 3430 52 " " 1126 9 Y12 2930 475 0.888 2.60 1235 10 Y32 21820 29 6.313 137.75 3995 11 " 22940 28 " 144.82 4055 12 " 15300 29 " 96.59 2801 13 Y25 15060 28 3.854 58.04 1625 14 Y32 3680 26 6.313 23.23 604 15 Y12	F 1	Y32	23810	52	6.313	150.31	7816	
3 Y25 13160 61 3.854 50.72 2587 4 Y32 6990 102 6.313 44.13 4501 5 Y25 16810 85 3.854 64.79 5507 6 " 21270 3 " 81.97 246 7 Y32 3430 52 6.313 21.65 1126 8 " 3430 52 " " 1126 9 Y12 2930 475 0.888 2.60 1235 10 Y32 21820 29 6.313 137.75 3995 11 " 22940 28 " 144.82 4055 12 " 15300 29 " 96.59 2801 13 Y25 15060 28 3.854 58.04 1625 14 Y32 3680 26 6.313 23.23 604 15 Y12 2930 121 0.888 2.60 315 4193	. 2	••		52	н	0.1.00	4399	
4 Y32 6990 102 6.313 44.13 4501 5 Y25 16810 85 3.854 64.79 5507 6 " 21270 3 " 81.97 246 7 Y32 3430 52 6.313 21.65 1126 8 " 3430 52 " " 1126 9 Y12 2930 475 0.888 2.60 1235 10 Y32 21820 29 6.313 137.75 3996 11 " 22940 28 " 144.82 4055 12 " 15300 29 " 96.59 2801 13 Y25 15060 28 3.854 58.04 1625 14 Y32 3680 26 6.313 23.23 604 15 Y12 2930 121 0.888 2.60 315 41938 kg Y32 60596 kg Y25 27549 kg		Y25			3.854			
5 Y25 16810 85 3.854 64.79 5507 6 " 21270 3 " 81.97 246 7 Y32 3430 52 6.313 21.65 1126 8 " 3430 52 " " 1126 9 Y12 2930 475 0.888 2.60 1235 10 Y32 21820 29 6.313 137.75 3995 11 " 22940 28 " 144.82 4055 12 " 15300 29 " 96.59 2801 13 Y25 15060 28 3.854 58.04 1625 14 Y32 3680 26 6.313 23.23 604 15 Y12 2930 121 0.888 2.60 315 41938 kg Y25 27549 kg Y16 1319 kg Y12 3514 kg			·					
6 " 21270 3 " 81.97 246 7 Y32 3430 52 6.313 21.65 1126 8 " 3430 52 " " 1126 9 Y12 2930 475 0.888 2.60 1235 10 Y32 21820 29 6.313 137.75 3995 11 " 22940 28 " 144.82 4055 12 " 15300 29 " 96.59 2801 13 Y25 15060 28 3.854 58.04 1625 14 Y32 3680 26 6.313 23.23 604 15 Y12 2930 121 0.888 2.60 315 Y32 60596 kg Y25 27549 kg Y16 1319 kg Y12 3514 kg								
7 Y32 3430 52 6.313 21.65 1126 8 " 3430 52 " " 1126 9 Y12 2930 475 0.888 2.60 1235 10 Y32 21820 29 6.313 137.75 3995 11 " 22940 28 " 144.82 4055 12 " 15300 29 " 96.59 2801 13 Y25 15060 28 3.854 58.04 1625 14 Y32 3680 26 6.313 23.23 604 15 Y12 2930 121 0.888 2.60 315 41938 kg Y32 60596 kg Y25 27549 kg Y16 1319 kg								
8 " 3430 52 " " 1126 9 Y12 2930 475 0.888 2.60 1235 10 Y32 21820 29 6.313 137.75 3995 11 " 22940 28 " 144.82 4055 12 " 15300 29 " 96.59 2801 13 Y25 15060 28 3.854 58.04 1625 14 Y32 3680 26 6.313 23.23 604 15 Y12 2930 121 0.888 2.60 315 41938 kg Y25 27549 kg Y16 1319 kg Y12 3514 kg					6.313			
9 Y12 2930 475 0.888 2.60 1235 10 Y32 21820 29 6.313 137.75 3995 11 " 22940 28 " 144.82 4055 12 " 15300 29 " 96.59 2801 13 Y25 15060 28 3.854 58.04 1625 14 Y32 3680 26 6.313 23.23 604 15 Y12 2930 121 0.888 2.60 315 41938 kg Y32 60596 kg Y25 27549 kg Y16 1319 kg Y12 3514 kg	8						1126	
10 Y32 21820 29 6.313 137.75 3995 11 " 22940 28 " 144.82 4055 12 " 15300 29 " 96.59 2801 13 Y25 15060 28 3.854 58.04 1625 14 Y32 3680 26 6.313 23.23 604 15 Y12 2930 121 0.888 2.60 315 41938 kg Y32 60596 kg Y25 27549 kg Y16 1319 kg Y12 3514 kg		Y12			0.888	2.60	1235	
11 " 22940 28 " 144.82 4055 12 " 15300 29 " 96.59 2801 13 Y25 15060 28 3.854 58.04 1625 14 Y32 3680 26 6.313 23.23 604 15 Y12 2930 121 0.888 2.60 315 41938 kg Y32 60596 kg Y25 27549 kg Y16 1319 kg Y12 3514 kg	10					137.75	3995	
12 " 15300 29 " 96.59 2801 13 Y25 15060 28 3.854 58.04 1625 14 Y32 3680 26 6.313 23.23 604 15 Y12 2930 121 0.888 2.60 315 41938 kg Y32 60596 kg Y25 27549 kg Y16 1319 kg Y12 3514 kg							4055	
13 Y25 15060 28 3.854 58.04 1625 14 Y32 3680 26 6.313 23.23 604 15 Y12 2930 121 0.888 2.60 315 41938 kg Y32 60596 kg Y25 27549 kg Y16 1319 kg Y12 3514 kg		11	7 7 7 7		9		2801	
14 Y32 3680 26 6.313 23.23 604 15 Y12 2930 121 0.888 2.60 315 41938 kg Y32 60596 kg Y25 27549 kg Y16 1319 kg Y12 3514 kg		Y25			3.854	58.04	1625	
15 Y12 2930 121 0.888 2.60 315 41938 kg Y32 60596 kg Y25 27549 kg Y16 1319 kg Y12 3514 kg				26		23.23	604	
Y32 60596 kg Y25 27549 kg Y16 1319 kg Y12 3514 kg						2.60	315	
Y25 27549 kg Y16 1319 kg Y12 3514 kg							41938	kg
Y16 1319 kg Y12 3514 kg			Y32		60596 I	rg		
Y12 3514 kg			Y25		27549 H	rg		
Y12 3514 kg		····			1319 F	g		
		*···			3514 H	(g		,

LIST OF REINFORCED BAR --- B2

er Protesta and			And the last of th	·	The state of the s			
MAR	ĸ	DIAMETER	LENGTH	NUMBAR	UNITWEIGHT	Pleceweight	WEIGHT	REMARK
3	1	Y32	23630	76	6.313	149.18	11338	
	2	.,	13400	76	**	84.59	6429	<u> </u>
	3	Y25	13160	75	3.854	50.72	3804	· · · · · · · · · · · · · · · · · · ·
	4	Y32	6900	150	6.313	43.56	6534	
	5	Y25	13980	88	3.854	53.88	4741	
	6	Y32	3300	76	6.313	20.83	1583	· · · · · · · · · · · · · · · · · · ·
	7	0	3300	76	13	"	1583	
	8	Y12	2750	693	0.888	2.44	1691	
· .			and the second second				37703	kg
	r							
W :	1	Y25	20920	,25	3.854	80.63	2016	
	2	Y16	19280	25	1.579	30.44	761	
	3	,,	18050	25	. "	28.50	713	
<u></u>	4	Y25	18540	25	3.854	71.45	1786	
· <u>·</u>	5	n .	8370	76	,,	32.26	2452	
	6	.,	8370	76	11	11	2452	
	7	Y12	1320	407	0.888	1.17	476	
	8	.,	1320	407	,	"	476	
			· '.		·		11132	kg
		· · · · · · · · · · · · · · · · · · ·						
F	1	Y32	23810	76	6.313	150.31	11424	
	2		13400	76	,,	84.59	6429	
	3	Y25	13160	75	3.854	50.72	3804	
	4	Y32	6990	150	6.313	44.13	6620	
	5	Y25	18980	88	3.854	73.15	6437	
	6	Y32	3430_	76	6.313	21.65	1645	
	7	,,	3430	76	н .	"	1645	
	8	Y12	2930	693	0.888	2.60	1802	
							39806	kg
			· · · · · · · · · · · · · · · · · · ·					
			Y32	· · · · · · · · · · · · · · · · · · ·	55230 k			
			Y25		27492 k	_		
		<u> </u>	Y16		1474 k			
			Y12		4445 k	g		

LIST OF REINFORCED BAR -- B3

WARK	1	1	·	D BAR			
,	DIAMETER	LENGTH	NUMBAR	UNITACIONE	PIECEWEIGHT	WEIGHT	REMARK
							<u>'</u>
s ı	Y32	23630	47	6.313	140 10		
2	11	13400	47	"	149.18	7011	
3	Y25	13160	46	3.854	84.59	3976	
4	Y32	6900	92	6.313	50.72	2333	
5	Y25	16180	85	3.854	43.56	4008 5301	· · ·
6	is H	21250	3	н :	81.90	246	
7	Y32	3300	47	6.313	20.83	979	
8	n'	3300	47	. 11		979	
9	Y12	2750	418	0.888	2.44	1020	
10	Y32	23480	34	6.313	148.23	5040	
71		22360	33	"	141.16	4658	
12	: "	15930	34		100.57	3419	· ·····
13	Y25	15690	33	3.854	60.47	1996	
14	Y32	3640	31	6.313	22.98	712	
. 15	Y12	2750	141	0.888	2.44	344	<u>.</u>
	••	· 		 -		42022	kg
<u> </u>	· ·						
1	Y25	22180	25	3.854	85.48	2137	
2	Y16	19700	25	1.579	31.11	778	
3		12870	25	*	20.32	508	· · .
4	Y25	12460	25	3.854	48.02	1201	· · · · · · · · · · · · · · · · · · ·
5		8370	47		32.26	1516	
6	. "	8370	47		"	1516	
7	-"-	8370	1	"		32	
8		8370	31		11	1000	·
9	Y12	1320	242	0.888	1.17	283	
10		1320	242		41	283	····
11		1540	159	"	1.37	218	
						9472	kg
7 1	Y32	23810	47	6.313	150 21	7005	
	132	13400	47	11	150.31 84.59	7065 3976	····
3	Y25	13160	46	3.854	50.72	2333	
4	Y32	6990	92	6.313	44.13	4060	
- 5	Y25				17110	- 3000	
		16180	0:1	4.650	62.36	5301	
6	"	16180 21250	85 3	3.854	62.36 81.90	5301 246	
6 7		21250	3	н	81.90	246	
		21250 3430				246 1018	
7	" Y32	21250 3430 3430	3 47 47	6.313	81.90 21.65	246 1018 1018	
8	" Y32	21250 3430	3 47	6.313	81.90 21.65 " 2.60	246 1018	
7 8 9	" Y32 " Y12	21250 3430 3430 2930	3 47 47 418	6.313 " 0.888	81.90 21.65	246 1018 1018 1087	
7 8 9	Y32 " Y12 Y32	21250 3430 3430 2930 22450	3 47 47 418 34	6.313 " 0.888 6.313	81.90 21.65 " 2.60 141.73	246 1018 1018 1087 4819	
7 8 9 10	Y32 " Y12 Y32 "	21250 3430 3430 2930 22450 23570	3 47 47 418 34 33	0.888 6.313	81.90 21.65 " 2.60 141.73 148.80	246 1018 1018 1087 4819 4910	
7 8 9 10 11	Y32 " Y12 Y32 "	21250 3430 3430 2930 22450 23570 15930	3 47 47 418 34 33 34	0.888 6.313 	81.90 21.65 " 2.60 141.73 148.80 100.57	246 1018 1018 1087 4819 4910 3419	
7 8 9 10 11 12	Y32 " Y12 Y32 " " Y25	21250 3430 3430 2930 22450 23570 15930	3 47 47 418 34 33 34 33	" 0.888 6.313 " 3.854	81.90 21.65 " 2.60 141.73 148.80 100.57 60.47	246 1018 1018 1087 4819 4910 3419, 1996	
7 8 9 10 11 12 13	Y32 Y12 Y32 " Y25 Y32	21250 3430 3430 2930 22450 23570 15930 15690 3760	3 47 47 418 34 33 34 33 31	" 6,313 " 0.888 6,313 " " 3,854 6,313	81.90 21.65 " 2.60 141.73 148.80 100.57 60.47 23.74	246 1018 1018 1087 4819 4910 3419 1996 736	
7 8 9 10 11 12 13 14	Y32 Y12 Y32 " Y25 Y32 Y12	21250 3430 3430 2930 22450 23570 15930 15690 3760 2930	3 47 47 418 34 33 34 33 31	" 6,313 " 0.888 6,313 " " 3,854 6,313 0.888	81.90 21.65 " 2.60 141.73 148.80 100.57 60.47 23.74 2.60	246 1018 1018 1087 4819 4910 3419 1996 736 374	kg
7 8 9 10 11 12 13 14	Y32 Y12 Y32 " Y25 Y32 Y12	21250 3430 3430 2930 22450 23570 15930 15690 3760 2930	3 47 47 418 34 33 34 33 31	" 6,313 " 0.888 6,313 " " 3,854 6,313 0.888	81.90 21.65 " 2.60 141.73 148.80 100.57 60.47 23.74 2.60 0.00	246 1018 1018 1087 4819 4910 3419 1996 736 374	kg
7 8 9 10 11 12 13 14	Y32 Y12 Y32 " Y25 Y32 Y12	21250 3430 3430 2930 22450 23570 15930 15690 3760 2930	3 47 47 418 34 33 34 33 31	" 6,313 " 0.888 6,313 " " 3,854 6,313 0.888 0.395	81.90 21.65 " 2.60 141.73 148.80 100.57 60.47 23.74 2.60 0.00	246 1018 1018 1087 4819 4910 3419 1996 736 374	kg
7 8 9 10 11 12 13 14	Y32 Y12 Y32 " Y25 Y32 Y12	21250 3430 3430 2930 22450 23570 15930 15690 3760 2930 1	3 47 47 418 34 33 34 33 31	" 6,313 " 0.888 6,313 " 3.854 6.313 0.888 0.395	81.90 21.65 " 2.60 141.73 148.80 100.57 60.47 23.74 2.60 0.00	246 1018 1018 1087 4819 4910 3419 1996 736 374	kg
7 8 9 10 11 12 13 14	Y32 Y12 Y32 " Y25 Y32 Y12	21250 3430 2930 22450 23570 15930 15690 3760 2930 1	3 47 47 418 34 33 34 33 31	" 6,313 " 0.888 6,313 " 3.854 6,313 0.888 0.395	81.90 21.65 " 2.60 141.73 148.80 100.57 60.47 23.74 2.60 0.00	246 1018 1018 1087 4819 4910 3419 1996 736 374	kg

LIST OF REINFORCED BAR --- WINGI

X/	\RX	DIAMETER	LENGTH	NUMBAR	UNITWEIGHT	PIECEWEIGHT	WEIGHT .	REMARK
	:							:
W	1	Y25	9370	4	3.854	36.11	144	
	2	,,	5580	17	tř	21.51	366	
	3	11	4690	18	H	18.08	325	
	4	••	6050	5	11	23.32	117	· · · · · · · · · · · · · · · · · · ·
	5	11	4450	6	11	17.15	103	
	6	Y20	9860	4	2.466	24.31	97	
	7	11	5840	18	11	14.40	259	
	8	3/11	5970	5	11	14.72	74	
	9	Y25	2620	23	3.854	10.10	232	
	10	Y20	4570	25	2.466	11.27	282	······································
	11	Y16	4840	27	1.579	7.64	206	
	12	Y12	11670	11_	0.888	10.36	10	
	13	,,	11440	1	. **	10.16	10	· · · · · · · · · · · · · · · · · · ·
	14	19	22940	1	18	20.37	20	
	15	,,	23510	1	,,	20.88	21	
	16	"	3270	2	.,	2.90	6	
	17		3200	48	ii.	2.84	136	
	18	••	640	27	11	0.57	15	
	19	Y25	680	5	3.854	2.62	13	
	20	11	680	18_	11	11	47	
	21	Y12	710	45	0.888	0.63	28	
							2511	kg
					1947	lo.or		
-			Y25		1347			
-			Y20		712	•	······································	
			Y16		206			
			<u>Y12</u>		246			
					2511	Kg		~.

BOXCULVERT FOR ROAD -NO (3) LIST OF REINFORCED BAR --- WING2

*	····	1	<u> </u>	γ		WINOZ		
	NARX	MATERALO	LENGTH	RYBHAK	UNITYEIGHT	PIECEVEIGHT	AEICHI	REMARK
				Γ	<u> </u>			
	w 1	Y25	12500	6_	3.854	48.18	289	· · · · · · · · · · · · · · · · · · ·
	2		11610	3_		44.74	134	
	3	1 ''' 1	6010	18		23.16	417	
	4	1	4680	19		18.04	343	· <u> </u>
	5		3530	6	· · ·	13.60	82	· .
ļ	6	Y32	14500	1_	6.313	91.54	92	
			14360	3_	11	90.65	272	·
j	8	- "	12410	8_	.,	78.34	627	
	9		5710	17	"	36.05	613	
	10	j j	6000	17	·	37.88	644	
	11		4420	. 8		27.90	223	
	12	Y20	12630	6_	2.466	31.15	187	
	13	"	10770	4	· 11	26.56	106	
	14		5840	18	1)	14.40	259	
-	15	Y25	14000	1	3.854	53.96	54	
	16		14220	3_	11.	54.80	164	
	17	"	11960	7		46.09	_323	
	18		5660	17		21.81	371	
	19	Y20	4860	32	2.466	11.98	383	
	20	Y25	4840	37	3.854	18.65	690	
Ì	21	716	5070	34	1.579	8.01	272	
	22	" "	4750	.36	11	7.50	270	
	23	"	15650	1	11	24.73	25	
	24	"	15160	1	11	23.94	24	
	25	"	17030	1	11.	26.89	27	
	26	11	16550	1	и.	26.13	26	
	27	Y12	37500	2	0.888	33.30	67	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,
[28		2460	24	3.854	9.48	228	
ļ	29		4060	24	6.313	25.63	615	
1	30	1	730	34	0.888	0.65	22	
	31	"	730	36	"	"	23	
,	32	"	3270	50	"	2.90	145	
	33		770	27	1.579	1.22	33	
	34	"	770	30	11073		37	
	35		810	59	0.888	0.72	42	···
	36	1		64	".	"	46	·
	30	Il	810	<u> </u>	I		8175	kø
								26
			Y32		3086	CE		<u> </u>
		· .	Y25		3095 1			
					935			
			Y20		714			
		1 · · · · · · · · · · · · · · · · · · ·	Y1]6		345			
			¥1.					
	<u> </u>		<u>. ft</u>	······································	8175 1	26		

BOXCULVERT FOR ROAD NO. 3 LIST OF REINFORCED BAR --- WALL

		7	L C 1 IA L	-		WALL	•			
	MARK	DIAMETER	LENGTH	NUMBAR	UNITWEIGHT	PIECENEIGHT	WEIGHT	REMARK		4
						<u> </u>	******************	<u> </u>		
	W 1	Y20	1420	54	2.466	10.90	589			•
•	2		3630	27	11 -	8.95	242			
•	3	Y16	4070	53	1.579	6.43	341			
•	4		16060	1	11	25.36	25			
	5	11	13380	8	11	21.13	169			
	6	.,	4930	13	"	7.78	101			
	7	Y12	16260	1	0.888	14.44	14			
•	8	.,	13270	8		11.78	94			
	9		5100	13		4.53	59			
	10	Y16	17650	2_	1.579	27.87	56			
<u>.</u> 	11	0.1	960	49	11	1.52	74			
	12	"	1030	4	11	1.63	7			
į.	13	Y12	720	68	0.888	0.64	44			
	11	Y16	860	1	1 579	1,36	1			**
	15	11	1040	22		1.64	36			-
							1852	kg		•
win n T										
	1 1	Y20	1850	72	2.166	4.56	328	 		
	2		1190	19	1.579	1.88	92			
	3	1 1	1490	72	2.466	3.67	264	·		
	4	Y16	1610	49	1 579	2.54	124	· · · · · · · · · · · · · · · · · · ·		
	5	Y20	3120	8	2.466	7 69	62			
	6	1	2090	7	1.579	3.30	23			
	7	i i	2350	13	2.466	5.80	75			
	. 8		2880	5	1.579	4.55	23			
**	. 9		16060	1	"	25.36	25			
	10	} [7610	1		12.02	12			
	11	"	15140	1	,~	23.91	24			
	12	<u> </u>	16150	1	11	25.50	26			
	13	{	8540	3	. 11	13.48	13			•
	14	1 1	17010	1		26.86	27			
	15		15360	2	0.888	13.64	27			
	16	1	8540	1	"	7.58	8			
	17		17010	1	11	15.10	15			
	18		14720	3		13.07	30		<u> </u>	
	19		7610	2	***	6.76	14		• • •	
					11	13.44	13		*	
	20		15140 1550	1		1.38	61			-
19	21	<u> </u>		44	1.579	1.52	9			
	22	YIG	960	6	1.919	1.24_		 νσ		
			·				. 1304	n.6		
		, <u>, , , , , , , , , , , , , , , , , , </u>	0.000	0.5	/\ 000	2.40	65			
	<u>K 1</u>	Y12	2700	27	0.888		28			
	2	ll	7880	4		7.00		k o		
							93_	<u>vp</u>	ļ	
			· · · · · · · · · · · · · · · · · · ·		1660				1	
			Y20		1560 1		··			
			Y16		1208 1					-
		····	Y12 計		481 1 3249 1				}	
1										

LIST OF REINFORCED BAR---BI = B2

					בור בור		
ŸARK	DIANETER	LENGTH	NUMBAR	UNITWEIGHT	PIECEWEIGHT	WEIGHT	REMARK
			v.		**************************************	·	<u> </u>
<u>s 1</u>	Y32	18760	44	6.313	118.43	5211	
. 2	11	9780	44	ii	61.74		·
3	Y25	9780	43	3.854	37.69	2717 1621	
4	Y32	5430	86	6.313	34.28	2948	
5	Y25	14110	64	3.854	54.38	3480	
6	Y32	1890	44	6.313	11.93	525	
7	"	1890	44	н	р	525	
8	Y12	1800	307	0.888	1.60	491	,
. 9	Y32	16750	3	6.313	105.74	317	
10		15630	3	11	98.67	296	
11		9830	3	"	62.06	186	
12	Y25	9830	3	3.854	37.88	114	
13	Y32	1890	4	6.313	11.93	48	
		· -				18479	k et
						10413	
W . j	Y25	13620	19	3.854	52.49	997	
2	Y16	13370	19	1.579	21.11	401	
. 3	3 11	14220	19		22.45	427	
4	Y25	14590	19	3.854	56.23	1068	
5		7120	44	"	27.44	1207	
6	"	7120	44		н	1207	
7	11	7120	4	12	"	110	
8	u	7120	1	р	11	27	
9	Y12	820	189	0.888	0.73	138	
10	"	820	189	10	"	138	
11	"	820	9	tr	"	7	
	<u> </u>					5727	kg
			·.	·		·	
F 1	1 Y 3 2	18940	44	6.313	119.57	5261	
2	- "	9780	44	н	61.74	2717	·
3	Y25	9780	43	3.854	37.69	1621	
4	¥32	5520	86	6.313	34.85	2997	<u> </u>
5	Y25	14110	64	3.854	54.38	3480	. :
6	Y32	2020	44	6.313	12.75	561	
			44	11	н	561	
7		2020					
8	 	1980	307	0.888	1.76	540	
	 				1.76 99.24		
8	Y12	1980	307	0.888		540	
8	Y12 Y32	1980 15720	307	0.888	99.24	540 298	
8 9 10	Y12 Y32	1980 15720 15720	307	0.888	99.24	540 298 298	
8 9 10	Y12 Y32 "	1980 15720 15720 9830	307	0.888	99.24	540 298 298 186	
8 9 10 11 12	Y12 Y32 "	1980 15720 15720 9830 9830	307 3 3 3 3	0.888 6.313 " " 3.854	99.24 " 62.06 37.88	540 298 298 186	
8 9 10 11 12 13	Y12 Y32 " " Y25 Y32	1980 15720 15720 9830 9830 2020	307 3 3 3 3 4	0.888 6.313 " 3.854 6.313 0.888	99.24 " 62.06 37.88 12.75	540 298 298 186 114 51	kg
8 9 10 11 12 13	Y12 Y32 " " Y25 Y32	1980 15720 15720 9830 9830 2020	307 3 3 3 3 4	0.888 6.313 " 3.854 6.313 0.888	99.24 ,, 62.06 37.88 12.75 1.76	540 298 298 186 114 51	kg
8 9 10 11 12 13	Y12 Y32 " " Y25 Y32	1980 15720 15720 9830 9830 2020	307 3 3 3 3 4	0.888 6.313 " 3.854 6.313 0.888	99.24 " 62.06 37.88 12.75 1.76	540 298 298 186 114 51	kg
8 9 10 11 12 13	Y12 Y32 " " Y25 Y32	1980 15720 15720 9830 9830 2020 1980	307 3 3 3 3 4	0.888 6.313 " 3.854 6.313 0.888	99.24 ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	540 298 298 186 114 51	kg
8 9 10 11 12 13	Y12 Y32 " " Y25 Y32	1980 15720 15720 9830 9830 2020 1980	307 3 3 3 3 4	0.888 6.313 " 3.854 6.313 0.888 25703 k 15046 k	99.24 ,, 62.06 37.88 12.75 1.76	540 298 298 186 114 51	kg
8 9 10 11 12 13	Y12 Y32 " " Y25 Y32	1980 15720 15720 9830 9830 2020 1980	307 3 3 3 3 4	0.888 6.313 " 3.854 6.313 0.888	99.24 "62.06 37.88 12.75 1.76	540 298 298 186 114 51	kg

LIST OF REINFORCED BAR --- WING1= 2

	l i			Charles of the Control of the Contro			
MARK	DIAMETER	LENGTH	NUMBAR	UNITWEIGHT	PIECEWEIGHT	WE I GHT	REMARK
W 1	Y25	9610	5	3.854	37.04	185	•
2	11	5510	18	lı .	21.24	382	
3	"	4380	16	"	16.88	270	
4	n	2420	7	ı n	9.33	65	
5	11	11080	3	. 11	42.70	128	
6	.,	5630	20	п	21.70	434	
7	11	5380	13	n	20.73	269	
8	tt .	2990	10	ц	11.52	115	
9	Y20	9490	5	2.466	23,40	117	
10	15	5390	18	11	13.29	239	
11	34	10880	3	tı	26.83	80	
12	21	5600	20	11	13.81	276	
13	Y12	28570	2	0.888	25.37	51	
14	Y20	4410	28	2.466	10.88	305	
15	**	4140	30	n	10.21	306	
16	Y16	4410	28	1.579	6.96	195	
17	.,	4140	30	"	6.54	196	
18	Y12	12850	1	0.888	11.41	11	
19	- 11	12850	1	11	11	11	
20	1	13220	1	: 11	11.74	12	
21	51	13220	1	51	11	12	
22	"	2310	34	*1	2.05	70	
23		650	28	0	0.58	16	
24	u	650	30	12	"	17	
25	Y25	2380	21	3.854	9.17	193	
26	11	2240	21	0	8.63	181	
27	"	3380	21	**	13.03	274	
28	,,	3380	21	11	31	274	
29	Y12	680	24	0.888	0.60	14	
30	"	680	24	п	11	14_	
31	31	720	46	ıı .	0.64	29	
32	11 -	720	46	11	11	29	
	<u>. </u>					4770	kg
		· · · · · · · · · · · · · · · · · · ·					
		Y25		2770		 	
<u> </u>		Y20		1323 k	6		
·		Y16		391 k			
		Y12		286 k	g		

LIST OF REINFORCED BAR --- BI = B2

MARK	DIAMETER	LENGTH	NUMBAR	UNITWEIGHT	PIECEWEIGHT	WEIGHT	REMARK
							I
S 1	Y32	18610	45	6.313	117 10		
2	,,	9780	45	"		5287	
3	Y25	9780	44	3.854	61.74	2778	
4		4170	88		37.69	1658	
5		14110	64	6.313	26.33	2317	
6		1890	45	3.854	54.38	3480	
7	102	1890		6.313	11.93	537	<u>.</u>
8	Y12		45		17	537	
	112	1800	315	0.888	1.60	504	
					the supplier	17098	kg
W 1	Y25	14110	19	3.854	54.38	1033	
2	Y16	13790	19	1.579	21.77	414	
3		13790	19	"	"	414	
4	Y25	14110	19	3.854	54.38	1033	
5	"	6970	45	"	26.86	1209	· · · · · · · · · · · · · · · · · · ·
6	n	6970	45	,,,	n 20.00	1209	
7	Y12	820	194	0.888	0.73	142	
8	"	820	194	"	n 0.73	142	·
	 	. 020	177			5596	ŀα
			-				
F 1	Y32	18790	45	6.313	118.62	5338	
2	.11	9780	45	11	61.74	2778	
3	Y25	9780	44	3.854	37.69	1658	
4	Y32	5460	88	6.313	34.47	3033	
5	Y25	14110	64	3.854	54.38	3480	
6	Y32	2020	45	6.313	12.75	574	
7	"	2020	45	'n	12	574	
8	Y12	1980	315	0.888	1.76	554	
						17989	kg
. 1		Y32		23753 }	ιg	··	
		Y25		14760	Kg		·
		Y16		828]	(g		
4.4°		Y12		1342 1	(g		
				40683 1	(g		
						٠	

LIST OF REINFORCED BAR---WING1-2

	1T	The state of the s	-				
MARK	DIAMETER	LENGTH	NUMBAR	UNITWEIGHT	PIECEWEIGHT	WEIGHT	REMARK
				· ·	•		
V 1	Y25	9960	3	3.854	38.39	115	·
2	.0	5480	. 20	11	21.12	422	<u> </u>
3	0	4380	15	u	16.88	253	
4	1)	2560	7	h	9.87	69	i
5	9	9360	4		36.07	144	
6	11	5250	19	ti	20.23	384	
7	11	3880	16	ti	14.95	239	e de la constanta de la const
8	u	2330	. 6	Ħ	8.98	54	
9	Y20	9880	3	2.466	24.36	73	
10	r	5400	20	н	13.32	266	
11	11	9280	4	. tı	22.88	92	
12	и	5180	19	n	12.77	243	
13	Y12	18020	2	0.888	16.00	32	
14	Y20	4070	29	2.466	10.04	291	
15	"	4270	27	"	10.53	284	
16	Y12	4070	29	0.888	3.61	105	
17		4270	27	н	3.79	102	
18	11	12930	1	u	11.48	11	
19	ęı	12930	i	11	tı .	11	
20	11	12610	1	"	11.20	11	
21	FF	12670	1	t i	11.25	11	
22	11	2260	34	11	2.01	68	
23	11	650	29	. 11	0.58	17	
24	u u	650	27	0.	11	16	
25	Y25	2300	20	3.854	8.86	177	
26	11	2300	20	ч .	11	177	
27	н	3380	20	fi .	13.03	261	
28	17	3380	20	н	11	261	
29	Y12	680	24	0.888	0.60	14	
30	ţ1	680	24	"1	14	14	
31	11	720	4.7	н	0.64	30	
32	,,	720	45	11	"	29	
	<u> </u>			-	: <u> </u>	4276 k	g
		Y25		2556 1	(g		
		Y20		1249 l	rg		-
		Y12		471 l	rg		·
				4276 F	g		

LIST OF REINFORCED BAR --- BI = B2

MARK	DIAMETER	LENGTH	NUMBAR	UNITWEIGHT	PIECEWEIGHT	TROISE	REMARK
· .						,	1
3	1 Y32	18610	43	6.313	117,48	5052	
	2	.9780	43	11	61.74	2655	
	3 Y25	9780	42	3.854	37.69	1583	
	4 Y32	5370	84	6.313	33.90	. 2848	
	5 Y25	13510	64	3.854	52.07	3332	
- 1	6 Y32	1890	43	6.313	11.93	513	
· · ·	7 "	1890	43	*1	11	513	
	8 Y12	1800	300	0.888	1.60	480	
		·	 _			16976	kg
	-1		·				
4	1 Y25	13510	19	3.854	52.07	989	
	2 Y16	13190	19	1.579	20.83	396	
- 1	3 "	13190	19	"	**	396	
	4 Y25	13510	19	3.854	52.07	989	
!	5 "	6970	43	11	26.86	1155	
	ŝ "	6970	43	11	11	1155	
	7 Y12	820	185	0.888	0.73	135	
	8 "	820	185	11	19	135	
						5350	kg
	·	·					
7	1 Y32	18790	43	6.313	118.62	5101	· · · · · · · · · · · · · · · · · · ·
	2 "	9780	43		61.74	2655	L <u></u>
<u> </u>	3 Y25	9780	42	3.854	37.69	1583	·
	4 Y32	5440	84	6.313	34.34	2885	L
	5 Y25	13510	64	3.854	52.07	3332	
	5 Y32	2020	43	6.313	12.75	548	
•	7 "	2020	43	11	"	548	
	8 Y12	1980	300	0.888	1.76	528	
	- :		· ·			17180	kg
		<u>Y32</u>		23318 1	(g	·	
	<u> </u>	Y25		14118 }	(g		
		Y16		792 }	(g .		
		Y12		1278 1	(g	·	
		Q. Maria		39506_1	¢g		

LIST OF REINFORCED BAR --- WING 1-2

	The second second	The second lives of the se		- Property of the same of the	ASHAOI*S		
MARK	DIAMETER	LENGTH	NUMBAR	UNITWEIGHT	PIECEWEIGHT	MEIGHT	REMARK
					The second secon		
W 1	Y25	9160	5	3.854	35.30	170	<u> </u>
2	н	5000	18	п	19.27	177	
3	з н	3880	16	п	14.95	347	
4	"	2300	6	41	8.86	239	
5	- 39	10930	3	н	42.12	53 126	<u> </u>
6	"	5000	20	11	19.27	385	
7	11	4380	15	u u	16.88	253	
8	11	2530	7	п	9.75		<u> </u>
9	Y20	9080	5	2.466	22.39	68 112	·-····································
10	11	4930	18	11	12.16	219	
11	0	9980	3	ţı.	24.61	74	
12	11	5380	20	H	13.27	265	
13	Y12	27920	2	0.888	24.79	50	
14	Y20	4380	27	2.466	10.80	292	<u> </u>
15	11	4080	30	n	10.06	302	:
16	Y16	4380	27	1.579	6.92	187	
17	11	4080	30	"	6.44	193	
18	Y12	12490	1	0.888	11.09	11	
19	11	12490	1	tt.	11	11	
20	H	12970	1	11	11.52	12	
21	11	12970	1	Tr.	17	12	
22	11	2220	33	t _I	1.97	65	
23	,,	650	27	11	0.58	16	
24	lt .	650	20	. 11	ti .	12	
25	Y25	2300	20	3.854	8.86	177	
26	11	2300	20	-11	ti	177	
27	22	3380	20	II.	13.03	261	
28	"	3380	20	11	"	261	
29	Y12	680	24	0.888	0.60	14	<u> </u>
30	"	680	24	11	II .	14	
31	11	720	40	н	0.64	26	
32	-,,	720	44		0	28	
		120	33	1		4439	kg
							· · · · · · · · · · · · · · · · · · ·
محب میشد و شده	·	Y25		2524 1	rg		·
		Y20		1264 F			·
	· · · · · · · · · · · · · · · · · · ·	Y16		380 1			
		Y12		271 1			
				4439 k			

LIST OF REINFORCED BAR --- BI= B4.

M.A.	RK	DIAMETER	LENGTH	NUMBAR	ТКОІЗЖТІКО	PIÈCEWEIGHT	WEIGHT	REMARK
						<u> </u>		<u> </u>
S	1	Y32	18660	35	6.313	117 00		
	2	"	9780	34	"	117.80	4123	
	3	Y25	9780	35	3.854	61.74	2099	
	4	Y32	5390	68	6.313		1319	· · · · · · · · · · · · · · · · · · ·
	5	Y25	13850	64	3.854	34.03	2314	
	6	Y32	1890	35	6.313	53.38	3416	
	7		1890	35		11.93	418	
	8	Y12	1800	247	0.888		418	
	9	Y32	18170	19	6.313	1.60	395	
	10	ţI.	17050	19	0.013		2179	
	11	* 11	12410	38	11	107.64	2045	
	12	*1	2030	18	"	78.34	2977	
	13	Y12	1800		0.888	12.82	231	
		· • • • •	: #384 I		v.0001	i 1.00j	94	
	- ; ·	1,1,421					22028	ng :
W	1	Y25	11020	19	3.854	42.47	807	
.,	2	Y16	11040	19	1.579	17.43	331	· · · · · · · · · · · · · · · · · · ·
	3	, "	16020	19	1.579			
	4	Y25	16670	19	3.854	25.30 64.25	1221	
	5	"	7020	35	3.854 "	27.06	1221	
	6	11	7020	35	11	27.06	947	
_	7	.,	7020	18	"	- 11	947	
	8	н	7020	10	1 ; H	.,	487	 · · · · · · · · · · · · · · · · ·
	9	Y12	820	149	0.888	0.73	27	
-	10	112	820	149	U.888 "	0.73	109	
	11	"	910	72	,,	0.81	109 58	
-		L	310	L1.E.		v. 011	5524	kø
		. 1 : -					.0024	
F .	1	Y32	18840	35	6.313	118.94	4163	
	2	n	9780	34	"	61.74	2099	
	3	Y25	9780	35	3.854	37.69	1319	
	4	Y32	5480	38	6.313		1315	
	5	Y25	13850	64	3.854	!	3416	
	6	Y32.	2020	35	6.313	12.75	446	
	7	132	2020	35	"	"	446	
	8	Y12	1980	247	0.888		435	
	8 9			19	6.313	108.20	2056	·
<u> </u>	10	Y32	17140 18260	19	0.313	115.28	2190	
	1	. 10		38	"	78.34	2977	
- -	11	· · ·	12410		11	13.64	246	
	12		2160	18 50	i	1.76	104	
	13	Y12	1980	59	0.888	1.10	21212	ke
							21212	<u> </u>
· 		·	Y32		32742 }	kg		
· · ·			106					
			V25		13906 1	KK .		
	 		Y25		13906 } 812 }		· · · · · · · · · · · · · · · · · · ·	
			Y25 Y16 Y12		13906 } 812 } 1304 }	⟨g		

LIST OF REINFORCED BAR --- B2 = B3

	-	T T			1	UL US		
MA	RK.	DIAMETER	LENGTH	NUMBAR	UNITWEIGHT	PIECEWEIGHT	WEIGHT.	REMARK
:								<u> </u>
<u>s</u>	1	Y32	18660	41	6.313	117.80	4830	
	2	"	9780	40	. 11	61.74	2470	
·	3	Y25	9780	41	3.854	37.69	1545	
·	4	Y32	5390	80	6.313	34.03	. 2722	
	5	Y25	12760	64	3.854	49.18	3148	
	6	Y32	1890	41	6.313	11.93	489	
· · ·	7	11	1890	41	11		489	
	8	Y12	1800	285	0.888	1.60	456	
							16149	kg
	· ;	····	· · · · · · · · · · · · · · · · · · ·	· . ·	· · · · · · · · · · · · · · · · · · ·			
W	1	Y25	12760	19	3.854	49.18	934	
	2	Y16	11880	19	1.579	18.76	356	
	3	.,	11880	19	"		356	
	4	Y25	12760	19	3.854	49.18	934	
	5	"	7020	41	"	27.06	1109	
	6		7020	41		"	1109	1.
<u>_</u>	7	Y12	820	176	0.888	0.73	128	
	8	,,	820	176	*1	11	128	
				:			5054	kg
F_	1	Y32	18840	41	6.313	118.94	4877	
	2	.,	9780	40	.,	61.74	2470	
	3	Y25	9780	41	3.854	37.69	1545	
-	4	Y32	5480	80	6.313	34.60	2768	
	5	Y25	12760	64	3.854	49.18	3148	
	6	Y32	2020	41	6.313	12.75	523	
	7	13	2020	41	y i	,,	523	
-	8	Y12	1980	285	0.888	1.76	502	ļ L
							16356	kg
			Y32		22161	kg		·
			Y25		13472 }			
			Y16		712			
			Y12		1214)			
					37559 1			

BOXCULVERT FOR ROAD - NO 10

LIST OF REINFORCED BAR ---WING

	\ R X	DIAMETER	LENGTH	RABKUK	UNITWEIGHT	PIECEMEIGHT	WEIGHT	REMARK
	:							
1	1	Y25	7120	3	3.854	27.44	82	
	. 2	н	9770	3_	n n	37.65	113	
	3	11	5870	8 1	u	22.62	407	
	4	11	3750	16	11	14.45	231	
	5	11	2930	4	. 11	11.29	45	· · · · · · · · · · · · · · · · · · ·
	6	11	4000	2	11	15.42	31	
	7	Y20	10820	4	2.466	26.68	107	
	8	11	6200	18	11	15.29	275	
	9	()	4500	28		11.10	311	
	10	Y16	4710	30	1.579	7.44	223	
· •	11		1500	77	11	2.37	182	
	12	н.	920	39	11	1.45	57	
:	13	**	990	1	111	1.56	2	
	14	+1	950	30	11	1.50	45	
	15		13260	2	l1	20.94	42	
	16	.,,	11720	2	11	18.51	37	
	17	"	11330	2	11	17.89	36	
	18	.,,	9240	4	11	14.59	58	
	19		7000	1	*1	11.05	11	-
	20		950	23	11	1.50	35	·
. :	21	Y12	710	49	0.888	0:63	31	
	22	Y25	3000	24	3.854	11.56	277	
-	- 1			,, 		·	2638 k	g
j		• #						
			Y25		1186 }	(g		
. 1			. Y20		693 }	ζg		
	·	* · · · · · · · · · · · · · · · · · · ·	Y16		728 1	(g		
	· · ·		Y12		31 1	ζg		
					2638 }	ζg	· ·	

BOXCULVERT FOR ROAD -NO 10

LIST OF REINFORCED BAR --- WALL

MARK	DIAMETER	LENGTH	NUMBAR	THOLENTIAU	PIECEWEIGHT	WEIGHT	REMARK
	1						
W 1	1	4560	45	2.466	11.24	506	
2	19	4700	22	"	11.59	255	
3	Y16_	4500	46	1.579	7.11	327	
4	-	860	45	"	1.36	61	
5	- "	920	1_	••	1.45	1	
6	· "	15510	2	.,	24.49	49	
7		13680	1		21.60	22	
8	"	12150	5		19.18	96	
9	17	5210	17	. 11	8.23	140	
10	Y12	13760	1_	0.888	12.22	12	
11		11970	6	11	10.63	64	
12	11	5030	16	11	4.47	72	
13	Y16	840	1	1.579	1.33	1	
14		970	23	0	1.53	35	
15	Y12	720	56	0.888	0.64	36	
			·			1677	kg
						<u> </u>	
1	Y20	2050	59	2.466	5.06	299	
2	Y16	1270	41	1.579	2.01	82	
3	Y20	1590	59	2 466	3.92	231	
4	Y16	1680	41	1.579	2.65	109	
5	Y20	3020	7	2.466	7.45	52	
. 6	Y16	2110	6	1.579	3.33	20	
7	Y20	2350	12	2.466	5.80	7.0	
8	Y16	2780	4	1.579	4.39	18	
9		13640	4	n	21.54	86	·
10	"	5080	4		8.02	32	:
11	.,	12660	1		19.99	20	
12	11	12960	1		20.46	20	
13		7340	1	11	11.59	12	
14	"	14520	1	et	22.93	23	<u> </u>
15	Y12	_13640	3	0.888	12.11	36	·
16	n	5080	4	"	4.51	18	
17	"	12660	1	"	11.24	11	
18	.,	12960	1	,,	11.51	12	
19	11	7340	1	11	6.52	7	
20	,,	14520	1	11	12.89	13	<u> </u>
21	Y16	810	10	1.579	1.28	13	
22	Y12	1550	37	0.888	1.38	51	
						1235	kg
		<u> </u>	<u> </u>				
1	Y12	2700	21	0.888	2.40	50	
2		5880	4	"	5.22	21	
	LJ	3000		·	•	71	kg
	<u> </u>						
		Y20		1413	g		
		Y16		1167			
		Y12		403 1			
 -		126		2983			

BOXCULVERT FOR ROAD - NO 10

LIST OF REINFORCED BAR --- WALL 2

MARK	DIAMETER	LENGTH	NAHRAK	UNITWEIGHT	PLECEMEICHT	WEIGHT	REMARK
, ,	V20 T	4000					
1	Y20	4290	55	2.466	10.58	582	
2		3400	26		8.38	218	
3	Y16	4230	<u> </u>	1.579	6.68	374	
4 5		860 920	55		1.36	75	
6		18100	1 2	"	1.45	1	
7		16570			28.58	57	
8		13400	<u>1</u> 8	,,	26.16	26	
9	,,	5160	12	11	21.16	169	
10	Y12	16760	1	0.888	8.15	98	
11	",	13650	8	"	14.88	15	
12	u	5170	13	,	12.12	97	
13	Y16	830	1	1.579	4.59	60	
14	111	1000	22	11.079	1.31	35	
15	Y12	790	68	0.888	0.70	48	
16	Y 8	: 1	1	0.395	0.00	0	•
			\	9.030	0.00	1856	kg
7 1	Y20	2070	71	2.466	5.10	377	
2	Y16	1220	51	1.579	1.93	98	
3	Y20	1520	74	2.466	3.75	278	<u> </u>
4	Y16	1700	51	1.579	2.68	137	
5	Y20	3120	9	2.466	7.69	. 69	
6	Y16	2090	7.	1,579	3.30	23	
7	Y20	2330	13	2.466	5.75	75	
. 8	Y16	2880	5	1.579	4.55	23	
9	"	16730	3	115	26.42	79	·
10		13480	2	12	21.28	43	
11		6750	2	"	10.66	21	
12	 " 	16630	11_		26.26	26	
13	"	15070	11_		23.80	24	
14		8500	1		13.42	13	
15		17500	1		27.63	28	
16	Y12	16600	2	0.888	14.74	29	
17		14930	1		13.26	13	
18		8500	1		7,55	8	·
19		17350	1	"	15.42	15	
20		13340	1	- 11	11.85	12	·
21	*	6750	2	13	5.99	12	
22		16490	1_	n	14.64	15	·
23	Y16	930	9	1.579	1.47	13	<u></u>
24	Υ12	1510	45	0.888	1.34	60	<u> </u>
25	Y 8	1	1	0.395	0.00	0	L
	: :	1				1491	kg
, ,	Y12	2700	28	0.888	2.40	RT	
2	Y12	2700 7880	28 4	0.888	7.00	67 28	
	L	7.000	· ·			95	kg
		Y20	24 2	1599			
	 	Y16		1364			
		Y12		479	(g		

BOXCULVERT FOR ROAD -NO 10

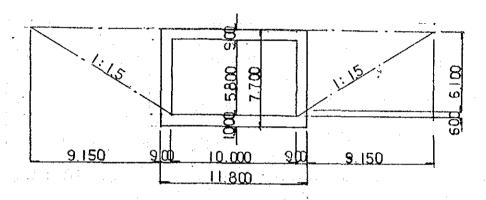
LIST OF REINFORCED BAR --- WALL3

NARK	DIAMETER	LENGTH	NUMBAR	UNITWEIGHT	PIECEWEIGHT	WEIGHT	REMARK
		· · · · · · · · · · · · · · · · · · ·				·	
, 1	Y20	4400	34	2.466	10.85	369	
2	'n	7540	1	11	18.59	19	
3	71	3400	17	14	8.38	142	
4	Y16	4330	32	1.579	6.84		
5	"	860	29	"	1.36	219 39	
6	.,	1390	3	11	2.19	7	
7	"	12360	1	Р	19.52	20	
8	"	9740	1	. 11	15.38		
9	••	4790	21	0	7.56	15 159	
10	"	11930	1	"	18.84	19	
11	Y12	9410	1	0.888	8.36	8	
12	*"	4710	22	"	4.18	92	
13	Y16	1000	22	1.579	1.58	35	
14	11	830	1	0	1.31	1	
15	Y12	770	48	0.888	0.68	33	
						1177	kg
<u> </u>							
1	Y20	2080	45	2.466	5.13	231	
2	Y16	1220	30	1.579	1.93	58	
3	Y20	1520	- 44	2.466	3.75	165	
4	Y16	1710	30	1.579	2.70	81	
5	Y20	3540	12	2.466	8.73	105	
6	Y16	2040	3	1.579	3.22	10	
7	Y20	2300	6	2.466	5.67	34	
8	Y16	3300	7	1.579	5.21	36	
9	17	9570	3	н	15.11	45	
10	"	5100	6		8.05	48	
11	te.	11680	1	11	18.44	18	·
12	٠,	6630	2	"	10.47	21	
13		8890	1	19	14.04	14	
14	Y12	9570	2	0.888	8.50	17	
15	11	5100	6		4.53	27	
16	- 11	11540	1	0	10.25	10	
17		6630	2	11	5.89	12	
18	"	8890	1	DF .	7.89	8	
19	Y16	930	9	1.579	1.47	13	
20	Y12	550	32	0.888	0.49	16	
						969	kg
- 4.1 		*					r
1-	Y12	2700	14	0.888	2.40	34	
2	"	3880	4		3.45	14	
		·				48	kg
			· · · · ·	**. **.			
		Y20	<u> </u>	1065 k	g		
	· · · · · · · · · · · · · · · · · · ·	Y16		858 1			
		Y12		271 k			·····
		·		2194 H	g		

NO 0 BOX CULVERT FOR ROAD

BACK-FILL

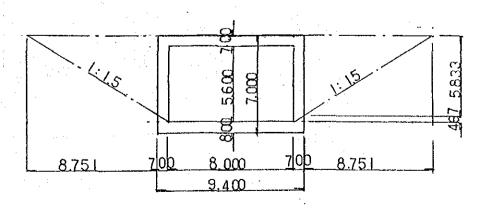
$$V = 6.10 \times 9.15 \times (32.30 - 0.009 - 0.019) = 1.734.3 \text{ m}^3$$



NO @ BOX CULVERT FOR ROAD

BACK-FILL

 $V = 5.833 \times 8.751 \times (27.50 - 0.60 \times 2) = 1342.5 \text{ m}^3$

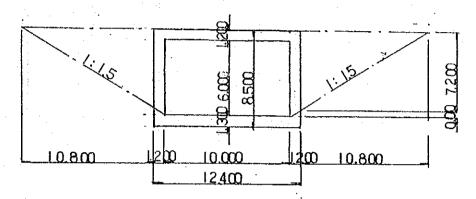


NO @ BOX CULVERT FOR ROAD

BACK-FILL --- include retaining wall.

 $V = 7.20 \times 10.80 \times 50.50$

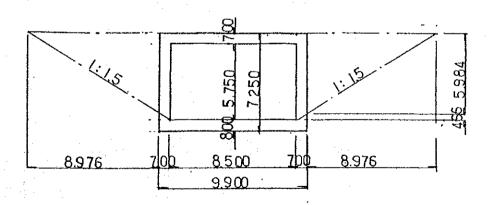
 $+\frac{1}{2}$ x 7.20 x 10.80 x 16.00 /3 = 4142.0 m³



NO @ BOX CULVERT FOR ROAD

BACK·FILL

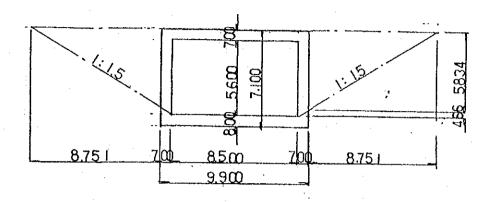
$$V = 5.984 \times 8.976 \times (26.70 - 0.603 \times 2)$$
 = 1369.3 M^3



NO 5 BOX CULVERT FOR ROAD

BACK-FILL

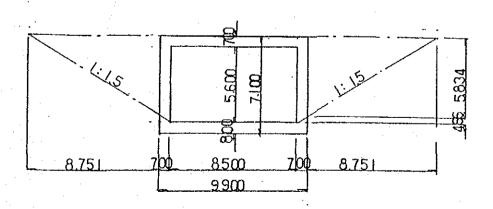
 $V = 5.834 \times 8.751 \times (26.70 - 0.60 \times 2) = 1301.8 \text{ m}^3$



NO @ BOX CULVERT FOR ROAD

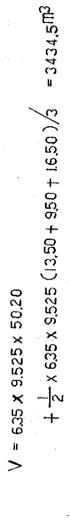
BACK-FILL

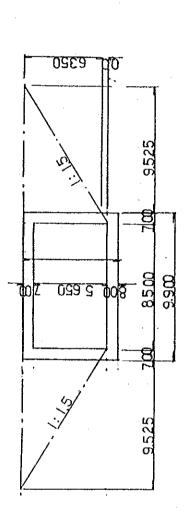
 $V = 5.834 \times 8.751 \times (25.50 - 0.60 \times 2)$ = 1240.6 M^3



NO @ BOXCULVERT FOR ROAD

BACK-FILL --- include retaining wall





BOX FOR ROAD

NO① UF2 A = $(11.80 + 10.00) \times 32.30 + 9.00 \times 0.60 \times 2 \times 2$ = 725.740 m² " $A = (9.40 + 8.00) \times 27.50 + (8.60 + 9.00) \times 0.60 \times 2$ NO(2) = 499.620 " " $A = (12.40 + 10.00) \times 50.50 + (9.90 + 11.50) \times 0.70 + 8.30 \times 0.60$ NO(3) $+(3.50+10.00)\times1/2\times16.00$ = 1187.160 " NO(4) " $A = (9.90 + 8.50) \times 26.70 + (8.60 + 9.30) \times 0.60 \times 2$ = 512.760 " " $A = (9.90 + 8.50) \times 26.70 + (9.00 + 8.40) \times 0.60 \times 2$ XO(\$) = 512.160 " " $A = (9.90 + 8.50) \times 25.50 + (8.20 + 9.10) \times 0.60 \times 2$ NO(6) = 489.960 " NO(7) " $A = (9.90 + 8.50) \times 50.20 + 8.70 \times \times 0.60$ $+(3.50+1.00)\times1/2\times(13.50+9.50+16.50)$ = 1017.775 "

indian Algorithm (1997) Subsection

```
NO@ BOX FOR ROAD
                       A = 6.70 \times 31.072 \times 2(\times 0.300)
  porous drainage
                                                                                         = 416.364 \text{ m}^{2} 124.909 \text{m}^{3}
  perforated pipe
                      L = 31.072 \times 2
                                                                                         = 62.144 m
  drain pipe
                                                                                         = 2 N
   ( $ 200 mm)
 NO(2) BOX FOR ROAD
  porous drainage A = 6.30 \times 26.30 \times 2(\times 0.300)
                                                                                         \approx 331.380 m<sup>2</sup> 99.414m<sup>3</sup>
  perforated pipe
                       L = 26.30 \times 2
                                                                                         = 52.600 m
  drain pipe
                        N =
                                                                                         = 2 N
  (φ 200 mm)
 NO3 BOX FOR ROAD
                        A = \{7.20 \times 50.60 \times 2 + \frac{1}{2} \times 7.20 \times 16.0\} \ (\times 0.300)
  porous drainage
                                                                                          = 786. 240 m<sup>2</sup> 235. 872m<sup>3</sup>
  perforated pipe
                        L = 50.60 \times 2 + 16.00
                                                                                          = 117,200 m
 drain pipe
                                                                                          = 4 N
  (φ200mm)
NO@ BOX FOR ROAD
  porous drainage
                        A = 6.45 \times 25.494 \times 2(\times 0.300)
                                                                                          = 328,872 m<sup>2</sup> 98,662m<sup>3</sup>
 perforated pipe
                        L = 25.494 \times 2
                                                                                          = 50.988 \text{ m}
 drain pipe
                        N =
                                                                                          = 2 N
   (\phi 200 mm)
NOS BOX FOR ROAD
                        A = 6.30 \times 25.50 \times 2(\times 0.300)
 porous drainage
                                                                                          = 321.300 m² 98.390m³
 perforated pipe
                        L = 25.50 \times 2
                                                                                          = 51.000 m
 drain pipe
                        N =
                                                                                          = 2 · N
  (\phi 200 \text{mm})
NO® BOX FOR ROAD
 porous drainage
                      A = 24.30 \times 6.30 \times 2(\times 0.300)
                                                                                         = 306.180 mt 91.854 m3
 perforated pipe
                       L = 24.30 \times 2
                                                                                         = 48.600 m
 drain pipe
                        N =
                                                                                         = 4 N
  (\phi 200 \text{ mm})
NOW BOX FOR ROAD
i)BOX
 porous drainage
                       A1 = 50.20 \times 6.35 \times 2
                                                                                          = 637.540 m
 perforated pipe
                       L1 = 50.20 \times 2
                                                                                          = 100.400 m
 drain pipe
                       N1 =
 (\phi 200 \text{ mm})
ii)wall
 porous drainage
                       A2 = 6.35(13.50 + 9.50 + 16.50)/2
                                                                                           = 125.412 m
 perforated pipe
                       12 = 13.50 + 9.50 + 16.50
                                                                                           = 39.50 m
 drain pipe
  (φ 200mm)
total
     A = (637.540 + 125.412) \times 0.300
                                                                                           = 762.952 m²
```

= 139.9 m

= 2 N

L = 100.400 + 39.500

45

N =

R Q 21.01 Waterproofing materials

NO(1) BOX for Road

A = $(32.30-0.618-0.609) \times (7.70 \times 2+11.80) + (9.00 \times 7.971 \times 2-9.00 \times 6.00) \times 2 = 1024.142$ m² NO@ BOX for Road

$$A = (27.50 - 0.60 \times 2) \times (7.10 \times 2 + 9.40)$$

$$+ \{(8.60 + 9.00) \times 7.464 - \frac{1}{2} \times (8.60 \times 5.733 + 9.00 \times 6.00)\} \times 2 = 780.109 \text{ m}^2$$

NO(3) BOX for Road

$$A = (50.48 - 0.692 - 0.844) \times (8.10 \times 2 + 12.00)$$

$$+ \frac{1}{2} \times (1.249 \times 8.496) \times 8.30 + \frac{1}{2} \times (1.405 + 8.50) \times 9.90$$

$$+ \frac{1}{2} \times (1.416 \times 8.708) \times 11.50 + \frac{1}{2} \times (6.90 + 0.40) \times 16.00 = 1586.305 \text{ m}^2$$

NO @ BOX for Road

A =
$$(26.70 - 0.603 \times 2) \times (7.25 \times 2 + 9.90)$$

+ $\{(8.60 + 9.30) \times 7.60 - \frac{1}{2} \times (8.60 \times 5.733 + 9.30 \times 6.20)\} \times 2$ = 787.170 m^2
NO ⑤ BOX for Road

 $A = (26.70 - 0.60 \times 2) \times (7.10 \times 2 + 9.90)$

+
$$\{(9.00+8.40)\times7.425-\frac{1}{2}\times(9.00\times6.00+8.40\times5.60)\}\times2$$
 = 771.900 m²

NO 6 BOX for Road

A =
$$(25.50-0.60\times2)\times(7.10\times2+9.90)$$

+ $\{(8.20+9.10)\times7.406-\frac{1}{2}\times(8.20\times5.467+9.10\times6.067)\}\times2$ = 741.839 m² NO ① BOX for Road

$$A = (50.20 - 0.658 - 0.346) \times (7.15 \times 2 + 9.90)$$

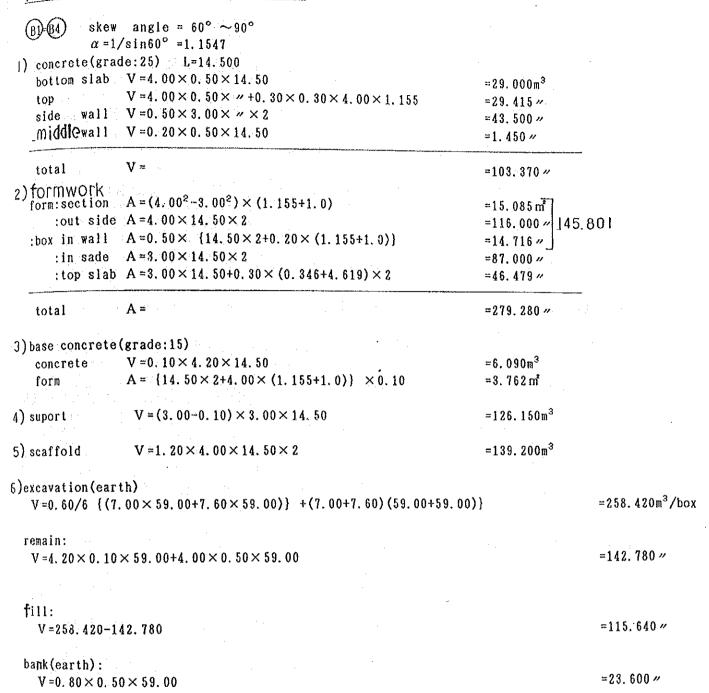
$$+ \frac{1}{2} \times (1.26 + 7.828) \times 8.70$$

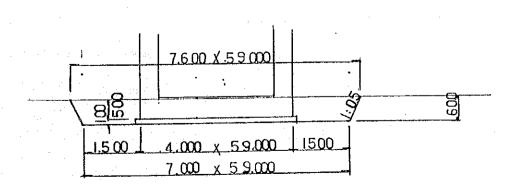
$$+ \frac{1}{2} \times (7.235 + 0.40) \times 13.50$$

$$+ \frac{1}{2} \times (6.85 + 0.40) \times 9.50$$

$$+ \frac{1}{2} \times (6.85 + 0.40) \times 16.50$$
= 1375.862 m²

NO1 BOX FOR DRAINAGE





NO1 BOX FOR DRAINAGE

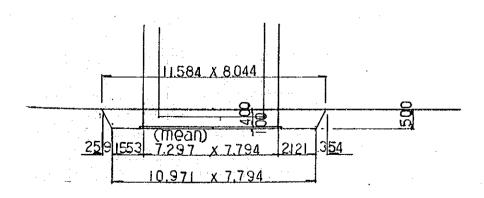
B2\B3

oncrete (grade: 25) L=15.00m bottom slab V=4.00×0.50×15.00 top V=4.00×0.50× side wall V=0.50×3.00× "×2 middle wall V=0.20×0.50×15.00	=30.000m ³ =30.000 \(\times \) =45.000 \(\times \) =1.500 \(\times \)
total V=	=106. 500 "
2) formwork. form: section $A = (4.00^2 - 3.00^2) \times 1$:out side $A = 4.00 \times 15.00 \times 2$ middle wall $A = 0.50 \times (0.20 + 15.00) \times 2$:in side $A = 3.00 \times 15.00 \times 2$:top slab $A = 3.00 \times 15.00$	=7.00 m² (0.000) =120.000 " =15.200 " =90.000 " =45.000 "
total A=	=277.200 // (270.200) (B2) (B3)
3) base concrete(grade:15)	
concrete $V = 0.10 \times 4.20 \times 15.00$ form $A = 0.10 \times 15.00 \times 2$ 4) joint filler $A = (4.00^2 - 3.00^2) \times 3$ 5) water stop $L = 3.50 \times 4 \times 3$	=6.300 m^3 =3.000 m^2 =21.000 m^2/box =42.000 m/box
(6) suport $V = (3.00-0.10) \times 3.00 \times 15.00$	=130.500m ³
7) scaffold $V = 1.20 \times 4.00 \times 15.00 \times 2$	=144.0m ³

NOI BOX FOR DRAINAGE

(m) Retaining wall

```
1) concrete(grade: 25)
    side wall V = (3.80+0.30) \times 1/2 \times (9.00+12.247) \times 0.40
                          +1.00 \times 0.40 \times 0.40 \times 2
                                                                                       =17.743m<sup>3</sup>
    hottom slab V = (4.444+10.150) \times 1/2 \times 7.794 \times 0.40+
                        0.30 \times 1.00 \times 10.15
                                                                                       =25.794 "
    total
                                                                                       =43.537 //
2) tormwork
   form: out side A = (4.20+0.70) \times 1/2 \times (9.000+12.247)
                                                                                       =52, 055 m<sup>2</sup>
   :bottom slab A = (0.311+0.424+10.15 \times 2) \times 1.00
                                                                                                      74331
                        +(0.897+1.225+0.414+0.566)\times0.40
                                                                                       =22.276 ".
        : in side A = (3.80+0.30) \times 1/2 \times (9.00+12.247)
                                                                                       =43.556 "
    total
                                                                                       =117.887 "
3) scaffold
                     V = 1.2 \times (3.80 + 1.0) (7.20 + 9.80)
                                                                                       ≈97.920m<sup>3</sup>
4) base concrete(grade:15)
    concrete: main V = 0.10 × (4.689+10.395) × 1/2 × 7.494+0.60 × (0.986+1.100) × 0.10=5.777 m<sup>2</sup>
    form: main A = 0.10 \times \{7.759+10.599+(1.00+1.366) \times 2+0.621+0.848\} = 2.450 \text{ m}^2
          and wing
5) gabion:
                    A = 5.00 \times 10.0
                                                                                       =50,00 m²
     t=500
6) joint filler A = (0, 414+0, 566) \times 3.80+4.444 \times 0.40
                                                                                       =5.502 m
7) water stop
                     L=4.00\times2+3.954
                                                                                       =11.954m
8) excavation
                                                           excavation (earth and gabion include)
                                                              V = 0.50/6 \{10.971 \times 7.794 + 11.584 \times 8.044\}
                                                               +(10.971+11.584)\times(7.794+8.044)
                                                               +0.30 \times 1.00 \times 10.15 + 5.00 \times 10.00 \times 0.500 = 72.705 \text{m}^3
                                                           remain:
                                                               V = 25.794 + 5.777 + 0.30 \times 1.00 \times 10.15 + 5.00 \times 10.00
                                                                   \times 0.50
                                                                                                                   =59.616m<sup>3</sup>
                                                           fill:
```



notice.

V = 72.705 - 59.616

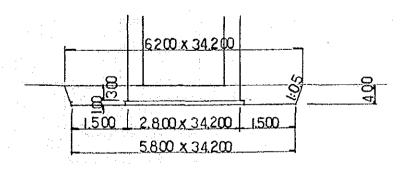
This size is consider skew angle.

=13.089 "

NO2 BOX FOR DRAINGE

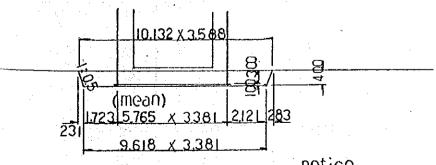


```
1) concrete (grade: 25) L=17.10
     bottom slab V=2.80\times0.40\times17.10
                                                                                          =19.152m^3
                      V = 2.80 \times 0.40 \times "
                                                                                         =19.152 "
           wall V=3.00\times0.40\times \times2
     side
                                                                                          =41.040 //
     middle wall
                    V = 0.20 \times 0.50 \times 17.10
                                                                                          =17.100 //
     total
                                                                                          =96.444 "
2) tormwork
    form: section A = (2.80 \times 3.80 - 2.00 \times 3.00) \times (1.0353 + 1.000)
                                                                                         =9, 444 m (4, 804)
                                                                                                               (151864)
        :out side A=3.80\times17.10\times2
                                                                                         =129.960 m
    :middle wall A=0.50\times0.20\times17.10
                                                                                                               = 156,504
                                                                                         =17.100 "
        : in side A=3.00\times17.10\times2
                                                                                         =102.600 ~
        :top slab A = 2.00 \times 17.10
                                                                                         =34.200 //
    total A=
                                                                                         =293.304 ~ (288.664)
                                                                                         : (Bi)
                                                                                                          (B<sub>2</sub>)
3) base concrete(grade:15)
    concrete V = 0.10 \times 3.00 \times 17.10
                                                                                         =5.130 \,\mathrm{m}^3
                      A = \{17.10 \times 2 + 3.00 \times (1.0353 + 1.0)\} \times 0.10
                                                                                         =4. 031 m<sup>2</sup>
4) joint filler A = (2.80 \times 3.80 - 2.00 \times 3.00)
                                                                                         =4.640 \,\mathrm{m}^2/\mathrm{box}
5) water stop
                      L=(2.40+3.40)\times 2
                                                                                         =11.600m/box
6) suport :
                     V = (3.00-0.1) \times 2.00 \times 17.10
                                                                                         =99.180 \,\mathrm{m}^3
7) scaffold:
                     V = 1.2 \times 3.80 \times 17.1 \times 2
                                                                                         =155.952m3
8) excavation
                                                             excavation(earth)
                                                              V = 0.40/6 \{(5.80 \times 34.20 + 6.20 \times 34.20)
                                                                  +(5.80+6.20)(34.20+34.20)
                                                                                                             =82.080 \,\mathrm{m}^3/\mathrm{box}
                                                             remain:
                                                              V = (3.00 \times 0.10 = 2.80 \times 0.30) \times 34.20 = 38.988 \text{m}^3/\text{box}
                                                             fill:
                                                              V = 82.080 - 38.988
                                                                                                             =43.092m^3/box
                                                             bank(earth):
                                                              V = 0.80 \times 0.50 \times 34.20
                                                                                                             =13.680 "
```



NO2 BOX FOR DRAINGE


```
i) concrete (grade: 25)
                     V = (3.70+0.80) \times 1/2 \times (5.019+6.147) \times 0.40 + (1.115+1.366) \times 0.40 \times 0.40 \times 9.330 \text{m}^3
    side
            wall
                     V = (3.098+8.432) \times 1/2 \times 3.881 \times 0.40+0.30 \times 1.00 \times 8.432
    bottom slab
    total
                                                                                                           =19.657 "
 2) formwork
    form: outside A = (4.10+0.70) \times 1/2 \times (5.019+6.147)
                                                                                                           ≈26.798 m
     : in side A = (3.70+0.30) \times 1/2 \times (5.019+6.147)
                                                                                                           ≈22.332 //
    :bottom slab A = (0.346+0.424+8.432 \times 2) \times 1.00+(1.115+1.366+0.462+0.566) \times 0.4
                                                                                                           ≥19.038 w
    total
            A =
                                                                                                           =68.168 "
3) scaffold
                     V = 1.2 \times (3.70 + 1.00) \times (4.00 + 4.90)
                                                                                              =50.196m<sup>3</sup>
4) base concrete(grade:15)
    concrete: main V = 0.10 × {(3.305+8.639) × 1/2 × 3.081+0.80 × 1.104 × 2}
        and wing
    form:main
                     A = 0.10 \times \{(3.825+4.371)+(1.231+1.508) \times 2+0.693+0.848\} = 1.522 \text{ m}^2
        and wing
                     A = 5.00 \times 9.00
 5) gabion:
                                                                                              =45,000 m
     t=500
6) joint filler A=0.414\times2\times3.70+2.899\times0.40
                                                                                              =4223 m
·7) water stop
                     L=3.90\times2+2.485
                                                                                              =10.285m
8) excavation
                                                         excavation(earth and gabion include)
                                                            V = 0.40/6 {9.618 × 3.381+10.132 × 3.588
                                                                +(9.618+10.132)(3.381+3.588)
                                                                +0.30 \times 1.00 \times 8.432 + 5.00 \times 9.00 \times 0.50 = 38.797 \text{m}^3
                                                            V = 5.765 \times 0.30 \times 3.381 + 1.972 + 0.30 \times 1.00 \times 8.432 + 5.00
                                                                \times 9.00 \times 0.50
                                                                                                             =32.849 "
                                                          fill:
                                                            V = 38.797 - 32.849
                                                                                                             =5.948 "
```



notice

This size is consider skew angle.

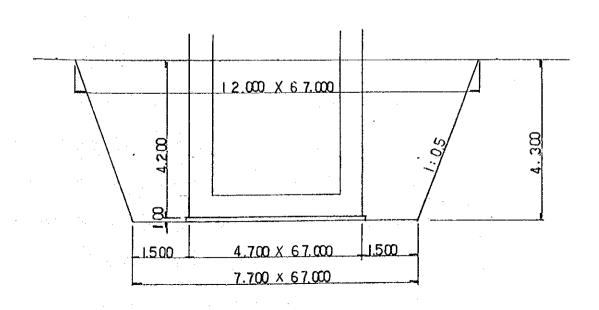
NO3 BOX FOR DRAINAGE

(B)+(B4)

```
|) concrete(grade:25)
     bottom slab V=4.70\times0.70\times18.50
                                                                                          =60.865m^3
                      V=4.70\times0.60\times
     top
                                                                                          =52.170 "
     side wall
                      V=3.00\times0.60\times \times \times 2
                                                                                          =66.600 //
     total
                                                                                          =179.635 //
 2) tormwork
    form: section A = (4.70 \times 4.30 - 3.50 \times 3.00) \times (1.0785 + 1.0)
                                                                                          =20. 182 m 1
                                                                                                        179282
        out side A=4.30\times18.50\times2
                                                                                          =159.100 //
        ; in side A=3.00\times18.50\times2
                                                                                         =111.000 "
        : top slab A = 3.50 \times 18.50
                                                                                         =64.750 //
    total
                      A =
                                                                                         =355.032 //
3) base concrete(grade:15)
    concrete
                    V = 0.10 \times 4.90 \times 18.50
                                                                                         =9.065m^3
    form A = \{18.50 \times 2 + 4.90 \times (1.0785 + 1.0)\} \times 0.10
                                                                                         =4.718 m
4) joint: filler A = (4.70 \times 4.30 - 3.50 \times 3.00) \times 3
                                                                                         =29. 130 \, \text{m}^2/\text{box}
5) water stop
                      L=(4.10+3.65)\times2\times3
                                                                                         =46.500m/box
                      V = (3.00-0.10) \times 3.50 \times 18.50
6) suport
                                                                                         ≈187.775m<sup>3</sup>
7)scaffold
                      V = 1.2 \times 4.30 \times 18.50 \times 2
                                                                                         =190.920 \,\mathrm{m}^3
8)excavation
                                                             excavation(earth)
                                                              V = 4.30/6 \{ (7.70 \times 67.00 + 12.00 \times 67.00 + (7.70 + 12.00) \}
                                                                  (67.00+67.00)}
                                                                                                              =2837.785 \text{m}^3/\text{box}
                                                             remain:
                                                              V = 4,90 \times 0.10 \times 67,00 + 4,70 \times 4,20 \times 67,00
```

fill: V=2837.785-1355.410 $=1355.410m^3/box$

 $=1482.375m^3/box$



NO3 BOX FOR DRAINAGE

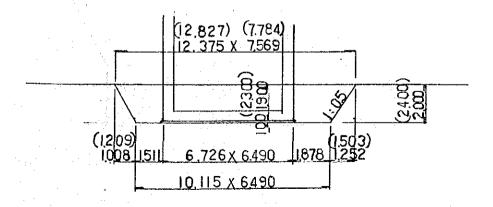
A Company of the company

B2+B3		
top	ade: 25) $V = 4.70 \times 0.70 \times 15.0$ $V = 4.70 \times 0.60 \times "$ $V = 3.00 \times 0.60 \times " \times 2$	=49.350m ³ =42.300 " =54.000 "
total	V = (0)	=145.650 //
out sid: in sid:	A = $(4.70 \times 4.30 - 3.50 \times 3.00) \times 1$ e A = $4.30 \times 15.00 \times 2$ e A = $3.00 \times 15.00 \times 2$ b A = 3.50×15.00	=9.710 m ² (0.0) ([29.00)) =129.000 " [38.710] =90.000 "
	A=	=52.500 // =281.210 // (271.500) (B2) (B3)
the state of the s	e(grade:15) in V = 0. 10×4 . 90×15 . 00 A = $(15.00 \times 2+4.70) \times 0.10$	=7. 350 m ² =3. 470 "
4) suport	$V = (3.00-0.10) \times 3.50 \times 15.00$	=152.250m ³
5) scaffold	$V = 1.2 \times 4.30 \times 15.00 \times 2$	=154.800 "

NO3 BOX FOR DRAINAGE

Retaining wall

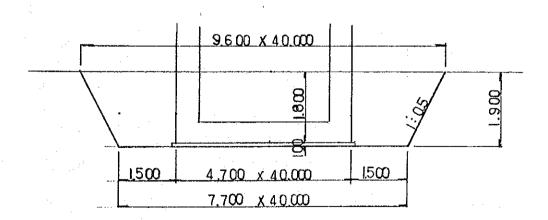
```
1) concrete (grade: 25)
   side wall V = (3.90+0.30) \times 1/2 \times (7.473+9.288) \times 0.40+(0.934+1.161) \times 0.40 \times 0.40 = 14.414 \text{m}^3
   bottom slab V = (4.679+8.773) \times 1/2 \times 6.49 \times 0.40+0.30 \times 1.00 \times 8.773
                    V =
   total
                                                                                                           =34.507 "
2) formiwork form:out side A = (4.30+0.70) \times 1/2 \times (7.473+9.288)
                                                                                                           =41.903 m
   :bottom slab A = (0.302+0.376+8.773\times2)\times1.00+0.40\times(0.934+7.869+1.161+0.403+0.501)=22.571 64474
       : in side A = (3.90+0.30) \times 1/2 \times (7.473+9.288)
                                                                                                            =35.198 "
                    Α=
   total
                                                                                                            =99.672 "
                    V = 1.2 \times (3.90 + 1.0) \times (6.00 + 7.50)
3) scaffold
                                                                                                            =79.380 \,\mathrm{m}^3
4) base concrete(grade:15)
   concrete:main V = 0.10 × {(4.905+8.999) × 1/2 × 6.190+0.60 × (1.032+1.286)}
                                                                                                            =4.442 "
   form: main A = 0.10 \times (6.237 + 7.751 + 1.032 \times 2 + 1.286 \times 2 + 0.605 + 0.751)
                                                                                                            =1.998 m<sup>2</sup>
        and wing
5) gabion:
                   A = 5.00 \times 9.00
                                                                                                            =45.000 //
     t=500
6) joint filler A = (0.403+0.501) \times 3.90+4.679 \times 0.40
                                                                                                            =5.397 //
7) water stop L=4.10 \times 2+4.227
                                                                                                            =12.427m
8) excavation (earth and gabion include)
          (2, 40)
                                          (12.827)(7.784)
                                                                      (12.827)
                                                                                        (7.784)
           V = 2.00/6 \{10.115 \times 6.49 + 12.375 \times 7.569 + (10.115 + 12.375) (6.49 + 7.569)\}
                                                                                                        (WI) (W2)
               +0.30 \times 1.00 \times 8,773 + 5.00 \times 9.00 \times 0.50
                                                                                                  =183.632m<sup>3</sup> (219.242)
                                        (2.30)
   remain: V = 4.442 + 6.726 \times 6.490 \times 1.90 + 0.30 \times 1.00 \times 8.773 + 5.00 \times 9.00 \times 0.50
                                                                                                  =112.512 // (129.973)
   fill: V=183.632-112.512
                                                                                                  =71.120 " (89.269)
              (219, 242-129, 973)
```



NO4 BOX FOR DRAINAGE

(B)(B3)

) concrete(gra	de: 25)	
bottom slab	$V = 4.70 \times 0.70 \times 12.50$	=41.125m ³
top	$V = 4.70 \times 0.60 \times "$	=35, 250 //
side wall	$V = 3.00 \times 0.60 \times " \times 2$	=45.000 <i>"</i>
total	V =	=121. 375 "
2) formwork	en e	
	$A = (4.70 \times 4.30 - 3.50 \times 3.00) \times (1.1326 + 1.0)$	=20. 708 m² 1 100 200
	$A = 4.30 \times 12.50 \times 2$	=107.500 // (28.208
in side:	$A = 3.00 \times 12.50 \times 2$	=75.000 "
:top slab	$A = 3.50 \times 12.50$	=43.750 //
total	A =	=246.958 //
3) base concrete	(grade: 15)	
• .	$V = 0.10 \times 4.90 \times 12.50$	-c 10r-3
the state of the s	$A = \{12.50 \times 2+4.90 \times (1.1326+1.0)\} \times 0.10$	=6.125m ³
	$A = (4.70 \times 4.30 - 3.50 \times 3.00) \times 2$	=3. 535 m ²
	$L=(4.10+3.65)\times 2\times 2$	$=19.420 \mathrm{m}^2/\mathrm{box}$
of water stop	L-(4. 10+3. 03) × 2 × 2	=31.100m/box
6) sapport		
	$V = (3.00-0.10) \times 3.50 \times 12.50$	=126.875m ³
7) scaffold		-120.37011
	$V = 1.2 \times 4.30 \times 12.50 \times 2$	=129.000 //
8) excavation(ear	rth)	
•	0/6 {(7.70×40.00+9.60×40.00+(7.70+9.60)(40.0	0+40 00)} =657 400m3/hav
remain: V = 4.90)×0.10×40.00+4.70×1.80×40.00	=358.000m ³ /box
and the second of the second o	400-358.000	$=299.400 \mathrm{m}^3/\mathrm{box}$
	*** ****	-233.4VVM / DOX



NO4 BOX FOR DRAINAGE

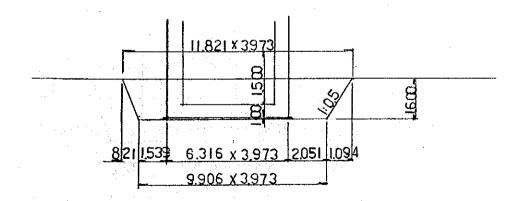
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w	ш	•

1) concrete(gade	:25)	
	$V = 4.70 \times 0.70 \times 15.00$	=49.350m ³
top	$V=4.70\times0.60\times$	=42.300 //
side wall	$V = 3.00 \times 0.60 \times " \times 2$	=54.000 //
total	V =	=145.650 //
a) tormwork		
form:out side	$A = 4.30 \times 15.00 \times 2$	=129. 000 m²
:in side	$A = 3.00 \times 15.00 \times 2$	=90.000 //
:top slab	$A = 3.50 \times 15.00$	=52.500 "
••••	A =	=271. 500 "
	the Quick of the control of the cont	
3) base concrete	(grade:15)	
concrete:mai:	$1 \text{ V} = 0.10 \times 4.90 \times 15.00$	=7.350m ³
form::main	$A = 15.00 \times 2 \times 0.10$	$=3.000\mathrm{m}^2$
The State of State		
4) suport	$V = (3.00-0.10) \times 3.50 \times 15.00$	=152, 250m ³
	$V = 1.2 \times 4.30 \times 15.00 \times 2$	=154.800m ³

NO4 BOX FOR DRAINAGE

(I) Retaining wall

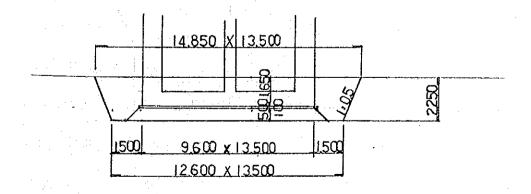
concrete(grade: 25) side wall $V = (3.90+0.30) \times 1/2 \times (4.98)$ bottom slab $V = (4.922+7.710) \times 1/2 \times 3.97$	$4+6.640$) \times 0.40+(0.906+1.207) \times 0.40 \times 73 \times 0.40+0.30 \times 1.00 \times 7.710	3 0. 40=10. 102m³ =12. 350 "
total V=		=22. 452 "
2) formwork form:outside $A = (4.30+0.70) \times 1/2 \times (4.984)$:in side $A = (3.90+0.30) \times 1/2 \times (4.984)$ bottom slab $A = (0.308+0.410+7.710 \times 2) \times 1/2 $	4+6.640)	=29.060 m ² =24.410 // 7+0.547)=20.067 /
total A=		=73. 537 //
3) scaffold $V=1.2\times(3.90+1.0)\times(4.00+1.0)$	5. 40)	=55. 272m ³
concrete:main V = 0.10 × {(5.162+7.950) × 1		=2.549 " =1.493 m ² =40.00 m ²
6) joint filler $A = (0.411+0.547) \times 3.90+4.92$ 7) water sutop $L=4.10 \times 2+4.443$?2×0.40	=5.705 m ² =12.643 m
8) excavation	excavation (earth and gabion; $\cap C$) $V = 1.60/6 \{9.906 \times 3.973 + 11.821 \times + (9.906 + 11.821) (3.973 + 3.973) = 0.30 \times 1.00 \times 7.710 + 5.00 \times 8.00$ remain: $V = 2.549 + 6.316 \times 3.973 \times 1.50 + 0.30 \times 8.00 \times 0.50$ fill: $V = 91.370 - 62.502$	× 3.973)})× 0.50=91.370m ³



NO5 BOX FOR DRAINAGE

(B)+(B9)

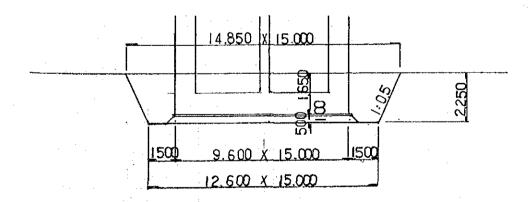
```
1) concrete(grade:25) L=13.500m(mean)
     bottom slab V=9.60\times1.00\times13.50
                                                                                             =129.600m<sup>3</sup>
                       V = 9.60 \times 1.00 \times "
     top
                                                                                             =129,600 "
              wall
                      V=3.50\times1.00\times \times \times 2
                                                                                             =94.500 //
                       V = 3.50 \times 0.60 \times 13.50
     middle wall
                                                                                             =28.350 //
                       V =
     total
                                                                                             =382.050 //
2) formwork
    form: section A = (9.60 \times 5.50 - 3.50^2 \times 2) \times (1.1547 + 1.0)
                                                                                             =60.978 m
        out side A=5.50\times13.50\times2
                                                                                             =148.500 //
      middle wall A=3.50\times13.50\times2
                                                                                             =94,500 "
         : in side A=3.50\times13.50\times2
                                                                                             =94.500 "
         :top slab A=3.50\times13.50\times2
                                                                                            =94.500 "
                       A =
     total
                                                                                             =492.978 "
3) base concrete(grade:15)
    concrete V = 0.10 \times 9.80 \times 13.50
                                                                                             =13.230m3
                       A = \{13.50 \times 2 + 9.80 \times (1.1547 + 1.0)\} \times 0.10
                                                                                             =4.812 m²
4) gravel (crusherran)
                       V = (9.80+10.80) \times 1/2 \times 0.50 \times 13.50
                                                                                            =69.525 \,\mathrm{m}^3
5) joint filler A = (9.60 \times 5.50 - 3.50^2 \times 2) \times 4 + (10.00 \times 5.90 - 3.50^2 \times 2) \times 4 = 251.200 \,\text{m}^2/\text{box}
6) water stop
                       L=(8.60 \times 2+4.50 \times 3) \times 4+(8.80 \times 2+4.70 \times 3) \times 4
                                                                                            =249.600 \text{m/box}
                       V = (3.50-0.10) \times 3.50 \times 13.50 \times 2
7) suport
                                                                                            =321.300m^3
8) scaffold
                       V = 1.2 \times 5.50 \times 13.50 \times 2
                                                                                            =178.200m<sup>3</sup>
9) excavation
                                                               excavation(earth)
                                                                 V = 2.25/6 \{ (12.60 \times 13.50 + 14.85 \times 13.50 \}
                                                                     +(12.60+14.85)(13.50+13.50)}
                                                                                                                    =416.897m<sup>3</sup>
                                                                 V = 69.525 + 13.230 + 9.60 \times 1.65 \times 13.50
                                                                                                                    =296.595m^3
                                                               fill:
                                                                 V = 416.897 - 296.595
                                                                                                                     =120.302m<sup>3</sup>
```



NO5 BOX FOR DRAINAGE

```
B2 B8 L=15.000m
```

) concrete(grad	le:25)	
bottom slab	$V = 9.60 \times 1.00 \times 15.00$	=144.000m ³
	$V = 9.60 \times 1.00 \times "$	=144.000 //
side wall	$V=3.50\times1.00\times "\times 2$	=105.000 //
middle wall	$V = 3.50 \times 0.60 \times 15.00$	=31.500 //
total	V =	=424.500 //
2) formwork		
form:out side	$A = 5.50 \times 15.00 \times 2$	= 165.000 m²
middle wall	$A = 3.50 \times 15.00 \times 2$	=105.000 "
in side:	$A = 3.50 \times 15.00 \times 2$	=105.000 "
	$A = 3.50 \times 15.00 \times 2$	=105.000 //
total	A = 1, 14.	=480.000 //
3) base concrete((grade:15)	•
concrete	$V = 0.10 \times 9.80 \times 15.00$	=14.700m ³
	$A = 15.00 \times 2 \times 0.10$	=3. 000 m²
•	$V = (9.80+10.80) \times 1/2 \times 0.50 \times$	15.00 =77.250m ³
5) suport	$V = (3.50-0.10) \times 3.50 \times 15.00 \times$	
	$V = 1.2 \times 5.50 \times 15.00 \times 2$	=198.000m ³
7) excavation		excavation(earth)
		$V = 2.25/6 \{(12.60 \times 15.00 + 14.85 \times 15.00)\}$
		+(12.60+14.85)(15.00+15.00) =463.219m ³ remain:
2 8 c 5 3	The Mark Day of the Control	$V = 77.250 + 14.700 + 9.60 \times 1.65 \times 15.00 = 329.550 \text{m}^3$
		fill:
		V = 463.219 - 329.550 = 133.669 m ³

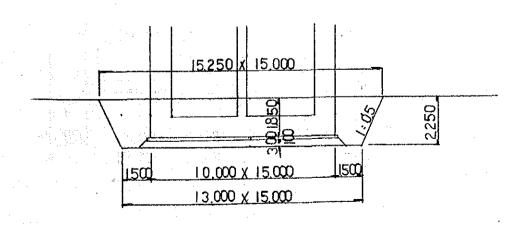


NO5 BOX FOR DRAINAGE

(B3)(B4)-(B7) L=15.000m

```
concrete (grade: 25)
 botton slab V=10.00\times1.20\times15.00
                                                                                         =180.000m<sup>3</sup>
                    V = 10.00 \times 1.20 \times "
                                                                                         =180.000 //
 side | wall \leq V = 3.50 \times 1.20 \times " \times 2
                                                                                         =126.000 "
 middle wall
                  V = 3.50 \times 0.60 \times 15.00
                                                                                         =31.500 "
  total
                                                                                         =517.500 "
                                                        (1)
formwork
                                                                                                                  (211500)
form: section A = (10.00 \times 5.90 - 3.50^2 \times 2) \times 2
                                                                                         =69,000 m² (34,500)
                                                                                                                  246,000
      out side A=5.90\times15.00\times2
                                                                                         =177.000 "
   middle wall A = 3.50 \times 2 \times 15.00
                                                                                         =105.000 //
     : in side A = 3.50 \times 15.000 \times 2
                                                                                         =105.000 "
     : top slab A = 3. 50 \times 2 \times 15.00
                                                                                         =105.000 //
total
                    A =
                                                                                          =561,000 / (526,500)
                                                                                             (B_3)
                                                                                                         (B_{4} \sim 7)
base concrete(grade:15)
 concrete: main V = 0.10 \times 10.20 \times 15.00
                                                                                         =15.300m<sup>3</sup>
                                              (1)
 form: main A = (15, 00 \times 2 + 10, 20 \times 2) \times 0, 10
                                                                                         =5.040 \,\mathrm{m}^2 (4.020)
 gravel
                  V = (10.20+10.80) \times 1/2 \times 0.30 \times 15.00
                                                                                         =47.250m^3
                    V = (3, 50-0, 10) \times 3, 50 \times 15, 00 \times 2
                                                                                         =357.000m^3
suport
scaffold
                  V = 1.20 \times 5.90 \times 15.00 \times 2
                                                                                         =212.400m<sup>3</sup>
```

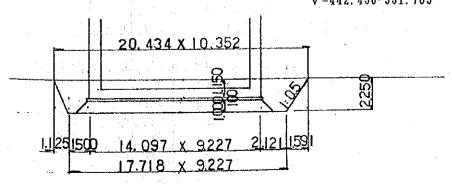
excavation



NOS BOX FOR DRAINAGE

(#) (#2) Retaining wall.

```
1) concrete(grade:25)
    sidewall
                      V = (4.80+0.30) \times 1/2 \times (9.093+12.86) \times 0.50
                         +(1.00+1.414)\times0.50\times0.50
                                                                                            =28.594 \text{m}^3
                     V = (9.983+18.21) \times 1/2 \times 8.227 \times 0.50+0.30 \times 1.00 \times 18.21 = 63.449
    bottom slab
                     V=
    total
                                                                                            =92.043 "
2) tormwork
   form: out side A = (5.30+0.80) \times 1/2 \times (9.093+12.86)
                                                                                            =66.957 \, \text{m}^2
    :bottom slab A = (0.30+0.424+18.21\times2)\times1.00
                         +0.50 \times (0.866+0.50+16.997+1.225+0.707)
                                                                                            =47.292 "
        in side A = (4.80+0.30) \times 1/2 \times (9.093+12.86)
                                                                                            =55.980 //
    total
                                                                                            =170.229 "
3) scaffold
                     V = 1.20 \times (4.80 + 1.00) (7.70 + 10.90)
                                                                                            =129.456m^3
4) base concrete(grade: 15)
    concrete: main V = 0.10 \times \{(10.224+18.451) \times 1/2 \times 7.927+0.60 \times (1.100+1.366)\} = 11.513
    form:main
                     A = 0.10 \times (7.927+11.022+1.10 \times 2+1.366 \times 2+0.70+0.99)
                                                                                            =2.557 \, \text{m}^2
          and wing
5) crusherran
                     V = (8.227+9.227) \times 1/2 \times 1.00 \times 15.338
                                                                                            =133.855m<sup>3</sup>
                     A = 5.00 \times 19.00
                                                                                            =95.000 m<sup>2</sup>
6) gabion:
     t=500
7) joint filler A = (0.50+0.707) \times 4.80+9.983 \times 0.50
                                                                                            =10.785 m<sup>2</sup>
                                                                                            =19.480m
8) water stop
                      L=5.05 \times 2+9.380
9) excavation
                                                             excavation(earth and gabion (NC)Ude)
                                                               V = 2.25/6 \{17.718 \times 9.227 + 20.434 \times 10.352
                                                                    +(17.718+20.434)(9.227+10.352)
                                                                   +2.285\times19.00\times0.50
                                                                                                                     =442.456m<sup>3</sup>
                                                             remain:
                                                               V = 5.00 \times 19.00 \times 0.50 + 133.855 + 11.513
                                                                 +0.30 \times 1.00 \times 18.21 + 14.097 \times 1.15 \times 8.227 = 331.703 \text{m}^3
                                                                V = 442, 456 - 331.703
                                                                                                                     =110.753m<sup>3</sup>
```



BOXCULVERT FOR DRAINAGE-NO®

LIST OF REINFORCED BAR --- B1 = B4

MARK	DIAMETER	LENGTH	HIMBYD		PLECEMETORT	i ·	ļ
,	V 111101 E I	LENVIA	NOWDAK	ONTEXATOR	PIECEREIGNI	WEIGHT	REMARK
		1					
<u>s 1</u>	Y12	8210	45	0.888	7.29	328	
2	Y16	3880	45	1.579	6.13	276	
3	Y12	3880	44	0.888	3.45	152	
4	Y16	2610	88	1.579	4.12	363	
5	Y12	14790	28	0.888	13.13	368	
6	.,	1330	45	11	1.18	53	
7	н	1330	45	11	11	53	
8	,,	1350	108		1.20	130	
9	Y16	7570	. 8	1.579	11.95	96	
10	.,	7010	7		11.07	77	
11	- 37	4480	15	"	7.07	106	
12	Y12	1430	7	0.888	1.27	9	
13	11	1350	5	- 11	1.20	6	
14	1,	1540	16	11	1.37	22	
18	0	4480	2	11	3.98	8	
					· · · · · · · · · · · · · · · · · · ·	2047	kg
W 1	Y12	13670	11	0.888	12.14	134	
2	.,,	13890	11	. 11	12.33	136	:
3	11	15690	- 11	и	13.93	153	
4		15910	11	11	14.13	155	
5	Y16	3870	- 45	1.579	6,11	275	
6	11	3870	45	"	n	275	
7	28	3870	7	"		43	
8	.,	3870	1	.,	11	6	
9	Y12	610	108	0.888	0.54	58	
10		610	108	"	n	58	
11		670	13	. "	0.59	. 8	
	1			L	<u> </u>	1301	ke .
1							········
F 1	Y16	8160	45	1.579	12.88	580	
2	110 n	3880	89	"	6.13	546	
3	"	2610	44		4.12	181	
4	"	2610	44	17	11	181	
<u>4</u> 5	Y12	14790	28	0.888		368	
6	112	14790	45	0,000	1.16	52	
	"	1310	45	13	, 1,10	52	
7	,,			11	1.18	127	
8	 -	1330	108	1.579	11.05	88	
9	Y16	7000	8	1.579		84	
	├──	7560	7	и	7.07	106	
10		4480	15_	0.885		108	
11	"			บเชชรไ	1.25	9	
11 12	Y12	J 410	7_		أمعي		
11	 	J 410 1330	5	11	1.18	6	1- 44
11 12	Y12				1.18	2380	kg
11 12	Y12						kg
11 12	Y12	1330		11	g		kg

BOXCULVERT FOR DRAINAGE-NO(1)

LIST OF REINFORCED BAR --- B2 = B3

MARK	DIAMETER	LENGTH	NUMBAR	UNITWEIGHT	PIECEWEIGHT	WEIGHT	REMARK
					hamman neuronal d	·	
3 1	Y12	8210	51	0.888	7.29	372	
2	Y16	3880	51	1.579	6.13	313	***************************************
3	Y12	3880	50	0.888	3:45	172	
4	Y16	2610	100	1.579	4.12	412	
5	Y12	15300	28	0.888	13.59	381	
6	11	1330	51	12	1.18	. 60	
7	п	1330	51	11	n	60	
. 8		1350	120	11	1.20	144	· · · · · · · · · · · · · · · · · · ·
·						1914	kg
	· · · · · · · · · · · · · · · · · · ·						
N 1	Y12	15300	11	0.888	13.59	149	
2	"	15300	11	11	ń	149	
3		15300	11	n n	tr	149	<u> </u>
4	it	15300	11	11	u i	149	
5	Y16	3880	51	1.579	6.13	313	
6	н	3880	51		n	313	
7	Y12	610	123	0.888	0.54	66	
. 8	<u>"</u>	610	123	" .	. "	66	
						1354	kg
<u> </u>	· · ·					·	
7 1	Y16	8180	51	1.579	12.92	659	
2	"	3880	101	ti .	6.13	619	
3	"	2610	50	11	4.12	206	
4	"	2610	50		u u	206	
5	Y12	15300	28	0,888	13.59	381	· · · · · · · · · · · · · · · · · · ·
6	"	1330	51	47	1.18	60	
7	"	1330	51	"	n	60	
8	h	1350	120	i,	1.20	144	
	:	4.				2335	kg
	· · · · · · · · · · · · · · · · · · ·	Y16		3041			
		Y12		2562 1			
		116		5603 1			

LIST OF REINFORCED BAR --- RETAINING WALL

MARK	DIAMETER	LENGTH	NUMBAR	UNITHEIGHT	PIECEWEIGHT	WEIGHT	REWARK
NO-1	BOX FOR	DRAINAGE			J		
WI	Y16	6260	56.	1 60			
2	Y12	2480	56	1.58	9.89	554	
3	Y16	3350	16	0.888	2. 20	123	
1	"	2690	· · · · · · · · · · · · · · · · · · ·	1.58	5.29	8.5	
5	Y12	700	10	<i>"</i>	4. 25	43	
6	, i.i.	710	8	0.888	0.62	4	· .
7	"	5050	•	. "	0.63	5	
8-1	,,		2.5		4.48	112	
8-2	,,	10000	7		8.88	62	
		2010	1	"	1.78	12	
9	"	586U	18	"	5. 20	94	
10	"	9540	4		8.47	3 4	
11-1		10000	4		8.88	3 6	
11-2		2990	4		2.66	11	
12		2500	1		2, 22	2	
13	. "	2000	. 1	"	1.78	2	
v ° 1	Y12	760	43	0.888	0.67	29	
		630	38	"	0.56	21_	
3	, " "	650	31		0.58	- 18	
4		520	41	" -	. 0.46	19	
5.	" " "	760	3	"	0.67	2	
6	"	650	3		0.58	2	
		· · · · · · · · · · · · · · · · · · ·				1270	kg
<u> </u>	T	·					
K 1	Y12	2750	24	0.888	2, 44	5.9	. •
2_		2820	. 8	"	2.50	20	
3-1	"	7000	. 8		6. 22	50	
3-2	"	3430	8	"	3,05	24	
k ^o 1	Y12	420	18	0.888	0.37	7	
2	"	490	- 6	"	0.44	3	
						153	kg
\$ 1	Y16	7150	28	1.58	. 11.30	316	
2	Y12	7950	2 6	0.888	7.06	184	
3	"	5680	22	0,000	5.04	111	
			12	"	4.00	48	
4	"	4500	16		1, 00		
s ^o i	VIA	1150	116	0.888	1.02	118	
S - 1	Y12	1130	110	V. 000	1.00	777_	kg
		<u> </u>					<u>.</u>
			Y16	998			
	· · · · · · · · · · · · · · · · · · ·			1212			
	··		Y12				
i				2210 kg			

BOXCULVERT FOR DRAINAGE-NO®

LIST OF REINFORCED BAR --- BI B2

VARY	DIANETER				RI = RS	i merana	1 _
ANNA	VIAMETER	LENGTH	MAGMUM	UNI I-XE IGHT	PIECEWEIGHT	REICHT	REMARK
				~~ <u>~~</u>		_	
S 1	Y16	6780	57	1.579	10.71	610	
2	Y12	2680	113	888.0	2.38	269	· · · · · · · · · · · · · · · · · · ·
	Y16	5020	5.6	1.579	7,93	444	
	Y12	17400	20	0.888	15.45	309	
	, "	1050	_57	,,	0.93	53	
6	"	1050	57	11	. 11	53	
7		1150	81	,,	1.02	83	
8	Y16	5760	3	1.579	9.10	27	
ç	, ".	5200	2		8.21	16	
10	Y12	2780	5	0.888	2.47	12	
11		1060	3	18	0.94	3	
12	,	1340	11	,,	1.19	13	
13	"	2770	2	н	2.46	5	
	: :					1897	kg
			:				
W 1	Y12	17040	11	0.888	15.13	166	
2	"	17110	11	+1	15.19	167	
3	"	17680	11	.,	15.70	173	i
4	13	17760	11	11	15.77	_ 173	
. 5	. "	3680	57	1)	3.27	186	
6		3680	57	19	11	186	
7		3680	3	"	.,	10	
8	3.0	3680	1			3	
9		510	138	11	0.45	62	
10		510	138	.,	1)	62	
. 11		520	3	17,	0.46	1	
					<u> </u>	1189	kg
							<u>,</u>
F 1	Y16	6780	57	1.579	10.71	610	
. 2	Y12	2680	113	0.888	2.38	269	
3	Y16	5020	56	1.579	7.93	444	
4	Y12	17400	20	0.888	15.45	309	·
5		1050	57		0.93	53	
6	"	1050	57	0 4		53	ļ
7	++	1150	81	11	1.02	83	
8	Y16	5200	3	1.579	8.21	25	
9	, ,,	5760	2	13	9.10	18	
10	Y12	2780	5	0.888	2.47	12	ļ
11	11	1060	3	. н	0.94	3	<u> </u>
	·	·		<u> </u>	<u> </u>	1879	kg
					······································		
		Y16	<u> </u>	2194 1	ξ <u>g</u>		
· · ·		Y12		2771	¢g		
	· .			4965	kg	····	

LIST OF REINFORCED BAR --- RETAINING WALL

		·			LIC LATEACH	SO SAUTE	
MARK	DIAMETER	LENGTH	NUMBAR	UNITWEIGHT	PIECEWEIGHT	WEIGHT	REMARK
NO-2	BOX FOR	DRAINAGE	·		<u> </u>		
₩ 1	Y 1 6	5660	2.8	1.58	8,94	250	
2	Y 1 2	2650	28	D. 888	2, 35	86	
3	Y16	3280	. 9	1.58	5. 18	47	
. 4	"	3020	2	"	4.77	10	
5	Y12	900	8	0.888	0,80	6	
·. 6	"	940	8	"	0.83	7	
7	,,	2920	23	,,	2,59	- 60	
. 8	"	3620	23	"	3. 21	74	
9	"	5900	4	"	5. 24	21	
10		6860	4	,,	8.09	24	
11	,,	2200	1	"	1.95	2	
12	,,	2500	1	,,	2. 22		
					2.62	2	
wo I	Y 1 2	760	18	0.888	0.67	1 2	
2	"	630	21	"	0.56	12	
3	,,	690	18	"	0.61		
4		560	20	"	0.50	11	· · · · · · · · · · · · · · · · · · ·
5		760	3	,,			
6		690	3	,,	0.67	2	
<u></u>	ل	030	1		0.61	2	1
1		1	1 11			618	kg
Κl	Y12	2750	18	0.888	2.44	44	·
2	"	2820	5	"	2.50	13	
3	"	2770	5	"	2.46	12	
4	"	8280	8	"	7.35	59	
k ^o 1	Y12	420	14	0.888	0.37	. 5	
2	. "	490	3	. "	0.44	I	
111						134	k g
		·			1		
S 1	Y16	5610	14	1.58	8,86	124	
2	Y12	3380	20	0.888	3.00	60	
		2540	10		2.26	23	 -
4		2670	6		2.37	. 14	
5	"	2500	12		2.22	27	
S0 1	Y12	1120	34	0.888	0.99	34	
				·		282	kg
				<u>:</u>		· · · · · ·	
		<u> </u>					
			¥16	431		<u> </u>	
			Y 1 2	431 603 034 kg			

LIST OF REINFORCED BAR =--B1 - B4

MARK	DIAMETER	-		UNITWEICHT	PIECEWEIGHT	WP LOUIS	T
*********		DONOTH		ORTHALIUM	FIECEREIGNI	WEIGHT	REMARK
	 			<u> </u>		····	
3		9280	59	3.854	35.77	2110	
	2 "	4570	117	11	17.61	2060	
	7	2880	58	6.313	18.18	1054	
	1 "	2880	58	11	н '	1054	
	Y20	19070	32	2.466	47.03	1505	<u> </u>
6	1	1590	59	0.888	1.41	83	
7		1590	59	11	n .	83	
	3 "	1570	140	11	1.39	195	************
{		9260	7	6.313	58,46	409	
10) "	8140	6	"	51.39	308	
11	1	4930	13	3.854	19.00	247	
12	Y12	1650	- 6	0.888	1.47	9	
13	3 "	1570	4	ii ii	1.39	6	
14	("	1710	18	- 11	1.52	27	<u> :</u>
15	<u>; </u>	4930	2	11	4.38	9	
<u> </u>	<u> </u>	1 1 2 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		· · ·		9159	kg
			·				
1- 1	Y20	18140	11	2.466	44.73	492	
2	Y12	18050	11	0.888	16.03	176	
3	3 "	19520	11	1)	17.33	191	
4	1 Y20	19990	11	2,466	49.30	542	
5	, "	4170	59	"	10.28	607	
e	11,	4170	59	n		607	
7	, ,,	4170	6	11	++	62	······
8	, ,	4170	1	"	1)	10	
9	Y12	710	143	0.888	0.63	90	
10	, ,	710	143	,,	11	90	
. 11	13.	750	10	н	0.67	7	
	<u> </u>		: · · ·			2874_	kg
				<u> </u>		·	
. 1	Y25	9480	59	3.854	36.54	2156	
. 2	. 11	4570	117	11	17.61	2060	
. 3	17	3020	58	ņ	11.64	675	
4	1	3020	58	н	н	675	
5	7	18930	. 31-	1.579	29.89	927	
6		1730	59	0.888	1.54	91	
. 7		1730	59	11	n	91	
8	 	1770	140	17	1.57	220	
9	1	7980	7	3.854	30.75	215	
10	1	9100	6	"	35.07	210	
	1	4930	13	t ı	19.00	247	
11	1	1790	6	0.888	1.59	10	
12			4	"	1.57	6	
13	<u> </u>	1770	4_		1.07	7583	kg
	<u> </u>		<u> </u>	· · · · · · · · · · · · · · · · · · ·		1963	<u>,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,</u>
				2025	· · · · · · · · · · · · · · · · · · ·		
		Y32		2825 }			-i
		Y25		10655 k			
		Y20		3825 k			
		Y16		927 F			
		Y12		1384 }	<u>, e </u>	·	
~····				19616 k	- 44		

BOXCULVERT FOR DRAINAGE-NO®

LIST OF REINFORCED BAR---B2 = B3

	<u> </u>		ç		DZ = いい		
MARK	DIAMETER	LENGTH	NUMBAR	UNITWEIGHT	PIECEWEIGHT	WEIGHT	REMARK
		n di series					
s 1	Y25	9280	51	3.854	35.77	1824	
2	11	4570	101	р	17.61	1779	
3	Y32	2880	50	6.313	18.18	909	
4		2880	50	"	n	909	
5	Y20	15570	32	2.466	38.40	1229	
6	Y12	1590	51	0.888	1.41	72	
7	"	1590	51	n	11	72	
. 8		1570	120	tı	1.39	167	
						6961	kg
W 1	Y20	15570	11	2.466	38.40	422	
2	Y12	15290	11	0.888	13.58	149	
3	**	15290	11	"	"	149	
4	Y20	15570	11	2.466	38.40	422	
5	"	4170	51	н	10.28	524	
6	п	4170	51	H	"	524	
7	Y12	710	123	0.888	0.63	77	
8	h	710	123	н	19	77	
						2344	kg
	· · · · ·						
F 1	Y25	9480	51	3.854	36.54	1864	
2	н	4570	101	"	17.61	1779	
3	11	3020	50	ń	11.64	582	
4		3020	50	н		582	
5	Y16	15430	32	1.579	24.36	780	
6	Ý12	1730	51	0.888	1.54	79	
7	n	1730	51	11	15	79	
8	, tr	1770	120	- 11	1.57	188	
						5933	kg
		e Fue					
		Y32		1818	(g		
	<u> </u>	Y25		8410 1	₹ g		
		Y20		3121 1	cg		
		Y16		780]	rg .		
	V	Y12		1109	(g		
	••••						

LIST OF REINFORCED BAR --- RETAINING WALL

MARK	DIAMETER	LENGTH	NUMBAR	UNITWEIGHT	PIECEWEIGHT	WEIGHT	REMARK
NO-3 I	BOX FOR	DRAINAGE	<u> </u>	<u> </u>			
M T	Y16	6150	46	1.58	9.72	447	
2	X15	2630	46	0.888	2.34	108	
3_	Y16	3400	13	1.58	5.37	70	
4	"	2860	7	"	4.52	32	
5	Y12	760	6	0.888	0.67	4	
6	"	780	8	"	0.69		
7	"	4400	25	"	3.91	98	
8	"	5300	25	"	4.71	118	
9	"	8170	4	"	1. 25	29	· · · · · · · · · · · · · · · · · · ·
10	,,	9800	4	,	8.70	3.5	
11	"	2000	1	"	1.78	2	:
12	"	2200	. 1	"	1.95	2	
•						:	
w ⁰ 1	. Y12	720	27	0.888	0.64	17	
2	2	590	30	. "	0.52	16	
3	"	650	26	"	0.58	15	
4	"	520	30	" "	0.45	14	
\$	"	650	3	"	0.58	2	
6	<i>"</i>	720	3	"	0.64	2	,
:				•		1017	kg
K 1	Y12	2740	22	0.888	2.43	5.3	
2	"	2790	8	, ,,	2.48	20	
3	"	8640	8	"	7.67	61	-
k ^o 1	Y12	410	15	0.888	0.36	5	
2	"	460	5	,,	0.41	. 2	
· ·						141	kg
							· ·
SI	41.6	6950	23	1.58	10.98	253	
2	Y12	6420	28	0.888	5.70	160	
3	"	4580	12		4.07	4.5	
4		4300	<u> 12</u>		3.82	46	
					1 00	0.6	
50 1	Y12]	1150	84	0.888	1.02	86	l a
· · · · · ·	<u> </u>		· · · · ·			594	kg
· <u>:</u>							
 	:		Y16	802		<u> </u>	
· · · · · · · · · · · · · · · · · · ·			Y 1 2	950			
		13,1	: 	1752 kg		· · · · · · · · · · · · · · · · · · ·	····

LIST OF REINFORCED BAR ... BI = B3

HIDV	ALLUCTED	L DUATE	uuuo b	·	BI = 83	Ī	<u> </u>
MARK	DIAMETER	LENGTH	NUMBAK	I OUT EXELORY	PIECEWEIGHT	AE1CH1	REMARK
				<u> </u>			F
3 1	Y20	9160	38	2.466	22.59	858	
<u> 2</u>	11	4570	75	**	11.27	845	
3		2900	37	3.854	11.18	414	
4		2900	37	."	11	414	
5		12920	3.2	1.579	20.40	653	· :.
6	Y12	1590	38	0.888	1.41	54	
7	"	1590	38	***	ti ·	54	l
8	- 11	1550	88	11	1.38	121	
9	Y25	9000	9	3.854	34.69	312	
10	**	8120	. 8		31.29	250	
11	A50	5180	17	2.466	12.77	217	
12	Y12	1690	8	0.888	1.50	12	:
13	11	1550	7	н	1.38	10	·
14	ų.	1710	19	.,	1.52	29	
15		5180	2	**	4.60	9	<u> </u>
		<u> </u>				4252	kg
				·	<u> </u>		
, 1	Y12	11570	11	0.888	10.27	113	
2		11820	11		10.50	116	
3	, ,,	13750	11	."	12.21	134	<u> </u>
4	"	14000	11		12,43	137	
5	Y16	4170	38	1.579	6.58	250	L
6		4170	38			250	
7	**	4170	8	"	"	53	
8	H	4170	1	18	H .	<u> </u>	<u> </u>
9	Y12	710	90	0.888	0.63	57	l <u></u>
10	n	710	90	. 0	- "	57	
11	""	770	15	<u>"</u> _	0.68	10	
			·	<u> </u>	·	1184	kg
	VIE	0000	90	1 570	14 90	ces	
F 1	Y16	9390	38	1.579	14.83	564	
2	Y20	4570	75	2.466	11.27	845	
3	"	3030	37	" "	7.47	276	
4	11	3030	37			276	
5	Y16	12920 -	32	1.579	20,40	- 653	
6	Y12	1730	38	0.888	1.54	59	
7	"	1730	38	"		59	<u> </u>
8		1750	88	"	1.55	136	
9	Y20	8040	9_	2.466	19.83	178	
10	**	8920	8	"	22.00	176	
11	"	5180	17	"	12.77	217	
12	Y12	1830	88	0.888	1.63	13	
13	,, ,	1750	7	"	1.55	11	L <u></u>
		<u> </u>				3463	kg
·				1000		····	
<u></u>		Y25		1390 l 3888 l			
		Y20				···,	
		VIC					
	<u> </u>	Y16	······	2430 l			
		Y16 Y12		1191) 8899)	(g		

BOXCULVERT FOR DRAINAGE -NO@

LIST OF REINFORCED BAR ---- B2

MARK	DIAMETER	LENGTH	NUMBAR	UNITWEIGHT	PIFCEWEIGHT	MEIGHT	OPHAR
····	<u> </u>				1		REMARK
							
S	Y20	9150	51	2.466	22.56	1151	
	"	4570	101	н	11.27	1138	
		2900	50	3.854	11.18	559	
	"	2900	50	n	, n	559	
!	Y16	15430	32	1.579	24.36	780	
	Y12	1590	51	0.888	1.41	72	
٠. ٩	"	1590	51	15	н	72	
	3	1550	120	11	1.38	166	
:			<u> </u>			4497	kg
A	Y12	15290	11	0.888	13.58	149	
2	2 "	15290	11	Н	**	149	
	3 "	15290	11	n		149	
	11	15290	11	11	1)	149	
	Y16	4170	51	1.579	6.58	336	
("	4170	51	ei .	n	336	
, s - 5, 9	Y12	710	123	0.888	0.63	77	
. 8	3 "	710	123	4	11	77	
						1422	kg
F 1	Y16	9390	51	1.579	14.83	756	
	Y20	4570	101	2.466	11.27	1138	
	 	3030	50	11	7.47	374	
,	 	3030	50	. ar	tr.	374	
: (1	15430	32	1.579	24.36	780	
. (1	1730	51.	0.888	1.54	79	:
	1	1730	51	"	"	79	
{		1750	120	10 .	1.55	186	
		1				3766	kg
		Y25		1118	rg		
		Y20		4175 k			
		Y16		2988	-		
		Y12		1404 P			+
		. 4 6		1101 F	· <u>····</u>		·

LIST OF REINFORCED BAR --- RETAINING WALL

	·						
XARK	DIAMETER	LENGTH	NUMBAR	UNITWEIGHT	PIECEWEIGHT	жетент	REMARK
NO-4 1	OX FOR	DIRAINAGE			· · · · · · · · · · · · · · · · · · ·	l	
W I	Y16.	5990	30	1.58	9, 46	284	
2	Y12	2700	30	0.888	2.40	_ 72	
3	9 LY	3500	8	1.58	5. 53	44	
4	. //	3060	4	,,	4.83	19	
5	Y12	830	6	0.888	0.74	4	
6	"	870	. 8	"	0.17	. 6	· · · · · · · · · · · · · · · · · · ·
7	"	2830	2.5	"	2.51	63	······································
8		4100	2 5	,,	3.64	91	
9	"	6000	4 .	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	5.33	21	
10	ii .	7370	4	,,	6.54	26	·
11	"	2000	1	"	1.78	2	
12	"	2200	1	11.	1.95	2	
w ⁰ 1	Y12	750	19	0.888	0.67	13	
2		620	21	"	0.55	15	
3	"	650	18	"	0.58	10	
4		520	- 23	"	0.46	11	
5		650	3	,,	0.58	2	
8	"	750	3_	,,	0.67	2	
1	·			·.	<u>.</u>	684	k g
	· · · · · · · · ·	· · · · · · · · · · · · · · · · · · ·			·		
K 1	Y12	2750	21	0.888	2.44	51	
2	,,	2810	5	. "	2.50	13	·
3		7570	8	"	6.72	54	:
et ii							
k ^o t	Y12	420	13	0.888	0.37	5	
2	"	480	3		0.43	. 1	
· .	14.			····		124	kg
				· .			·
S 1	Y16	6190	15.	1.58	9,78	147	
2	Y12	3960	30	0.888	3.52	106	
3	11	3040	8	,,	2.70	22	
4	,,	2500	6	"	2.22	13	
SO 1	818	1150	46	0.888	1.02	47	_ _
							kg
			Y 1 6	494			
			Y12	649			
				143 kg			

BOXCULVERT FOR DRAINAGE-NOS

LIST OF REINFORCED BAR ==- BI = B9

MARK	DIAMETER	LENGTH	NUMBAR	TEOLOWELKU	PIECEWEIGHT	#EICHT	REMARK
S 1	Y20	6160	37	2.466	15,19	562	
2	Y25	4770	37	3.854	18.38	680	117
3	Y20	6190	-37:	2.466	15.26	565	
4	11	9470	73	*1	23.35	1705	
5	n }	4790	36	п	11.81	425	
6	Y25	4770	36	3,854	18.38	662	
7	Y20	5460	36	2.466	13.46	485	
8	11	14060	56	11	34.67	1942	
9	Y12	2710	37	0.888	2.41	89	
10	"	2710	37	п	11	89	
1,1	11	2350	204	17	2.09	426	
12	Y25	17040	19	3.854	65.67	1248	
13	***	16340	18	11	62.97	1133	
14	Y20	11640	37	2.466	28.70	1062	
15	Y12	2920	17	0.888	2.59	44	
16	11	2100	74	11	1.86	138	
17	п	2250	19	11	2.00	38	
18	11	2350	46	.17	2.09	96	
19	35 100 m m m m	2510	38	н	2.23	85	· .
20	+1	11360	2) 1	10.09	20	
						11494 1	(g
W 1	Y16	11190	13	1.579	17.67	230	
2	Y12_	11550	13	0.888	10.26	133	
3	Ťŧ	16010	13	11	14.22	185	
4	Y16	16650	13	1.579	26.29	342	
5	Y12	13640	13	0.888	12.11	157	
6	11	13920	13_	11	12.36	161	
7	Y16	5370	74	1.579	8.48	628	
8	11	5370	74	11	11	628	
9	• • • • • • • • • • • • • • • • • • • •	5370	35	"	n	297	
10	п	5370	2_	+1	11	17	
11	Y12	1100	88	0.888	0.98	86	
	11	1100	88_	Ŋ	11	86	
12					1 10	42	
12 13	11	- · · 1240	38_	, "	1.10	- 42	

BOXCULVERT FOR DRAINAGE-NOS

LIST OF REINFORCED BAR == B1 = B9

					- DI D	,	
MARK	DIAMETER	LENGTH	NUMBAR	UNITWEIGHT	PIECEWEIGHT	WEIGHT	REMARK
:						1	
F 1	Y20	6190	37	2.466	15.26	565	· /************************************
2	Y25	4770	37	3.854	18.38	680	
3	Y20	6160	37	2.466	15.19	562	
4		9470	73	,,	23.35	1705	
5	Y25	14750	36	3.854	56.85	2047	
6	Y20	14060	56	2.466	34.67	1942	
7	Y12	2710	37	0.888	2.41	89	
8		2710	37	n n	**	89	
9	11	2350	204	Ħ	2.09	426	
10	Y25	16340	19	3.854	62.97	1196	
11	11	17040	18	39	65.67	1182	
12	Y20	11640	37	2.466	28.70	1062	
13	Y12	2920	17	0.888	2.59	44	
14		2100	74	11	1.86	138	
15	"	2250	19	.,	2.00	38	
16	, "	2350	46	,,	2.09	96	
*						11861	kg
·.		<u>·</u>			·	<u> </u>	
·		.Y25		8828	kg		~~~~
!		Y20		12582	κg		<u> </u>
		Y16		2142	ζg		
•		Y12		2807 1	(g		
				26359 1	кg		
				•		•.	

BOXCULVERT FOR DRAINAGE-NO®

LIST OF REINFORCED BAR --- B2 = B8

ΧA	RK .	DIAMETER	LENGTH	NUMBAR	UNITAEIGHT	PIECEWEIGHT	WEIGHT	REMARK
	•							
3	1	Y20	6160	51	2.466	15.19	775	
	2	Y25	4980	51	3.854	19.19	979	
	_3	Y20	5980	51	2.466	14.75	752	
	4	11	9470	101	13	23.35	2358	
	. 5	u	4580	50	11	11.29	564	
	6	Y25	4980	50	3.854	19.19	960	
	_7	Y20	5460	50	2.466	13.46	673	
	8	0	15570	56	i i	38.40	2150	
	9	Y12	2710	51	0.888	2.41	123	
	10	11	2710	51	17	"	123	
	11	11	2100	102	110	1.86	190	
_	12	0 .	2350	288		2.09	602	
_					The same of the	<u> </u>	10249	kg
							· · · · · · · · · · · · · · · · · · ·	
1	_1	Y16	15430	13	1.579	24.36	317	
	2	Y12	15290	26	0.888	13.58	353	
<u>.</u>	3	+	15290	26	11	. 0	353	
	4	Y16	15430	13	1.579	24.36	317	
	5	"	5370	102	11	8.48	865	
	6	+1	5370	102	11	tr	865	
	_ 7	Y12	1100	123	0.888	0.98	121	
	8	11	1100	123	11	lf .	121	
	9	"	700	123	"	0.62	76	
							3388	kg
					· · · · · · · · · · · · · · · · · · ·	`		
F	1	Y20	5980	51.	2,466	14.75	752	
	2	Y25	4980	51	3.854	19.19	979	
	3	Y20	6160	51	2.466	15.19	775	···
	4		9470	101	"	23.35	2358	
	5	Y25	14960	50	3.854	57.66	2883	
	6	Y20	15570	56	2.466	38.40	2150	
	7	Y12	2710	51	0.888	2.41	123	· · ·
	8	. 0	2710	51	11	"	123	
	9	"	2100	102	11	1.86	190	
٠.	10	ft.	2350	288	0	2.09	602	
			. :				10935	kg
			<u> </u>					
			Y25		5801	¢g		
. : '	: .		Y20		13307 1	۲g		
	٠.		Y16		2364	¢g		
			Y12		3100 1	(g		
					24572_1	e 0		

BOXCULVERT FOR DRAINAGE-NOS

LIST OF REINFORCED BAR --- B3 ~ B7

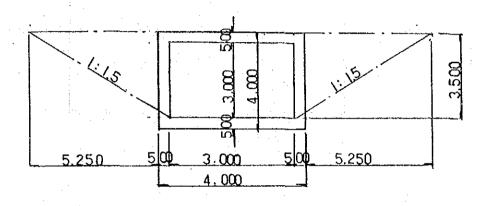
DIAMETER	LENGTH	NUMBAR	UNITWEIGHT	PIECEWEIGHT	MEIGHT	REMARK
***************************************	And the second s	·				<u>!</u>
1 Y25	6760	51	3.854	26.05	1329	
2 Y32	5220					
3 Y25	6760					
4 "	9860	51	U			
5 Y20	9860	50	2.466			
6 Y25	15740	50	3.854			
7 Y20	15560	56	2.466		2149	
8 Y12	3270	51	0.888	2.90	148	
9 "	3270	51	11	15	148	
0 "	2370	51	. 0	2.10	107	
1 "	2760	288	"	2.45	706	
<u> </u>					13782	kg
	·				·	·
1 Y20	15560	13	2,466	38.37	499	
2 Y12	15280	26	0.888	13.57	353	
3 "	15280	26	17	н .	353	
4 Y20	15560	13	2.466	38.37	499	
5 "	5760	102	. "	14.20	1448	
6 "	5760	102	0	ti .	1448	,
7 Y12	1310	123	0.888	1.16	143	
8 "	1310	123	"		143	
9 "	700	123	0	0.62	76	
······································	<u> </u>	· .			4962	kg
1			· · · · · ·			
1 Y32	18690	51				
	9860	101	3.854	38.00	3838	
1	15740	50		60.66		
	15560	56				· ·
						
<u></u>						·
-						
3 "	2760	288_		2.45		
					10140	<u> </u>
			7607 1	τσ		-
· · ·						
	120			and the second second		
	Y12		3286 J	< ⊈		
	2 Y32 3 Y25 4 " 5 Y20 6 Y25 7 Y20 8 Y12 9 " 0 " 1 Y20 2 Y12 3 " 4 Y20 5 " 6 " 7 Y12 8 " 9 " 1 Y32 2 Y25 3 " 4 Y20 6 Y12 6 " 7 Y12	2 Y32 5220 3 Y25 6760 4 " 9860 5 Y20 9860 6 Y25 15740 7 Y20 15560 8 Y12 3270 0 " 2370 1 " 2760 1 " 2760 1 " 5760 5 " 5760 6 " 5760 7 Y12 1310 8 " 1310 9 " 700 1 Y32 18690 9 " 700	1 Y25 6760 51 2 Y32 5220 51 3 Y25 6760 51 4 " 9860 51 5 Y20 9860 50 6 Y25 15740 50 7 Y20 15560 56 8 Y12 3270 51 9 " 3270 51 1 " 2370 51 1 " 2760 288 1 Y20 15560 13 2 Y12 15280 26 4 Y20 15560 13 5 " 5760 102 7 Y12 1310 123 8 " 1310 123 8 " 1310 123 9 " 700 123	1 Y25 6760 51 3.854 2 Y32 5220 51 6.313 3 Y25 6760 51 3.854 4 " 9860 51 " 5 Y20 9860 50 2.466 6 Y25 15740 50 3.854 7 Y20 15560 56 2.466 8 Y12 3270 51 0.888 9 " 3270 51 " 0 " 2370 51 " 1 " 2760 288 " 1 15280 26 " 4 Y20 15560 13 2.466 5 " 5760 102 " 6 " 5760 102 " 7 Y12 1310 123 0.888 8 " 1310 123 " 9 " 700 123 " 1 Y32 18690 51 6.313 2 Y25 9860 101 3.854 3 " 15740 50 " 4 Y20 15560 56 2.466 5 " 5760 56 2.466 6 " 370 51 " 7 Y12 1310 123 " 9 " 700 123 " 1 Y32 18690 51 6.313 2 Y25 9860 101 3.854 3 " 15740 50 " 4 Y20 15560 56 2.466 5 Y12 3270 51 0.888 6 " 3270 51 " 7 " 2370 51 " 7 " 2370 51 " 8 " 2760 288 "	1	1 Y25

LIST OF REINFORCED BAR --- RETAINING WALL

	·	· · · · · · · · · · · · · · · · · · ·		·		KC TAINII	IO WYLL	
	MARK	DIAMETER	LENGTH	NUMBAR	UNITWEIGHT	PIECEWEIGHT	REIGHT	REMARK
	NO-5	BOX FOR D	RAINAGE				·	
ļ	W 1	Y16	6850	84	1.58	10.82	909	
	2	Y12	3150	84	0.888	2.80	235	·
	3	Y16	4150	2 3	1.58	6.56	151	
	44	,,	3400	16	,,	5.37	86	
	5	Y12	860	6	0.888	0.76	5	·
	6	<i>"</i>	880	8	. "	0.78	6	
		"	5100	31	,,	4.53	140	
	8-1	"	10000	11	"	8.88	98	
	8-2	"	1910	11	"	1.70	19	
	9	"	5580	20	"	4.96	99	·
	10	"	10000	4	"	8.88	36	
	11-1	<i>"</i>	10000	4	,,	8, 88	36	
	11-2	,,	3880	4	"		*	
	12	,,	2000	1	- "	3, 45	14	
	13	"	2200			1.78	2	
	1		0033	1		1,95	2	
	жо I	Y12	900		0 000	0.00	-	
	2	" .		46	0.888	0.80	37	
			770	79		0.68	54	
ı	3	"	740	45		0.66	30	i
	4		610	78	"	0.54	42	
	. 5		740	3	<i>"</i>	0.86	2	•
	6		900	3		0.80	2	
	<u></u>	:	·				2005	
			;		<u>-</u>			-:
	<u>K 1</u>	Y12	2940	46	0.888	2.61	120	
	<u>2</u>		3020	15		2.68	- 40	
	3-1		10000	8		8.88	71	
	3 - 2	"	8490	8	"	7.54	60	
	k ⁰ 1	Y12	410	33	0.888	0.36	12	
	2		490	11	"	0.44	5	
	31	· ·	i alain				308	
		VI.E	7500	42	1.58	11.85	198	
l	S 1	Y16		84	1.58	5.00	504	
. : .	2	"	3800		0.888_	6.66	280	
	3	Y12	7500	42		7.20	446	
	4		8110	62		5.98	155	
	5		6730	26	"	5.33	139	
-	6	"	6000	26		0.33	100	
		-				, , ,	420	
	<u> 80 1</u>	Y12	1250	378	0.888	1.11		· · · · · · · · · · · · · · · · · · ·
	-						2442	
				· · · · · · · · · · · · · · · · · · ·				<u></u> .
	<u> </u>			Y16	2148	· · · · · · · · · · · · · · · · · · ·		
	1			Y 1 2	2607			
	 				4755 kg			•

NO 0 BOX CULVERT FOR DRAINAGE BACK FILL

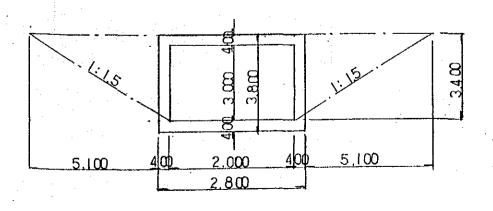
 $V = 3.50 \times 5.25 \times (59.00 + \frac{1}{3} \times 10.00 \times 2) = 1206.6 \, \text{m}^3$



NO @ BOX CULVERT FOR DRAINAGE

BACK-FILL

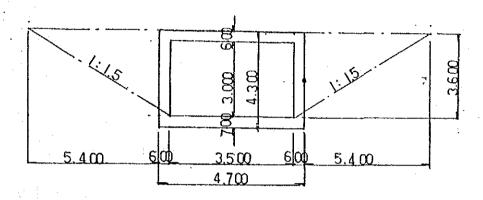
$$V = 3.40 \times 5.10 \times (34.20 + \frac{1}{3} \times 4.50 \times 2) = 645.0 \,\mathrm{m}^3$$



note include retaining <u>wall</u>

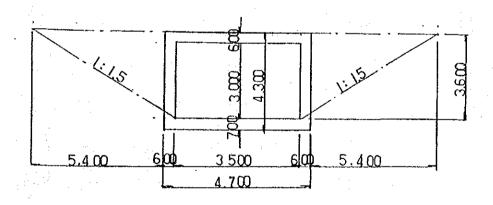
NO 3 BOX CULVERT FOR DRAINAGE BACKFILL

 $V = 3.60 \times 540 \times (67.00 + \frac{1}{3} \times 8.00 \times 2) = 1406.2 \text{ m}^3$



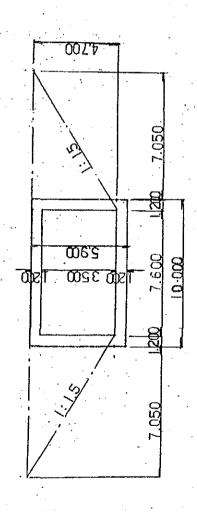
NO @ BOX CULVERT FOR DRAINAGE BACK FILL

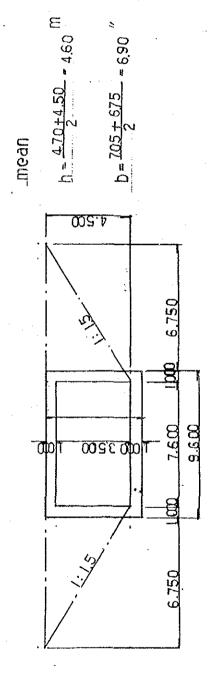
 $V = 3.60 \times 5.40 \times (40.00 + \frac{1}{3} \times 5.50 \times 2) = 848.9^{\text{m}^3}$



NO 6 BOX CULVERT FOR DRAINAGE BACKFILL

 $V = 4.60 \times 6.90 \text{ C}(32.00 + \frac{1}{3} \times 10.50 \times 2)$





BOX FOR DRAINAGE

```
NO① UF2 A = (4.00+3.00) \times 59.00+ \{(3.464+9.17) \times 1/2 \times 7.794 \times (9.00+12.247) \times 0.40\} \times 2

= 528.467 m<sup>2</sup>

NO② "A = (2.80+2.00) \times 34.20+ \{(2.07+7.404) \times 1/2 \times 3.381 \times (5.019+6.147) \times 0.40\} \times 2

= 205.124 "
```

NO③ " A =
$$(4.70+3.50) \times 67.00 + \{(3.775+7.869) \times 1/2 \times 6.490 \times (7.473+9.288) \times 0.40\} \times 2$$

= 638.378 "

NO@
$$"A = (4.70 + 3.50) \times 40.00 + \{(3.964 + 6.752) \times 1/2 \times 3.973 \times (4.984 + 6.64) \times 0.40\} \times 2$$

$$= 379.874 "$$

NO(5) " A =
$$(9.60+7.00) \times 28.50 \times 2 + (10.00+7.00) \times 75.00$$

+ $\{(8.776+16.997) \times 1/2 \times 8.227 + (9.093+12.86) \times 0.50\} \times 2$ = 2455.187 "

NO(1) BOX FOR DRAINAGE

porous drainage $\Lambda = \{59.00 \times 3.20 \times 2 + \frac{1}{2} \times 10.00 \times 3.20 \times 2 \times 2\}$ (×0.300) = 441.600 m³ perforated pipe $L = 59.00 \times 2 + 10.00 \times 2 \times 2$ = 158.000 m drain pipe $N = (59.00 / 5.00 + 1) \times 2 + (10.00 / 5.00 + 1) \times 2 \times 2$ = 38 N

NO@ BOX FOR DRAINAGE

porous drainage $A = \{34.20 \times 3.10 \times 2 + \frac{1}{2} \times 4.50 \times 3.10 \times 2 \times 2\}$ (×0.300) = 239.940 m³ perforated pipe $L = 34.20 \times 2 + 4.50 \times 2 \times 2$ = 86.400 m drain pipe $N = (34.20 \times 5.00 + 1) \times 2 + (4.50 \times 5.00 + 1) \times 2 \times 2$ = 26 N (ϕ 75mm)

NO3 BOX FOR DRAINAGE

porous drainage $A = \{67.00 \times 3.30 \times 2 + \frac{1}{2} \times 8.00 \times 3.30 \times 2 \times 2\}$ (×0.300) = 495.000 m³ perforated pipe $L = 67.00 \times 2 + 8.00 \times 2 \times 2$ = 166.000 m drain pipe $N = (67.00 / 5.00 + 1) \times 2 + (8.00 / 5.00 + 1) \times 2 \times 2$ = 40 N (ϕ 75mm)

NO@ BOX FOR DRAINAGE

porous drainage $A = \{40.00 \times 3.30 \times 2 + \frac{1}{2} \times 5.50 \times 3.30 \times 2 \times 2\}$ (×0.300) = 300.300 m² perforated pipe $L = 40.00 \times 2 + 5.50 \times 2 \times 2$ = 102.000 m drain pipe $N = (40.00 / 5.00 + 1) \times 2 + (5.50 / 5.00 + 1) \times 2 \times 2$ = 26 N (ϕ 75mm)

NOS BOX FOR DRAINAGE

porous drainage $A = \{132.0 \times 4.30 \times 2 + \frac{1}{2} \times 10.50 \times 4.30 \times 2 \times 2\}$ (×0.300) = 1225.500 m² perforated pipe $L = 132.0 \times 2 + 10.50 \times 2 \times 2$ = 306.000 m drain pipe N = 66 N (ϕ 75mm)

B. Q 21.01 Waterproofing materials

NOO BOX for Drainage

$$A = 59.00 \times 4.00 \times 3 + \frac{1}{2} \times 3.90(9.00 + 12.247) \times 2 = 790.863 \text{ m}^2$$

NO(2) BOX for Drainage

$$A = 34.20(3.80 \times 2 + 2.80) + \frac{1}{2} \times 3.80(5.019 + 6.147) \times 2 = 397.070 \text{ m}^2$$

NO(3) BOX for Drainage

$$A = 67.00(4.30 \times 2 + 4.70) + \frac{1}{2} \times 4.00(7.473 + 9.288) \times 2$$

NO@ BOX for Drainage

$$A = 40.00(4.30 \times 2 + 4.70) + \frac{1}{2} \times 4.00(4.984 + 6.640) \times 2$$
 = 578.496 m²

NO(5) BOX- for drainage

$$A = 75.00(5.90 \times 2 + 10.00) + 28.50(5.50 \times 2 + 9.60) \times 2 + \frac{1}{2} \times 5.00(9.093 + 12.860) \times 2$$
$$= 2918.965 \text{ nf}$$

NO1 BOX FOR FOOTPATH

(B)-(B2)

```
concrete(grade:25)
   bottom slab V=3.80\times0.40\times14.00
                                                                                        =21.280m^3
                    V = 3.80 \times 0.40 \times "
   top
                                                                                        =21.280 //
            wall V=0.40\times3.00\times \times 2
   side
                                                                                        =33.600 //
                    V = (13.60 \times 4.155 - 2.933 \times 4.40 - 3.80 \times 3.80) \times 0.40
   wing
                       =(29.163)\times0.40
                                                                                        =11.665 //
   total
                                                                                        =87.825 //
2) formwork
   form-section A = (3.80^2 - 3.00^2) \times 2 = 5.440 \times 2
                                                                                        =10.880 m² (5.440)
                    A = 3.80(14.00-0.40) \times 2
          side.
                                                                                        =103.360 //
                                                                                                                 (175.374)
         (wing) A = 3.80 \times 0.40 \times 2
                                                                                        =3.040 //
                                                                                                                 = 180.814
   wing side
                       = \times 29.163 \times 2
                                                                                        =58, 326 //.
                       =0.40(1.222+\sqrt{4.40^2+2.933^2})\times 2
   section
                                                                                        =5, 208 "
                    A = 3.00 \times 14.00 \times 2
          side
                                                                                        =84.000 "
   in
          slab
                    A = 3.00 \times 14.00
                                                                                        =42.000 "
   top
   total
                  A =
                                                                                        =306.814 / (301.374)
                                                                                            (B_{\rm I})
                                                                                                         (B<sub>2</sub>)
3) base concrete(grade:15)
   concrete: main V = 0.10 \times 4.0 \times (14.00 - 0.40 + 0.10) + 0.10 \times 4.80 \times 0.40 = 5.673 m<sup>3</sup>
      and wing
                    A = \{2(14.00+0.100)+4(4.80-4.00)+4.00\} \times 0.10
   form:main
                                                                                        =3.540 m
      andwing
4) base (crushierran)
                                                                                        =11.280m<sup>3</sup>
                    V = 4.00 \times 14.10 \times 0.20
5)suport
   top slab V = 3.00(3.00-0.10) \times 14.00
                                                                                        =121.800m
                                                                                                        = 126,786
                                                                                        =4.986 "
              V = 4.40 \times (2.933 - 0.10) \times 0.40
                                                                                        =127.680 //
6) scaffold:main V = 1, 20 \times 3, 80 \times 14, 00 \times 2
                                                                                                         217,056
                                                                                        =89.376m "
                    V = 1.20 \times 3.80 \times 4.90 \times 2 \times 2
7) joint filler A = 3.80^2 - 3.00^2
                                                                                        =5.440 \, \text{m}^2/\text{box}
                                                                                        =13.600 \text{m/box}
8) water stop L=3.40\times4
                                                           excavation(earth)
9) excavation
                                                            V = 0.960/6 {6.80 × 31.00+7.76 × 31.96
                                                                +(6.80+7.760)(31,00+31.960)
                                                                                                              =220.081 \,\mathrm{m}^3/\mathrm{box}
                                                            V = 4.00 \times 28.2 \times 0.30 + 3.80 \times 28.0 \times 0.660 = 104.064 \text{ m}^3/\text{box}
                                                           fill
                                                                                                              =116.017m^3/box
                                                            V = 220.081 - 70.224
                              7.760 X 31960
                                      + -
                                                                    earth
                             3.800 X 28.000
```

6.800 X 31.000

NO2 BOX FOR FOOTPATH

(B) (B2)

```
concrete(grade:25)
    hottom slab
                     V = 3.80 \times 0.40 \times 12.250
                                                                                           =18.620m<sup>3</sup>
            111/3
                      V = 3.80 \times 0.40 \times "
    top
                                                                                           =18.620 //
                      V = 0.40 \times 3.00 \times " \times 2
    side
             wall
                                                                                           =29.400 //
                      V = (13.60 \times 4.155 - 2.933 \times 4.40 - 3.8 \times 3.8) \times 0.40
    wing
                         =(29.163)\times0.40
                                                                                           =11.665 //
    total
                                                                                           =78.305 "
2) formwork
    form: section A = (3.80^2 - 3.00^2) \times 2
                                                                                           (5. 440 °) 10. 880 m
             side A = 3.80(12.250-0.40) \times 2
                                                                                           =90.060 //
                                                                                                                      (162074)
            (wing) A = 3.80 \times 0.40 \times 2
                                                                                           =3.040 "
                                                                                                                      167.514
    wing side
                        = 29.163 \times 2
                                                                                           =58. 326 m²
                        =0.40(1.222+\sqrt{4.40^2+2.933^2})\times 2
    section
                                                                                           =5.208 "
    in side
                      A = 3.00 \times 12.250 \times 2
                                                                                           =73.500 //
                      A = 3.00 \times "
    top slab
                                                                                           =36.750 "
    tota!
                                                                                           =277.764 m (272.324)
                                                                                               (Bi)
                                                                                                            (B<sub>2</sub>)
3) base concrete(grade:15)
    concrete: main V = 0.10 \times 4.00 \times (12.250 - 0.40 + 0.10) + 0.10 \times 4.80 \times 0.40 = 4.972 \text{m}^3
    form : main A = \{2(12, 250+0.10)+4(4, 80-4, 00)+4.00\} \times 0.10
                                                                                           =3.190 m
       and wing
4) base(crusherran)
                      V = 4.00 \times 12.350 \times 0.20
                                                                                           =9.880 \,\mathrm{m}^3
5) suport
             slab V = 3.00(3.00-0.10) \times 12.25
                                                                                            =106.575m<sup>3</sup>
    :top
                                                                                            =4.986 "
             wing V = 4.40 \times (2.933 - 0.10) \times 0.40
6) scaffold:main V = 1.20 \times 3.80 \times 12.25 \times 2
                                                                                            =111,720m<sup>3</sup>
                                                                                                               201,096
                                                                                            =89.376 "
                      V = 1.20 \times 3.80 \times 4.90 \times 2 \times 2
7) joint filler A = 3.80^2 - 3.00^2
                                                                                            =5.440 \,\mathrm{m}^2/\mathrm{box}
                                                                                            =13.600m/box
8) water stop
                       L=3.40\times4
                                                            excavation (earth)
9) excavation
                                                              V = 1.44/6 \{ (6.80 \times 27.50 + 8.240 \times 28.940 ) \}
                                                                                                                =305.837m^3/box
                                                                 +(6, 80+8, 240) (27, 50+28, 940)}
                                                            backfill
                                                            remain
                                                              V = 4.00 \times 24.70 \times 0.30 + 3.80 \times 24.50 \times 1.140 = 98.534 \text{m}^3/\text{box}
                                                            fill
                                                                                                                  =207.303 \,\mathrm{m}^3/\mathrm{box}
                                                              V = 305.837 - 98.534
                       8 240 X 28 940
                                                             earth
```

NO3 BOX FOR FOOTPATH

(B)(B2)

```
1) concrete(grade: 25)
   bottom slab
                     V = 3.80 \times 0.40 \times 12.250
                                                                                            =18.620 m<sup>3</sup>
             slab
                     V=3.80\times0.40\times
                                                                                            =18.620 "
    top
                     V = 0.40 \times 3.00 \times " \times 2
             wall
                                                                                            =29.400 "
   side
                      V = \{13.50 \times 4.20 - 1/2(2.60 \times 3.90 + 3.20 \times 4.80) - 3.80^2\} \times 0.40
    wing
                        = \{29, 510\} \times 0.40
                                                                                            =11.804 //
                                                                                            =78.444 //
    total
   TOTIMOTK
    form: section A = (3.80^2 - 3.00^2) \times 2 = 5440 \times 2
                                                                                            ≈10.880 m² (5.440)
                                                                                                                      (162782)
                      A = 3.80(12.250-0.40) \times 2
                                                                                            =90.060 "
             side
            (wing) A = 3.80 \times 0.40 \times 2
                                                                                            =3.040 "
                                                                                                                      168,222
                        = 29.510 \times 2
                                                                                            =59,020 m²
            side
    :wing
                        =0.40(1.60+1.00+\sqrt{2.60^2+3.90^2}+\sqrt{3.20^2+4.80^2})
    section
                                                                                            =5.222
                                                                                            =73,500 "
                      A = 3.00 \times 12.250 \times 2
             side
             slab
                      A = 3.00 \times "
                                                                                            =36.750 //
    :top
                                                                                             =278.472(273.032)
    total
                                                                                                (B_{i})
                                                                                                           (B<sub>2</sub>)
3) base concrete(grade:15)
    concrete: main V = 0. 10 \times 4. 00 \times (12.250 - 0.40 + 0.10) + 0.10 \times 4. 80 \times 0.40 = 4.972 \times 2 = 9.944 \text{m}^3/\text{box}
            and wing
           :main A = \{2(12.250+0.10)+4(4.80-4.00)+4.00\} \times 0.10
                                                                                            =3. 190 m<sup>2</sup>
    form
           and wing
                                                                                            =9.880 \times 2 = 19.760 \text{ m}^3/\text{box}
                      V = 4.00 \times 12.350 \times 0.20
   base
5) suport
                                                                                             =106.575m<sup>3</sup>
                      V = 3.00(3.00-0.10) \times 12.250
                                                                                                              111.501
    top slab
                      V = 0.40(2.50 \times 3.90 + 3.10 \times 4.80)/2
                                                                                             =4.926 "
    wing
                                                                                             =111.720m<sup>3</sup>
6) scaffold:main V = 1. 20 \times 3.80 \times 12.250 \times 2
                                                                                                              200.184
                                                                                             =88.464m^3
                      V = 1.20 \times 3.80 \times (4.40 + 5.30) \times 2
    :wing
                                                                                             =5.440 \,\text{m}^2/\text{box}
7) joint filler A = 3.80^2 - 3.00^2
                                                                                             =13.600 \text{m/box}
8) water stop L=3.40\times4
                                                             excavation(earth)
9) excavation
                                                              V = 1.070/6 (6.80 \times 27.50 + 7.870 \times 28.570 +
                                                                                                                    =220.133m^3/box
                                                                  (6.80+7.870) (27.50+28.570)}
                                                             backfill
                                                             remain:
                                                              V = 4.00 \times 24.70 \times 0.30 + 3.80 \times 24.50 \times 0.770 = 64.087 \text{m}^3/\text{box}
                                                                                                                    =156.046m<sup>3</sup>/box
                                                               V =220. 133-64, 087
                                   7.870 X 28.570
                                                                         earth
```

3.800 X 24.500

NO4 BOX FOR FOOTPATH

```
Box length=12.350m(mean) skew angle = 76° 54'30"90°
                                                \alpha = 1/\sin 76^{\circ} 54'30'' = 1.0267
 i) concrete(grade:25)
     bottom slab V=3.80\times0.40\times12.350
                                                                                          =18.772 \text{ m}^3
     dot
                       V = 3.80 \times 0.40 \times "
                                                                                          =18,772 "
     side
              wall.
                       V = 0.40 \times 3.00 \times " \times 2
                                                                                          =29.640 //
                       V = \{13, 602 \times 4, 105 - 1/2 (2, 740 \times 4, 110 + 3, 073 \times 4, 610) - 3, 902 \times 3, 80\} \times 0, 40
     wing
                         = \{28, 295\} \times 0.40
     total
                                                                                          =78.502 \,\mathrm{m}^3
 2) formwork
     form: section A = (3.80^2 - 3.00^2)(1+1.0267)
                         =5.440\times( " )
                                                                                          =11.025 \,\mathrm{m}^2(5.585)
                      A = 3.80(12.350-0.40 \times 1.0267) \times 2
   :out
              side
                                                                                          =90.738 //
                                                                                                                  (161, 184)
             (wing) A = 3.80 \times 0.40 \times 1.0267 \times 2
                                                                                          =3.121 //
                                                                                                                  166,624
   :wing: side
                         = 28.295 \times 2
                                                                                          =56.590 m
    section
                         =0. 40(1. 365+1. 032+\sqrt{2.740^2+4.110^2}+\sqrt{3.073^2+4.610^2})=5. 150 "
   :in
              side
                      A = 3.00 \times 12.350 \times 2
                                                                                          =74, 100 //
                      A = 3.00 \times "
              slah
   :top
                                                                                          =37.050 //
    total
                                                                                          =166.624 // (161.184)
3) base concrete(grade:15)
    concrete: main V = 0.10 \times 4.00 \times {12.350-(0.40-0.10) \times 1.0267}
           and wing
                         +0.10\times4.882\times0.40
                                                                                          =5.012 \times 2 = 10.024 \text{ m}^3/\text{box}
    form : main A = \{2(12.350+0.10\times1.0267)+(4.882-4.0\times1.0267)\times4
         andwing
                         +4.0 \times 1.0267 \times 0.10
                                                                                          =3. 211 m
4) base(crucherran)
                    V = 4.00 \times (12.350 + 0.10 \times 1.0267) \times 0.20
                                                                                         =9.962 \times 2 = 19.924 \text{ m}^3/\text{box}
5) suport
    :top slab
                    V = 3.00(3.00-0.10) \times 12.350
                                                                                         =107.445m3
                                                                                                           112530
              V = 0.40(2.740 \times 4.110 + 3.073 \times 4.610)/2
                                                                                          =5.085 //
6) scaffold:main V = 1.20 \times 3.80 \times 12.350 \times 2
                                                                                          =112.632m^3
                                                                                                           201.096
             :wing V = 1. 20 \times 3, 80 \times (4.60 + 5.10) \times 2
                                                                                         =88.464m<sup>3</sup>
7) joint filler A = 3.80^2 - 3.00^2
                                                                                          =5.440 m²/box
8) water stop L=3.40\times4
                                                                                         =13.600m/box
9) excavation.
                                                             excavation(earth)
                                                              V = 1.210/6 \{6.80 \times 27.70 + 8.010 \times 28.910 + (6.80 + 8.010)\}
                                                                  (27.70+28.910)}
                                                                                                               =253.761m<sup>3</sup>/box
                                                            backfill
                                                             remain
                                                               V = 4.00(24.70 \pm 0.20 \times 1.0267) \times 0.30 \pm 3.80 \times 24.70
                                                                  \times 0.810
                                                                                                              =77.755m<sup>3</sup>/box
                                                            fill
                                                               V = 253. 761-77. 755
                                                                                                              =176.006m^3/box
                                 8.010 x 28.910
                                                                     earth
                   1.500
                                                          500
                               3.800 X24700
                              6.800 X27.700
```

BOXCULVERT FOR FOOTPATH-NO (1)

LIST OF REINFORCED BAR --- BI = B2

			T	P				40.00.00
MA	RK	DIAMETER	LENGTH	NUMBAR	UNITWEIGHT	PIECEWEIGHT	WEIGHT	REMARK
					And the second of the second o	**************************************	(************************************)
3	1	Y16	7820	48	1.579	12.35	593	
	2	11	3700	.95	**	5.84	555	
T	3	li ·	2440	47	11	3.85	181	
	4	- 11	2440	47	11	"	.181	····
:	5	Y12	14320	28	0.888	12.72	356	
7	6	н	1090	48	H	0.97	47	
	7		1090	48	H	11	47	
	8	H	1190	113	 H	1.06	120	
-							2080	k ø
							2000	·· Q
1	1	Y12	14320	11	0.888	12.72	140	
	2	11	14320	11	l r	17	140	· .
· ·	3	, , н	14320	11	lt .	11	140	-
· .	4	1)	14320	11	,11	11	140	
· :	5	Y16	3690	48	1.579	5.83	280	
· -	6	. 11	3690	48	· 11	11 :	280	
:	7	Y12	530	115	0.888	0.47	54	
<u> </u>	8	17	530	115	11	11	54	
,,							1228	kg
:	· · · ·	· · · · · · · · · · · · · · · · · · ·						
7	1	Y16	7800	48	1.579	12.32	591	
	2	11	3700	95	11	5.84	555	
	3	n	2430	47	··· · · · · · · · · · · · · · · · · ·	3.84	180	
:	4	1,	2430	47		",	180	
<u> </u>	5	Y12	14320	28	0.888	12.72	356	
	6	11	1080	48	11	0.96	46	
:	7	11	1080	48	¹ 41 ,	11	46	
· ·	8	ъ	1170	113	11	1.04	118	
		· ·				· ·	2072	kg
: 								·
<u> </u>	<u> 1</u>		Y16	<u>.</u>	3576		152 kg	
			Y12		1804		608 "	
			+ 1+ 1		5380	$kg \times 2 = 10$	760 /	

BOXCULVERT FOR FOOTPATH - NO. 0

LIST OF REINFORCED BAR --- WING 1=2

MARK [LAHETER	LENGTH	NUMBAR	UNITWEIGHT	PIECEWEIGHT	KEICHT	REMARK		
1-1				<u>,, </u>					
W 1	Y16	5440	5	1.579	8.59	43			
2	. "	3340	19	11	5.27	100	· · · · · · · · · · · · · · · · · · ·		
3	Y12	4050	2	0.888	3.60	7			
4	"	2650	14	11	2.35	33			
5	n	7230	1	18	6.42	6			
6	Y16_	5440	5	1.579	8.59	43	·		
7	n -	3340	19	: 11 .	5.27	100			
8	Y12	4050	2	0.888	3.60	7			
9	11	2650	14	it.	2.35	33			
10	1)	7230	1	11	6.42	6			
11	11	5440	2	n	4.83	10	<u></u>		
12		3340	10	tt	2.97	30			
13	"	4050	2	11	3.60	7			
14		2650	14	11	2.35	33			
15	11	7230	1	11	6.42	6			
16	11	5440	2	11	4.83	10			
17	1)	3340	10	,,	2.97	30	<u> </u>		
18	n	4050	2	17	3.60	7			
19	n	2650	14		2.35	33			
20	u u	7230	1	"	6.42	6			
21	Y16	1370	12	1.579	2.16	26			
22	Y12	500	12	0.888	0.44	5			
23	Y16	1370	12	1.579	2.16	26	<u> </u>		
24	Y12	500	12	0.888	0.44	5			
25	н	480	16	."	0.43	7			
26	11	480	16	"	" .	7	h		
27	0	1590	13	"	1.41	18			
28	**	14060	2	lt .	12.49	25			
29	1)	14060	2	"	"	25			
30	,,	500	2	,,	0.44	<u> </u>			
30	.,	500	2	n	11	1			
32	Y16	3240	12	1.579	5.12	61			
33	Ħ	3240	12	"	,,	61			
34	Y12	530	14	0.888	0.47	7	<u> </u>		
. 35	11	530	14	. 11 .		7			
						832	kg		
		Y16		100	460 kg x 2 = 920 kg				
		Y12			372 kg x 2 = 744 "				
		<u> </u>		832	$kg \times 2 = 1$	664 /			

BOXCULVERT FOR FOOTPATH-NO @

LIST OF REINFORCED BAR --- BI = B2

WARK	DIAMETER	LENGTH	NUMBAR	UNITWEIGHT	PIECEWEIGHT	MEICHT	REMARK
· .				· · · · · · · · · · · · · · · · · · ·			
3 1	Y12	7850	42	0.888	6.97	293	
2	Y16	3700	41	1.579	5.84	239	
3	Y12	3700	42	0.888	3.29	138	
4	Y16	2440	82	1.579	3.85	316	
5	Y12	12570	28	0.888	11.16	312	
6	"	1090	42	11	0.97	41	
7	11	1090	42	11	l1	41	
8	11	1190	98	11	1.06	104	
			···			1484	kg
W 1	Y12	12570	11	0.888	11.16	123	
2	11	12570	11	11	ti	123	
3	71	12570	11	11	n	123	
4	,,	12570	11	11	n	123	
5	Y16	3690	42	1.579	5.83	245	
6	**	3690	42	11	n	245	
7	Y12	530	100	0.888	0.47	47	
8	12	530	100	11	"	47	
	13/12 11		:		.·	1076	kg
				,			
F 1	Y12	7830	42	0.888	6.95	292	
2	Y16	3700	41	1.579	5.84	239	
3	Y12	3700	42	0.888	3.29	138	
4		2430	82	1.579	3.84	315	
5		12570	28	0.888	11.16	312	
6		1080	42	11	0.96	40	
7	,,	1080	42	11	.91	40	
8	11	1170	98	••	1.04	102	<u> </u>
	1					1478	kg
							*.
		Y16		1599 }	кg		
		Y12		2439 1			
				4038]			

BOXCULVERT FOR FOOTPATH - NO. @

LIST OF REINFORCED BAR --- WING

MARK	DIAMETER	LENGTH	NUMBAR-	UNITWEIGHT	PIECEWEIGHT	TROISY	REMARK
<u> </u>							
W 1	Y16	5440	5	1.579	8.59	43	
2	n	3340	19	11	5.27	100	
3	Y12	4050	2	0.888	3.60	7	
4	/99	2650	14	11	2.35	33	
5	u l	7230	1	11	6.42	. 6	
6	Y16	5440	5	1.579	8.59	43	
7	: n -	3340	19	31	5.27	100	
8	Y12	4050	2	0.888	3.60	7	
9	. 11	2650	14	e1	2.35	33.	
10	u	7230	1	11	6.42	6	
11	••	5440	2	11	4.83	10	
12	11	3340	10))	2.97	30	
13	II	4050	2	11	3.60	7	
14	iii	2650	14	99	2.35	33	
15	. 11	7230	1	. # .	6.42	6	
16	"	5440	2	Ħ	4.83	10	<u> </u>
17	, n	3340	10	0	2.97	30	
18	п -	4050	2	н :	3.60	7.	
19	- 11	2650	14:	27	2.35	33	
20	U.	7230	1	17	6.42	6	
21	Y16	1370	12	1.579	2.16	26	· · · · · · · · · · · · · · · · · · ·
22	Y12	500.	. 12.	0.888	0.44	5	
23	Y16	1370	12	1.579	2.16	26	
2 4	Y12	500	12	0.888	0.44	5	
25	II.	480	16	11	0.43	7	
26	,,	480	16	1)	n	7	
27	4 (*).	.1590	13	11	1 41	18	
28	16	14060	2	1)	12.49	25	
29	11	14060	2_	"	"	25	
30	11	500	2_	. 11.	0.44	1	
30	17	500	2	79	15.	1	
32	Y16	3240	12	1,579	5.12	61	
33	11	3240	12	"	"	61	
34	Y12	53 <u>0</u>	14	0.888	0.47	7	
35		530	14.			7.	
						832	kg
		Y16		460	kg		
		Y12		372	kg		·
				832	kg	·	

BOXCULVERT FOR FOOTPATH-NO 3

LIST OF REINFORCED BAR --- BI = B2

·		-						
MA	RK	DIAMETER	LENGTH	NUMBAR	UNITWEIGHT	PIECEWEIGHT	WEIGHT	REMARK
:				:				
S	1	Y12	7850	42	0.888	6.97	293	
	2	Y16	3700	41	1.579	5.84	239	
	3	Y12	3700	42	0.888	3.29	138	
: 	4	Y16	2440	82	1.579	3.85	, 316	
	5	Y12	12570	.28	0.888	11.16	312	
:	6	- 11	1090	_42	11	0.97	41	
	7	ս	1090	42	11	11	41	
	8	11	1190	98	Ħ	1.06	104	
	* 17					:	1484	kg
W	1	Y12	12570	11	0.888	11.16	123	
	2	11	12570	11	11	93	123	
: '	3	11	12570	11	11	11	123	
	4	11	12570	11	11	31	123	
	5	Y16	3690	42	1.579	5.83	245	
	6	21	3690	42	17	#I	245	
	7	Y12	530	100	0.888	0.47	47	
1	8		530	100	"	. 11.	47	
				<u>. </u>			1076	kg
:								
F	1	Y12	7830	42	0.888	6.95	292	
	2	Y16	3700	41	1.579	5.84	239	
;	3	Y12	3700	42	0.888	3.29	138	
:	4	Y16	2430	82	1.579	3.84	315	
	5	Y12	12570	28	0.888	11.16	312	
	6	tı	1080	42	11	0.96	40	
1	7	11	1080	42	11	ţ1	40	
:	8	11	1170	98	11	1.04	102	
		· · · · · · · · · · · · · · · · · · ·					1478	kg
				:				
:			Y16	:	1599	kg		
:			Y12		2439	kg		<u> </u>
		41			4038	kg		

BOXCULVERT FOR FOOTPATH - NO. 3

	MARK	DIAMETER		NUMBAR		PIECEWEIGHT	ТКОІЗЖ	REMARI
						<u> </u>		
	W 1	Y16	4940	8	1,579	7 90	60	
	2	T	3070	16	"	7.80	78	· -
	3		4090	2	0.888	3.63	7	<u>:</u>
	4	11	2830	12	11	2.51	30	
	5	. 11	7000	1	п .	6.22	6	· .
	6	Y16	5840	4	1.579	9.22	37	
	7	"	3520	20	11	5.56	111	
	8	Y12	4090	2	0.888	3.63	7	
	9	: ::: • H : :	2530	15	11	2.25	34	·
	10	11	7490	1	11	6.65	7	
	11	"	4900	4	*1	4.35	17	
	12	,,	2910	8	11	2.58	21	
	13	"	4090	2	tt.	3.63	7	
	14	"	2830	12	. 11	2.51	30	·
	15	"	7000	1	11	6.22	6	
	16	"	5800	2	11	5.15	10	
	17		3360	10	17	2.98	30	
	18	in I	4090	2	11	3.63	7	
	19	"	2530	15	11	2.25	34	
	20	"	7490	1	. H ·	6.65	7	
	21	Y16	1370	12	1.579	2.16	26	
	22	Y12	500	12	0.888	0.44	5	
	23	Y16	1370	12	1.579	2.16	26	
	24	Y12	500	12	0.888	0.44	5	· ··· ·
	25	"	480	14	11	0.43	6	
	26	11	480	17	ži.		7	
	27	11	1680	14	11	1.49	21	
.	28	**	13820	2	11	12.27	25	
	29	"	13820	2	· n	"	25	,-,
	30	n	500	2	11	0.44	1	- · · · · ·
	31	11	500	2		<u> </u>	1	
	32	Y16	3240	12	1.579	5.12	61	
	33	1)	3240	12	ts.	n 11	61	·
	34	Y12	530	14	0.888	0.47	7	
	35	11	530	- 15	0		7	
ļ		1			· · · · · · · · · · · · · · · · · · ·	·	832 1	(g
			Y16		462 1			······································
			Y12	· · · · · · · · · · · · · · · · · · ·	370 1	(g		

LIST OF REINFORCED BAR --- BI = B2

-					RI = R5		_
MARK	DIAMETER	LENGTH	NUMBAR	UNITWEIGHT	PIECEWEIGHT	WEICHT	REMARK
S 1	Y12	7850	40	0 900	C 0.0		<u> </u>
2		3700	40	0.888	6.97	279	
3		3700	39	1.579	5.84	228	
4		2440	40	0.888	3.29	132	
5		12620	78 28	1.579	3.85	300	
6	"	1090		0.888	11.21	314	
7	,,	1090	40	h	0.97	. 39	
8	17	1190		#)		39	<u> </u>
9		6800	95 3		1.06	101	
10	"	6240	3	1.579	10.74	32	•
11	tı	3800	ა 6	11	9.85	30	
12	Y12				6.00	36	
13		1110 1190	3	0.888	0.99	3	
13	<u> </u>	1190	<u> </u>	<u>ا </u>	1.06	1	
						1534	кg
W 1	Y12	12190	11	0.888	10 00	1,0	
2	112	12190	11	0.888	10.82	119 120	
3	18	12980	11	11			
4	11	13050	11	,,	11.53 11.59	127	
5	Y16	3690	40	1.579	5.83	127 233	
6	"	3690	40	1.579	9,00	233	
7	21	3690	3	. 11	11	17	
8	- 11	3690	1	. 11	72	6	
9	Y12	530	95	0.888	0.47	45	· · · ·
10	112			"	17		
	n	530	95	**	0.48	45 1	·
11	<u> </u>	540	3_	<u> </u>	0.40		le or
7					· · · · · · · · · · · · · · · · · · ·	1073	<u> </u>
F 1	Y12	7830	40	0.888	6.95	278	
2	Y16		39	1.579	5.84	228	
		3700			3.29		
3	Y12	3700	40 78	0.888 1.579	3.84	132 300	
4 5	Y16	2430	28	0.888	11.21	314	
<u> </u>	Y12	12620 1080	40	"	0.96	38	
7	i : ".	1080	40	n n	"	38	
		1.00		11	1.04	99	
8		1170	95		9.84	30	· · · · · · · · · · · · · · · · · · ·
9	Y16	6230	3	1.579	10.72	30	
10	† †	6790	3	"		36	
11		3800	6		6.00		
12	Y12	1090	3	0.888	0.97	3	
13		1170	1		1.04	1520	le or
<u> </u>	· · · · · · · · · · · · · · · · · · ·		··	<u> </u>		1529	кg
	<u></u>	VIA		1741	(P	·	
		Y16		2395		<u> </u>	
		Y12		4136		 	
				4100 1			

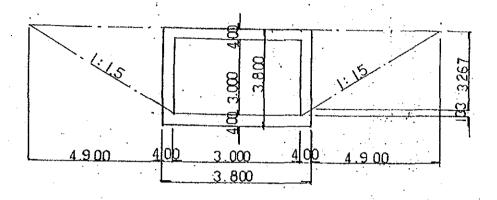
BOXCULVERT FOR FOOTPATH - NO @

LIST OF REINFORCED BAR --- WING 1= 2

MARK !	PLAMETER	LENGTH	NUMBAR	ÜNITWEIGHT	PIECEWEIGHT	TROLER	REMARK
· · · · · · · · · · · · · · · · · · ·				:			
4 1	Y16	5240	6	1.579	8.27	50	
2	"	3160	18	- 11	4.99	90	
3	Y12	4000	. 2	0.888	3.55	7	
4	n	2690	13	11	2.39	31	
5	n	7100	11	11	6.30	. 6	
6	Y16	5570	4	1.579	8.80	35	
7	н	3220	20	11	5.08	102	
8	Y12	4000	2	0.888	3.55	7	
9	11	2560	15	11	2.27	34	
10	n	7210	1	*17	6.40	6	
11	u	5120	3	. 17	4.55	14	·
12	11	2930	10		2.60	26	
13	.,,	4000	2	*1	3.55	7	
14	It .	2690	13	"	2.39	31	
15	tr	7030	1		6.24	6	
16	11	5600	2	n ·	4.97	10	·
17	. 11	3130	11	11	2.78	31	
18	u	4000	1	-11	3.55	4	
19	11	2560	15	i'. W	2.27	34	
20	11	7280	1_	11 -	6.46	6	<u> </u>
21	Y16	1480	11	1.579	2.34	26	
22	Y12	500	12	0.888	0.44	5	····
23	Y16	1300	11	1.579	2.05	23	
24	Y12	500	12	0.888	0.44	5	
25	iı	480	15	. 11	0.43	6	,
26	**	480	16		"	7	
27	**	1490	14	11	1.32	18	
28	. 11	13920	2	11	12.36	25	
29	n	13920	2	11	11	2.5	
30	11	500	2	,,	0.44	1	
31		500	2	11	n n		
32	Y16	3240	11	1.579	5.12	56	
33	***	3240	11	11	"	56	
34	Y12	530	14	0.888	0.47	7	
35	н	- 530	12	0	<u></u>	6	
· · ·	· · · · ·	<u></u>	· 	1 18		804	kg
	· · · ·					·····	
		Y16		438 1			
		Y12		366 1	kg		

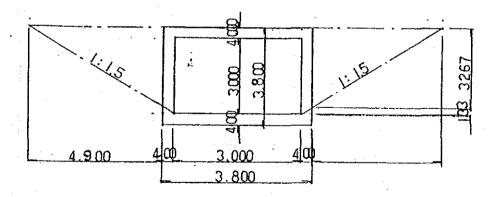
NO @ BOX CULVERT FOR FOOTPATH

 $V = 3.267 \times 4.90 \times (28.00 - 0.40 \times 2) = 435.4 \text{ m}^3$



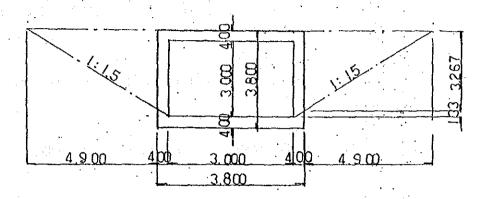
NO @ BOX CULVERT FOR FOOTPATH BACK-FILL

 $V = 3.267 \times 4.90 \times (24.50 - 0.40 \times 2) = 379.4 \text{ m}^3$



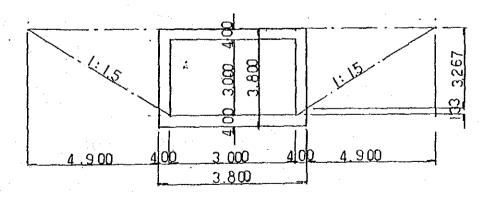
NO 3 BOX CULVERT FOR FOOTPATH BACK-FILL

 $V = 3.267 \times 4.90 \times (24.50 - 0.40 \times 2) = 379.4 \text{ m}^3$



NO @ BOX CULVERT FOR FOOTPATH BACK FILL

 $V = 3.267 \times 4.90 \times (24.70 - 0.411 \times 2) = 382.3^{\text{m}^3}$



BOX FOR FOOTPATH

NO① UF2 A = $(3.80 + 3.00) \times 28.00 + 4.90 \times 0.40 \times 2 \times 2$ = 198.240 m²

NO② "A = $(3.80 + 3.00) \times 24.50 + 4.90 \times 0.40 \times 2 \times 2$ = 174.440 "

NO③ "A = $(3.80 + 3.00) \times 24.50 + (4.40 + 5.30) \times 0.40 \times 2$ = 174.360 "

NO④ "A = $(3.80 + 3.00) \times 24.70 + (4.60 + 5.10) \times 0.40 \times 2$ = 175.720 "

```
NOO BOX FOR FOOTPATH
                                                                                                      = 184,960 \,\mathrm{m}^{2} \, 55.488 \,\mathrm{m}^{3}
 porous drainage
                          A = 37.20 \times 3.40 \times 2 \times (\times 0.300)
perforated pipe
                          L = 27.20 \times 2
                                                                                                      = 54.400 m
drain pipe
                                                                                                      = 2 N
  ( $ 200 mm )
NO@ BOX FOR FOOTPATH
 porous drainage
                          A = 23.7 \times 3.40 \times 2 \times (\times 0.300)
                                                                                                      = 161.160 m<sup>2</sup> 48.348 m<sup>3</sup>
                          L = 23.70 \times 2
                                                                                                      = 47.400 m
 perforated pipe
                                                                                                      = 4 N
 drain pipe
  (\phi 200 \text{mm})
NO(3) BOX FOR FOOTPATH
                                                                                                      = 161.160 m<sup>2</sup> 48.348 m<sup>3</sup>
                          A = 23.7 \times 3.40 \times 2 \times (\times 0.300)
 porous drainage
                        L = 23.70 \times 2
                                                                                                      = 47, 400 in
 perforated pipe
                                                                                                      = 4 N
 drain pipe
  (φ 200 mm)
```

NO@ BOX FOR FOOTI	PATH		
porous drainage	$A = 23.879 \times 3.40 \times 2 \times (\times 0.300)$	•	$= 162.377 \mathrm{m}^2 48.713 \mathrm{m}^3$
perforated pipe	$L = 23.879 \times 2$		= 47.758 m
drain pipe	$N = (23.879/5.00+1) \times 2$	÷	= 2 N

B.Q 21,01 Waterproofing materials

NOO BOX for footpath

 $A = (28.00 - 2 \times 0.40) \times 3.80 \times 3 + (4.90 \times 4.155 - 155)$

NO@ BOX for footpath

= 325.807 m² $\times 2.933 \times 4.40) \times 2 \times 2$ $A = (24.50 - 2 \times 0.40) \times 3.80 \times 3 + (4.90 \times 4.155 - 155)$

NO® BOX for footpath

$$A = (24.50 - 2 \times 0.40) \times 3.80 \times 3 + (4.40 \times 4.20 - \frac{1}{2} \times 3.90 \times 2.60) \times 2$$

$$+ (5.30 \times 4.20 - \frac{1}{1} \times 4.80 \times 3.20) \times 2$$

$$= 326.160 \text{ m}^{2}$$

NO@ BOX for footpath

$$A = (24.70 - 2 \times 0.411) \times 3.80 \times 3 + (4.60 \times 4.105 - \frac{1}{2} \times 4.11 \times 2.74) \times 2$$

$$+ (5.10 \times 4.105 - \frac{1}{10} \times 4.61 \times 3.073) \times 2 = 326.418$$

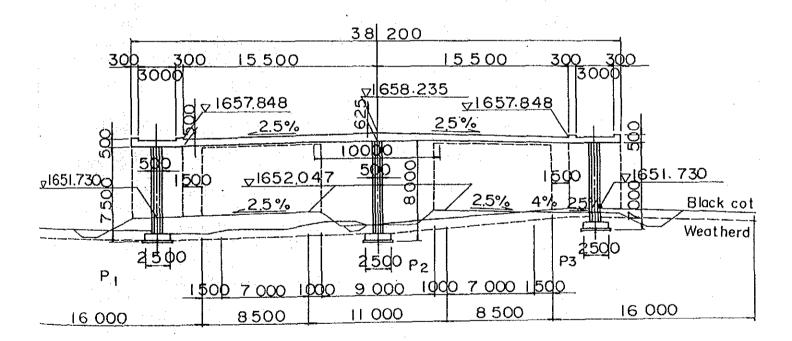
BILL OF QUANTITIES

OVERBRIDGES

PEDESTRIAN	i .	ΒF	R I	DO	GE	(1))		 1
PEDESTRIAN			R I	D (ЭE	(2))		 - 11
VEHICULE B	R	ΙĘ	G	E	(1))	W	=	6m	23
VEHICULE R	D	IT) G	E.	(0)	`	w	_	10m	99 41

NO ① PEDESTRIAN BRIDGE Super structure - Main bridgei) Concrete. (Gread=30) $V = \{0.50 \times 28.20 + (0.50 + 0.625) \times 5.00\} \times 3.60$ $-0.30\times3.00\times0.26\times2$ + $(3.60 \times 0.10 \times 2+37.60 \times 0.10+31.60 \times 0.10) \times 0.30$ $= \{19.725\} \times 3.60+(7.640) \times 0.30-0.468$ 72.834m³ UF_2 : A = 38.20 × 3.60 137. 520 m 2) formwork. side. $A = \times 19.725 \times 2 + (31.60 + 37.60 + 3.30 + 3.60) \times 0.10 \times 2$ $+ 0.10 \times 0.30 \times 6 + 0.30 \times 0.26 \times 2 \times 2$ 55, 162 m $A = 3.60 \times 38.20 - 1.50 \times 0.50 \times 3$ 135.270m³ $V = 1/2(6.90-0.10) + (6.30-0.10) \times 3.60 \times 38.20$ 3)Support 893.880m³ A)Scaffold $V = (3.60+1.00) \times 38.20$ 175.720m3 5)Handrail $= 31.60+37.60+3.60\times 2$

76.400m



```
NO (1) PEDESTRIAN BRIDGE
   Sub structure main pier : P1
 1) concrete: pillar V = \times 0.741 \times 7.00
                                                                                                             5.187m<sup>3</sup>
             : footing V = 2.50 \times 3.00 \times 0.50
                                                                                                             3.750 \, \mathrm{m}^3
  total
                                                                                                             8.937m^3
           UF_2 : V = \%0.741 + 2.50 \times 3.00 - \%0.741
                                                                                                             7.500 m<sup>2</sup>
2) formwork : pillar
                           V = 3.828 \times 7.00
                                                                                                            26.796 m²
   : footing A = 2(2.50+3.00) \times 0.50
                                                                                                             5.000 m²
3) base concrete
                          V = 2.70 \times 3.20 \times 0.10
                                                                                                             0.864m^3
A) base (crusherran)
                           V = 2.70 \times 3.20 \times 0.20
                                                                                                             1.728 m^3
5) scaffold
                           V = 1.20 \{(1.50+2.00)+(0.50+2.00)\} \times 7.00 \times 2
                                                                                                           100.800m<sup>3</sup>
                                                           earth
                                   2.700x3.200
6) excavation
       earth'
                  V = 1.00/6 \{2.70 \times 3.20 + 3.70 \times 4.20 + (2.70 + 3.70) (3.20 + 4.20)\}
                                                                                                            10.073m^3
                  V = 3.750+0.704+1.408+\%0.741 \times 0.20
  remain
                                                                                                             5.260m<sup>3</sup>
  backfill
                  V = 10.073-5.260
                                                                                                             4.813m<sup>3</sup>
```

* where . quantity of unit length for Piller

```
P1 : H = 7.500 \text{ M}

A = 0.50×1.50-(4×0.10<sup>2</sup>-\pi×0.10<sup>2</sup>)

= 0.750-(0.0085)

Q R = \pi × 0.10×2

Q S = 2(0.50+1.50)-8×0.10

= × 0.741 m<sup>2</sup>

= 0.628 m

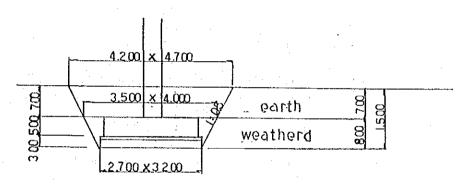
= 3.200 m
```

NOO PEDESTRIAN BRIDGE

Sub structure

6) excavation

main pier) concrete : pillar : footing	: P ₂		5. 557m ³ 3. 750m ³
			0.15011
total	=		9.307m ³
UF ₂ : A=		=	7, 500 m²
2) formwork : pillar	A = 2(0, 212, 22)	=	28.710 m²
	$A = 2(2. 0+3.00) \times 0.50$	=	5. 000 m²
3) base concrete		=	$0.864 m^3$
A) base (Crusherran)	$V = 2.50 \times 3.20 \times 0.20$	×	$1.728m^3$
5) scaffold	$V = 1.20 \{(1.50+2.00)+(0.50+2.00)\} \times 7.50 \times 2$	=	100.800m ³



```
excavation
                  V = 0.70/6 \{3.50 \times 4.00 + 4.20 \times 4.70 + (3.50 + 4.20)(4.00 + 4.70)\}
     earth
                                                                                                                       10.229 m^3
excavation
     weatherd V = 0.80/6 \{2.70 \times 3.20 + 3.50 \times 4.00 + (2.70 + 3.50) (3.20 + 4.00)\}
                                                                                                                        7.530 \, \mathrm{m}^3
                  V = 3.000+0.704+1.408+ \times 0.741 \times 0.70
                                                                                                                        5.630 \,\mathrm{m}^3
backfill :
                  V = 10.229+7.530-5.630
                                                                                                                       12.129m<sup>3</sup>
```

* where . quantity of unit-length for Piller

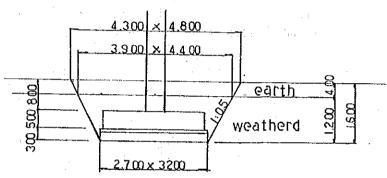
P2 : H = 8,000 M

$$A = 0.50 \times 1.50 - (4 \times 0.10^{2} - \pi \times 0.10^{2})$$

$$= 0.750 - (0.0085)$$

$$Q = 0.628 \text{ m}$$

```
NO @ PEDESTRIAN BRIDGE
  Sub structure main pier : P3
i) concrete: pillar \sim V = \%0.741 \times 6.50
                                                                                                   4.816m<sup>3</sup>
   : footing V = 2.50 \times 3.00 \times 0.50
                                                                                                   3.750m<sup>3</sup>
  total
                                                                                                   8.566m3
         UF2 : A =
                                                                                                   7.5 00 m<sup>2</sup>
2) formwork: pillar, A = 3.828 \times 6.50
                                                                                                  24.882 m²
    : footing A = 2 \times (2.50 + 3.00) \times 0.50
                                                                                                   5.000 m²
3) base concrete V = 2.50 \times 3.20 \times 0.10
                                                                                                   0.864m<sup>3</sup>
A) base (crusherran) V = 2.50 \times 3.20 \times 0.20
                                                                                                   1.728 m^3
                  = 1.20 {(1.50+2.00)+(0.50+2.00)} \times 6.50 \times 2
5) scaffold
                                                                                                  93.600m<sup>3</sup>
6)excavation
```



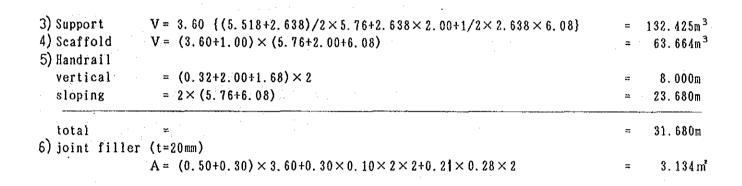
```
excavation earth V = 0.40/6 \{3.90 \times 4.40 + 4.30 \times 4.80 + (3.90 + 4.30) (4.40 + 4.80)\} = 6.629 \text{m}^3 excavation weatherd V = 1.20/6 \{2.70 \times 3.20 + 3.90 \times 4.40 + (2.70 + 3.90) (3.20 + 4.40)\} = 12.912 \text{m}^3 remain V = 3.000 + 0.704 + 1.408 + \% 0.741 \times 0.80 = 5.704 \text{m}^3 backfill V = 6.629 + 12.912 - 5.704 = 13.837 \text{m}^3
```

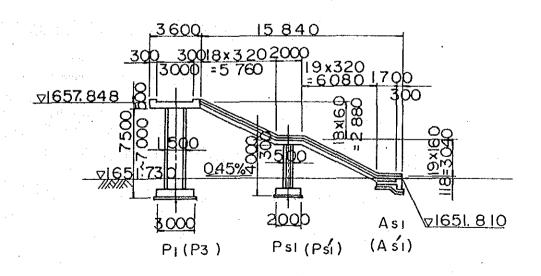
* where a quantity of unit length for Piller

P3 : H = 7.000 M $A = 0.50 \times 1.50 - (4 \times 0.10^2 - \pi \times 0.10^2)$ = 0.750 - (0.0085) $\Omega = \pi \times 0.10 \times 2$ $\Omega = 0.628 \text{ m}$ $\Omega = 2(0.50 + 1.50) - 8 \times 0.10$ = 3.200 m

= X 3.828m

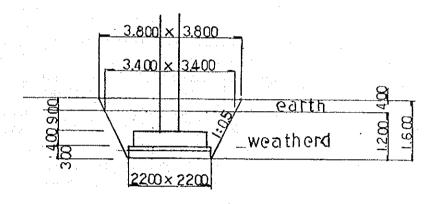
NO ① PEDESTRIAN BRIDGE Super - structure - Stair bridge (1):(2) -1) Concrete (Gread=30) slab : $0.30 \times 3.60(5.76+6.08)+1/2 \times 0.19 \times 0.38 \times 3.60$ + 0. 30 \times 0. 24 \times 3. 0 +0. 30 \times 3. 60(2. 00+1. 68) $17.107m^3$ stair : $1/2 \times 0.32 \times 0.16(18+19) \times 3.00$ 2.841m³ Parapet: $(0.10+0.26)(0.30+0.32\times3)\times0.30$ + 0.10(1.68+1.36) \times 0.30 \times 2+2 \times 0.26(17+18) \times 0.32 \times 0.30 $2.065m^3$ total 22.013m³ UF_2 : A = 3.00(0.28+5.76+2.00+6.08+1.68) $+ 2 \times 0.30(0.30+5.76+2.00+6.08+1.36)$ 56.700 m² 2) form work. Vertical $A = 0.30(2.00+1.68) \times 2+0.10(1.68+1.36) \times 2$ 2.816 m2 horizontal $A = 3.60(0.38+2.00)-0.50\times0.50$ 8.318 m² Sloping $A = (0.10+0.26)(0.30+0.32\times3)+0.26\times0.32(17+18)\times2\times2$ + 0.30 \times (5.76+6.08) \times 2+1/2 \times 0.19 \times 0.38 \times 2 + 3. 60 (5. 76-0. 40+6. 08) \times 1. 1180 65. 321 m[‡]





NO ① PEDESTRIAN BRIDGE Sub-structure

```
Stair pier Ps1 = Ps2
 I) concrete: pillar V = \%0.241 \times 3.60
                                                                                                                        0.867m<sup>3</sup>
                  footing V = 2.00 \times 2.00 \times 0.40
                                                                                                                        1.600m<sup>3</sup>
   total
                                                                                                                        2.467m<sup>3</sup>
      UF<sub>2</sub>: A = \%0.241+2.00 \times 2.00-\%0.241
                                                                                                                        4.000 m²
2) formwork: pillar A = \%1.828 \times 3.60
                                                                                                                       6.580 m²
                footing V = 2(2.00+2.00) \times 0.40
ete V = 2.20 \times 2.20 \times 0.10
                                                                                                                       3. 200 m²
3) base concrete
                                                                                                                       0.484 \, \mathrm{m}^3
4) base (crusherran) V = 2.20 \times 2.20 \times 0.20
                                                                                                                       0.968m^3
                              V = 1, 20(0.50+2.00) \times 3.60 \times 4
5) scaffold
                                                                                                                      43.200m<sup>3</sup>
6) excavation
```

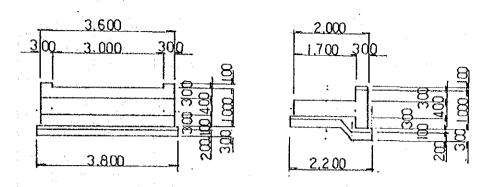


excavation earth	$V = 0.40/6 \{3.40 \times 3.40 + 3.80 \times 3.80 + (3.40 + 3.80)(3.40 + 3.80)\}$	=	5.189m ³
excavation			
weatherd	$V = 1.20/6 \{2.20 \times 2.20 + 3.40 \times 3.40 + (2.20 + 3.40)(2.20 + 3.40)\}$	=	9.552m ³
remain	$V = 1.60+0.484+0.968+ \times 0.241 \times 0.90$	***	3.268m ³
backfill	V = 5.189 + 9.552 - 3.268	=	11.473m ³

* where quantity of unit-length for Pillar

Q =		= × 1.828m
$\&$ S = $4 \times 0.50 - 8 \times 0.10$		= 1.200m
$\Omega = \pi \times 0.10 \times 2$		= 0.628 m
$A = 0.500^2 - 0.0085$		= 🐇 0.241 m²
Ps ₂ : 4.000	•	
Ps ₁ : 4.000		

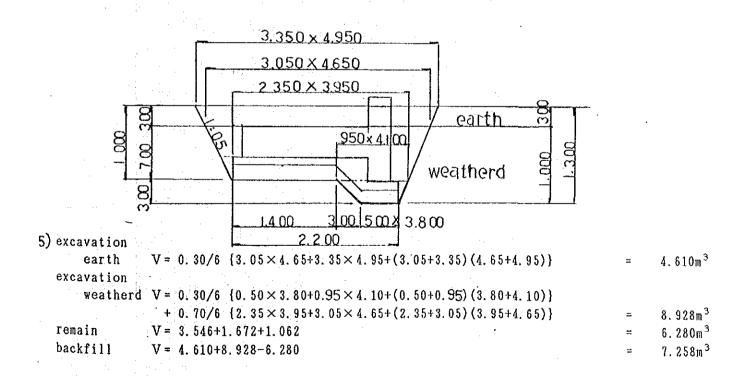
NO ① PEDESTRIAN BRIDGE ASI = AS2 : Edge Dedestal



```
A = 3.60(0.70+0.30)=7.20 \text{ m}^3
                      V = 3.60 \times 0.40 \times 1.70 + (3.60 \times 1.00 + 0.30 \times 0.10 \times 2) \times 0.30
1) concrete
                                                                                                                               3.546m<sup>3</sup>

p) formwork

                       A = 0.40(3.60+1.70\times2)+0.10\times0.30\times4\times2
                         +2\times1.00\times(3.60+0.30)-0.40\times3.60
                                                                                                                               9.400 m<sup>2</sup>
3) base(crusherran) V = 3.80 \times 2.20 \times 0.20
                                                                                                                               1.672m^3
4) base concrete
                       V = \{2.20 \times 0.10 + 1/2(0.10 + 0.40) \times 0.30\} \times 3.60
   concrete
                         = \{0.295\} \times 3.60
                                                                                                                               1.062m3
                      A = ( \% \ 0.295 + 3.80 \times 0.10) \times 2
   formwork
                                                                                                                               1.350 m<sup>2</sup>
```



LIST OF REINFORCED BAR --- SUPERSTRUCTURE

	MARK	DIAMETER					RUCTUR	<u> </u>
	ļ	<u> </u>	<u> </u>	RUMBAR	UNITWEIGHT	PIECEMEIGHT	THOLSK	REMARK
	1	Nol PEDE			· · · · · · · · · · · · · · · · · · ·	· · · · · · · · · · · · · · · · · · ·		
	<u>S</u> 1		8000	31	6.313	50.50	2929	
	2-1		4420	28	3.854	17.03	303	
	2-2		3630	30	n	13,09	578	
	2-3	T	10000	30	n	38.54	370	
	3-1		3680	62	6.313	23.23	462	
	3-2	1	9000	82	*	56.82	321	
	4-1		4500	15	3.854	17.34	· 1031	
	4-2		7250	15	h	27.94	1273	
`	5	 	4420	30	7	17.03	1831	·
	6	1	3450	11	2.468	8.51	696	
	7	1	3450	120	1.579	5.45	925	
	8	 	3450	203	 	5.45	240	
	10	1	3580	22	3.854	13.80	272	
	11		3580	10	1.579	5.65	687	
	12		1070 1240	42	*	1.69	105	
·	13	1		42		1.96	22	- :
	14-1		1050	264	0.888	0.93	1357	
-	14-1	 	10000	12	n .	8.88	71	
	14 - 2	 	9680 6030	8	- "- T	8.60	82	
	16		3500	6	· · · · • · · ·	5.35 3.11	307 24	
	10		3300			3.11		
	So 1	Y12	1310	308	0.888	1.16	452	·
	2	1	1440	162	7	1.18	402	
	K 1		3790	50	2.466	9.35	13161	
	2		6210	28_	0.888	5.51	143	
	3	la .	7960	28		7.07	184	
	4	1	5900	50	1.579	9.32	466	
	5		3790	50	0.888	3.37	189	
	6		7960	50	1.579	12.57	629	
	7	'n	3480	50		5.49	275	
	8	1	3480	238	0.888	3.09	735	
·	9		940	42	1.579	1.48	62	
	10	, n	850	42	*	1.34	56	ļ
		1				1		
	11	Y12	1080	42	0.888	0.96	40	
	12	Y12	860	212		0.76	161	
	12 13	712 "	860 6210	212 8	ri Fi	0.76 5.51	161 44	
	12 13	712 "	860 6210 3790	212 8 8	# #	0.76 5.51 3.37	161 44 27	
	12 13	712 "	860 6210	212 8	ri Fi	0.76 5.51	161 44	
	12 13 14 15	Y12	860 6210 3790 7960	8 8 8	n n n	0.76 5.51 3.37 7.07	161 44 27 57	
	12 13	Y12	860 6210 3790	212 8 8	# #	0.76 5.51 3.37	161 44 27	
	12 13 14 15 Ko 1	Y12	860 6210 3790 7960	212 8 8 8 8	0.888	0.76 5.51 3.37 7.07	161 44 27 57 232	
	12 13 14 15 Ko 1	Y12 " " " " Y12	860 6210 3790 7960 940	212 8 8 8 8 280	0.888	0.76 5.51 3.37 7.07 0.82	161 44 27 57 232	
	12 13 14 15 Ko 1	Y12 " " " " Y12	860 6210 3790 7960	212 8 8 8 8	0.888	0.76 5.51 3.37 7.07	161 44 27 57 232 414 229	
	12 13 14 15 Ko 1	Y12 " " " " Y12	860 6210 3790 7960 940	212 8 8 8 8 280	0.888	0.76 5.51 3.37 7.07 0.82	161 44 27 57 232	kg
	12 13 14 15 Ko 1	Y12 " " " " Y12	860 6210 3790 7960 940	212 8 8 8 280 518 74	0.888	0.76 5.51 3.37 7.07 0.82	161 44 27 57 232 414 229	ks
	12 13 14 15 Ko 1	Y12 " " " " Y12	860 6210 3790 7960 940 900 3480	212 8 8 8 280 518 74	0.888	0.76 5.51 3.37 7.07 0.82 0.80 3.09	161 44 27 57 232 414 229	kg
	12 13 14 15 Ko 1	Y12 " " " " Y12	860 6210 3790 7960 940 900 3480	212 8 8 8 280 518 74	0.888	0.76 5.51 3.37 7.07 0.82 0.80 3.09	161 44 27 57 232 414 229	kg
	12 13 14 15 Ko 1	Y12 " " " " Y12	860 6210 3790 7960 940 900 3480	212 8 8 8 280 518 74	0.888	0.76 5.51 3.37 7.07 0.82 0.80 3.09	161 44 27 57 232 414 229	ks
	12 13 14 15 Ko 1	Y12 " " " " Y12	860 6210 3790 7960 940 900 3480 732 725	212 8 8 8 280 518 74	0.888	0.76 5.51 3.37 7.07 0.82 0.80 3.09	161 44 27 57 232 414 229	kg

LIST OF REINFORCED BAR --- NO OPEDESTRIAN -PIER

MARK	DIANETER	LENGTH	NUMBAR	ТИБІЗЖТІЙИ	PIECEWEIGHT	METCHL	REMARK
. (3) H	≖ 7.500 [™]	7.000			L	
C 1	Y25	9500	26	3.854	36.61	952	
2	Y16	2360	94	1.579	3.73	351	
3	Y12	1640	12	0.888	1.46	18	
			:		· · · · · · · · · · · · · · · · · · ·	1321	kg
F 1	Y20	3600	18	2.466	8.88	160	
2	.,	3070	. 21	£1	7.57	159	
3	Y12	3600	18	0.888	3.20	58	
4	11	3070	21	19	2.73	57	
5	"	1280	28	tı .	1.14	32	
6	11	1400	4	11	1.24	5	
10.00	·					471 1	(g
·		-1-10	·		· · · · · · · · · · · · · · · · · · ·		**
		Y25		952 }	g		<u> </u>
		Y20		319 }	g		
		Y16		351 }	(g		
		Y12		170 1	(g		· - · · · · · · · · · · · · · · · · · ·
1 1				1792 }	(g		
<u> </u>		·	•				

LIST OF REINFORCED BAR---NO PEDESTRIAN-PIER

					1			
MA	RK	DIAMETER	LENGTH	NUMBAR	UNITWEIGHT	PIECEWEIGHT	AEICHL	REMARK
	E	2)	H=8.000 ^[f]					
c	1	Y25	10000	26	3.854	38.54	1002	
	2	Y16	2360	100	1.579	3.73	373	
	_3	Y12	1640	12	0.888	1.46	18	
			·				1393	kg
					r	i		
F	_1	Y20	3600	18	2.466	8.88	160	
1	2	1 11	3070	21	п	7.57	. 159	
	_3	Y12	3600	18	0.888	3.20	58	
34	4	11	3070	21	u .	2.73	57	· .
<u>:</u> .	5	11	1280	28	11	1.14	32	
	6		1400	4	"	1.24	5	-
<u>:</u>		4					471	kg
<u>:</u>								
			Y25		1002 }	(g		· · · · · · · · · · · · · · · · · · ·
:			Y20		319 1	(g		
<u> </u>			Y16		373 F	(g		
			Y12		170 F	rg		•••••
<u> </u>				· ·	1864 J	rg .		

LIST OF REINFORCED BAR---NO OPEDESTRIAN-PIER

MARK	01	ANETER	LENGTH	NUMBYS	UNITWEIGHT	PIECEWEIGHT	MEICHT	REMARK
·	Θ	H	≈ 7,000 M			·		
<u>c</u>	1	Y25	9000	26	3.854	34.69	902	
	2	Y16	2360	86	1.579	3.73	321	
	3	Y12	1640	12	0.888	1.46	18	
							1241	kg
F	ı	Y20	3600	18	2.466	8.88	160	
	2		3070	21	91	7.57	159	
		Y12	3600	18	0.888	3.20	58	
	4	"	3070	21	**	2.73	57	
	5		1280	28	n	1.14	32	
	6	.	1400	4	11	1.24	5	
		<u>-</u>					471	kg
·	·							
			¥25		902 1	8		
			Y20	······································	319 }			
			Y16	<u> </u>	321)			
		·	Y12		170 k			
<u>.</u>					1712 k			
				•				

(STAIRCASE)
LIST OF REINFORCED BAR---NO (STAIRCASE)

X/	RK	DIANETER	LENGTH	HUMBAR	UNITWEIGHT	PIECEWEIGHT	MEIGHT	REMARK
0)=(Ŋ	H= 4.000 ^m			·		<u> </u>
c'	1	Y25	4900	12	3.854	18.88	227	1
	2	Y12	2000	16	0.888	1.78	2.8	<u> </u>
							255	kg
				<u> </u>				:
F	1	Y12	2380	60	0.888	2.11	127	
	2		1090	12	,11	0.97	12	<u> </u>
	1+						139	kg
			Y25	· · · · · · · · · · · · · · · · · · ·	227 1	ιg		
			Y12		167 1	⟨g		
				:	394 1	⟨g		
	•							

MARK	DIAMETER	LENGTH	NUMBAR	UNITWEIGHT	PIECEWEIGHT	WEIGHT	REMARK
(D=(A)						
r 1	Y12	1010	18	0.888	3.59	65	
2	111	2440	30		2.17	65	
3		1940	11		1.72	19	
. 4		2140	4	11	1.90	8	
5	,,,	540	15	11	0.48	7	
6	19	1880	4		1.67	7	
7	"	1090	15	, y	0.97	15	
						186	(g
		Y12		186	<u> </u>	·	
				186 1	κg		:

NO @ PEDESTRIAN BRIDGE

Super structure -- Main bridge-

|) concrete (Gread=30)

 $V = (0.50 \times 48.35) \times 3.60 - 0.30 \times 0.26 \times 3.00 \times 2$

+ $(3.60 \times 0.10 + 48.05 \times 0.10 + 44.75 \times 0.10) \times 0.30$

= (24.175) \times 3.60-0.468+(9.640) \times 0.30

 UF_2 : A = 48.35 × 3.60

= 89.454m³ = 174.060 m³

2) formwork

side : $A = \frac{24.175 \times 2+ \{(3.30+3.60)/2+48.05+44.75\} \times 0.10 \times 2\}}{100}$

+ 0. 50×3 . 60+0. 10×0 . $30 \times 5+0$. 30×0 . $26 \times 2 \times 2$

= 69,862 m²

top slab: $A = 3.60 \times 48.35 - 1.50 \times 0.50 \times 4$

171.060m³

3) support $V = 1/3 \{(6.00-0.10)+(6.30-0.10)+(6.30-0.10)\} \times 3.60 \times 48.35$

 $= 1061.766 m^3$

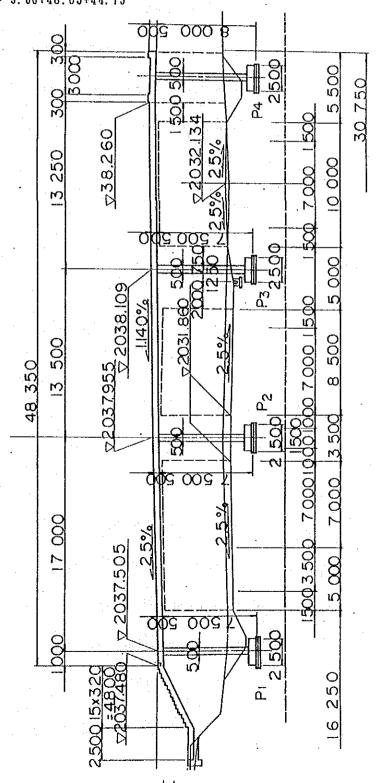
A) scaffold $V = (3.60+1.00) \times 48.35$

222,410m³

5) Handrail

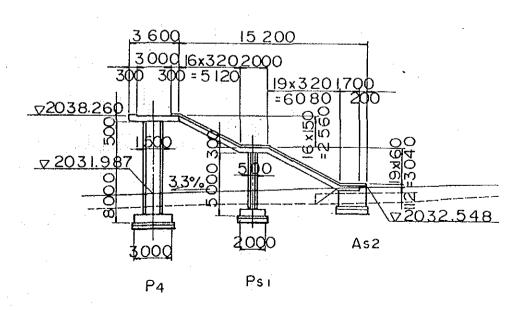
= 3.60+48.05+44.75

= 96.400m

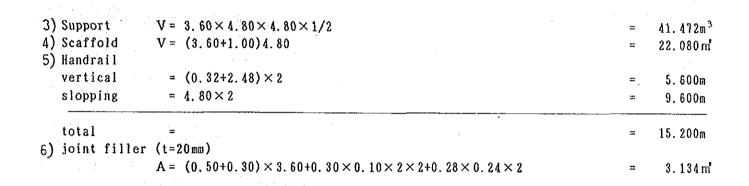


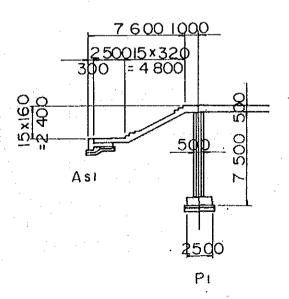
NO @ PEDESTRIAN BRIDGE Super structure -Stair bridge ()-1) Concrete, (Grade=30) $V = 0.30 \times 3.60(5.12+6.08)+1/2 \times 0.19 \times 0.38 \times 3.60$ + 0. $30 \times 0.24 \times 3.0 + 0.30 \times 3.60(2.00 + 1.68)$ 16.416m³ stair $V = 1/2 \times 0.32 \times 0.16(16+19) \times 3.00$ 2.688m³ parapet $V = (0.10+0.26)(0.30+0.32\times3)\times0.30$ + 0.10(1.68+1.36) \times 0.30 \times 2+0.26(15+18) \times 0.32 \times 0.30 $1.965m^3$ total 21.069m3 UF_2 : A = 3.00(0.28+5.12+2.00+6.08+1.68) $+ 2 \times 0.30 (0.30 + 5.12 + 2.00 + 6.08 + 1.36)$ 54.396 m3 2) form work . Vartical. $A = 0.30(2.00+1.68) \times 2+0.10(1.68+1.36) \times 2$ 2.816 m² horizontal $A = 3.60 (0.38+2.00)-0.50\times0.50$ 8.318 m Slopping $A = (0.10+0.26)(0.30+0.32\times3)+0.26\times0.32(15+18)\times2\times2$ + 0.30(5.12+6.08) \times 2+1/2 \times 0.19 \times 0.38 \times 2 + 3. 60 (5. 12-0. 40+6. 08) \times 1. 1180 61.696 m²

3) Support4) Scaffold5) Handrail	$V = 3.60 \{ (5.20+2.64)/2 \times 5.12+2.64 \times 2.00+1/2 \times 2.64 \times 6.08 \}$ V = (3.60+1.00)(5.12+2.00+6.08)	$= 120.153 m^3$ $= 60.720 m^2$
vertical slopping	= $(0.32+2.00+1.68) \times 2$ = $2(5.12+6.08)$	= 8.000m = 22.400m
total 6) joint fille	er (t=20mm)	= 30.400m
	A = $(0.50+0.30) \times 3.60+0.30 \times 0.10 \times 2 \times 2+0.28 \times 0.24 \times 2$	= . 3. 134 m ²



NO@ PEDESTRIAN BRIDGE Super structure - Stair bridge (2)-1) Concrete (Grade=30) $V = 0.30 \times 3.60 \times 4.80 + 1/2 \times 0.19 \times 0.38 \times 3.60$ + 0.28 \times 0.24 \times 3.00+0.30 \times 3.60 \times 2.48 $8.193 \, \mathrm{m}^3$ stair 1 V = 1/2×0.32×0.16×15×3.00 $1.152m^3$ parapet $V = (0.10+0.26)(0.30+0.32) \times 0.30$ + $0.10 \times 2.16 \times 0.30 \times 2 + 2 \times 14 \times 0.26 \times 0.32 \times 0.30$ 0.895m³ total 10.240m³ UF₂: $A = 3.00(0.28+4.80+2.48)+2\times0.30\times(0.30+4.80+2.16)$ 27. 036 m 2) form work . Vartical $A = 0.30 \times 2.48 \times 2 + 0.10 \times 2.16 \times 2$ 1.920 m horizontal A = 3.60(0.38+0.80)4.248 m² Slopping $A = (0.10+0.26)(0.30+0.32)+0.26\times0.32\times14\times2\times2$ + $0.30 \times 4.80 \times 2+1/2 \times 0.19 \times 0.38 \times 2+3.60(4.80-0.40) \times 1.1180$ 25.543 m



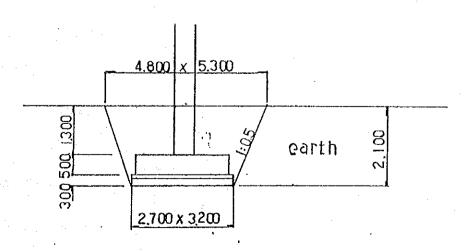


Sub-structure Pier : Pi concrete : pillar	STRIAN BRIDGE Main bridge — = P ₂ = P ₃ (P ₄) V=\(\times 0.741 \times 7.00 (7.50) \)	=5.187 (5.557)	m ^{·3}
	$V = 2.50 \times 3.00 \times 0.50$	=3.750	e m
total = =		= 8.937 (9.307)	. m 3
UF ₂ : $A = \%$ 2) formwork : piller	$A = 3.828 \times 7.00 (750)$	= 7.500 = 26.796 (28.710)	m [‡] m²
footing 3) base concrete 4) " (curusherran)	A = $2(2.50+3.00) \times 0.50$ V = $2.50 \times 3.20 \times 0.10$ V = $2.50 \times 3.20 \times 0.20$ V = $1.20 \{(1.50+2.00)+(0.50+2.00)\} \times 7.00 \times 2$	= 5.000 = 0.864 =_1.728 =100.800 (108.000)	տ ³ տ [*] տ ³

* where : quantity of unit length for Piller

P123. (P4) H = 7.500 (8.000)

	$A = 0.50 \times 1.50 - (4 \times 0.10^{2} - \pi \times 0.10^{2})$ $= 0.750 - (0.0085)$ $Q R = \pi \times 0.10 \times 2$ $Q S = 2(0.50 + 1.50) - 8 \times 0.10$	=	* 0.741 m² 0.628 m 3.200 m
en e	Q =	=	X 3.828 m
6) excavation			
earth	$V = 2.10/6 \{2.70 \times 3.20 + 4.80 \times 5.30 + (2.70 + 4.80) (3.20 + 5.30)\}$	**	29.778m³
remain	$V = 3.750+0.704+1.408+\%0.741 \times 1.300$	=	6.075m ³
backfill	V = 29.778 - 6.075	=	23.703m ³

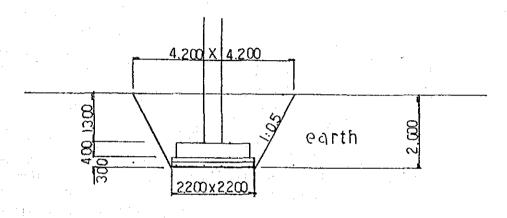


NO @ PEDESTRIAN BRIDGE

Sub - structure : stair - bridge

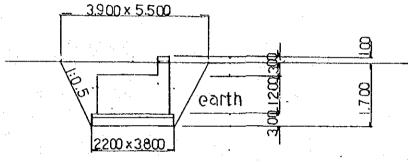
Pier : Ps₁

) concrete: pillar $V = \%0.241 \times 4.60$ footing $V = 2.00 \times 2.00 \times 0.40$	# #	1.108m ³ 1.600m ³
total =	=	2,708m ³
UF_2 : A = $\%$ 0, 241+2, 00 × 2, 00-0, 241	=	4.000 m²
2) form work piller $A = \%1.828 \times 4.60$	n	8. 408 m²
footing $A = 2(2.00+2.00) \times 0.40$. =	
3) base concrete $V = 2.20 \times 2.20 \times 0.10$.	0.484m ³
4) " (curusherran) $V = 2.20 \times 2.20 \times 0.20$	=	0.908 m²
5) Scaffold $V = 1.20 \times (0.50 + 2.00) \times 4.60 \times 4$	=	55, 200m ³



6) excavation			
earth	$V = 2.00/6 \{2.20^2 \times 4.20^2 + (2.20 + 4.20^2)\}$	=	21.146m ³
remain	$V = 1.600+0.484+0.968+0.241 \times 1.30$	=	3.365m ³
backfill	V = 21.146 - 3.365	=	17.781m ³
			- •

```
NO @ PEDESTRIAN BRIDGE
   stair - bridge ()
   Ast: Abutment -- Edge pedestal
(Gread=25)
                V = 3.60 \times 1.50 \times 2.00 + 0.30 + 1.10 \times 0.30 \times 2.00
                   - 3.60×0.30×1.70-\pi/4×0.750<sup>2</sup>×2.00
                                                                                                           8.098m^3
       UF_2: A = 3.60(1.70+0.30)
                                                                                                           7. 200 m²
2) formwork
              A = 1.20(1.70 \times 2 + 3.60) + 0.10 \times 0.30 \times 4 \times 2
                   + 2.0(0.30+3.60) × 0.30-\pi /4×0.75<sup>2</sup>×2
                                                                                                          10.096 m2
                           V = 3.80 \times 2.20 \times 0.10
                                                                                                           0.836m<sup>3</sup>
                           A = 0.10(3.80+2.20) \times 2
                                                                                                           1. 200 m<sup>2</sup>
4) base (crusherran)
                           V = 3.80 \times 2.20 \times 0.20
                                                                                                           1.672m3
```



5) excavation

Carth V = 1.70/6 {2.20 × 3.80+3.90 × 5.50+(2.20+3.90)(3.80+5.50)} = 24.519m³

remain V = 0.836+1.672+3.60(1.20 × 2.00+0.30 × 0.20) = 11.364m³

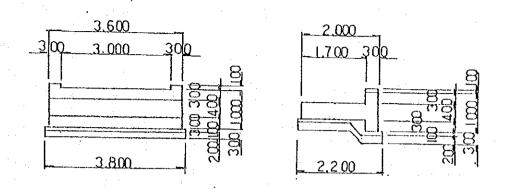
backfill V = 24.519-11.364 = 13.155m³

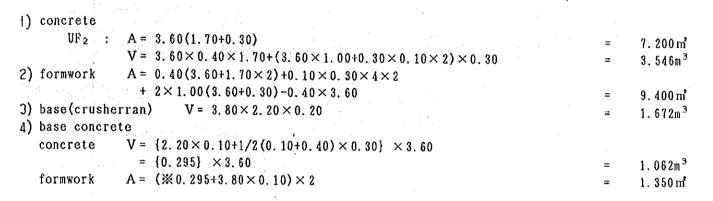
Pipe - pu \$\phi\$ 600 × 2.000m

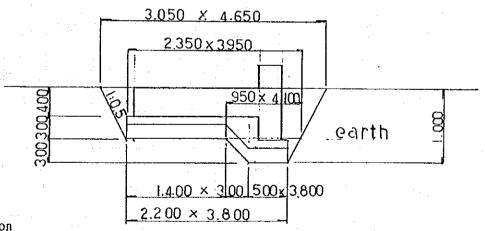
NO @ PEDESTRIAN BRIDGE

stair bridge 2

Ast: Abutment - Edge pedestal







5) excavation

Qarth $V = 0.30/6 \{0.500 \times 3.80 + 0. \times 4.10 + (0.50 + 0.)(3.80 + 4.10)\}$ + 0.70/6 $\{2.35 \times 3.95 + 3.05 \times 4.65 + (2.35 + 3.05)(3.95 + 4.65)\}$ = 8.928m³

remain $V = 1.062 + 1.672 + 3.60(1.70 \times 0.40 + 0.30 \times 0.30)$ = 5.506m³

backfill V = 8.928 - 5.506 = 3.422m³

LIST OF REINFORCED BAR --- SUPERSTRUCTURE

MARK	DIAMETER	A STATE OF THE PERSON NAMED IN	-		PIECEMEIGHT	WEIGHT	REMAR
	No 2 - PEI	DESTRIAN				W- market and a second	
S 1	Y32	8000	58	6.313	F0		1
2	1	5250	15	3.854	50.50	2929	
3-1	9	10000	15	n n	20.23	303	ļ
3-2	17	6400	15	n	38.54	578	
4-1		8000	15	'n	24.67	370	
4-2	1	5550	15	,,	30.83 21.39	462	
5-1	Y32	10000	29	6.313	63.13	321 1031	
5-2	1	6950	29	. "	43.88	1273	
6-1	17	10000	29	я	63.13	1831	
6-2	,,	3800	29	n	23.99	696	
7		8000	30	3.854	30.83	925	
8		5250	29	1.579	8.29	240	ļ.———
9		3450	32	2.466	8.51	272	 -
10		3450	126	1.579	5.45	687	
11		3540	12	2.466	8.73	105	
12		3540	4	1.579	5.59	22	
13	Y16	3450	249	7.078			
14	"	1070	42	17	5.45	1357	
15	n	1240	42	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	1.69	71	
16					1.96	82	ļ
17		1050	330	0.888	0.93	307	
17	Y16	1000	15	1.579	1.58	24	
 	V10	1910	200	0.000	1 10	150	
So 1	Y12	1310	390	0.888	1.16	452	L
	 					14338	кд
					······································		
Н 1	Y12	900	238	0.888	0.80	190	<u> </u>
<u>Н 1</u> 2	1177	3480	35	9.000	3.09	108	<u> </u>
	نـــــا	3400			3.00	2143	l
						4170	<u> </u>
	 ;			<u></u>		······································	. <u>. </u>
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the Design

LIST OF REINFORCED BAR --- SUPERSTRUCTURE

	RK	DIAMETER	LENGTH	NUMBAR	UNITWEIGHT	PIECEWEIGHT	WEIGHT	REMARK
		No2-PED	ESTRIAN					<u> </u>
K	1	Y 20	4240	25	2.466	10.46	262	
	2	Y12	5270	13	0.888	4.68	61	
	3	"	7740	13	н	6.87	89	
	4	Y16	4960	25	1.579	7.83	196	
	5	Y12	4240	25	0.888	3.77	94	<u> </u>
	6	Y16	7740	25	1.579	12.22	306	
	7	Y 25	3480	9	3.854	13.41	121	
	8	Y16	3480	9	1.579	5.49	49	
	9	Y12	3480	122	0.888	3.09	377	
	10	Y16	940	21	1.579	1.48	31	
	11	"	850	21	17	1.34	28	
	12	Y12	1080	21	0.888	0.96	20	
	13	**	860	102	77	0.76	78	
	14	ъ	1060	21	11	0.94	20	·
Ko	1	11	940	136	n	0.83	113	<u> </u>
_								
K	21	Y12	2440	23	0.888	2.17	50	
	22	n	6020	23	"	5.35	123	
	23	Y20	2440	25	2.466	6.02	151	
	24	71	5970	25	n	14.72	368	
	25	Y12	3480	32	0.888	3.09	99	· · · · ·
	26	27	3480	49	п	3.09	151	
	27	Y16	940	21	1.579	1.48	31	
	28	35	850	21	,,	1.34	28	
	29	Y12	1080	21	0.888	0.96	20	:
	30	n	860	50	77	0.76	38	
	31	12	1060	21	ĸ	0.94	20	
				· · ·				
Ko	2	Y12	940	60	0.888	0.83	50	
	:	·						
ĸ	21	Y12	900	98	0.888	0,80	78	
	22	,,	3480					
	44	l 1	3400	15	"	3,09	46	l

LIST OF REINFORCED BAR---NO @ PEDESTRIAN - PIER

	DIAMETER	LENGTH	NUMBAR	UNITWEIGHT	PIECEWEIGHT	WEIGHT	REMARK
DEX	3)	H= 7.500 ^M			L	·	
C 1	Y25	9500	26	3.854	36.61	952	*
2	Y16	2360	94	1.579	3.73	351	
3	Y12	1640	12	0.888	1.46	18	
	:					1321 1	g
	: '' .						
<u>F 1</u>	Y20	3600	18	2.466	8.88	160	
2	"	3070	21	11	7.57	159	
3	Y12	3600	18	0.888	3.20	58	
4	"	3070	21	11	2.73	57	
5	"	1280	28	. "	1.14	32	. <u></u>
6		1400	4	»	1.24	5	
						471 }	g
· · · · · · · · · · · · · · · · · · ·		·				· · · · · · · · · · · · · · · · · · ·	
		Y25		952 }	(g		
· ·		Y20		319 1	rg		
·		Y16	<u>-</u> -	351 1	(g		
	<u> </u>	Y12		170 }	ςg .		
			: -	1792 1	(g		

LIST OF REINFORCED BAR --- NO @PEDESTRIAN - PIER

MARK	DIAMETER	LENGTH	NUMBAR	UNITWEIGHT	PIECEWEIGHT	WEIGHT	REWARK
(<u>В</u>) <u>Н</u>	* 8.000 M	· · · · · · · · · · · · · · · · · · ·	·		······································	
Ç 1	Y25	10000	26	3.854	38.54	1002	
	Y16	2360	100	1.579	3.73	373	
	3 Y12	1640	12	0.888	1.46	18	
		···· , ··· , ··· , · , · , · , · , · ,				1393	kg
			· · · · · · · · · · · · · · · · · · ·	· · · · · · · · · · · · · · · · · · ·			~
F 1	Y20	3600	18	2 466	8,88	160	
		3070	21	,"	7.57	159	
	Y12	3600	18	0.888	3.20	58	
	"	3070	21	"	2.73	57	
	<u>, " '</u>	1280	28_	;n	1.14	32	
	<u> </u>	1400	4	.,	1.24	5	
5					·	471	kg
~~~~~						· · · · · · · · · · · · · · · · · · ·	
		Y25		1002 }	(g		
		Y20		319 }	(g ·		
		Y16		373 }	(g	· · · · · · · · · · · · · · · · · · ·	
	Line Heli	Y12		170 1	к <u>е</u>		
				1864 i	(g		

(STAIRCASE)

ЖА	RK	DIAMETER	LENGTH	NUMBAR	UNITWEIGHT	PIECEWEIGHT	MEICHL	. REMARK
	E	D	H= 5.000 ^M		A-11-4-11-11-11-11-11-11-11-11-11-11-11-1			<u></u>
C_	1	Y25	5900	12	3.854	22.74	273	
	2	Y12	2000	19	0.888	1.78	34	
			· · · · · · · · · · · · · · · · · · ·			·	307	kg
		·		<del></del>	, , , , , , , , , , , , , , , , , , , ,			
F	_1	Y12	2380	60	0.888	2.11	127	
	2	"	1090	12		0.97	12	
· ·							139	kg
	<u> </u>					· : "		
			Y25	· · · · · · · · · · · · · · · · · · ·	273	kg		
			Y12		. 173	kg	· · · · · · · · · · · · · · · · · · ·	<u></u>
		· ·			446	kg		

(STAIRCASE)
LIST OF REINFORCED BAR---NO@PEDESTRIAN-AI

MARK	DIAMETER	LENGTH	RABKUK	UNITWEIGHT	PIECEWEIGHT	WEIGHT	REMARK
. (	<u>41)</u>						
F 1	Y12	4040	18	0.888	3.59	65	
2	"	2440	30	11	2.17	65	
3		1940	11	n	1.72	19	
		2140	4	,,	1.90	8	
		540	15	n	0.48	7	
(		1880	4	n	1.67	7	
7	7 11	1090	15	pi	0.97	15	
						186	kg
		Y12		186	kg		
				186	kg		
			-				

(STAIRCASE)
LIST OF REINFORCED BAR --- NO. PEDESTRIAN - A2

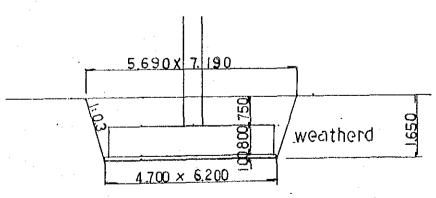
r.,	MARK	DIAMETER	LENGTH	NUMBAR	UNITWEIGHT	PIECEWEIGHT	WEIGHT	REMARK
	A	2)				<u> </u>		
	F 1	Y12	2240	12	0.888	1.99	24	
	2		4040	12	11	3.59	43	
	3		2150	3	11	1.91	6	
	4	11	2110	3	71	1.87	6	
	5	11	3840	9	11	3.41	31	· <del></del>
	6	• • • • • • • • • • • • • • • • • • • •	5640	9	et .	5.01	45	
	7		540	4	11	0.48	2	
	8	+1	980	3		0.87	3	
	9	19	1680	12	**	1.49	18	
	10	13	180	4	11,	0.16	1	
	11	11	3480	2	71	3.09	6	
	12	11	760	6	H.	0.67	4	
	13	11	2310	6	11	2.05	12	· · · · · · · · · · · · · · · · · · ·
	14	11	2240	6	n,	1.99	12	
	15	••	1000	8_	11	0.89	7	
	16	<u> </u>	3150	2	"	2.80	6	
	<del></del>	· · · · · · · · · · · · · · · · · · ·		· · · · · · · · · · · · · · · · · · ·	· .	<u> </u>	226 k	g
			Y12		226 k	ig		
					226 k			

```
NO ① VEHICLE BRIDGE
   Super structure
1) Concrete (Grade=30)
   Girder slab and parapet : V=¾3.225×30.05
                                                                                                                   96.911m<sup>3</sup>
   cross girder = 1.60 \times 0.65(0.60 \times 3+0.35 \times 2) \times 2
                                                                                                                    5, 200m<sup>3</sup>
   total
                                                                                                                  102.111m<sup>3</sup>
       Of UF<sub>2</sub>: A = 30.05 \times (6.00 + 0.60 \times 2)
                                                                                                                  216.360 m²
2) formwork
   horizontal
                     A = 30.05(\sqrt{0.15^2+1.10^2 \times 2+0.60 \times 3+1.60 \times 2})
                                                                                                                 216.961 m2
                     A = 30.05(0.20+0.65+0.80 \times 2+0.15 \times 2) \times 2
   Vertical
                       + 0.65\times1.60\times2\times8-(0.65\times0.60\times3+0.65\times0.35\times2)\times2
                                                                                                                 178,665 m
3) Support
                     V = \{(0.65+0.80) \times 1.10+0.80 \times 1.60 \times 2\} \times 30.05
                         +6.05\times7.20\times(30.10-0.80\times2-0.50)
                                                                                                              = 1344.538 m^3
                     V = (7.20+1.00) \times 30.05
4) Scaffold
                                                                                                                 246.410m<sup>3</sup>
5) Guard rail
                       = 2(30.05+10.20+11.20)
                                                                                                                 102.900m
6) Joint filler (t=25mm) ··· expansion joint : = 6.00×2
                                                                                                                   12.000m
                     A = \{(0.20+0.35) \times 1.10+0.80 \times 5.00\} \times 2
                                                                                                                    9. 210 m
                     V = (0.050+0.095) \times 3.00 \times 30.05
7) Asphalt
                                                                                                                   13.071m<sup>3</sup>
                                                                                                                (180.300 m²)
                   X Section of Superstructure.
                     A = 2 \times 0.15 \times 0.60 + (0.20 + 0.35) \times 1.10 + (1.00 \times 5.00 - 0.80 \times 1.60 \times 2)
                                                                                                                   3. 225 m
```

#### NO (D VEHICLE BRIDGE Sub structure P₁: Pier 1) Concrete. beam $V = \% 2.991 \times 0.60$ 1.794m³ Pillar $V = \times 2.241 \times 6.10$ 13.670m³ $V = 4.50 \times 6.00 \times 0.80$ footing 21.600m3 total $37.064m^3$ Of. UF₂: $A = (4.50 \times 6.00 - \%2.241) + \%2991$ 27.750 m3 2) form work. horizontal $A = \frac{1}{2}.991 = 2.241$ Vertical $A = \frac{1}{2}.828 \times 0.60 = 9.828 \times 6.10$ 0.750 m² 67.647 m² " (footing) $A = (4.50+6.00) \times 0.80 \times 2$ 9.600 m² 3) base concrete $V = 4.70 \times 6.20 \times 0.10$ $2.914m^3$ " formwor $A = (4.70+6.20) \times 0.10 \times 2$ 2.180 m 4) Support $V = (2.991-2.241) \times (6.10-0.10)$ 4.500m³ $V = 1.20 \{(0.50+2.00)+(6.00+2.00)\} \times 6.70 \times 2$ 5) Scaffold 168.840m³

#### * Where, quantity of unit length

Piller {	A = $4.50 \times 0.50 - (0.10^2 \times 4 - \pi \times 0.10^2)$ Q R = $\pi \times 0.10 \times 2$ Q S = $(4.50 + 0.50) \times 2 - 8 \times 0.10$	= 'X' 2.241 m ² = 0.628 m ² = 9.200 m ²
	Q=	= • <b>X</b> 9.828 m²
Beam (	$A = 6.00 \times 0.50 - (0.10^{2} \times 4 - \pi \times 0.10^{2})$	= 💥 2.991 m²
$\prec$	$Q R = \pi \times 0.10 \times 2$	= 0.628m
L	$Q S = (6.00+0.50) \times 2-8 \times 0.10$	= 12.200m
	L-	= × 12.828m



6) excavation Hard	$V = 1.65/6 \{4.70 \times 6.20 + 5.69 \times 7.19 + (4.70 + 5.690)(6.20 + 7.190)\}$	= 57.522m ³
remain	$V = 2.914+21.600+ \times 2.241 \times 0.750$	$= 26.194 \text{m}^3$
back fill	V = 57.522-26.194	$= 31.328 m^3$

```
NOO VEHICLE BRIDGE
   Sub structure A_1 = A_2
1)Concrete
       V = 0.15 \times 0.60 \times 0.30 \times 2+7.20 (1.10 \times 0.30+1.10 \times 7.60+5.00 \times 0.80)
                                                                                                          91.422m3
      Or UF<sub>2</sub>: A = 7.20 \{ (5.00-1.10) + 0.30 + 0.80 \}
                                                                                                          36,000 m²
2) formwork.
   Vertical A = (0.30+0.60) \times 0.15 \times 2 \times 2 + (0.30+7.20) \times 1.10 \times 2
           + (1.10+7.20) \times 7.60 \times 2
                                                                                                         143. 200 m²
     " (footing)
                            A = (5.00+7.20) \times 0.80 \times 2
                                                                                                          19.520 m
3)Scaffold
                            V = 1.20 \{(1.10+2.00)+(7.20+2.00)\} \times 8.70 \times 2
                                                                                                         256.824m<sup>3</sup>
1) base concrete
                            V = 5.20 \times 7.40 \times 0.10
                                                                                                           3.848m<sup>3</sup>
                           A = (5.20+7.40) \times 0.10 \times 2
                                                                                                           2.520 m<sup>2</sup>
                               7.180 \times 9.380
                                                                      weatherd
                                      5,200 x 7,400
5) excavation
       Hard
                   V = 3.30/6 \{5.20 \times 7.40 + 7.18 \times 9.38 + (5.20 + 7.18)(7.40 + 9.38)\}
                   V = 3.848+7.20 \times (5.0 \times 0.80+1.10 \times 2.40)
                                                                                                          51,656m<sup>3</sup>
   remain
```

120.804m³

back fill

V = 172.460-51.656

WING: A1

concrete

 $V = \{1/2(8.851+8.858)$ 

 $\times 2.40+1/2(8.858-2.896+0.628) \times 7.50$ 

 $\times 0.70 \times 2$ 

 $= \{45.963\} \times 0.70 \times 2$ 

 $= 64.348 \text{ m}^3$ 

 $UF_2 : A = 9.90 \times 0.70 \times 2$ 

 $= 13.860 \,\mathrm{m}^2$ 

form work. Vertical

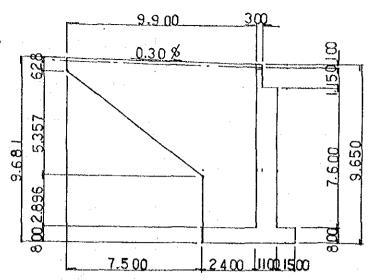
 $A = 2 \times 0.70(0.628+2.896)$ 

 $+ \%45.963 \times 2 \times 2$ 

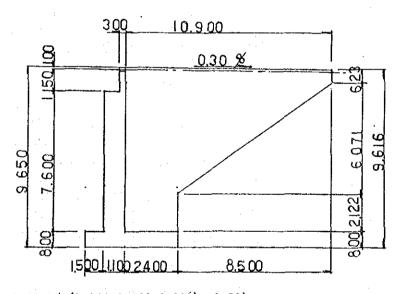
 $= 188,785 \,\mathrm{m}^2$ 

Sloping  
A = 
$$\sqrt{5.357^2+7.50^2} \times 0.70 \times 2$$

 $= 12.903 \text{m}^3$ 



WING: A2



concrete

 $V = \{1/2(8.849+8.841) \times 2.40+1/2(8.841-2.122+0.623) \times 8.50\}$ 

 $\times$  0. 70  $\times$  2

 $= \{52, 431\} \times 0.70 \times 2$ 

73.403m³

 $UF_2$ : A = 10.90 × 0.70 × 2

15. 260 m²

formwork.

Vertical

 $A = 2 \times 0.70 (0.623+2.122) + \%52.431 \times 2 \times 2$ 

213.567 m²

Sloping

 $A = \sqrt{8.50^2 + 6.071^2} \times 0.70 \times 2$ 

14,623"

LIST OF REINFORCED BAR --- NO O VEHICLE -SUPERSTRUCTURE

	<del></del>		<del></del>	<del></del>	T	NOW ACI		
	NARK	DIAMETER	LENGTH	NUMBAR	UNITAEIGHT	PIECEWEIGHT	MEICHT	REMARX
		· 		r	,			
	2 1	Y 1 6	7120	140	1.579	11.24	1574	
-	2	Y12	4850	140	0.888	4.31	603	
	3	Y16	1720	280	1.579	2.72	762	
	4-1	Y12	8000	132	0.888	7,10	937	
	6-2		3790	88		3.37	297	
	5	Y16	1660	16	1.579	2.62	42	
		· · · · · · · · · · · · · · · · · · ·		<del></del>			4215	8
	K 1	Y12	1120				<u>, , , , , , , , , , , , , , , , , , , </u>	·
			1120	140	0.888	0.99	139	
	2-1		8000	12		7.10	85	:
	2-2	<u> </u>	4070	8		3.61	29	
							253 ]	(g
	B 1-1	Y 3 2	10000	12	6 212	62 12	250	
	1-2		6000	24	6.313	63.13	758	<del></del>
	1-3	"	7030	24		37.88	909	
* .	2		8000	12	<i>"</i>	44.38	1065	<del></del>
	3-1	"	8000		6.313	50.50	606	<u> </u>
-	3-1	<u>"</u>		36	"	50.50	1818	
			5670	24	<i>"</i>	35.79	859	
	4	Y 25	10000	24	3.854	38.54	925	
	5-1	100	7000	36	0.888	6.22	224	
į	5-2	"	5290	24	"	4.70	113	
ļ	B0 1-1	Y16	2730	162	1.579	4.31	698	
	1-2		960	162	"	1.52	246	
	2-1	Y12	2600	126	0.888	2.31	291	
	2-2	"	840	126		0.75	. 95	
	3		690	135	"	0.61	8 2	
					<u>-</u>	•	8689	kg
		Y 2 5	5 6 0 0	12	3.854	21.58	259	
	2		6160	12		23.74	285	
	3	Y12	1850	12	0.888	4.31	52	
	4	Y 25	5600	6	3.854	21.58	129	
	5	. "	6160	6	"	23.74	1 ( 2	<u> </u>
	6	Y 1 2	4850	8	0.888	4.31	34	
	CO 1-1	SILA	2220	36	0.888	1, 97	71	<u> </u>
	1-2	"	840	36		0.75	27_	<b> </b> -
	2-1		1840	24	"	1.63	39	<b> </b>
	2-2		600	24	"	0.53	13	L
						····	1051	x g
.* + .								
				<del> </del>				
		···	Y 3 2			6015 kg		
			Y 2 5			1740	·	
	<u> </u>	·	<u> </u>			3322		
		<del></del>	Y 1 2	<u> </u>		3131		
		: · · · · ·				14208 kg.	:	

÷ . . .

LIST OF REINFORCED BAR---NO () VEHICLE - A1

MARK	DIAMETER	LENGTH	NUMBAR	UNITWEIGHT	PIECEWEIGHT	MEICHL	REMAR
P 1	Y12.	2000	88	0.888	1.78	157	
	"	660	43	11	0.59	25	<del> </del>
3	"	7060	8	н	6.27	50	   
4	<u> </u>	400	10	"	0.36	4	
						236	kg
A 1	Y32	5000	22	6.313	31.57	695	
2	"	6000	21		37.88	795	
3		4830	22	3.854	18.61	409	
4		3830	21	11	14.76	310	
5		8650	25	"	33.34	834	
6	Y16	8520	4	1.579	13.45	54	
7		1450	45	"	2.29	103	<del></del>
8		7060	. 5	11	11.15	56	
9		7060	25	3.854	27.21	680	
10		7060	25	1.579	11.15	279	
11	Y12	1210	60	0.888	1.07	64	
		<u> </u>				4279	kg
E 1	Y12	1140	19	0.888	1.01	19	
2		7060	2	"	6.27	13	<u> </u>
	· ·	.,,,,,	<u> </u>		0,27	32	և Մա
	······································				1.	02	<u> </u>
F 1	Y25	3750	45	3.854	14,45	650	
2	Y20	2680	25_	2.466		165	
3	Y25	3050	49	3.854	11.75	576	
4	Y20	4040	25	2.466	9.96	249	
5	Y16	7060	33	1.579	11.15	368	
6	11	7580	4	<b>u</b> ,	11.97	48	
7	1)	5390	4	11	8.51	34	
8	**	1150	8	11	1.82	15	
9	Y12	1950	48	0.888	1.73	83	
10000						2188	kg
							1
S 1	Y16	2880	40	1.579	4.55	182	
2	Y12	5880	11	0.888	5.22	57	<u> </u>
						239	kg
	· - · · · · · - · · - ·						
1		үз2		1490 1			
<u> </u>		Y25		3459 F		····	· · · · · · · · · · · · · · · · · · ·
		Y20		414 }			
				414 k 1139 k 472 k	(g		

LIST OF REINFORCED BAR---NO 1 VEHICLE -AI---WING

		T				<u> </u>		
AK.	RK.	DIAMETER	LENGTH	NUMBAR	UNITWEIGHT	PIECEWEIGHT	MEICHT	REMARK
:								
W	1	Y16	10630	2	1.579	16.78	34	
·	2	Y25	7580	20	3.854	29.21	584	
:	3	,,	3750	9	1)	14.45	130	
:	4	н .	3750	25	11	11	361	
<u>.</u>	5	Y20	7510	20	2.466	18.52	370	
: 	<u> </u>	,,	3670	9	11	9.05	81	
	7	,	9830	8	11	24.24	194	
·	8		5000	4	33	12.33	49	
	9	,,	3500	24		8.63	207	<u> </u>
<u> </u>	10	Y16	9770	8	1.579	15.43	123	
· <u>·</u>	11	,,	3440	24	31	5.43	130	
<u> </u>	12	- 11	10000	1	,,	15.79	16	··
<u> </u>	13	11	10000	1	11	11	16	
<u>:</u> <u>:</u>	14	. 11	950	34	n 	1.50	51	
<u> </u>	15	110	1050	32		1.66	53	·
· .	16	11	1050	30	ļ		50	
-	17	Y25	2700	4	3.854	10.41	42	
	18	11	3690	24	11	14.22	341	
\$ 4	<u>19</u>	11	2500	3_	i ,, 	9.64	29	
<u>.</u>	20	Y12	800	48	0.888	0.71	34	
<del></del>					<u>.</u>		2895	kg
} : <del></del>	: 		<u>}</u>	·				
		· · · · ·	Y25		1487	kg		
:	··		Y20		901	kg		<del></del>
			Y16	· · · · · · · · · · · · · · · · · · ·	473	kg		
		·	Y12	<u> </u>	34	kg		
· · · · · · · · · · · · · · · · · · ·		· · · · · · · · · · · · · · · · · · ·			2895	kg		

LIST OF REINFORCED BAR --- NO VEHICLE - A2---WING

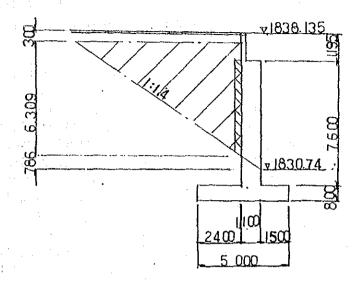
NA.	RK.	DIAMETER	LENGTH	NUMBAR	UNITWEIGHT	PIECEWEIGHT	WEIGHT	REMARK
	!				_			
W	1	Y16	11630	2	1.579	18.36	37	
	2_	Y32	7190	29	6.313	45.39	1316	
·	3	Y25	3750	25	3.854	14.45	361	
	4	1)	7130	29	,,	27.48	797	
	5	Y20	3850	27	2.466	9.49	256	•
	6	.,	9800	8	H	24.17	193	
	7	11	4500	4_		11.10	44	
	8	Y16	3790	27	1.579	5.98	161	·
	9		9740	8	11	15.38	123	
	10		12000	1	H	18.95	19	
	11	,,	12000	1	**	17	19	·
	12	Y32	2760	4_	6.313	17.42	70	
	13	11	3870	24	"	24.43	586	
	14		2460	3	"	15.53	47	
	15	Y16	1040	35	1.579	1.64	57	
	16	. 11	950	37	,,	1.50	56	
	17		1040	29	1)	1.64	48	
	18		800	59	0.888	0.71	42	
		<u> </u>					4232	kg
				· · · · · · · · · · · · · · · · · · ·				
	·		Y32		2019	kg		· · · · · · · · · · · · · · · · · · ·
<u> </u>			Y25		1158	kg		
	· 		Y20		493	kg		<del></del>
		· · · · · · · · · · · · · · · · · · ·	Y16		520	kg	<u> </u>	
			Y12		42	kg		
-			·		4232	kg <u> </u>		

LIST OF REINFORCED BAR --- NO OVEHICLE-PIER

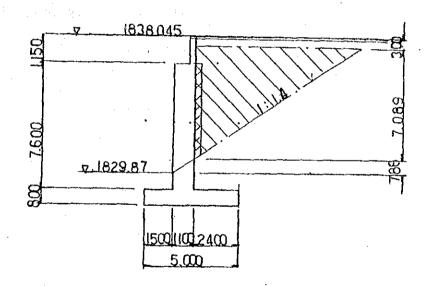
AK	RK _	DIAMETER	LENGTH	NUMBAR	UNITWEIGHT	PIECEWEIGHT	WEIGHT	REMARK
		· · · · · · · · · · · · · · · · · · ·						-
В	1	Y12	6780	6	0.888	6.02	36	
	2	-11	2000	4	н	1.78	7	
·.	3	0	6600	4	11	5.86	23	
	_4	tt .	1530	27	11	1.36	37	
· · ·	5		580	27	11	0.52	14	
<u>.</u>		2			·		117	kg
	1	Y32	7660	60	6.313	48.36	2902	
	2	Y16	5360	52	1.579	8.46	440	
,	3	11	1340	126	71	2.12	267	
······································			<u></u>				3609	kg
F	1.	Y25	5700	39	3.854	21.97	857	
	2	Y20	4940	21	2.460	12.18	256	
:	3	11	7200	15	11	17.76	266	
<del></del>	4]	Y12	6200	15	0.888	5.51	83	
•	5	Y16	6360	4	1.579	10.04	40	,
	6	11	4860	6	n ·	7.67	46	
	7		1980	50	11	3.13	156	
							1704	kg
			V0.0	· · · · · · · · · · · · · · · · · · ·	2002 1		<del> </del>	
<del></del>			Y32		2902 } 857 }			<u>*</u>
			Y25		522 1		·	
			Y20		949 1			
		:	Y16 Y12		200 1		<del></del>	
<del></del>	·		114		5430 1	<del></del>	:	··

# NO 0 VEHICLE -- ABUTMENT

# AI --- BACK FILL



## A2 --- BACKFILL



BACKFILL

A1: 
$$V = \frac{1}{2} \times 6.309^2 \times 1.5 \times 5.80 = 173.2^{\text{m}^3}$$
  
A2:  $V = \frac{1}{2} \times 7.089^2 \times 1.5 \times 5.80 = 218.6^{\text{m}^3}$ 

= 391.8

#### NO ② VEHICLE BRIDGE Super structure i) Concrete (Grade=30) Girder slab and parapet : $V = \frac{34.985 \times 28.050}{1}$ 139.829m³ cross girder $= 1.50 \times 0.65(0.60 \times 3+0.35 \times 2) \times 4$ 9.750m³ total 149.579m³ $UF_2$ : A = 28.05 × (10.00+0.60 × 2) 314.160 m 2) formwork $A = 28.05(\sqrt{0.15^2+1.10^2\times2+0.60\times5+1.50\times4})$ horizontal 314.721 m $A = 28.05 \times (0.20+0.65+0.80 \times 4+0.15 \times 2) \times 2$ Vertical + 0. $65 \times 1.50 \times 4 \times 8 - (0.65 \times 0.60 \times 3 + 0.65 \times 0.35 \times 2) \times 2$ 271.985 m 3)Support $V = \{(0.65+0.80) \times 1.10+0.80 \times 1.50 \times 4\} \times 28.05$ $+ 6.05 \times 11.20 \times (28.10 - 0.80 \times 2 - 0.50)$ 1941.139m³ 4) Scaffold $V = (11.20+1.00) \times 28.05$ 342.210m³ 5)Guard rail $Q = 2 \times 28.05$ 56.100 m 6) Joint filler (t=25mm) $\cdots$ expansion joint : $Q = 10.000 \times 2$ 20.000m $A = \{(0.20+0.35) \times 1.10+0.80 \times 9.00\} \times 2$ 15,610 m² 7) Asphalt $V = (0.05+0.125) \times 5.00 \times 28.05$ 24.543m3 (280, 500 m²)

#### * Section of Superstructure.

 $A = 2 \times 0.15 \times 0.60 + (0.20 + 0.35) \times 1.10 + (1.00 \times 9.00 - 0.80 \times 1.50 \times 4) = 4.985 m^{2}$ 

```
NO@ VEHICLE BRIDEG
   Sub structure A_1 = A_2
1) Concrete
                    V = 0.15 \times 0.60 \times 0.30 \times 2 + 11.20 (1.10 \times 0.30 + 1.10 \times 7.50 + 5.00 \times 0.90)
                                                                                                       = 146.550 \text{m}^3
     UF<sub>2</sub>: A = 11.20 \{ (5.00-1.10) + 0.30 + 0.80 \}
                                                                                                            56.000 m
2) formwork.
                    A = (0.30+0.60) \times 0.15 \times 2 \times 2 + (0.30+11.20) \times 1.10 \times 2
   Vertical
                      + (1.10+11.20) \times 7.50 \times 2
                                                                                                           210.340 m²
    " (footing) A = (5.00+11.20) \times 0.90 \times 2
                                                                                                            29.160 m
                             V = 1.20 \{(1.10+2.00)+(11.20+2.00)\} \times 8.60 \times 2
3) Scaffold
                                                                                                           336.432m3
4) base concrete
                          V = 5.20 \times 11.40 \times 0.10
                                                                                                             5.928m<sup>3</sup>
                          A = (5.20+11.40) \times 0.10 \times 2
                                                                                                             3. 320 m2
5) base(curusherran)
                          V = 5.20 \times 11.40 \times 0.20
                                                                                                            11.856 m
                                    8.700 \times 14.900
                                                                               earth
                                              5.200 x 11.400
6) excavation
        earth
                   V = 3.50/6 \{5.20 \times 11.40 + 8.70 \times 14.90 + (5.20 + 8.70) (11.40 + 14.90)\}
                                                                                                       = 323.446m^3
   remain
                   V = 5.928+11.856+11.20(5.0\times0.90+1.10\times2.30)
                                                                                                       = 96.520 \text{ m}^3
                   V = 323, 446-96, 520
   back fill
                                                                                                       = 226,926m^3
     Wing: A<sub>1</sub>
7) SPP400: $500×9×8.000 × 2010
```

where.

SPP: Pile of Steel Pipe

#### NOO VEHICLE BRIDGE Sub structure P₁: Pier 1) Concrete. beam $V = 34.991 \times 1.00$ 4.991m³ Pillar $V = 2.991 \times 5.60$ 16.749m³ footing $V = 4.50 \times 8.00 \times 0.90$ $32.420m^3$ 54.140m³ UF₂: $A = (4.50 \times 8.00 - \%2.991) + 4.991$ 38.000 m² 2) formwork. $A = \frac{X_1}{4}$ . 991-2. 991 horizontal 2.000 m² $A = ^{12}20.828 \times 1.00 + ^{12}2.828 \times 5.60$ Vertical : 92.664 m " (footing) $A = (4.50+8.00) \times 0.90 \times 2$ 22, 500 m2 3) base concrete $V = 4.70 \times 8.20 \times 0.10$ $3.854m^3$ formwork $A = (4.70+8, 20) \times 0.10 \times 2$ 2.580 m² 4) base(crusherran) $V = 4.70 \times 8.20 \times 0.20$ 7.708 m² 5) Support $V = (4.991-2.991) \times (5.60-0.10)$ 11.000m³ 6) Scaffold $V = 1.20 \{(0.50+2.00)+(10.00+2.00)\} \times 6.60 \times 2$ 229,680m³ dia 7) SPP400 : $\phi 500 \times 9 \times 8.500^{\text{mm}} \times 8^{\text{no}}$ * where quantity of unit lensth. $A = \{6.00 \times 0.50 - (0.10^{2} \times 4 - u \times 0.10^{2})\}$ = -X 2.991 m² $Q R = (u \times 0.10 \times 2)$ 0.628 m² $Q S = \{(6.00+0.50) \times 2-8 \times 0.10\}$ 12. 200 m² = 💥 12.828 m² $A = 10.00 \times 0.50 - (0.10^2 \times 4 - u \times 0.10^2)$ = X 4.991m $Q R = u \times 0.10 \times 2$ 0.628m Beam $Q S = (10.00+0.50) \times 2-8 \times 0.10$ 20.200m = X 20.828m earth 4,700 X 8,200 📆 8) excavation $V = 1.85/6 \{4.70 \times 8.20 + 6.55 \times 10.05 + (4.70 + 6.55) (8.20 + 10.05)\}$ 95. 484m³ Hard

 $V = 3.854+7.708+32.40+2.991\times0.650$ 

V = 95,484-45,906

remain

back fill

45, 906m³

49.578m³

LIST OF REINFORCED BAR --- NO VEHICLE-SUPERSTRUCTURE

			<del></del>		***************************************	HULL-SUP	~NOTINOU
MARK	DIAMETER	LENGTH	NUMBAR	<b>ТКОІЗЖТІКО</b>	PIECEWEIGHT	WEIGHT	REMARK
·	·	·. ·					
<u>s 1-1</u>	Y 16	6000	172	1.579	9.47	1629	
1-2	<i>p</i>	5680	172	"	8.97	1543	
2	Y12	8850	172	0.888	7.86	1352	
3	Y16	1720	344	1.579	2. 72	936	
4-1	Y12	10000	60	0.888	8.88	532	
4-2	"	9410	120	"	8.36	1003	
5	Y 1 6	1670	16	1.579	2.54	42	
		· · · · · · · · · · · · · · · · · · ·				. 7037 1	g
· · · · · · · · · · · · · · · · · · ·							
<u>K</u> 1	Y12	1120	172	0.888	0.99	170	
2-1	"	10000	4	,,	8.88	36	
2-2	<u>"</u>	9410	8		8.36	67	
				·		273 1	8
	٠.						
B 1-1	Y32	7000	60	6.313	44.19	2651	1
1-2	"	6530	- 40	,,	41.22	1649	
2	"	8000	20	"	50.50	1010	
3-1	"	7000	60	"	44.19	2651	
3-2	"	6170	40	,,,	38.95	1558	
4	Y 25	10000	40	3.854	38.54	1542	
5-1	Y12	10000	20	0.888	8.88	178	
5-2	,,	9370	40	"	8.32	333	
B0 1-1	Y 1 6	2730	280	1.579	4.31	1207	:
1-2	"	960	280	"	1.52	426	
2-1	Y12	2600	160	0.888	2.31	370	
2-2	"	840	160	"	0.75	120	
3	,,	690	225	,,	0.61	137	
					:	13832	kg
							<u> </u>
C 1	Y 25	6000	12	3.854	23.12	277	
2-1	,,	7500	12	,,	28.91	347	
2-2	,,	3540	12	"	13.64	164	
3	Y 1 2	4850	i 2	0.888	4. 31	5 2	
-	Y 25	9600	6	3.854	37.00	222	
5-1	"	7500	6	"	28.91	173	
5-2	"	3540	6	,, .	13.64	82	
- 6	Y12	8850	5	0.888	7.86	. 47	
00 1-1	Y12	2220	60	0.888	1.97	118	
1-2	,,	840	60	"	0.75	45	
2-1	,,	1840	40	,,	1.63	6.5	
2 - 2	"	600	40	"	0.53	21	
					·	1613	kg
		<del></del>	<u> </u>				
······································		Y 3 2			9519 kg		
		132 Y25			2807		
	······································	145 Y16			5783		
<u> </u>	·			<del> </del>	4646		
<del></del>	-	Y12			22755 kg	· · · · · · · · · · · · · · · · · · ·	•

LIST OF REINFORCED BAR -- NO VEHICLE - AI - A2

М	IARK	DIAMETER	LENGTH	NUMBAR	UNITWEIGHT	PIECEWEIGHT	WEIGHT	REMARK
-		TT						
P	1		2000	154	0.888	1.78	274	
~	2		660	77	"	0.59	45	
	3		11480	8	"	10.19	82	
	4	<u>''</u>	400	16	hr .	0.36	6	
<del></del>	<del></del>	<del>ing.</del>		<del>.</del>	· · · · · · · · · · · · · · · · · · ·		407	kg
Ā	1	Y32	5000	39	6,313	21 57	1003	
	2	1	6000	38	"	31.57	1231	
	3		4740	39	3.854	18.27	1439	
	4	.,	3740	38	"	14.41	713 548	
	5	.,	8560	39	v	32.99	1287	
	6	Y16	8430	6	1,579	13.31	80	- <del> </del>
	7	**	1450	77	"	2.29	176	
	8	"	11620	5	11	18.35	92	<del></del>
	9	Y25	11940	24	3.854	46.02	1104	<del> </del>
	10		11620	24	1.579	18.35	440	<del></del>
	11	Y12	1210	150	0.888	1.07	160	<del></del>
							7270	kg
· · ·								
E	1	Y12	1140	33	0.888	1.01	33	
	2		11480	2	u	10.19	20	
	. <u> </u>	<del></del>	. '	·		· .	53	kg
	<del></del>	· · · · · · · · · · · · · · · · · · ·	<del> </del>		<del></del>	· · · · · · · · · · · · · · · · · · ·	: 	
F	1	Y25	3750	71	3,854	14.45	1026	
	2	Y20	2680	39	2.466	6.61	258	
	3	Y25	3040	71	3.854	11.72	832	
· ·	4	A50	4060	39	2.466	10.01	390	
	5	Y16	11620	33	1.579	18.35	606	
	6	11	12140	4		19.17	77	! 
	7		5390	4	**	8.51	34	 
	8	"	1210	22	**	1.91	42	
	9		1990	90	11	3.14	283	
	<u> </u>	<u> </u>		<del></del> _			3548	kg
S		VIE	2880	67	1.579	4.55	305	
<u> </u>	2	Y16	9880	11	0.888	8.77	96	
		Y12	3000	11	0.0001		401	kg
	::							
			Y32		2670 l	s g		<del></del>
		<u></u>	Y25		5510 l			
· 		· · ·	Y20		648 l			<del></del>
	·		Y16	<u>.                                    </u>	2135 )			·
			Y12		716 1	g		
					11679 H	g		

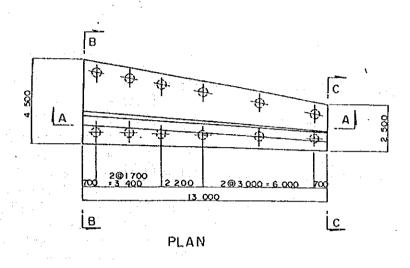
LIST OF REINFORCED BAR --- NO @ VEHICLE-PIER

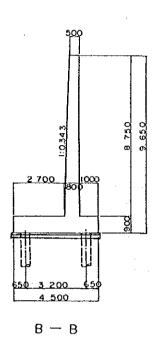
NA.	RK	DIAMETER	LENGTH	NUMBAR	UNITWEIGHT	DIROCK VI		
			SONG 11	TOMBAN	OULLWEIGHT	PIECEWEIGHT	WEIGHT	REMARK
·	<del></del>	· · · · · · · · · · · · · · · · · · ·		r				
3	1	Y32	11850	4	6.313	74.81	299	
	2	,,,	9850	44	11	62.18	249	
	3	Y25	12410	2'	3.854	47.83	96	
1	4	**	5000	4_	+)	19.27	77	
	5	<u>"</u>	11490	6_		44.28	266	
;	·	i ling it <del>The first to the</del>					987	kg
				r:				
	1_	Y32	7550	96	6.313	47.66	4575	
	2	Y16	6860	48	1.579	10.83	520	
	3	"	1430	160	)t	2.26	362	
				**	·		5457	kg
	:			· · · · · · · · · · · · · · · · · · ·	ı <del></del>			
7	1	Y25	5900	63	3.854	22.74	1433	
· · · · ·	2	Y20	4940	33	2.460	12.18	402	
	3	1,	9400	19	"	23.18	440	<del></del>
	4	Y12	8200	19	0.888	7.28	138	
	5	Y16	8360	4	1.579	13.20	53	· · · · · · · · · · · · · · · · · · ·
	6	,,	4860	6	11	7.67	46	
	7	١,	2130	82	11	3.36	276	
							2788	kg
				·				·
			Y32		5123	ξg		
		<u>i.</u>	Y25		1872 1	(g		
			Y20	, <del></del>	842 1	ζg		
			Y16	· ·	1257 1	κg		
-			Y12		138 ]	•		
	:				9232			

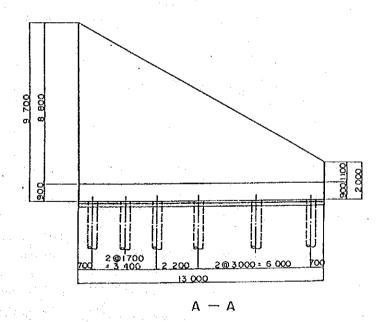
LIST OF REINFORCED BAR---NO VEHICLE - WALL

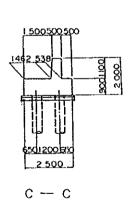
•						NO & VE		F WALL
	MARK	DIAMETER	LENGTH	NUMBAR	UNITWEIGHT	PIECEWEIGHT	#EIGHT	REMARK
							**************************************	<del></del>
	W 1	Y25	5980	53	3.854	22.05	1000	
· ·	2	11	4500	26	"	23.05	1222	
	3	Y20	5900	53	2.466	17.34	451	
	4		850	53	1.579	14.55	771	
	5		15350	2	0.888	1.34	71	-
·	6		13430	4	1.579	21.21	27 85	
	7	. 11	11950	7	"	18.87	132	
	8	n	5020	23		7.93	182	
	9	Y12	13290	4	0.888	11.80	47	
	10	11	11810	7	"	10.49	73	
	11	n	5020	23	11	4.46	103	
:	12	Y16	960	34	1.579	1.52	52	
	13	11	830	4	n,	1.31	5	
	14	Y12	760	113	0.888	0.67	76	44, 1
•							3297	kg
*	F 1	Y25	3310	53	3.854	12.76	676	
	2	Y20	1590	53	2.466	3.92	208	
	3	Y25	1970	53_	3.854	7.59	402	
	4	Y20	2920	53	2.466	7.20	382	
	5	Y25	2350	26	3.854	9.06	236	
	6	н	3670	26	++	14.14	368	
	7	Y16	13430	12	1.579	21.21	255	
	8	**	13440	1_	. 11	21.22	21	
	9	"	6420	1		10.14	10	·
4 P	10	ę,	13520	1	"	21.35	21	
有有	11	. 11	4910	4	11.	7.75	31	
	12	Y12	13290	10	0.888	11.80	118	
	13	11	13300	1	**	11.81	12	<del></del>
	14	. 71	6420	1	11	5.70	6	<u> </u>
. *	15	Ð	13380	1	11	11.88	12	
	16	11	4910	4	11	4.36	17	
	17	Y16	1130	22	1.579	1.78	39	
	18	Y12	1930	117	0.888	1.71	200	
	·			· · · · · · · · · · · · · · · · · · ·	<del></del>	<del></del>	3014	kg
			Y25		3355 J			<del></del>
		· · ·	Y20	<del></del>	1361		······································	
			Y16		904 1			
		·	Y12		691		· · · · · · · · · · · · · · · · · · ·	
					6311 }	(g		
	OLEMAN BEFORENS NAMES	, may may mana ay and made and the first of the			padencs MANY tory		and the state of t	2-74-14-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1

RETAINING WALL: W1 = W2 = W3 = W4







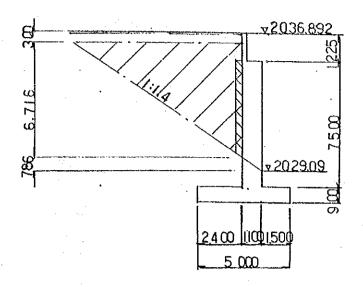


RETAINING WALL

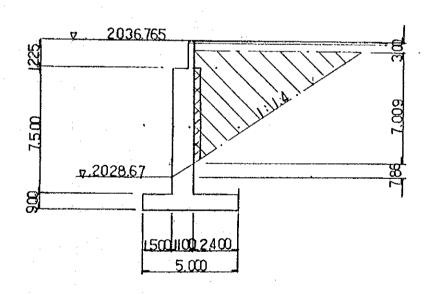
#### NO ② VEHICIE BRIDGE WALL: W₁ = W₂ = W₃ = W₄

V = $1/2$ {(0.50+0.80)/2×8.75+(0.50+0.538)/2×1.10} ×13.00 = $1/2$ {6.258} ×13.00	Œ	45. 373m ³
footing $V = 1/2(4.50+2.50) \times 0.90 \times 13.00$	=	45.675m ³
total =	<b>=</b>	91.048m ³
$UF_2$ : A = 1/2(4.50+2.50) - 1/2(0.80+0.538)} × 14.50+0.50×13.00	=	48. 300 m
2) formwork.		
$F_2$ (Y) $A = 1/2(8.75+1.10) \times 13.00$	=	71. 412 m
vertical $F_1$ (V) $A = (0.50+0538)/2 \times 1.10+(8.75+1.10) \times 13.00 \times 1/2$	=	71. 982 m
footing $F_1$ (V) $A = 2.50 \times 0.90 + 2 \times 0.90 \times 13.00$	=	28. 350 ni
total =	. =	100. 332 ni
V = 1.20(8.750+1.10) $\times$ 13.00	=	171.390m ³
) joint filler $A = 1/2(0.50+0.80) \times 8.75$	=	5. 687 ni
) water stop $Q = (8.75-0.10)$	=	8.650m
) base concrete $V = 1/2(4.70+2.70) \times 13.00 \times 0.10$	=	5.365m
$A = (4.70+2.70+13.00\times2)\times0.10$	=	3.640 n
7) base(crusherran) $V = 1/2(4.70+2.70) \times 13.00 \times 0.20$	=	10.730m ²
7.200 ×.16.700		
	•	
200 / 2300		
a y earth m		
2000 (12000 (17)		
3700 × 13200 " 1		
9) excavation		019 000
earth $V = 3.50/6 \{3.70 \times 13.20 + 7.20 \times 16.70 + (3.70 + 7.20) (13.20 + 16.70)\}$	=	317.356m
remain $V = 5.365+10.730+45.675+0.585 \times 2.30 \times 13.00$	-	81,280m 236,076m
backfill V= 317.356-81.280	=	230.U10M
9) SPP400 $\phi_{500} \times 9 \times 800^{m} \times 12^{n}$		

# NO. 2 VEHICLE AI --- BACK-FILL



### A2 --- BACK-FILL



___BACK-FILL --- include retainin8wall.

A1: 
$$V = \frac{1}{2} \times 6.716^2 \times 1.5 (11.20 + 6.716 \times 1.5 \times \frac{1}{3} \times 2) = 606.1 \text{ m}^3$$
  
A2:  $V = \frac{1}{2} \times 7.009^2 \times 1.5 (11.20 + 7.009 \times 15 \times \frac{1}{3} \times 2) = 670.9 \text{ m}^3$ 

NO(1) Vehicle bridge			
porous drainage A	porous drainage A = $6.00 \times 5.90 \times 2 \times (0.300)$		$= 70.800 \text{ m}^2 21.240 \text{m}^3$
perforated pipe L	pipe $L = 6.00 \times 2$		= 12.000 m
drain pipe( $\phi$ 75mm)N	$\phi 75 \text{mm}$ ) N = $(6.00 / 5.00 + 1) \times 2$		N 7 =
NO② Vehicle bridge			
porous drainage A	porous drainage A = $(10.00 \times 5.90 + \frac{1}{2} \times 5.9 \times 8.80 \times 2) \times 2(\times 0.300)$	$\langle 2 \rangle \times 2 (\times 0.300)$	$= 221.840 \text{ m}^3 66.552 \text{m}^3$
perforated pipe L	pipe $L = (10.00 + 8.80 \times 2) \times 2$	14.	= 55.200 m
drain pipe( $\phi$ 75mm)N	$\langle \sigma 75 \text{ mm} \rangle N = \{(10.00 / 5.00 + 1) + (8.80 / 5.00 +) \times 2\} \times 2$	-)×2} ×2	= 18 N

B. Q 21.01 Waterproofing materials

NO⊖ Vel	NO© Vehičle bridge	
super	structure: A = $30.050 \times 6.00 + 6.00 \times 3.00 \times 2 \times 2$	= 252.300 m
qns	" A1 : A = $(7.20-2\times0.70)\times8.70$	4
	$+(8.851+8.858)\times 2.40+(0.628+8.858-2.896)\times 7.50$	= 142.386 ~
	$A2 : A = (7.20 - 2 \times 0.70) \times 8.70$	
	$+(8.849+8.841)\times 2.40+(8.841-2.122+0.623)\times 8.50$	= 155.323 "
	total	= 550.009 m²
NO@ Vehi	shicle bridge	
super	super structure: $A = 28.050 \times 10.00 + 10.00 \times 3.00 \times 2 \times 2$	$= 400.500 \text{ m}^{2}$
qns	" $A1=A2$ : $A = 11.20 \times 8.60 \times 2$	= 192.640 "
	#ail: A = $\frac{1}{2}$ (8.75+1.10)×14.50×4	= 285.850 ~
	total	= 878.790 m²

```
Vehicle ① bridge
   approch slab
     concrete:
                       V = 3.00 \times 0.20 \times 6.00 \times 2
                                                                                                      = 7.200 \text{ m}^3
     UF2
                        \Lambda = 3.00 \times 6.00 \times 2
                                                                                                      = 36.000 m<sup>2</sup>
     joint filler \Lambda = 0.20 \times 6.00 \times 2
                                                                                                      = 3.400 \text{ m}^2
           (t=20)
   Sub, structure
     concrete:
                        \forall = 0.30 \times 0.30 \times 6.00 \times 2
                                                                                                     = 1.080 \text{ m}^3
     form work
                        \Lambda = (0.30 + 0.30) \times 6.00 \times 2
                                                                                                     = 7.200 \text{ m}^2
Vehicle ② bridge
  approach slab
     concrete: V = 3.00 \times 0.20 \times 10.00 \times 2
                                                                                                      = 12.000 \text{ m}^3
                        A = 3.00 \times 10.00 \times 2
                                                                                                      = 60.000 m
     joint filler \Lambda = 0.20 \times 10.00 \times 2
                                                                                                      = 4.000 m²
      (t=20)
  Sub, structure
   concrete:
                        V = 0.30 \times 0.30 \times 10.00 \times 2
                                                                                                      = 1.800 \text{ m}^3
     form work
                        A = (0.30 + 0.30) \times 10.00 \times 2
                                                                                                      = 12.000 m²
```

MORTAR OF SHOE --- THE CALCULATION BE ABRIDGE TO A LITTLE.

